

THE APPROVED CONSTRUCTION PLANS, DOCUMENTS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

Re: J1097074 Jeff Strobl

FULL SIZED LEDGIBLE COLOR PLANS ARE REQUIRED TO BE PROVIDED BY THE PERMITEE ON SITE FOR INSPECTION Tri-State Engineering, Inc. 12810 NE 178th Street Suite 218 Woodinville, WA 98072 425.481.6601

1922 5TH AVE SW DETACH GARAGE/LOFT

The truss drawing(s) referenced below have been prepared by Tri-State Engineering under my direct supervision based on the parameters provided by The Truss Company (Sumner).

Pages or sheets covered by this seal:	I14054061	thruI14054064
My license renewal date for the state	of Washington is	August 20, 2022.



November 8,2021

Terry Powell

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1.





L			24-0-0					
1			24-0-0					I
LOADING (psf) TCLL 25.0 (Roof Snow=25.0) TCDL TCDL 8.0 BCLL 0.0 * BCDU 7.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.14 BC 0.07 WB 0.04 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.00 19 0.00 19 0.00 19 0.00 18	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 104 lb	GRIP 185/148 FT = 20%
LUMBER-	1		BRACING-					

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 HF No.2 BOT CHORD 2x4 HF No.2 OTHERS 2x4 DF Stud

REACTIONS. All bearings 24-0-0.

(lb) - Max Horz 2=44(LC 12) Max Uplift All uplift 100 lb or less at joint(s) 2, 18, 29, 30, 31, 32, 33, 34, 35, 27, 25, 24, 23, 22, 21, 20 Max Grav All reactions 250 lb or less at joint(s) 2, 18, 28, 29, 30, 31, 32, 33, 34, 35, 27, 25, 24, 23, 22, 21.20

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(13)

- 1) Wind: ASCE 7-16; Vult=110mph (3-second gust) Vasd=87mph; TCDL=4.8psf; BCDL=4.2psf; h=12ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 9-0-0, Corner(3R) 9-0-0 to 15-0-0, Exterior(2N) 15-0-0 to 22-6-0, Corner(3E) 22-6-0 to 25-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 23.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 29, 30, 31, 32, 33, 34, 35, 27, 25, 24, 23, 22, 21, 20.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) All dimensions given in feet-inches-sixteenths (FFIISS) format.



City of Puyallup

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



November 8,2021



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. esign valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown Is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the rector. Additional permanent bracing of the overall structure is the responsibility of the isolation, quality control, storage, delivery, erection and bracing, consult **ANS/ITPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



		8-0-3		7-11-10			8-0-3					
Plate Offsets (X,Y) [2:0)-3-14,0-1-8], [4:0-3-0,Ed	lge], [6:0-3-14,0	-1-8]		7 11 10					000	
LOADING (ps TCLL (Roof Snow=2 TCDL BCLL BCDL	sf) 25.0 5.0) 8.0 0.0 * 7.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC 0. BC 0. WB 0. Matrix-M	57 65 27 ISH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.16 -0.28 0.07	(loc) 10-16 8-10 6	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 83 lb	GRIP 185/148 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 HF No 2x4 HF No 2x4 DF Sto	0.2 0.2 ud				BRACING- TOP CHORI BOT CHORI	D 8 D F	Structura Rigid cei	al wood s ling direc	heathing dire tly applied or	ctly applied or 3-1-13 10-0-0 oc bracing.	oc purlins.
REACTIONS.	(size) Max Horz Max Uplift Max Grav	6=0-5-8, 2=0-5-8 2=-44(LC 13) 6=-64(LC 9), 2=-64(LC 6=1092(LC 20), 2=1092	8) 2(LC 19)									
FORCES. (III: TOP CHORD BOT CHORD WEBS	o) - Max. Con 2-3=-240 2-10=-98 3-10=-56	mp./Max. Ten All force:)2/161, 3-4=-2119/153, 4 3/2246, 8-10=-46/1423, 6 ;5/95, 4-10=-19/822, 4-8=	s 250 (lb) or less -5=-2119/153, 5 -8=-98/2246 =-19/822, 5-8=-5	s except when 5-6=-2402/161 565/95	n shown.							
NOTES- (9 1) Wind: ASCE MWFRS (et Interior(1) 1 exposed;C- 2) TCLL: ASC 3) Unbalanced 4) This truss h non-concurr 5) This truss h) E 7-16; Vult= nvelope) gab 5-0-0 to 22 C for membo E 7-16; Pf=2 d snow loads as been des rent with oth as been des	=110mph (3-second gust) ble end zone and C-C Ex 6-0, Exterior(2E) 22-6-0 t ers and forces & MWFRS 5.0 psf (Lum DOL=1.15 have been considered f igned for greater of min er live loads.) Vasd=87mph; terior(2E) -1-6-C to 25-6-0 zone; S for reactions s Plate DOL=1.15 or this design. roof live load of	TCDL=4.8psf;) to 1-6-0, Inte cantilever left : hown; Lumber 5); Is=1.0; Rou 16.0 psf or 1.0	; BCDL=4.: prior(1) 1-6- and right e r DOL=1.6- ugh Cat B; 00 times flater rent with au	2psf; h=12ft; Cat 0 to 9-0-0, Exter xposed ; end ve 0 plate grip DOL Partially Exp.; C at roof load of 25	. II; Exp rior(2R) rtical lef =1.60 e=1.0; (.0 psf o	o B; Enc 9-0-0 to ft and rig Cs=1.00 on overh	losed; o 15-0-0, ght ; Ct=1.10 angs		City Development ISSU Building Engineering	of Puyallup A Permitting Services ED PERMIT Planning Public Works Traffic

6) * This truss has been designed for a live load of 23.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.
This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) All dimensions given in feet-inches-sixteenths (FFIISS) format.



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Scale = 1:49.7



 			28-0-0					
LOADING (psf) TCLL 25.0 (Roof Snow=25.0) TCDL TCDL 8.0 BCLL 0.0 BCDL 7.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.14 BC 0.07 WB 0.05 Matrix-SH	DEFL. Vert(LL) 0 Vert(CT) 0 Horz(CT) 0	in (loc) .00 21 .00 21 .00 20	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 130 lb	GRIP 185/148 FT = 20%
LUMBER-			BRACING-					

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 HF No.2 BOT CHORD 2x4 HF No.2 OTHERS 2x4 DF Stud

REACTIONS. All bearings 28-0-0.

(lb) - Max Horz 2=51(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 32, 33, 34, 35, 36, 37, 38, 39, 30, 28, 27, 26, 25, 24, 23 22

All reactions 250 lb or less at joint(s) 2, 20, 31, 32, 33, 34, 35, 36, 37, 38, 39, 30, 28, 27, 26, Max Grav 25, 24, 23, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(13)

1) Wind: ASCE 7-16; Vult=110mph (3-second gust) Vasd=87mph; TCDL=4.8psf; BCDL=4.2psf; h=12ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 11-0-0, Corner(3R) 11-0-0 to 17-0-0, Exterior(2N) 17-0-0 to 26-6-0, Corner(3E) 26-6-0 to 29-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

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