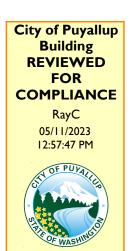
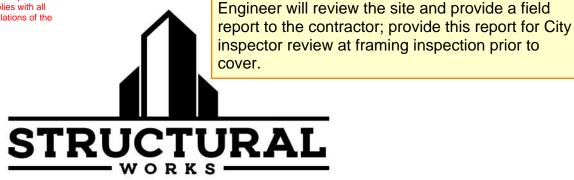
#### PRCTI20230325

Approval of submitted plans is not an approval of omissions or oversight by this office or noncompliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable building codes and regulations of the local government.

THE APPROVED CONSTRUCTION PLANS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.





#### **Submittal Documents**

## Library Book Locker Anchorage

Project

## Puyallup Library

Location

Puyallup, WA

Contractor

## **Lamb Contracting**

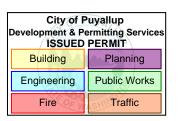
Project #: 2329

3/7/23

Prepared by: Structural Works, PLLC 1412 Beach Drive NE, Tacoma, WA

REV0







## **Table of Contents**

Description	<u>Pages</u>
BASIS FOR DESIGN	3 - 4
DETAILS	5 - 6
CALCULATIONS	7 - 13
APPENDIX	14 - 19





### **Basis for Design**

#### **BUILDING CODE:**

2018 EDITION OF THE INTERNATIONAL BUILDING CODE SUPPLEMENTED BY THE ASCE 7-16

#### **MATERIAL SPECIFICATIONS:**

PLATE, ANGLE, MISC. STEEL SHAPES: ASTM A36 (Fy = 36,000 PSI)

**BOLTS: ASTM A325** 

WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS HAVING CURRENT WELDING CERTIFICATES. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH THE APPPLICABLE PORTION OF THE CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION OF THE AWS. WELDING SHALL BE PERFORMED USING A SHIELDED ARC PROCESS USING APPROVED ELECTRODES CONFORMING TO AWS SPECIFICATION E70XX (LOW HYDROGEN).

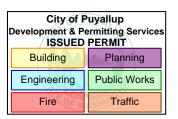
28-DAY COMPRESSIVE STRENGTH OF CONCRETE ASSUMED TO BE 3,000 PSI

COLD FORMED STEEL SHALL BE GRADE 50 FOR 16 GA OR HEAVIER AND GRADE 33 FOR 18 GA AND LIGHTER.

SHEET METAL SCREWS SHALL CONFORM TO ICC REPORT ESR-1408

#### **BOLT/STRUT NUT TORQUE (IF NOT SUPPLIED BY THE MANUFACTURER):**

3/8" DIA: 19 FT-LBS 1/2" DIA: 50 FT-LBS 5/8" DIA: 100 FT-LBS



### Basis for Design (cont.)

#### SCOPE OF WORK:

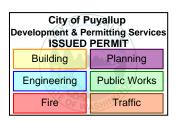
THE SUPPORTING STRUCTURE IS BEYOND THE SCOPE OF THIS SUBMITTAL. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO SUBMIT THESE CALCULATIONS AND ASSOCIATED DOCUMENTS TO THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION TO ANALYZE THE ABILITY OF THE SUPPORTING STRUCTURE TO ACCOMMODATE THE REACTIONS FROM THE CONNECTIONS SPECIFIED IN THIS SUBMITTAL. EQUIPMENT DIMENSIONS USED IN CALCULATIONS ARE BASED ON EQUIPMENT DATA SHEETS ATTACHED. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS. THIS SET OF CALCULATIONS IS BASED ON THE LOADS AND ASSUMPTIONS STATED WITHIN THIS SUBMITTAL. CONTRACTOR PROCEEDS AT THEIR OWN FABRICATION/INSTALLATION RISK PRIOR TO FINAL APPROVED SUBMITTAL. IF THE LOADS AND ASSUMPTIONS ARE NOT CORRECT THIS SUBMITTAL SHALL BE REVISED. FOR ANY SPECIAL INSPECTIONS REQUIRED REFER TO ISAT DRAWINGS/DETAILS AND BASIS FOR DESIGN FOR APPLICABLE ESR REPORT(S).

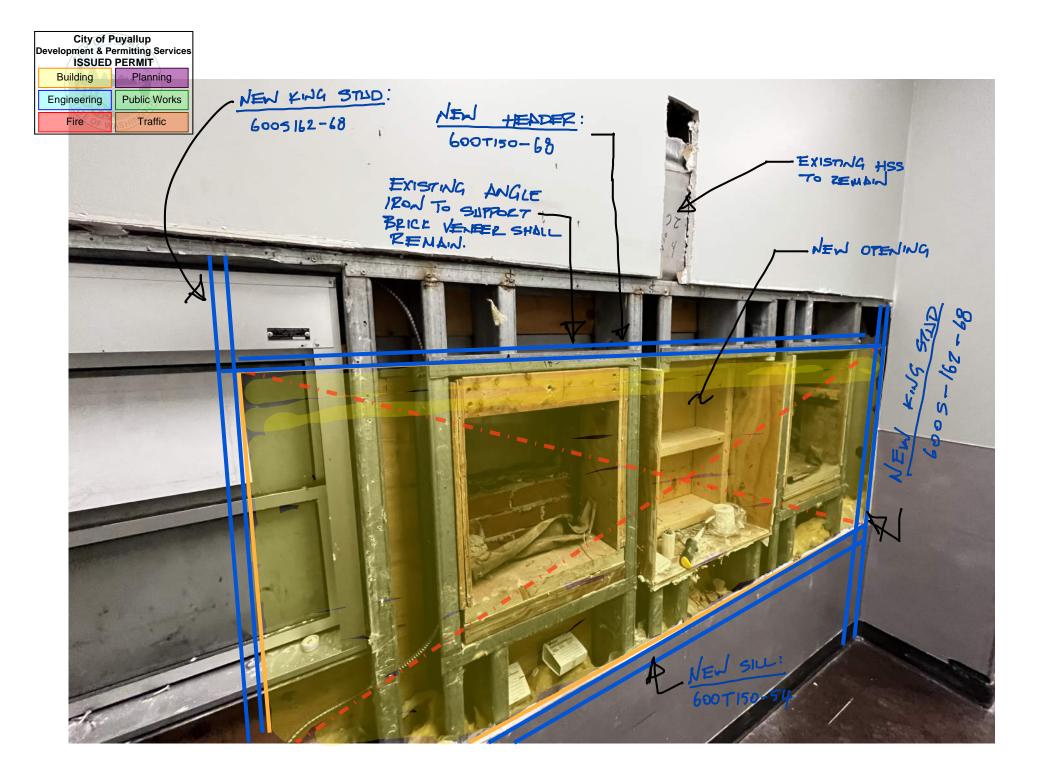
ALL STRUCTURAL STEEL AND ANCHORS EXPOSED TO WEATHER, MOIST CONDITIONS OR CHEMICAL ATTACK SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL OR TREATED FOR CORROSION RESISTANCE PER PROJECT SPECIFICATIONS. FASTENER HOLES SHALL BE MAXIMUM 1/16" DIA. LARGER THAN BOLT DIAMETER. DOES NOT APPLY TO VERTICAL ONLY SUPPORTS, USE WASHERS AS NECESSARY FOR OVERSIZED HOLES. IF HOLES ARE OVERSIZED, THE FASTENERS OR ANCHORS CAN BE MODIFIED BY WELDING A 1/4" THICK 1 5/8" SQUARE WASHER TO THE MOUNTING HOLE WITH A 3/16" FILLET WELD APPLIED TO A MINIMUM OF (2) SIDES OF THE WASHER, BY FILLING VOID WITH EPOXY OR JB WELD PART NUMBER 8265S PRIOR TO PLACEMENT OF WASHER OR BY USE OF NEOPRENE GROMMETS. WHERE EQUIPMENT IS ANCHORED TO A HOUSE KEEPING PAD, ATTACHMENT OF PAD TO SLAB TO BE ADDRESSED BY OTHERS. WHERE ANCHORS ARE INSTALLED IN HKP TOTAL CONCRETE THICKNESS INCLUDES EXISTING SLAB THICKNESS.





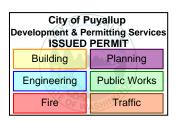
## **Details**







# **Calculations**



ATC Hazards by Location 3/7/23, 11:39 AM

A This is a beta release of the new ATC Hazards by Location website. Please contact us with feedback.

1 The ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

# ATC Hazards by Location

#### **Search Information**

Address: 324 S Meridian, Puyallup, WA 98371, USA

Coordinates: 47.1895629, -122.2954369

**Elevation:** 47 ft

2023-03-07T19:39:33.202Z Timestamp:

**Hazard Type:** Seismic

Reference

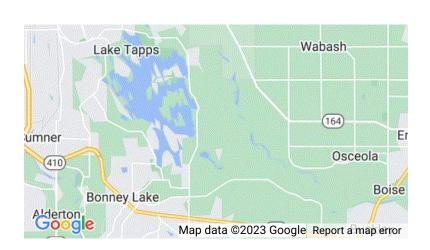
**Document:** 

Site Class:

ASCE7-16

Ш **Risk Category:** 

D-default



#### **Basic Parameters**

Name	Value	Description
S <sub>S</sub>	1.272	MCE <sub>R</sub> ground motion (period=0.2s)
S <sub>1</sub>	0.438	MCE <sub>R</sub> ground motion (period=1.0s)
S <sub>MS</sub>	1.526	Site-modified spectral acceleration value
S <sub>M1</sub>	* null	Site-modified spectral acceleration value
S <sub>DS</sub>	1.018	Numeric seismic design value at 0.2s SA
S <sub>D1</sub>	* null	Numeric seismic design value at 1.0s SA

<sup>\*</sup> See Section 11.4.8

#### **▼**Additional Information

Name	Value	Description
SDC	* null	Seismic design category
Fa	1.2	Site amplification factor at 0.2s



ATC Hazards by Location 3/7/23, 11:39 AM

F <sub>v</sub>	* null	Site amplification factor at 1.0s
CR <sub>S</sub>	0.914	Coefficient of risk (0.2s)
CR <sub>1</sub>	0.898	Coefficient of risk (1.0s)
PGA	0.5	MCE <sub>G</sub> peak ground acceleration
F <sub>PGA</sub>	1.2	Site amplification factor at PGA
PGA <sub>M</sub>	0.6	Site modified peak ground acceleration
TL	6	Long-period transition period (s)
SsRT	1.272	Probabilistic risk-targeted ground motion (0.2s)
SsUH	1.392	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
SsD	1.5	Factored deterministic acceleration value (0.2s)
S1RT	0.438	Probabilistic risk-targeted ground motion (1.0s)
S1UH	0.487	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
S1D	0.6	Factored deterministic acceleration value (1.0s)
PGAd	0.5	Factored deterministic acceleration value (PGA)

City of Puyallup Development & Permitting Services ISSUED PERMIT					
Building	Planning				
Engineering	Public Works				
Fire OF W	Traffic				

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Please note that the ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

#### Disclaimer

Hazard loads are provided by the U.S. Geological Survey Seismic Design Web Services.

While the information presented on this website is believed to be correct, ATC and its sponsors and contributors assume no responsibility or liability for its accuracy. The material presented in the report should not be used or relied upon for any specific application without competent examination and verification of its accuracy, suitability and applicability by engineers or other licensed professionals. ATC does not intend that the use of this information replace the sound judgment of such competent professionals, having experience and knowledge in the field of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the results of the report provided by this website. Users of the information from this website assume all liability arising from such use. Use of the output of this website does not imply approval by the governing building code bodies responsible for building code approval and interpretation for the building site described by latitude/longitude location in the report.

<sup>\*</sup> See Section 11.4.8

#### ASCE 7-16 (IBC 2018) WIND: BUILDING DATA:

Basic Wind Speed = 115 MPH WIND CHECK

Exposure B

Building Roof Height H = 30 ft
Component Shape = Square
Component Height h = 18 ft
Component Width W = 8 ft
Component Depth D= 8 ft

#### **Design Wind Load on Other Components**

 $F = q_z G C_f A_f$ 

 $q_z = .00256 K_z K_{zt} K_d V^2$ 

Ht. z at the centroid of area Af = 4 ft

Exposure coefficient  $K_z = 0.7$ 

Topography factor  $K_{zt} = 1.00$  26.8.1

Directionality factor  $K_d = 0.85$ 

 $q_z = 20.14 \text{ psf}$ 

Gust Effect factor G = 0.85

h/D = 2.25

Force coeff  $C_f = 1.321$  Table 29.5-1

Design wind pressure,  $F/A_f = 22.62$  psf

x0.6 for ASD

Design wind pressure, F/Af = 13.57 psf 4

CALCIALATIONS

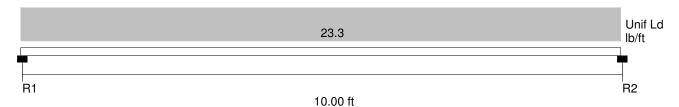
# Structural Works, Pllc Consulting Engineering

JOB	
SHEET NO.	OF
CALCULATED BY	DATE
PROJECT NO	

SEISMIC	
Fp = 0.4 x Sps x = p x (1+2=h) x wp	
$S_{DS} = 1.02$ $4p = 1.0$ $2p = 2.5$	
$2h = 0 \qquad \exists r = 1.0$	
$\overline{T}_{P} = \frac{0.4 \times 1.02 \times 1.0}{2.5 / 1.0} \times (1 + 2 \times 0) = 0.$	مر یہ کا
FPun = 0.3 x Sps x Ip r Wp	
4pm, _ 0.3 x 1.02 x 1.0 x wp = 0.31 wp	A GOVERNS
Fp = 400 x 0.31 = 124 1BS	
WIND GOVERNS.	
	City of Puyallup Development & Permitting Services ISSUED PERMIT Building Planning
	Engineering Public Works  Fire Traffic



Project: CFS Standards Header Type 2 Model:



Section: 600T150-54 Built-Up (X-X Axis)

**Fy** = 50.0 ksi **Va** = 2728.3 lb Maxo = 1519.8 Ft-Lb Moment of Inertia, I = 2.400 in^4

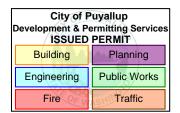
Loads have not been modified for strength checks Loads have not been modified for deflection calculations

#### **Flexural and Deflection Check**

	Mmax	Mmax/	Mpos	Bracing	Ma(Brc)	Mpos/	_	flection
<b>Span</b> Center Span	<b>Ft-Lb</b> 291.7	<b>Maxo</b> 0.192	<b>Ft-Lb</b> 291.7	<b>(in)</b> None	<b>Ft-Lb</b> 333.6	<b>Ma(Brc)</b> 0.874	<b>(in)</b> 0.074	<b>Ratio</b> L/1618
Combined Bend	ding and Wel	o Crippling						
Reaction		Load	Brng	Pa	Mmax		Intr.	Stiffen
Pt Load R1		<b>P(lb)</b> 116.7	<b>(in)</b> 1.00	<b>(lb)</b> 443.4	<b>(Ft-Lb</b> ) 0.0		<b>Value</b> 0.32	Req'd ? No
R2		116.7	1.00	443.4	0.0		0.32	No

#### **Combined Bending and Shear**

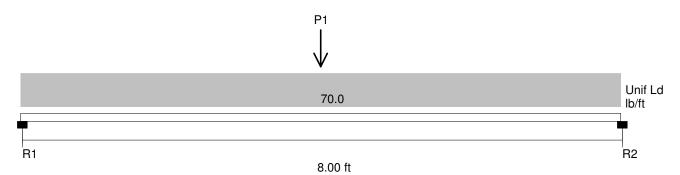
Reaction or	Vmax	Mmax	Va			Intr.	Intr.
Pt Load	(lb)	(Ft-Lb)	Factor	V/Va	M/Ma	Unstiffen	Stiffen
R1	116.7	0.0	1.00	0.04	0.00	0.00	NA
R2	116.7	0.0	1.00	0.04	0.00	0.00	NA





Project: CFS Standards

20PSF Lateral Load - 10ft Opening Model:



**Point Loads** P1 Load(lb) 400 X-Dist.(ft) 4.00

Section: 600S162-68 Single C Stud (X-X Axis)

**Fy** = 50.0 ksi Maxo = 3288.8 Ft-Lb Moment of Inertia, I = 3.525 in^4 Va = 5350.3 lb

Loads have not been modified for strength checks Loads have not been modified for deflection calculations

(lb)

476.0 (c)

#### **Flexural and Deflection Check**

Span

Center Span

	Mmax	Mmax/	Mpos	Bracing	Ma(Br	c) Mpos/	De	flection
Span	Ft-Lb	Maxo	Ft-Lb	(in)	Ft-Lb	Ma(Brc)	(in)	Ratio
Center Span	1360.0	0.414	1360.0	72	1958.	9 0.694	0.133	L/722
Combined Ben	ding and Wo	eb Crippling						
Reaction	or	Load	Brng	Pa		Mmax	Intr.	Stiffen
Pt Load	t	P(lb)	(in)	(lb)		(Ft-Lb)	Value	Req'd?
R1		480.0	1.50	1031.0		0.0	0.56	No
R2		480.0	1.50	1031.0		0.0	0.56	No
P1		400.0	3.50	2596.0		1360.0	0.60	No
Combined Ben	ding and Sh	<u>ear</u>						
Reaction	or	Vmax	Mmax	Va			Intr.	Intr.
Pt Load	t	(lb)	(Ft-Lb)	Factor	V/Va	M/Ma	Unstiffen	Stiffen
R1		480.0	0.0	1.00	0.09	0.00	0.01	NA
R2		480.0	0.0	1.00	0.09	0.00	0.01	NA
P1		200.6	1360.0	1.00	0.04	0.41	0.17	NA
Combined Ben	ding and Ax	ial Load						
		Axial Ld	Bracing	g (in)	Max	Allow Ld		Intr.

KtLt

None

KL/r

171

(lb)

2699.1 (c)

KyLy

None



Value

0.88

P/Pa

0.18



# **Appendix**





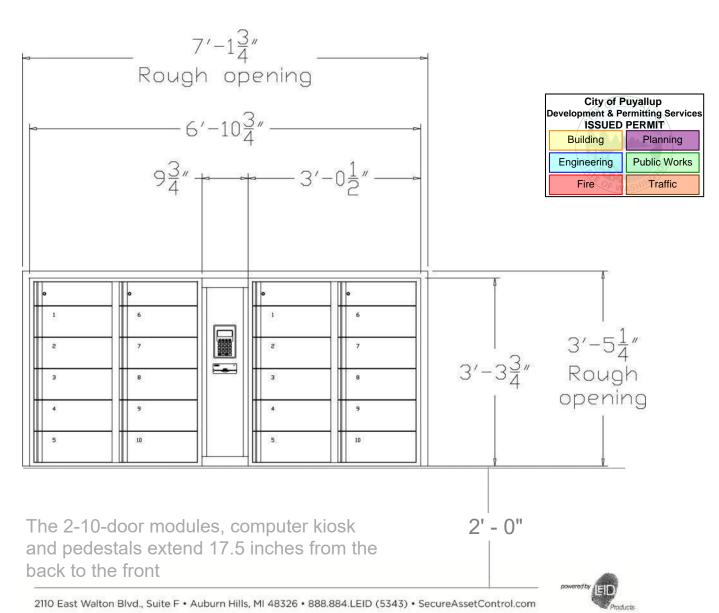
### **Configuration and System Drawings:**

Based on the current solicitation the following equipment and services will be provided.

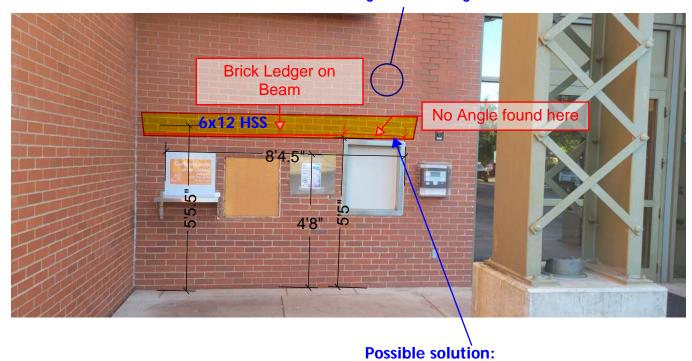
#### System Design and Dimensions: 20-Door System Graphic

- 1 wirelessly accessed, web enabled controller with LCD interface w/ card reader in the computer kiosk
- 2 [LKR-10D-MD] 10 Door Medium Passthrough Lockers
- 1 wireless router
- 1 barcode reader

Freight – local / common carrier (inside delivery)



### No signs of settling



Weld angle to header where missing prior to brick in-fill if necessary

