

Re: J1128612 Mick Allen





Tri-State Engineering, Inc. 12810 NE 178th Street Suite 218 Woodinville, WA 98072 425.481.6601

The truss drawing(s) referenced below have been prepared by Tri-State Engineering under my direct supervision based on the parameters provided by The Truss Company (Sumner).

Pages or sheets covered by this seal:	14512906	thru I14512930
My license renewal date for the state of	of Washington is	August 20, 2024.

THE APPROVED CONSTRUCTION PLANS, DOCUMENTS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

FULL SIZED LEDGIBLE COLOR PLANS ARE REQUIRED TO BE PROVIDED BY THE PERMITEE ON SITE FOR INSPECTION



October 28,2022

Terry Powell

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Plate Offsets (X,Y) [7:Ed	ge,0-2-0]											
LOADING (ps TCLL (Roof Snow=2 TCDL BCLL BCDL	sf) 25.0 5.0) 8.0 0.0 * 7.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC BC WB Matri	0.14 0.03 0.04 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.00 0.00	(loc) 1 1 7	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 27 lb	GRIP 185/148 FT = 20%	
LUMBER- TOP CHORD	2x4 HF No.2					BRACING- TOP CHORE	o s	tructura	al wood sl	heathing dire	ectly applied or 6-0-0 o	oc purlins,	

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD	2x4 HF No.2
BOT CHORD	2x4 HF No.2
WEBS	2x4 DF Stud
OTHERS	2x4 DF Stud

REACTIONS. All bearings 6-6-0.

(lb) - Max Horz 2=83(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 7, 8, 9, 10

Max Grav All reactions 250 lb or less at joint(s) 7, 8, 9, 10 except 2=256(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (12)

- 1) Wind: ASCE 7-16; Vult=110mph (3-second gust) Vasd=87mph; TCDL=4.8psf; BCDL=4.2psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 3-4-4, Corner(3E) 3-4-4 to 6-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- s) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 23.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7, 8, 9, 10.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

referenced standard ANSI/TPI 1.

City of Puyallup ment & Permitting ISSUED PERMIT Iding Plan

12) All dimensions given in feet-inches-sixteenths (FFIISS) format.

October 28,2022

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TCLL 25.0	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl L/d	PLATES GRIP
(Roof Snow=25.0) TCDL 8.0	Lumber DOL 1.15	BC 0.61	Vert(CT) -0.23	4-7 4-7	>330 240	WI120 165/146
BCLL 0.0 * BCDL 7.0	Code IRC2018/TPI2014	Matrix-MP	Hol2(C1) 0.00	2	n/a n/a	Weight: 21 lb FT = 20%
LUMBER-			BRACING-			

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 HF No.2 BOT CHORD 2x4 HF No.2 2x4 DF Stud WEBS

REACTIONS. (size) 2=0-5-8, 4=0-1-8 Max Horz 2=84(LC 11) Max Uplift 2=-41(LC 12), 4=-31(LC 12)

Max Grav 2=475(LC 19), 4=353(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-4=-268/82

NOTES-(11)

City of Puvallup

- 1) Wind: ASCE 7-16; Vult=110mph (3-second gust) Vasd=87mph; TCDL=4.8psf; BCDL=4.2psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 3-4-4, Exterior(2E) 3-4-4 to 6-4-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 23.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) All dimensions given in feet-inches-sixteenths (FFIISS) format.

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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Job	Truss	Truss Type	Qty	Ply	Mick Allen
J1128612	D01	GABLE	1	1	114512912
					Job Reference (optional)
The Truss Company (Sumne	er), Sumner, WA - 98390,			8.530 s Au	g 11 2022 MiTek Industries, Inc. Fri Oct 28 06:09:56 2022 Page 2
		ID:tU	JrQVQk_h	P9cdUwrP	L48u9ySCrA-fhZT0hCezitZVh7ilWxJhTdyncXAZ63GV82Z1ryOzTv

NOTES- (14)

Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
All dimensions given in feet-inches-sixteenths (FFIISS) format.

PRRASF20221703

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LOADING (psf) TCLL 25.0 (Roof Snow=25.0) TCDL TCDL 8.0 BCLL 0.0 BCDL 7.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.18 BC 0.03 WB 0.04 Matrix-R	DEFL. i Vert(LL) 0.0 Vert(CT) 0.0 Horz(CT) 0.0	(loc) 11 11 11 12	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES GRIP MT20 185/148 Weight: 52 lb FT = 20%
LUMBER-			BRACING-				

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x4 HF No.2
BOT CHORD	2x4 HF No.2
WEBS	2x4 DF Stud
OTHERS	2x4 DF Stud

REACTIONS. All bearings 10-0-0.

(lb) -Max Horz 20=-89(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 20, 12, 17, 18, 19, 15, 14, 13 Max Grav All reactions 250 lb or less at joint(s) 20, 12, 16, 17, 18, 19, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

- Wind: ASCE 7-16; Vult=110mph (3-second gust) Vasd=87mph; TCDL=4.8psf; BCDL=4.2psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 2-0-0, Corner(3R) 2-0-0 to 8-0-0, Exterior(2N) 8-0-0 to 8-6-0, Corner(3E) 8-6-0 to 11-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 9) Gable studs spaced at 1-4-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 23.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 12, 17, 18, 19, 15, 14, 13.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) All dimensions given in feet-inches-sixteenths (FFIISS) format.

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

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- BOT CHORD 2-5=-58/349
- WEBS 3-5=-432/141

NOTES- (11)

- Wind: ASCE 7-16; Vult=110mph (3-second gust) Vasd=87mph; TCDL=4.8psf; BCDL=4.2psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 5-4-4, Exterior(2E) 5-4-4 to 8-4-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 23.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
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Job	Truss	Truss Type	Qty	Ply	Mick Allen
J1128612	H01	Common Girder	1	2	I14512922
					Job Reference (optional)

The Truss Company (Sumner), Sumner, WA - 98390,

8.530 s Aug 11 2022 MiTek Industries, Inc. Fri Oct 28 06:10:06 2022 Page 2 ID:tUrQVQk_hP9cdUwrPL48u9ySCrA-McAF75Kwcm88iDuduc7f5a1fFes1uaJkoiT5OGyOzTI

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-66, 3-4=-66, 1-5=-14

Concentrated Loads (lb)

Vert: 11=-992(F) 12=-992(F) 13=-992(F) 14=-992(F) 15=-992(F) 16=-996(F)

PRRASF20221703

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will fit between the bottom chord and any other members.

ISSUED PERMIT

Public Works

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
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ISSUED PERMI

Public Works

Building

October 28,2022

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during constructions is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

