GENERAL NOTES:

The Structure has been designed to resist code-required vertical and lateral forces after the construction of all structural elements have been completed. Stability of the structural elements prior to completion is the sole responsibility of the General Contractor. Builder is to verify all dimensions prior to starting work. All changes to structure must be reviewed and approved by Phillips Structural Engineering. All materials, methods, and workmanship shall be in accordance with the International Building Code (IBC) 2018 Edition. The Builder is responsible for using safe work practices and conforming to all safety ordinances.

Any construction observation performed by the Structural Engineer is for general conformance with design aspects per IBC 1704.6 and is not intended in any way to review the General Contractor's construction procedures. Structural observation does not waive the responsibility for special inspections per Section 1705.

STANDARDS

All methods, materials and workmanship shall conform to the 2018 International Building Code (IBC) as amended and adopted by the local Building Official or applicable jurisdiction.

STRUCTURAL DESIGN CRITERIA AND LOADING:

ROOF LOADING:

- DEAD LOAD = 15PSF (NO TILE WEIGHT INCLUDED)
- ROOF LIVE LOAD (Lr) = 20PSF
- TYPICAL FLAT ROOF SNOW LOAD (Pf) = 25PSF
- SNOW EXPOSURE FACTOR (Ce) = 1.0 • SNOW LOAD IMPORTANCE FACTOR (Is) = 1.0
- **TYPICAL ROOF SNOW LOAD SHALL NOT BE LESS THAN 25PSF

FLOOR LOADING

- DEAD LOAD = 12PSF (STANDARD WOOD FRAMING WITHOUT HEAVY FINISH)
- TYPICAL RESIDENTIAL OCCUPANCY LIVE LOADING (L) = 40PSF
- DECK LIVE LOAD (L) = 60PSF (1.5X FOR AREA SERVED)

WIND DESIGN DATA:

- BASIC DESIGN WIND SPEED (V) = 110 MPH
- ALLOWABLE STRESS DESIGN WIND SPEED (Vasd) = 85 MPH
- RISK CATEGORY = II
- WIND IMPORTANCE FACTOR (lw) = 1.0
- WIND EXPOSURE = "B"
- TOPOGRAPHICAL EFFECT (Kzt) = 1.00

EARTHQUAKE DEIGN DATA:

- RISK CATEGORY = II
- SEISMIC IMPORTANCE FACTOR (Ie) = 1.0
- MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS - Ss = 1.26
 - -S1 = 0.44
- SEISMIC DESIGN CATEGORY (SDC) = "D"
- BASIC SEISMIC FORCE-RESISTING SYSTEM = LIGHT-FRAMED (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANEL RATED FOR SHEAR RESISTANCE
- RESPONSE MODIFICATION FACTOR (R) = 6.5
- ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE

SOIL INFORMATION

In the absence of a soils report (prepared by a Licensed Geotechnical Engineer) an Allowable Soil Bearing Pressure (ASBP) of 1500psf has been assumed for design. It is the contractor's responsibility to verify that all footings bear on firm/unvielding, undisturbed earth or compacted "Structural Fill" that meets or exceeds the assumed allowable soil bearing pressure. Soils under footings and slabs to be a minimum of 95% compacted per the Modified Proctor Density Test (ASTM D1557). All construction on fill shall be reviewed by a licensed Geotechnical Engineer.

FOUNDATIONS:

Bottom of exterior footings shall be a minimum of 18" below finished grade for frost protection U.N.O. bearing on undisturbed native soils. Backfill with dry, free draining soils. Backfill next to any retaining walls with a minimum of 12" gravel or free draining soil per typical detailing. Appropriate drainage shall be provided adjacent to the base of exterior foundations draining to daylight per Soils Engineer requirements.

All concrete materials and placement shall conform to the current ACI 318 code. Concrete shall be made with Portland Cement ASTM C-150 Type II or Type I and shall be Ready-Mixed per ASTM C-94. Minimum compressive strength shall be 3000 PSI* at 28 days with a minimum water/cement ratio of 0.45. All concrete shall be air entrained 5 ±1%. Max aggregate size = $\frac{7}{8}$ ". *Special Inspection not required for concrete compressive strength. 3000 PSI compressive strength has been specified for weathering protection. Structural design of concrete has been based on 2500 PSI compressive strength.

engineered documents, site conditions, product availability, etc. All information listed above must be verified prior to construction and fabrication. Any changes or deficiencies on or to the plan must be transmitted to Phillips Structural Engineering, PLLC for written approval.

PLAN VERACITY: Every attempt has been made to insure the accuracy of these

LICENSE OF ENGINEERING DOCUMENTS:

1. Grant of License: Phillips Structural Engineering, PLLC grants the Licensee a nonexclusive Right of Use for the purpose of constructing a single structure (use) from this engineering package. Future uses of this drawing set are permitted when accompanied by an originally stamped "Site Specific Re-Use Letter" for each additional site and payment has been made to Phillips Structural Engineering, PLLC.

2. Ownership of Engineering Calculations and Drawings: Phillips Structural Engineering, PLLC shall retain "Title 17 USC Rights and Ownership" of the Copyright Law of The United States of America of these Engineering Documents and all subsequent copies of the engineering. The Licensee is not permitted unlimited reuse of these documents without prior consent

3. Copy and Transfer Restrictions: Unauthorized copying of the Engineering documents is strictly forbidden. The Engineering is permitted to be used by the Licensee only, and may not be transferred to a 3rd party without prior written consent of Phillips Structural Engineering, PLLC.

REINFORCING STEEI

All reinforcing steel to be GRADE 60 PER ASTM A-615. Reinforcement lap splices shall be the greater of 32 bar diameters or 18". Lap horizontal steel at all corners and intersections in footings and walls with continuous corner bars. Footing reinforcement shall run continuous thru pad/expanded footings.

Minimum concrete cover over reinforcing steel shall be provided as follows:

- INTERIOR FACES OF SLAB AND WALL BARS = 1½ "
- EXPOSED TO WEATHER OR EARTH = 1½" AT #5 AND SMALLER AND 2" AT #6 AND LARGER.
- CONCRETE CAST AGAINST SOIL = 3"

Structural Engineering for possible alternatives.

WOOD FRAMING AND CARPENTRY

General Requirements: Provide minimum nailing per 2018 IBC table 2304.10.1 or as indicated on the drawings. Do not notch or drill structural members, except as permitted by the Engineer. Framing Connectors: Only ICC / IAPMO approved connectors shall be used in framing applications as manufactured by Simpson Strong-Tie or equivalent. Provide maximum size and quantity of fasteners shown in the manufacturer's catalog U.N.O.

Fasteners: Bolts shall be per ASTM A-307 with standard cut washers or malleable iron washers. Post-installed anchors require Engineer's approval prior to installation. Contact Phillips

Nails shall be common wire nails or equivalent pneumatically drive nails (P-nails), American or Canadian manufacturer only as indicated below. P-nails shall be installed per the manufacturer's guidelines.

COMMON WIRE NAIL	DIAMETER (INCHES)	MINIMUM LENGTH (INCHES)	NAIL APPLICATION
8d COMMON	0.131	2½"	SHEATHING
10d COMMON	0.148	2½"	SHEATHING
N/A	0.131	3"	FRAMING
12d COMMON	0.148	31⁄4"	FRAMING
16d COMMON	0.162	3½"	FRAMING

Wood Sheathing (Structural):

Roof sheathing shall be $\frac{1}{2}$ " CDX or $\frac{7}{16}$ " OSB nailed w/ 8d @ 6" o.c. along panel edges, and 12" o.c. in field. Span index shall be 24/0. Plywood Sub Flooring shall be $\frac{3}{4}$ " T&G CDX or OSB (glued & nailed). Nailing shall be 10d @ 6"o.c. along panel edges, and 12"o.c. in field (U.N.O.) Span index shall be 48/24. Stagger all end laps per typical detailing. All sheathing shall bear the grade trademark of the American Plywood Association (APA).

All studs shall be kiln dried (KD) or surface dried (SD). Each stud shall bear the stamp of the West Coast Lumber Inspection Bureau (WCLIB) or Western Wood Products Association (WWPA) showing grade mark or approved equal. All studs and framing material shall be 2x nominal minimum material and species shall be a minimum of Hem-Fir stud grade meeting the following minimum strength properties: Fb= 675psi, Fv=150psi, E=1,200,000psi.

All headers (HDR) not specified or otherwise noted on the plan with spans ≤5'-0" are to be 4x8 DF#2 or (2)2x10 HF#2 with at least one cripple and one king stud at each end. Spans between 5'-0" and 8'-0" shall have headers as noted on plan and have at least two cripples and one king stud U.N.O. Spans greater that 8'-0" shall have framing and support per plan.

All timbers above sizes listed above including posts and beams shall be Doug-Fir #2 or better (Typically 6x beams and posts)

Glu-Laminated Beams (GLB)

All GLB shall be in conformance with ANSI A190.1, American National Standards for Structural Glue-Laminated Timber. Grade 24F-V4 shall meet or exceed the following minimum structural properties: Fb=2400psi, Fv=240psi, E=1,800,000psi and shall be used at simple spans. Grade 24F-V8 shall be used at continuous and cantilevered spans. As an alternative at continuous/cantilevered spans an equivalent size (width x depth) PSL beam may be substituted (see below).

Parallel Strand Lumber (PSL): All PSL shall be in conformance with ASTM D2559 and NER-292. PSL strength requirements shall meet or exceed the following minimum structural properties: Fb=2,900psi, Fv=290psi, E=2,000,000psi.

Laminated Veneer Lumber (LVL):

All LVL shall be in conformance with ASTM D2559. LVL shall be made of Doug-Fir (DF)and meet or exceed the following minimum structural properties: Fb= 2,600psi, Fv=285psi, E=1,900,000psi.

Preservative Treatment (P.T.):

All exposed framing including lumber, plywood and deck materials shall be pressure treated with 0.25#/cf pentachlorophenol per AWPA specification P-5 or other approved treatment. All cutting and boring after pressure treatment shall be cared for in accordance with AWPA specification M-4. Exposed framing includes, but is not limited to:

1. Joists, girders and subfloor that is closer than 18" to exposed ground in crawl spaces. 2. Wood framing (including sheathing) that rest on exterior foundation stem walls and is 8" or less from exposed earth.

3. Any other wood product in direct contact with concrete or masonry.

Wood Connectors at P.T. Conditions:

3/4" T&G PLYWOOD OR OSB-

SHEATHING AT FLOOR & 7/16"

SHEATHING AT ROOF (APA RATED)

PANEL EDGE BOUNDARY ----

RIM JOIST/BLOCKING -

INTERMEDIATE FRAMING -

(FIELD) MEMBER NAILING (12" O.C. MAX. SPACING)

) PROVIDE APA APPROVED GLUE,

NAIL ALL PLYWOOD TO JOISTS

2.) STAGGER PANELS TO OFFSET

JOINTS AS SHOWN.

NOTES:

NAILING (6" O.C. MAX. SPACING)

Metal connectors which are in contact with pressure treated wood shall be protected with one of the following: Simpson "ZMAX" G185 Galvanization, Triple Zinc Coated, hot Dipped Galvanized or other approved method.

TYPICAL HORIZONTAL PLYWOOD DIAPHRAGM LAYOUT

(ROOF AND FLOOR)

PANEL EDGE NAILING

STAGGER NAILING @ PANEL EDGES)

JOISTS/TRUSS/RAFTER PER PLAN

Pre-Engineered Roof and Floor Trusses:

All brefabricated wood roof and floor trusses shall be designed by or under the direct supervision of a licensed Professional Engineer registered in the state where the structure is located. The truss shop drawings shall bear the stamp of that Engineer and shall be fabricated and installed per the latest Truss Plate Institute standards. Where trusses are not provided to complete the roof system, overframing members with positive connections shall be provided as directed by the Truss Engineer. Details on overframing shall be provided in the truss shop drawing package which shall be consistent with the modeling of the trusses. All necessary temporary and permanent bridging, blocking, pre-notched plates, hangers, etc. for the stability of the truss elements under gravity and lateral loads shall be designed, detailed/specified and furnished by the manufacturer. The Truss Manufacturer shall verify all setbacks, dimensions and bearing points prior to fabrication. Deviations from the layout shown on approved engineering plans shall be submitted to PSE for further evaluation. Maximum allowable deflections shall be as follows:

- ~Roof Total Load = L/360 or 1" (whichever is less)
- ~Roof Snow/Live Load = L/480 or $\frac{3}{4}$ " (whichever is less)
- ~Floor Total Load = L/360 or $\frac{3}{4}$ " (whichever is less)
- ~Floor Live Load = L/480 or $\frac{1}{2}$ " (whichever is less)

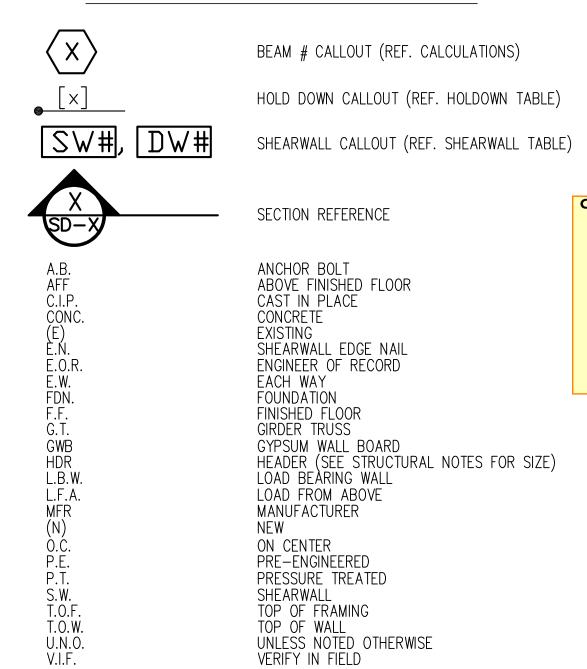
Trusses shall be designed for the spans and conditions shown and be constructed from a minimum of Hem-Fir timber and be furnished and installed in conformance with the manufacturer's published specifications. Additional concentrated loads from mechanical units and misc. equipment shall be accounted for/coordinated with sub-contractors, the Designer of Record and Truss Engineer. Framing has been designed assuming Hem-Fir plates w/ 405psi crushing capacity, truss engineer to confirm compatibility and add bearing enhancers or additional support as required.

Where trusses align with shear walls, truss engineer to design and provide a truss that has been designed to transfer lateral wind and seismic forces as shown on the plans. Loading indicated (100plf minimum) shall be designed by the truss engineer to transfer from roof or floor sheathing to shearwall below. Shop drawings including truss engineering shall be submitted to the Engineer of Record for approval prior to submittal to the jurisdiction and fabrication. Should the truss engineering not be submitted to the Engineer of Record for review the builder is assuming responsibility and proceeding at their own risk.

Alteration of the truss layout indicated on the plans may require supporting structural and foundation changes, therefore prior approval by the designer and Structural Engineer is required. Trusses shall not be field altered in any way without written approval from the Licensed Truss Engineer of Record.

> FULL SIZED LEDGIBLE COLOR PLANS ARE REQUIRED TO BE PROVIDED BY THE PERMITTEE ON SITE FOR ALL INSPECTIONS (MIN. PLAN SIZE 24" X 36")

SYMBOL & ABBREVIATIONS LEGEND:



City of Puyallup Building ACCEPTED **JMontgomery** 07/19/2023 10:43:26 AM

TOP OF WALL UNLESS NOTED OTHERWISE

VERIFY IN FIELD

-SIMPSON ST6224 W/ (28) 0.1620 x 2½" NAILS (CENTER ON SPLICE) -(2) ROWS OF (4) 16d NAILS 2X CRIPPLE & (1) 2X KING MIN. - TYPICAL WALL FRAMING

BEAM FLUSH TO DBL TOP PLATE

		Minimum Required		Stud &	Rim Joist Or	2x Bottom Plate	Sill Plate Attac	Sill Plate Attachment	
Label	ASD S.W. Capacity (PLF) Seismic/Wind	APA Rated Sheathing (panel) (A) (B) (C) (D) (N)	Panel Edge Nailing Sheathing Edges (D) (E) (K)	Blocking Size action To Top Plate (C) (H): (Does Nailing To Wood		Nailing To Wood Below	(ø)Anchor Bolt & Spacing To Concrete Below (J) (K) (M)	Sill Plate Size @ Fdn.	
SW6	242/339	¼ ₆ "OSB (1) Side	0.131x2½" @ 6" O.C.	2x_	LTP4 @ 36"OC	16d @ 6" O.C. OR 12d @ 4" O.C.	%" @ 48" O.C.	2x	
SW4	350/495	%6"OSB (1) Side	0.131x2½" @ 4" O.C.	2x_	LTP4 @ 20"OC	16d @ 4" O.C. OR 12d @ 3" O.C.	%"	2x	
SW3	456/637	¾6"OSB (1) Side	0.131×2½" @ 3" O.C.	3x_	LTP4 @ 12"0C	16d @ 3½" O.C.	%" @ 24" O.C.	2x	
SW2	595/832	¾6"OSB (1) Side	0.131x2½" @ 2" O.C.	3x_	LTP4 @ 10"0C	16d @ 3" O.C.	%" @ 18" O.C.	2×	
DW4	700/990	⅓6"OSB Each Side	0.131×2½" @ 4" O.C.	2x_	LTP4 @ 8"0C	(2) ROWS 16d @ 4" O.C. & LTP4 @ 16" OC AT BLK'G TO PLATE	%" @ 16" O.C.	2x	
DW3	911/1274	⅓ ₆ "OSB Each Side	0.131×2½" @ 3" O.C.	3x_	LTP4 @ 6"0C	(2) ROWS 16d @ 4" O.C. & LTP4 @ 16" OC AT BLK'G TO PLATE	%" @ 10" O.C.	2x	

- ALL SHEATHING PANELS SHALL BE INSTALLED HORIZONTALLY WHERE FRAMING EXCEEDS 16" O.C. SPACING. BLOCK ALL PANEL EDGES. WHEN SHEATHING IS APPLIED TO BOTH SIDES OF SHEARWALL (DW4 & DW3), STAGGER PANEL EDGES ON OPPOSITE SIDES OF WALL SUCH THAT JOINTS ON OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUDS.
- BLOCK ALL SHEATHING PANEL EDGES SHEARWALL SHEATHING/NAILING SHALL BE PROVIDED THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. ENDS OF FULL HEIGHT
- SHEARWALLS ARE DESIGNATED BY EXTERIOR BUILDING CORNERS, CORRIDORS, WALL PENETRATIONS OR AS DESIGNATED ON THE PLANS. SEE PLANS & TABLE FOR REQUIRED HOLDOWNS (ALL SHEARWALLS REQUIRE SHEATHING ABOVE & BELOW OPENINGS WITH SAME NAIL PATTERN) EDGE NAIL SHEATHING AT ALL HOLDOWN POST/STUDS. EDGE NAILING IS REQUIRED TO EACH STUD USED IN A BUILT—UP HOLDOWN POST
- (MULTIPLE 2X STUDS). SEE TYPICAL DETAILS FOR FURTHER INFORMATION. INTERMEDIATE STUDS SHALL BE 2X_ MINIMUM AND NAILED WITH 0.131" & X 21/2" FIELD NAILING AT 12" O.C. (WHERE STUD SPACING EXCEEDS
- USE 0.131"øx2兆" NAILS TYPICALLY FOR LTP CLIPS WHEN INSTALLING OVER SHEATHING. IF LTP CLIPS ARE INSTALLED DIRECTLY TO FRAMING. 0.131"ØX11/2" NAILS MAY BE USED. LTP CLIPS MUST BE INSTALLED WITH LONG DIMENSION HORIZONTALLY. A35 OR RBC CLIPS MAY BE USED
- LTP4'S NOT REQ'D WHERE RIM SHEATHING OVERLAPS THE SECOND FLOOR BOTTOM PLATE. SHEATHING SHALL BE CONTINUOUS OVER BOTTOM PLATE AND RIM OR SILL PLATE PER TYPICAL DETAILING WITH EDGE NAILING AT EACH. WHEN TWO ROWS OF NAILING ARE REQ'D AT DOUBLE SIDED SHEARWALLS SEE TYPICAL DETAILING FOR FASTENERS AND BLKG.
 ALL ANCHOR BOLTS SHALL HAVE STEEL PLATE WASHERS 0.229"x3"X3". ANCHOR BOLTS W/ STANDARD 90' BEND SHALL BE EMBEDDED 7" INTO
- PROVIDE "HOT-DIPPED" GALVANIZED NAILS/BOLTS AND METAL CONNECTORS WHEN ATTACHING TO PRESSURE TREATED MATERIALS. WHERE 3x STUDS ARE REQ'D AT ADJOINING PANEL EDGES (2) 2x STUDS MAY BE NAILED TOGETHER IN PLACE OF USING A SINGLE 3x. FACE NAIL
- STUDS TOGETHER W/ 16d AT SAME SPACING SHOWN IN 2X BOTTOM PLATE ATTACHMENT COLUMN. PANEL EDGE NAILING IS REQUIRED TO EACH OF
- POST INSTALLED ANCHORS MAY NOT BE A VIABLE ALTERNATIVE. CONTACT PHILLIPS STRUCTURAL ENGINEERING FOR CONSULTATION.
- APA RATED PLYWOOD MAY BE USED IN PLACE OF OSB ALL NAILING SHALL BE PER IBC TABLE 2304.10.1 U.N.O.

CONCRETE. PLATE WASHERS SHALL EXTEND TO WITHIN ½" OF SHEATHING EDGE.

HOLDOWN AND				
Strap/Holdown Attachmer				
Required <u>NAILING</u> and Boundary Studs	Required <u>SCREWS</u> and Boundary Studs	Anchorage to Foundation (D)		
X STUDS W/ (30) 0.148"øx3¼" NAILS	N/A	EMBEDDED STRAP		
N/A	(2) 2X STUDS W/ (14) 1/4"x21/2" SDS SCREWS	SIMPSON SB%x24		
N/A	(3) 2X STUDS W/ (20) 1/4"x3" SDS SCREWS	SIMPSON SB%X24 (SEE NOTE		
N/A	6x6 DF POST W/ (24) ¼"x2½" SDS SCREWS	SIMPSON SB1X30 (SEE NOTE		

N/A

N/A

N/A

N/A

N/A

N/A

N/A

N/A

NOT USED MANDATORY NOTES

Simpson Strong-Tie

Product Label (SKU)

(A) (B) (C) (F)

STHD14/STHD14RJ

HDU5-SDS2.5

HDQ8-SDS3

HHDQ11-SDS2.5

NOT USED

MST60 STRAP

MSTC48B3 STRAP

MSTC66B3 STRAP

Holdown Label (G)

- INSTALLATION OF ALL HOLDOWN ANCHORS AND STRAPS SHALL BE PER THE MANUFACTURE'S RECOMMENDATIONS ALL METAL CONNECTORS IN CONTACT WITH PRESSURE TREATED (P.T.) MATERIAL SHALL BE COATED WITH SIMPSON "Z-MAX" (G-185) GALVANIZATION, TRIPLE ZINC COATED FOR CORROSION PROTECTION
- SUPPLEMENTAL NOTES

2) 2X STUDS W/ (34) 0.162"øx2½" NAILS

2) 2X STUDS W/ (46) 0.162"øx2½" NAILS

(2) 2x STUDS W/ (38) 0.148"øx3" NAILS

(2) 2x STUDS W/ (38) 0.148"øx3" NAILS

- ALTERNATE HOLDOWN TYPES MAY BE ACCEPTABLE WITH WRITTEN APPROVAL OF PHILLIPS STRUCTURAL ENGINEERING. ALTERNATE ADHESIVE ANCHORS MAY BE AVAILABLE, CONTACT PHILLIPS STRUCTURAL ENGINEERING
- IF 6" STEMWALLS ARE USED, THREADED ROD WITH DOUBLE NUT AND PLATE EXTENDED INTO THE FOOTING IS REQ'D. SEE ALTERNATE
- DETAILING. SB%"X24 & SB1X30 MAY ONLY BE USED IN 8" STEMWALLS.
- SIMPSON HDU5-SDS2.5 MAY BE USED AS AN ALTERNATE TO STHD14(RJ).
- STRAPS MAY BE INSTALLED OVER SHEATHING

IMPORTANT FRAMING NOTE: GENERAL CONTRACTOR/FRAMER SHALL VERIFY ALL CONCENTRATED LOADS FROM ABOVE TO HAVE SOLID BEARING CONTINUOUS TO THE FOUNDATION (INCLUDING SQUASH BLOCKING IN CRAWLSPACE/FLOOR FRAMING). ANY CHANGES TO FOUNDATION AND STRUCTURE

TYPICAL FRAMING NOTES:

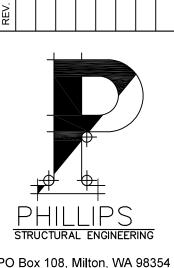
1. Roof and floor diaphragm nailing per Wood Sheathing Notes.

AFFECTED PARTIES SHALL BE NOTIFIED.

2. Solid blocking shall be provided between the bearings of every rafter or truss. Attach every rafter & truss to framing with Simpson H2.5T or clip indicated in typical detailing. 3. Provide solid built-up studs or posts under all girder trusses, roof beams and floor beams. Solid blocking (squash blocking) is required in all floor cavities under built-up studs/posts. Built-ups or posts shall run continuous to the foundation.

FROM ARCHITECTURAL PLANS SHALL BE NOTED AND ALL

- 4. Exterior walls shall be nailed & detailed per SW6 U.N.O. All panel edges shall be blocked per shearwall table.
- 5. Where (2) shearwalls meet at a corner/intersection, the sheathing of each wall shall be edge nailed to the studs/post which the holdown is attached to.
- 6. Provide a solid rim joist where floor joist bear on exterior walls per typical details. 7. Where floor framing runs parallel to exterior walls, install 2x or I-joist blocking panels between rim joist and first joist @ 48" o.c. ~ nail sheathing to blocking w/ 8d @ 3" o.c. (crawlspace framing where rim joist bears on concrete stemwalls are omitted from this
- 8. Roof and floor joists/trusses are shown schematically on the plan and are not intended to show every location of every joist/truss.
- 9. Provide double joists under interior partition walls when running parallel to each other. 10. Top plates are assumed to be continuous and may be spliced per typical detailing on this
- 11. All columns not specified or shown on the plans are to be a minimum of (2) 2x studs spike laminated together with 16d nails @ 6" o.c. (stagger).
- 12. All post-beam intersections shall contain positive connections to resist against uplift and/or lateral displacement. 13. Anchorage of walls to the foundation shall be provided in accordance with the shearwall
- 14. Typical Walls shall be framed with Hem-Fir U.N.O. 15. Walls shall be anchored to the foundation per the minimum requirements of SW6 ($\frac{5}{8}$ "Ø
- A.B. @ 48" o.c. embedded 7" and no more than 12" from ends of each sill and 2 bolts
- minimum) with $3"x3"x\frac{1}{4}"$ square washers firmly attached between plate and nut. 16. All walls over 10' tall shall have bracing/blocking at 48" o.c. (flat or on edge).



Phone: (253) 344-1666 www.PhillipsSE.com



ALL VARIATIONS FROM DIMENSIONS AND CONDITIONS SHOWN ON THE DRAWINGS SHALL BE REPORTED TO THE DESIGNER FOR RESOLUTION WITH THE ENGINEER PRIOR TO PROCEEDING WITH WORK; FAILURE O COMPLY BY THE CONTRACTOR SHALL BE THEIR SOLE RESPONSIBILITY FOR ANY COSTS

NECESSARY FOR REMEDIAL WORK. UNAUTHORIZED ALTERATION OF ANY OF THE INFORMATION ON THIS DOCUMENT WILL INVALIDATE THE DOCUMENT, ENGINEER'S SEAL AND SIGNATURE. THE DESIGNS AND IDEAS NCORPORATED HEREIN, AS A TOOL OF PROFESSIONAL SERVICE. IS THE PROPERTY OF PHILLIPS STRUCTURA ENGINEERING, PLLC AND IS NOT TO BE USED IN PART OR IN WHOLE BY

ANY PARTY FOR ANY OTHER

PROJECT WITHOUT WRITTEN

AUTHORIZATION FROM PHILLIPS

STRUCTURAL ENGINEERING, PLLC.

E SW WA DUPL 109 43RD AVE PUYALLUP,

WIND SPEED: WIND EXPOSURE: ROOF SNOW LOAD: 25 PSF

 $\overline{}$

GENERAL NOTES PRRNSF20230918

DRAWN BY: AMS APR. 5, 2023 DRAWING DATE N.T.S. (U.N.O.) PSE NUMBER: PSE 21.047 RUEPPELL DESIGNER:

SHEET NO:

PHILLIPS STRUCTURAL ENGINEERING

TYPICAL TOP CHORD SPLICE

TYPICALLY - FACE NAIL DOUBLE TOP PLATES

1-1/2" MIN -

(12) 0.131"ø x 3" LONG P-NAIL @ EACH

STUD @ BOT SPLICE

LOCATION (TYP)

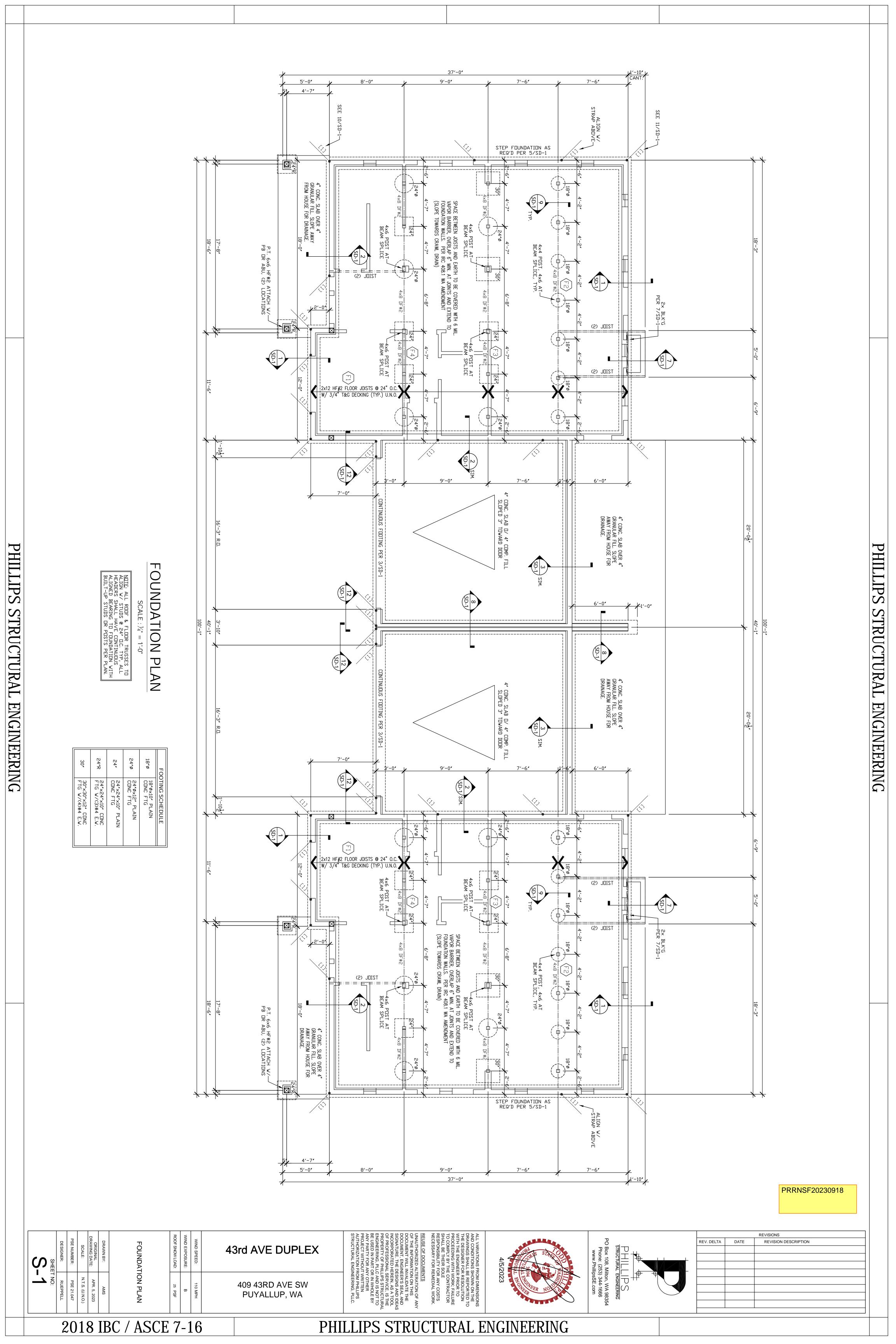
W/ 0.131"ø X 3" @ 12" O.C. STAGGERED

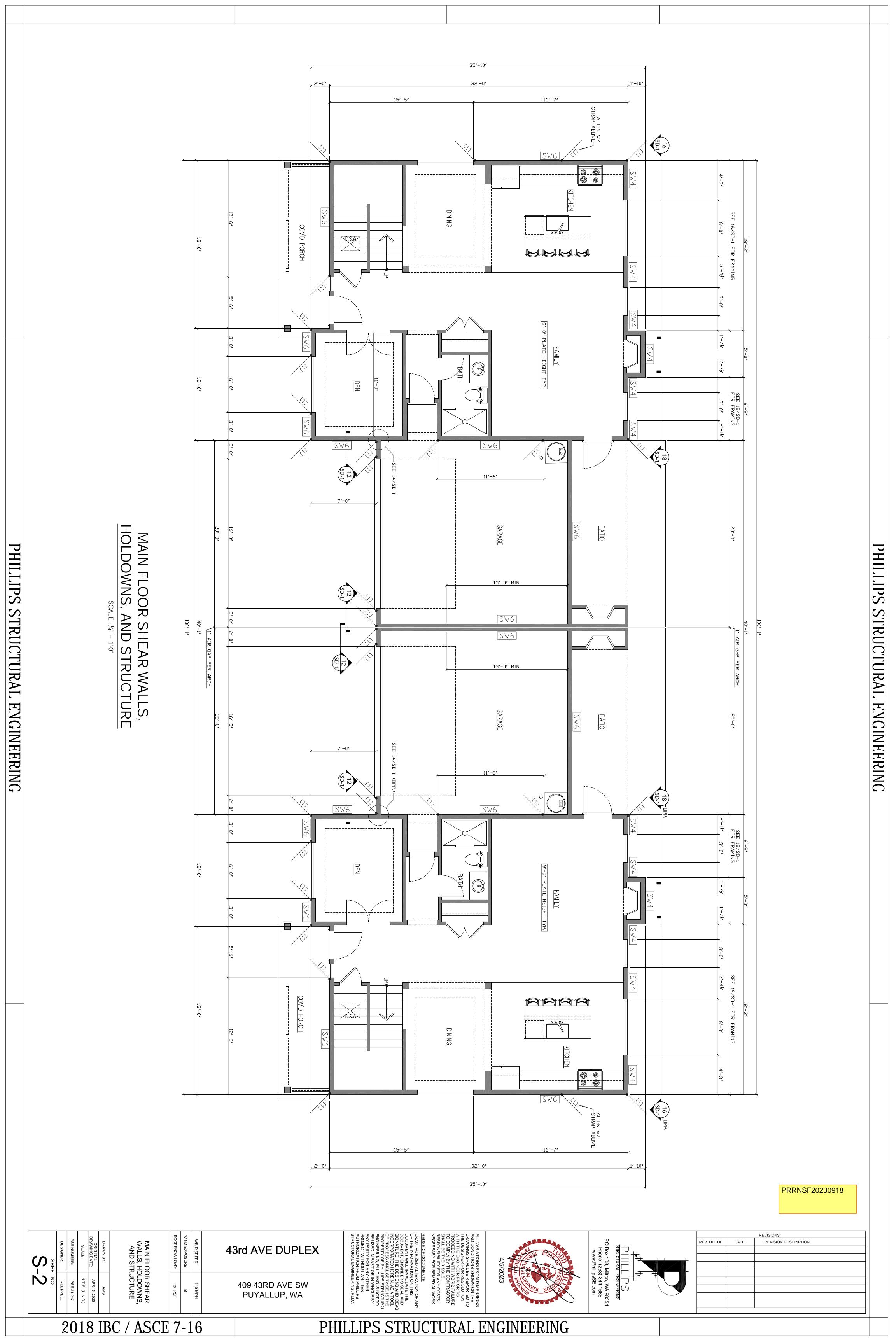
WHERE PLATE DISCONTINUITIES ARE CREATED BY BEAMS

& PIPES, ETC. STRAP W/ (1)-SIMPSON ST6224 UNLESS NOTED OTHERWISE. STRAP MAY BE PLACED ON TOP

OR SIDE OF PLATES @ CONTRACTOR'S OPTION.

- SPLICE





STRAP HDR TO TRIMMER-W/ LSTA21, BOTH ENDS

4×12 DF#2

(2)2x6 TRIMMERS & (1) KING STUD, BOTH ENDS:

P.T. 6x6 HF#2 ATTACH W/ LCE4, BOTH ENDS

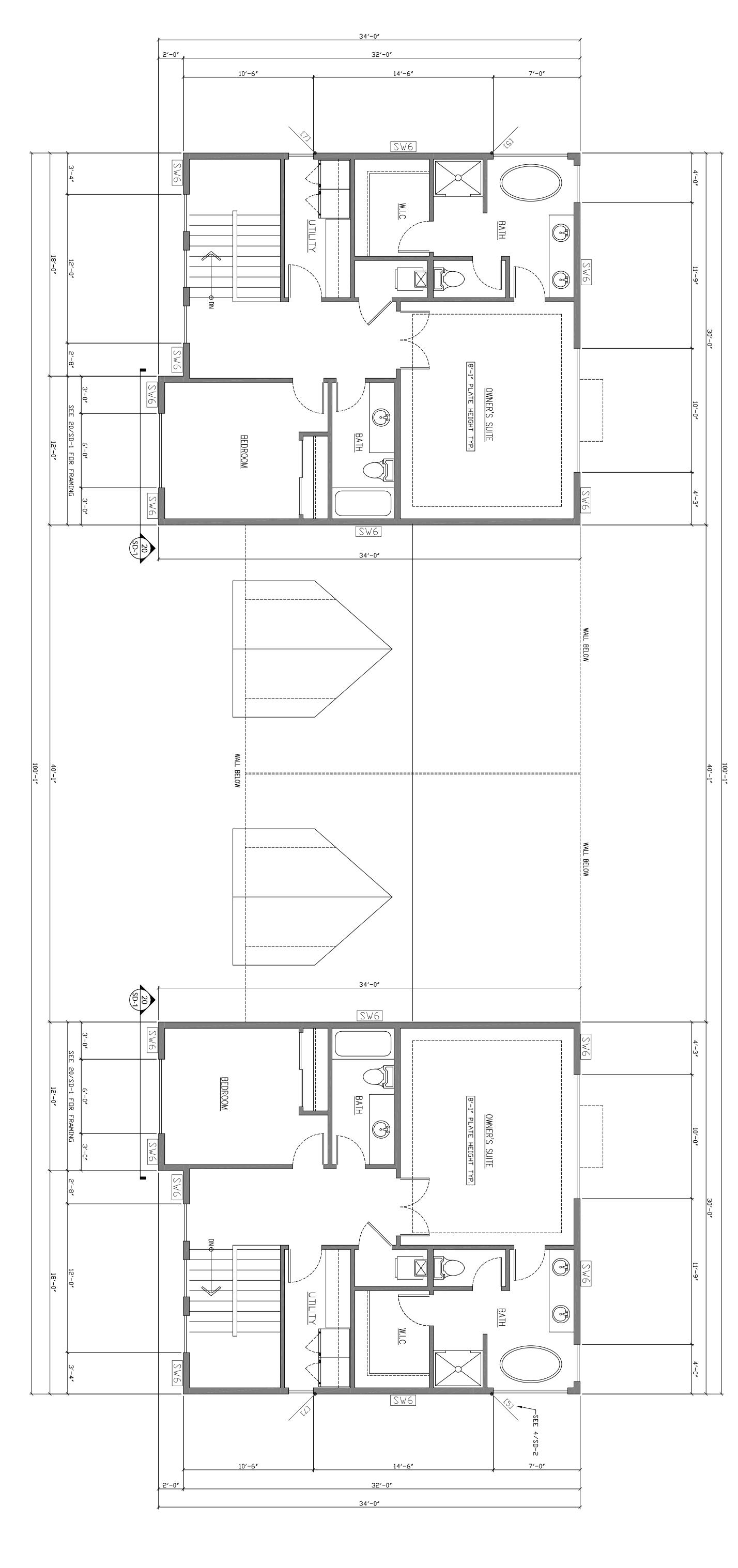
PRRNSF20230918

PHILLIPS STRUCTURAL ENGINEERING

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