GENERAL NOTES

BUILDING CODE
THE 2018 EDITION OF THE 'INTERNATIONAL EXISTING BUILDING CODE (IEBC), AS ADOPTED OR AMENDED BY THE CITY OF PUYALLUP, SHALL GOVERN DESIGN AND CONSTRUCTION.

SCOPE OF WORK
DESIGN OF THE WALL DOOR CUTOUTS ON THE WEST AND NORTH SIDES OF THE EXISTING BUILDING. THE SCOPE OF THIS PACKAGE IS NOT A SEISMIC UPGRADE OF THE ENTIRE BUILDING, OUR IS LIMITED TO REPLACING THE SEISMIC STRENGTH LOST BY WALL CUTOUTS. A FULL SEISMIC UPGRADE IS BEYOND THE SCOPE OF THIS TENANT IMPROVEMENT. THE WEST WALL REQUIRES ADDITIONAL SEISMIC CAPACITY AND A CONCRETE SHEARWALL WAS ADDED. THE NORTH WALL OF THE BUILDING IS APPROXIMATELY 190-FEET LONG AND DID NOT REQUIRE ADDITIONAL SEISMIC CAPACITY. ADDITIONAL OUT OF PLANE WALL SEISMIC BRACING WAS ADDED ALONG THE WEST WALL TO ACCOMMODATE THE ADDITIONAL OUT OF PLANE LOADS AT THE LOCATION OF THE NEW CONCRETE SHEAR WALL. NEW CUTOUTS WERE FRAMED WITH STRUCTURAL STEEL TO SUPPORT ROOF FRAMING AND COIL DOORS. THE 190-FOOT FAST-WEST LENGTH OF THE BUILDING SPANS NUMEROUS TENANTS AND THE TENANT IMPROVEMENT ASSOCIATED WITH THIS SUBMITTAL COVERS THE WESTERN 50-FEET OF THIS BUILDING. WE DID NOT OBSERVE THE INTERIOR FRAMING OF THE EASTERN 140-FEET OF THIS BUILDING, IT IS ASSUMED TO BE IN SIMILAR CONDITION TO THE WESTERN 50-FOOT OF THE BUILDING THAT WE DID OBSERVE.

ENGINEER
THE TERM 'ENGINEER', 'EOR', AND/OR 'SE' AS USED IN THESE STRUCTURAL DOCUMENTS SHALL MEAN BRIENEN STRUCTURAL ENGINEERS, P.S.

REFERENCE STANDARDS
ALL WORK SHALL CONFORM TO ALL REQUIREMENTS OF THE REFERENCE STANDARDS AND CODES INDICATED IN THE DRAWINGS UNLESS NOTED OTHERWISE. REFERENCE TO ASTM AND OTHER STANDARDS SHALL MEAN THE LATEST EDITION AS OF THE BID DATE OR DATE OF OWNER-CONTRACTOR AGREEMENT, WHICHEVER IS LATER, UNLESS NOTED IN THESE DOCUMENTS OR DESIGNATED BY THE BUILDING CODE.

PRIME CONTRACT DRAWINGS
THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. STRUCTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL, AND LANDSCAPING, AMONG OTHERS, ARE SUPPLEMENTARY TO THE ARCHITECTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS TO THE PRIME CONTRACT DRAWINGS, STRUCTURAL DRAWINGS, AND OTHER SUPPLEMENTARY DRAWINGS.

OMISSIONS/CONFLICTS
CONTRACTOR SHALL VERIFY ALL DIMENSIONS, FLOOR ELEVATIONS, DEPRESSIONS, FINISHES, STAIR DETAILS, GUARDRAILS, AND ETC. WITH OTHER DISCIPLINES INCLUDING ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE.

TYPICAL DETAILS
TYPICAL DETAILS SHOWN ON THE DRAWINGS SHALL APPLY UNLESS NOTED OTHERWISE. SOME TYPICAL DETAILS ARE CUT OR OTHERWISE REFERENCED IN THE DRAWINGS HOWEVER MOST OR NOT. WHERE TYPICAL DETAILS ARE NOTED ON THE DRAWINGS THE SPECIFIC DETAIL SHALL BE USED. WHERE NO DETAIL IS NOTED IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO CHOOSE THE APPROPRIATE DETAIL FROM THOSE PROVIDED.

SUBSTITUTIONS
THE CONTRACTOR SHALL SUBMIT ALL SUBSTITUTION REQUESTS (MATERIAL, PROCEDURE, CONFIGURATION, AND/OR DETAIL) TO THE ARCHITECT/ENGINEER PRIOR TO SHOP DRAWING PRODUCTION.

CONSTRUCTION MEANS AND METHODS AND SAFETY
CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS/METHODS AND FOR VERIFYING STRUCTURAL CAPACITY PRIOR TO APPLYING CONSTRUCTION LOADING. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY AT THE SITE AND FOR PROVIDING THE STRENGTH AND STABILITY OF ALL PARTIALLY COMPLETED STRUCTURE CONFORMING TO ASCE 37 'DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION'.

CONTRACTOR PROVIDED DESIGN SUBMITTALS
THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE ITEMS NOTED IN THE DEFERRED SUBMITTALS SECTION OF THE GENERAL NOTES.

STRUCTURAL OBSERVATION
THE ENGINEER WILL PERFORM PERIODIC OBSERVATION DURING CONSTRUCTION OF THE FOUNDATION AND STRUCTURAL FRAME AS REQUIRED BY THE BUILDING CODE.

SIGNS AND AWNINGS (PE)
SIGNS, AWNINGS, AND THEIR CONNECTIONS TO THE PRIMARY STRUCTURE SHALL BE DESIGNED BY THE CONTRACTOR'S LICENSED DESIGN PROFESSIONAL AND UTILIZE THE ATTACHMENT POINTS SHOWN IN THE STRUCTURAL DRAWINGS WITHOUT MODIFICATION.

GEOTECHNICAL
THE EXISTING STRUCTURE DOES NOT HAVE OBSERVABLE STRUCTURAL SETTLEMENTS, THUS IT IS OUR OPINION THERE WAS LIKELY A PROPERLY COMPACTED SUBBASE USED BELOW THE FOUNDATIONS. BASED ON THIS VISUAL OBSERVATION THE ASSUMED GEOTECHNICAL VALUES ARE AS FOLLOWS:

ALLOWABLE BEARING PRESSURE = 3000 PSF (DEAD + LIVE):
A 1/3 INCREASE IS ALLOWED FOR WIND OR SEISMIC

SUBMITTALS

SHOP DRAWINGS AND MATERIAL SUBMITTALS FOR THE ITEMS SHOWN BELOW SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION OR CONSTRUCTION. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWINGS SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER REVIEW, THE CONTRACT DOCUMENTS CONTROL AND SHALL BE FOLLOWED.

1. CONCRETE MIX DESIGNS
2. NON-SHRINK BASEPLATE GROUT MATERIAL SPECIFICATIONS
3. STRUCTURAL STEEL SHOP AND ERECTION DRAWINGS

DESIGN CRITERIA

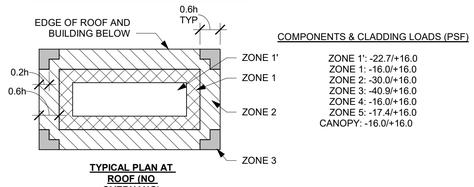
BUILDING CATEGORY
STRUCTURAL RISK CATEGORY II
IMPORTANCE FACTOR SNOW ---------- Is = 1.0
IMPORTANCE FACTOR SEISMIC -------- Ie = 1.0

GRAVITY LOADS
ROOF SNOW = 25PSF

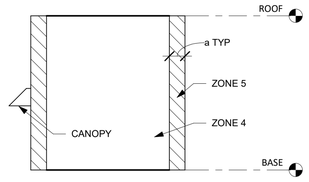
SEISMIC LOADS
SITE CLASS = D (DEFAULT)
SEISMIC DESIGN CATEGORY = D
MAPPED SPECTRAL RESPONSE PARAMETERS
Ss = 1.279 g; S1 = 0.44 g
Sds = 1.023 g; Sd1 = NA

WIND LOADS
EXPOSURE CATEGORY = B
BASIC WIND SPEED = 97 MPH
Kzt = 1.0

COMPONENT AND CLADDING WIND PRESSURE



TYPICAL PLAN AT ROOF (NO OVERHANG)



TYPICAL BUILDING ELEVATION

NOTES:
1. WIND LOADS FOR COMPONENT AND CLADDING ARE STRENGTH LEVEL AND DETERMINED IN ACCORDANCE WITH ASCE 7-16, CHAPTER 30, PART 1.
2. EXTERIOR COMPONENTS AND CLADDING SHALL BE DESIGNED TO ACCOMMODATE WORST-CASE WIND LOAD SHOWN.
3. POSITIVE PRESSURE ACTS TOWARDS THE SURFACE OF THE STRUCTURE. NEGATIVE PRESSURE ACTS OUTWARD AS SUCTION ON THE BUILDING SURFACE.
4. PRESSURE ARE CALCULATED USING MINIMUM EFFECTIVE AREA OF 10 sf. FOR ROOF AREAS GREATER THAN 10 sf EXCEPT AT OVERHANGS, NEGATIVE PRESSURE MAY BE REDUCED AS FOLLOWS:
   - 20 sf ≤ AREA < 50 sf — 5% REDUCTION
   - 50 sf ≤ AREA < 80 sf — 12% REDUCTION
   - 80 sf ≤ AREA < 200 sf — 16% REDUCTION
   - 200 sf ≤ AREA — 20% REDUCTION

   FOR ALL OVERHANGS, NO WIND LOAD MAY BE REDUCED.
   FOR WALL AREAS AND PARAPET AREAS GREATER THAN 10 sf, POSITIVE PRESSURE MAY BE REDUCED AS FOLLOW:
   - 20 sf ≤ AREA < 50 sf — 5% REDUCTION
   - 50 sf ≤ AREA < 80 sf — 12% REDUCTION
   - 80 sf ≤ AREA < 200 sf — 16% REDUCTION
   - 200 sf ≤ AREA — 20% REDUCTION

   FOR WALL AREAS AND PARAPET AREAS GREATER THAN 10 sf, NEGATIVE PRESSURE MAY BE REDUCED AS FOLLOW:
   - 20 sf ≤ AREA < 50 sf — 3% REDUCTION
   - 50 sf ≤ AREA < 80 sf — 8% REDUCTION
   - 80 sf ≤ AREA < 200 sf — 10% REDUCTION
   - 200 sf ≤ AREA — 15% REDUCTION
5. EDGE PRESSURE SHALL BE USED FOR A DISTANCE "a" FROM THE BUILDING CORNERS, WHERE "a" IS THE SMALLER OF 10% OF THE LEAST HORIZONTAL DIMENSION OR 0.4*h BUT NOT LESS THAN EITHER 4% OF THE LEAST HORIZONTAL DIMENSION OR 3'-0".
6. NET PRESSURE ON THE PARAPETS SHALL BE AS SHOWN ABOVE. ATTACHMENTS TO THE INTERIOR SIDE OF THE PARAPETS SHALL BE DESIGNED TO ACCOMODATE NEGATIVE PRESSURE OF THE ADJACENT ROOF.

MASONRY NOTES

REFERENCE STANDARDS

BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES - TMS 402/602 - 2016 EDITION
STANDARD SPECIFICATION FOR CONCRETE BILDING BRICK - ASTM C55 - 2017 EDITION
STANDARD SPECIFICATION FOR LOADBEARING CONCRETE MASONRY UNITS - ASTM C90 - 2021 EDITION
STANDARD SPECIFICATION FOR NONLOAD BEARING CONCRETE MASONRY UNITS - ASTM C129 - 2017 EDITION
STANDARD SPECIFICATION FOR CONCRETE FACING BRICK AND OTHER CONCRETE MASONRY FACING UNITS - ASTM C1637 - 2020 EDITION

MATERIAL CRITERIA

CONCRETE MASONRY UNITS (CMU)
CMU SHALL CONFORM TO ASTM C90 AND FOLLOW MINIMUM FACE SHELL THICKNESS REQUIREMENTS OF ASTM C90 SECTION 5.3.1. ALL CMU BLOCKS SHALL BE GRADE N, MEDIUM WEIGHT, WITH MINIMUM COMPRESSIVE STRENGTH TO MEET OR EXCEED f'm LISTED IN THE MASONRY ASSEMBLIES SECTION OF THESE GENERAL NOTES.

HOLLOW CLAY MASONRY UNITS
HOLLOW CLAY MASONRY UNITS SHALL BE SOLID SHELL HOLLOW BRICK UNITS THAT CONFORM TO ASTM C652 AND FOLLOW MINIMUM FACE SHELL AND WEB THICKNESS REQUIREMENTS OF ASTM C652 TABLE 1. ALL HOLLOW CLAY MASONRY UNITS SHALL BE GRADE SW, TYPE HBS, CLASS H60V BRICK, WITH MINIMUM COMPRESSIVE STRENGTH TO MEET OR EXCEED f'm LISTED IN THE MASONRY ASSEMBLIES SECTION OF THESE GENERAL NOTES.

MASONRY VENEER
MASONRY VENEER SHALL BE PER THE ARCHITECTURAL SPECIFICATIONS AND CONFORM TO THE REQUIREMENTS OF IBC CHAPTER 14. MASONRY OR STONE VENEER LAYERS SHALL BE NO GREATER THAN 5" THICK, MEASURED PERPENDICULAR TO THE SUPPORTING WALLS, UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS. REFER TO THE ANCHORED VENEER REQUIREMENTS OF THESE GENERAL NOTES FOR ANCHORAGE REQUIREMENTS.

MORTAR
MORTAR SHALL BE TYPE S PER THE IBC AND CONFORM TO ASTM C270, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.

GROUT
GROUT SHALL CONFORM TO ASTM C476 AND ACI 530. GROUT SHALL HAVE A FLUID CONSISTENCY WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH f'g=2000 PSI AT 28 DAYS.

MASONRY ASSEMBLIES
MASONRY ASSEMBLIES SHALL COMPLY WITH THE REQUIREMENTS OF IBC 2104, AND f'm SHOULD BE TESTED IN ACCORDANCE WITH THE REQUIREMENTS OF IBC 2105. THE MINIMUM MASONRY ASSEMBLY COMPRESSIVE STRENGTH SHALL BE:
1. CONCRETE MASONRY ASSEMBLIES: f'm=1900 PSI
2. HOLLOW CLAY MASONRY ASSEMBLIES: f'm=2500 PSI

ALL MASONRY ASSEMBLIES, EXCEPT VENEERS, SHALL BE SOLID-GROUTED, UNLESS NOTED OTHERWISE. ALL GROUT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION DURING PLACEMENT TO FILL THE GROUT SPACE, AND CARE SHALL BE TAKEN TO ENSURE THAT CONSOLIDATION OCCURS BEFORE THE LOSS OF PLASTICITY TO PREVENT VOIDS. GROUT POURS LESS THAN OR EQUAL TO 12" IN HEIGHT SHALL BE MECHANICALLY VIBRATED OR PUDDLED. GROUT POURS GREATER THAN 12" IN HEIGHT SHALL BE RECONSOLIDATED BY MECHANICAL VIBRATION 15 TO 20 MINUTES AFTER PLACEMENT TO MINIMIZE VOIDS DUE TO WATER LOSS. GROUT POURS IN EXCESS OF 48" IN HEIGHT ARE NOT PERMITTED WITHOUT PRIOR APPROVAL BY THE ENGINEER OF RECORD.

ALL MASONRY WORK SHALL BE COVERED AND PROTECTED FROM MOISTURE UNTIL THE ROOFING IS COMPLETE.

CONTRACTOR SHALL PROVIDE MASONRY WALL REINFORCING ELEVATION DRAWINGS OF EACH STRUCTURAL MASONRY WALL, INCLUDING REINFORCING PLACEMENT, LOCATIONS OF ARCHITECTURAL AND MECHANICAL OPENINGS, ALL SOUND BLOCK EXTENTS, LOCATIONS OF MECHANICAL, PLUMBING, AND ELECTRICAL CONDUIT IN THE WALLS, ETC. THIS SUBMITTAL SHALL BE REVIEWED AND APPROVED PRIOR TO THE START OF CONSTRUCTION.

ADHESIVE GROUT IN MASONRY

ADHESIVE GROUT IN SOLID-GROUTED MASONRY OR BRICK SHALL BE HILTI HY-200, SIMPSON SET, OR PRE-APPROVED EQUAL. ALL ANCHORS IN MASONRY WALL SHALL BE INSTALLED IN GROUTED CELLS.

TO INSTALL ADHESIVE GROUT, FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS, INCLUDING THE USE OF APPROVED EQUIPMENT OF MIX AND DISPENSE THE COMPONENTS AND STRICTLY ADHERING TO RECOMMENDED INSTALLATION TEMPERATURE AND TIME LIMITS. QUARTZ SAND AGGREGATE MAY BE PERMITTED, IF RECOMMENDED AND PROPORTIONED BY THE MANUFACTURER.

THE STRUCTURAL DESIGN OF ALL ADHESIVE ANCHORS ASSUMES FULL AND PROPER CLEAN OUT OF THE HOLES, INCLUDING A DRY HOLE, BRUSHED WITH A WIRE BRUSH, AND CLEANED WITH PRESSURIZED AIR, JUST PRIOR TO INSTALLING GROUT. FAILURE TO CLEAN THE HOLE, EVEN IF PERMITTED BY THE MANUFACTURER, WILL RESULT IN REJECTION OF THE ANCHOR.

STEEL NOTES

REFERENCE STANDARDS

AISC STEEL CONSTRUCTION MANUAL - 14TH EDITION - ANSI/AISC 360-16
AISC SEISMIC DESIGN MANUAL - 2ND EDITION - ANSI/AISC 341-16
AISC CODE OF STANDARD PRACTICE - AISC 303-16
RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS - AISC 348-14
AWS D1.1 STRUCTURAL WELDING CODE
AWS D1.4 REINFORCING STEEL
AWS D1.8 SEISMIC SUPPLEMENT

MATERIAL CRITERIA

STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS UNLESS NOTED OTHERWISE ON PLANS:

STRUCTURAL STEEL MEMBERS	
WIDE FLANGE (W) AND (WT) SHAPES	ASTM A992, Fy = 50 KSI
PLATES, ANGLES AND CHANNELS UNO	ASTM A36, Fy = 36 KSI
TUBE STEEL (HSS)	ASTM A500, GRADE C, Fy = 50 KSI
PIPE 12" OR LESS IN DIAMETER	ASTM A53, GRADE B, TYPE E OR S, FY = 35...
WELDING	
WELDING ELECTRODES	AWS D1.1 E70XX Fy = 70 KSI, AWS D1.1 E71TXX Fy = 70 KSI (AS REQUIRED BY WELDING PROCESS)

GENERAL REQUIREMENTS

ERECTORS
ALL STEEL ERECTION SHALL BE PERFORMED BY AN AISC CERTIFIED STEEL ERECTOR, CATEGORY CSE. THE CONTRACTOR SHALL PROVIDE AND COORDINATE ALL ERECTION AIDS.

SHOP DRAWINGS
ALL WORK SHALL CONFORM TO THE AISC SPECIFICATIONS. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER, AND BE APPROVED, PRIOR TO COMMENCING FABRICATION. ALL STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AND/OR GRID LINES UNLESS NOTED OTHERWISE. THE CONTRACTOR SHALL COORDINATE THE USE OF TYPICAL DETAILS AND THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS.

ARCHITECTURAL EXPOSED STRUCTURAL STEEL (AESS)
ALL MEMBERS DESIGNATED AS (AESS) OR SHOWN AS EXPOSED STEEL ON THE ARCHITECTURAL DRAWINGS SHALL CONFORM TO SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE. FIELD AND SHOP WELDS SHALL BE FULLY BROUGHT BACK UP TO AISC SPECIFICATIONS PRIOR TO PAINTING OR TOUCH-UP PAINTING.

EXPOSED STRUCTURAL STEEL
ALL MEMBERS EXPOSED TO EARTH OR WEATHER AND NOT DESIGNATED BY THE STRUCTURAL OR ARCHITECTURAL DRAWINGS AS (AESS) SHALL BE HOT DIPPED GALVANIZED AND CONFORM TO ASTM A123, A384, AND A385. FIELD AND SHOP WELDS SHALL BE FULLY BROUGHT BACK UP TO AISC SPECIFICATIONS PRIOR TO GALVANIZING OR TOUCH-UP GALVANIZING.

NON-EXPOSED STRUCTURAL STEEL
STRUCTURAL STEEL THAT IS NOT EXPOSED TO EARTH OR WEATHER OR DESIGNATED AS (AESS) SHALL BE UNPAINTED, CLEAN OF LOOSE RUST, LOOSE MILL SCALE, GREASE, OIL, AND MEET THE REQUIREMENTS OF SSPC-SP1.

WELDING

ALL STRUCTURAL STEEL WELDING SHALL CONFORM TO AWS D1.1.
ALL REINFORCING STEEL WELDING SHALL CONFORM TO AWS D1.4.
ALL WELDING OF THE (SLRS) SYSTEM SHALL ALSO CONFORM TO AISC 341 AND AWS D1.8.

FIELD WELD SYMBOLS
NOT ALL FIELD WELDS ARE INDICATED WITH A FIELD WELD SYMBOL. THE CONTRACTOR SHALL COORDINATE ALL FIELD WELDS BETWEEN THE FABRICATOR AND ERECTOR AND PROPERLY IDENTIFY SUCH WELDS AS SUCH ON THE SHOP DRAWINGS.

WELDER CERTIFICATION
ALL WELDING SHALL BE PERFORMED BY AWS/WABO CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1.

WELD FILLER MATERIAL
ALL WELD FILLER METAL INCLUDING SHALL BE COMPATIBLE. ALL WELD FILLER METAL AND PROCESS SHALL PROVIDE TENSILE STRENGTH CHARPY V-NOTCH RATING AS FOLLOWS:

GRAVITY MEMBERS		
WELD TYPE	**FILLER MATERIAL**	**CVN RATING**
FILLET	70 KSI	NONE
PARTIAL PENETRATION	70 KSI	NONE
COMPLETE PENETRATION	70 KSI	20 FT-LBS AT -20°F

INSPECTION
ALL WELDS SHALL BE INSPECTED AS REQUIRED IN THE "STATEMENT OF SPECIAL INSPECTION" OF THESE GENERAL NOTES. ALL FULL PENETRATION WELDS SHALL BE ULTRASONIC TESTED EXCEPT PLATE LESS THAN 1/4-INCH SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE AND APPROVAL OF THE ENGINEER AND BUILDING OFFICIAL.

City of Puyallup
Building
ACCEPTED
JMontgomery
04/06/2023
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THE APPROVED CONSTRUCTION PLANS, DOCUMENTS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

FULL SIZED LEDGIBLE COLOR PLANS ARE REQUIRED TO BE PROVIDED BY THE PERMITEE ON SITE FOR INSPECTION

City of Puyallup
Development & Permitting Services
ISSUED PERMIT
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PRCTI20230247

BSE
Brienen
Structural
Engineers
1316 Central Ave. S., Suite 200
Kent, WA 98032
(206) 397-0000 ~ www.bse-ps.com

PAUL J. BRIENEN — STATE OF WASHINGTON — STRUCTURAL ENGINEER — PROFESSIONAL ENGINEER

ENTERPRISE RENT-A-CAR TENANT IMPROVEMENT
733 RIVER ROAD
PUYALLUP, WA 98371

ISSUE	DATE
PERMIT SET	2/17/2023

Job # 22460
Drawn Author
Checked Checker

GENERAL NOTES

S1.0

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PROVIDE ALL SPECIAL INSPECTION REPORTS TO INSPECTOR.

STATEMENT OF SPECIAL INSPECTIONS

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED PER THE REQUIREMENTS OF IBC SECTION 1704 AND AS NOTED HEREIN.

CONCRETE

VERIFICATION AND INSPECTION	C	P	REFERENCED STANDARD	NOTES
REINFORCING STEEL AND PLACEMENT		X	ACI 318: 20.2, 25.2, 25.3 IBC 1908.4	SPECIAL INSPECTION SHALL CONFORM TO ACI 26.13 UNO SPECIAL INSPECTIONS NOT REQUIRED FOR THE FOLLOWING CONDITIONS: NON-STRUCTURAL SLAB ON GRADE CONCRETE FOUNDATION WALLS WITH F'c ≤ 2500 PSI ISOLATED SPREAD FOOTINGS FOR BUILDINGS THREE-STORIES AND LESS ABOVE GRADE PLANE CONTINUOUS FOOTINGS SUPPORTING WALLS OF THREE-STORIES AND LESS ABOVE GRADE PLANE WHERE WALLS ARE LIGHT-FRAME CONSTRUCTION AND STRUCTURAL DESIGN IS BASED ON F'c ≤ 2500 PSI
ANCHORS CAST IN CONCRETE		X	ACI 318: 26.7	
VERIFY USE OF REQUIRED DESIGN MIX		X	ACI 318: 4, 26.4.1 IBC 1908.2, 1908.3	
SAMPLING OF FRESH CONCRETE, SLUMP TEST, AIR CONTENT, TEMPERATURE OF CONCRETE AT TIME OF MAKING SPECIMENS	X		ACI 318: 26.12 ASTM C 172, C 31 IBC 1908.10	
CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION	X		ACI 318: 26.5.1, 26.5.2 IBC 1908.6, 1908.7, 1908.8	
MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X	ACI 318: 26.5.3 IBC 1908.9	
INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAIN TENSION LOADS. B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN A.	X X		ACI 318: 26.7	
ERECTION OF PRECAST CONCRETE		X	ACI 318: 26.9	
APPLICATION OF PRESTRESSING FORCES	X		ACI 318: 26.10.2 IBC 1908.4	
VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS AND PRIOR TO STRESSING OF TENDONS		X	ACI 318: 26.11.2.1	
INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X	ACI 318: 26.11.1.2	
MATERIAL VERIFICATION OF REINFORCEMENT STEEL FOR ASTM A615 REINFORCING		X	ACI 20.2.2.5 (b)	MANUFACTURER SHALL PROVIDE MILL TEST REPORTS

STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	C	P	REFERENCED STANDARD	NOTES
MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS		X	AISC 360 N5.6	
HIGH-STRENGTH BOLTING A. SNUG-TIGHT JOINTS B. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST OFF BOLTS OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION		X X	AISC 360 N5.2 AISC 341 J7	
MATERIAL VERIFICATION OF STRUCTURAL STEEL A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360 B. MANUFACTURER'S CERTIFIED MILL TEST REPORTS		X X	AISC 360 N5 AISC 341 J6	MANUFACTURER TO PROVIDE CERTIFIED MILL TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER MATERIALS A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATIONS LISTED IN GENERAL NOTES B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE		X X	AISC 360 N5	MANUFACTURER TO PROVIDE CERTIFICATE OF COMPLIANCE
INSPECTION OF WELDING A. COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS B. MULTI-PASS FILLET WELDS C. SINGLE-PASS FILLET WELDS > 5/16" D. PLUG AND SLOT WELDS E. SINGLE-PASS FILLET WELDS ≤ 5/16" F. FIELD-INSTALLED WELDED STUDS G. WELDING OF STAIRS AND RAILING SYSTEMS	X X X X	 X X X	AISC 360 N5.4, N5.5 AISC 341 CHP J6 AWS D1.1	SPECIAL INSPECTIONS IN THIS SECTION ARE WAIVED WHERE FABRICATION IS PERFORMED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED IN ACCORDANCE WITH IBC SECTION 1704.2
INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS		X	AISC 360 N5.7	
INSPECTION OF COMPOSITE CONSTRUCTION, INCLUDING PLACEMENT OF STEEL DECK AND STEEL HEADED STUD ANCHORS			AISC 360 N5.6 AWS D1.3	

MASONRY

VERIFICATION AND INSPECTION	C	P	REFERENCED STANDARD	NOTES
VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS		X	ACI 530.1: 1.5	
VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
A. PROPORTIONS OF SITE-PREPARED MORTAR AND GROUT		X	ACI 530.1: 2.1, 2.6A, 2.6B, 2.6C, 2.4G.1.b	
B. GRADE TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS		X	ACI 530: 1.16 ACI 530.1: 2.4, 3.4	
C. PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS		X	ACI 530.1: 3.3B	
D. PLACEMENT OF REINFORCEMENT AND CONNECTORS	X		ACI 530: 1.16 ACI 530.1: 3.2E, 3.4, 3.0A	
E. GROUT SPACE PRIOR TO GROUTING	X		ACI 530.1: 3.2D, 3.2F	
F. PLACEMENT OF GROUT	X		ACI 530.1: 3.5, 3.6C	
G. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X	ACI 530.1: 3.3F	
H. TYPE, SIZE, AND LOCATION OF ANCHORS INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	X		ACI 530: 1.16, 4.3, 1.17.1	
I. WELDING OF REINFORCEMENT	X		ACI 530: 2.1.7.7.2, 3.3.3.4(c), 8.3.3.4(b)	
J. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMP. BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMP. ABOVE 90°F (32.2°C))		X	ACI 530.1: 1.8C, 1.8D	
OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	X		ACI 530.1: 1.4B.2.a.3, 1.4B.2.b.3, 1.4B.2.c.3, 1.4B.3, 1.4B.4	
ANCHORS POST-INSTALLED IN MASONRY	X		MFR EVAL REPORT AND MFR PUBLISHED INSTALLATION INSTRUCTIONS	SEE CONCRETE FOR INSPECTION CRITERIA

SPECIAL INSPECTION OF ELECTRICAL AND MECHANICAL COMPONENTS PER IBC 1705.12.6 WHERE APPLICABLE.

"**C**" DENOTES CONTINUOUS INSPECTION
"**P**" DENOTES PERIODIC INSPECTION

TESTING AND SPECIAL INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A DAILY BASIS WHENEVER WORK IS PERFORMED ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO OWNER, CONTRACTOR, BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER.

STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNATED REPRESENTATIVE IN ACCORDANCE WITH IBC 1704.6.
STRUCTURAL OBSERVATION SHALL BE PERFORMED AS FOLLOWS:

- PERIODIC VISUAL OBSERVATION OF STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES.
- REVIEW OF TESTING AND INSPECTION REPORTS.
- REPORTS SHALL BE PREPARED FOR EACH SITE VISIT AND SHALL BE DISTRIBUTED TO ARCHITECT.

GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL INCLUDE ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

CONCRETE NOTES

REFERENCE STANDARDS

ACI 117 STANDARD SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION
ACI 301 SPECIFICATIONS FOR STRUCTURAL CONCRETE
ACI 304 GUIDE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE
ACI 305.1 SPECIFICATIONS FOR HOT WEATHER CONCRETING
ACI 306.1 STANDARD SPECIFICATIONS FOR COLD WEATHER CONCRETING
ACI 308.1 SPECIFICATIONS FOR CURING CONCRETE
ACI 309 GUIDE FOR CONSOLIDATION OF CONCRETE
ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
ACI 347 GUIDE TO FORMWORK FOR CONCRETE
ACI 506.2 SPECIFICATION FOR SHOTCRETING
ACI SP-15 FIELD REFERENCE MANUAL
ACI SP-66 ACI DETAILING MANUAL

MATERIAL CRITERIA

MATERIAL SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS UNLESS NOTED OTHERWISE ON PLANS:

CONCRETE MIX MATERIALS	
CEMENT	ASTM C150, C595
AGGREGATES	ASTM C33 (SEE NOTE 1)
ADMIXTURES	ASTM C260, C494, C1017
WATER	ASTM C94
SLAG	ASTM C989
FLY ASH	ASTM C618, CLASS F OR C (SEE NOTE 2)
REINFORCING STEEL	
REINFORCING STEEL UNO	ASTM A706, Fy = 60KSI, SEE NOTE 1

NOTES:
1. ASTM GRADE 60 MAY BE SUBSTITUTED IF THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED THE SPECIFIED YIELD STRENGTH BY MORE THAN 18 KSI AND THE RATIO OF ACTUAL ULTIMATE TENSILE STRENGTH TO ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25.

MIX DESIGNS

MIX TABLE
MIXES SHALL MEET OR EXCEED EACH REQUIREMENT SPECIFIED IN THE CONCRETE MIX DESIGN TABLE.

MIX DESIGN SUBMITTALS
SUBMIT A DESIGN FOR EACH CONCRETE MIX TO THE ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION. CONFORM TO IBC SECTION 1905. DATA SUBMITTED FOR REVIEW SHALL ACCURATELY REFLECT PROPOSED MIX DESIGN.

WATER/CEMENTITIOUS RATIO
W/C RATIO SHALL BE BASED ON TOTAL CEMENTITIOUS MATERIAL, INCLUDING CEMENT AND POZZOLANS SUCH AS FLY ASH, SILICA FUME, AND BLAST FURNACE SLAG.

CONCRETE MIX DESIGN TABLE					
LOCATION	MAX W/C RATIO	MIN PCY FLYASH OR SLAG	ASTM AGGREGATE GRADING	f'c DESIGN STRENGTH (PSI)	MIX NOTES
WALL	0.50	100	57 OR 67	3000 @ 28 DAYS	-
SLAB	0.40	100	57 OR 67	2500 @ 28 DAYS	1

NOTES:
1. MIX SHALL REACH F'C=5000 FOR DURABILITY, INCLUDE 5% AIR ENTRAINMENT AND FIBEROUS REINFORCMENT, STRENGTH TESTING NOT REQUIRED.

1

POST-INSTALLED ANCHORS

GENERAL
POST-INSTALLED ANCHORS SHALL NOT BE INSTALLED WITHOUT PRIOR APPROVAL OF STRUCTURAL ENGINEER OF RECORD UNLESS SPECIFIED ON THE STRUCTURAL DRAWINGS. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS.

ADHESIVE ANCHORS
ADHESIVE FOR ANCHORS INSTALLED INTO CONCRETE (THREADED RODS OR REINFORCING BARS) SHALL BE "HILTI HIT-RE 500 v3", INSTALLED PER ICC REPORT NUMBER ESR-3814 OR "SIMPSON AT-XP", INSTALLED PER UES REPORT NUMBER 263.

EXPANSION ANCHORS
EXPANSION ANCHORS INTO CONCRETE AND MASONRY UNITS SHALL BE "HILTI KWIK BOLT TZ 2", INSTALLED PER ICC REPORT NUMBER ESR-4266 OR "SIMPSON STRONG BOLT 2" INSTALLED PER ICC REPORT NUMBER ESR-3037.

MISCELLANEOUS

GROUT
USE NON-SHRINK GROUT WITH MINIMUM COMPRESSIVE STRENGTH OF 6000 PSI AT 28 DAYS UNLESS NOTED OTHERWISE. FOR STRUCTURAL STEEL COLUMN BASE PLATE GROUT SEE THE STRUCTURAL STEEL SECTION OF THE GENERAL NOTES

COLD FORMED STEEL NOTES

REFERENCE STANDARDS

NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS - AISI S100-16
NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS - AISI S200-12
NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - FLOOR AND ROOF SYSTEM DESIGN - AISI S210-12
NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - WALL STUD DESIGN - S211-12
NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - HEADER DESIGN - AISI S212-12
NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - LATERAL DESIGN - AISI S213-12
NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL STRUCTURAL FRAMING - AISI S240-1

MATERIAL CRITERIA

MATERIAL
COLD-FORMED STEEL MATERIAL SHALL BE MANUFACTURED AND FORMED, PER ASTM A1003/A1003M, FROM GALVANIZED ASTM A653 SS GRADE 50 STEEL FOR 54, 68 AND 97 MIL BASE THICKNESS MATERIAL AND FROM GALVANIZED ASTM A653 SS GRADE 33 MATERIAL FOR 43 AND 33 MIL BASE THICKNESS MATERIAL, UNO. WHERE NOTED, PAINTED COLD-FORMED STEEL MATERIAL SHALL CONFORM TO ASTM A570 SS GRADE 80. MINIMUM COLD-FORMED STEEL ACCEPTANCE CRITERIA SHALL BE PER ICC-ES AC46.

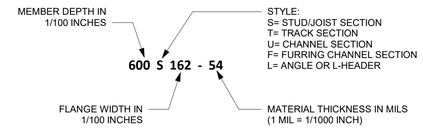
ALL GALVANIZED MEMBERS SHALL CONFORM TO ASTM A924 WITH THE FOLLOWING MINIMUM COATING REQUIREMENTS: NON-STRUCTURAL MEMBERS (ASTM C545 - G40 COATING), STRUCTURAL MEMBERS (ASTM C955 - G60 COATING), EXPOSED EXTERIOR MEMBERS (ASTM C955 - G90 COATING).

EACH MEMBER SHALL BEAR A LEGIBLE STICKER, STAMP, STENCIL, OR EMBOSSMENT, SPACED A MAXIMUM OF 48"OC ON THE WEB OF THE FRAMING MEMBER, INDICATING THE MINIMUM STEEL SHEET THICKNESS, METALLIC-COATING DESIGNATION, MINIMUM YIELD STRENGTH, PRODUCT DESIGNATION, AND NAME OF MANUFACTURER. WHERE MEMBERS ARE NOT LABELED, THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THAT THE STEEL IS IN COMPLIANCE WITH THE PROJECT SPECIFICATIONS.

SUBMITTALS
PRODUCT DATA FOR ALL MEMBERS, ACCESSORIES, AND FASTENERS SHALL BE SUBMITTED TO THE ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING DEPARTMENT FOR REVIEW AND ACCEPTANCE PRIOR TO FABRICATION AND ERECTION. FRAMING SUBSTITUTIONS SHALL BE SUBJECT TO REVIEW AND ACCEPTANCE BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION AND INSTALLATION.

COLD-FORMED STEEL FRAMING
ALL COLD-FORMED STEEL FRAMING SHALL BE IN ACCORDANCE WITH AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", AS AMENDED BY THE INTERNATIONAL BUILDING CODE AND SHALL STRICTLY CONFORM WITH ICC REPORT ER-4943P.

ALL COLD-FORMED STEEL PRODUCTS SHALL BE MANUFACTURED BY CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA). MATERIAL DESIGNATIONS NOTED ON THE DRAWINGS, RELATING TO MEMBER TYPES AND SIZES OR MISCELLANEOUS FRAMING ITEMS, REFER TO PRODUCT IDENTIFICATION STANDARDS ADOPTED BY THE SSMA. SSMA PRODUCTS HAVE A FOUR PART IDENTIFICATION CODE AS INDICATED IN THE FOLLOWING EXAMPLE:



INSTALLATION
EACH JOIST, RAFTER, TRUSS AND STRUCTURAL WALL STUDS SHALL BE ALIGNED WITHIN 3/4 FROM CENTERLINE OF HORIZONTAL FRAMING MEMBER TO CENTERLINE OF VERTICAL FRAMING MEMBER, UNO, OR AS SPECIFIED IN FIGURE C1-1 OF THE AISI STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS". STRUCTURAL FRAMING MEMBERS SHALL BE INSTALLED PER ASTM C1007 AND NON-STRUCTURAL FRAMING MEMBERS PER ASTM C754.

CONCRETE BEARING SURFACES AT STRUCTURAL FRAMING SHALL PROVIDE A UNIFORM BEARING SURFACE WITH A MAXIMUM 1/4" GAP BETWEEN THE TRACK AND THE CONCRETE. STEEL BEARING SHIMS OR NON-SHRINK GROUT CAN BE USED TO ACHIEVE THIS REQUIREMENT. THE BOTTOM TRACK OF LOAD BEARING WALLS SHALL NOT EXTEND OVER THE EDGE OF FORMED CONCRETE BEARING SURFACES BELOW.

COLD-FORMED STEEL SHALL NOT BE IN DIRECT CONTACT WITH THE GROUND UNLESS NOTED OTHERWISE.

COLD-FORMED STEEL CONNECTIONS

SCREWS
FOR STEEL-TO-STEEL CONNECTIONS AND FOR STRUCTURAL SHEATHING-TO-STEEL CONNECTIONS SHALL BE SELF TAPPING, SELF DRILLING FASTENERS IN COMPLIANCE WITH ASTM C1513 AND SHALL HAVE A TYPE II COATING IN ACCORDANCE WITH ASTM B633 "ELECTRO-DEPOSITED COATING OF ZINC ON IRON AND STEEL". SELF-PIERCING SCREWS PER ASTM C1002 ARE PERMITTED FOR CONNECTION OF 33 MILS STEEL OR THINNER. THE SCREW MANUFACTURER SHALL PROVIDE VERIFICATION OF THE FASTENERS' RESISTANCE TO HYDROGEN EMBRITTLEMENT. SCREWS SHALL CONFORM TO SAEJ78 "STANDARD SPECIFICATION FOR SELF-DRILL TAPPING SCREWS". SCREW ACCEPTANCE SHALL BE BASED ON ICC-ES AC118 "ACCEPTANCE CRITERIA FOR TAPPING SCREW FASTENERS".

SCREW CONNECTIONS SHALL BE IN COMPLIANCE WITH THE AISI STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS, 2007 EDITION". SCREW CONNECTIONS SHALL BE MADE FROM THE LIGHTER MATERIAL INTO THE THICKER MATERIAL, UNO. SCREWS SHALL EXTEND THROUGH THE STEEL CONNECTION A MINIMUM OF THREE EXPOSED THREADS AND SHALL HAVE MINIMUM CENTER-TO-CENTER SPACING AND EDGE DISTANCES OF THREE TIMES THE NOMINAL SCREW DIAMETER. SCREWS SHALL BE INSTALLED AND TIGHTENED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SHALL NOT CAUSE PERMANENT SEPARATION BETWEEN COMPONENTS. SHEATHING FASTENERS SHALL MAINTAIN A MINIMUM 3/8" EDGE DISTANCE IN SHEATHING AND SHALL HAVE THEIR HEADS FLUSH WITH THE SHEATHING (BUT NO MORE THAN 1/16" BELOW THE SURFACE OF THE SHEATHING).

STRIPPED SCREWS IN DIRECT TENSION SHALL BE CONSIDERED INEFFECTIVE AND SHALL BE REPLACED. STRIPPED SCREWS IN SHEAR THAT CONSTITUTE MORE THAN 25% OF THE TOTAL SCREWS IN THE CONNECTION SHALL BE CONSIDERED INEFFECTIVE AND SHALL BE REPLACED. STRIPPED SCREWS ARE PERMITTED TO BE REMOVED AND REPLACED WITH SCREWS OF THE NEXT LARGER DIAMETER.

MINIMUM SCREW SIZES IN COLD-FORMED STEEL TABLE	
CONNECTION	MINIMUM SCREW SIZE
METAL TO METAL (68 MILS)	#10-16 (#3 POINT)
METAL TO METAL (33 MILS - 54 MILS)	#8-18 (#2 POINT)
METAL TO METAL (SHEAR WALLS)	#8-18 (#2) POINT) WAFER HEAD
APA SHEATHING (SHEAR WALLS)	#8-18 (#2 POINT FLAT HEAD w/ 0.292"Ø HEAD MIN
GWB OR GYPSUM SHEATHING	#6 x 1" (#2 POINT) DRYWALL
METAL DECK TO FRAMING	#12-14 (#3 POINT)
SIMPSON HARDWARE	PER SIMPSON CATALOG

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ENTERPRISE RENT-A-CAR TENANT IMPROVEMENT
733 RIVER ROAD
PUYALLUP, WA 98371

ISSUE	DATE
PERMIT SET	2/17/2023
CONSTRUCTION DRAWINGS	7/17/2023

Job #	22460
Drawn	Author
Checked	Checker

GENERAL NOTES - INSPECTIONS

S1.1

City of Puyallup
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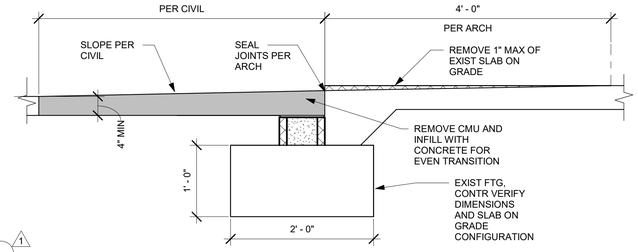
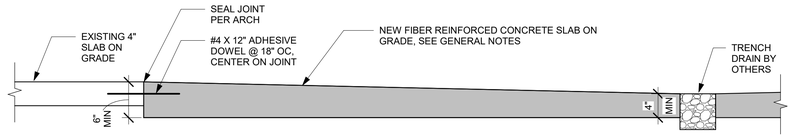
PROVIDE SEALED COPY FROM ENGINEER OF RECORD TO INSPECTOR FOR ALL MODIFICATIONS

NOTES:

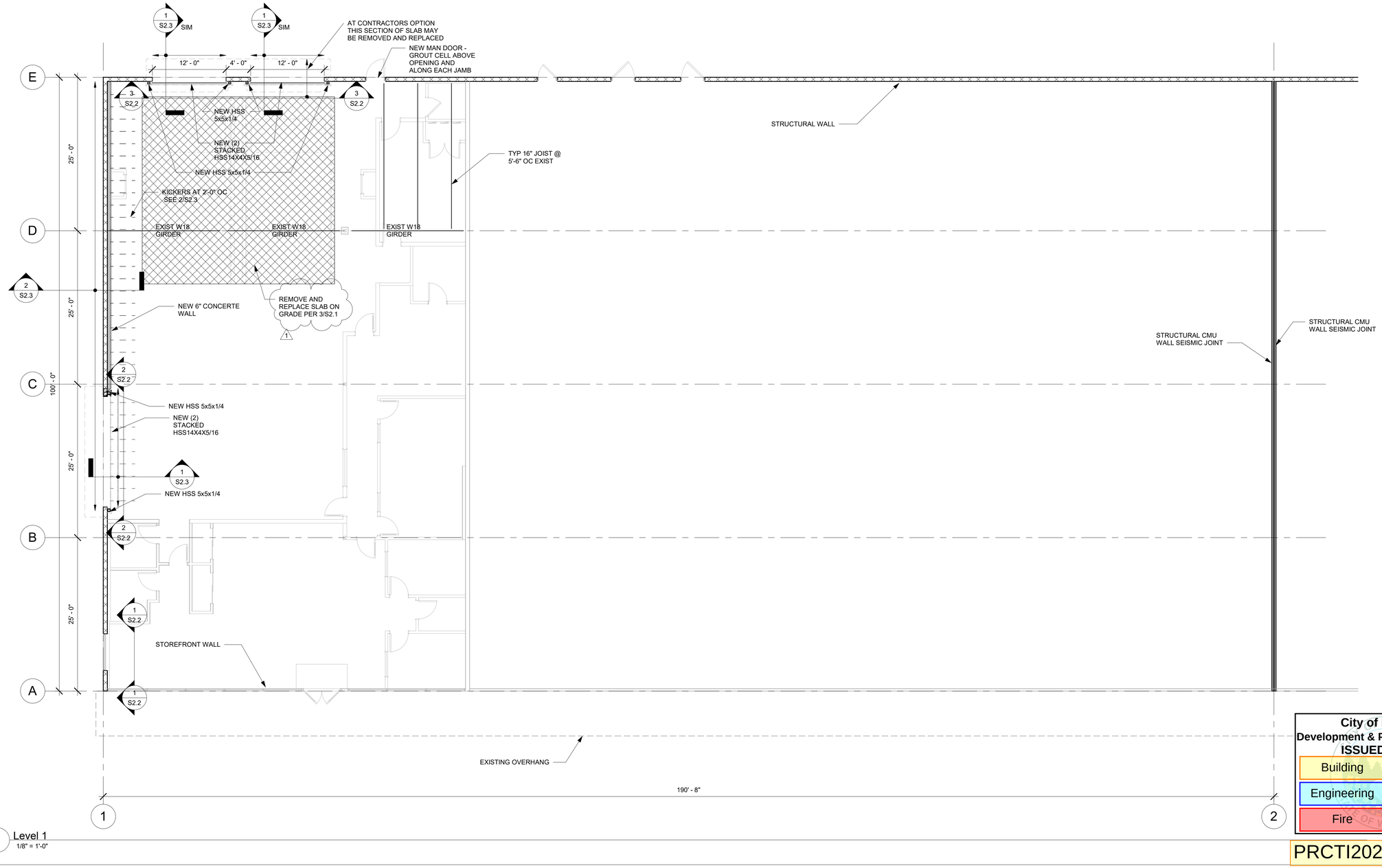
1. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS.
2. NOTIFY ENGINEER OF ANY EXISTING BUILDING STRUCTURAL CONCERNS OR DEFECTS DISCOVERED DURING CONSTRUCTION.
3. TEMPORARY SHORING AND SUPPORT OF EXISTING STRUCTURE IS THE CONTRACTORS RESPONSIBILITY AND NOT REPRESENTED IN THESE DOCUMENTS.
4. CONTRACTOR IS TO VERIFY EXISTING ROOF METAL DECK TO CMU WALL CONNECTION AND SUBMIT AN AS-BUILT SKETCH OF THIS CONDITION TO ENGINEER FOR REVIEW.



2 SECTION AT COILING DOOR ENTRANCE SLOPE
1" = 1'-0"



3 SECTION AT CARWASH SLAB
1" = 1'-0"



1 Level 1
1/8" = 1'-0"

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STRUCTURAL PLAN

S2.1

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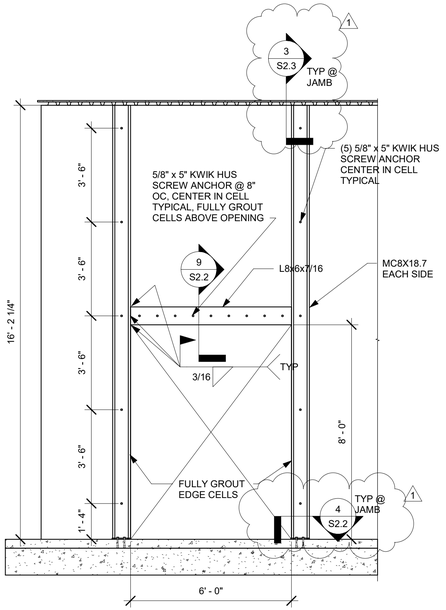
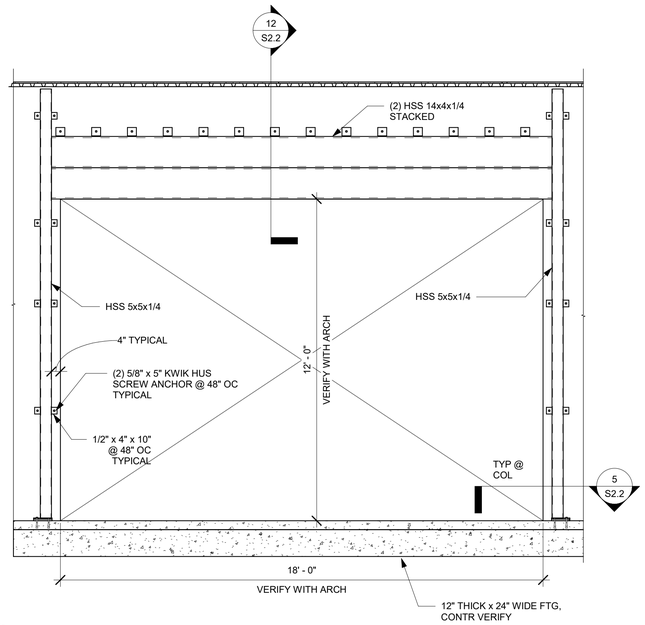
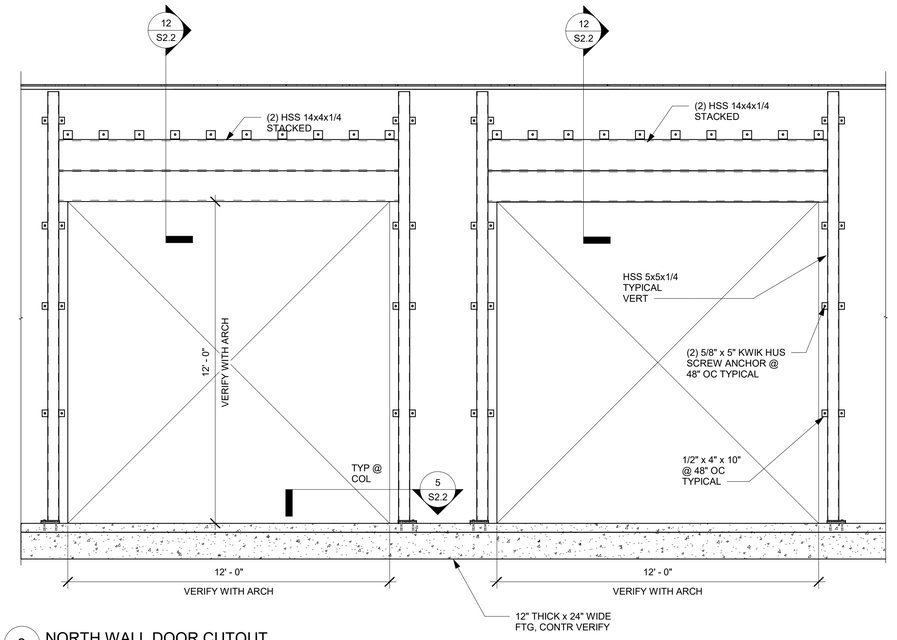


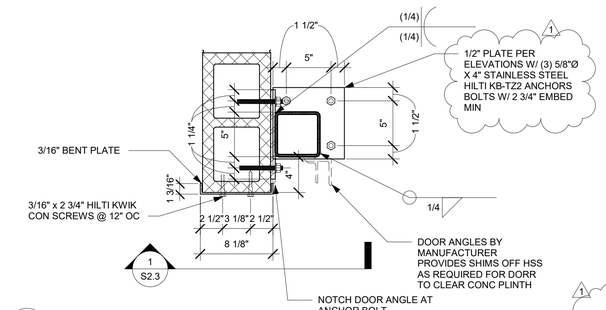
1 WEST WALL STOREFRONT CUTOUT
3/8" = 1'-0"



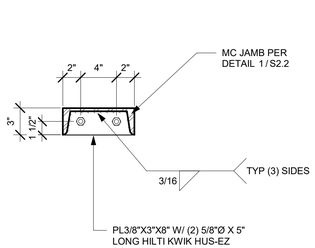
2 WEST WALL DOOR CUTOUT
3/8" = 1'-0"



3 NORTH WALL DOOR CUTOUT
3/8" = 1'-0"



5 SECTION
1 1/2" = 1'-0"



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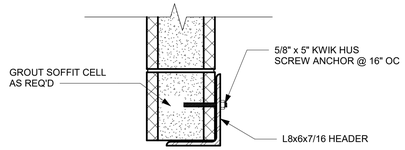

9 SECTION
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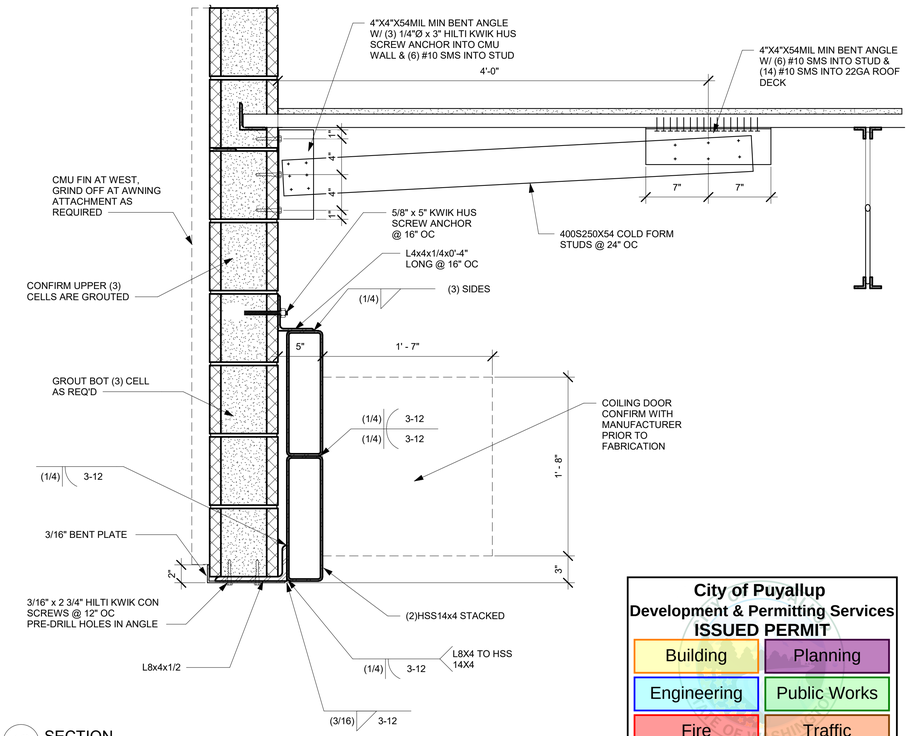

12 SECTION
1 1/2" = 1'-0"

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ELEVATIONS AND DETAILS

S2.2

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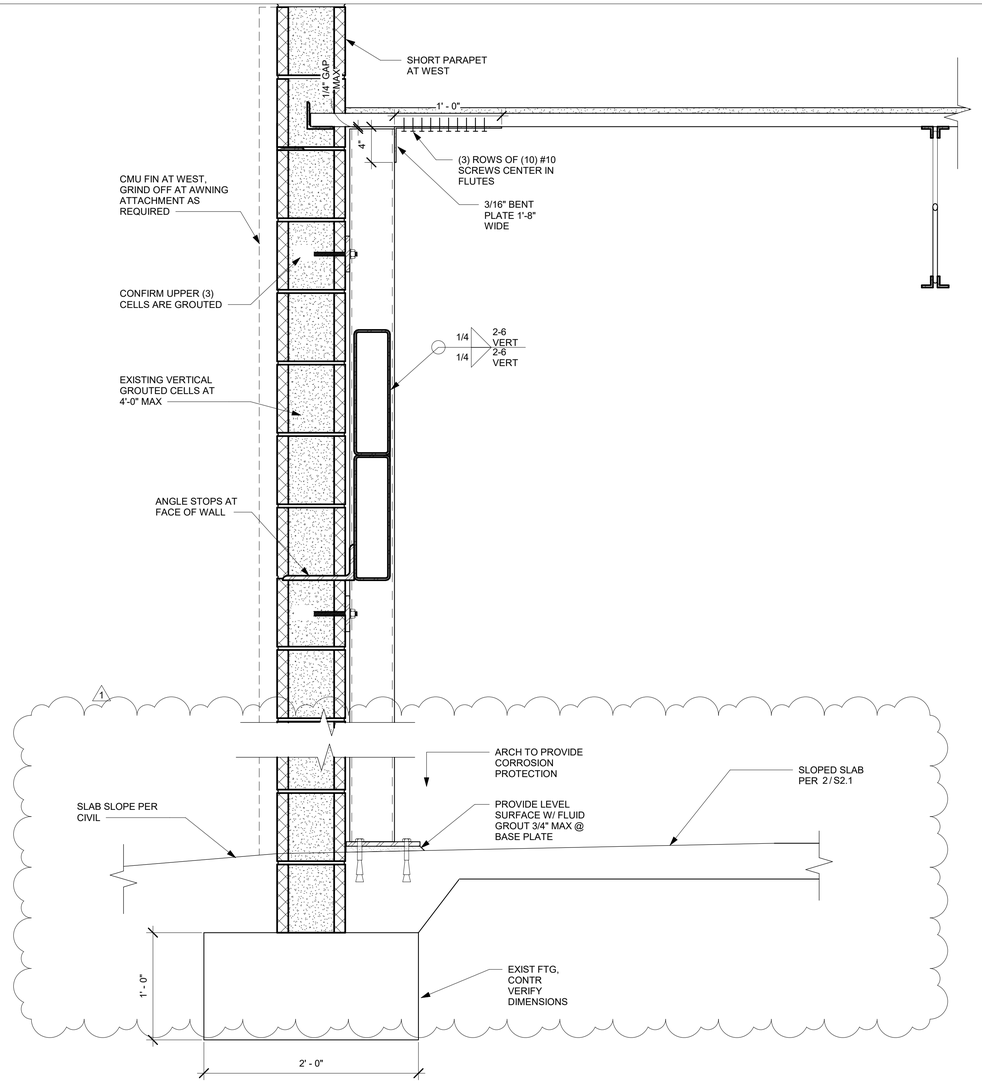
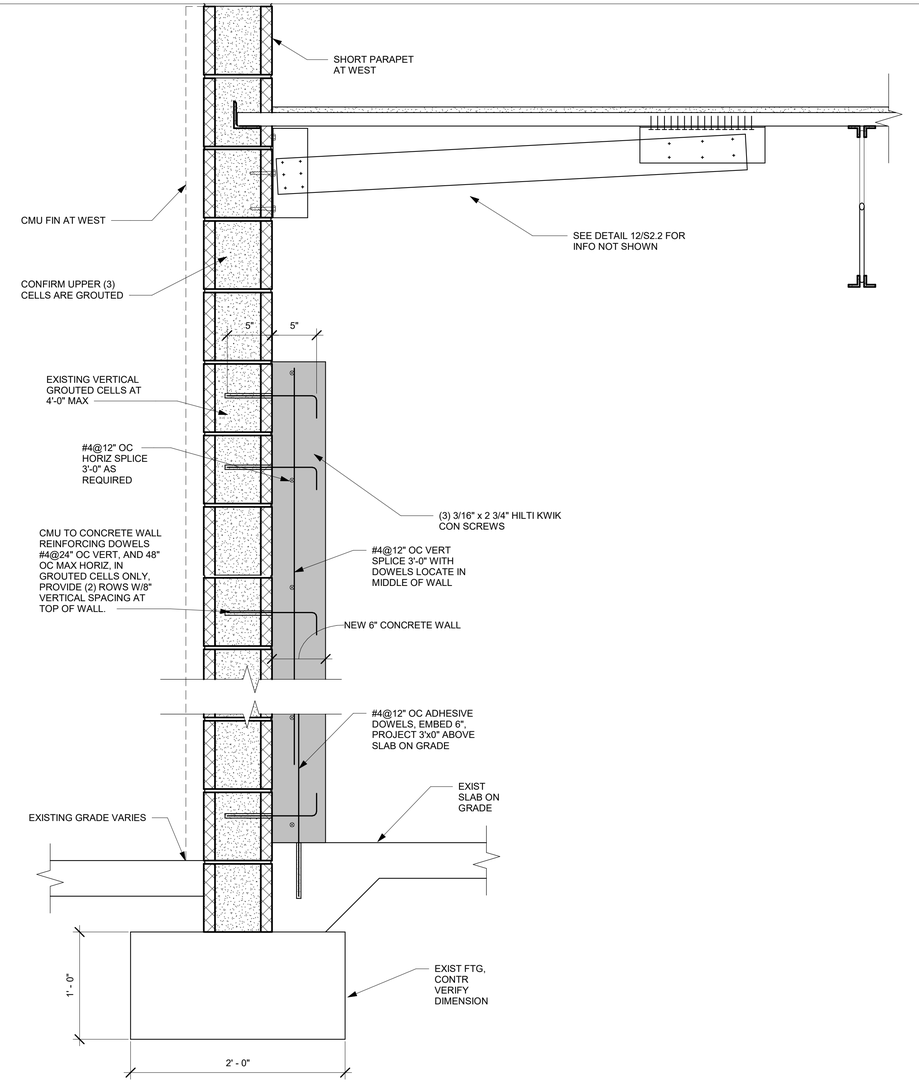


1 SECTION
1 1/2" = 1'-0"



2 SECTION
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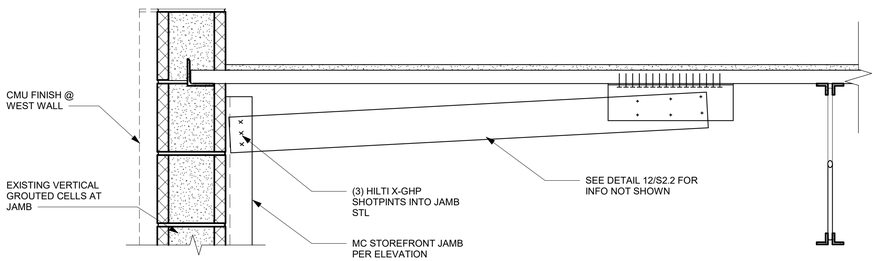
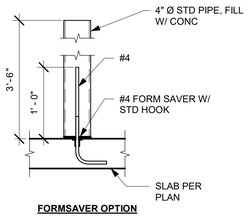


1
3 SECTION
1 1/2" = 1'-0"



FORMSAVER OPTION

1
4 TYPICAL BOLLARD IN PT SLAB
1" = 1'-0"

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DETAILS

S2.3

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