PROPOSED STORAGE SHED
CIMCO SALES
2315 INTER AVENUE
PUYALLUP, WASHINGTON 98372

PRPF20231137

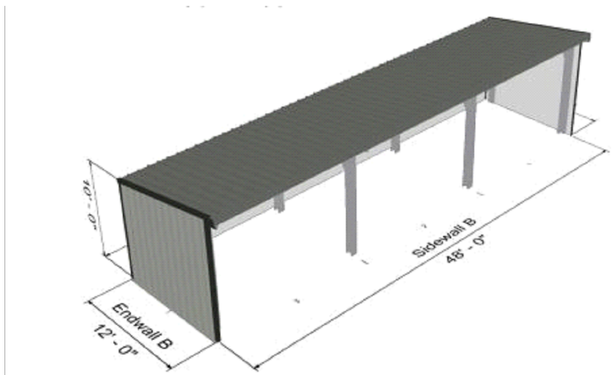
CITY OF PUYALLUP
Planning Division Approved Site Plan
(253) 864-4165
MINIMUM SETBACK REQUIREMENTS

Front Yard: 20' Rear Yard: 0'
Interior Side Yard: Left: 0' Right: 0'
Street Side Yard: N/A
Zoning District: ML
Permit #: PRPF20231137

Additional Conditions/Comments
N/A

Staff: NComstock
Date: 08/16/2023
Front, rear, and side yard property lines shall be marked with string from surveying pins prior to footing inspection.

CONCEPT IMAGE



SHEET INDEX

VICINITY MAP

City of Puyallup Building
REVIEWED FOR COMPLIANCE
RayC
08/02/2023
4:43:53 PM

Approval of submitted plans is not an approval of omissions or oversight by this office or noncompliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable building codes and regulations of the local government.

THE APPROVED CONSTRUCTION PLANS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

City of Puyallup
Development & Permitting Services
ISSUED PERMIT
Building | Planning
Engineering | Public Works
Fire | Traffic

PROJECT DIRECTORY

OWNER
CIMCO Sales
2315 Inter Avenue
Puyallup, WA 98372
Contact: Rick Velasquez
Phone: (253) 224-4428
Email: rick@cimcopnw.com

ARCHITECT
Castino Architecture
8911 71st Ave NW
Gig Harbor, WA 98332
Contact: James H. Castino, AIA
Phone: (253) 973-6680
Email: jimpilot22@gmail.com

CIVIL ENGINEER
Larson & Associates
9027 Pacific Avenue #4
Tacoma, WA 98444
Contact: Grant Middleton, P.E.
Phone: (253) 474-3404
Email: gmiddleton@rrlarson.com

SITE DATA

A. STREET ADDRESS
2315 INTER AVENUE, PUYALLUP, WA 98372

B. LEGAL DESCRIPTION

C. PARCEL NUMBER
2105200140

D. PROPERTY AREA

BUILDING CODE DATA

A. APPLICABLE CODE
2018 INTERNATIONAL BUILDING CODE
2018 WA STATE ENERGY CODE AS APPLICABLE TO SEMI-HEATED SPACES.

B. USE
TEMPORARY STORAGE OF PRODUCTS ON PALLETS

C. CONSTRUCTION TYPE
TYPE II-B, NON-SEPARATED USES
NON-SPRINKLERED

D. OCCUPANCY TYPE
S1 MODERATE HAZARD STORAGE

Storage limited to quantities and commodities as adopted by the State of Washington building and fire codes for non-sprinklered buildings.

E. AREA OF BUILDING (SF)
PROPOSED BUILDING: 576 SF
ALLOWED AREA: 17,500 SF
(PER TABLE 506.2)

PROJECT DESCRIPTION

CONSTRUCT A 576 SF PRE-ENGINEERED METAL CANOPY OPEN ON EAST WALL, INCLUDING CONCRETE PIER FOOTINGS ON EXISTING ASPHALT PAVING. NO FLOOR SLAB.

PROPOSED STORAGE SHED
CIMCO SALES
2315 INTER AVENUE
PUYALLUP, WASHINGTON 98372

PERMIT SET

REVISIONS:

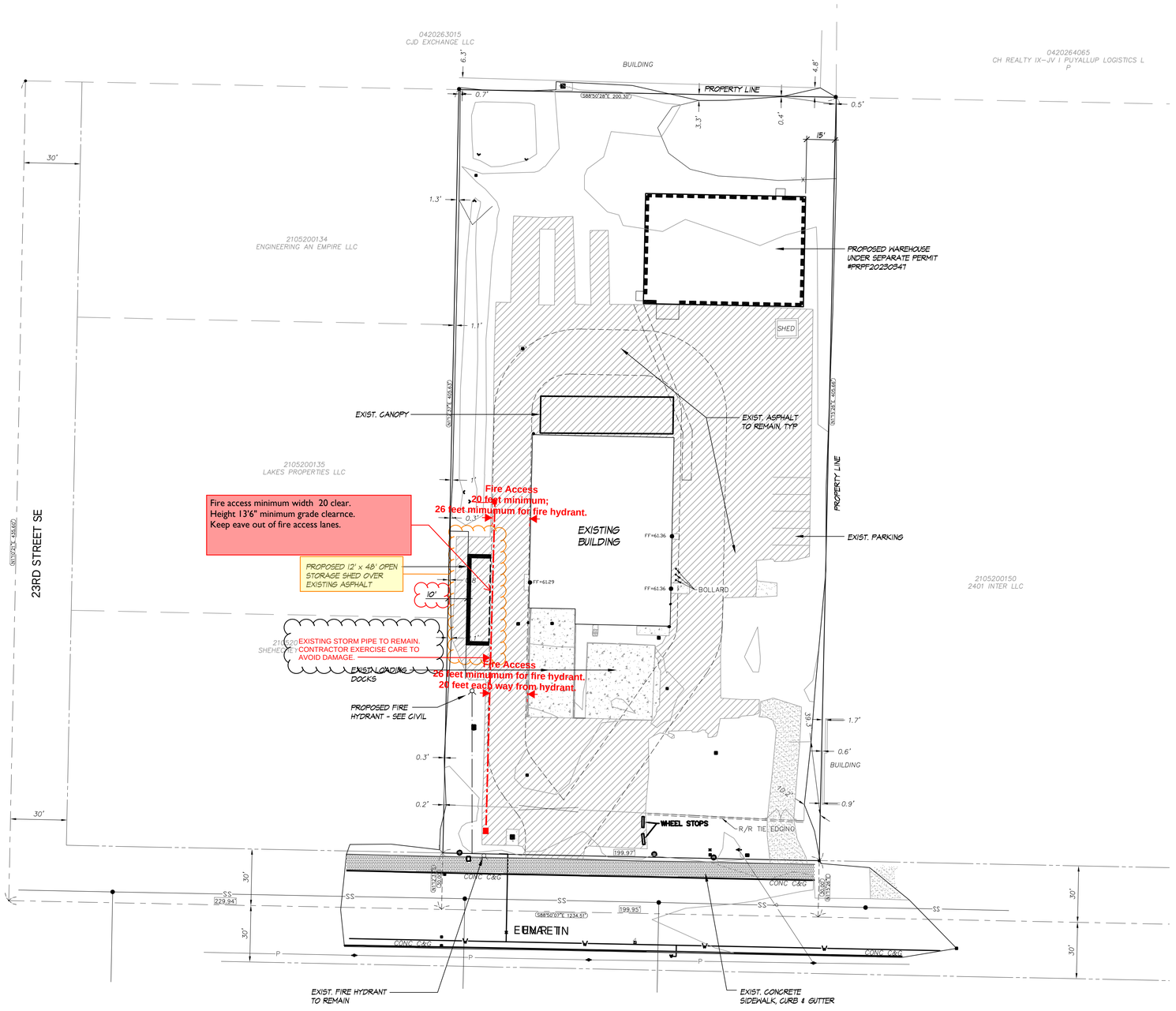
5438 REGISTERED ARCHITECT
JAMES H. CASTINO
STATE OF WASHINGTON

Castino Architecture
James Castino
Principal
8911 71st Ave. NW
Gig Harbor, WA 98332
PHONE: (253) 973-6680
EMAIL: jimpilot22@gmail.com

DATE: AUG. 1, 2023
DRAWN BY: PDS
PROJECT NO.:
FILE NAME:

COVER SHEET

SHEET NO:
G1.1



City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

8E SITE PLAN

SCALE: 1" = 30'

PROPOSED STORAGE SHED
CIMCO SALES
2315 INTER AVENUE
PUYALLUP, WASHINGTON 98372

PERMIT SET

REVISIONS:

CONTRACTOR SHALL VERIFY AND COORDINATE THE DIMENSIONS AND DETAILS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK, AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR VARIATIONS FROM CONDITIONS SHOWN. DO NOT SCALE DRAWINGS. THESE DRAWINGS ARE THE PROPERTY OF CASTINO ARCHITECTURE. ALL DRAWINGS, DESIGNS AND OTHER INFORMATION ON THE DRAWINGS ARE FOR USE ON THE SPECIFIED PROJECT ONLY, AND SHALL NOT BE USED OTHERWISE WITHOUT THE EXPRESS WRITTEN PERMISSION BY JAMES H. CASTINO, AIA. ARCHITECT ACCEPTS NO LIABILITY AND SHALL BE HELD HARMLESS FROM ANY RESPONSIBILITY FOR DAMAGES RESULTING FROM REVISIONS, CHANGES OR ADAPTATIONS TO THESE DRAWINGS.

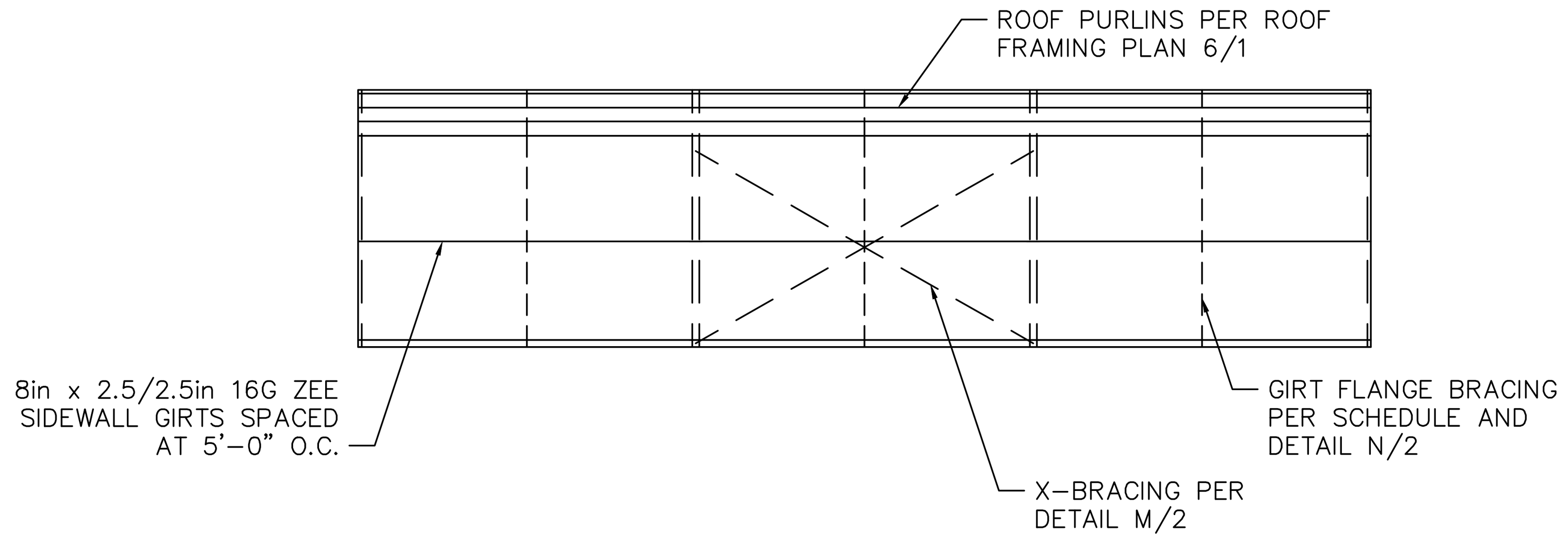
5438 REGISTERED ARCHITECT
JAMES H. CASTINO
STATE OF WASHINGTON

Castino Architecture
James Castino
Principal
8911 71st Ave. NW
Gig Harbor, WA 98332
PHONE: (253) 973-6680
EMAIL: jimpilot22@gmail.com

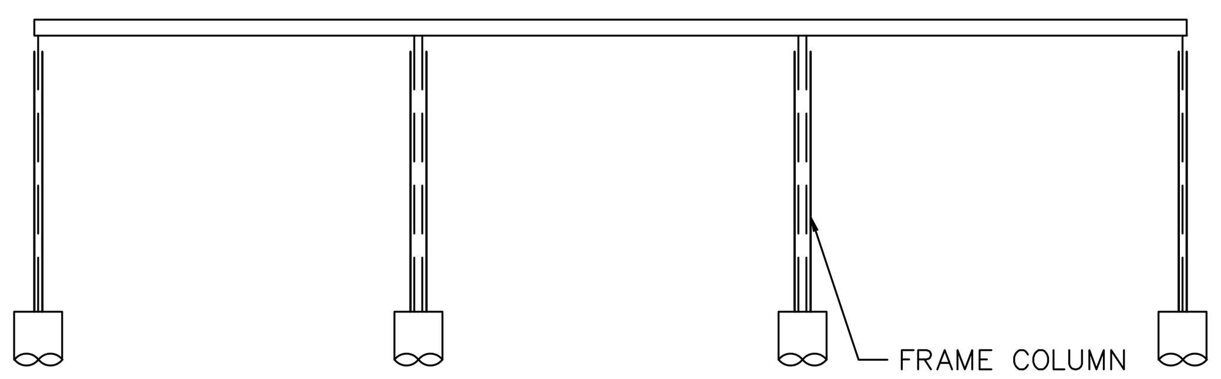
DATE: AUG. 1, 2023
DRAWN BY: PDS
PROJECT NO.:
FILE NAME:

SITE PLAN

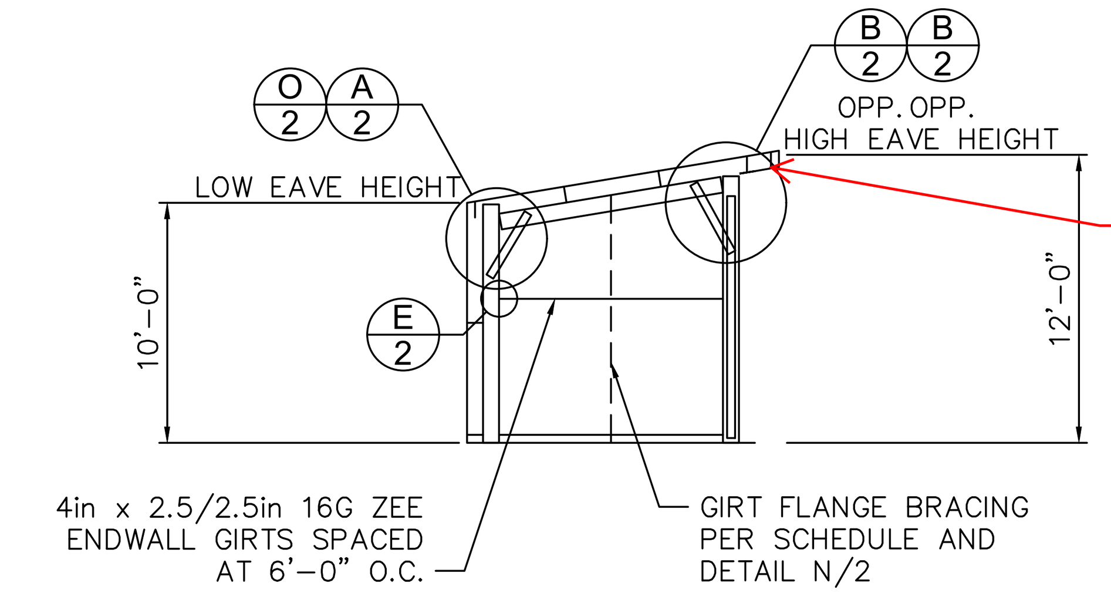
SHEET NO:
A1.0



2/1 SIDEWALL 'A' EXTERIOR ELEVATION
SCALE: 1/8" = 1'-0"



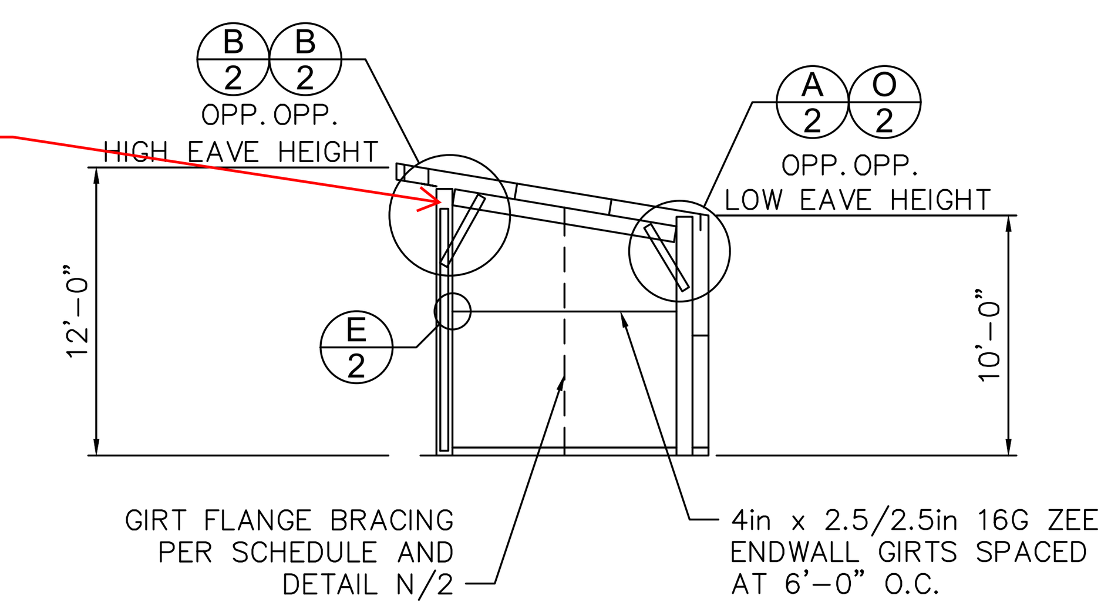
3/1 SIDEWALL 'B' EXTERIOR ELEVATION
SCALE: 1/8" = 1'-0"



Fire access minimum width 20 clear.
Height 13'6" minimum grade clearnce.
Keep eave out of fire access lanes.

5/1 ENDWALL 'A' INTERIOR ELEVATION
SCALE: 1/8" = 1'-0" FRAME #1

Fire access minimum width 20 clear.
Height 13'6" minimum grade clearnce.
Keep eave out of fire access lanes.



4/1 ENDWALL 'B' INTERIOR ELEVATION
SCALE: 1/8" = 1'-0" FRAME #4

IMPORTANT: IN ADDITION TO THESE ENGINEERING PLANS (WHICH ALWAYS TAKE PRECEDENCE), YOU SHOULD HAVE THE FOLLOWING FROM ACT BUILDING SYSTEMS:

- CONSTRUCTION PACKAGE
- INSTALLATION MANUALS
- CONSTRUCTION VIDEOS

PLEASE CONTACT YOUR SALES REP IF YOU HAVE NOT RECEIVED THESE PRIOR TO STARTING CONSTRUCTION.

PROJECT DESIGN CRITERIA

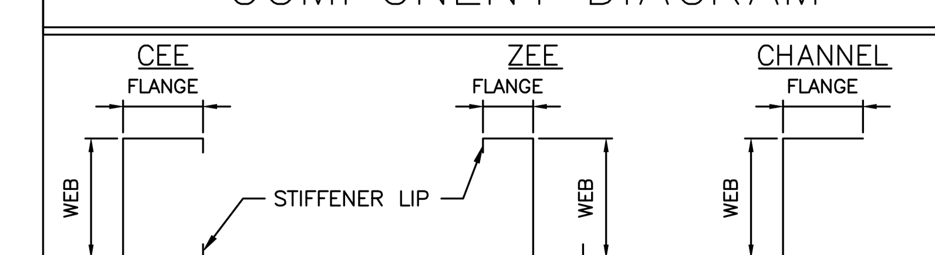
ROOF DEAD LOAD: 3 psf
ROOF COLLATERAL LOAD: 0 psf
GROUND SNOW LOAD: 25 psf Ct = 1.2
ROOF SNOW LOAD: 25 psf
ROOF LIVE LOAD: 20 psf
WIND SPEED: 110 mph
WIND EXPOSURE: C
Ss: 1.259 Sds: 1.007
S1: 0.433 Sd1: 0.539
SEISMIC DESIGN CATEGORY:
D ('short' period) D ('1-sec' period)
R transverse: 3.0 R longitudinal: 3.0
RISK CATEGORY: II
SOIL BEARING PRESSURE: 1500 psf

WIND DESIGN OF LATERAL FORCE-RESISTING SYSTEMS IS BASED ON THE DIRECTIONAL DESIGN PROCEDURE OF ASCE 7-16, CHAPTER 27

SEISMIC DESIGN OF LATERAL FORCE-RESISTING SYSTEMS ARE AS FOLLOWS:
-- TRANSVERSE: ORDINARY STEEL MOMENT FRAME (SEISMIC DESIGN IS BASED ON ASCE 07-16, SECTIONS 12.1 - 12.13)
-- LONGITUDINAL: ORDINARY STEEL BRACED FRAME. (SEISMIC DESIGN IS PERFORMED USING THE SIMPLIFIED DESIGN PROCEDURE (ASCE 07-16, SECTION 12.14).

DESIGN BASE SHEAR: IS SHOWN ON CALCULATION SHEET M2.

COMPONENT DIAGRAM



TYP. = TYPICAL U.N.O. = UNLESS NOTED OTHERWISE

FOUNDATION DETAIL KEYS

(A) ADD SINGLE CEE STIFFENER TO MAIN BUILDING CORNER COLUMN PER DETAIL L/2

DEFLECTION LIMITS

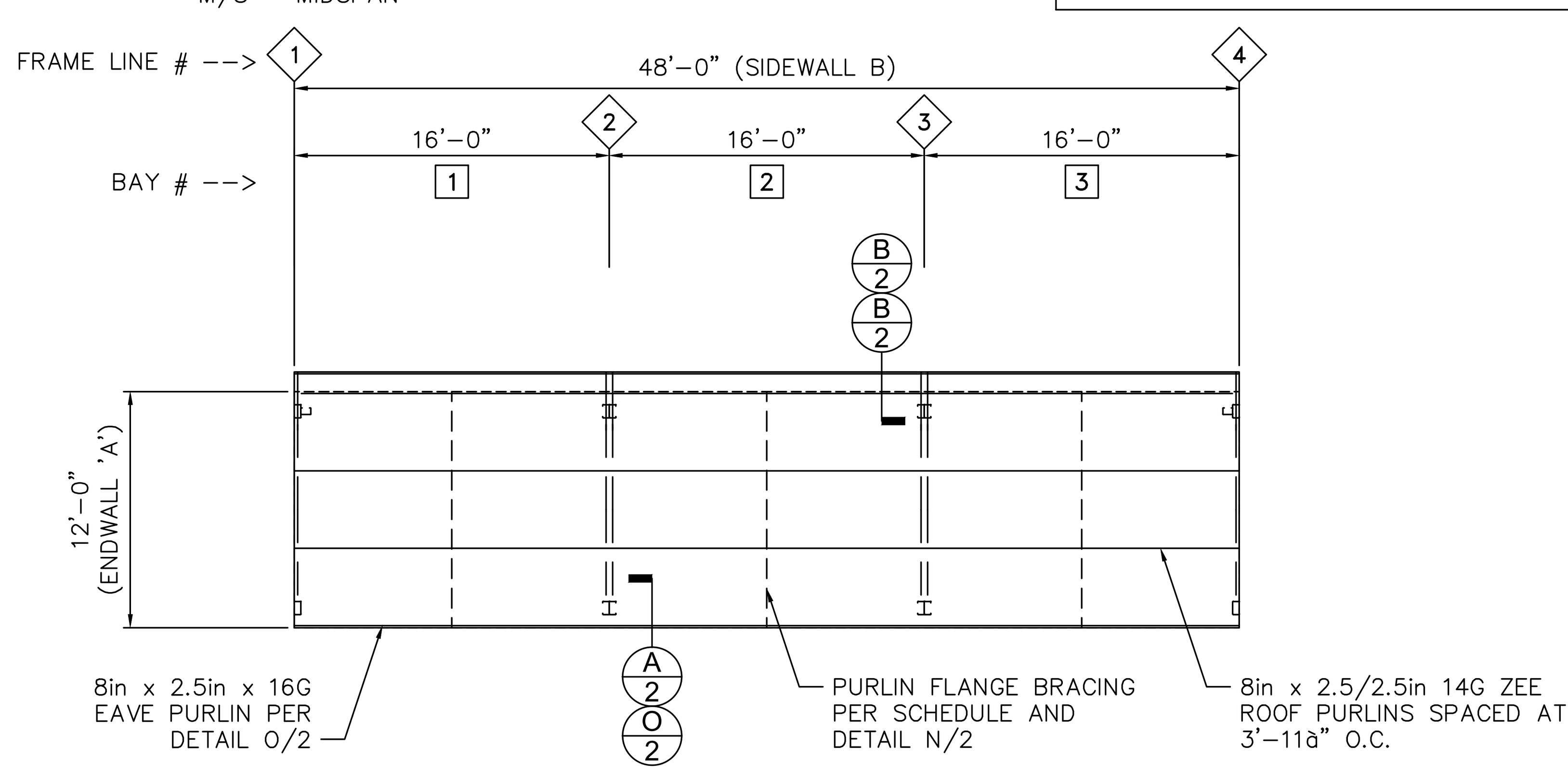
PURLINS:	L/150 (STD)
GIRTS:	L/90 (STD)
EW WIND COLUMNS:	L/120 (STD)
WALL PANEL:	L/60 (STD)

FLANGE BRACING SCHEDULE

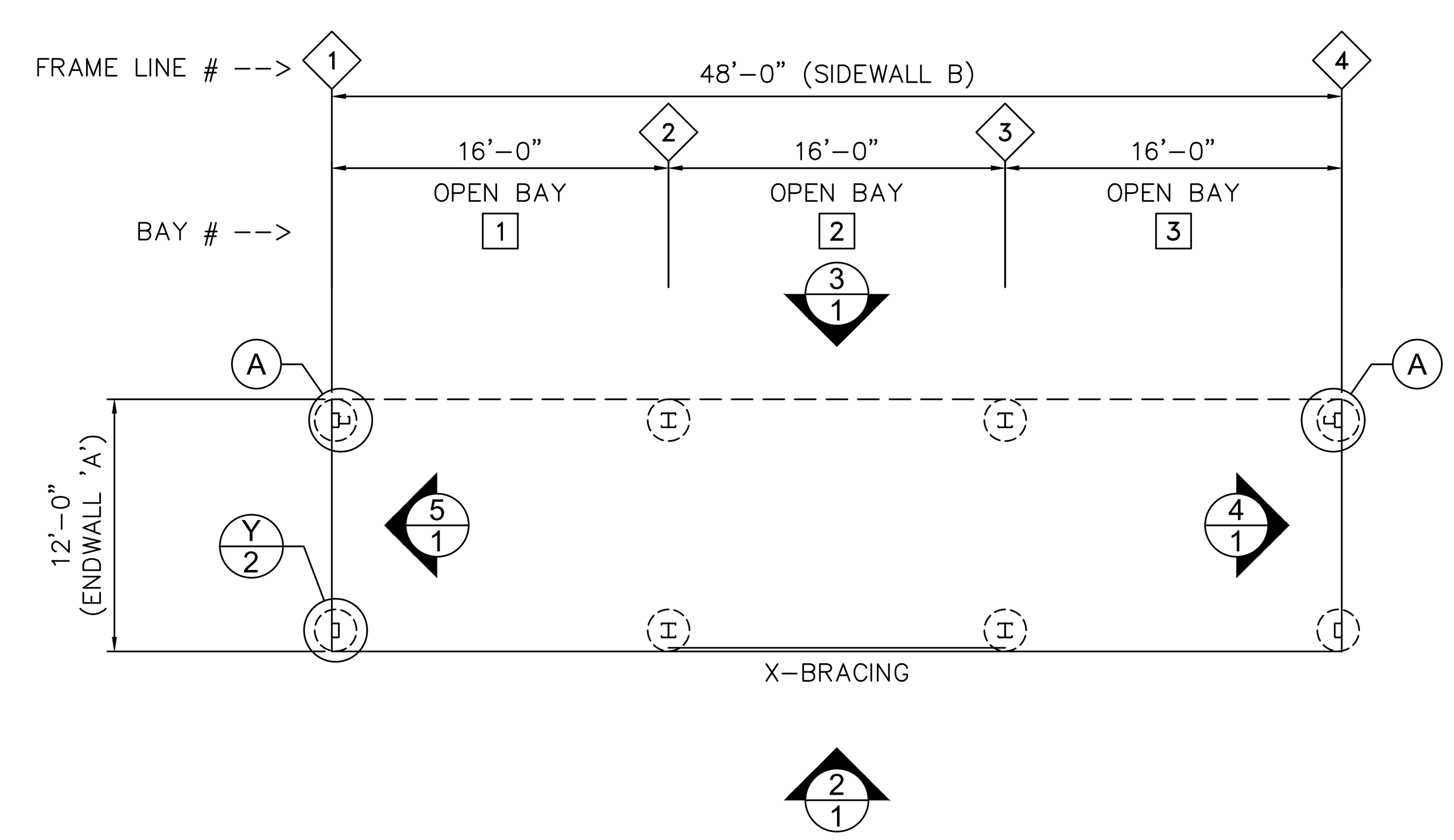
	BAY #1	BAY #2	BAY #3
Sidewall 'A'	M/S	M/S	M/S
Endwall 'A'	M/S	---	---
Endwall 'B'	M/S	---	---
Main Roof	M/S	M/S	M/S

--- = NONE REQUIRED
M/S = MIDSPAN

ROOF DIAPHRAGM NOTE
ROOF SHEETING IS USED AS DIAPHRAGM TO BRACE THE BUILDING AND IS NOT TO BE CUT UNDER ANY CIRCUMSTANCES



6/1 ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"



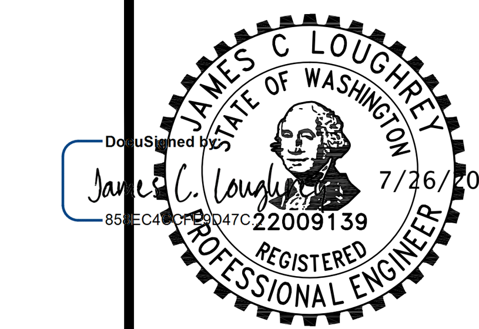
NOTE: USE 1" X 3" DEWALT 'SCREW-BOLT+' ANCHOR IN 3⅞" DEEP HOLES AT ANCHOR LOCATIONS PER BASE DETAIL F/2, INSTALLED PER ICC REPORT ESR-3889, SECTION 4.3.

NOTE: SEE "TYP. FRAME CROSS-SECTION" DETAIL ON SHEET 2 FOR SPECIFIC FRAME DETAIL INFORMATION.

1/1 FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic



ALLIANCE ENGINEERING
aeOregon.com
2700 MARKET ST NE
SALEM, OR 97301
503 589-1727
FAX 503 589-1728

ACTBUILDING SYSTEMS

DISTRIBUTOR: JM Buildings & Construction
JOB NAME: Cimco Sales, Inc
JOB ADDRESS: 2315 Inter Avenue
Puyallup, WA 98372

DRAWN

CHECKED

DATE 7/26/2023

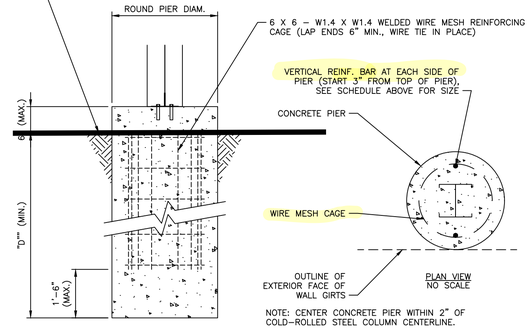
JOB NO. COLA93804467

SHEET S-1 OF 2

LOCATION	DIAM.	"D"	VERT. REINFORCING
SIDEWALLS & CORNERS	24"	5'-2"	(1)-#4

EXISTING ASPHALT OVER

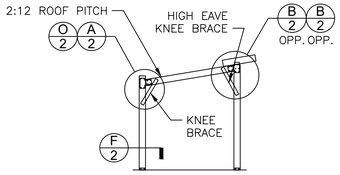
UNDISTURBED NATIVE SOIL (ANY OVER-EXCAVATION MUST BE BACKFILLED EITHER WITH 2000 psi CONCRETE OR WITH CLEAN SAND THOROUGHLY COMPACTED BY TAMPING IN LAYERS NOT MORE THAN 8" IN DEPTH)



NOTE: CENTER CONCRETE PIER WITHIN 2" OF COLD-ROLLED STEEL COLUMN CENTERLINE.

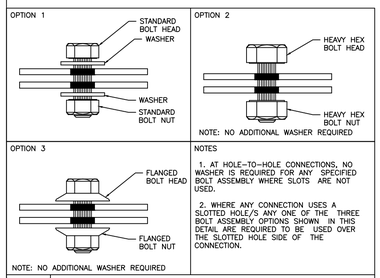
Y ROUND CONC. PIER FOUNDATION

SCALE: NO SCALE



1/2 TYP. FRAME CROSS-SECTION

SCALE: 1/8" = 1'-0" FRAMES 2-4



NOTES

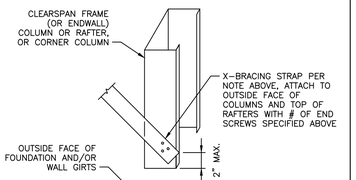
1. AT HOLE-TO-HOLE CONNECTIONS, NO WASHER IS REQUIRED FOR ANY SPECIFIED BOLT ASSEMBLY WHERE SLOTS ARE NOT USED.

2. WHERE ANY CONNECTION USES A SLOTTED HOLE/S ANY ONE OF THE THREE BOLT ASSEMBLY OPTIONS SHOWN IN THIS DETAIL ARE REQUIRED TO BE USED OVER THE SLOTTED HOLE SIDE OF THE CONNECTION.

T BOLT OPTIONS

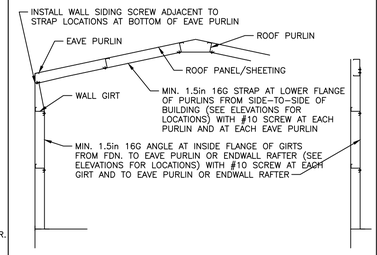
ENDWALL & ROOF: N/A

SIDEWALLS: SGL. 1.5in 16G STRAP WITH (3) #14 SCREWS AT EACH END

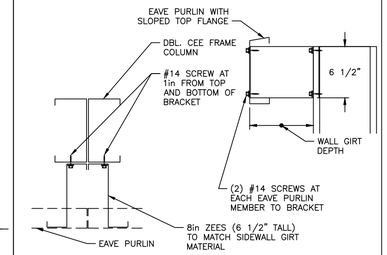


NOTES:
1) CONNECT STRAP AT TOP OF ADJACENT COLUMN OR RAFTER IN SAME MANNER.
2) IF DOUBLE STRAPS ARE SPECIFIED ABOVE, INSTALL SIDE-BY-SIDE, NOT ON TOP OF EACH OTHER.

M ROOF AND WALL X-BRACING CONNECTION

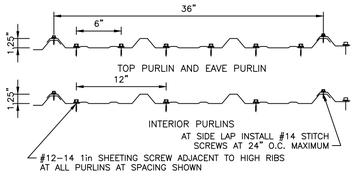


N PURLIN AND GIRT FLANGE BRACING



O EAVE PURLIN BRACKET

NOTE: ONLY STRUCTURAL INFORMATION IS INCLUDED IN THIS DETAIL. CONSULT PANEL MANUFACTURER FOR ADDT'L WEATHERTIGHTNESS RECOMMENDATIONS.

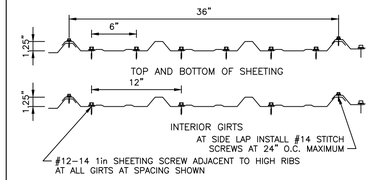


Curfman Steel Corp.
R-Panel 26G

Ix(t) = 0.0367 in^4/ft Sx(t) = 0.0367 in^3/ft Fy = 80 ksi Ix(b) = 0.0317 in^4/ft Sx(b) = 0.0458 in^3/ft

H ROOF SHEETING

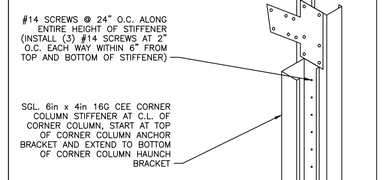
NOTE: ONLY STRUCTURAL INFORMATION IS INCLUDED IN THIS DETAIL. CONSULT PANEL MANUFACTURER FOR ADDT'L WEATHERTIGHTNESS RECOMMENDATIONS.



Curfman Steel Corp.
R-Panel 26G

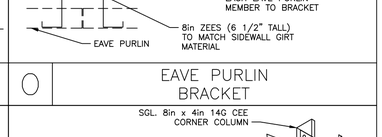
Ix(t) = 0.0367 in^4/ft Sx(t) = 0.0367 in^3/ft Fy = 80 ksi Ix(b) = 0.0317 in^4/ft Sx(b) = 0.0458 in^3/ft

I WALL SHEETING

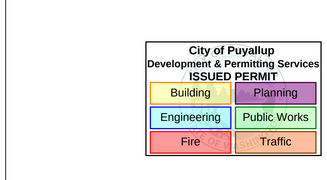


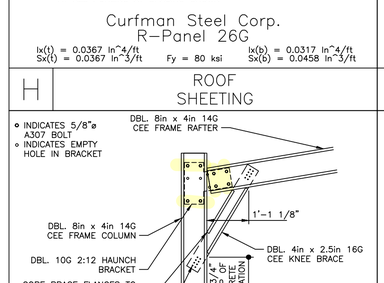
NOTE: SEE DETAIL B/2 FOR ADDITIONAL INFORMATION.

L CORNER COLUMN STIFFENER



NOTE: FIRST PLACE KNEE BRACE BEFORE STIFFENER

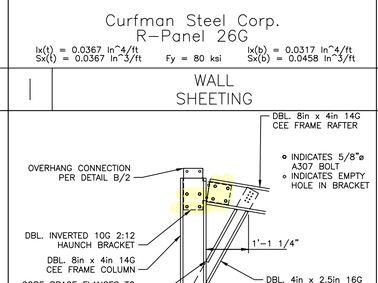




NOTE: SEE DETAIL F/2 FOR BASE DETAIL.

NOTE: ALL COMPONENTS NOTED AS 'DBL.' SHALL BE 'SGL.' AT ENDWALLS.

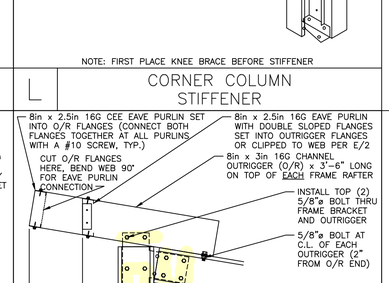
A HAUNCH CONNECTION



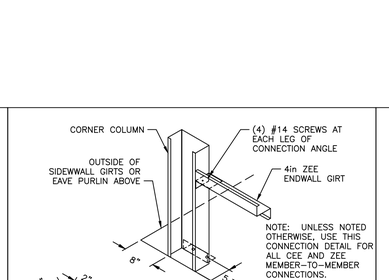
NOTE: SEE DETAIL F/2 FOR BASE DETAIL.

NOTE: ALL COMPONENTS NOTED AS 'DBL.' SHALL BE 'SGL.' AT ENDWALLS.

B HIGH EAVE HAUNCH CONNECTION

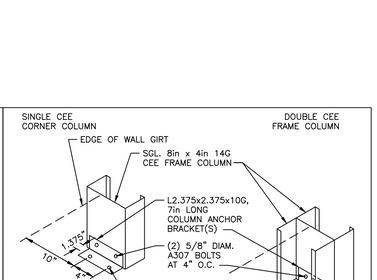


D ZEE PURLIN/GIRT CONNECTION



TYP. CONNECTION ANGLE

E ENDWALL GIRT AT CORNER COLUMN



NOTE: CENTER ALL BOLTS ON COLUMN ANCHOR BRACKET LENGTHS. AT SINGLE FRAME COLUMN, BRACKET MAY BE PLACED ON EITHER SIDE OF COLUMN. EXCEPT AT CORNER COLUMN, BRACKET MUST BE INSTALLED ON SIDE OF COLUMN SHOWN.

F FRAME COLUMN BASE DETAIL

STRUCTURAL GENERAL NOTES

1. GOVERNING CODE: 2018 INTERNATIONAL BUILDING CODE

2. DRAWING OWNERSHIP:
THESE DRAWINGS ARE JOINTLY OWNED BY CSC BUILDING SYSTEMS (CSC) AND ALLIANCE ENGINEERING OF OREGON, INC. DRAWINGS ARE PROVIDED FOR THE SOLE PURPOSE OF OBTAINING BUILDING PERMITS. ENGINEERING SEAL IS VALID FOR THE CONSTRUCTION OF A SINGLE BUILDING AT THE JOB ADDRESS SHOWN IN DRAWING TITLEBLOCK. ANY OTHER USE OF THESE DRAWINGS WITHOUT WRITTEN AUTHORIZATION FROM CSC AND ALLIANCE ENGINEERING OF OREGON, INC IS PROHIBITED.

3. DRAWING SEAL REQUIREMENTS:
THESE DRAWINGS ARE NOT VALID UNLESS 1) THE SEAL (STAMP) ON A PAPER COPY IS WET SIGNED IN INK BY THE ENGINEER, OR 2) THE PAPER COPIES ARE OF A DRAWING DIGITALLY SIGNED BY THE ENGINEER, OR 3) THE ELECTRONIC FILE OF THE DRAWING IS DIGITALLY SIGNED BY THE ENGINEER. IF A COPY OF THESE DRAWINGS IS DISTRIBUTED WITHOUT EITHER A PROPER WET SIGNATURE OR A DIGITAL SIGNATURE, THE DRAWING IS CONSIDERED INVALID. IF A COPY OF THESE DRAWINGS IS DISTRIBUTED WITHOUT EITHER A PROPER WET SIGNATURE OR A DIGITAL SIGNATURE, THE DRAWING IS CONSIDERED INVALID. THE ENGINEER ACCEPTS NO LIABILITY OR RESPONSIBILITY FOR DRAWINGS CONSIDERED INVALID AS NOTED ABOVE.

4. CONTRACTOR RESPONSIBILITIES:
CONTRACTOR SHALL VERIFY AND CONFIRM ALL EXISTING CONDITIONS AND DIMENSIONS. ALLIANCE ENGINEERING OF OREGON, INC (ENGINEER) SHALL BE NOTIFIED OF ANY DISCREPANCIES BETWEEN DRAWINGS AND EXISTING CONDITIONS PRIOR TO START OF WORK.
CONTRACTOR MUST SUBMIT IN WRITING ANY REQUEST FOR MODIFICATION TO THE PLANS AND/OR SPECIFICATIONS AND NO STRUCTURAL CHANGES FROM THE APPROVED PLANS SHALL BE MADE IN THE FIELD UNLESS, PRIOR TO MAKING CHANGES, WRITTEN APPROVAL IS OBTAINED FROM THE ENGINEER. SHOP DRAWINGS SUBMITTED TO THE ENGINEER FOR REVIEW DO NOT CONSTITUTE "IN WRITING" UNLESS IT IS NOTED THAT SPECIFIC CHANGES ARE BEING REQUESTED. IF CHANGES ARE MADE WITHOUT WRITTEN APPROVAL, SUCH CHANGES SHALL BE THE LEGAL AND FINANCIAL RESPONSIBILITY OF THE CONTRACTOR OR SUB-CONTRACTORS INVOLVED AND IT SHALL BE THEIR FULL RESPONSIBILITY TO REPLACE OR REPAIR THE CONDITION AS DIRECTED BY THE ENGINEER.
CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING, OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING ERECTION. THESE TEMPORARY PROVISIONS SHALL REMAIN IN PLACE UNTIL SUFFICIENT PERMANENT MEMBERS ARE ERECTED TO INSURE THE SAFETY OF PARTIALLY ERECTED STRUCTURES. CONTRACTOR IS RESPONSIBLE FOR MEETING ALL LAWS REGULATING THE ERECTION OF STEEL BUILDINGS.
THESE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. BUILDING IS NOT CONSIDERED COMPLETE UNTIL THE INSTALLATION OF ALL COMPONENTS AND DETAILS SHOWN HEREIN ARE INSTALLED ACCORDING TO THE DRAWINGS.

5. ENGINEERING:
THE SUPPLYING OF STAMPED ENGINEERING CALCULATIONS AND DRAWINGS FOR THIS METAL BUILDING DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT ALLIANCE ENGINEERING OF OREGON, INC IS ACTING AS THE ENGINEER OR ARCHITECT OF RECORD OR THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE FOR THE WHOLE OF THE PROJECT.
THIS BUILDING HAS BEEN REVIEWED BY ALLIANCE ENGINEERING OF OREGON, INC FOR CONFORMITY ONLY TO THE STRUCTURAL DESIGN PORTIONS OF THE GOVERNING CODE. THE BUILDING OWNER IS RESPONSIBLE TO SEEK PROFESSIONAL ADVICE IN ADDRESSING ANY OTHER CODE REQUIREMENTS (INCLUDING, BUT NOT LIMITED TO, FIRE AND LIFE SAFETY, ENVIRONMENTAL, ACCESSIBILITY, OR ELECTRICAL) THAT MAY APPLY TO THIS PROJECT.
DRAWINGS SCALES INDICATED ON DRAWINGS ARE APPROXIMATE AND INTENDED TO BE USED FOR REFERENCE ONLY. DO NOT SCALE DRAWINGS FOR CONSTRUCTION PURPOSES.
THESE DOCUMENTS ARE STAMPED ONLY AS TO THE COMPONENTS FURNISHED BY CSC. IT IS THE RESPONSIBILITY OF THE PURCHASER TO COORDINATE DRAWINGS PROVIDED BY ALLIANCE ENGINEERING OF OREGON, INC WITH OTHER PLANS AND/OR OTHER COMPONENTS THAT ARE PART OF THE OVERALL PROJECT. IN CASES OF DISCREPANCIES, DRAWINGS PROVIDED BY ALLIANCE ENGINEERING OF OREGON, INC SHALL GOVERN. THE UNDERSIGNED ENGINEER WILL NOT SUPERVISE THE FABRICATION OR ERECTION OF THIS STRUCTURE. ANY OBSERVATION VISITS TO THE PROJECT SITE BY THE UNDERSIGNED ENGINEER ARE NOT TO BE CONSTRUED AS BEING INSPECTIONS FOR THE CONSTRUCTION OF ANY COMPONENT OF THIS BUILDING.

6. INSPECTIONS:
NO SPECIAL INSPECTIONS ARE REQUIRED BY THE GOVERNING CODE ON THIS JOB. ALL SPECIAL INSPECTIONS AND ANY OTHER ADDITIONAL INSPECTIONS REQUESTED BY BUILDING DEPARTMENT SHALL BE AT OWNER'S EXPENSE.

7. SOIL REQUIREMENTS:
ALLOWABLE SOIL BEARING VALUE INDICATED ON DRAWING SHEET 1 OCCURS AT 12" BELOW FINISH GRADE, OR EXISTING NATURAL GRADE, OR AT FROST DEPTH SPECIFIED BY BUILDING DEPARTMENT, WHICHEVER IS THE LOWEST ELEVATION. FOUNDATION DESIGN SHOWN ASSUMES BOTTOM OF FOOTING BEARS ON NATIVE SOILS.
FOUNDATION DESIGN SHOWN DOES NOT ACCOUNT FOR EXPANSIVE SOIL CONDITIONS OR FOR CONCRETE THAT WILL BE EXPOSED TO SULFATE CONTAINING SOLUTIONS OR CHLORIDES. OWNER SHALL CONTACT ENGINEER PRIOR TO CONSTRUCTION IF ANY OF THESE CONDITIONS EXIST.

8. CONCRETE REQUIREMENTS:
ALL CONCRETE SHALL HAVE A MIN. 28-DAY STRENGTH OF 2500 psi. HIGHER STRENGTH CONCRETE MAY BE USED, AT OWNER'S DISCRETION, FOR FINISH AND DURABILITY PURPOSES. CEMENT SHALL COMPLY WITH ASTM C150, TYPE 2, AND SHALL CONTAIN NO FLYASH.
ALL CONCRETE PLACEMENT SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE", WHICH IS HEREBY MADE A PART OF THESE DOCUMENTS.
CONCRETE REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60 FOR #4 BARS AND LARGER, GRADE 40 FOR #3 BARS. WELDED WIRE MESH SHALL CONFORM TO ASTM A185 (Fy MIN. OF 70 ksi). FOOTING REINFORCING BARS TO BE CONTINUOUS AROUND CORNERS. LAP SPLICE FOOTING REINFORCING MIDWAY BETWEEN COLUMNS. ALL LAP SPLICES TO BE 48 BAR DIAMETERS MIN., U.N.O.
CONCRETE GRADE BEAMS, THICKENED SLAB EDGES, PIERS, AND SPREAD FOOTINGS SHALL BE POURED ONTO UNDISTURBED, NATIVE SOIL WHICH IS FREE FROM ANY MATERIAL THAT WILL ADVERSELY AFFECT THE ALLOWABLE SOIL BEARING PRESSURE SPECIFIED ON SHEET 1.
CONCRETE ANCHOR INSTALLATION SHALL BE DONE IN ACCORDANCE WITH ICC REPORT ESR-3889, SECTION 4.3.

9. STRUCTURAL STEEL REQUIREMENTS:
ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 (Fy MIN. OF 36000 psi), U.N.O. ALL BOLTS SHALL CONFORM TO ASTM A307, U.N.O. BOLT HOLE DIAMETERS SHALL BE 1/16" LARGER THAN NOMINAL BOLT DIAMETER. ALL INSTALLATION SHALL BE IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE".
NO WELDING IS REQUIRED ON THIS JOB.

10. LIGHT GAUGE STRUCTURAL STEEL REQUIREMENTS:
ALL LIGHT GAUGE STEEL FRAMING MATERIAL AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
ALL LIGHT GAUGE STEEL MATERIAL SHALL CONFORM TO ASTM A653 HAVING A MINIMUM YIELD STRENGTH OF 55000 psi. THE GRADE AND ASTM SPECIFICATION NUMBER SHALL BE INDICATED BY PAINTING, DECAL, TAGGING, OR OTHER SUITABLE MEANS, ON EACH LIFT OR BUNDLE OF FABRICATED ELEMENTS.
UNLESS NOTED OTHERWISE, CEE, ZEE, AND CHANNEL MEMBERS' WEB AND FLANGE DIMENSIONS (IN INCHES) SHALL BE AS NOTED IN DETAILS IN THE FOLLOWING FORMAT: [WEB DEPTH]in x [FLANGE WIDTH]in [GAUGE]G. FOR ZEES WITH UNEQUAL FLANGES, THE WIDTHS FOR BOTH FLANGES WILL BE LISTED, SEPARATED BY A " /". MIN. FLANGE STIFFENER LIPS SHALL BE 0.885" FOR 12G CEES, 0.800" FOR 14G CEES, 0.773" FOR 16G CEES, 0.790" FOR 12G ZEES, 0.750" FOR 14G ZEES, AND 0.710" FOR 16G ZEES.
ALL BEND RADIUSES SHALL BE .1875". FOR ANGLES, THE FIRST TWO NUMBERS ARE THE LEG DIMENSIONS.
DECIMAL THICKNESS OF THE DELIVERED LIGHT GAUGE STEEL MATERIAL, ACCORDING TO NOMINAL GAUGES, SHALL MEET OR EXCEED 95% THE FOLLOWING DESIGN VALUES

GAUGE NO.	DECIMAL THICKNESS, IN.	GAUGE NO.	DECIMAL THICKNESS, IN.	GAUGE NO.	DECIMAL THICKNESS, IN.
10	0.135	14	0.070	18	0.048
12	0.105	16	0.059	20	0.036

EXCEPT AS SHOWN ON DRAWINGS, CEE COLUMN AND RAFTER MEMBERS SHALL NOT BE DRILLED OR NOTCHED WITHOUT PRIOR APPROVAL OF THE ENGINEER. DOOR JAMB, ROOF PURLIN, AND WALL GIRT ENDS MAY HAVE FLANGES COPED 3" MAX. IF CONNECTION IS MADE TO PERPENDICULAR MEMBER PER DETAIL E/2, ROUND HOLES MAY BE DRILLED THROUGH ANY GIRT OR PURLIN MEMBER WITHIN THE MIDDLE THIRD OF THE DEPTH OF THAT MEMBER AND NOT WITHIN 24" OF MEMBER END (FIELD-DRILLED BOLT HOLES INDICATED AT ENDS OF KNEE OR APEX BRACE WEBS AND SHOP-PUNCHED HOLES IN BRACE FLANGES EXCEPTED).
ALL BOLTS USED TO CONNECT LIGHT GAUGE MATERIAL SHALL CONFORM TO ASTM A307. BOLTS TO BE SNUG TIGHT PER THE RCSC AND AISC SPECIFICATIONS, UNLESS SPECIFICALLY NOTED OTHERWISE. BOLTS SHALL BE SPACED NO LESS THAN 3 BOLT DIAMETERS BETWEEN CENTERS. DISTANCE FROM BOLT CENTER TO THE END OR EDGE OF ANY LIGHT GAUGE MEMBER SHALL BE A MIN. OF 1.5 BOLT DIAMETERS. ALL SCREWS USED TO CONNECT LIGHT GAUGE MATERIAL SHALL BE SELF-DRILLING SCREWS AND SHALL HAVE A MIN. TENSILE BREAKING STRENGTH OF 100,000 psi. SCREWS SHALL BE SPACED NO LESS THAN 1" O.C. AND EDGE OR END DISTANCE SHALL NOT BE LESS THAN 1". UNLESS NOTED OTHERWISE, ALL REFERENCES TO 'SCREWS' CONNECTING MATERIAL THICKER THAN 20 ga. SHALL BE MIN. #14 SCREWS AND SHALL HAVE MIN. 14 THREADS PER INCH.
SCREW ROOT DIAMETERS SHALL NOT BE LESS THAN: #14 SCREW: .200" #12 SCREW: .177" #10 SCREW: .153"

11. STEEL ROOF AND WALL PANELS (CLADDING):
LIGHT GAUGE STEEL ROOF AND WALL PANELS SHALL CONFORM TO ASTM A653 AND THE STEEL DECK INSTITUTE SPECIFICATIONS AND HAVE A MIN. YIELD STRENGTH OF 80000 psi.
DECIMAL THICKNESSES, ACCORDING TO NOMINAL GAUGES, SHALL MEET OR EXCEED THE FOLLOWING:

GAUGE NO.	DECIMAL THICKNESS, IN.	GAUGE NO.	DECIMAL THICKNESS, IN.	GAUGE NO.	DECIMAL THICKNESS, IN.
22	0.0299	26	0.0179	29	0.0134
24	0.0239	28	0.0149	30	0.0120

SEE DETAILS H/2 AND I/2 FOR ROOF AND WALL PANEL FASTENER TYPES AND SPACINGS.

JAMES C. LOUGHREY STATE OF WASHINGTON 22009139 REGISTERED PROFESSIONAL ENGINEER
DocuSigned by: James C. Loughrey 7/26/2023
ALLIANCE ENGINEERING
aeOregon.com
2700 MARKET ST NE
SALEM, OR 97301
503 589-1727
FAX 503 589-1728
ACTBUILDING SYSTEMS®

DISTRIBUTOR: JM Buildings & Construction
JOB NAME: Cimco Sales, Inc
JOB ADDRESS: 2315 Inter Avenue Puyallup, WA 98372

DRAWN
CHECKED
DATE 7/26/2023
JOB NO. COLA93804467
S-2
OF 2