



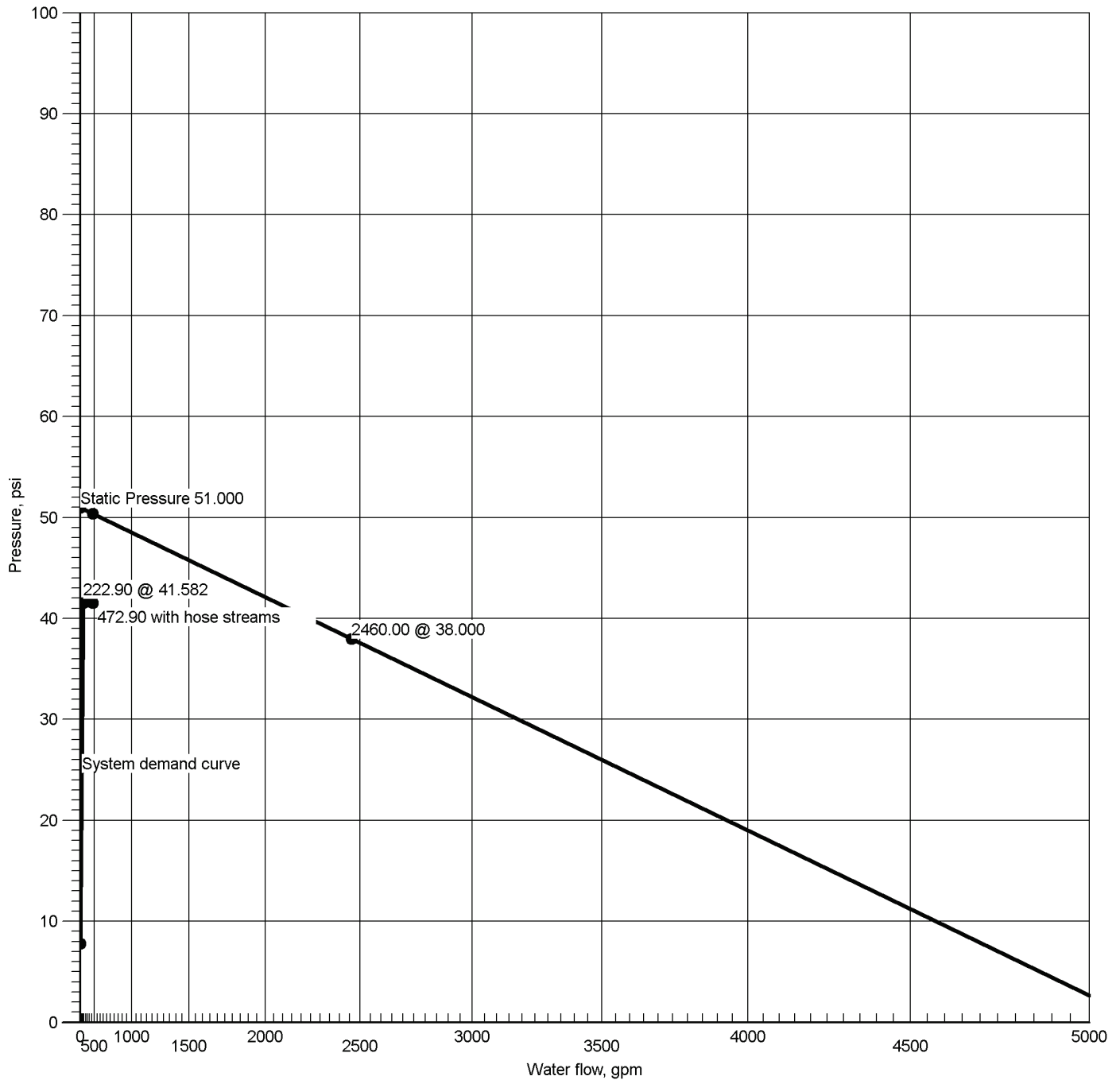
Hydraulic Summary

Job Number: S23-90G
Report Description: Ordinary Group I

Job											
Job Number S23-90G					Designer RICHARD M						
Job Name: CHC PUYALLUP GARAGE					State Certification/License Number						
Address 1 111 W. MAIN, PUYALLUP, WA 98371					AHJ						
Address 2					Job Site/Building						
Address 3					Drawing Name S23-90 GARAGE NEW SYSTEM						
System					Remote Area(s)						
Most Demanding Sprinkler Data 5.6 K-Factor 19.50 at 12.125					Occupancy Ordinary Group I			Job Suffix			
Hose Allowance At Source 250.00					Density 0.15 gpm/ft ²			Area of Application 900 ft ² (Actual 930 ft ²)			
Additional Hose Supplies <u>Node</u> <u>Flow(gpm)</u>					Number Of Sprinklers Calculated 11		Number Of Nozzles Calculated 0		Coverage Per Sprinkler 130 ft ²		
Total Hose Streams 250.00											
System Flow Demand 222.90			Total Water Required (Including Hose Allowance) 472.90								
Maximum Pressure Unbalance In Loops 0.000											
Maximum Velocity Above Ground 11.46 between nodes 10 and 12											
Maximum Velocity Under Ground 2.31 between nodes 18 and 1											
Volume capacity of Wet Pipes 1032.57 gal			Volume capacity of Dry Pipes								
Supplies											
Node	Name	Hose Flow (gpm)	Static (psi)	Residual (psi)	@	Flow (gpm)	Available (psi)	@	Total Demand (gpm)	Required (psi)	Safety Margin (psi)
17	Water Supply	250.00	51.000	38.000		2460.00	50.385		472.90	41.582	8.803
Contractor											
Contractor Number 22		Contact Name ROBERT ARCHER					Contact Title OWNER				
Name of Contractor: ARCHER CONSTRUCTION						Phone 253-872-7222			Extension		
Address 1 7855 S 206TH ST KENT WA 98032						FAX					
Address 2						E-mail					
Address 3						Web-Site					



Water Supply at Node 17



Hydraulic Graph

Water Supply at Node 17

Static: Pressure

51.000

Residual: Pressure

38.000 @ 2460.00

Available Pressure at System Demand

50.385 @ 472.90

Required Pressure at System Demand

41.582 @ 222.90

Required Pressure at System Demand (Including Hose Allowance at Source)

41.582 @ 472.90



Summary Of Outflowing Devices

Job Number: S23-90G

Report Description: Ordinary Group I

Device		Actual Flow (gpm)	Minimum Flow (gpm)	K-Factor (K)	Pressure (psi)			
Sprinkler	100	21.62	19.50	5.6	14.903			
Sprinkler	103	19.88	19.50	5.6	12.604			
Sprinkler	104	19.80	19.50	5.6	12.496			
Sprinkler	105	20.07	19.50	5.6	12.845			
Sprinkler	107	20.05	19.50	5.6	12.824			
⇒ Sprinkler	108	19.50	19.50	5.6	12.125			
Sprinkler	109	19.73	19.50	5.6	12.410			
Sprinkler	110	19.64	19.50	5.6	12.306			
Sprinkler	600	21.52	19.50	5.6	14.770			
Sprinkler	607	21.46	19.50	5.6	14.681			
Sprinkler	608	19.63	19.50	5.6	12.292			

⇒ Most Demanding Sprinkler Data



Node Analysis

Job Number: S23-90G

Report Description: Ordinary Group I

Node	Elevation(Foot)	Fittings	Pressure(psi)	Discharge(gpm)
17	-6'-11	S	41.582	222.90
100	8'-6	Spr(-14.903)	14.903	21.62
103	10'-9	Spr(-12.604)	12.604	19.88
104	10'-9	Spr(-12.496)	12.496	19.80
105	8'-6	Spr(-12.845)	12.845	20.07
107	8'-6	Spr(-12.824)	12.824	20.05
108	8'-6	Spr(-12.125)	12.125	19.50
109	11'-2	Spr(-12.410)	12.410	19.73
110	11'-2	Spr(-12.306)	12.306	19.64
600	8'-5	Spr(-14.770)	14.770	21.52
607	8'-5	Spr(-14.681)	14.681	21.46
608	8'-5	Spr(-12.292)	12.292	19.63
1	-6'-11	E(24'-2)	36.123	
2	2'-0	PO(26'-4)	32.134	
3	7'-3	E(9'-5)	23.480	
4	0'-0		33.077	
5	7'-9	mecT(26'-4)	21.728	
6	7'-9	mecT(20'-2)	21.124	
7	10'-4	C(9'-11)	16.740	
8	10'-4	PO(5'-0)	15.984	
9	10'-4	PO(5'-0)	15.857	
10	10'-9	C(9'-11)	16.168	
11	10'-9	PO(5'-0)	12.998	
12	10'-9	PO(5'-0)	13.823	
13	10'-9	PO(5'-0)	15.832	
14	11'-2	PO(5'-0)	13.614	
15	11'-2	PO(5'-0)	12.796	
16	11'-2	T(9'-11)	15.923	
18	-6'-11	T(47'-3)	41.565	



Hydraulic Analysis

Job Number: S23-90G
Report Description: Ordinary Group I

Pipe Type	Diameter	Flow	Velocity	HWC	Friction Loss	Length	Pressure
Downstream	Elevation	Discharge	K-Factor	Pt	Pn	Eq. Length	Summary
Upstream						Total Length	
Route 1							
DR	1.0490	19.50	7.24	120	0.124177	5'-7	Pf 1.807
108	8'-6	19.50	5.6	12.125	Sprinkler,	9'-0	Pe -1.136
15	11'-2			12.796	2E(2'-0), PO(5'-0)	14'-7	Pv
BL	1.6820	58.87	8.50	120	0.096209	8'-6	Pf 0.821
15	11'-2	39.37		12.796	Flow (q) from Route 3		Pe -0.002
14	11'-2			13.614		8'-6	Pv
BL	1.6820	78.93	11.40	120	0.165478	4'-1	Pf 2.310
14	11'-2	20.05		13.614	Flow (q) from Route 5	9'-11	Pe -0.001
16	11'-2			15.923	T(9'-11)	14'-0	Pv
CM	3.2600	78.93	3.03	120	0.006594	10'-10	Pf 0.071
16	11'-2			15.923			Pe 0.173
10	10'-9			16.168		10'-10	Pv
CM	3.2600	179.76	6.91	120	0.030233	12'-8	Pf 0.383
10	10'-9	79.38 + 21.46		16.168	Flow (q) from Route 2 and 7		Pe 0.190
7	10'-4			16.740		12'-8	Pv
CM	3.2600	222.90	8.57	120	0.045009	48'-4	Pf 3.264
7	10'-4	43.14		16.740	Flow (q) from Route 8	24'-2	Pe 1.120
6	7'-9			21.124	Ee1(4'-0), mecT(20'-2)	72'-6	Pv
CM	4.2600	222.90	5.02	120	0.012231	23'-0	Pf 0.604
6	7'-9			21.124		26'-4	Pe
5	7'-9			21.728	mecT(26'-4)	49'-4	Pv
CM	3.2600	222.90	8.57	120	0.045009	24'-9	Pf 1.536
5	7'-9			21.728		9'-5	Pe 0.216
3	7'-3			23.480	E(9'-5)	34'-2	Pv
CM	4.2600	222.90	5.02	120	0.012231	5'-3	Pf 6.386
3	7'-3			23.480		26'-4	Pe 2.268
2	2'-0			32.134	BV(-6.000), PO(26'-4)	31'-7	Pv
CM	6.3570	222.90	2.25	120	0.001741	6'-0	Pf 0.076
2	2'-0			32.134		37'-9	Pe 0.867
4	0'-0			33.077	PO(37'-9)	43'-9	Pv
UG	6.4000	222.90	2.22	140	0.001267	6'-11	Pf 0.039
4	0'-0			33.077		24'-2	Pe 3.006
1	-6'-11			36.123	E(24'-2)	31'-2	Pv
UG	6.2800	222.90	2.31	140	0.001389	204'-10	Pf 5.442
1	-6'-11			36.123		113'-6	Pe
18	-6'-11			41.565	3E(22'-1), BFP(-5.000), T(47'-3)	318'-4	Pv
UG	8.3900	222.90	1.29	140	0.000339	50'-9	Pf 0.017
18	-6'-11			41.565			Pe
17	-6'-11			41.582	Water Supply	50'-9	Pv
		250.00			Hose Allowance At Source		
17		472.90					
Route 2							
DR	1.0490	19.63	7.29	120	0.125755	4'-7	Pf 1.710
608	8'-5	19.63	5.6	12.292	Sprinkler,	9'-0	Pe -1.003
11	10'-9			12.998	2E(2'-0), PO(5'-0)	13'-7	Pv
BL	1.6820	59.31	8.56	120	0.097538	8'-5	Pf 0.824
11	10'-9	39.68		12.998	Flow (q) from Route 4		Pe
12	10'-9			13.823		8'-5	Pv
BL	1.6820	79.38	11.46	120	0.167246	4'-1	Pf 2.345
12	10'-9	20.07		13.823	Flow (q) from Route 6	9'-11	Pe
10	10'-9			16.168	C(9'-11)	14'-0	Pv
Route 3							
BL	1.6820	19.64	2.84	120	0.012630	8'-5	Pf 0.106
110	11'-2	19.64	5.6	12.306	Sprinkler		Pe -0.002
109	11'-2			12.410		8'-5	Pv
BL	1.6820	39.37	5.68	120	0.045707	8'-6	Pf 0.389
109	11'-2	19.73	5.6	12.410	Sprinkler		Pe -0.002
15	11'-2			12.796		8'-6	Pv
Route 4							
BL	1.6820	19.80	2.86	120	0.012810	8'-5	Pf 0.108
104	10'-9	19.80	5.6	12.496	Sprinkler		Pe
103	10'-9			12.604		8'-5	Pv
BL	1.6820	39.68	5.73	120	0.046363	8'-6	Pf 0.395
103	10'-9	19.88	5.6	12.604	Sprinkler		Pe
11	10'-9			12.998		8'-6	Pv
Route 5							



Hydraulic Analysis

Job Number: S23-90G
Report Description: Ordinary Group I

Pipe Type	Diameter	Flow	Velocity	HWC	Friction Loss	Length	Pressure
Downstream	Elevation	Discharge	K-Factor	Pt	Pn	Eq. Length	Summary
Upstream					Fittings	Total Length	
DR	1.0490	20.05	7.44	120	0.130781	5'-9	Pf 1.928
107	8'-6	20.05	5.6	12.824	Sprinkler,	9'-0	Pe -1.138
14	11'-2			13.614	2E(2'-0), PO(5'-0)	14'-9	Pv
Route 6							
DR	1.0490	20.07	7.45	120	0.130981	5'-11	Pf 1.953
105	8'-6	20.07	5.6	12.845	Sprinkler,	9'-0	Pe -0.975
12	10'-9			13.823	2E(2'-0), PO(5'-0)	14'-11	Pv
Route 7							
DR	1.0490	21.46	7.97	120	0.148208	5'-8	Pf 2.176
607	8'-5	21.46	5.6	14.681	Sprinkler,	9'-0	Pe -1.025
13	10'-9			15.832	2E(2'-0), PO(5'-0)	14'-8	Pv
BL	1.6820	21.46	3.10	120	0.014869	12'-8	Pf 0.336
13	10'-9			15.832		9'-11	Pe
10	10'-9			16.168	Z, C(9'-11)	22'-7	Pv
Route 8							
DR	1.0490	21.52	7.99	120	0.149043	3'-10	Pf 1.914
600	8'-5	21.52	5.6	14.770	Sprinkler,	9'-0	Pe -0.828
9	10'-4			15.857	2E(2'-0), PO(5'-0)	12'-10	Pv
BL	1.6820	21.52	3.11	120	0.014953	8'-6	Pf 0.127
9	10'-4			15.857			Pe 0.000
8	10'-4			15.984		8'-6	Pv
BL	1.6820	43.14	6.23	120	0.054127	4'-1	Pf 0.756
8	10'-4	21.62		15.984	Flow (q) from Route 9	9'-11	Pe 0.000
7	10'-4			16.740	C(9'-11)	14'-0	Pv
Route 9							
DR	1.0490	21.62	8.03	120	0.150279	3'-5	Pf 1.863
100	8'-6	21.62	5.6	14.903	Sprinkler,	9'-0	Pe -0.782
8	10'-4			15.984	2E(2'-0), PO(5'-0)	12'-5	Pv

Equivalent Pipe Lengths of Valves and Fittings (C=120 only)

C Value Multiplier

$$\left(\frac{\text{Actual Inside Diameter}}{\text{Schedule 40 Steel Pipe Inside Diameter}} \right)^{4.87} = \text{Factor}$$

Value Of C	100	130	140	150
Multiplying Factor	0.713	1.16	1.33	1.51



Hydraulic Analysis

Job Number: S23-90G
Report Description: Ordinary Group I

Pipe Type	Diameter	Flow	Velocity	HWC	Friction Loss		Length	Pressure
Downstream	Elevation	Discharge	K-Factor	Pt	Pn	Fittings	Eq. Length	Summary
Upstream							Total Length	

Pipe Type Legend	Units Legend		Fittings Legend	
AO Arm-Over	Diameter	Inch	ALV Alarm Valve	
BL Branch Line	Elevation	Foot	AngV Angle Valve	
CM Cross Main	Flow	gpm	b Bushing	
DN Drain	Discharge	gpm	BaIV Ball Valve	
DR Drop	Velocity	fps	BFP Backflow Preventer	
DY Dynamic	Pressure	psi	BV Butterfly Valve	
FM Feed Main	Length	Foot	C Cross Flow Turn 90°	
FR Feed Riser	Friction Loss	psi/Foot	cplg Coupling	
MS Miscellaneous	HWC	Hazen-Williams Constant	Cr Cross Run	
OR Outrigger	Pt	Total pressure at a point in a pipe	CV Check Valve	
RN Riser Nipple	Pn	Normal pressure at a point in a pipe	DeIV Deluge Valve	
SN Swing Nipple	Pf	Pressure loss due to friction between points	DPV Dry Pipe Valve	
SP Sprig	Pe	Pressure due to elevation difference between indicated points	E 90° Elbow	
ST Stand Pipe	Pv	Velocity pressure at a point in a pipe	EE 45° Elbow	
UG Underground			Ee1 11¼° Elbow	
			Ee2 22½° Elbow	
			f Flow Device	
			fd Flex Drop	
			FDC Fire Department Connection	
			fE 90° FireLock(TM) Elbow	
			fEE 45° FireLock(TM) Elbow	
			flg Flange	
			FN Floating Node	
			fT FireLock(TM) Tee	
			g Gauge	
			GloV Globe Valve	
			GV Gate Valve	
			Ho Hose	
			Hose Hose	
			HV Hose Valve	
			Hyd Hydrant	
			LtE Long Turn Elbow	
			mecT Mechanical Tee	
			Noz Nozzle	
			P1 Pump In	
			P2 Pump Out	
			PIV Post Indicating Valve	
			PO Pipe Outlet	
			PRV Pressure Reducing Valve	
			PrV Pressure Relief Valve	
			red Reducer/Adapter	
			S Supply	
			sCV Swing Check Valve	
			Spr Sprinkler	
			St Strainer	
			T Tee Flow Turn 90°	
			Tr Tee Run	
			U Union	
			WirF Wirsbo	
			WMV Water Meter Valve	
			Z Cap	

MEMORANDUM

TO: BRIAN JOHNSON, WATER SYSTEM
SPECIALIST
FROM: KERRI SIDEBOTTOM, P.E.
DATE: NOVEMBER 13, 2023
SUBJECT: 111 – 201 WEST MAIN FIRE FLOW
AVAILABILITY
CITY OF PUYALLUP, PIERCE COUNTY,
WASHINGTON
G&O #21415.17

Per your request, I have analyzed the available fire flow at two existing hydrants located at 111 and 201 West Main, in the central part of the City's water service area. The setup of the hydraulic model and the assumptions used to determine the static pressure and available fire flow are noted as follows.

- The available fire flows and pressures are measured at Nodes J2228 and J2230, corresponding to existing hydrants NW290 and SW209, respectively, as shown in the attached Figure 1.
- Water system demands are based on projected 2038 demands and reservoirs are depleted of fire suppression and equalizing storage, as established in the *2019 Water System Plan* (WSP) approved by the Department of Health (DOH). The City's water model was updated in 2021 to reflect additional system improvements since the WSP was developed.
- All pump stations are idle, and the Salmon Springs source is operating at 1,100 gpm.

The hydrants are located in Zone 1, which is supplied by Maplewood Springs and the 15th Avenue SE Reservoirs. The system was modeled as-is, with no new piping proposed at this time.

The available pressure under 2038 peak hour demands at the hydrant is included in Table 1.

TABLE 1

Peak Hour Pressure

Node	Hydrant	Elevation, feet	Peak Hour Pressure, psi
J2228	NW290	44	51
J2230	SW209	45	51



November 13, 2023
Page 2

Available fire flow was measured at two existing hydrants: Hydrant NW290 (Node J2228) and Hydrant SW209 (Node J2230). The first hydrant is located on an existing 6-inch fire main dead-end within the property, while the other is located on an existing 8-inch main in the street. The results of this modeling are included in Table 2. The modeled fire flow is available at any hydrant individually, but not simultaneously.

TABLE 2
Modeled Fire Flow Availability

Node	Hydrant	Available Fire Flow, gpm	Residual Pressure at Available Fire Flow, psi	Minimum System Pressure at Available Fire Flow, psi
J2228	NW290	880 ⁽¹⁾	42	30
J2230	SW209	2,460 ⁽¹⁾	38	30

(1) Limited by maximum system-wide velocity of 10 fps.

Fire flow to both of the hydrants is limited by the 10-fps maximum velocity through the existing 6-inch fire main and the 8-inch pipe on West Main. The dead-end fire main supplying Node J2228 is only 6 inches in diameter and cannot supply more than 880 gpm without exceeding the 10-fps velocity limit. However, Node J2230 is located on an 8-inch diameter main that provides water from two directions, and as such, can sustain a higher flow under the velocity and pressure constraints.

The Department of Health and City Standards for water distribution systems are to meet the peak hourly demand of the system while providing a minimum pressure of 30 psi, system-wide. Under peak daily demand with a fire flow, the system is designed to maintain a minimum pressure of 20 psi, system-wide. Although the peak hourly demand pressure may currently be higher than these standards, the Developer must recognize that the City may not provide pressure higher than 30 psi in the future. The flows and pressures determined in this memo are based on the approximate hydrant elevation at ground level. The Developer may design their sprinkler system for whatever pressure they wish; however, they must recognize and be responsible for conditions when the pressure may be less than currently exists.

KS/sr

PRHM20231549

THE MAP FEATURES ARE APPROXIMATE AND ARE INTENDED ONLY TO PROVIDE AN INDICATION OF SAID FEATURE. THIS IS NOT A SURVEY. THE CITY ASSUMES NO LIABILITY FOR VARIATIONS ASCERTAINED BY ACTUAL SURVEY. ALL DATA IS EXPRESSLY PROVIDED 'AS IS' AND 'WITH ALL FAULTS'. THE CITY MAKES NO WARRANTY OF FITNESS FOR A PARTICULAR PURPOSE.

201

3" SERVICE

6" FIRE

Node J2228
Elevation: 44 ft
PHD Pressure: 51 psi
Modeled Fire Flow: 880 gpm

NW 290

105

8" D.I.

W MAIN

SW 209

2ND ST SW

8" D.I.

122

118

116

112

108

110

106

120

Node J2230
Elevation: 45 ft
PHD Pressure: 51 psi
Modeled Fire Flow: 2,460 gpm

101

103

105

0 20 40 60 80 FEET



CITY OF PUYALLUP
PUBLIC WORKS
WATER DIVISION

HYDRAULIC MODEL FOR 111 - 201 W MAIN

SCALE AS SHOWN

11/08/2023

COP\PW\WATER\S_MAINT\PDF\QSEC\
PG109\MODEL FOR 111-201 W MAIN

TOLBrace™ Seismic Bracing Calculations

Project Address: CHC Puyallup Garage
201 W. Main St
Puyallup, WA
Job # 23-090

Contractor: Archer Construction
Address: 7855 S. 206th Street
Kent, WA 98032
Phone: _____
License: _____



Calculations based on 2016 NFPA Pamphlet #13

Brace Information	TOLCO™ Brace Components																		
Maximum Brace Length <u>7' 0" (2.134 m)</u> Diameter of Brace <u>1" Sch.40</u> Type of Brace <u>Sch. 10</u> Angle of Brace <u>45° Min.</u> Least Rad. of Gyration <u>0.42" (11 mm)</u> L/R Value <u>200</u> Max Horizontal Load <u>1310 lbs (594 kg)</u>	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>TOLCO™ Component</th> <th>Listed Load</th> <th>Adjusted Load</th> </tr> </thead> <tbody> <tr> <td>Fig. 1001 Clamp</td> <td>2015 lbs (914 kg)</td> <td>1425 lbs (646 kg)</td> </tr> <tr> <td>Fig.909 No-Thread Swivel</td> <td>2015 lbs (914 kg)</td> <td>1425 lbs (646 kg)</td> </tr> <tr> <td colspan="3">See Fastener Information</td> </tr> <tr> <td colspan="3" style="text-align: center;">*Calculation Based on CONCENTRIC Loading</td> </tr> <tr> <td colspan="3">*Please Note: These calculations are for TOLCO™ components only. Use of any other components voids these calculations and the listing of the assembly.</td> </tr> </tbody> </table>	TOLCO™ Component	Listed Load	Adjusted Load	Fig. 1001 Clamp	2015 lbs (914 kg)	1425 lbs (646 kg)	Fig.909 No-Thread Swivel	2015 lbs (914 kg)	1425 lbs (646 kg)	See Fastener Information			*Calculation Based on CONCENTRIC Loading			*Please Note: These calculations are for TOLCO™ components only. Use of any other components voids these calculations and the listing of the assembly.		
TOLCO™ Component	Listed Load	Adjusted Load																	
Fig. 1001 Clamp	2015 lbs (914 kg)	1425 lbs (646 kg)																	
Fig.909 No-Thread Swivel	2015 lbs (914 kg)	1425 lbs (646 kg)																	
See Fastener Information																			
*Calculation Based on CONCENTRIC Loading																			
*Please Note: These calculations are for TOLCO™ components only. Use of any other components voids these calculations and the listing of the assembly.																			
	Seismic Brace Assembly Detail																		
Fastener Information Orientation to Connecting Surface <u>NFPA Type B</u> Fastener Type <u>POWERS POWER-STUD +SD2</u> Diameter <u>1/2in.</u> Length <u>3 1/4in.</u> Maximum Load <u>499 lbs (226 kg)</u> Prying Factor <u>1.002</u>	Brace Identification on Plans <u>C1</u> Brace Type Lateral [X] Longitudinal [] 4-Way []																		

Sprinkler System Load Calculation (Fpw = CpWp)					
Cp = <u>0.67</u>					
Diameter	Type	Length	Total Length	Weight Per Unit Length	Total Weight
4" (100 mm)	Sch. 10	40 ft (12.2 m)	40 ft (12.2 m)	11.78 lb/ft (17.53 kg/m)	471 lbs (214 kg)
Subtotal Weight					471 lbs (214 kg)
Wp (incl. 15%)					542 lbs (246 kg)
Total (Fpw)					363 lbs (165 kg)
Maximum Fpw per 9.3.5.5.2 (if applicable)					769 lb (348 kg)

TOLBrace™ Seismic Calculation

CHC Puyallup Garage

Job # 23-090

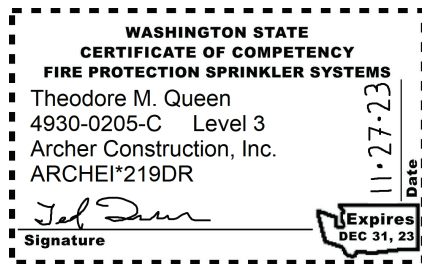
201 W. Main St



Brace Identification	C1
Brace Type (Per NFPA#13)	NFPA Type B
Braced Pipe (ft)	4" Sch.10 Steel Pipe
Spacing of Brace	40' 0" (12.19 m)
Orientation of Brace	Lateral
Bracing Material	1" Sch.40
Maximum Brace Length	7' 0" (2.13 m)
Slenderness Ratio used for Load Calculation	200
True Angle of Brace for Calculation	45°
Type of Fastener	POWERS POWER-STUD +SD2 1/2in. x 3 1/4in. (3,000 PSI Light Weig
Length of Fastener	3 1/4in.

Summary of Pipe within Zone of Influence

4" Sch.10 Steel Pipe (101.6 mm)	40 ft (12.2 m)
---------------------------------	----------------



G-Factor Used 0.67

Allowance for Heads and Fittings	15%
----------------------------------	-----

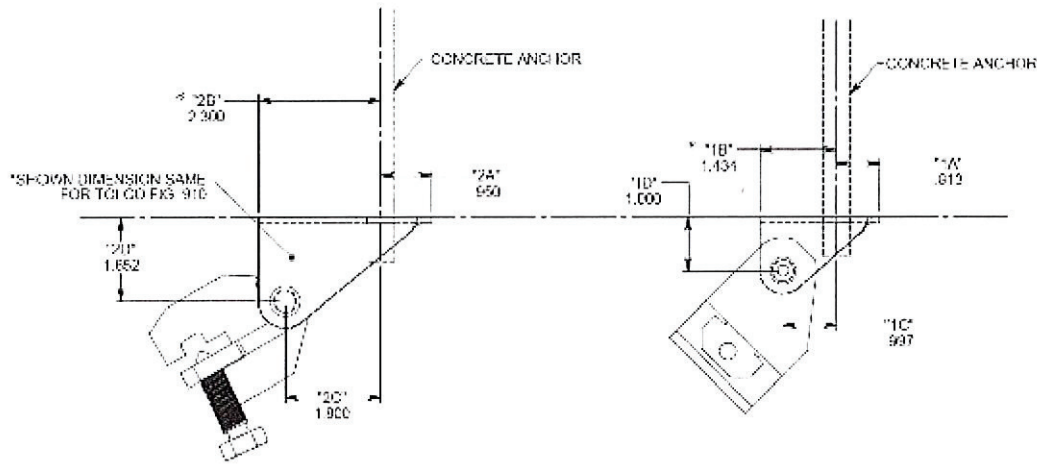
Conclusions

Total Adjusted Load of Pipe in Zone of Influence	363 lbs (165 kg)
Material Capacity	1310 lbs (594 kg)
Fastener Capacity	499 lbs (226 kg)
Fig. 1001 Clamp	1425 lbs (646 kg)
Fig.909 No-Thread Swivel	1425 lbs (646 kg)
Structural Member	Concrete Deck

Calculations prepared by Queen

* The description of the Structural Member is for informational purposes only.
TOLBrace™ software calculates the brace assembly only, not the structure it is attached to.
Calculated with TOLBrace™ 8
Visit us at www.tolco.com

DETAIL PER NFPA 13, 2016 FIGURE A9.3.5.12.1(a - c)



TOLCO FIG. 980

TOLCO FIG. 909

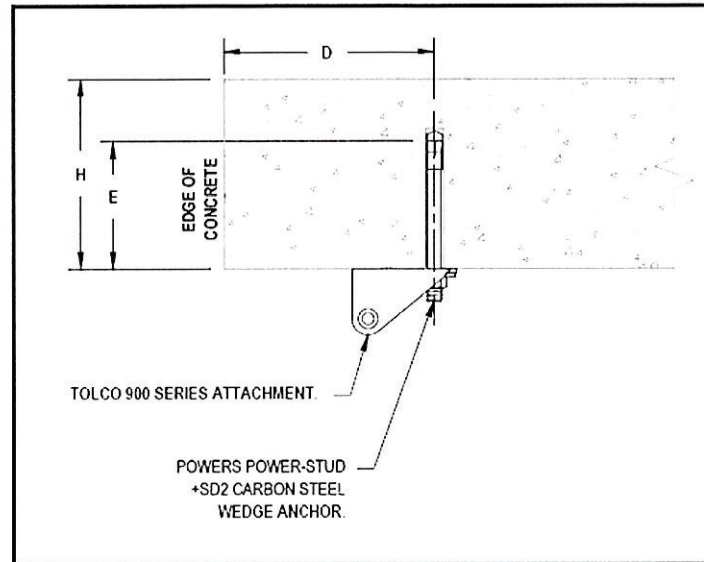
Prying Factors per NFPA 13, 2016 Section 9.3.5.12 when installed in concrete slab decks								
Fig. 980/910								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
3.275	1.156	1.738	1.461	1.850	2.894	3.478	2.459	2.008
Fig. 909								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
2.626	1.002	1.230	1.513	1.487	2.226	2.460	1.740	1.420

Prying Factors per NFPA 13, 2016 Section 9.3.5.12 when installed in concrete metal decks with 1" center offset								
Fig. 980/910								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
3.275	1.156	1.738	-	-	-	-	-	-
Fig. 909								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
2.626	1.002	1.230	-	-	-	-	-	-

Prying Factors per NFPA 13, 2016 Section 9.3.5.12 when installed in concrete metal decks with 1.125" center offset								
Fig. 980/910								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
3.275	1.156	1.738	-	-	-	-	-	-
Fig. 909								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
2.626	1.003	1.230	-	-	-	-	-	-

*When installed in a concrete metal deck (Type W 4 1/2" x 3"), dimension 'B' would be dependent upon the contact area. For SD2 anchors the max offset is 1" so 'B' would be 1.25". For Bang-It and Wond-Knocker II + anchors, the max offset is 1.125" so 'B' would be 1.125".

POWERS POWER-STUD+SD2 WEDGE ANCHORS IN 3,000 PSI LIGHT WEIGHT CONCRETE



NFPA BRACE ORIENTATIONS A, B, AND C SHOWN ABOVE (MINUS ANGLES), OTHER ORIENTATIONS SIMILAR

Fig. 980/ Fig. 910													
ANCHOR DIA.	NOMINAL EMBED. DEPTH	"E" MIN. EFFECTIVE EMBED. DEPTH	"D" DESIGN EDGE DISTANCE	"H" MIN. BASE MATERIAL THICKNESS	ALLOWABLE STRENGTH DESIGN (ASD)								
					MAX. ALLOWABLE HORIZONTAL LOAD (LBS.)								
					NFPA BRACE ORIENTATION PRYING FACTOR								
					A	B	C	D	E	F	G	H	I
					3.275	1.156	1.738	1.461	1.850	2.894	3.478	2.459	2.008
3/8"	2 3/8"	2"	4"	4"	106	218	169	149	162	127	84	119	146
1/2"	3 3/4"	3 1/4"	6"	5 3/4"	222	460	355	315	340	262	177	250	308
5/8"	3 7/8"	3 1/4"	6"	5 3/4"	225	473	363	326	347	262	181	255	314
3/4"	4 1/2"	3 3/4"	8"	7"	287	644	483	453	461	325	241	340	418

VALUES CALCULATED IN ACCORDANCE WITH NFPA 13-16 ANNEX E.7 AND INCLUDE OVER STRENGTH FACTOR $\phi = 2.0$ AND AMPLIFIED BY 1.2

Fig. 909													
ANCHOR DIA.	NOMINAL EMBED. DEPTH	"E" MIN. EFFECTIVE EMBED. DEPTH	"D" DESIGN EDGE DISTANCE	"H" MIN. BASE MATERIAL THICKNESS	ALLOWABLE STRENGTH DESIGN (ASD)								
					MAX. ALLOWABLE HORIZONTAL LOAD (LBS.)								
					NFPA BRACE ORIENTATION PRYING FACTOR								
					A	B	C	D	E	F	G	H	I
					2.626	1.002	1.230	1.513	1.487	2.226	2.460	1.740	1.420
3/8"	2 3/8"	2"	4"	4"	126	236	210	146	187	161	105	147	182
1/2"	3 3/4"	3 1/4"	6"	5 3/4"	264	499	444	310	394	337	221	311	384
5/8"	3 7/8"	3 1/4"	6"	5 3/4"	268	514	455	320	403	341	227	320	394
3/4"	4 1/2"	3 3/4"	8"	7"	350	706	618	443	542	423	309	434	535

VALUES CALCULATED IN ACCORDANCE WITH NFPA 13-16 ANNEX E.7 AND INCLUDE OVER STRENGTH FACTOR $\phi = 2.0$ AND AMPLIFIED BY 1.2

NOTES:

- 1.) ALLOWABLE LOADS ARE FOR ANCHORS INSTALLED IN STONE AGGREGATE CONCRETE HAVING A MIN. COMPRESSIVE STRENGTH OF 3,000 PSI AT THE TIME OF INSTALLATION AND DETERMINED PER ICC ESR-2502 FOR ANCHORS IN CRACKED CONCRETE. ALLOWABLE LOADS HAVE BEEN CALCULATED IN ACCORDANCE WITH ACI 318-14, CHAPTER 17
- 2.) PER ICC-ESR, PERIODIC SPECIAL INSPECTIONS ARE REQUIRED IN ACCORDANCE WITH SECTION 1705.1.1 AND TABLE 1705.3 OF THE 2015 AND 2012 IBC. THE SPECIAL INSPECTOR MUST MAKE PERIODIC INSPECTIONS DURING THE ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE COMPRESSIVE STRENGTH, HOLE DIMENSIONS, HOLE CLEANING PROCEDURE, ANCHOR SPACING, EDGE DISTANCES, CONCRETE MEMBER THICKNESS, ANCHOR EMBEDMENT, TIGHTENING TORQUE AND ADHERENCE TO THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. CONSULT LOCAL CODES FOR ANY ADDITIONAL SPECIAL INSPECTION REQUIREMENTS.
- 3.) FOLLOW ALL WEDGE ANCHOR INSTALLATION REQUIREMENTS PER ICC ESR-2502
- 4.) WHEN INSTALLING ANCHORS IN REINFORCED CONCRETE, AVOID DAMAGING REINFORCING STEEL
- 5.) WHEN INSTALLING ANCHORS IN PRESTRESSED CONCRETE, LOCATE PRESTRESSING STEEL AND AVOID DAMAGING PRESTRESSING STEEL.
- 6.) STRUCTURAL ENGINEER OF RECORD SHALL VERIFY ADEQUACY OF THE STRUCTURE FOR THE TABULATED ALLOWABLE LOADS.
- 7.) TOLCO 900 SERIES ATTACHMENT DIAMETER SHALL BE EQUAL TO THE ANCHOR DIAMETER
- 8.) IF SUBSTRATE CONDITIONS DIFFER FROM WHAT IS LISTED IN THE ABOVE TABLES, CONTACT EATON B-LINE AT TOLCOSUPPORT@EATON.COM

TOLBrace™ Seismic Bracing Calculations

Project Address: CHC Puyallup Garage
 201 W. Main St
 Puyallup, WA
 Job # 23-090

Contractor: Archer Construction
Address: 7855 S. 206th Street
 Kent, WA 98032
Phone:
License:



Calculations based on 2016 NFPA Pamphlet #13

Brace Information	TOLCO™ Brace Components																		
Maximum Brace Length 7' 0" (2.134 m)	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>TOLCO™ Component</th> <th>Listed Load</th> <th>Adjusted Load</th> </tr> </thead> <tbody> <tr> <td>Fig. 4L Clamp</td> <td>2015 lbs (914 kg)</td> <td>1425 lbs (646 kg)</td> </tr> <tr> <td>Fig. 909 No-Thread Swivel</td> <td>2015 lbs (914 kg)</td> <td>1425 lbs (646 kg)</td> </tr> <tr> <td colspan="3">See Fastener Information</td> </tr> <tr> <td colspan="3" style="text-align: center;">*Calculation Based on CONCENTRIC Loading</td> </tr> <tr> <td colspan="3">*Please Note: These calculations are for TOLCO™ components only. Use of any other components voids these calculations and the listing of the assembly.</td> </tr> </tbody> </table>	TOLCO™ Component	Listed Load	Adjusted Load	Fig. 4L Clamp	2015 lbs (914 kg)	1425 lbs (646 kg)	Fig. 909 No-Thread Swivel	2015 lbs (914 kg)	1425 lbs (646 kg)	See Fastener Information			*Calculation Based on CONCENTRIC Loading			*Please Note: These calculations are for TOLCO™ components only. Use of any other components voids these calculations and the listing of the assembly.		
TOLCO™ Component	Listed Load	Adjusted Load																	
Fig. 4L Clamp	2015 lbs (914 kg)	1425 lbs (646 kg)																	
Fig. 909 No-Thread Swivel	2015 lbs (914 kg)	1425 lbs (646 kg)																	
See Fastener Information																			
*Calculation Based on CONCENTRIC Loading																			
*Please Note: These calculations are for TOLCO™ components only. Use of any other components voids these calculations and the listing of the assembly.																			
Diameter of Brace 1" Sch.40																			
Type of Brace Sch. 10																			
Angle of Brace 45° Min.																			
Least Rad. of Gyration 0.42" (11 mm)																			
L/R Value 200																			
Max Horizontal Load 1310 lbs (594 kg)																			
Fastener Information	Seismic Brace Assembly Detail																		
Orientation to Connecting Surface NFPA Type B																			
Fastener Type POWERS POWER-STUD +SD2																			
Diameter 1/2in.																			
Length 3 1/4in.																			
Maximum Load 672 lbs (305 kg)																			
Prying Factor 1.002	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Brace Identification on Plans</th> </tr> </thead> <tbody> <tr> <td>Brace Type</td> <td>Lateral [] Longitudinal [X] 4-Way []</td> </tr> </tbody> </table>	Brace Identification on Plans		Brace Type	Lateral [] Longitudinal [X] 4-Way []														
Brace Identification on Plans																			
Brace Type	Lateral [] Longitudinal [X] 4-Way []																		

Sprinkler System Load Calculation (Fpw = CpWp)

Cp = 0.67

Diameter	Type	Length	Total Length	Weight Per Unit Length	Total Weight
4" (100 mm)	Sch. 10	70 ft (21.3 m)	70 ft (21.3 m)	11.78 lb/ft (17.53 kg/m)	825 lbs (374 kg)
Subtotal Weight					825 lbs (374 kg)
Wp (incl. 15%)					949 lbs (430 kg)
Total (Fpw)					636 lbs (288 kg)
Maximum Fpw per 9.3.5.5.2 (if applicable)					N/A

TOLBrace™ Seismic Calculation

CHC Puyallup Garage

Job # 23-090

201 W. Main St



Brace Identification	C2
Brace Type (Per NFPA#13)	NFPA Type B
Braced Pipe (ft)	4" Sch.10 Steel Pipe
Spacing of Brace	70' 0" (21.34 m)
Orientation of Brace	Longitudinal
Bracing Material	1" Sch.40
Maximum Brace Length	7' 0" (2.13 m)
Slenderness Ratio used for Load Calculation	200
True Angle of Brace for Calculation	45°
Type of Fastener	POWERS POWER-STUD +SD2 1/2in. x 3 1/4in. (3,000 PSI Normal W
Length of Fastener	3 1/4in.

Summary of Pipe within Zone of Influence

4" Sch.10 Steel Pipe (101.6 mm)	70 ft (21.3 m)
---------------------------------	----------------



G-Factor Used 0.67

Allowance for Heads and Fittings	15%
----------------------------------	-----

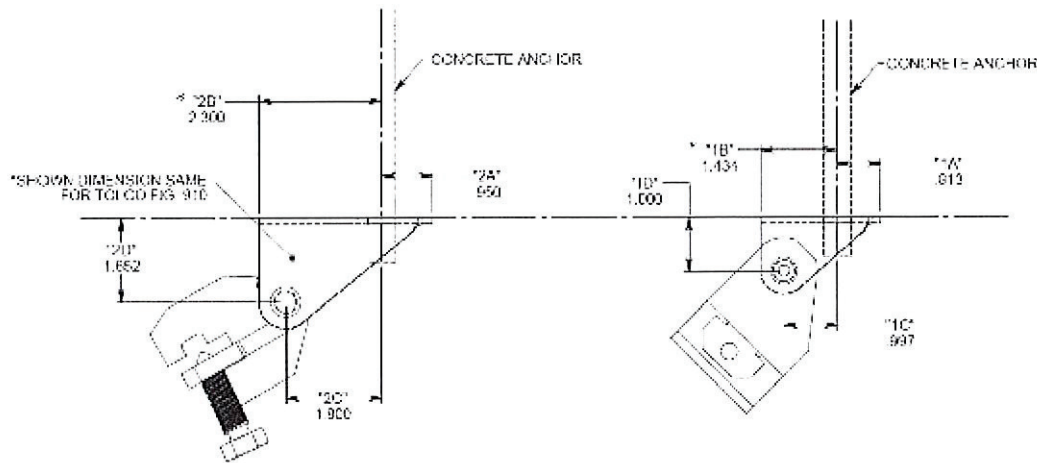
Conclusions

Total Adjusted Load of Pipe in Zone of Influence	635 lbs (288 kg)
Material Capacity	1310 lbs (594 kg)
Fastener Capacity	672 lbs (305 kg)
Fig. 4L Clamp	1425 lbs (646 kg)
Fig.909 No-Thread Swivel	1425 lbs (646 kg)
Structural Member	Concrete Deck

Calculations prepared by Queen

* The description of the Structural Member is for informational purposes only.
TOLBrace™ software calculates the brace assembly only, not the structure it is attached to.
Calculated with TOLBrace™ 8
Visit us at www.tolco.com

DETAIL PER NFPA 13, 2016 FIGURE A9.3.5.12.1(a - c)



TOLCO FIG. 980

TOLCO FIG. 909

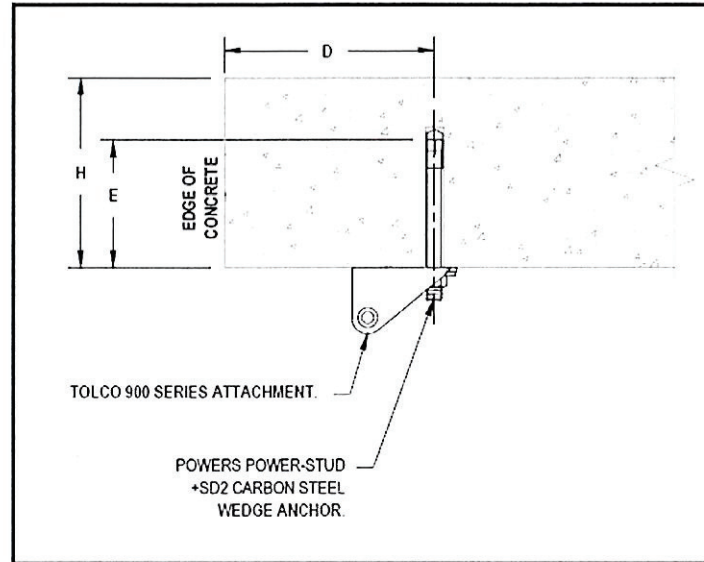
Prying Factors per NFPA 13, 2016 Section 9.3.5.12 when installed in concrete slab decks								
Fig. 980/910								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
3.275	1.156	1.738	1.461	1.830	2.894	3.478	2.459	2.008
Fig. 909								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
2.626	1.002	1.230	1.513	1.487	2.226	2.460	1.740	1.420

Prying Factors per NFPA 13, 2016 Section 9.3.5.12 when installed in concrete metal decks with 1" center offset								
Fig. 980/910								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
3.275	1.156	1.738						
Fig. 909								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
2.626	1.002	1.230	-	-	-	-	-	-

Prying Factors per NFPA 13, 2016 Section 9.3.5.12 when installed in concrete metal decks with 1.125" center offset								
Fig. 980/910								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
3.275	1.156	1.738	-	-	-	-	-	-
Fig. 909								
A	B	C	D	E	F	G	H	I
P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r	P _r
2.626	1.003	1.230	-	-	-	-	-	-

*When installed in a concrete metal deck (Type W 4 1/2" x 3"), dimension 'B' would be dependent upon the contact area. For SD2 anchors the max offset is 1" so 'B' would be 1.25". For Bang-It and Wopac-Knocker II+ anchors, the max offset is 1.125" so 'B' would be 1.125".

POWERS POWER-STUD+SD2 WEDGE ANCHORS IN 3,000 PSI NORMAL WEIGHT CONCRETE



NFPA BRACE ORIENTATIONS A, B, AND C SHOWN ABOVE (MINUS ANGLES), OTHER ORIENTATIONS SIMILAR

Fig. 980/ Fig. 910													
ANCHOR DIA.	NOMINAL EMBED. DEPTH	"E" MIN. EFFECTIVE EMBED. DEPTH	"D" DESIGN EDGE DISTANCE	"H" MIN. BASE MATERIAL THICKNESS	ALLOWABLE STRENGTH DESIGN (ASD)								
					MAX. ALLOWABLE HORIZONTAL LOAD (LBS.)								
					NFPA BRACE ORIENTATION PRYING FACTOR								
					A	B	C	D	E	F	G	H	I
					3.275	1.156	1.738	1.461	1.850	2.894	3.478	2.459	2.008
3/8"	2 3/8"	2"	4"	4"	140	296	227	204	217	163	113	159	196
1/2"	3 3/4"	3 1/4"	6"	5 3/4"	292	618	473	427	453	339	236	332	409
5/8"	3 7/8"	3 1/4"	6"	5 3/4"	331	696	534	479	511	386	267	375	462
3/4"	4 1/2"	3 3/4"	8"	7"	422	929	700	650	669	478	350	493	606

VALUES CALCULATED IN ACCORDANCE WITH NFPA 13-16 ANNEX E.7 AND INCLUDE OVER STRENGTH FACTOR $\phi = 2.0$ AND AMPLIFIED BY 1.2

Fig. 909													
ANCHOR DIA.	NOMINAL EMBED. DEPTH	"E" MIN. EFFECTIVE EMBED. DEPTH	"D" DESIGN EDGE DISTANCE	"H" MIN. BASE MATERIAL THICKNESS	ALLOWABLE STRENGTH DESIGN (ASD)								
					MAX. ALLOWABLE HORIZONTAL LOAD (LBS.)								
					NFPA BRACE ORIENTATION PRYING FACTOR								
					A	B	C	D	E	F	G	H	I
					2.626	1.002	1.230	1.513	1.487	2.226	2.460	1.740	1.420
3/8"	2 3/8"	2"	4"	4"	167	322	285	201	252	211	142	200	247
1/2"	3 3/4"	3 1/4"	6"	5 3/4"	348	672	595	419	526	440	297	417	515
5/8"	3 7/8"	3 1/4"	6"	5 3/4"	394	756	670	471	594	501	335	470	580
3/4"	4 1/2"	3 3/4"	8"	7"	509	1017	892	637	784	621	446	627	773

VALUES CALCULATED IN ACCORDANCE WITH NFPA 13-16 ANNEX E.7 AND INCLUDE OVER STRENGTH FACTOR $\phi = 2.0$ AND AMPLIFIED BY 1.2

NOTES:

- 1.) ALLOWABLE LOADS ARE FOR ANCHORS INSTALLED IN STONE AGGREGATE CONCRETE HAVING A MIN. COMPRESSIVE STRENGTH OF 3,000 PSI AT THE TIME OF INSTALLATION AND DETERMINED PER ICC ESR-2502 FOR ANCHORS IN CRACKED CONCRETE. ALLOWABLE LOADS HAVE BEEN CALCULATED IN ACCORDANCE WITH ACI 318-14, CHAPTER 17
- 2.) PER ICC-ESR, PERIODIC SPECIAL INSPECTIONS ARE REQUIRED IN ACCORDANCE WITH SECTION 1705.1.1 AND TABLE 1705.3 OF THE 2015 AND 2012 IBC. THE SPECIAL INSPECTOR MUST MAKE PERIODIC INSPECTIONS DURING THE ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE COMPRESSIVE STRENGTH, HOLE DIMENSIONS, HOLE CLEANING PROCEDURE, ANCHOR SPACING, EDGE DISTANCES, CONCRETE MEMBER THICKNESS, ANCHOR EMBEDMENT, TIGHTENING TORQUE AND ADHERENCE TO THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. CONSULT LOCAL CODES FOR ANY ADDITIONAL SPECIAL INSPECTION REQUIREMENTS.
- 3.) FOLLOW ALL WEDGE ANCHOR INSTALLATION REQUIREMENTS PER ICC ESR-2502
- 4.) WHEN INSTALLING ANCHORS IN REINFORCED CONCRETE, AVOID DAMAGING REINFORCING STEEL
- 5.) WHEN INSTALLING ANCHORS IN PRESTRESSED CONCRETE, LOCATE PRESTRESSING STEEL AND AVOID DAMAGING PRESTRESSING STEEL.
- 6.) STRUCTURAL ENGINEER OF RECORD SHALL VERIFY ADEQUACY OF THE STRUCTURE FOR THE TABULATED ALLOWABLE LOADS.
- 7.) TOLCO 900 SERIES ATTACHMENT DIAMETER SHALL BE EQUAL TO THE ANCHOR DIAMETER
- 8.) IF SUBSTRATE CONDITIONS DIFFER FROM WHAT IS LISTED IN THE ABOVE TABLES, CONTACT EATON B-LINE AT TOLCOSUPPORT@EATON.COM

ASCE 7 HAZARD TOOL

Location

201 W Main, Puyallup, Washington, 98371

Elevation

48 ft with respect to North American Vertical Datum of 1988 (NAVD 88)

Lat:

47.192084

Long:

-122.29529

Standard:

ASCE/SEI 7-22

Risk Category:

II

Soil Class:

Default

Seismic

Overlay ☐

Risk Category II

DETAILS

FULL REPORT

SUMMARY

All data are per the requirements of the ASCE/SEI 7 standard; local requirements may vary.

ASCE

?

© 2023

ASCE 7 Online

A faster, easier way to work with Standard ASCE 7

REPORT SUMMARY

X

Site Information

Address: 201 W Main, Puyallup, Washington, 98371

Elevation: 48 ft (NAVD 88)

Lat: 47.192084

Long: -122.29529

Standard: ASCE/SEI 7-22

Risk Category: II

Soil Class: Default

Seismic Data

S_D

1.44

S_1

0.43

S_{M18}

1.59

S_{M18}

0.91

S_{S18}

1.05

S_{L18}

0.61

T_L

6

PGA_M

0.56

V_{S30}

250

Seismic Design Category

D

Note

Where values of the multi-period 5%-damped MCER response spectrum are not available from the USGS Seismic Design Geodatabase, the design response spectrum shall be permitted to be determined in accordance with Section 11.4.5.2.

Linear interpolator

Fill in five values and leave one blank. Click the Calculate button, and the blank value will be filled in by linear interpolation. ([Help and details](#))

x	y
1.4	.65
1.44	0.67
1.5	.7

Calculate