



City of Puyallup
Building
REVIEWED
FOR
COMPLIANCE
BSnowden

11/17/2023 7:49:07 AM



Calculations required to be provided by the Permittee on site for all Inspections



STRUCTURAL ANALYSIS

TAC Ferris Wheel

Rooftop Site

110 9th Ave SW Puyallup, WA 98371



250 4th Ave S Ste 200 Edmonds, WA 98020 Phone: (425) 778-8500 Fax: (425) 778-5536

CG Project No.: 23088.203

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INTRODUCTION

CG Engineering was retained by Lynx Consulting to provide structural analysis of the existing rooftop structure for the site modifications proposed by Verizon.

The structural analysis completed by CG Engineering was inclusive of the structural elements that were affected by the addition of equipment and antennas associated with the proposed Verizon site modifications. Where applicable, this includes the antenna and equipment support structure and affected portions of the existing main structure.

SITE DESCRIPTION

This site is located on the exterior wall of a one-story concrete building. The appurtenances are mounted to flush wall mounts. The equipment cabinets are located on a concrete equipment pad at grade.

Lynx Consulting provided us with original architectural drawings dated 04/29/14. Photos of the site were also provided for the proposed revisions. All geometry, member sizes, and material strengths used in our analysis were based on this information. If anything differs from the information contained in these documents, CG Engineering should be notified to revise our analysis.

APPURTENANCE CONFIGURATION

The structure was analyzed using the appurtenance configuration specified in the following table. All loading was provided to us from Lynx Consulting. This table includes all known existing and future antennas for this site.

Sector	Existing Appurtenance Configuration	Proposed Appurtenance Configuration (Bold=New)	Mount Type
G	(1) Amphenol Antennas HTXCWW4513FX00 (1) Ericsson RRU (#RRUS12 B2) (1) Ericsson RRU (#RRUS12 B4) (1) Ericsson RRU (#RRUS11 B13) (1) Raycap OVP Box (#OVP6)	(1) Amphenol Antennas HTXCWW4513FX00 (1) Ericsson Antennas KRE105281/1 (1) Ericsson Antennas AIR 4435 (1) Ericsson RRU (#RRUS11 B13) (1) Raycap OVP Box (#OVP6) (1) Ericsson RRU (#4402 B2 DC) (1) Ericsson RRU (#4402 B66A DC)	Flush Wall Mount

The coax cables that serve the antennas weigh less than 5 lb/ft and are therefore exempt from the requirements of Chapter 13 of ASCE 7.

ANALYSIS CRITERIA

The parameters in the following table were used in our analysis of the structure based on its location.

City of Puyallup, WA								
Wind Criteria Seismic Criteria								
Basic Wind Speed w/o Ice (3-s Gust):	110 mph	Risk Category:	II	Sds:	1.016			
Exposure:	С	Kzt (ASCE 7-16):	1.0	Sd1:	0.543			
Notes: 1. Refer to the attached topographic profile(s) used to determine the topographic factor.								
2 Parameters hased on the 2018	International	Building Code (IBC) and	rafarancad st	andards				



CONCLUSIONS/RECOMMENDATIONS

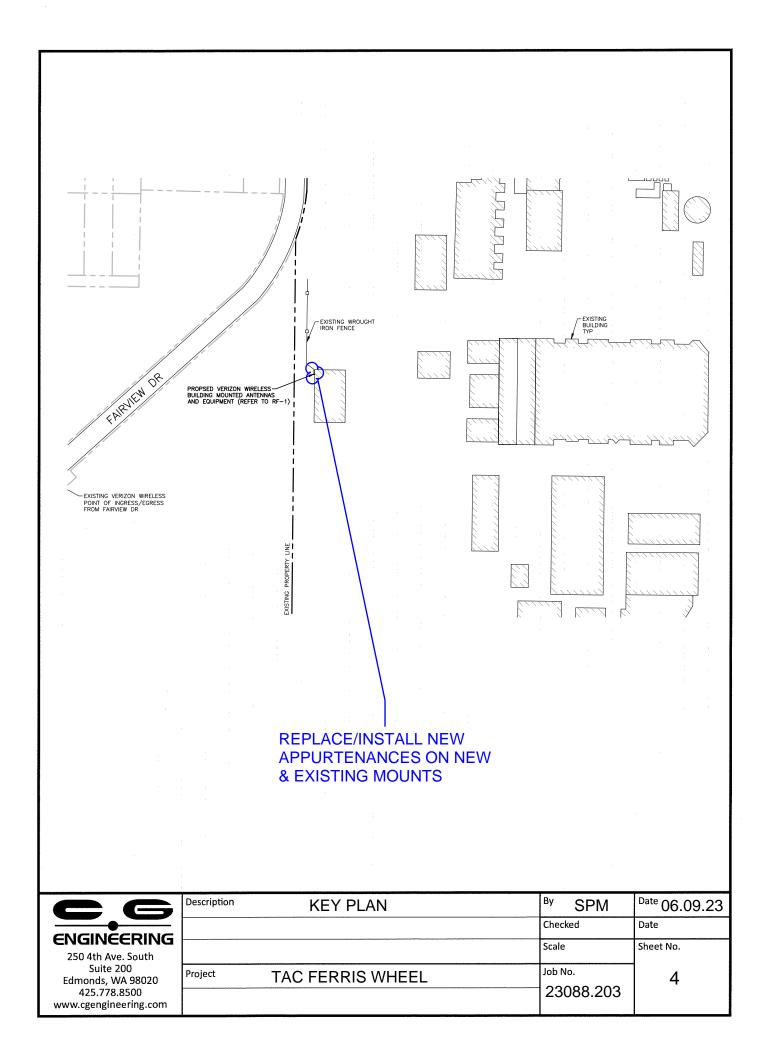
We have determined that no upgrades to the existing structures are required for the proposed Verizon installation as described above in the appurtenance and equipment configuration tables. Refer to the mount analysis report by CG Engineering under a separate cover for analysis of the proposed antenna mounts.

CONDITIONS OF ANALYSIS

This structural analysis is based on the documentation that was available to us. CG Engineering did not perform an observation of this site to verify the accuracy of the provided structure and appurtenance data, and we should be contacted immediately if there are any discrepancies with the information stated within this report.

Our analysis is based on the assumption that the original structural design and all subsequent structural analyses were properly designed and permitted per the applicable building codes, and that the structure has been properly installed and is maintained to the minimum standards required by code. We assume the structure has no known deterioration or damage that would adversely affect its capacity.





▲ This is a beta release of the new ATC Hazards by Location website. Please contact us with feedback.

1 The ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

ATC Hazards by Location

Search Information

Address: 110 9th Ave SW, Puyallup, WA 98371, USA

Coordinates: 47.1846168, -122.2947465

Elevation: 43 ft

Timestamp: 2023-06-08T23:07:20.080Z

Hazard Type: Wind



ASCE 7-16		ASCE 7-10		ASCE 7-05	
MRI 10-Year	. 67 mph	MRI 10-Year	72 mph	ASCE 7-05 Wind Speed	85 mph
MRI 25-Year	. 73 mph	MRI 25-Year	79 mph		
MRI 50-Year	. 78 mph	MRI 50-Year	85 mph		
MRI 100-Year	. 82 mph	MRI 100-Year	91 mph		
Risk Category I	92 mph	Risk Category I	100 mph		
Risk Category II	97 mph	Risk Category II	110 mph		
Risk Category III	104 mph	Risk Category III-IV	115 mph		
Risk Category IV	108 mph	110 PER CITY (PUYALLUP)F		

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Please note that the ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area – in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.

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Table R3	Table R301.2(1)										
Climatic	Climatic and Geographical Design Criteria										
Ground	Wind De	sign	Seismic	Subject to Dar	nage from		Winter	Ice Shield	Flood	Air	Mean
Snow Load	Speed d (mph)	Topograp hical effects ^k	Design Category ^f	Weathering ^a	Frost Line Depth ^b	Termites ^c	Design Temp ^e	Underlay h	Hazards ^g	Freeze Index	Annual Temp ^j
20 lbs/fiv	85	No	D-1	Moderate	12 inches	Slight to Moderate	17°	No	Puyallup Municipal Code 21.07	250	50°

85 MPH FACTORED WIND SPEED; 110 MPH DESIGN WIND SPEED ▲ This is a beta release of the new ATC Hazards by Location website. Please contact us with feedback

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ATC Hazards by Location

Search Information

Address: 110 9th Ave SW, Puyallup, WA 98371, USA

Coordinates: 47.1846168, -122.2947465

Elevation: 43 ft

Timestamp: 2023-06-08T23:07:58.541Z

D-default

Hazard Type: Seismic

Reference Document: ASCE7-16

Risk Category: II



Basic Parameters

Site Class:

Name	Value	Description
S _S	1.27	MCE _R ground motion (period=0.2s)
S ₁	0.437	MCE _R ground motion (period=1.0s)
S _{MS}	1.524	Site-modified spectral acceleration value
S _{M1}	* null	Site-modified spectral acceleration value
S _{DS}	1.016	Numeric seismic design value at 0.2s SA
S _{D1}	* null	Numeric seismic design value at 1.0s SA

^{*} See Section 11.4.8

▼Additional Information

Name	Value	Description
SDC	* null	Seismic design category
Fa	1.2	Site amplification factor at 0.2s
F _v	* null	Site amplification factor at 1.0s
CR _S	0.914	Coefficient of risk (0.2s)
CR ₁	0.898	Coefficient of risk (1.0s)
PGA	0.5	MCE _G peak ground acceleration
F _{PGA}	1.2	Site amplification factor at PGA
PGA _M	0.6	Site modified peak ground acceleration
T_L	6	Long-period transition period (s)
SsRT	1.27	Probabilistic risk-targeted ground motion (0.2s)
SsUH	1.39	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
SsD	1.5	Factored deterministic acceleration value (0.2s)
S1RT	0.437	Probabilistic risk-targeted ground motion (1.0s)
S1UH	0.487	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
S1D	0.6	Factored deterministic acceleration value (1.0s)
PGAd	0.5	Factored deterministic acceleration value (PGA)

^{*} See Section 11.4.8

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

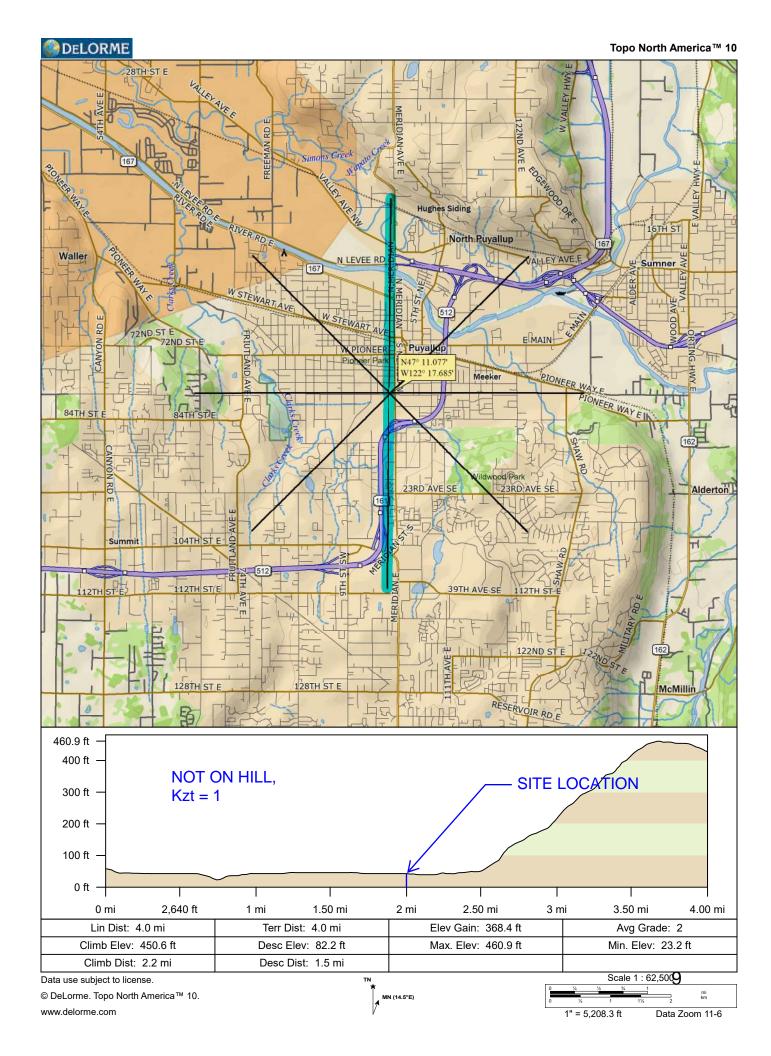
Please note that the ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

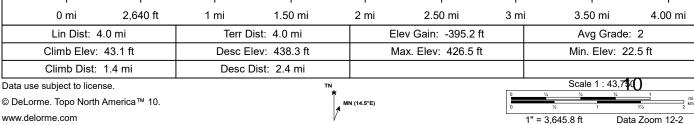
Disclaimer

Hazard loads are provided by the U.S. Geological Survey Seismic Design Web Services.

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7





1" = 3,645.8 ft

Data Zoom 12-2

www.delorme.com

Seismic Load Calculation for Components and System

(Reference: 2018 IBC Section 1613 & ASCE 7-16 Section 13.3)

Seismic Force:

0.2s Spectral Response Acceleration, Site Class B, S _c	=	1.270	(ASCE 7, I	Figure 22-1 thru	22-8)
1.0s Spectral Response Acceleration, Site Class B, S ₁	_	0.437	•	Figure 22-1 thru	
, , , , , ,		D (assumed /	,		,
Site Class	=	default)	(ASCE 7, 5	Section 11.4.3)	
Seismic Design Category	=	D	(ASCE 7,	Tables 11.6-1 &	11.6-2)
Site Coefficient per S _s & Site Class, F _a	=	1.20	(ASCE 7,	Table 11.4-1)	
Site Coefficient per S ₁ & Site Class, F _v	=	1.86	(ASCE 7,	Table 11.4-2)	
$S_{MS} = F_a S_s$	=	1.524	(ASCE 7, 5	Section 11.4.4)	
$S_{M1} = F_v S_1$	=	0.814	(ASCE 7, 5	Section 11.4.4)	
$S_{DS} = 2/3 S_{MS}$	=	1.016	(ASCE 7, 5	Section 11.4.5)	
$S_{D1} = 2/3 S_{M1}$	=	0.543	(ASCE 7, 5	Section 11.4.5)	
(Per ASCE 7-16, 13.3)					
Component Amplification Factor, a _p	=	1.0	(ASCE 7,	Table 13.6-1)	
Component Response Modification Factor, $\mathbf{R_p}$	=	2.5	(ASCE 7,	Table 13.6-1)	
Overstrength Factor, $oldsymbol{\Omega}_{oldsymbol{0}}$	=	2.0	(ASCE 7,	Table 13.6-1)	
Component Importance Factor, I _p	=	1.0	(ASCE 7,	Table 1.5-2)	
Component Operating Weight, $\mathbf{W_p}$	=	W_p	(lb)		
Height in structure at lowest point of attachment of component, $\mathbf{z_1}$	=	23	(ft)		
Height in structure at highest point of attachment of component, z_2	=	23	(ft)		
Average Roof Height of Structure, h	=	23	(ft)		
Seismic design force, $\mathbf{F_p}$	=	0.4a _p S _{DS} W _P	-(1+27/h)		
р		R_p/I_p	(= - ==,,	(Eq. 13.3-1)	
Max. seismic design force, F _{pmax}	=	$1.6S_{DS}I_{p}W_{p}$		(Eq. 13.3-2)	
Min. seismic design force, F _{pmin}	=	$0.3S_{DS}I_{p}W_{p}$		(Eq. 13.3-3)	
Seismic design force at lowest point, F _{P1}	=	0.488	W _p		
				F _{p (AVG)} =	0.488
Seismic design force at highest point, F _{P2}	=	0.488	W _p		
			•		
Min. seismic design force, F _{pmin}	=	0.305	W _p		
Max. seismic design force, F _{pmax}	=	1.626	W _p		
Seismic design force, F _p (ASD)	=	0.348	W _p		
1				1	

	Description	SPM	6/9/2023
ENGINĚERING	Seismic Loads For Components & Systems	Checked	Date
250 4th Ave. South	Project TAC Ferris Wheel	Scale N.T.S.	Sheet No.
Suite 200 Edmonds, WA 98020		Job No. 23088.203	

Wind Load Calculation for Other Structures

(Reference: 2018 IBC Section 1609 & ASCE 7-16 Chapter 29)

Wind Velocity Pressure:

Mean Roof Height of Building, h (ft)=23(Per Architectural Drawings)Basic Wind Speed, V3s (mph)=110(ASCE Figure 26.5-1)Exposure Category=C(ASCE Section 26.7.3)Risk Category=II(ASCE Table 1.5-1)

Elevation Factor, K_e = 1.00 (ASCE Table 26.9-1)

Velocity Pressure, qh (psf) = **0.00256KhKztKdKeV^2** (ASCE Eq. 26.10-1) **qh** = **24.29** psf

Design Wind Load on Other Structures

Horiz Gust Effect/Force Coefficient, (GCr) = 1.9 (ASCE Section 29.4.1)

Vert Gust Effect/Force Coefficient, (GCr) = 1.5 (ASCE Section 29.4.1)

Projected Area Normal to Wind Dir, A_t or A_r (ft²) = A_t or A_r (Projected Wind Area)

Design Lateral Wind Load, F (lbs) = qz(GCr)Af (ASCE Eq. 29.4-2 & 29.4-3)

LDED	Horiz Force, Fh =	46.2	psf x A _f
LRFD	Vert Force, Fv =	36.4	psf x Ar
ASD	Horiz Force, Fh =	27.7	psf x A _f
	Vert Force, Fv =	21.9	psf x Ar

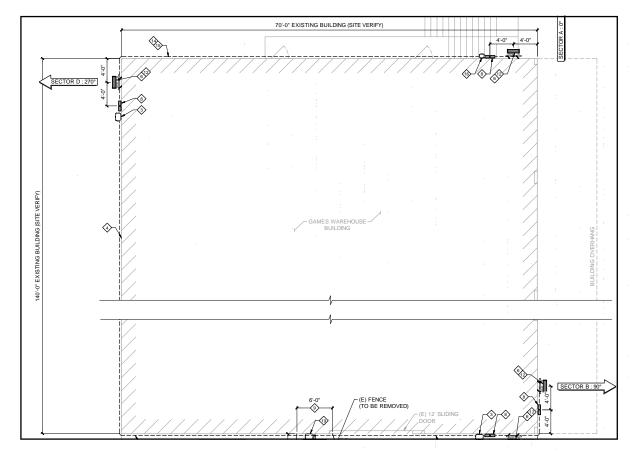
Kh or K, (ASCE Table 26.10-1)

Height Z (ft)	Exposure B	Exposure C	Exposure D
0	0.57	0.85	1.03
15	0.57	0.85	1.03
20	0.62	0.90	1.08
25	0.66	0.94	1.12
30	0.70	0.98	1.16
40	0.76	1.04	1.22
50	0.81	1.09	1.27
60	0.85	1.13	1.31
70	0.89	1.17	1.34
80	0.93	1.21	1.38
90	0.96	1.24	1.40
100	0.99	1.26	1.43
120	1.04	1.31	1.48
140	1.09	1.36	1.52
160	1.13	1.39	1.55
180	1.17	1.43	1.58
200	1.20	1.46	1.61

	Description		Ву	SPM	Date	6/9/2023
ENGINEERING		Wind Loads For Components and Systems	Checked	-	Date	
	Project	TAC Ferris Wheel	Scale	N.T.S.	Sheet No.	
Suite 200 Edmonds, WA 98020			Job No.	23088.203		

ANTENNA/EQUIPMENT COMPARISON (CONTINUED)

				Proposed				
ector	Quantity	Model	Height (in)	Width (in)	Depth (in)	Weight (LBS)	Tot. Weight (LBS)	Area (ft2)
	1	HTXCWW4513FX00	24.10	16.00	7.10	11.7	11.70	2.6
	1	KRE105281/1	8.41	7.88	4.13	10.14	10.14	0.4
	1	AIR 4435	14.76	7.87	3.74	14.3	14.33	0.8
Alpha	1	RRUS11 B13	19.7	17	7.2	51	51.00	2.3
Aipiia	1	OVP6	19.18	15.73	10.25	32	32.00	2.1
	1	4402 B2 DC	8.41	7.87	4.13	10.4	10.36	0.4
	1	4402 B66A DC	8.41	7.87	4.13	10.4	10.36	0.4
	1	4408 B48 DC	7.87	7.87	4.13	10.1	10.14	0.4
						Total	150.03	9.7
							0.00	0.0
							0.00	0.0
							0.00	0.0
Doto							0.00	0.0
Beta							0.00	0.0
							0.00	0.0
							0.00	0.0
							0.00	0.0
						Total	0.00	0.0
		1					0.00	0.0
							0.00	0.0
							0.00	0.0
							0.00	0.0
Gamma							0.00	0.0
							0.00	0.0
							0.00	0.0
							0.00	0.0
						Total	0.00	0.0
							150.00	
					Total All Se	ctors	150.03	9.7
Fauriti-							0.00	0.0
Equip.							0.00	0.0
Area							0.00	0.0
						Total	0.00	0.0
		Description			Ву	SPM	Date	6/9/2023
\longrightarrow			Antenna Comparison		Checked		Date	
ENGIN	IÉERING	Project			Scale	N.T.S.	Sheet No.	
	Ave. South				Job No.	14.1.5.		
	te 200 s, WA 98020	TAC	Ferris Wheel	טטו ועט.	23088.203			



EXISTING ROOF LAYOUT

ESTIMATED TOTAL WEIGHT OF ROOF = (140')(70')(12 PSF) = 117 K TOTAL WEIGHT OF APPURTENANCES < 0.2 KIPS

PROPOSED APPURTENANCES ARE FACE MOUNTED ON THE BUILDING WALL. NO INCREASE IN WIND AREA.

INCREASE IN WEIGHT FROM APPURTENANCES NEGLIGIBLE COMPARED TO OVERALL WEIGHT OF THE EXISTING STRUCTURE. VERTICAL LOADS DETERMINED TO BE LESS THAN 5% INCREASE AND LATERAL LOADS DETERMINED TO BE LESS THAN 10% INCREASE. EXISTING STRUCTURE ADEQUATE PER IEBC.

	Description	STRUCTURAL ANALYSIS	By SPM	Date 06.12.23
ENGINEERING			Checked	Date
250 4th Ave. South			Scale	Sheet No.
Suite 200 Edmonds, WA 98020	Project	TAC FERRIS WHEEL	Job No.	15
425.778.8500 www.cgengineering.com			23088.203	



verizon

PROJECT NAME:

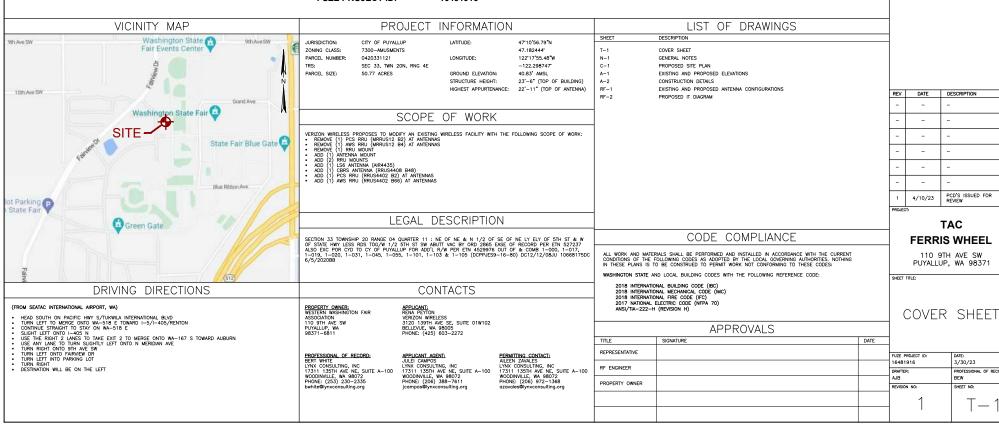
TAC FERRIS WHEEL-NODE 13

PROJECT LOCATION:

110 9TH AVE SW PUYALLUP, WA 98371

FUZE PROJECT ID:

16481916



verizon v



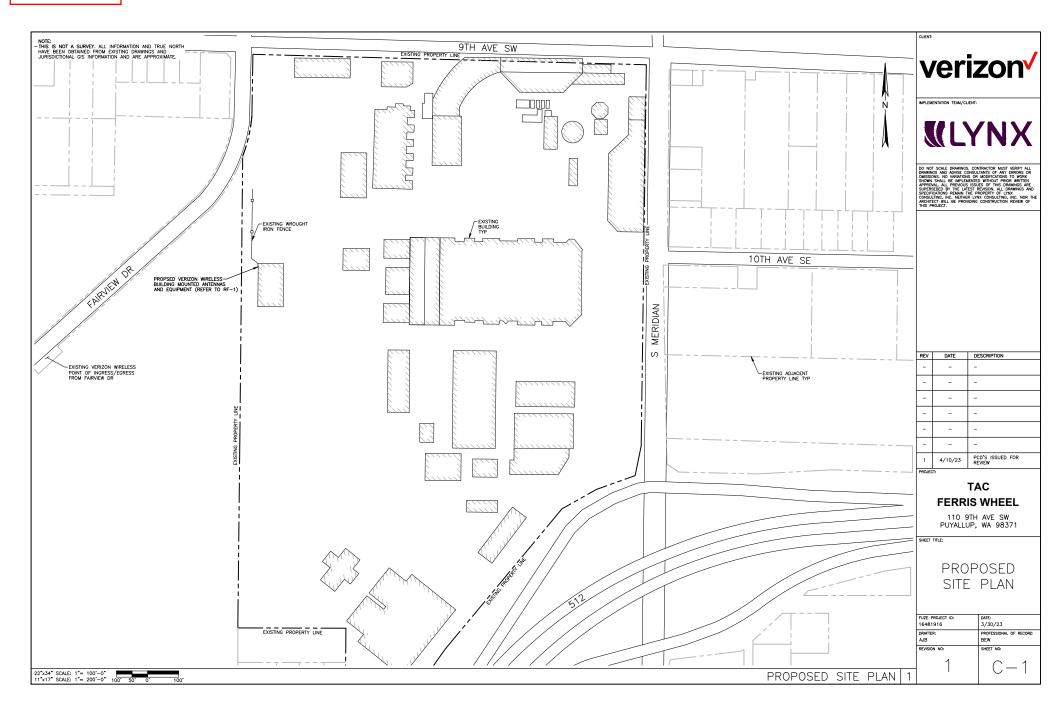
KEV	DATE	DESCRIPTION
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1	4/10/23	PCD'S ISSUED FOR REVIEW

FERRIS WHEEL

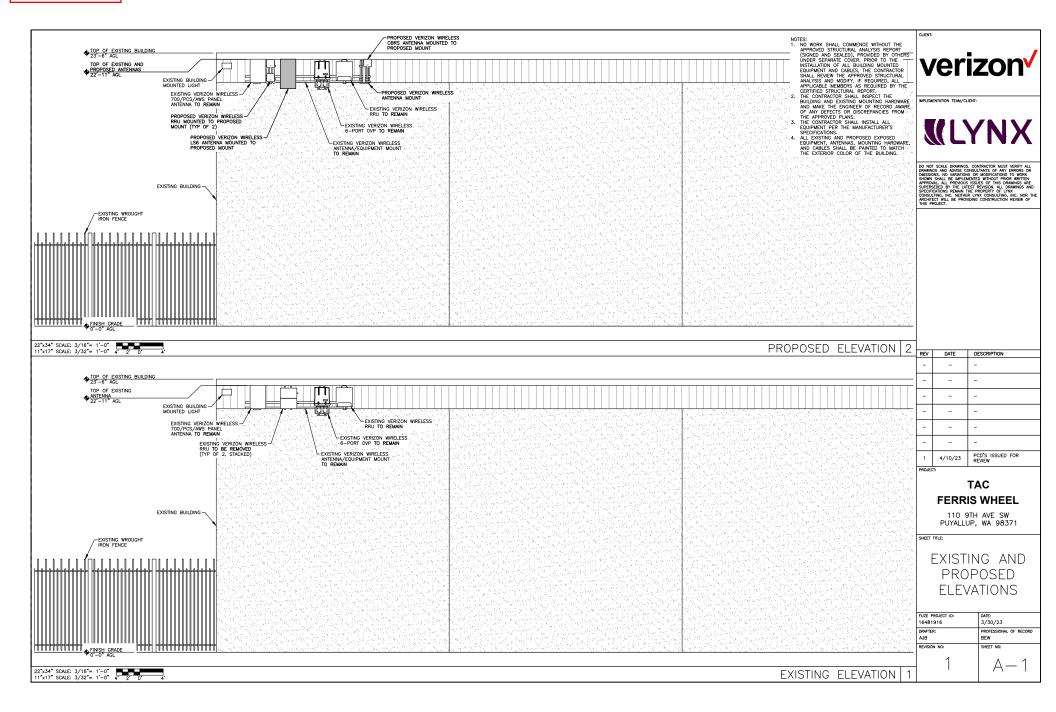
110 9TH AVE SW PUYALLUP, WA 98371

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F	UZE PROJECT ID:	DATE:
1	6481916	3/30/23
	RAFTER: JB	PROFESSIONAL OF RECORD BEW
R	EVISION NO:	SHEET NO:
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	l	

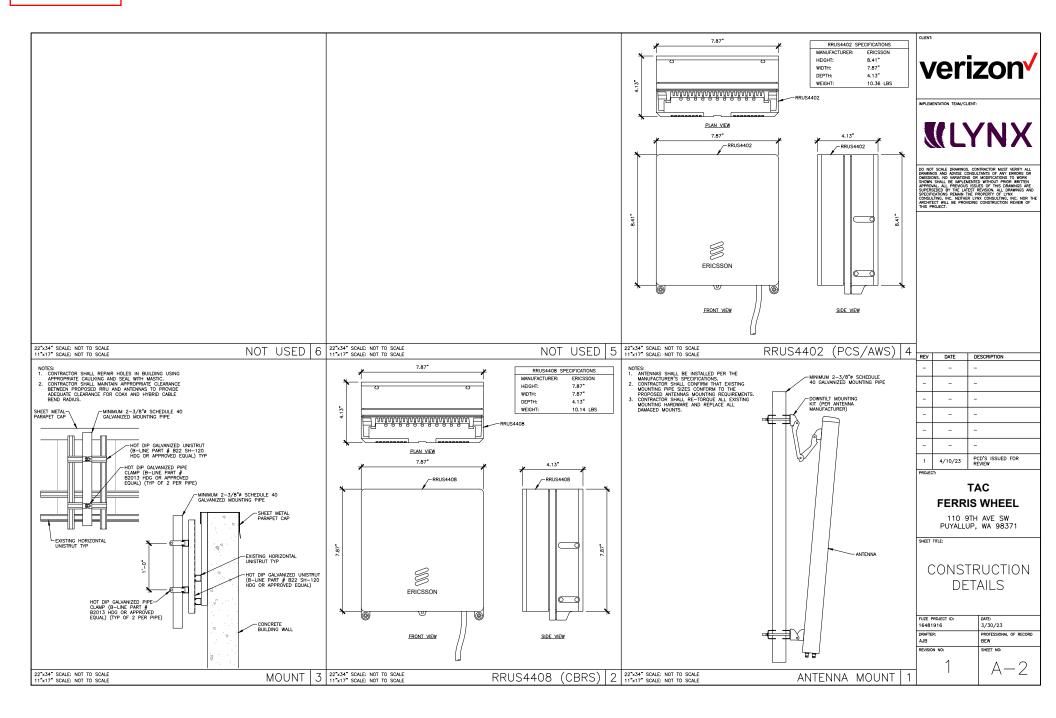
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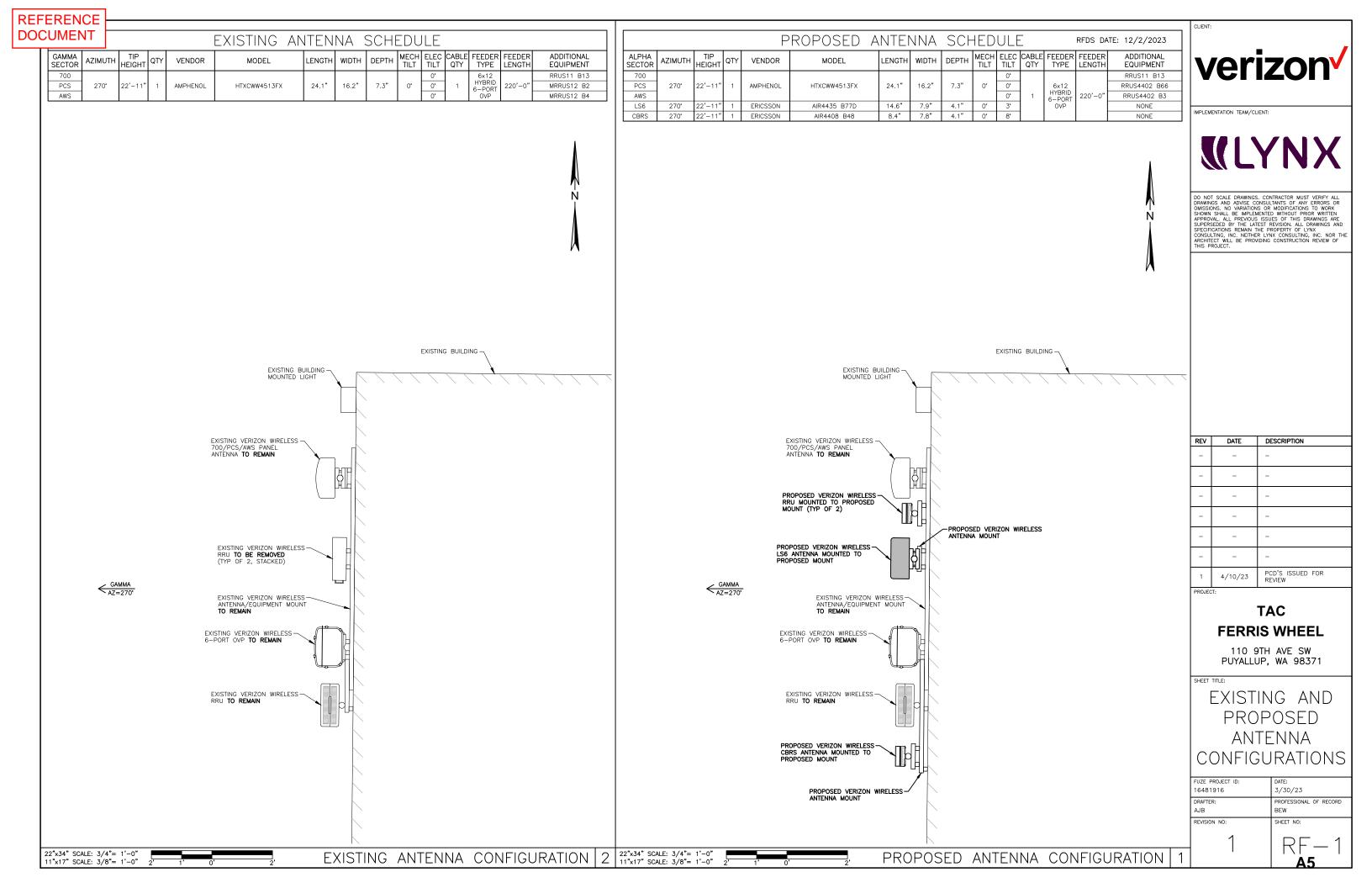






REFERENCE DOCUMENT







WEST > Pacific > Pacific Northwest > Seattle > FERRIS WHEEL_DUS5

RF Submit by: Poudel, Praful - praful.poudel@verizonwireless.com - 12/2/2022, 12:44:15 PM

EE Submit by: , - -

Project Details
FUZE Project ID: 16481916
Project Name: 5G L-Sub6 - Carrier Add
Project Alt Name: 5G L-Sub6 - Carrier Add
Project Type: Modification
Modification Type: RF
Designed Sector Carrier 4G: 6
Designed Sector Carrier 5G: N/A
Additional Sector Carrier 4G: N/A
Additional Sector Carrier 5G: N/A
FP Solution Type & Tech Type: MODIFICATION;4G_AWS3,4G_CBRS,5G_L-Sub6-Prep
Carrier Aggregation: false
MPT Id:
eCIP-0: false
Suffix:

Location Information
Site ID: 2567242
E-NodeB ID: 001622,701622,0017622,9009221
PSLC: 312171
Switch Name: Tacoma
Tower Owner:
Tower Type: Building Side-Mounted
Site Type: SMALL-CELL
Site Sub Type: SPOKE
Street Address: 110 9th Ave SW
City: Puyallup
State: WA
Zip Code: 98371
County: Pierce
Latitude: 47.182444 / 47° 10' 56.7984" N
Longitude: -122.298747 / 122° 17' 55.4892" W

RFDS Project Scope: Adding 4435 for C Band



Antenna Summary

Added																
700	1900	AWS	AWS3	CBRS	L-Sub6	Make	Model	Centerline	Tip Height	Azimuth	RET	4xRx	Inst. Type	Quantity	Item ID	
				LTE		ERICSSON	KRE105281/1	21.9	22.2	270(07)	false	false	PHYSICAL	1		
					5G	Ericsson	AIR4435 B77D	21.9	22.8	270(0003)	false	false	PHYSICAL	1		
Removed	d															
700	1900	AWS	AWS3	CBRS	L-Sub6	Make	Model	Centerline	Tip Height	Azimuth	RET	4xRx	Inst. Type	Quantity	Item ID	
									No data	available.						
Retained																
700	1900	AWS	AWS3	CBRS	L-Sub6	Make	Model	Centerline	Tip Height	Azimuth	RET	4xRx	Inst. Type	Quantity	Item ID	
LTE	LTE	LTE	LTE			AMPHENOL	HTXCWW4513FX00	21.9	22.9	270(03)	false	false	PHYSICAL	1		

Added: 2	Removed: 0	Retained: 1
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Proprietary and Confidential. Not for disclosure outside of Verizon.



Equipment Summary

Added														
Equipment Type	Location	700	1900	AWS	AWS3	CBRS	L-Sub6	Make	Model	Cable Length	Cable Size	Install Type	Quantity	Item ID
RRU	Tower		LTE					Ericsson	4402 B2 DC			PHYSICAL	1	KRC161737/1
RRU	Tower			LTE	LTE			Ericsson	4402 B66A DC			PHYSICAL	1	KRC161742/1
RRU	Tower					LTE		Ericsson	4408 B48 DC			PHYSICAL	1	KRC161746/1
RRU	Tower						5G	Ericsson	AIR4435 DC			PHYSICAL	1	
Removed														
Equipment Type	Location	700	1900	AWS	AWS3	CBRS	L-Sub6	Make	Model	Cable Length	Cable Size	Install Type	Quantity	Item ID
RRU	Tower		LTE					Ericsson	mRRUS12 B2			PHYSICAL	1	
RRU	Tower			LTE				Ericsson	mRRUS12 B4			PHYSICAL	1	
Retained														
Equipment Type	Location	700	1900	AWS	AWS3	CBRS	L-Sub6	Make	Model	Cable Length	Cable Size	Install Type	Quantity	Item ID
RRU	Tower	LTE						Ericsson	RRUS11 B13			PHYSICAL	1	
OVP Box	Tower							RAYCAP	OVP6			PHYSICAL	1	
OVP Box	Shelter							RAYCAP	OVP6			PHYSICAL	1	

Proprietary and Confidential. Not for disclosure outside of Verizon.

TAC FERRIS WHEEL WAREHOUSE - NODES 10-13

110-9TH AVENUE SW PUYALLUP, WA 98371



VICINITY MAP



DRIVING DIRECTIONS

- GET ON I-90 W TAKE I-405 S AND WA-167 S TO S MERIDIAN IN PUYALLUP. TAKE THE EXIT TOWARD MERIDIAN STREET S FROM WA-512 W
- MERGE ONTO I-90 W
- TAKE EXIT 10 FOR INTERSTATE 405 S TOWARD
- MERGE ONTO 1-405 S
- TAKE EXIT 2A TO MERGE ONTO WA-167 S TOWARD KENT/AUBURN
- TAKE THE EXIT ONTO WA-512 W TOWARD WA-161 S/PUYALLUP/OLYMPIA
 TAKE THE EXIT TOWARD MERIDIAN STREET S
- DESTINATION WILL BE ON THE LEFT

APPROVAL / SIGN OFF

5	APPROVED BY	DATE	SIGNATURE						
5	SITE ACQUISITION								
econ	ZONING								
2	RF								
5	CONSTRUCTION MANAGER								
ć	PROJECT MANAGER								
rammg:	REVIEWERS SHALL CLEARLY PLACE INITIALS ADJACENT TO FACH REDLINE NOTE AS DRAWINGS ARE BEING REVIEWED								

PROJECT CONTACT LIST

APPLICANT: VERIZON WIRELESS 3245 158TH AVENUE SE, MS231 BELLEVUE, WA 98008

PROJECT ARCHITECT: LDC, INC. 14201 NE 200TH ST, SUITE 100 WOODINVILLE, WA 98072 CONTACT: RICHARD B. HALL, AIA PHONE: (425) 806-1869 FAX: (425) 482-2893

STRUCTURAL ENGINEER:

14201 NE 200TH ST, SUITE 100 WOODINVILLE, WA 98072 CONTACT: JESSICA WREN, SE PHONE: (425) 806-1869 FAX: (425) 482-2893

PROJECT SURVEYOR:

LDC, INC. 14201 NE 200TH ST, SUITE 100 WOODINVILLE, WA 98072 CONTACT: VANCE BLUE, PLS FAX: (425) 482-2893

DRAWING INDEX DWG NO. DESCRIPTION PROPERTY OWNER:

WESTERN WASHINGTON FAIR ASSOC 110 9TH AVENUE SW PUYALLUP, WA 98371 CONTACT: DEBBIE BAKER

PHONE: (253) 841-5011

PHONE: (253) 218-9017

LDC. INC.

PROJECT CONSULTANT:

14201 NE 200TH ST, SUITE 100 WOODINVILLE, WA 98072 CONTACT: RICK CARDOZA

1-1	TITLE SHEET
G-1	GENERAL NOTES
C-1/1A	CIVIL SURVEY
A-1	SITE PLAN
A-2	ENLARGED SITE PLAN
A-3	ELEVATIONS
A-4	CONSTRUCTION DETAILS
A-4.1	SIGNAGE DETAILS
S-1	STRUCTURAL NOTES (PENDING)
S-2	STRUCTURAL DETAILS (PENDING)
RF-1	ANTENNA CONFIGURATION
E-1	SCHEMATIC GROUNDING PLAN
E-2	GROUNDING DETAILS
E-3	GROUNDING DETAILS

LEGAL DESCRIPTION

SECTION 33 TOWNSHIP 20 RANGE 04 QUARTER 11 NE OF NE & N 1/2 OF SE OF NE LY ELY OF 5TH ST & W OF STATE HWY LESS RDS TOG/W 1/2 5TH ST SW ABUTT VAC BY ORD 2865 FASE OF RECORD PER ETN 527237 OUT OF & COMB 1-000. (DCPP.JES9-16-80) DC12/12/08.JU

PROJECT INFORMATION

CODE INFORMATION: ZONING CLASSIFICATION: BUILDING CODE: 7300 - AMUSEMENTS IBC 2012 CONSTRUCTION TYPE:

CITY OF PUYALLUP JURISDICTION: PROPOSED BUILDING USE: UNMANNED TELECOM

SITE LOCATION (NAD83):

47° 10' 55.77" N 122° 17' 55.18" W (47.182158° N) (122.298661° W) LATITUDE: LONGITUDE: TOP OF STRUCTURE: 64.82' AMSL 40.83' AMSL 23.99' AGI BASE OF STRUCTURE:

PROJECT LEASE AREA:

PARCEL NUMBER:

NEW IMPERVIOUS AREA:

AREA OF PARCEL:

GENERAL INFORMATION:

PARKING REQUIREMENTS ARE UNCHANGED.
 TRAFFIC IS UNAFFECTED.

3 SIGNAGE IS LINAFFECTED

PROJECT DESCRIPTION:

VERIZON WIRELESS PROPOSES TO CONSTRUCT AN UNMANNED TELECOMMUNICATIONS FACILITY CONSISTING OF (4) ANTENNAS AND (8) MRRUS MOUNTED ON AN EXISTING BUILDING. ALSO, IN ADDITION OF (1) SMALL CELL ENCLOSURE CABINET MOUNTED NEXT TO BUILDING.

UTILITY COMPANIES

POWER: TELEPHONE:

SCALE DISCLAIMER

ALL DIMENSIONS AND ADVISE CONSULTANTS OF ANY DRAWINGS ARE SUPERSEDED BY THE LATEST REVISION

PROPRIETARY INFORMATION

THE INFORMATION CONTAINED IN THIS SET OF CONSTRUCTION DOCUMENTS IS PROPRIETARY BY NATURE. ANY USE OR DISCLOSURE OTHER THAN THAT WHICH RELATES TO VERIZON WIRELESS SERVICES IS STRICTLY







DATE:	4-29-14
DRAWN BY:	AT
CHECKED BY:	RBH

REVISIONS								
ΕV	DATE	DESCRIPTION	BY					
1	4-29-14	PRELIMINARY ZONING	RBH					
2	5-5-14	FINAL ZONING	RBH					
3	8-8-14	PRELIMINARY CONSTRUCTION	RBH					
4	8-18-14	FINAL CONSTRUCTION	RBH					
5	9-23-14	REVISED FINAL CONSTRUCTION	RBH					
3	10-1-14	REVISED FINAL CONSTRUCTION	RBH					
7	10-9-14	REVISED FINAL CONSTRUCTION	RBH					
3	11-4-14	REVISED FINAL CONSTRUCTION	RBH					



COUNTY STAMP

SITE TAC FERRIS WHEEL

WAREHOUSE - NODES 10-13 110-9TH AVENUE SW PUYALLUP, WA 98371

> SHEET TITLE TITLE SHEET

SHEET NUMBER T-1

GENERAL

- 1.1. ALL CONSTRUCTION SHALL CONFORM TO THE 2012 INTERNATIONAL BUILDING CODE.

 REFERENCE TO OTHER STANDARDS OR CODES SHALL MEAN THE LATEST STANDARD OR CODE

 ADOPTED & PUBLISHED.
- DRAWINGS SHOW TYPICAL & CERTAIN SPECIFIC CONDITIONS ONLY. FOR DETAILS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN.
- SECTIONALE SHOWN.

 1.3. EXISTING STRUCTURES & UNDERGROUND UTILITIES/STRUCTURES ARE ON DRAWINGS FOR CLARITY ONLY. VERIFY ALL EXISTING CONDITIONS, DIMENSIONS & ELEVATIONS BEFORE STARTING WORK. NOTIFY STRUCTURAL ENGINEER IN WRITING OF ANY INTERFERENCE AND/OR DISCREPANCIES THAT MIGHT EXIST.
- 1.4. THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING TEMPORARY SUPPORTS, ETC., IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- COORDINATE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING & CIVIL. NOTIFY STRUCTURAL ENGINEER OF ANY CONFLICT AND/OR
- 1.6. COORDINATE & VERIFY FLOOR, ROOF AND WALL OPENING SIZES & LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING & ELECTRICAL DRAWINGS. FOR ADDITIONAL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE ARCHITECTURAL & MECHANICAL
- 1.7 FOR DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
- 1.8. REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW & CHECK SHOP DRAWINGS RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW A CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS & OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, & DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- 1.9. STRUCTURAL DESIGN DRAWINGS SHALL NOT BE REPRODUCED AS SHOP DRAWINGS. CONTRACTOR & HIS SUBCONTRACTORS SHALL PREPARE ORIGINAL SHOP DRAWINGS.
- 1.10. CONTRACTOR SHALL REVIEW & STAMP ALL SHOP DRAWINGS BEFORE SUBMITTAL FOR REVIEW. PROPOSED FABRICATION CHANGES FROM DESIGN DRAWINGS SHALL BE NOTED IN SHOP DRAWINGS. ANY DISCREPANCIES BETWEEN ARCHITECTURAL & STRUCTURAL DRAWINGS SHALL. BE NOTED TO BE VERIFIED ON SHOP DRAWINGS.
- 1.11. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF ALL APPLICABLE SPECIALTY ITEMS INCLUDING CURTAIN WALL GLAZING SYSTEMS, LIGHT GAUGE STEEL FRAMING, ORNAMENTAL GUARDRAILS, SKYLIGHTS, METAL GRATING & STAIRS SHALL BE SEALED & SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF WASHINGTON & SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION.
- 1 12 RISK CATEGORY II
- 1.13. DESIGN GRAVITY LOADS SNOW LOAD:

	GROUND SNOW LOAD, Pg	25 PS
1.14.	WIND LOADS:	
	ULTIMATE WIND SPEED (3 SEC. GUST), Vult	110 M
	NOMINAL DESIGN WIND SPEED: Vasd	85 MF
	EXPOSURE CATEGORY	В
	INTERNAL PRESSURE COEFFICIENT, GC,	±0.00
1.15.	SEISMIC LOADS:	
	SEISMIC IMPORTANCE FACTOR, Ie	1.0
	MAPPED SPECTRAL RESPONSE ACCELERATION PA	ARAMETERS:
	(SHORT SECOND) Ss	1.255
	(1-SECOND PERIOD) S ₁	
	OUTE OLAGO	

DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENTS:

COMPONENT RESPONSE MODIFICATION FACTOR, Ro 2.5

REINFORCED CONCRETE

- 2.1. ALL CONCRETE WORK SHALL CONFORM TO ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS. DESIGN IS BASED ON ACI 318-11, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
- 2.2. UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL BE NORMAL WEIGHT & SHALL HAVE MINIMUM 28 DAY STRENGTHS AS FOLLOWS:

0.837

SLAB-ON-GRADE......3000 PSI

(SHORT SECOND) Sno

(1-SECOND PERIOD) S_{DL} SEISMIC DESIGN CATEGORY

COMPONENT AMPLIFICATION FACTOR, a

- 2.3. THE PROPOSED MATERIALS & MIX DESIGN SHALL BE FULLY DOCUMENTED & REVIEWED BY THE OWNER'S TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S.
- 2.4. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT
- 2.5. HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE INDICATED. THE LOCATIONS OF VERTICAL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY MECHANICAL MEANS &
- 2.6. UNLESS NOTED OTHERWISE, CHAMFER OR ROUND ALL EXPOSED CORNERS MINIMUM 3/4". SEE ARCHITECTURAL DRAWINGS FOR CHAMFER OR REVEAL REQUIREMENTS FOR ARCHITECTURAL CONCRETE.
- DETAIL CONCRETE REINFORCEMENT & ACCESSORIES IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315 & ACI DETAILING MANUAL (LATEST EDITION). SUBMIT SHOP DRAWINGS FOR REVIEW SHOWING ALL FABRICATION DIMENSIONS & LOCATIONS FOR PLACING REINFORCING STEEL & ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED &
- 2.8. DETAIL ALL CONCRETE WALLS & BEAMS ON THE SHOP DRAWINGS IN ELEVATION UNLESS SPECIFICALLY APPROVED OTHERWISE
- 2.9. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
- 2.10. WELDED WIRE FABRIC (MESH) SHALL CONFORM TO ASTM A185.
- 2.11. TIE ALL REINFORCING STEEL & EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES.
- 2.12. PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE FULL TENSION SPLICE UNLESS NOTED
- 2.13. REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED

OTTERWISE.	
CONCRETE AGAINST EARTH (NOT FORMED)	3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER	
#6 THROUGH #18 BARS	2"
#5 BARS & SMALLER	1-1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER	
SLABS & WALLS	1"
BEAMS (STIRRUPS) & COLUMNS (TIES)	1-1/2"

- 2.14. DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE
- STRUCTURAL ENGINEER. 2.15. STEEL REINFORCEMENT TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706 & THAT WELDING SHALL BE IN ACCORDANCE WITH AWS D14, STRUCTURAL WELDING CODE - REINFORCING STEEL BY AMERICAN WELDING SOCIETY FOR COMPLIANCE WITH ACI 318-11
- 2.16. SEE CIVIL & ARCHITECTURAL DRAWINGS FOR EXTERIOR SLAB WORK & JOINTING.

.17.	INCLUDE AIR ENTRAINING ADMIXTURE IN ALL CONCRETE THAT WILL BE EXPOSED TO WEATHER	
	EXCEPT IN FOOTINGS.	

- 2.18. INCLUDE WATER REDUCING ADMIXTURE IN ALL CONCRETE MIXES.
- 2.19. CONCRETE THAT WILL BE EXPOSED TO WEATHER SHALL HAVE WATER CONTENT LIMITED TO A MAXIMUM OF SIX (6) GALLONS PER SACK OF CEMENT.
- 2.20. THE PROPOSED MATERIALS & MIX DESIGN SHALL BE FULLY DOCUMENTS & REVIEWED BY THE THE PROPOSED WHITENEDS AN INDICATION SHALLER FULL DOCUMENTS A REVIEWED BY THE OWNERS TESTING LABORATORY, RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S. RESULTS OF COMPRESSIVE STRENGTH TESTS TO BE AVAILABLE ON SITE FOR INSPECTOR'S REVIEW.
- 2.21. BARS, OTHER THAN GRADE 40, SHALL BE MILL MARKED SO THAT TYPE, GRADE & YIELD STRENGTH ARE VISIBLY IDENTIFIABLE.
- 2.22. PROVIDE CORNER BARS AS PER TYPICAL DETAIL AT CORNERS & INTERSECTIONS OF ALL GRADE BEAMS & WALLS.
- 2.23. PROVIDE #3 @ 12" DOWELS FROM ALL ADJACENT CONCRETE GRADE BEAMS & WALLS TO INTERIOR SLABS-ON-GROUND, U.N.O.
- 2.24. ALL REINFORCING LAP SPLICES, UNLESS OTHERWISE SHOWN, SHALL SATISFY THE FOLLOWING SCHEDULE:

CONCRETE REINFORCEMENT LAP SPLICE LENGTH (in) GRADE 60										
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11	
TOP BAR	28	37	47	57	81	93	104	117	130	
OTHER	21	28	36	43	62	71	80	90	100	
ALL DAD DEVELOPMENT LENGTHS LINESSES OTHERWISE SHOWN SHALL SATISEVITIES										

FOLLOWING SCHEDULE:

CONCRETE REINFORCEMENT DEVELOPMENT LENGTH (in) GRADE 60									
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11
TOP BAR	21	28	36	43	62	71	80	90	100
OTHER	17	22	28	33	48	55	62	70	77

TOP BAR SHALL BE DEFINED AS ANY HORIZONTAL BARS PLACED SUCH THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE BAR, IN ANY SINGLE CONCRETE PLACEMENT, HORIZONTAL WALL BARS ARE CONSIDERED TOP BARS.

3. POST-INSTALLED REBAR & ANCHORS

3.1. SPECIFIC PRODUCT, DIAMETER, AND EMBEDMENT SHALL BE SHOWN IN THE DETAILS. INSTALL PRODUCTS IN ACCORDANCE WITH MANUFACTURER' PRINTED INSTALLATION INSTRUCTIONS (MPII). CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE FOR PRODUCT INSTALLATION TRAINING AND SHALL SUBMIT LETTER TO THE ENGINEER-OF-RECORD (EOR) INSTALLATION TRAINING AND SHALL SUBMIT LETTER TO THE PROJECT BUILDING CODE ANDIOR INDICATING TRAINING HAS TAKEN PLACE. REFER TO THE PROJECT BUILDING CODE ANDIOR EVALUATION REPORT FOR SPECIAL INSPECTIONS AND PROOF LOAD REQUIREMENTS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTSED BELOW MAY BE SUBMITTED BY THE CONTRACTOR TO THE EOR FOR REVIEW. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A RESEARCH REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPROPAGATION FOR THE PROJECT BUILDING CODE. SUBSTITUTION REQUEST SHALL INCLUDE CALCULATIONS THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT.

3.2 FOR ANCHORING INTO CONCRETE:

- 3.2.a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS.
- ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI355.4 AND ICC-ES AC305 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS.
 DESIGN ADHESIVE BOND STRENGTH HAS BEEN BASED ON ACI 355.4 TEMPERATURE DESIGN ADHESIVE BOND STRENGTH HAS BEEN BASED ON ACT355.4 LBMPERATURE CATEGORY B WITH INSTALLATIONS INTO DRY HOLES DRILLED USING A CARBIDE DRILL BIT INTO CRACKED CONCRETE THAT HAS CURED FOR AT LEAST 21 DAYS. ADHESIVE ANCHORS REQUIRING CERTIFIED INSTALLATIONS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER PER ACT 318-11 TO 22.2 INSTALLATIONS REQUIRING CERTIFIED INSTALLERS SHALL BE INSPECTED PER ACT 318-11 D.9.2.4.
- 3.2.c. POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES

4. SPECIAL INSPECTIONS

- 4.1. STRUCTURAL TESTS AND INSPECTIONS SHALL COMPLY WITH THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE
 - 4.1.a. THE INSPECTOR SHALL BE HIRED AND PAID FOR BY THE OWNER.
 - 4.1.b. THE INSPECTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH THE APPROVED STRUCTURAL PLANS AND SHALL SUBMIT PROGRESS REPORTS AND INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE STRUCTURAL ENGINEER OF RECORD.
- 4.2. SATISFY MINIMUM INSPECTION AND QUALITY CONTROL REQUIREMENTS OF THE INTERNATIONAL
- BUILDING CODE 4.3. SEE S101 FOR SCHEDULE OF SPECIAL INSPECTIONS.

MATERIAL/ACTIVITY	SERVICE		APPLICABLE TO 1		
MIATERIAL/ACTIVITY	SERVICE	Y/N	EXTENT	AGENT	DATE COMPLETED
1704.2.5 INSPECTION OF FABRICATORS					
1. VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	IN-PLANT REVIEW (3)	Υ	PERIODIC		
1705.3 CONCRETE CONSTRUCTION					
1. INSPECTION OF REINFORCING STEEL INSTALLATION (SEE 1705.22 FOR WELDING)	SHOP (3) AND FIELD INSPECTION	Υ	PERIODIC		
4. INSPECTION OF ANCHORS AND REINFORCING STEEL POST-INSTALLED IN HARDENED CONCRETE: PER RESEARCH REPORTS INCLUDING VERIFICATION OF ANCHOR TYPE, ANCHOR DIMENSIONS, HOLE DIMENSIONS, HOLE CLEANING PROCEDURES, ANCHOR SPACING, EDGE DISTANCE, CONCRETE MINIMUM THICKNESS, ANCHOR EMBEDMENT, AND TIGHTENING TORQUE	FIELD INSPECTION	Υ	PERIODIC OR AS REQUIRED BY THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE		
5. VERIFY USE OF APPROVED DESIGN MIX	SHOP (3) AND FIELD INSPECTION	Υ	PERIODIC		
6. FRESH CONCRETE SAMPLING, PERFORM SLUMP AND AIR CONTENT TESTS AND DETERMINE TEMPERATURE OF CONCRETE	SHOP (3) AND FIELD INSPECTION	Y	CONTINUOUS		
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	SHOP (3) AND FIELD INSPECTION	Υ	CONTINUOUS		
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	SHOP (3) AND FIELD INSPECTION	Υ	PERIODIC		
12. INSPECTION OF FORMWORK FOR SHAPE, LINES, LOCATION, AND DIMENSIONS	FIELD INSPECTION	Υ	PERIODIC		
13. CONCRETE STRENGTH TESTING AND VERIFICATION OF COMPLIANCE WITH CONSTRUCTION DOCUMENTS	FIELD TESTING AND REVIEW OF LABORATORY REPORTS	Υ	PERIODIC		
1705.11.6 MECHANICAL AND ELECTRICAL COMPONENTS SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE					
INSPECTION DURING THE ANCHORAGE OF ELECTRICAL EQUIPMENT FOR EMERGENCY OR STANDBY POWER SYSTEMS	FIELD INSPECTION	Υ	PERIODIC		
2. INSPECTION DURING THE ANCHORAGE OF OTHER ELECTRICAL EQUIPMENT	FIELD INSPECTION	Υ	PERIODIC		
INSPECTION DURING INSTALLATION AND ANCHORAGE OF PIPING SYSTEMS DESIGNED TO CARRY HAZARDOUS MATERIALS AND THEIR ASSOCIATED MECHANICAL UNITS	FIELD INSPECTION	Υ	PERIODIC		
4. INSPECTION DURING THE INSTALLATION AND ACNHORAGE OF HVAC DUCTWORK THAT WILL CONTAIN HAZARDOUS MATERIALS	FIELD INSPECTION	Υ	PERIODIC		
5. INSPECTION DURING THE INSTALLATION AND ANCHORAGE OF VIBRATION ISOLATION SYSTEMS	FIELD INSPECTION	Υ	PERIODI		
1705.11.7 STORAGE RACKS SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE					
1. INSPECTION DURING THE ANCHORAGE OF STORAGE RACKS 8 FEET OR GREATER IN HEIGHT	FIELD INSPECTION		-		
1705.12.3 SEISMIC CERTIFICATION OF NONSTRUCTURAL COMPONENTS					
REVIEW CERTIFICATE OF COMPLIANCE FOR DESIGNATED SEISMIC SYSTEM COMPONENTS	CERTIFICATE OF COMPLIANCE REVIEW	Υ	EACH SUBMITTAL		
1705 POST-INSALLED ANCHORS					
1. PREPARE A REPORT INCLUDING THE FOLLOWING DETAILS:					
A. ANCHOR DESCRIPTION, INCLUDING THE ANCHOR PRODUCT NAME, BOLT DIAMETER, AND ANCHOR LENGTH	FIELD INSPECTION	Υ	CONTINUOUS		
B. HOLE DESCRIPTION INCLUDING VERIFICATION OF DRILL BIT COMPLIANCE WITH ANSI B212.15-1994. RECORD INSTALLATION DESCRIPTION, INCLUDING VERIFICATION OF MASONRY/CONCRETE COMPRESSIVE STRENGTH AND ANCHOR INSTALLATION AND LOCATION (SPACING AND EDGE DISTANCE) IN ACCORDANCE WITH MANUFACTURERS PUBLISHED INSTALLATION INSTRUCTIONS.	FIELD INSPECTION	Υ	CONTINUOUS		



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4201 NE 200th St. #100 www.LDCcorp.cor

Structura

DATE: 4-29-14 AT DRAWN BY CHECKED BY RBH

		REVISIONS	
REV	DATE	DESCRIPTION	B
1	4-29-14	PRELIMINARY ZONING	RB
2	5-5-14	FINAL ZONING	RB
3	8-8-14	PRELIMINARY CONSTRUCTION	RB
4	8-18-14	FINAL CONSTRUCTION	RB
5	9-23-14	REVISED FINAL CONSTRUCTION	RB
6	10-1-14	REVISED FINAL CONSTRUCTION	RB
7	10-9-14	REVISED FINAL CONSTRUCTION	RB
8	11-4-14	REVISED FINAL CONSTRUCTION	RB



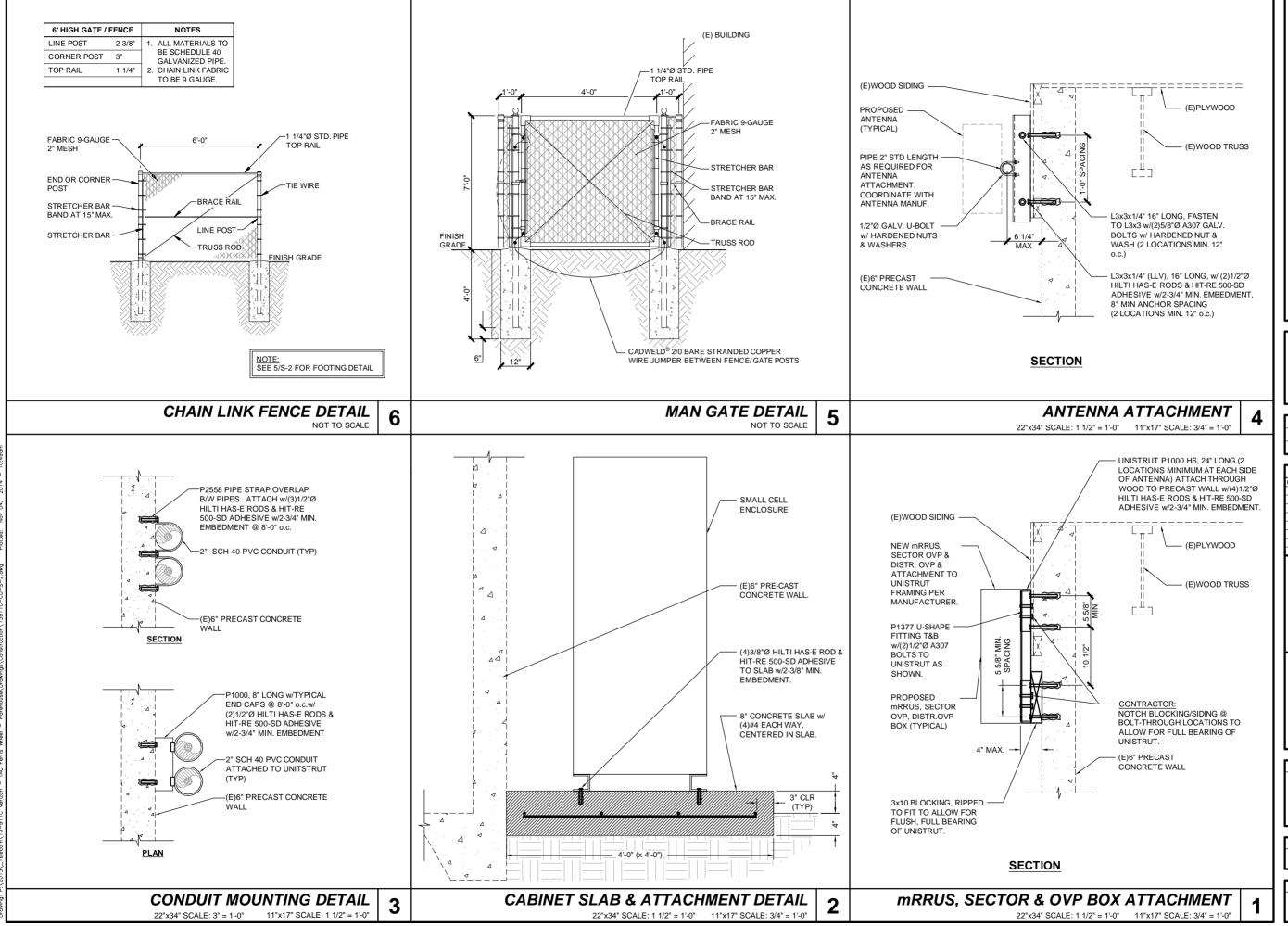
COUNTY STAMP

TAC FERRIS WHEEL WAREHOUSE - NODES 10-13

110-9TH AVENUE SW PUYALLUP, WA 98371

> SHEET TITLE STRUCTURAL NOTES

SHEET NUMBER





Architectura
Engineering
Structura
1HE CIVIL ENGINEERING GROUP
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DATE: 4-29-14
DRAWN BY: AT

RBH

CHECKED BY:



COUNTY STAMP

TAC FERRIS WHEEL WAREHOUSE - NODES 10-13

> 110-9TH AVENUE SW PUYALLUP, WA 98371

SHEET TITLE STRUCTURAL PLANS AND DETAILS

SHEET NUMBER