A	<u>CRITERIA</u> <u>L MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION</u> SHALL CONFORM TO THE DRAWINGS,	17.	FOUNDATION NOTE
	PECIFICATIONS, THE 2018 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC).		VALUES LISTED O THEREFORE MUST NOTIFY THE STRUC
R	DOF SNOW LOAD 25 PSF		FOOTINGS SHALL
ST	LOOR LIVE LOAD I25 PSF OR 2,000 LBS TAIR AND EXITS LIVE LOAD I00 PSF		OR BOTH) AT LEA SHOWN ON PLANS OF FOOTINGS MUS
Gl	JARDRAILS/BALCONY RAILS (EXIT FACILITY) 50 PLF OR 200 LBS. JARDRAILS/BALCONY RAILS (OTHER THAN EXIT FACILITY) 20 PLF OR 200 LBS.		FOOTINGS SHALL
	ECHANICAL UNITS WEIGHTS FURNISHED BY MANUFACTURER		BACKFILL BEHIND SUBSURFACE DRA
<u>/1</u>	RISK CATEGORY II IO4 MPH		<u>THE STRUCTURAL</u> <u>REPORT</u> :
	EXPOSURE "B" TOPOGRAPHIC FACTOR Kzt = 1.0		ALLOWABLE SOIL LATERAL EARTH
Ē	ARTHQUAKE : ANALYSIS PROCEDURE: IBC "EQUIVALENT LATERAL FORCE PROCEDURE" SEISMIC DESIGN CATEGORY (SDC) = D RISK CATEGORY = II SEISMIC SITE CLASS = D IMPORTANCE FACTOR  e = 1.0 MAPPED MCE Ss = 1.26; S <sub>1</sub> = 1.01 DESIGN ACCELERATION Sds = 0.43; Sd <sub>1</sub> = 0.54		SEISMIC SURCHAN PASSIVE SOIL PR SOIL COEFFICIEN SOIL DENSITY
_	ATERAL LOADS ARE TRANSFERRED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR WALLS R BRACED FRAMES. MOMENTS, SHEARS AND ROTATIONAL FORCES ARE BASED ON THE RIGIDITY OF	18.	DEMOLITION: VE SHALL BE INSTAL
E٨	ACH SHEAR WALL OR BRACED FRAME AND ARE CARRIED BY THE SHEAR WALLS OR BRACED RAMES TO THE FOUNDATION.		TO THE WORK SE PLANS. SAW CUT SAVED. DEMOLI
A	TRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING ND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY		STRUCTURE. LIMI SYSTEMS TO 40
	ND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.		A. ALL NEW OP SAW CUTTING
C	OMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE TENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.		B. VERIFY ALL C. SMALL ROUN D. WHERE NEW
	<u>ONTRACTOR</u> SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL OMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.		EXISTING CC OTHERWISE 1
SE	ONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, EQUENCES OR PROCEDURES REQUIRED TO PERFORM THEIR WORK. THE STRUCTURAL ENGINEER HAS NO		
M	VERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC ORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY RADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE,	19.	CONCRETE SHALL
С	ORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR THER ENTITIES OR PERSONS AT THE PROJECT SITE.		CONSTRUCTION T 28-DAY STRENG SLABS-ON-GRAD
5	<u>ONTRACTOR-INITIATED</u> CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND TRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN		SECTION 1904.1. / CEMENT PER CUE (BEFORE THE AD
ות	N SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT. RAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT		FOOTINGS AND C FOR FOOTINGS A
SF	PECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF ONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE		THE MINIMUM AMO PERFORMANCE N
ST SF Al	PECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF ONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE IRUCTURAL ENGINEER. WHERE INFORMATION ON THE DRAWINGS IS IN CONFLICT WITH THE PECIFICATIONS, THE MORE STRINGENT SHALL APPLY, SUBJECT TO REVIEW AND APPROVAL BY THE RCHITECT AND THE STRUCTURAL ENGINEER. DO NOT SCALE THE DRAWINGS.		APPROVAL TWO THE PERFORMAN
<u>A</u>	<u>L STRUCTURAL SYSTEMS</u> WHICH ARE COMPOSED OF FIELD ERECTED COMPONENTS SHALL BE		COARSE AGGREG CONCRETE YIELD ADMIXTURES AND
IN	IPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.		PERCENTAGE OF PERFORMANCE N
SI	<u>HOP DRAWINGS</u> FOR REINFORCING STEEL (FOR BOTH CONCRETE AND MASONRY CONSTRUCTION), IRUCTURAL STEEL, AND METAL DECKING, SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL NGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.		THE ATTENTION C ONLY THAT INFOI CONTRACTOR MA
C	ONTRACTOR SHALL SUBMIT WALL ELEVATION DRAWINGS OF AT LEAST 1/8" = 1'-0" SCALE INDICATING		ALL CONCRETE A
	OCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS FOR REVIEW PRIOR TO CONSTRUCTION. ONTRACTOR SHALL COORDINATE WITH REINFORCEMENT SHOP DRAWINGS.		CONCRETE SHAL STEEL TROWELEI
R	<u>HOP DRAWING REVIEW</u> : DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF ECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND FAMB DRAWINGS REFOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW	20	. <u>REINFORCING ST</u> DETAILED (INCLU
5 Di Oi	TAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW RAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS = CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.		CONTINUOUS REIN BARS AT ALL WA
	<u>HOP DRAWING SUBMITTALS</u> PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE = SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE		DIAMETERS OR 2 318, CLASS B. P
С	ONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO E FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION	21.	SLABS EXTENDIN
Tł	ETHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND HE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING JBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL		FOOTINGS AND C
C	ONTROL AND SHALL BE FOLLOWED.		FORMED SURFAC (#6 BARS OR LA
51	<u>EFERRED SUBMITTALS OF DESIGN BUILD COMPONENTS</u> SHALL BEAR THE STAMP AND SIGNATURE OF A FATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER AND SHALL BE APPROVED BY THE OMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS		(#5 BARS OR SM COLUMN TIES OR
IM Co	POSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE ONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON		SLABS AND WAL
D	RCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND IRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE AND SHALL INCLUDE DESIGN CALCULATIONS ITH THE ENGINEER'S STAMP.	22	. <u>CAST-IN-PLACE</u> OF DOOR AND M
Tł	E FOLLOWING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT:		AND LOCATION C ARCHITECTURAL TEXTURE, AND O
	TAIRS, RAILINGS, AND METAL BUILDINGS <u>ECHANICAL UNIT CONNECTIONS</u> TO THE BUILDING SHALL BE DESIGNED BY THE MANUFACTURER FOR THE		AND PRECAST.
SL	ESIGN CRITERIA AND CONDITIONS SHOWN ON THE STRUCTURAL DRAWINGS." MANUFACTURER SHALL JBMIT DETAIL DRAWINGS AND CALCULATIONS, BOTH OF WHICH BEAR THE STAMP AND SIGNATURE OF A	23	NON-SHRINK GRC APPROVED MANU MANUFACTURER'S
R	TATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER.° MANUFACTURER'S ENGINEER SHALL BE ESPONSIBLE FOR DESIGN, CODE CONFORMANCE, AND CONNECTION OF THE UNIT TO THE BASIC IRUCTURE.° ALL NECESSARY BRACING, TIES, ANCHORAGE, DISTRIBUTION MEMBERS, AND SIMILAR		THE MATERIAL O
EL	EMENTS SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH SUBMITTED DRAWINGS AND ALCULATIONS.	24	. <u>POLYSTYRENE (R</u> SHALL BE RIGID MINIMUM COMPRE
	<u>PECIAL INSPECTION:</u> CONCRETE CONSTRUCTION, MASONRY CONSTRUCTION, STRUCTURAL STEEL ABRICATION AND ERECTION (INCLUDING FIELD WELDING AND HIGH-STRENGTH FIELD BOLTING), METAL		RESISTANCE OF BLOCK JOINTS B
DI Ef	ECK INSTALLATION, EXPANSION BOLTS AND THREADED EXPANSION INSERTS, SCREW ANCHORS, AND POXY GROUTED INSTALLATIONS SHALL BE SUPERVISED IN ACCORDANCE WITH IBC SECTIONS 1704 &		RECOMMENDATIC
0	05 AND THE PROJECT SPECIFICATIONS BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE MNER. THE TESTING AGENCY AND INSPECTOR SHALL BE REGISTERED WITH WABO AND SHALL SEND OPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE OWNER, ARCHITECT,		
	INTES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE OWNER, ARCHITECT, IRUCTURAL ENGINEER, CONTRACTOR AND BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET ROJECT SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.		

## GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

#### <u>GEOTECHNICAL</u>

ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE BASED ON PREVIOUSLY PERMITTED BUILDING PLANS DATED DECEMBER 22, 1995 AND E VERIFIED IN THE FIELD. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, TURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

3EAR ON FIRM, UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL 5T 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD. UNLESS OTHERWISE NOTED, BE CENTERED UNDER COLUMNS OR WALLS ABOVE.

ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR VAGE.

DESIGN IS BASED ON THE FOLLOWING VALUES FROM THE REFERENCED GEOTECHNICAL

BEARING PRESSURE	2,000 PSF
PRESSURE (RESTRAINED/UNRESTRAINED)	55 PCF/35 PCF
GE PRESSURE (RESTRAINED/UNRESTRAINED)	8H PSF/5H PSF
ESSURE	350 PCF
OF FRICTION	0.35
	120 PCF

#### <u>RENOVATION</u>

IFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING ED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE VENCES. EXISTING REINFORCING SHALL BE SAVED WHERE AND AS NOTED ON THE ING, IF AND WHERE USED, SHALL NOT CUT EXISTING REINFORCING THAT IS TO BE ON DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR

NINGS THROUGH EXISTING WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY WHEREVER POSSIBLE

EXISTING CONDITIONS AND LOCATION OF MEMBERS PRIOR TO CUTTING ANY OPENINGS. OPENINGS SHALL BE ACCOMPLISHED BY CORE DRILLING, IF POSSIBLE EINFORCING TERMINATES AT EXISTING CONCRETE, REBAR DOWELS EPOXIED INTO THE ICRETE SHALL BE PROVIDED TO MATCH HORIZONTAL REINFORCING, UNLESS DTED ON PLANS.

#### CONCRETE

BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301. LERANCES SHALL NOT EXCEED THOSE LISTED IN ACI 117. CONCRETE SHALL ATTAIN A OF I'C = 3,000 PSI. ALL CONCRETE EXPOSED TO THE WEATHER AND ALL GARAGE SHALL ATTAIN A 28-DAY STRENGTH 1'C OF 3,500 PSI IN ACCORDANCE WITH IBC ND ACI 318 TABLE 19.3.2.1. MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF , YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS ITION OF ADMIXTURES). THE WATER/CEMENT RATIO SHALL NOT EXCEED 0.55 FOR 45 FOR ALL SLABS AND EXPOSED CONCRETE UNLESS OTHERWISE NOTED. EXCEPT D SLAB ON GRADE, AGGREGATE SIZE SHALL NOT EXCEED 3/4".

UNT OF CEMENT AND THE MAXIMUM SLUMP MAY BE CHANGED IF A CONCRETE IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT FOR EEKS PRIOR TO PLACING ANY CONCRETE. (THE W/C RATIO LIMITS STILL APPLY). MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND ATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. CHEMICAL FLY ASH SHALL CONFORM TO ASTM C494 AND C618 RESPECTIVELY. FLY ASH OTAL CEMENTITIOUS MATERIAL SHALL NOT EXCEED 20%. THE USE OF A REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE BROUGHT TO THE OWNER. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES MATION PRESENTED CONFORMS GENERALLY TO CONTRACT DOCUMENTS. NTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

ITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AGENT CONFORMING TO ASTM C260. TOTAL AIR CONTENT FOR FROST-RESISTANT BE IN ACCORDANCE WITH ACI 318-14 TABLE 19.3.3.1. ALL CONCRETE TO RECEIVE A FINISH SHALL NOT BE AIR-ENTRAINED.

"L SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT SI), AND SHALL BE ING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 AND 318. LAP ALL ORCEMENT #5 AND SMALLER 60 BAR DIAMETERS, 2'-0" MINIMUM. PROVIDE CORNER . AND FOOTING INTERSECTIONS. LAP CORNER BARS #5 AND SMALLER 60 BAR -O" MINIMUM. LAPS OF LARGER BARS SHALL BE MADE IN ACCORDANCE WITH ACI OVIDE (2) #5 MIN. U.N.O. TRIM BARS AROUND ALL OPENINGS IN CONCRETE WALLS OR 5 2'-6" PAST CORNERS, TYPICAL.

CTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

HER UNFORMED SURFACES CAST AGAINST EARTH	3"
IS EXPOSED TO EARTH (I.E. WALLS BELOW GROUND) OR WEATHER GER) LLER)	2"  - /2"
BPIRALS AND BEAM STIRRUPS	- /2"

(INTERIOR FACE) GREATER OF (BAR DIAMETER PLUS 1/8") OR 3/4"

DNCRETE: SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS IDOW OPENINGS IN ALL CONCRETE WALLS. SEE MECHANICAL DRAWINGS FOR SIZE MISCELLANEOUS MECHANICAL OPENINGS THROUGH CONCRETE WALLS. SEE RAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, ER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES, BOTH CAST-IN-PLACE

SHALL BE NON-METALLIC CONFORMING TO ASTM CILOT AND BE FURNISHED BY AN ACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO WHICH IT IS PLACED (5000 PSI MINIMUM).

IGID INSULATION) LIGHTWEIGHT STRUCTURAL FILL PLACED BELOW CONCRETE SLABS LLULAR POLYSTYRENE CONFORMING TO ASTM D6817 OR ASTM C578, WITH A SIVE RESISTANCE OF 5 PSI @ 1% DEFORMATION AND A MINIMUM COMPRESSIVE 5 PSI @ 10 % DEFORMATION, U.O.N. MAXIMUM DENSITY SHALL BE 2.0 PCF. OFFSET TWEEN ADJACENT LAYERS AND ATTACH BLOCKS PER THE MANUFACTURER'S

25. EXPANSION BOLTS INTO CONCRETE SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-2302 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

<u>ANCHORAGE</u>

- 26. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-1385 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 27. EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3037 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR JAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 28. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH IAPMO UES REPORT NO. ER-240 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 29. SCREW ANCHORS INTO CONCRETE SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3027 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 30. SCREW ANCHORS INTO GROUT FILLED CMU SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3056 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 31. DRIVE PINS, SHOT PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE LOW VELOCITY TYPE FASTENERS AS MANUFACTURED BY HILTI CORPORATION. WHEN CALLED FOR IN THE DRAWINGS, PROVIDE THE APPROPRIATE FASTENER AS NOTED IN THE TABLE BELOW FOR EACH GIVEN APPLICATION. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORTS NO. ESR-2269 FOR THE X-U FASTENERS AND ESR-2379 FOR THE X-CP FASTENERS. MINIMUM EMBEDMENT IN CONCRETE SHALL BE I" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3" TO NEAREST CONCRETE EDGE AND 4" CENTER TO CENTER SPACING. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES.

ALLOWABLE APPLICATION	ALLOWABLE FASTENER TYPE	SHEAR CAF
	X-CP 72 P8 523 w/ 1.33" EMBED	250
LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN. TO CONCRETE (2000 PSI MIN.)	X-U 27 P8 515	190
2X LUMBER TO STRUCTURAL STEEL (3/16" MIN., 36 OR 50 KSI)	X-U 52 MX PLUS R-23 WASHERS	250
LIGHT GAUGE STEEL 43 & 33 MILS (18 & 20 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO II/16" MAX)	X-U 19 P8 TH	445
LIGHT GAUGE STEEL 97, 68 \$ 54 MILS (12, 14 \$ 16 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	720
LIGHT GAUGE STEEL (ALL GA.) TO STRUCTURAL STEEL (3/4" AND GREATER)	X-U 19 P8 TH	350
2X TREATED LUMBER TO GROUTED CMU	X-CP 72 P8 523	105
	X-11 32 PB 615	220

LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN.	X-U 32 P8 515	220
TO GROUTED CMU		

- 32. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO CONCRETE SHALL BE INSTALLED USING "HIT-RE 500 V3" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3814, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.
- 33. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO GROUT FILLED CMU SHALL BE INSTALLED USING "HIT HY 270" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-4143, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

#### MASONRY

34. CONCRETE MASONRY UNIT WALLS SHALL BE CONSTRUCTED OF MEDIUM WEIGHT UNITS CONFORMING TO ASTM C90, LAID IN A RUNNING BOND. CONTACT ENGINEER FOR RE-DESIGN OF REINFORCING WHERE STACK BOND LAYOUT IS REQUIRED. LINEAR SHRINKAGE SHALL NOT EXCEED 0.065%. MORTAR SHALL BE TYPE "S" IN ACCORDANCE WITH ASTM C270. GROUT SHALL CONFORM TO IBC REQUIREMENTS AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS, DESIGN F'M = 2,000 PSI. FULL STRESSES ARE REQUIRED. STRENGTH SHALL BE VERIFIED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH TMS 602-16.

PROVIDE (2) #5 ((2)#4 @ 6" AND 4" WALLS) VERT. AT EACH SIDE OF OPENINGS, AT WALL CORNERS AND INTERSECTIONS AND AT FREE ENDS OF WALLS AND (2) #4 HORIZ. AT ELEVATED FLOOR AND ROOF LEVELS, AT TOPS OF WALLS AND ABOVE AND BELOW ALL OPENINGS. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAMS. EXTEND REINFORCEMENT AROUND OPENINGS 2'-O" BEYOND FACE OF OPENING. IF 2'-O" IS UNAVAILABLE. EXTEND AS FAR AS POSSIBLE AND HOOK. PROVIDE CORNER BARS TO LAP HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS. UNLESS NOTED OTHERWISE, LAP ALL REINFORCING STEEL IN CMU 48 BAR DIAMETERS, 2'-O" MINIMUM.

ALL CELLS ARE TO BE SOLID GROUTED UNLESS NOTED AS PARTIAL GROUTING. FOR PARTIAL GROUTING FILL ALL CELLS CONTAINING REINFORCEMENT OR EMBEDDED ITEMS AND ALL CELLS BELOW GRADE WITH GROUT. ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING AND SHALL BE HELD AT TOP, BOTTOM AND 192 BAR DIAMETERS (MAX.) O.C. PER TMS 602 SPECIFICATION 3.5, GROUT POURS SHALL NOT EXCEED 5'-4" IN HEIGHT UNLESS A TEST PANEL IS CONSTRUCTED BY THE MASON AND APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE CLEANOUT HOLES AT BOTTOM OF ALL CELLS CONTAINING REINFORCEMENT FOR ALL GROUT POURS OVER 5'-4" IN HEIGHT. PROVIDE 1 1/2 IN. GROUT KEYS BETWEEN EACH POUR.

Fy

APACITY (LBS) TENSION CAPACITY (LBS)

175
165
175
360
535
375
100
225

35. STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE BASED ON THE LATEST EDITIONS OF THE A.I.S.C. SPECIFICATIONS AND CODES:

A. AISC - STEEL CONSTRUCTION MANUAL, 15<sup>TH</sup> EDITION B. AISC 303-16 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

C. 2014 RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS.

36. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

#### TYPE OF MEMBER

		2
A. WIDE FLANGE (W AND WT) SHAPES	A992	50 KSI
B. ALL OTHER SHAPES	A36	36 KSI
C. PLATE	A36 OR A572	36 KSI (MIN)
D. PIPE SECTIONS	A53 (TYPE E OR S, GRADE B)	35 KSI
E. STRUCTURAL TUBING (SQUARE OR RECTANGULAR)	A500 (GRADE C)	50 KSI
F. ANCHOR BOLTS AND THREADED RODS	F1554 (GRADE 36) OR	36 KSI
(EMBEDDED IN MASONRY OR CONCRETE)	FI554 (GRADE 55, SUPP. SI)	55 KSI
G. CONNECTION BOLTS	F3125 GRADE A325-N	92 KSI
(7/8" ROUND, UNLESS SHOWN OTHERWISE)		
H. HEADED SHEAR STUDS	A29	49 KSI
I. THREADED RODS	A36	36 KSI
J. STAINLESS STEEL	AISI 316L	30 KSI

ASTM SPECIFICATION

SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE SHALL NOT BE ALLOWED WITHOUT PRIOR APPROVAL OF THE ENGINEERALL STEEL TO BE FIREPROOFED SHALL BE LEFT UNPAINTED. ALL OTHER STEEL SHALL HAVE ONE COAT OF APPROVED SHOP PAINT.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER OR EARTH SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A123. GALVANIZE BOLTS AND SIMILAR THREADED FASTENERS EXPOSED TO WEATHER OR EARTH IN ACCORDANCE WITH ASTM A153. ALL FIELD WELDS EXPOSED TO WEATHER OR EARTH SHALL BE COATED WITH BRUSH APPLIED ZINC RICH PAINT COMPLYING WITH ASTM A780 (Z.R.C. OR EQUIVALENT).

A MINIMUM OF TWO BOLTS ARE REQUIRED FOR ALL CONNECTIONS. ALTERNATE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ENGINEER.

ALL MEMBERS ARE TO BE ERECTED WITH THE NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM CAMBER ON THE DRAWINGS IS THE UPWARD CAMBER REQUIRED IN THE BEAM AS DELIVERED TO THE JOBSITE. CONTRACTOR TO CONSIDER CAMBER LOSS, IF ANY, DUE TO SHIPPING AND HANDLING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES AND UNEQUAL PARTS.

37. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL SHALL CONFORM TO SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

38. ALL A325 CONNECTION BOLTS SHALL BE INSTALLED TO THE SNUG-TIGHT CONDITION PER RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. ALL NUTS SHALL CONFORM TO ASTM A563. ALL WASHERS SHALL CONFORM TO ASTM F436 OR ASTM F959 TYPE 325. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS OTHERWISE NOTED.

39. ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.S.C. AND A.W.S. STANDARDS AND SHALL BE PERFORMED BY W.A.B.O. CERTIFIED WELDERS USING ETO XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.W.S.) SHALL BE USED. ALL WELDING OF STAINLESS STEEL SHALL USE E309 ELECTRODES WITH A GMAW PROCESS. ALL WELDING SHALL BE PERFORMED BY WELDERS WITH AWS / W.A.B.O. CERTIFICATION WITH THE MATERIAL AND METHOD REQUIRED.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. WELDS SHOWN ON DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON PLATE THICKNESS. MINIMUM WELDING SHALL BE 3/16-INCH. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD ARROWS ARE SHOWN WHERE A FIELD WELD IS REQUIRED BY THE STRUCTURAL DESIGN; THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL DELIVERY AND ERECTION. SEE THE SPECIFICATIONS AND DRAWINGS FOR ADDITIONAL WELDING REQUIREMENTS, ESPECIALLY AT SPECIAL MOMENT RESISTING FRAMES AND OTHER SEISMIC CRITICAL WELDS.

40. WELDING OF LATERAL FORCE RESISTING MEMBERS SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS DI.I (INCLUDING AWS DI.8 SEISMIC SUPPLEMENT) AND APPROVED BY THE STRUCTURAL ENGINEER BEFORE WORK BEGINS. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER METAL MANUFACTURER. WELDING ELECTRODES SHALL BE ETOTT-K2 OR ETOT-6 WITH A MINIMUM SPECIFIED CHARPY V-NOTCH (CVN) OF 20 ft-lbs AT -20 DEGREES FAHRENHEIT AND 40 ft-lbs AT 70 DEGREES FAHRENHEIT. REMOVE BOTTOM FLANGE WELD TAB AT MOMENT FRAME CONNECTIONS AND REINFORCE WITH 5/16" FILLET WELD IN CONFORMANCE WITH FEMA-353 RECOMMENDATIONS. WELD ACCESS HOLE DETAILING AT MOMENT FRAME CONNECTIONS SHALL CONFORM WITH FEMA-350 AND FEMA-353 RECOMMENDATIONS.

41. METAL FLOOR AND ROOF DECKING - PROVIDE SIZE, TYPE, GAUGE, AND ATTACHMENT TO THE SUPPORTING STRUCTURE AS SHOWN ON THE PLANS. ALTERNATES MUST BE CONNECTED ACCORDING TO PUBLISHED I.C.C. OR IAPMO UES CRITERIA FOR DIAPHRAGM SHEARS SHOWN. PROVIDE SHORING WHERE REQUIRED PER MANUFACTURER'S PUBLISHED CRITERIA. ALL DECKING SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.

42. HEADED STUDS FOR COMPOSITE CONNECTION OF STRUCTURAL STEEL TO CAST-IN-PLACE CONCRETE SHALL BE MANUFACTURED FROM MATERIAL CONFORMING TO ASTM A29 AND SHALL BE WELDED IN CONFORMANCE WITH A.W.S. REQUIREMENTS.

43. DEFORMED BAR ANCHORS (DBA) SHALL BE TYPE D2L ANCHORS BY NELSON STUD WELDING DIVISION, TRW ASSEMBLIES AND FASTENERS GROUP (OR EQUIVALENT). ANCHORS SHALL BE MADE FROM COLD ROLLED, DEFORMED STEEL CONFORMING TO ASTM A1064.

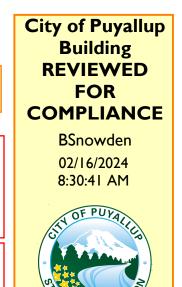
### Approval for Foundation only. Other items must be approved via separate permit submittal.

he approved construction plans, documents, and all ngineering must be posted on the job at all inspections in a isible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection

oversights by this office or non compliance with any applicable equiations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

Approval of submitted plans is not an approval of omissions of





## GENERAL STRUCTURAL NOTES SHEET NO.

	NORTH YARD		2/5/24	
NO.	DESCRIPTION		DATE	BY
ISSU	ES:	RE	VISIONS:	$\overline{\bigtriangleup}$
<b>P.M</b> .		SHT		
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DRA	WN BY:	SC		
SCA	LE:	AS SHO	NWC	
DAT	E:	2/5/24		
JOB	NO.	23444.	01	
SHE	ET TITLE:			

City of Puyallup Development & Permitting Services ISSUED PERMIT			
Building	Planning		
Engineering	Public Works		
Fire	Traffic		

**APPROVAL:** 

SEAL:

**1023 39TH AVENUE** SOUTHEAST **PUYALLUP, WASHINGTON** 





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**1511 THIRD AVENUE** 

**SEATTLE, WA 98101** 

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FAX 206.957.3901

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SUITE 323

	FRAMING ITEMS, REFER TO CATALOG NUMBERS OF MEMBERS MANUFACTURED BY THE STEEL STUE MANUFACTURERS ASSOCIATION (SSMA). PRODUCTS BY OTHER MANUFACTURERS MAY BE SUBSTITUTED FOR FRAMING SHOWN, PROVIDED THEY ARE EQUIVALENT IN SHAPE, SIZE, STIFFNESS AND STRENGTH. ALTERNATE FRAMING SHALL BE SUBJECT TO REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION. ALL COLD-FORMED STEEL FRAMING SHALL CONFORM TO THE A.I.S.I. "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
	<u>MATERIAL:</u> METAL FRAMING SHALL BE GALVANIZED UNLESS OTHERWISE NOTED, CONFORMING AS FOLLOWS:
	ASTM A653 SS GRADE 50, CLASS I OR 3 Fy = 50 KSI 118, 97, 68, AND 54 MIL ASTM A653 SS GRADE 33 Fy = 33 KSI 43 AND 33 MIL
	WHERE NOTED, PAINTED STUDS SHALL CONFORM TO: ASTM AIOII SS GRADE 50
	<u>WELDING</u> OF COLD-FORMED METAL FRAMING SHALL CONFORM TO AWS DI.3 AND SHALL BE PERFORMED BY WELDERS QUALIFIED TO PRODUCE THE SPECIFIED CLASSES OF WELD.
	MALL FRAMING: ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 4005162-43 @ 16" O.C. AT INTERIOR WALLS AND 6005162-43 AT 16" O.C. AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS TWO 8005162-54 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS UNLESS OTHERWISE NOTED JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS. SOLID BLOCKING FOR MULTI-STUD OR STEEL COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW. PROVIDE CONTINUOUS FULL WIDTH BLOCKING AT 1/3 POINTS OF ALL STUD WALLS UNLESS NOTED OTHERWIS MAXIMUM GAP BETWEEN STUD AND TRACK AT ANY POINT SHALL NOT EXCEED 1/16-INCH. NO SPLICES ARE PERMITTED IN STUDS.
	ALL STUD WALLS SHALL HAVE THEIR BOTTOM TRACKS ATTACHED TO FRAMING BELOW WITH #10 SCREWS AT 16" O.C. OR ATTACHED TO CONCRETE WITH 5/32" DIAMETER DRIVE-PINS @ 16" O.C. UNLESS INDICATED OTHERWISE. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE WELDED OR SCREWED TO EACH OTHER IN ACCORDANCE WITH THE DETAILS. REFER TO THE PLANS AND SHEA WALL SCHEDULE FOR REQUIRED SHEATHING AND STRAP BRACING. WHEN NOT OTHERWISE NOTED, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES AND GYPSUM SHEATHING ON EXTERIOR SURFACES SCREWED TO ALL STUDS, TOP AND BOTTOM TRACKS, AND BLOCKING WITH SCREWS A 12" O.C. ALL SCREWS SHALL BE "GRABBER" TYPE FASTENERS COMPLYING WITH I.C.C. REPORT ESR-12TI ALL SPECIFIED PNEUMATIC FASTENERS SHALL BE ET&F, COMPLYING WITH I.C.C. REPORT NO. ESR-17T1 ALL BEARING STUDS SHALL BE LATERALLY SUPPORTED TO PREVENT WEAK AXIS BUCKLING WITH
	CENTER U-CHANNEL AT 1/3 POINTS AS SHOWN IN THE DETAILS AND CONNECTING EACH FLANGE TO GYPSUM WALLBOARD PER IBC SECTION 2508.1.
	TRACK SECTIONS SHALL BE UNPUNCHED AND HAVE AT LEAST I" FLANGES AND MATCH STUD THICKNESS.
	WALLS WHICH HAVE SHEATHING CONNECTED ON ONE SIDE ONLY SHALL HAVE UNSHEATHED FLANGES LATERALLY SUPPORTED IN ACCORDANCE WITH THE DETAILS.
C.	ALL COLUMNS SHALL BE DESIGNED ASSUMING THEY ARE FREE TO ROTATE, DO NOT FIX COLUMN BASES. DESIGN AND PROVIDE SUPPORTS AROUND OVERHEAD DOORS. A MINIMUM COLLATERAL LOADIN OF IO PSF SHALL BE INCLUDED IN THE DESIGN OF THE ROOF TO ACCOUNT FOR MISCELLANEOUS DEAD LOAD. THE LATERAL DRIFT OF THE BUILDING SHALL BE LIMITED TO 0.02H, WHERE H IS THE HEIGHT OF THE BUILDING. ALL ROOF MEMBERS SHALL BE LIMITED TO A MAXIMUM TOTAL
D.	DEFLECTION OF L/180. COORDINATE ALL DETAILS WHICH ARE SHOWN ON THESE DRAWINGS WITH THE PREFABRICATED BUILDING DESIGN.
E.	COORDINATE THE FINAL FOUNDATION LOADING AND BASE PLATE CONFIGURATION WITH THE STRUCTURAL ENGINEER. FOUNDATION DESIGN SHOWN IN THESE PLANS MAY NEED TO BE REVISED BASED UPON THE FINAL DESIGN AND/OR COLUMN LOCATIONS.
	SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. DESIGN SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER. THE BUILDING MANUFACTURER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON THE BASIC STRUCTURE.
	STRUCTURAL OBSERVATION
STRU BUT N THE ( SPEC	OTED IN IBC SECTION 1704.6, STRUCTURAL OBSERVATION IS REQUIRED FOR THIS PROJECT. CTURAL OBSERVATION MEANS THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM, INCLUDIN NOT LIMITED TO, THE ELEMENTS AND CONNECTIONS AT SIGNIFICANT CONSTRUCTION STAGES AND COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND VIFICATIONS. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY OF NSPECTIONS REQUIRED BY IBC SECTIONS 110 AND 1704.
AS C SPEC ACCE INDIC BE IN	R STRUCTURAL OBSERVATION, WE WILL SELECT PORTIONS OF WORK TO REVIEW CLOSELY AS WE OBSERVE THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND VIFICATIONS. SUCH REVIEW PROCEDURES WILL BE CONDUCTED IN ACCORDANCE WITH COMMONLY EPTED STANDARDS OF PRACTICE. THE BUILDING OFFICIAL UNDERSTANDS THAT SUCH PROCEDUR VATE ACTUAL CONDITIONS ONLY WHERE THE REVIEW IS PERFORMED AND THAT THE RESULTS WILL IFERRED TO EXIST IN OTHER AREAS NOT REVIEWED.
MINIM DESI	BUILDING OFFICIAL ALSO RECOGNIZES THAT STRUCTURAL REVIEW IS A TECHNIQUE EMPLOYED TO 11ZE THE RISK OF PROBLEMS ARISING DURING CONSTRUCTION. STRUCTURAL OBSERVATION BY TH GN PROFESSIONAL DOES NOT CONSTITUTE WARRANTY OR GUARANTEE OF ANY TYPE. IN ALL ES, THE CONTRACTOR SHALL RETAIN RESPONSIBILITY FOR THE QUALITY OF WORK AND FOR ERENCE TO THE APPROVED PLANS AND SPECIFICATIONS.

## GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

	VIATIONS	
At Panny (Naile)	L LB.	Angle Pound
Penny (Nails) Diameter	LD. LL	Live Load
Degrees	LLH	Long Leg Horizontal
Pounds Number		Long Leg Vertical
NUMBER	LONGIT. LT. WT.	Longitudinal Lightweight
Above	<u> </u>	
Anchor Bolt	MAX.	Maximum
Additional	MECH.	Mechanical
Alternate Approximate	MEZZ. MF	Mezzanine Moment Frame
Architect	MFR.	Manufacturer
otress Design	MIN.	Minimum
- -	MISC.	Miscellaneous
Below Bottom of	MK.	Mark
Fraced Frame	(N)	New
Blocking	N.	North
Building	N.S.	Near Side
Beam Bottom	NOM. NTS	Nominal Not to Scale
Bearing	NIG	not lo scale
Between	O.C.	On Center
	0.D.	Outside Diameter
Centerline Camber	0.F. 0.H.	Outside Face Overhang
Cast In Place	OPNG.	Opening
Control Joint	OPP.	Opposite
t Penetration		
Ceiling Clear	PAF PC	Powder Actuated Fastener Precast
Masonry Unit	PERM.	Permanent
Column	PERP.	Perpendicular
Concrete		Partial Joint Penetration
Connections Construction	PL or PL PLF	Plate Pounds per linear Foot
Continuous	PLYWD	Plywood
Countersink	PREFAB.	Prefabricated
	PSF	Pounds per Square Foot
d Bar Anchor Double	PSI P.T. or PT	Pounds per Square Inch Post-Tensioning
Degree	P/T	Pressure-Treated
oug Fir-Larch		
Diameter	RAD.	Radius
Diagonal Diaphragm	REF. REINF.	Reference Reinforce or Reinforcement
Dimension	REQD.	Required
Down	REV.	Revise
Ditto	R.O.	Rough Opening
Detail Die Top Plate	S.	South
Drawing	SCH. or SCHE	
- -	SECT.	Section
Existing East	SHT. SIM.	Sheet Similar
	<b>U</b> .	
Each	50G	Slab On Grade
Each Each Face	SOG SPEC.	Slab On Grade Specification
Each Each Face Elevation	SOG SPEC. SQ.	Slab On Grade Specification Square
Each Each Face Elevation Elevator	SOG SPEC. SQ. SQ. FT.	Slab On Grade Specification Square Square Feet
Each Each Face Elevation	SOG SPEC. SQ.	Slab On Grade Specification Square
Each Each Face Elevation Elevator dment Length Engineer Equal	SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way	SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard
Each Each Face Elevation Elevator dment Length Engineer Equal	SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion	SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard
Each Each Face Elevation Elevator dment Length Engineer Equal Each May Expansion Exterior Foundation	SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish	SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural
Each Each Face Elevation Elevator dment Length Engineer Equal Each May Expansion Exterior Foundation Finish Floor	SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical
Each Each Face Elevation Elevator dment Length Engineer Equal Each May Expansion Exterior Foundation Finish Floor rced Polymer Far Side	506 SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet	50G SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove
Each Each Face Elevation Elevator dment Length Engineer Equal Each May Expansion Exterior Foundation Finish Floor rced Polymer Far Side	50G SPEC. 5Q. 5Q. FT. 5Q. IN. SPF 5.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing	50G SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized	506 SPEC. 5Q. 5Q. FT. 5Q. IN. SPF 5.5. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary
Each Each Face Elevation Elevator dment Length Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Wall
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge	50G SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Wall Transverse
Each Each Face Elevation Elevator dment Length Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Wall
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized we Laminated m Wall Board	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board Header Hem Fir	506 SPEC. 50. 50. FT. 50. IN. SPF 5.5. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Vall Transverse Tube Steel
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized we Laminated m Wall Board	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board Header Hem Fir Hanger	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board Header Hem Fir Hanger Horizontal	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Tube Steel Typical Unless Otherwise Noted
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal tural Section Height	506 SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&B T&B SYM. T/ T&B T&B T&B. SYM. T/ T&B T&B T&B T&B. SYM. T/ T&B T&B. SYM. T/ T&B T&B. SYM. T/ T&B. T&B. SYM. T/ T&B. T&B. T&D. SYM. T/ T&B. T&D. SYM. T/ T&B. T&D. SYM. T/ T&B. T&D. SYM. T/ T&B. T&D. SYM. T/ T&B. T&D. SYM. T/ TAB. T&D. SYM. T/ T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Vall Transverse Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized we Laminated m Wall Board Header Hem Fir Hanger Horizontal	506 SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Tube Steel Typical Unless Otherwise Noted
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized we Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal ctural Section Height	506 SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G T&MP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field West With
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal ctural Section Height	SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G SYM. T/ T&B T&G SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized we Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal ctural Section Height	506 SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P. W.T.S.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical Uhless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Work Point Welded Threaded Stud
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal ctural Section Height	SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G SYM. T/ T&B T&G SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without
Each Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal ctural Section Height	506 SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P. W.T.S.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Work Point Welded Threaded Stud Welded Wire Fabric
Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal tural Section Height ide Diameter Inside Face Inch Information Interior	506 SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P. W.T.S. WWF X SECT. X-STR	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Work Point Welded Threaded Stud Welded Wire Fabric
Each Face Elevation Elevator dment Length Engineer Equal Each Way Expansion Exterior Foundation Finish Floor rced Polymer Far Side Foot or Feet Footing Gauge Galvanized ue Laminated m Wall Board d Galvanized Header Hem Fir Hanger Horizontal tural Section Height ide Diameter Inside Face Inch Information Interior	SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P. W.T.S. WWF X SECT.	Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Work Point Welded Threaded Stud Welded Wire Fabric

(A) A.B. ADD'L ALT. APPROX. ARCH. A.S.D. Allowable Str (B) B/ BF BLKG. BLDG. BM. BOT. BRG. BTWN. Bro CL or Q C CIP C.J. ( CJP CLG. CLR. CMU COL. CONC. CONN. CONST. CONT. CSK. Construction Joint or C Complete Joint Concrete N DBA DBL. DEG. DF DIA. DIAG. Deformed Dou DIAPH. DIM. DN. DO DTL. DTP DWG. Double (E) L. EA. E.F. ELEV. EMBED. ENGR. EQ. E.W. EXP. EXT. Embedr FDN. FIN. FLR. FRP F.S. FT. FTG. Fiber Reinforc F GA. GALV. GL GWB Glue Gypsum HDG HDR. HF HGR. HORIZ. HSS HT. Hot Dipped Hollow Structu I.D. I.F. Inside IN. INFO. INT. JT. K KSF KSI Kips per Square Foot Kips per Square Inch



## GENERAL STRUCTURAL NOTES SHEET NO.

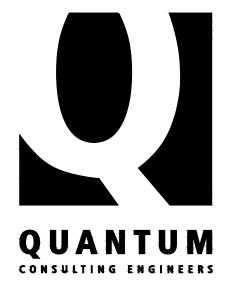
	NORTH YARD		2/5/24	
NO.	DESCRIPTION		DATE	В
ISSU	IES:	RE	VISIONS:	7
P.M.		SHT		
P.E.		тум		
DRA	WN BY:	SC		
SCA	LE:	AS SHO	OWN	
DAT	E:	2/5/24		
JOB	NO.	23444.0	01	

City of Puyallup Development & Permitting Services ISSUED PERMIT			
Building Planning			
Engineering	Public Works		
Fire Traffic			

1023 39TH AVENUE SOUTHEAST PUYALLUP, WASHINGTON APPROVAL:

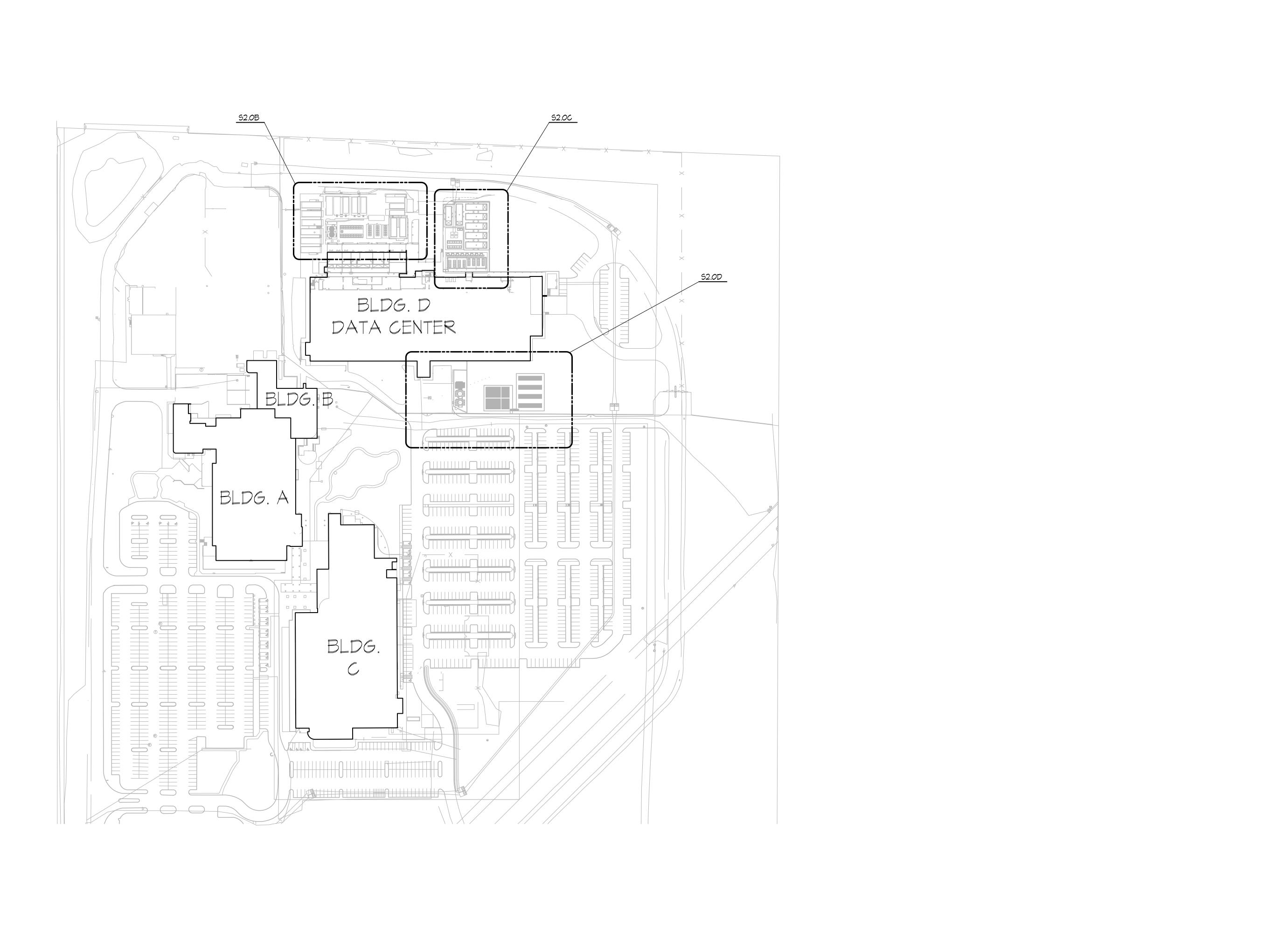






1511 THIRD AVENUE SUITE 323 SEATTLE, WA 98101 TEL 206.957.3900 FAX 206.957.3901 www.quantumce.com

: 444–s200a.dwg Plotted: 02/05/24 11:44 am







SHEET NO.

## PARTIAL SITE REFERENCE PLAN

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				<u> </u>
	NORTH YARD		2/5/24	
NO.	DESCRIPTION		DATE	BY
ISSU	ES:	RE	VISIONS:	
<b>P.M</b> .		SHT		
P.E.		тум		
DRAWN BY:		SC		
SCALE:		AS SHOWN		
DATE:		2/5/24		
JOB NO.		23444.0	01	
SHEI	ET TITLE:			

City of Puyallup Development & Permitting Services ISSUED PERMIT			
Building Planning			
Engineering Public Works			
Fire Traffic			

APPROVAL:

SEAL:

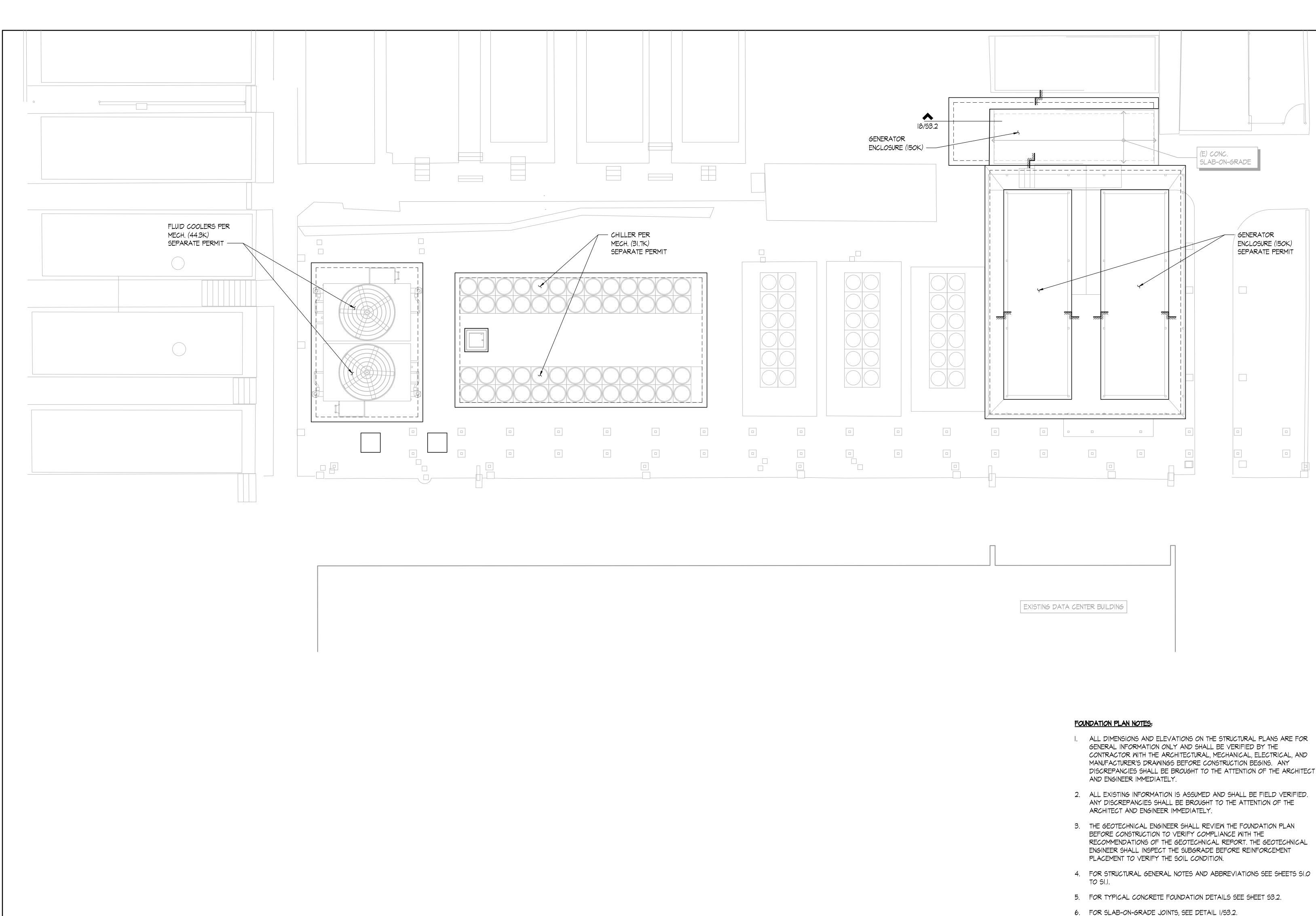
1023 39TH AVENUE Southeast Puyallup, Washington













 $\bigoplus_{\text{SCALE: } |/8" = |'-0"} \text{NORTH YARD FOUNDATION PLAN (WEST)}$ 

- DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT

- Detail 1 on S3.2 was not provided. Please

provide a detail on site for inspection.



## NORTH YARD FOUNDATION PLAN (WEST) SHEET NO.

	NORTH YARD		2/5/24		
NO.	DESCRIPTION		DATE	BY	
ISSU	ISSUES:		VISIONS:	$\overline{\Delta}$	
<b>P.M</b> .		SHT			
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DRAWN BY:		SC			
SCALE:		AS SHOWN			
DATE:		2/5/24			
JOB	JOB NO.		23444.01		
SHEI	SHEET TITLE:				

City of Puyallup Development & Permitting Services ISSUED PERMIT			
Building Planning			
Engineering Public Works			
Fire Traffic			

APPROVAL:

SEAL:

**1023 39TH AVENUE** SOUTHEAST PUYALLUP, WASHINGTON

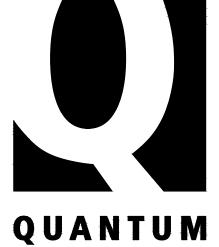




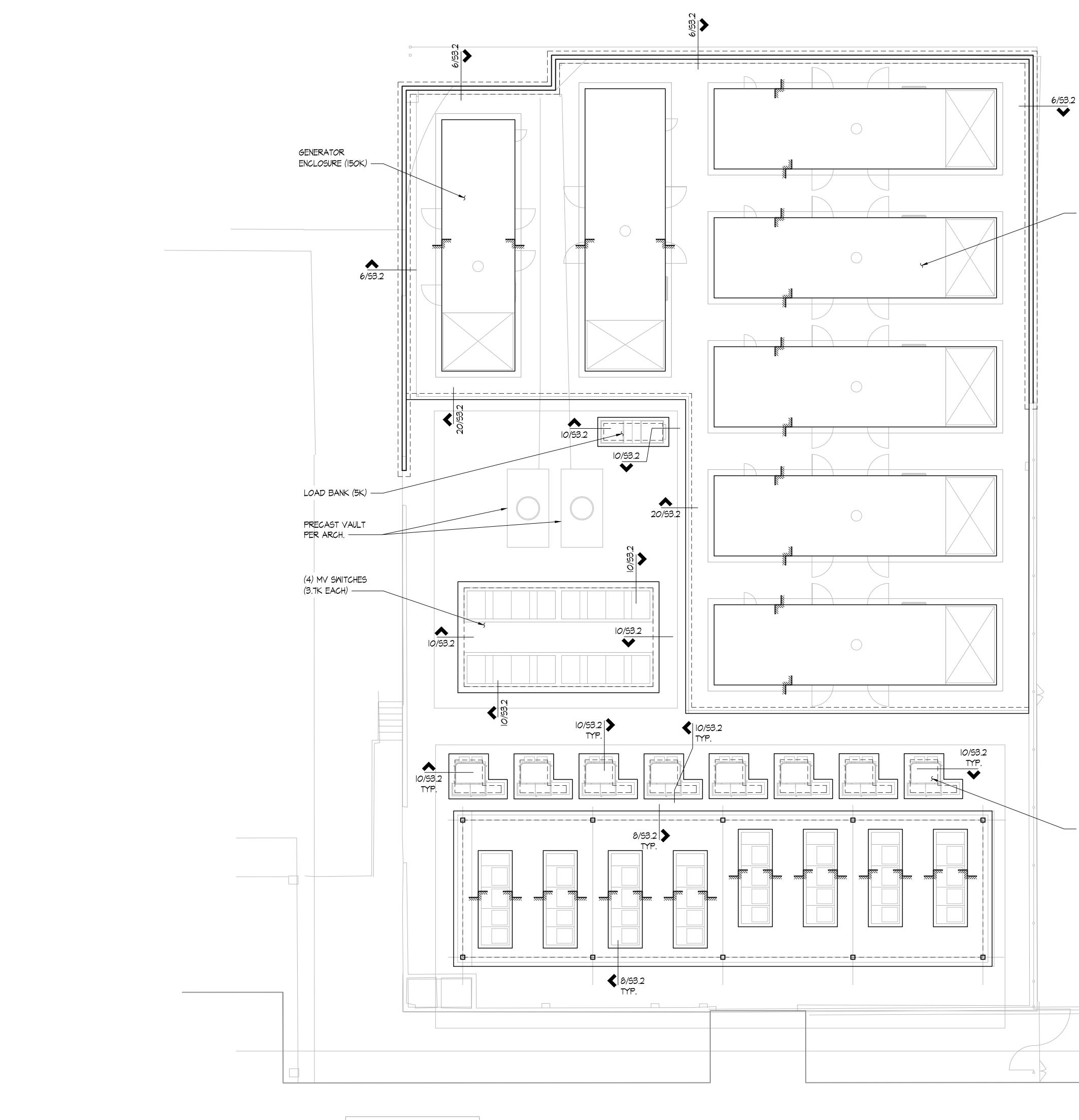




CONSULTING ENGINEERS



PRFO20240187



EXISTING DATA CENTER BUILDING

NORTH YARD FOUNDATION PLAN (EAST) SCALE: 1/8" = 1'-0"

#### - GENERATOR ENCLOSURE (157K), TYP. U.O.N.

#### FOUNDATION PLAN NOTES:

- I. ALL DIMENSIONS AND ELEVATIONS ON THE STRUCTURAL PLANS ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED BY THE CONTRACTOR WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND MANUFACTURER'S DRAWINGS BEFORE CONSTRUCTION BEGINS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- 2. ALL EXISTING INFORMATION IS ASSUMED AND SHALL BE FIELD VERIFIED. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- 3. THE GEOTECHNICAL ENGINEER SHALL REVIEW THE FOUNDATION PLAN BEFORE CONSTRUCTION TO VERIFY COMPLIANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT. THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE BEFORE REINFORCEMENT PLACEMENT TO VERIFY THE SOIL CONDITION.
- 4. FOR STRUCTURAL GENERAL NOTES AND ABBREVIATIONS SEE SHEETS SI.O TO 51.1.
- 5. FOR TYPICAL CONCRETE FOUNDATION DETAILS SEE SHEET 53.2.

- (6) TRANSFORMERS (14.6K EACH)

# S2.0C

## NORTH YARD FOUNDATION PLAN (EAST) SHEET NO.

	NORTH YARD		2/5/24		
NO.	DESCRIPTION		DATE	BY	
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DRA	WN BY:	SC			
SCALE:		AS SHOWN			
DAT	DATE:		2/5/24		
JOB	NO.	23444.01			
SHEET TITLE:					

City of Puyallup Development & Permitting Services ISSUED PERMIT			
Building Planning			
Engineering Public Works			
Fire Traffic			

APPROVAL:

SEAL:

**1023 39TH AVENUE** SOUTHEAST PUYALLUP, WASHINGTON

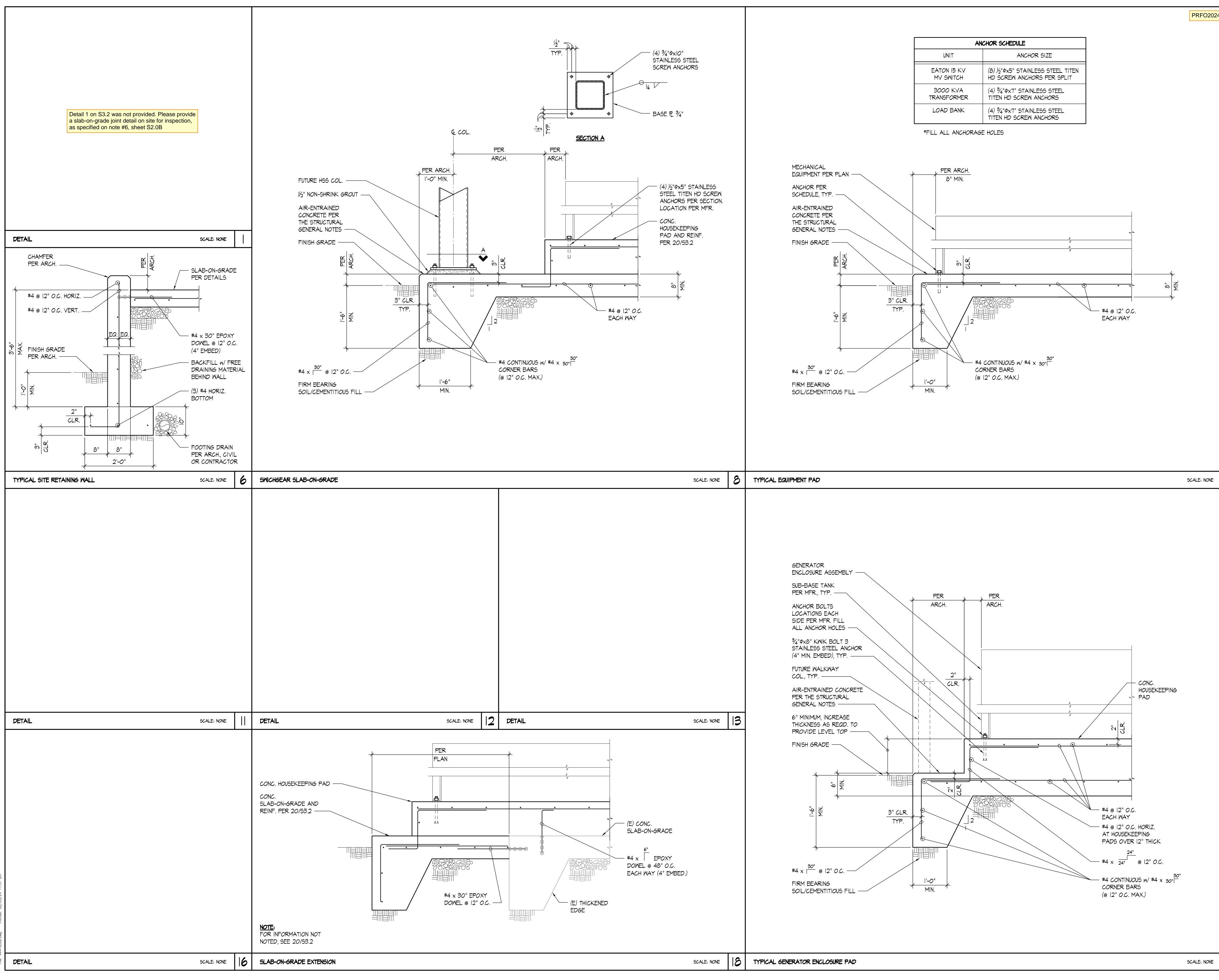




Centers for Information Systems







ANCHOR SCHEDULE		
UNIT ANCHOR SIZE		
EATON 15 KV MV SWITCH	(8) $\frac{1}{2}$ " $\phi_{x5}$ " stainless steel titen HD screw anchors per split	
3000 KVA TRANSFORMER	(4) $\frac{3}{4}$ " $\phi_{X}$ 7" STAINLESS STEEL TITEN HD SCREW ANCHORS	
LOAD BANK	(4) $\frac{3}{4}$ " $\Phi \times 7$ " STAINLESS STEEL TITEN HD SCREW ANCHORS	



SHEET NO.

## DETAILS

				<u> </u>
				<u> </u>
				<b></b>
	NORTH YARD		2/5/24	
NO.	DESCRIPTION		DATE	BY
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<b>P.M</b> .		SHT		
P.E.		TVM		
DRA	WN BY:	SC		
SCALE:		AS SHOWN		
DATE:		2/5/24		
JOB NO.		23444.0	01	
SHE	ET TITLE:			

City of Puyallup Development & Permitting Services ISSUED PERMIT			
Building Planning			
Engineering Public Works			
Fire Traffic			

APPROVAL:

SEAL:

**1023 39TH AVENUE** SOUTHEAST PUYALLUP, WASHINGTON



centeris Centers for Information Systems

FAX 206.957.3901 www.quantumce.com

