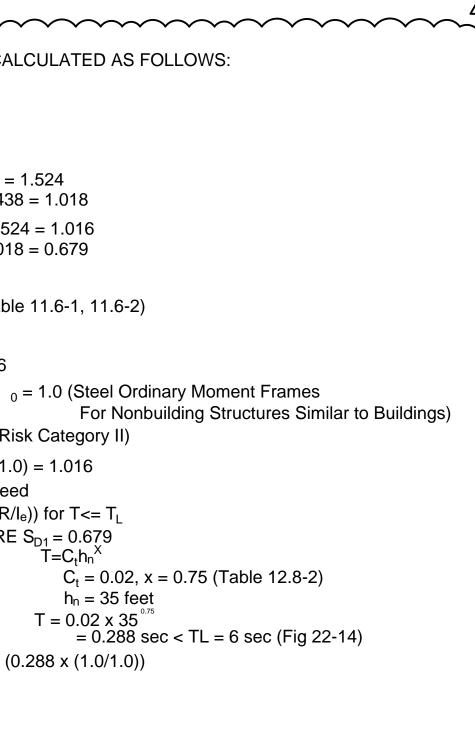
ensile Membrane Structu Puyallup , Washington	re	
ED PER ASCE 7-16		F. SEISMIC LOADS ARE CALCULATED AS FOLLOWS:
RESS IS 15 POUNDS PE	ER LINEAR INCH IN BOTH	$S_s = 1.27, S_1 = 0.438$
	ORM (ignore)	Site Class E $F_a = 1.2$ $F_v = 2.324$
		$S_{MS} = F_a S_s = 1.2 \times 1.27 = 1.524$ $S_{M1} = F_v S_1 = 2.324 \times 0.438 = 1.018$
		S <sub>DS</sub> = $2/3S_{MS} = 2/3 \times 1.524 = 1.016$
ully Exposed Roof) tures) building)		S <sub>D1</sub> = $2/3S_{M1} = 2/3 \times 1.018 = 0.679$ At Risk Category II, Seismic category D (Table 11.6-1, 11.6-2)
v  load) > 20 psf, P <sub>m</sub> = 20(1	l <sub>s</sub> )= 20 psf	$C_{\rm s} = S_{\rm DS}/({\rm R/I_e})$
	$P_m = 20 \text{ psf}$	$S_{DS} = 1.016$ $R = 1.0,  _{0} = 1.0 \text{ (Steel Ordinary Moment Frames}$
Balanced Snow (SL1) nced Snow (SL2) nced Snow (SL3)		For Nonbuilding Structures Similar to Building $I_e = 1.0$ (Risk Category II)
ROOF ARE BASED ON A	WIND VELOCITY OF 98 MPH.	C <sub>s</sub> = $1.016/(1.0/1.0) = 1.016$ C <sub>s</sub> need not exceed
ET, EXPOSURE "C"	egory II)	$C_{s} = S_{D1}/(T(R/I_{e})) \text{ for } T \le T_{L}$ $WHERE S_{D1} = 0.679$ $T = C_{t}h_{n}^{X}$
exposure C)		$C_{t} = 0.02, x = 0.75 \text{ (Table 12.8-2)}$ $h_{n} = 35 \text{ feet}$
		$T = 0.02 \times 35^{0.75}$ = 0.288 sec < TL = 6 sec (Fig 22-14)
N <sub>d</sub> x N <sub>e</sub> x V 2.0 ) x 0.85 x 1.0 x 98**2.0		C <sub>s</sub> ==0.679 / (0.288 x (1.0/1.0))
or ASD Load		2.358 Therefore
	11 psf x C <sub>p</sub>	C <sub>s</sub> = 1.016 Horizontal Seismic Load Effect including Overstrength
		$E_{mh} = {}_{0}Q_{E}$
		$= 1.0 \times 1.016 \times W = 1.02 W$ (Note) Apply 100% (1.02W) of the force for one direction plus
slope	coefficient	30% (0.31W) of the force for perpendicular direction.
90 to 45 deg. 45 to 38	1.1 inward 1.3 inward	Vertical Seismic Load Effect $E_v = 0.2S_{DS}D$
38 to 30 30 to 15 15 to 0	1.3 inward 1.1 inward 1.1 inward	=0.2 X 1.016 X W = 0.20 W
slope 90 to 45 deg.	coefficient 0.9 inward	G. SIGN CONVENTION: POSITIVE FORCES ARE DEFINED TO BE IN THE DIRE OF THE X-Y AXIS SHOWN ON THE REACTION NODE MAP WITH Z ACTING OUT OF THE PAGE.
38 to 30 30 to 15	0.2 inward 0.4 outward	H. THE EFFECTS OF MEMBRANE PRESTRESS AND DEAD LOAD ARE INCLUD LOAD CASES.
13 10 0	0.5 Outward	I. THE LOADS TABULATED REPRESENT FORCES THAT ARE APPLIED TO THE OR FOUNDATION AT THAT LOCATION.
slope 90 to 45 deg. 45 to 38	coefficient 0.3 outward 0.2 outward	J. THE COORDINATES SHOWN MAY NOT REFLECT THE FINAL WORK POINTS STRUCTURE, THEY ARE FOR ANALYSIS PURPOSES ONLY.
38 to 30 30 to 15	0.1 outward 0.1 inward	
15 to 0	0.2 inward	LOAD CASE DESCRIPTION
90 to 45 deg. 45 to 38 38 to 30 30 to 15 15 to 0	Coefficient 0.5 outward 0.8 outward 1.1 outward 1.2 outward	<pre>(Case 1) Prestress : DL+PS (Case 2) Uniform Balanced Snow Load : DL+PS+SL1 (Case 3) Unbalanced Snow Load :1 DL+PS+SL2 (Case 4) Unbalanced Snow Load :1 DL+PS+SL3 (Case 5) Wind Load (Uniform Uplift Wind) : DL+PS+WL1 (Case 6) Wind Load (+X wind at case A) : DL+PS+WL2 (Case 7) Wind Load (+Y wind at case A) : DL+PS+WL3 (Case 8) Wind Load (-Y wind at case A) : DL+PS+WL4 (Case 9) Wind Load (+Y wind at case A) : DL+PS+WL5 (Case 10) Wind Load (+X wind at case B) : DL+PS+WL6 (Case 11) Wind Load (+X wind at case B) : DL+PS+WL6 (Case 11) Wind Load (+Y wind at case B) : DL+PS+WL8 (Case 12) Wind Load (-Y wind at case B) : DL+PS+WL9 (Case 13) Wind Load (+XY wind at case B) : DL+PS+WL9 (Case 14) Uniformed Balanced Snow Load + Wind Load : DL+0.75SL1+0.75W (Case 15) X Unbalanced Snow Load + Wind Load : DL+0.75SL2+0.75WL3 (Case 16) Y Unbalanced Snow Load + Wind Load : DL+0.75SL3+0.75WL3 (Case 17) +X Seismic Load : DL+PS+(0.7Ev + 0.7Emh)@+X (Case 18) +Y Seismic Load : DL+PS+(0.7Ev + 0.7Emh)@+Y (Case 20) +XY Seismic Load : DL+PS+(0.7Ev + 0.7Emh)@+X (Case 21) +X Seismic Load : DL+PS+(0.7Ev + 0.7Emh)@+X (Case 21) +X Seismic Load + Snow Load : DL+PS+(0.525Ev + 0.525Emh)@+X (Case 22) +Y Seismic Load + Snow Load : DL+PS+(0.525Ev + 0.525Emh)@+X (Case 23) -Y Seismic Load + Snow Load : DL+PS+(0.525Ev + 0.525Emh)@+Y (Case 24) +XY Seismic Load + Snow Load : DL+PS+(0.525Ev + 0.525Emh)@-Y (Case 24) +XY Seismic Load + Snow Load : DL+PS+(0.525Ev + 0.525Emh)@-Y</pre>
	ensile Membrane Structu Puyallup , Washington A – IBC 2018/ASCE 7-16 RESS IS 15 POUNDS PE IRECTIONS. NOPY IS 12.0 PSF UNIF CULATED AS FOLLOWS: g ully Exposed Roof) tures) building) / load) > 20 psf, P <sub>m</sub> = 20(1 x 25 = 18.9 psf> USE calanced Snow (SL1) nced Snow (SL2) nced Snow (SL2) nced Snow (SL3) ROOF ARE BASED ON A PH (3 Sec Gust, Risk cate ET, EXPOSURE "C" exposure C) K <sub>d</sub> x K <sub>e</sub> x V**2.0 ) x 0.85 x 1.0 x 98**2.0 or ASD Load C <sub>p</sub> ) = 10.77 x C <sub>p</sub> > USE APPLIED TO THE STRU BULATED BELOW (Pitcl Slope 90 to 45 deg. 45 to 38 38 to 30 30 to 15 15 to 0 Slope 90 to 45 deg. 45 to 38 45 to 38	A - IBC 2018/ASCE 7-16 - OPEN STRUCTURE ED PER ASCE 7-16 RESS IS 15 POUNDS PER LINEAR INCH IN BOTH IRECTIONS. NOPY IS 12.0 PSF UNIFORM (ignore) ULATED AS FOLLOWS: g Illy Exposed Roof) tures) building) (load) > 20 psf, Pm = 20(ls) = 20 psf x 25 = 18.9 psf> USE Pm = 20 psf talanced Snow (SL1) need Snow (SL2) need Snow (SL2) need Snow (SL3) ROOF ARE BASED ON A WIND VELOCITY OF 98 MPH. PH (3 Sec Gust, Risk category II) ET, EXPOSURE "C" xposure C) $K_{4} x K_{6} x V^{*2.0}$ $x 0.85 x 1.0 x 98^{*2.0}$ or ASD Load $C_{p}$ = 10.77 x $C_{p}$ > USE 11 psf x $C_{p}$ APPLIED TO THE STRUCTURE ON AN ELEMENT BY IBULATED BELOW (Pitched Free Roofs – ASCE7-16 Fig.27 Slope slope slope coefficient 90 to 45 deg. 1.1 inward 38 to 30 1.3 inward 38 to 30 0.2 inward 30 to 15 0.4 outward 30 to 15 0.4 outward 30 to 15 0.1 outward 30



SITIVE FORCES ARE DEFINED TO BE IN THE DIRECTION WN ON THE REACTION NODE MAP WITH Z ACTING UP AND

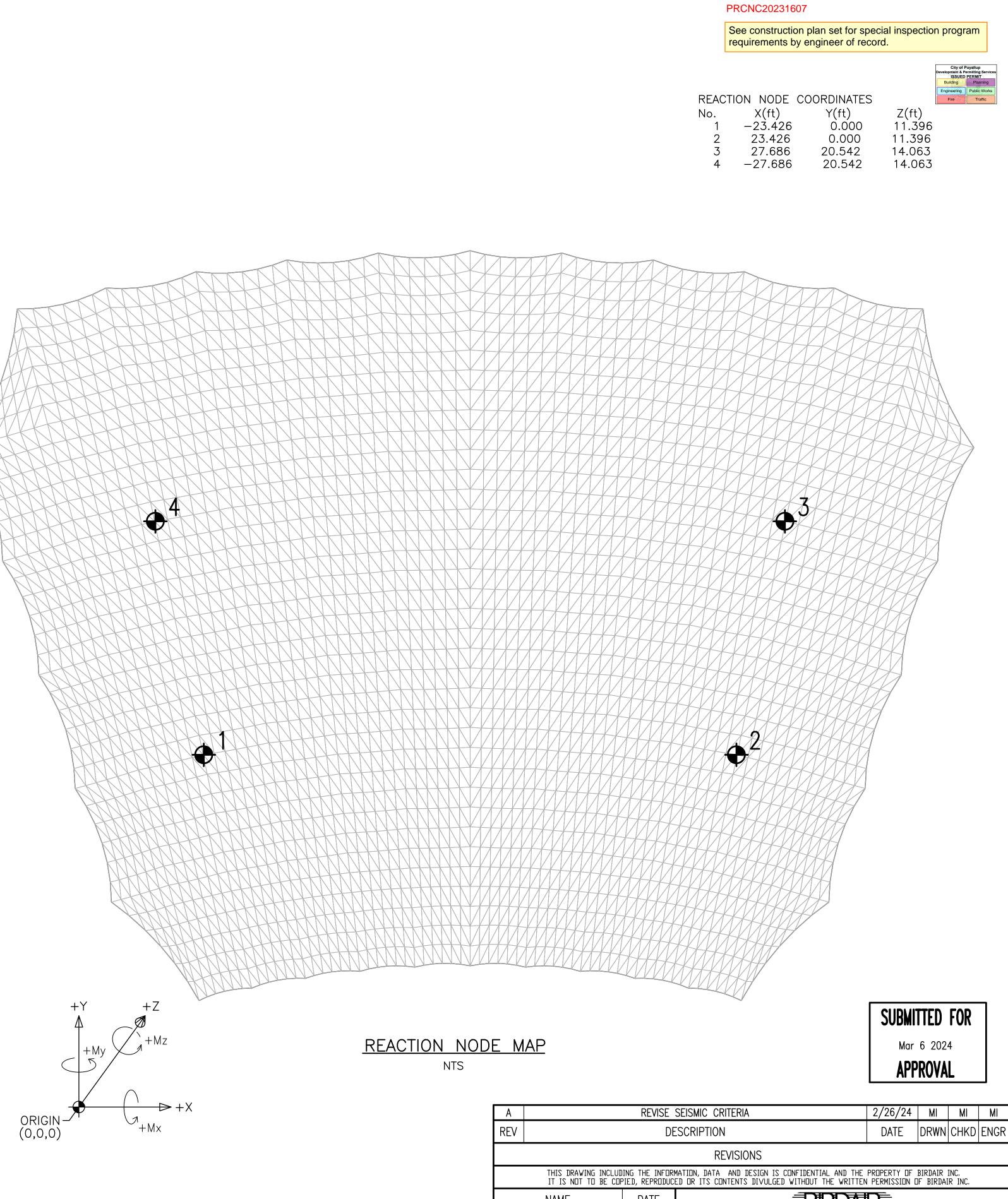
BRANE PRESTRESS AND DEAD LOAD ARE INCLUDED IN ALL

REPRESENT FORCES THAT ARE APPLIED TO THE BUILDING HAT LOCATION.

WN MAY NOT REFLECT THE FINAL WORK POINTS IN THE E FOR ANALYSIS PURPOSES ONLY.

## TION

Snow Load : DL+PS+SL1 w Load :1 DL+PS+SL2 w Load :1 DL+PS+SL3 rm Uplift Wind) : DL+PS+WL1 ind at case A) : DL+PS+WL2 ind at case A) : DL+PS+WL3 nd at case A) : DL+PS+WL4 wind at case A) : DL+PS+WL5 wind at case B) : DL+PS+WL6 wind at case B) : DL+PS+WL7 vind at case B) : DL+PS+WL8 wind at case B) : DL+PS+WL9 nced Snow Load + Wind Load : DL+0.75SL1+0.75WL3 now Load + Wind Load : DL+0.75SL2+0.75WL3 now Load + Wind Load : DL+0.75SL3+0.75WL3 I : DL+PS+(0.7E<sub>v</sub> + 0.7E<sub>mh</sub>)@+X  $1: DL+PS+(0.7E_v + 0.7E_{mh})@+Y$ :  $DL+PS+(0.7E_v + 0.7E_{mh})@-Y$ ad : DL+PS+(0.7E<sub>v</sub> + 0.7E<sub>mh</sub>)@+XY d + Snow Load : DL+PS+(0.525E<sub>v</sub> + 0.525E<sub>mh</sub>)@+X + 0.75SL1 d + Snow Load : DL+PS+(0.525E<sub>v</sub> + 0.525E<sub>mh</sub>)@+Y + 0.75SL1 I + Snow Load : DL+PS+(0.525E<sub>v</sub> + 0.525E<sub>mh</sub>)@-Y + 0.75SL1 ad + Snow Load : DL+PS+(0.525E<sub>v</sub> + 0.525E<sub>mh</sub>)@+XY + 0.75SL1





	4/26/2024
Expires 07/26/2025	

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NAM	E	DATE	BRDAR	
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CHECKED BY	MI	10/13/23	TELEPHONE: 716–633–9500 FAX: 716–204–1234 TITLE	
ENGINEER	MI	10/13/23	INTERFACE LOADING CRITERIA AND REACTION NODE MAP	
FINAL REVIEW			WASHINGTON STATE FAIRGROUNDS	
SCALE	DRAWING AS NOTED	I SIZE	DWG. NO. 23008 - 1000 REV A	

REACTION L	(NOTE) LOA	DS ARE ASD	COMBINATIO	IN LOAD	~~~~~
1 PRESTRES No. 1 2 3 4 Total	RFy(Kip) 2.1 2.1 -2.1 -2.1 0.0	RFz(Kip) RI -11.8 -11.8 -17.4 -17.4 -58.5	0.0 0.0 0.0	RMy(Kip*ft) 0.1 -0.1 -0.1 0.1	RMz(Kip*ft) 0.1 -0.1 -0.1 0.1
2 SNDW LOA No. 1 2 3 4 Total	RFy(Kip) 5.7 5.7 -5.7	-27.6 -27.6	-0. 1 -0. 1 -0. 1	RMy(Kip*ft) 0,2 -0,2 -0,4 0,4	RMz(Kip*ft) 0.1 -0.1 -0.2 0.2
3 SNOW LOA No. 1 2 3 4 Total			Mx(Kip*ft) 0.0 -0.1 -0.1 0.0	RMy(Kip*ft) 0.1 -0.2 -0.3 0.2	RMz(Kip*ft) 0.1 0.0 -0.2 0.1
4 SNOW LOA No. 1 2 3 4 Total		RFz(Kip) RI -43.5 -43.5 -9.5 -9.5	-0. 1 -0. 1	RMy(Kip*ft) 0.2 -0.2 -0.1 0.1	RMz(Kip*ft) 0.0 0.0 -0.1 0.1
5 UNIFORM No. 1 2 3 4 Total	RFy(Kip) -1.3 -1.3 -0.9	-5.2 -5.2 0.6 0.6	0, 0	RMy(Kip*ft) 0.0 0.0 -0.1 0.1	RMz(Kip*ft) 0.0 0.0 0.0 0.0 0.0
6 +X WIND No. 2 3 4 Total	RFy(Kip) 3.3 1.6 -1.5	-4.4 -8.5 -13.9	0. 0 0. 0 0. 0	RMy(Kip*ft) 0.1 0.0 -0.1 0.2	RMz(Kip*ft) 0.0 -0.1 -0.1 0.1
7 +Y WIND No. 1 2 3 4 Total		-21.2		RMy(Kip*ft) 0.2 -0.2 -0.3 0.3	RMz(Kip*ft) 0, 2 -0, 2 -0, 2 0, 2 0, 2
8 -Y WIND No. 1 2 3 4 Total		RFz(Kip) RI -8.8 -8.8 -13.4 -13.4 -44.4	Mx(Kip*ft) 0.0 0.0 0.0 0.0 0.0	RMy(Kip*ft) 0.0 0.0 -0.1 0.1	RMz(Kip*ft) 0.1 -0.1 -0.1 0.1 0.1
9 +X+Y WIN No. 2 3 4 Total		RFz(Kip) RI -15.8 -9.5 -19.0 -35.7 -80.0	Mx(Kip*ft) −0.1 0.0 0.0 −0.1	RMy(Kip*ft) 0.2 0.0 -0.1 0.3	RMz(Kip*ft) 0.1 -0.1 -0.2 0.1
10 +X WIND No. 1 2 3 4 Total		RFz(Kip) RI 0.0 -4.2 4.3 -7.1 -7.0	Mx(Kip*ft) 0.0 0.0 0.0 0.0 0.0	RMy(Kip*ft) 0.0 0.0 0.0 0.0 0.0	RMz(Kip*ft) 0.0 0.0 -0.1 0.0
11 +Y WIND No. 2 3 4 Total	RFy(Kip) 1.6 1.6	-16.0 -16.0 -11.3 -11.3	Mx(Kip*ft) 0.0 0.0 0.0 0.0 0.0	RMy(Kip*ft) 0.1 -0.1 -0.2 0.2	RMz(Kip*ft) 0.1 -0.1 -0.1 0.1 0.1
12 -Y WIND No. 1 2 3 4 Total	RFy(Kip) -2.0 -2.0 -0.8	-4.4 -4.4 4.7	Mx(Kip*ft) 0.0 0.0 0.0 0.0 0.0	RMy(Kip*ft) 0.0 0.0 0.0 0.0 0.0	RMz(Kip*ft) 0.0 0.0 0.0 0.0 0.0

$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	b) RFy(Kip) RFz 4 2,5 9 −1,0 3 −1,1	z(Kip) RMx(K -10.6 -5.2 -0.8 -17.9 -34.6	ip*ft) 0.0 0.0 0.0 0.0 0.0	0, 1 0, 0	RMz(Kip*ft) 0.0 -0.1 -0.1 0.0
14 BALANCED SNUW No. RF×(Ki 1 -6. 2 6. 3 15. 4 -15. Total 0.	5) RFy(Kip) RFz 5 7,4 5 7,4 5 −3,9 5 −3,9	-30. 1 -30. 1	ip*ft) -0.1 -0.1 -0.1 -0.1 -0.1	0, 3 -0, 3 -0, 4	0, 2
15 X UNBALANCED No. RFx(Ki 1 -4. 2 5. 3 11. 4 -11. Total 0.	o) RFy(Kip) RFz 5 4,9 1 7,7 0 −4,2 6 −1,3	-19. 1 -33. 4		0, 2 -0, 2	0. 2
16 Y UNBALANCED No. RFx(Ki 1 -8. 2 8. 3 8. 4 -8. Total 0.	5) RFy(Kip) RFz 4 7.0 4 7.0 5 −3.5 5 −3.5	z(Kip) RMx(K -42.0 -42.0 -21.3 -21.3 -126.7	ip*ft) -0.1 -0.1 0.0 0.0		RMz(Kip*ft) 0.1 -0.1 -0.2 0.2
17 +X SEISMIC LD No. RFx(Ki 1 4. 2 10. 3 18. 4 6. Total 41.	b) RFy(Kip) RFz ∂ 3,5 5 7,5 5 −0,1 ∂ 1,6	-4. 8 -12. 8 -32. 6	ip*ft) -0.1 0.1 0.1 -0.2	0, 4 0, 3 0, 2	RMz(Kip*ft) 0.0 -0.2 -0.3 0.0
1 -0.	<ul> <li>b) RFy(Kip) RFz</li> <li>12.4</li> <li>13.6</li> <li>7.3</li> <li>7.8</li> </ul>	3. 9 1. 5 -38. 1	ip*ft) -0.1 0.0 0.0 -0.1	0, 2 0, 0 -0, 1	RMz(Kip*ft) 0.2 -0.2 -0.3 0.2
1 -5. 2 0. 3 -4.	b) RFy(Kip) RFz 5 −7,9 9 −9,1 1 −11,8 7 −12,3	z(Kip) RMx(K -31.7 -29.2 0.3 -4.9 -65.5	ip*ft) 0.0 -0.1 0.0 0.0	0. 0 -0. 2	RMz(Kip*ft) -0.1 0.1 0.1 0.0
20 +XY SEISMIC L No. RFx(Ki 1 2. 2 8. 3 16. 4 1. Total 29.	<ul> <li>b) RFy(Kip) RFz</li> <li>8, 4</li> <li>11, 2</li> <li>4, 1</li> <li>5, 2</li> </ul>	z(Kip) RMx(K 0.7 -5.0 -36.7 -24.5 -65.5	ip*ft) -0.1 0.0 0.0 -0.1	RMy(Kip*ft) 0.3 0.2 0.1 0.4	RMz(Kip*ft) 0.1 -0.2 -0.3 0.1
21 +X SEISMIC LD No. RFx(Ki 1 0. 2 11. 3 20. 4 -1. Total 30.	b) RFy(Kip) RFz 1 6.0 5 8.9 9 −3.4 3 −2.1	-18. 1 -24. 3	ip*ft) -0.1 0.0 0.0 -0.1	0, 5 0, 1 0, 0	RMz(Kip*ft) 0.1 -0.2 -0.3 0.1
1 -3.	b) RFy(Kip) RFz 3 12.6 3 13.5 7 2.1 9 2.5	-11.6 -13.5	ip*ft) -0.1 -0.1 0.0 -0.1	0, 3 -0, 1 -0, 3	0.2
1 -7. 2 4. 3 3. 4 -9.	o) RFy(Kip) RFz	-38.4 -36.5 -26.9	ip*ft) -0.1 -0.1 -0.1 -0.1 0.0	0. 1 -0. 3	RMz(Kip*ft) 0.0 0.1 -0.1 0.1
24 +XY SEISMIC L No. RFx(Ki 1 -1. 2 9. 3 19. 4 -6. Total 21.	b) RFy(Kip) RFz 5 9,7 7 11,7 5 −0,3 0 0,6	z(Kip) RMx(K -14.1 -18.4 -54.7	ip*ft) -0.1 0.0 0.0 -0.1	RMy(Kip*ft) 0.4 0.0 -0.1 0.6	RMz(Kip*ft) 0.1 -0.2 -0.3 0.2



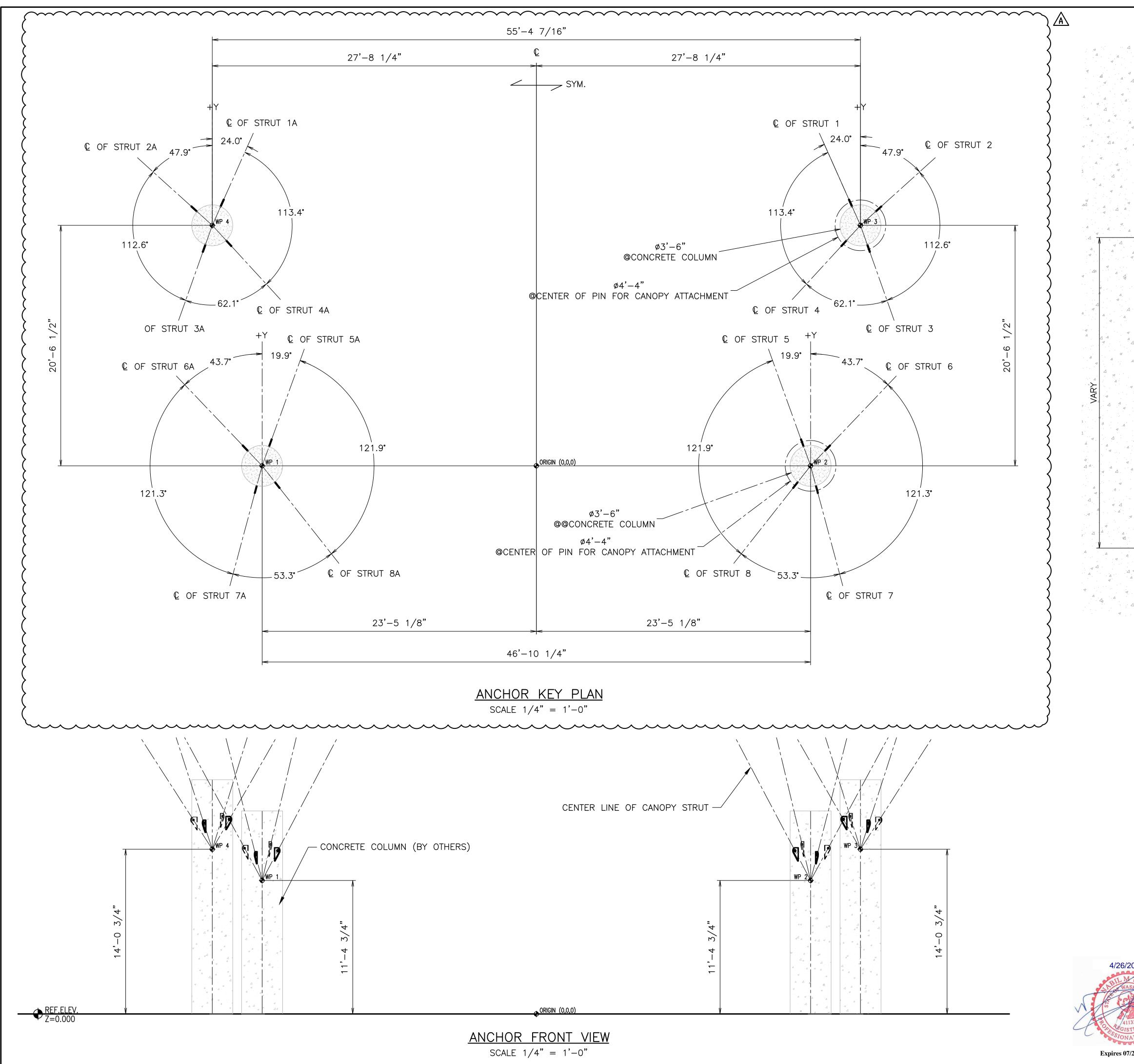
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CHECK	(ED BY MI	10/13/23	WASHINGTON STATE FAIRGROUNDS				
ENGIN	eer <b>MI</b>	10/13/23					
FINAL	REVIEW						
SCALE	as noted dwg. no. 23008 - 1001				rev A		

SUBMITTED FOR

Mar 6 2024

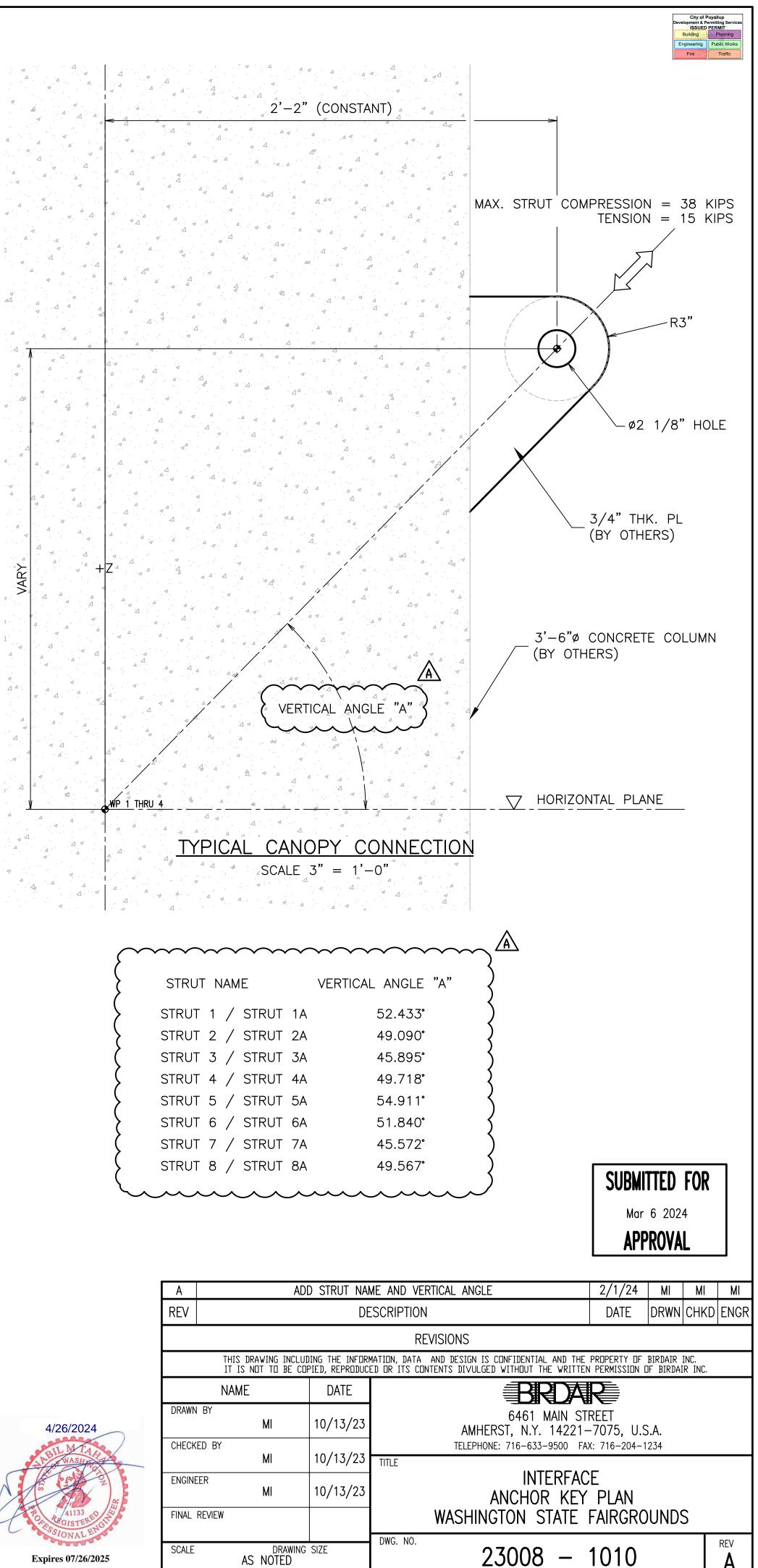
APPROVAL

City of Puyallup Development & Permitting Service: ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic



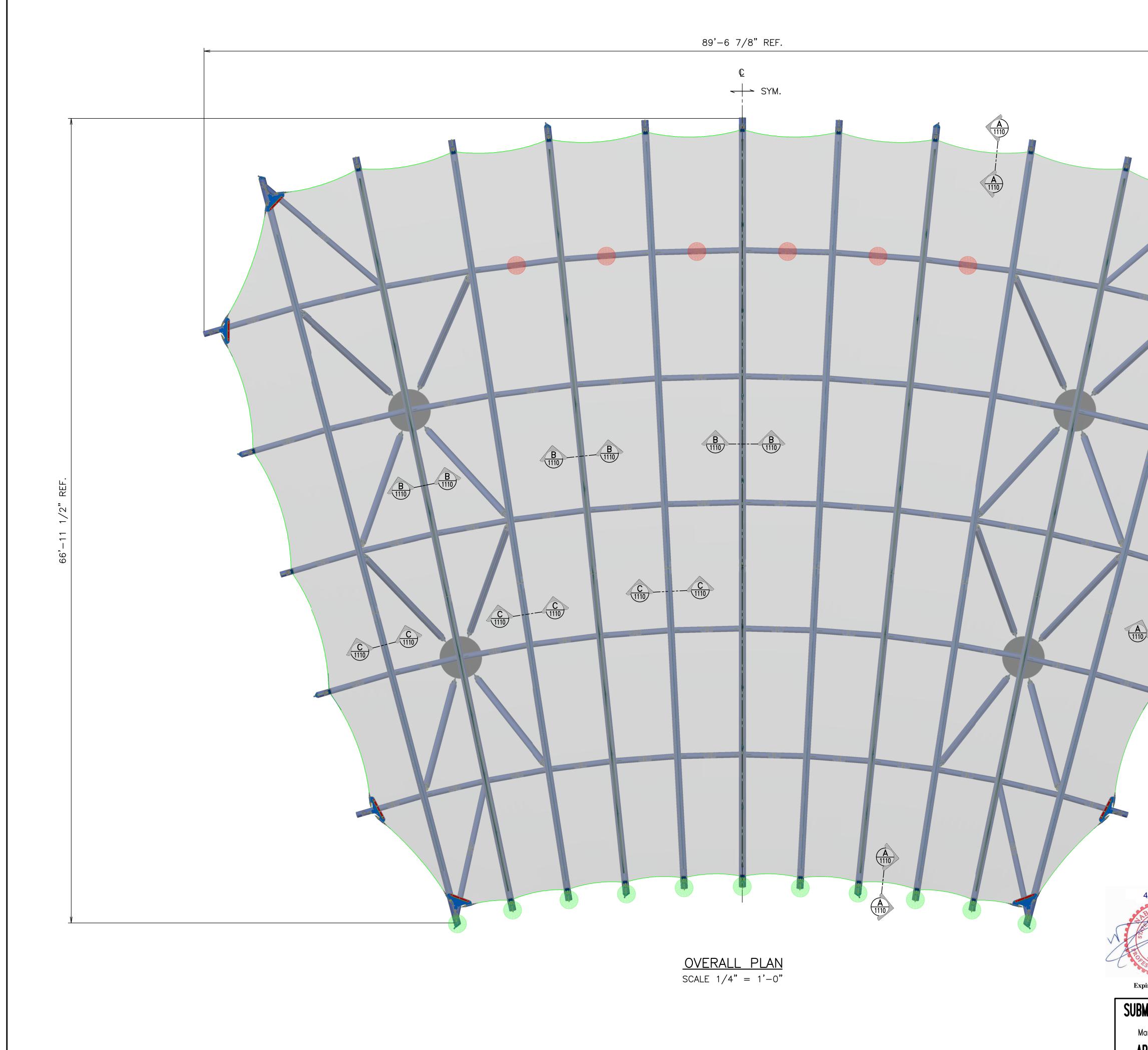
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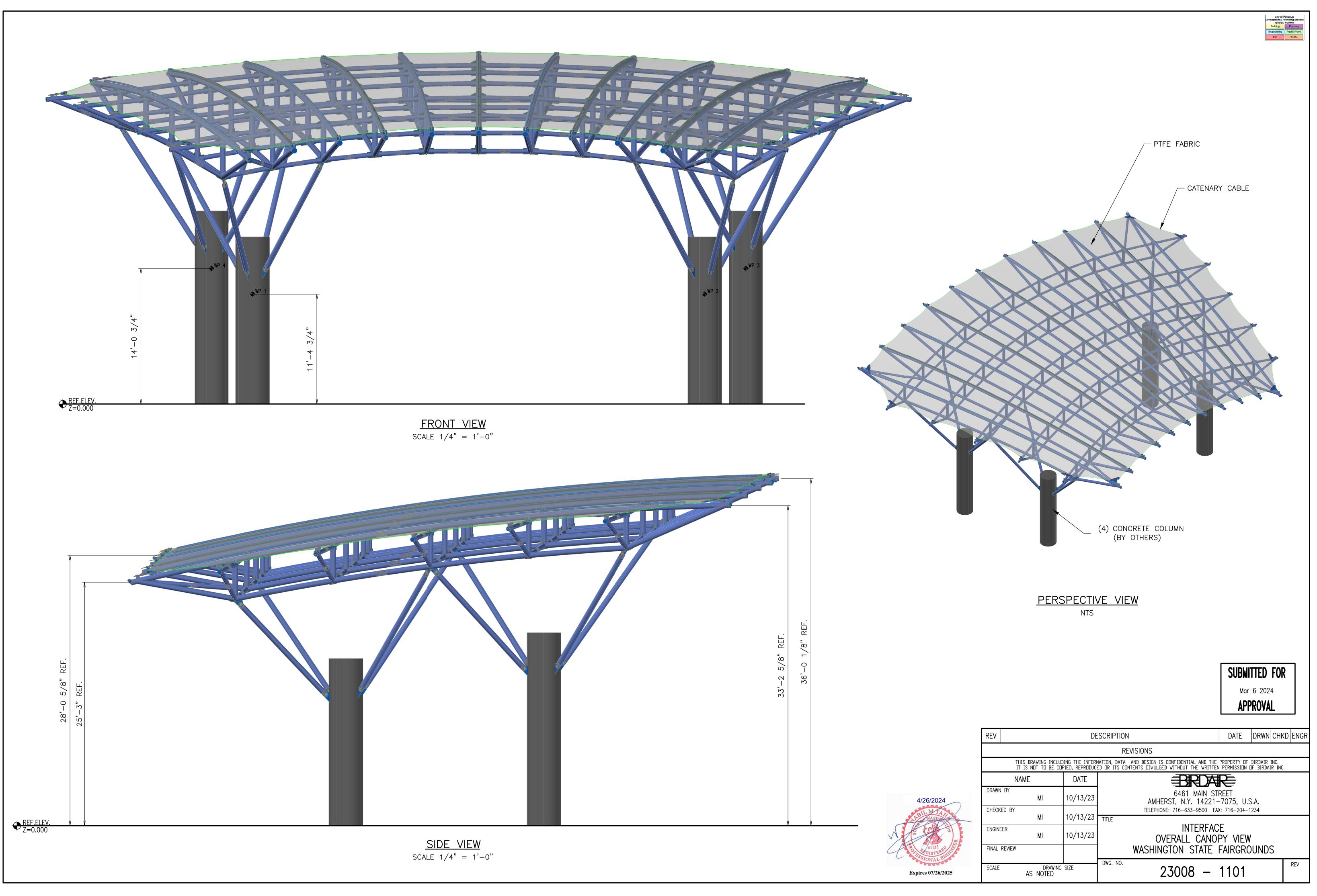
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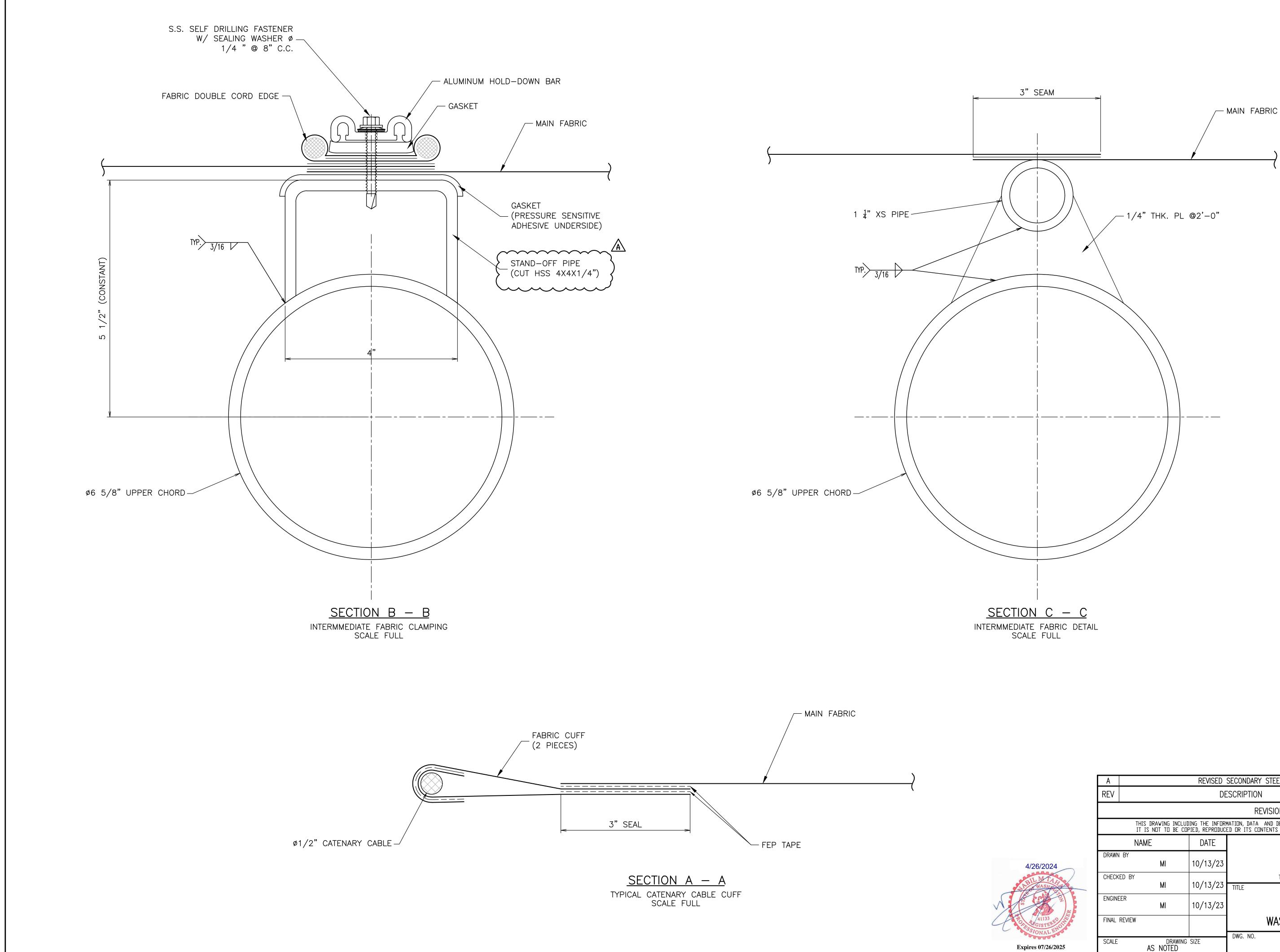
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4/26/2024         4/26/2024	S SIMILAR PLACES SEE DWG1120 TYPICAL CABLE INTERMEDIATE DETAIL <cd-2> 24 SIMILAR PLACES SEE DWG1120</cd-2>	
NZ6/2024     Image: Strain Strai	<pre></pre>	
N26/2024       DESCRIPTION       DATE       DRWN       CHKD       EN         NAME       DATE       DATE       DRWN       CHKD       EN         NAME       DATE       DATE       DRWN       CHKD       EN         DRAWN BY       MI       10/13/23       6461       MAIN STREET         DRAWN BY       MI       10/13/23       CHECKED BY       G461       MAIN STREET         MITTED FOR       MI       10/13/23       TITLE       INTERFACE         OVERALL       CANOPY PLAN       WASHINGTON STATE FAIRGROUNDS       DWS. NO.	B REVISE NOTE 2/29/24 MI	
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NAME       DATE         NAME       DATE         DRAWN BY       MI         10/13/23       6461 MAIN STREET         AMHERST, N.Y. 14221–7075, U.S.A.         TELEPHONE: 716–633–9500 FAX: 716–204–1234         ITTED FOR         r 6 2024         PROVAL	REVISIONS	
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MI       10/13/23       MHERST, N.Y. 14221-7075, U.S.A.         TITTED FOR       MI       10/13/23         ITTED FOR       MI       10/13/23         FINAL REVIEW       MI       10/13/23         FINAL REVIEW       MI       10/13/23         PROVAL       DWG, NO.       DWG, NO.	DRAWN BY	
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r 6 2024 FINAL REVIEW DWG, NO. OVERALL CANOPY PLAN WASHINGTON STATE FAIRGROUNDS	MI 10/13/23 TITLE	
PROVAL FINAL REVIEW WASHINGTON STATE FAIRGROUNDS	$M^{I} = \begin{bmatrix} 10/13/23 \end{bmatrix} \qquad OVERALL CANOPY PLAN$	
TRUYAL DWG. NO. REV	FINAL REVIEW WASHINGTON STATE FAIRGROUNDS	, )
scale drawing size AS NOTED Braining Size 23008 - 1100		REV

City of Puyallup Development & Permitting Services /ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic



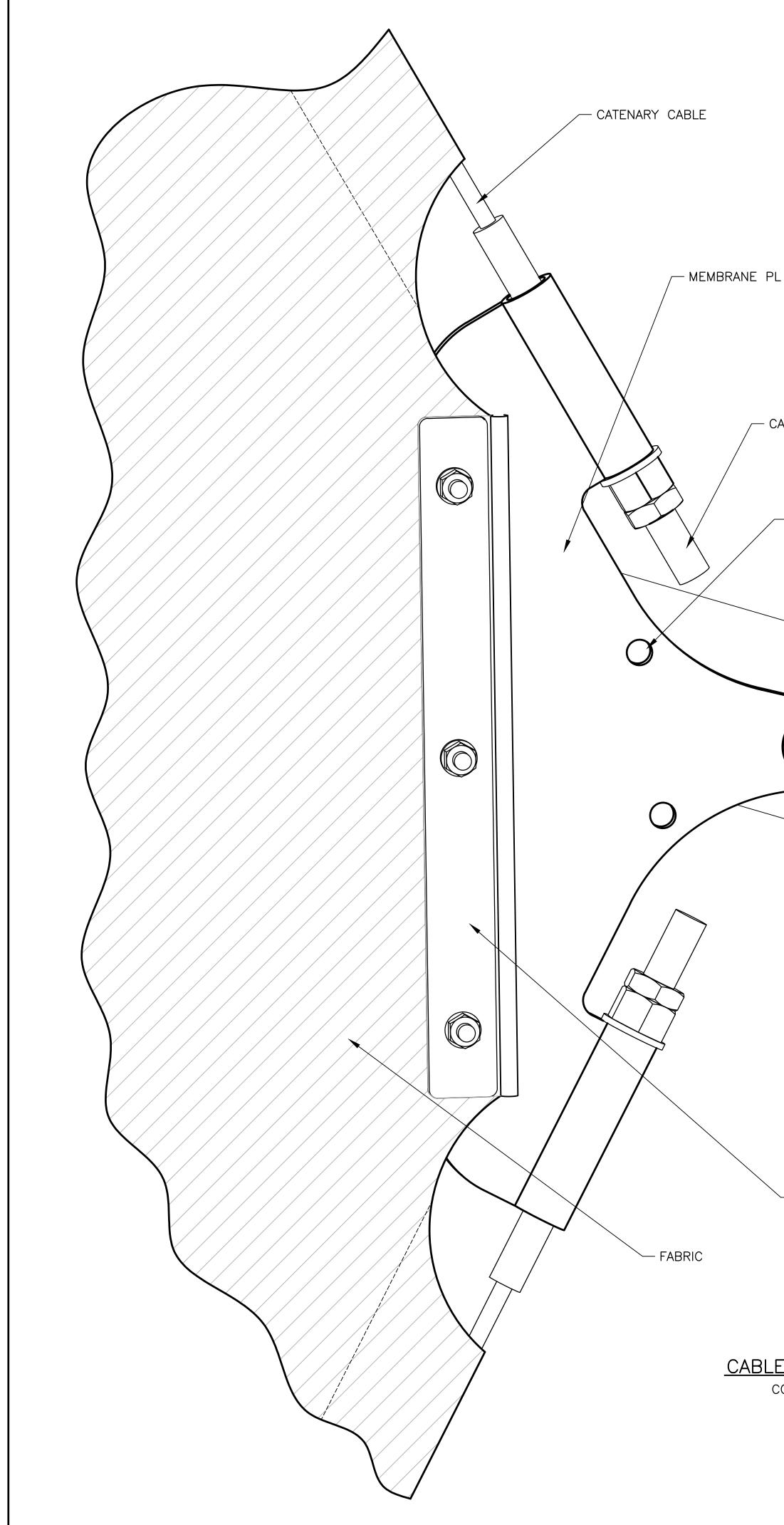


SUBMITTED FOR

City of Puyallup Development & Permitting Services /ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic

Mar 6 2024 APPROVAL

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N	IAME	DATE	BRDA	R			
DRAWN BY	MI	10/13/23	6461 MAIN STREET AMHERST, N.Y. 14221–7075, U.S.A.				
CHECKED BY	MI	10/13/23	TELEPHONE: 716-633-9500 FAX: 716-204-1234 TITLE				
ENGINEER	MI	10/13/23	INTERFACE FABRIC CLAMPING DETAIL				
FINAL REVIEW			WASHINGTON STATE FAIRGROUNDS				
SCALE DRAWING SIZE AS NOTED		ING SIZE	dwg. no. 23008 —	1110			rev A



- CABLE STUD END

- RIGGING HOLES

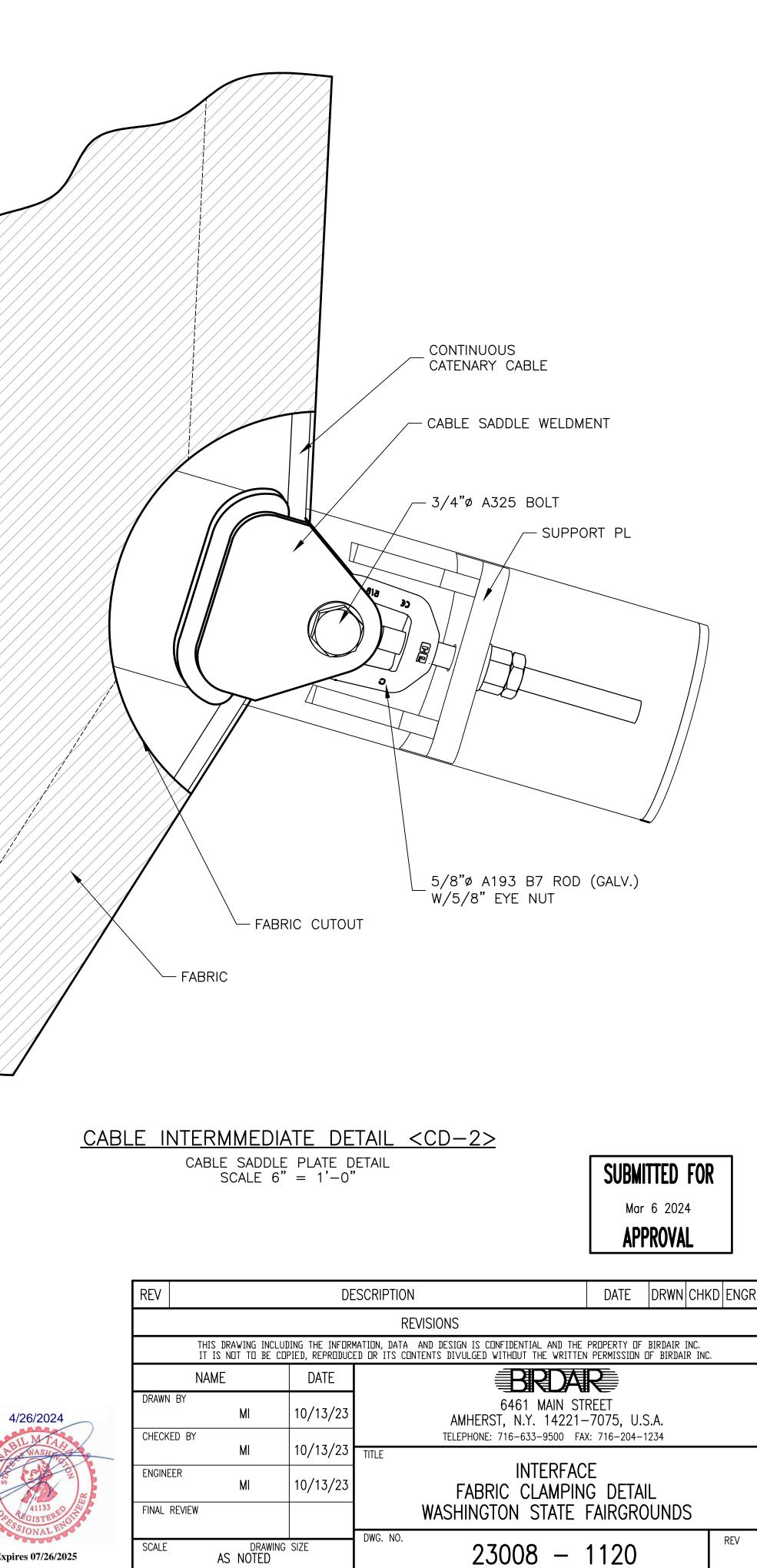
- SUPPORT PL 0 └─ 5/8"ø A193 B7 ROD (GALV.) -5/8" Shackle W/#2 CLEVIS

- TRUSS TOP CHORD

ALUMINUM CLAMP BAR W/S.S. FASTENERS

CABLE CORNER DETAIL <CD-1> CORNER MEMBRANE PLATE DETAIL SCALE 6'' = 1'-0''





Expires 07/26/2025

City of Puyallup Development & Permitting Services (ISSUED PERMIT Building Planning Engineering Public Works Fire Contemport

## STEEL SPECIFICATIONS

<u>1.0 GENERAL</u>

- 1.1 ALL WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING AMERICAN STANDARD SPECIFICATIONS AND CODES WITH MODIFICATIONS AS SPECIFIED HEREIN:
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS."
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

AMERICAN WELDING SOCIETY - "STRUCTURAL WELDING CODE" AWS D1.1 & AWS A2.4.

- AMERICAN SOCIETY FOR TESTING AND MATERIALS AS REFERENCED HEREIN.
- 1.2 IN THE EVENT OF CONFLICT BETWEEN PERTINENT CODES AND REGULATIONS AND THE REQUIREMENTS OF THE REFERENCED STANDARDS OR THESE SPECIFICATIONS. PROVISIONS OF THE MORE STRINGENT SHALL GOVERN.

2.0 MATERIALS

- 2.1 MEMBERS SHALL CONFORM TO THE FOLLOWING REQUIREMENTS UNLESS NOTED OTHERWISE.
- 2.1.1 PIPE SHALL CONFORM TO ASTM A 53, GRADE B, TYPE E OR S.
- 2.1.2 STRUCTURAL TUBING SHALL CONFORM TO ASTM A 500, GRADE B.
- 2.1.3 PLATES AND BARS SHALL CONFORM TO ASTM A 572, GRADE 50 THROUGHOUT.
- 2.1.4 PINS SHALL CONFORM TO ASTM A36, ASTM A 572 GRADE 50 TO 4 INCHES IN DIAMETER, OR ASTM A588, GRADE 50 TO 4 INCHES DIAMETER, AS NOTED ON THE DRAWINGS. HIGH STRENGTH PINS LARGER THAN 4 INCHES IN DIAMETER SHALL CONFORM TO ASTM A 572 GRADE 42. ALL PINS SHALL BE GALVANIZED UNLESS NOTED OTHERWISE.
- 2.1.5 PINS 3 INCHES (75 mm) OR LARGER IN DIAMETER SHALL CONTAIN A 1/4 INCH (6 mm) CHAMFER UNLESS NOTED OTHERWISE.
- 2.2 BOLTS AND WELDS SHALL CONFORM TO THE FOLLOWING REQUIREMENTS UNLESS NOTED OTHERWISE.
- 2.2.1 HIGH STRENGTH STEEL BOLTS SHALL CONFORM TO ASTM A 325, TYPE 1, HEAVY HEX. NUTS SUITED FOR HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM A 563, GRADE DH, HEAVY HEX. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F 436, TYPE 1, CIRCULAR.
- 2.2.2 COMMON BOLTS SHALL CONFORM TO ASTM A 307. NUTS SUITED FOR COMMON BOLTS SHALL CONFORM TO ASTM A 563, GRADE A, HEX. COMMON FLAT WASHERS SHALL CONFORM TO ASTM F 844. OR ANSI B18.22.1 (USS).
- 2.2.3 THREADS FOR BOLTS. STUDS AND NUTS SHALL CONFORM TO ANSI/ASME B1.1. RIGHT HAND. UNIFIED COARSE THREAD SERIES (UNC). AND SHALL HAVE CLASS 2A TOLERANCE UNLESS NOTED OTHERWISE.
- 2.2.4 DIMENSIONS OF FASTENERS SHALL CONFORM TO THE REQUIREMENTS OF ANSI/ASME B 18.2.1, "SQUARE AND HEX BOLTS AND SCREWS". UNLESS NOTED OTHERWISE.
- 2.2.5 WHERE NOTED, ALL HOT DIP GALVANIZED, "GALVANIZED" FASTENERS SHALL CONFORM TO ASTM A153, CLASS C.
- 2.2.6 WHERE NOTED, ALL MECHANICALLY DEPOSITED, "PLATED" FASTENERS SHALL CONFORM TO ASTM B695, CLASS 50.
- 2.2.7 ELECTRODES SHALL BE E70XX, 70 KSI YIELD STRENGTH.
- 2.3 BIRDAIR SHALL BE SUPPLIED WITH MILL CERTIFICATES INDICATING ALL MATERIALS MEET OR EXCEED THEIR SPECIFIED YIELD STRENGTHS.
- 2.4 FABRICATED PIECES DESIGNATED AS GALVANIZED SHALL BE HOT DIP GALVANIZED PER ASTM A 123. NO PAINT SHALL BE APPLIED TO THESE PIECES, UNLESS NOTED OTHERWISE.

	<u>TWO-PAR</u>
EXECUTION	1.0 SURFACE
1 INSPECTION AND QUALITY CONTROL	1.1 SURFAC
3.1.1 STEEL FABRICATOR SHALL PROVIDE EFFECTIVE, FULL TIME QUALITY CONTROL OVER ALL FABRICATION ACTIVITIES. BIRDAIR OR ITS TESTING AGENCY MAY VISIT THE PLANT AT ANY TIME TO VERIFY THAT A QUALITY CONTROL PROGRAM IS IN PLACE, AND TO SPOT CHECK WELDMENTS AND WELDING PROCEDURES. THIS INSPECTION DOES NOT RELIEVE THE STEEL FABRICATOR FROM MEETING THE QUALITY AND WORKMANSHIP REQUIREMENTS OF THIS SPECIFICATION.	SSPC-S AND WE HOURS OF THE 1.2 PROTEC TO PAIN SUBCON
3.1.2 STEEL FABRICATOR SHALL PROVIDE VISUAL INSPECTION TO ENSURE ALL WELDS CONFORM TO AWS STANDARDS. IN ADDITION, STEEL FABRICATOR SHALL PROVIDE NON-DESTRUCTIVE TESTING INCLUDING ULTRASONIC, MAGNETIC PARTICLE, DYE-PENETRANT, OR X-RAY AS NOTED ON DRAWINGS OR PROJECT SPECIFICATION.	CLEANIN 1.3 PRIMER POXIPRI SSPC F
3.1.3 STEEL FABRICATOR SHALL TEST FIT AND MATCH MARK ALL MEMBERS, WELDMENTS AND CONNECTIONS AS NOTED ON THE DRAWINGS.	1.4 THE PR MANUFA OF SSP SHALL
3.2 SHOP DRAWINGS	<u>2.0 FINISH CO</u>
3.2.1 STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS BASED UPON DESIGN DRAWINGS PROVIDED BY BIRDAIR.	2.1 FINISH COLOR TO SSP
3.2.2 SHOP DRAWINGS SHALL SHOW ALL SHOP AND ERECTION DETAILS OF THE STRUCTURAL STEEL MEMBERS AND COMPONENTS INCLUDING BUT NOT LIMITED TO: OVERALL ERECTION PLAN(S) INDICATING WHERE ALL PIECES ARE LOCATED, IDENTIFICATION MARKS, CUTS, COPES, HOLES, FASTENERS, HOLE SPACINGS, AND WELDS, BOTH SHOP AND FIELD, USING WELDING SYMBOLS AS RECOMMENDED BY AWS.	2.2 FINISH MANUFA SHALL
3.2.3 IF REQUIRED THE STEEL FABRICATOR SHALL SUBMIT A MINIMUM OF TWO (2) PRINTS OF ALL SHOP DRAWINGS TO BIRDAIR.	2.3 MINIMUN <u>3.0 COLOR</u>
3.3 WORKMANSHIP	3.1 FINISH PURCHA
3.3.1 GRIND ALL SHARP EDGES AND CORNERS.	4.0 QUALITY
3.3.2 STAMP ALL ASSEMBLIES WITH THE APPROPRIATE PART NUMBER.	4.1 DRY PA PUDDLE
3.3.3 ALL WORK SHALL BE FREE OF OIL, GREASE, AND MACHINING CHIPS. 3.3.4 WELDED CONNECTIONS	ALTHOU OPERAT THE RE REQUIRI
3.3.4.1 ALL WELDED JOINTS SHALL CONFORM TO AWS PREQUALIFIED WELDED JOINTS AS	<u>5.0 CARE AND</u>
<ul> <li>DESIGNATED BY THE STANDARD WELD SYMBOLS AND TERMS AS SHOWN ON THE DRAWINGS.</li> <li>3.3.4.2 WELDS SHALL BE MADE ONLY BY OPERATORS WHO HAVE BEEN PREVIOUSLY QUALIFIED BY TESTS, AS PRESCRIBED IN THE LATEST EDITION OF THE AMERICAN WELDING SOCIETY, D1.1, "STRUCTURAL WELDING CODE", TO PERFORM THE TYPE OF WORK REQUIRED. SUBMIT WELDERS CERTIFICATION TO BIRDAIR FOR RECORD PURPOSES.</li> <li>3.3.4.3 NO MEMBERS SHALL BE SPLICED WITHOUT PRIOR APPROVAL OF BIRDAIR.</li> </ul>	5.1 PAINTING TO ENS HANDLE NYLON <u>6.0 CERTIFICAT</u> 6.1 PAINTING
3.3.5 BOLTED CONNECTIONS	NAME, WRITTEN

3.3.5.1 ALL BOLTED JOINTS SHALL BE SHEAR/BEARING CONNECTIONS AND SHALL BE INSTALLED AND TIGHTENED TO A SNUG TIGHT CONDITION.

3.3.5.2 NO OXYGEN-CUT HOLES FOR BOLTED CONNECTIONS SHALL BE PERMITTED.



(NOTE) (1) MINIMUM YIELD STRENGTH OF STEEL TUBE IS 42KSI. (2) MINIMUM YIELD STRENGTH OF STEEL PLATE AND PIN IS 50KSI. (3) ALL FILLET WELD ARE  $\frac{5}{16}$ " UNLESS NOTED.

## ART PAINT SYSTEM SPECIFICATIONS

### PREPARATION AND PRIME COAT

ACE PREPARATION SHALL BE COMMERCIAL BLAST CLEANING -SP-6. AFTER ALL FABRICATION OPERATIONS SUCH AS MACHINING WELDING ARE COMPLETE. THERE SHALL BE A MAXIMUM OF EIGHT RS ELAPSED TIME BETWEEN SURFACE PREPARATION AND APPLICATION THE PRIME COAT.

ECT ALL DRILLED AND TAPPED HOLES AND/OR THREADED STUDS PRIOR AINTING SUCH THAT ALL BOLTED CONNECTIONS CAN BE MADE BY CONTRACTOR OR MEMBRANE STRUCTURE ERECTOR WITHOUT FIRST NING THREADS.

ER SHALL BE TNEMEC SERIES N68 (FORMERLY 68–1255) (BEIGE) PRIME II PRIMER OR APPROVED EQUAL, AND SHALL CONFORM TO PAINT SPECIFICATION NO. 22.

PRIMER SHALL BE MIXED AND APPLIED IN ACCORDANCE WITH FACTURER'S INSTRUCTIONS AND SHALL MEET THE REQUIREMENTS SPC PAINT SPECIFICATION NO. 22. THE MINIMUM DRY FILM THICKNESS BE 3-5 MILS.

### <u>COAT</u>

I COAT SHALL BE TNEMEC SERIES 1075 (FORMERLY 75) OR ENDURA-SHIELD OR APPROVED EQUAL, AND SHALL CONFORM SPC-PS GUIDE 17.00.

I COAT SHALL BE MIXED AND APPLIED IN ACCORDANCE WITH FACTURER'S INSTRUCTIONS AND MINIMUM DRY FILM THICKNESS BE 3-5 MILS.

IUM SYSTEM THICKNESS SHALL BE 8 MILS.

PAINT COLOR SHALL BE AS SPECIFIED ON THE DRAWINGS OR HASE ORDER.

PAINT SHALL BE UNIFORM AND CONTINUOUS WITH NO VOIDS OR LES AND SHALL NOT BE BROKEN BY SCRATCHES OR NICKS. DUGH BIRDAIR OR THE STEEL FABRICATOR MAY WITNESS THE PAINTING ATION, THIS DOES NOT RELIEVE THE PAINTING SUBCONTRACTOR OF RESPONSIBILITY FOR MEETING THE QUALITY AND WORKMANSHIP IREMENTS OF THIS SPECIFICATION.

### <u>ND HANDLING</u>

ING SUBCONTRACTOR SHALL MAKE EVERY REASONABLE EFFORT NSURE THE PAINTED STEEL IS THOROUGHLY DRY AND IT IS LED CAREFULLY TO PREVENT AESTHETIC OR STRUCTURAL DAMAGE. ON SLINGS SHALL BE USED WHEN HANDLING PAINTED STEEL.

### CATION

ING SUBCONTRACTOR SHALL CERTIFY THE PAINT MANUFACTURER'S . PAINT IDENTIFICATION. CONFORMANCE WITH MANUFACTURER'S TEN INSTRUCTIONS, AND THE PAINT DRY MIL THICKNESS.

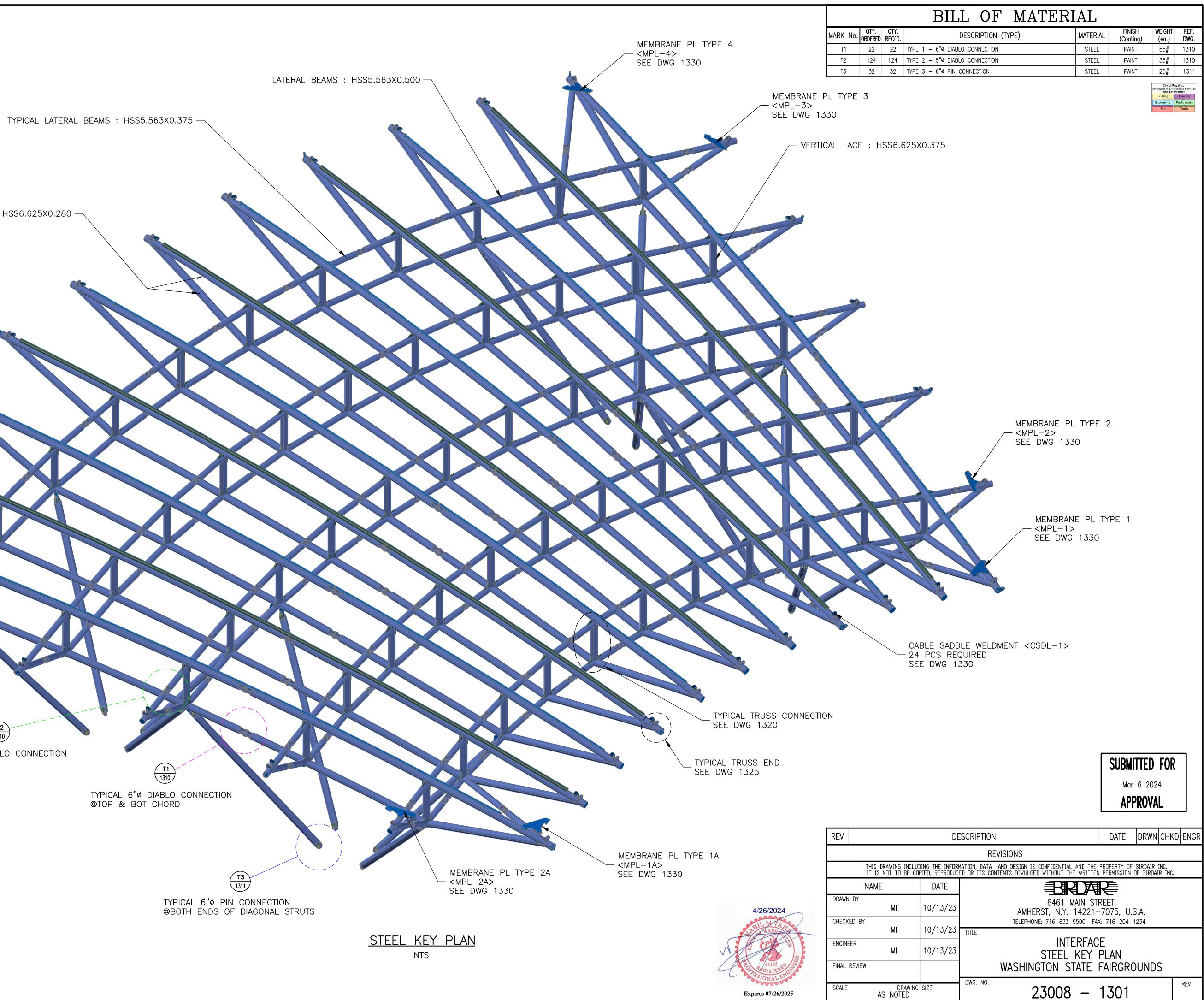
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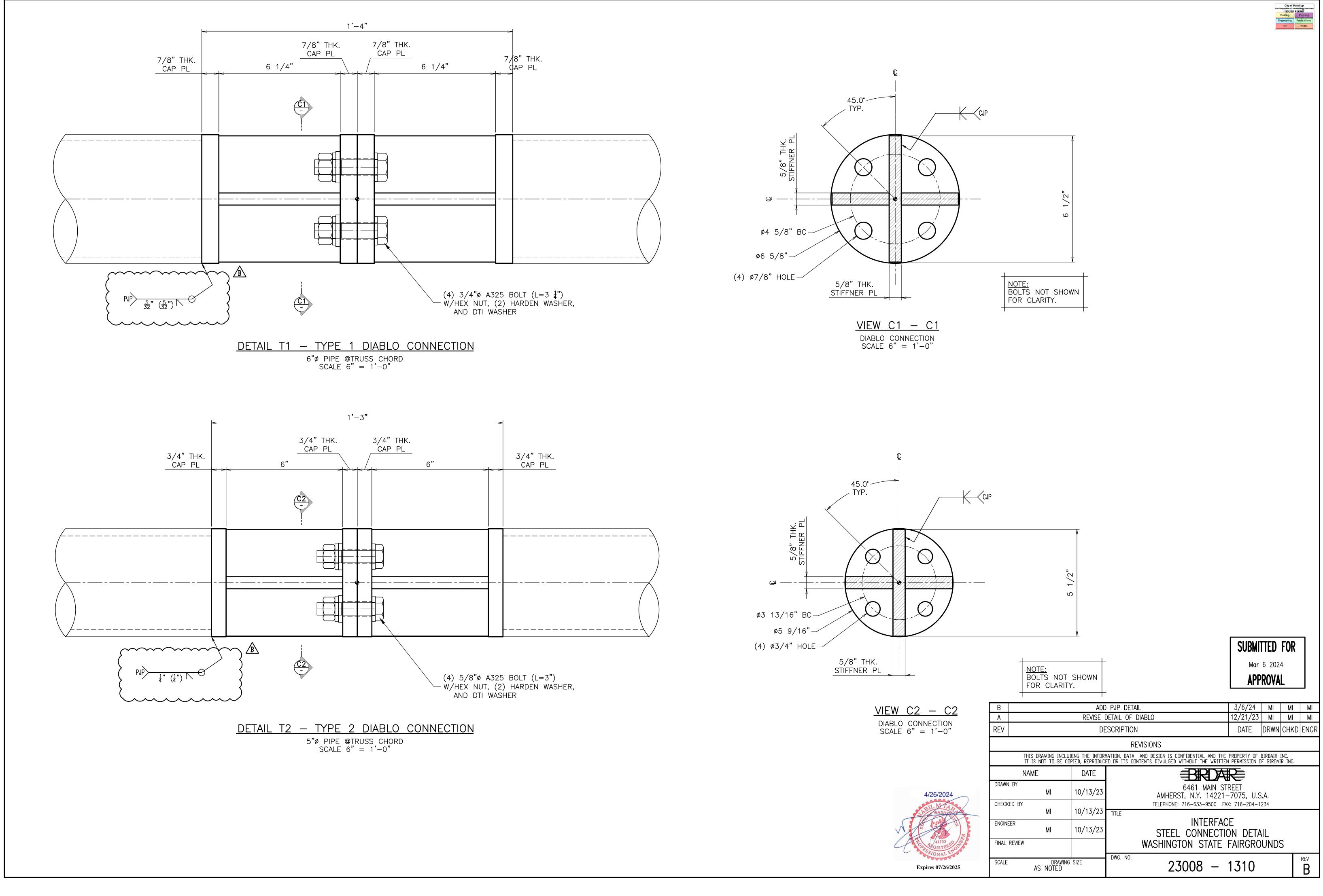
Mar 6 2024

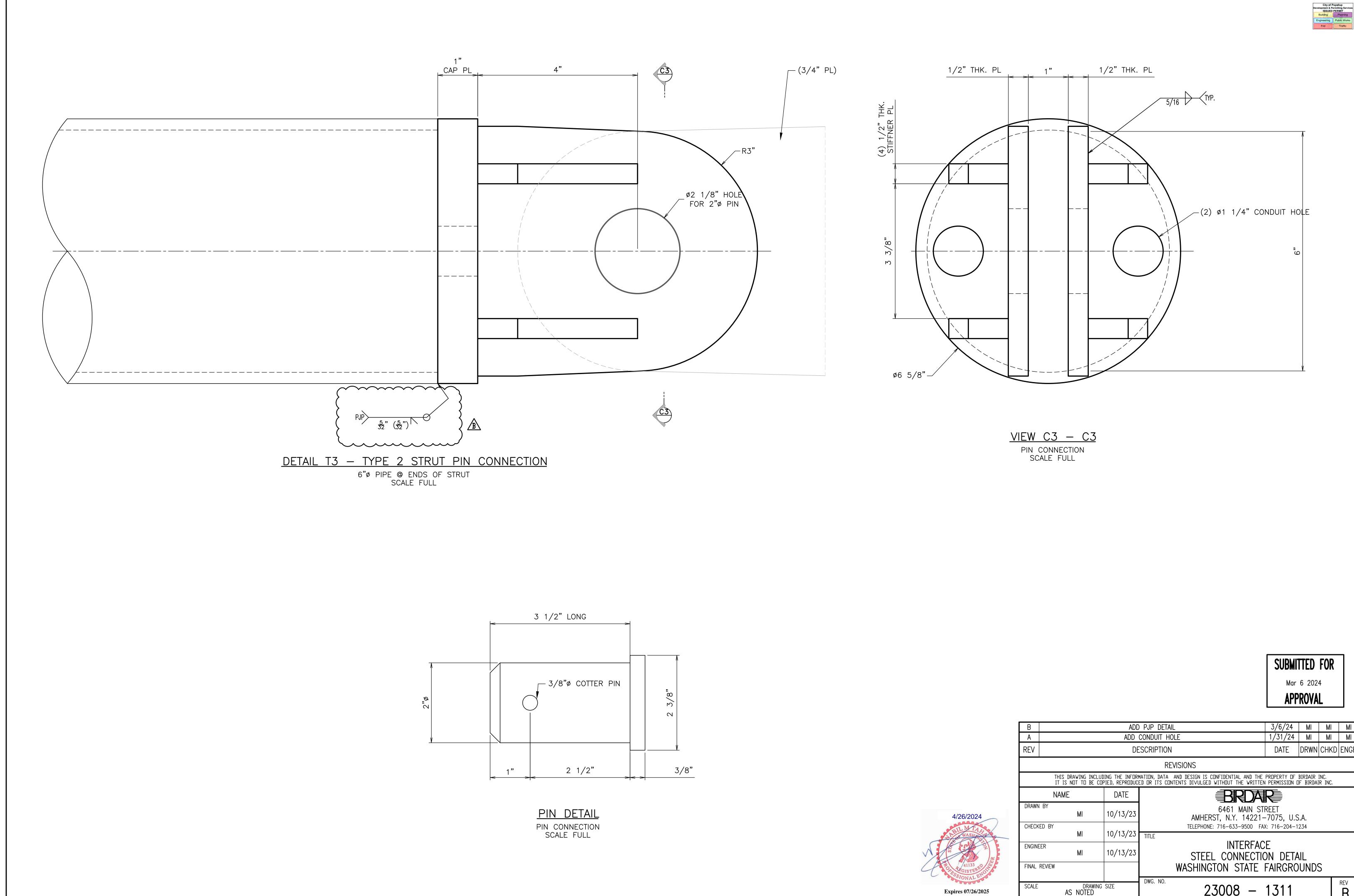
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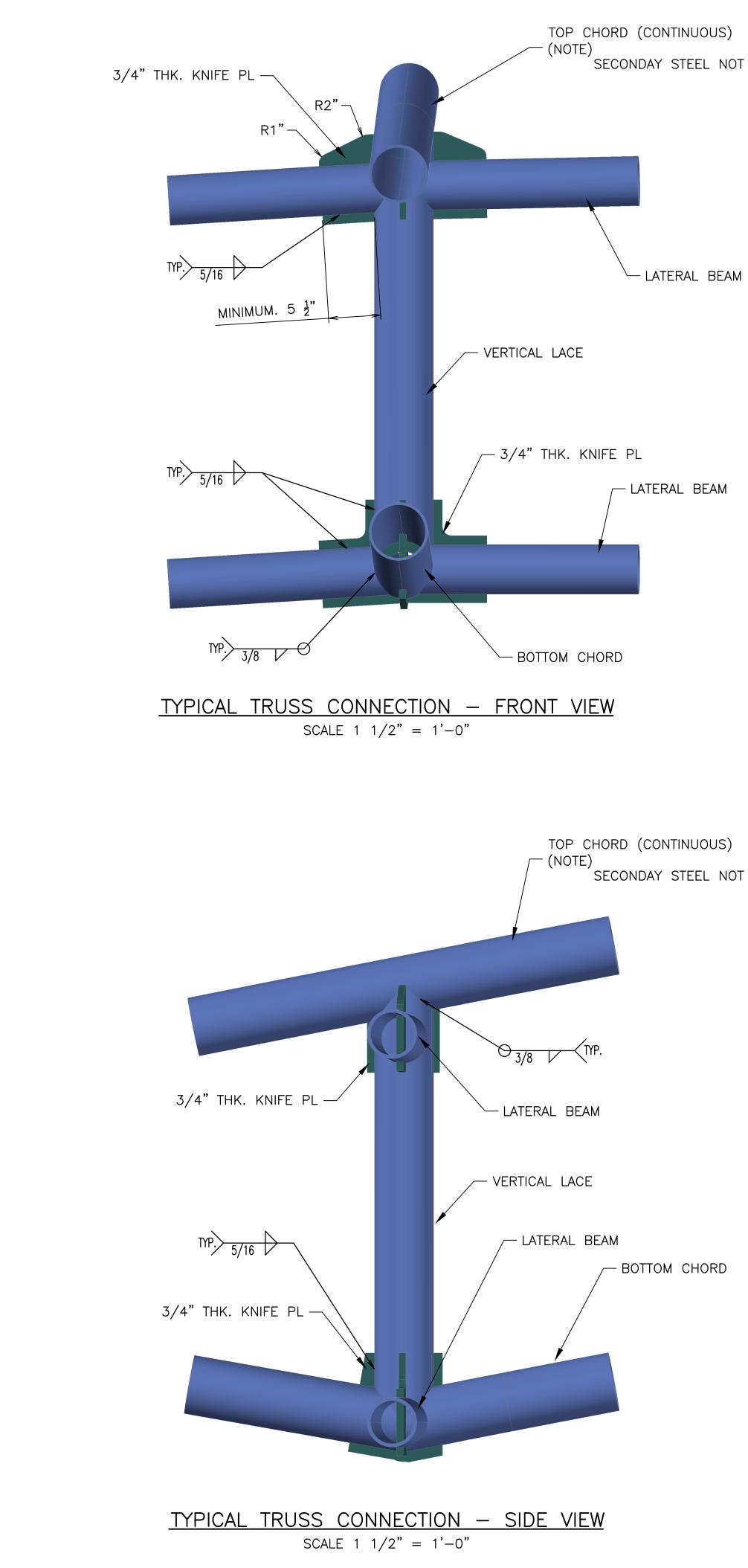
TRUSS CHORD @TOP & BOT : HSS6.625X0.280 -MEMBRANE PL TYPE 4A <MPL-4A> ----SEE DWG 1330 MEMBRANE PL TYPE 3A <MPL-3A> — SEE DWG 1330 T2 1310 TYPICAL 5"Ø DIABLO CONNECTION @LATERAL BEAM





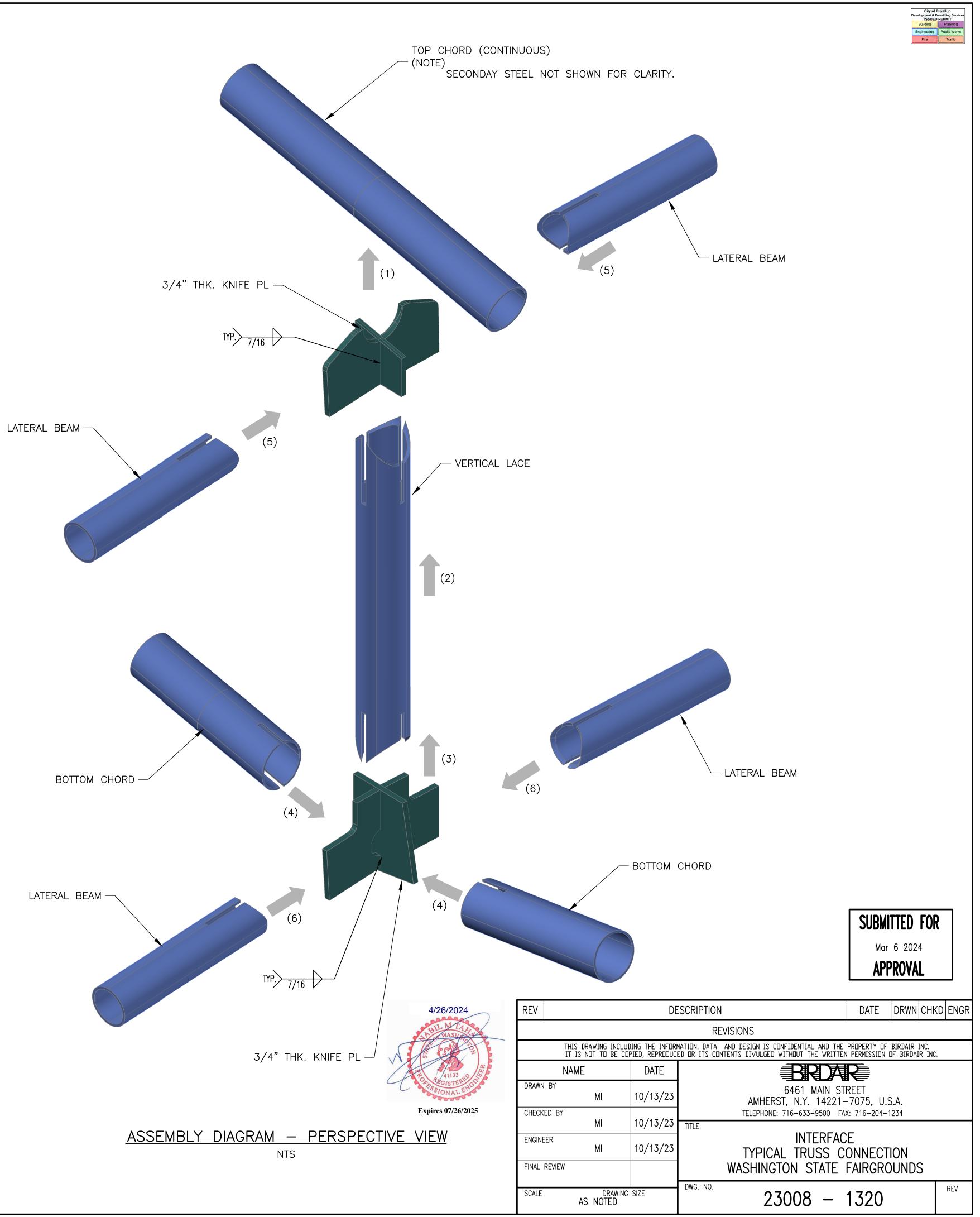
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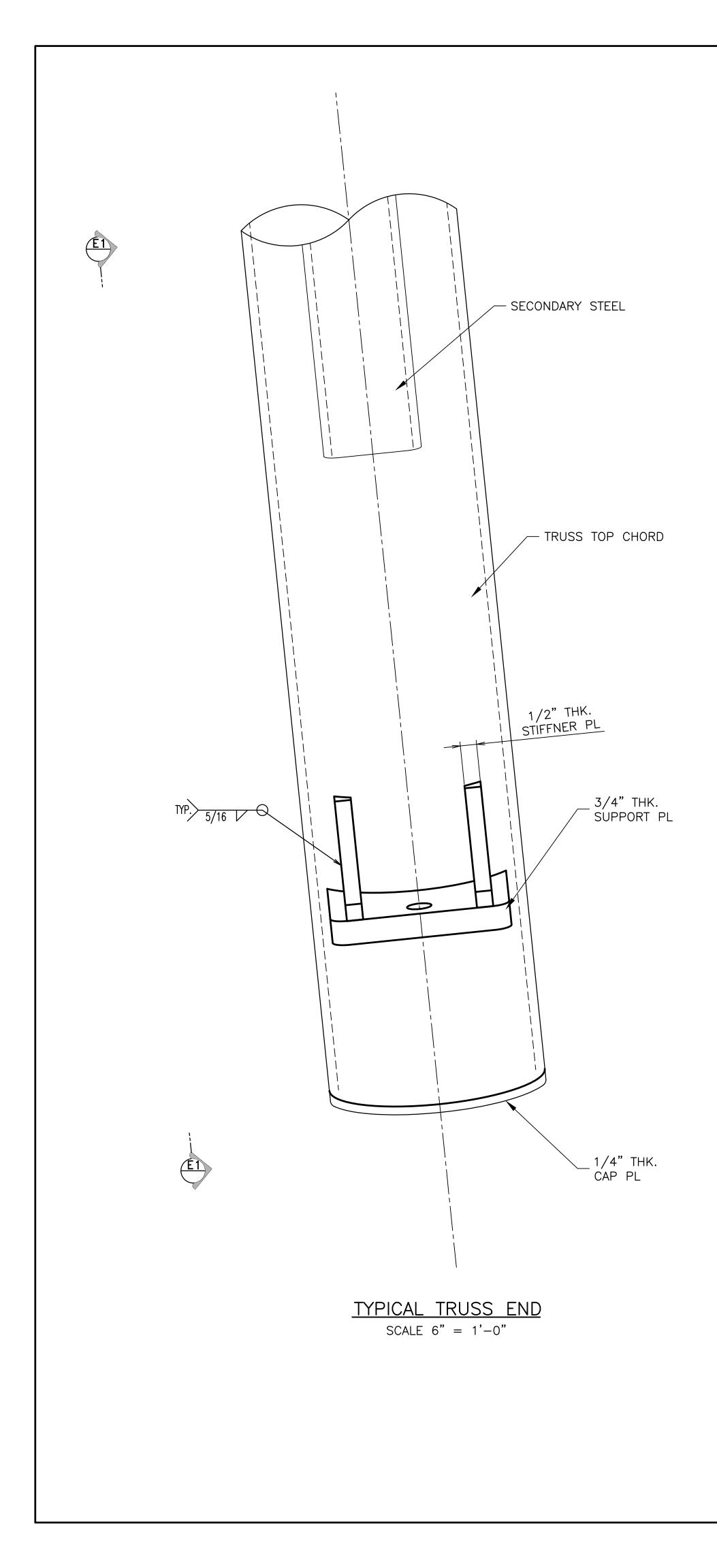
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REV		DE	SCRIPTION	DATE	DRWN	CHKD	ENGR
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DRAWN BY	МІ	10/13/23	6461 MAIN STREET AMHERST, N.Y. 14221–7075, U.S.A. TELEPHONE: 716–633–9500 FAX: 716–204–1234				
CHECKED BY	МІ	10/13/23					
ENGINEER	MI	10/13/23	INTERFACE STEEL CONNECTION DETAIL				
FINAL REVIEW			WASHINGTON STATE	FAIRGRC	UNDS	S	
SCALE	DRAWIN AS NOTED	NG SIZE	dwg. no. <b>23008</b> –	1311			rev <b>B</b>

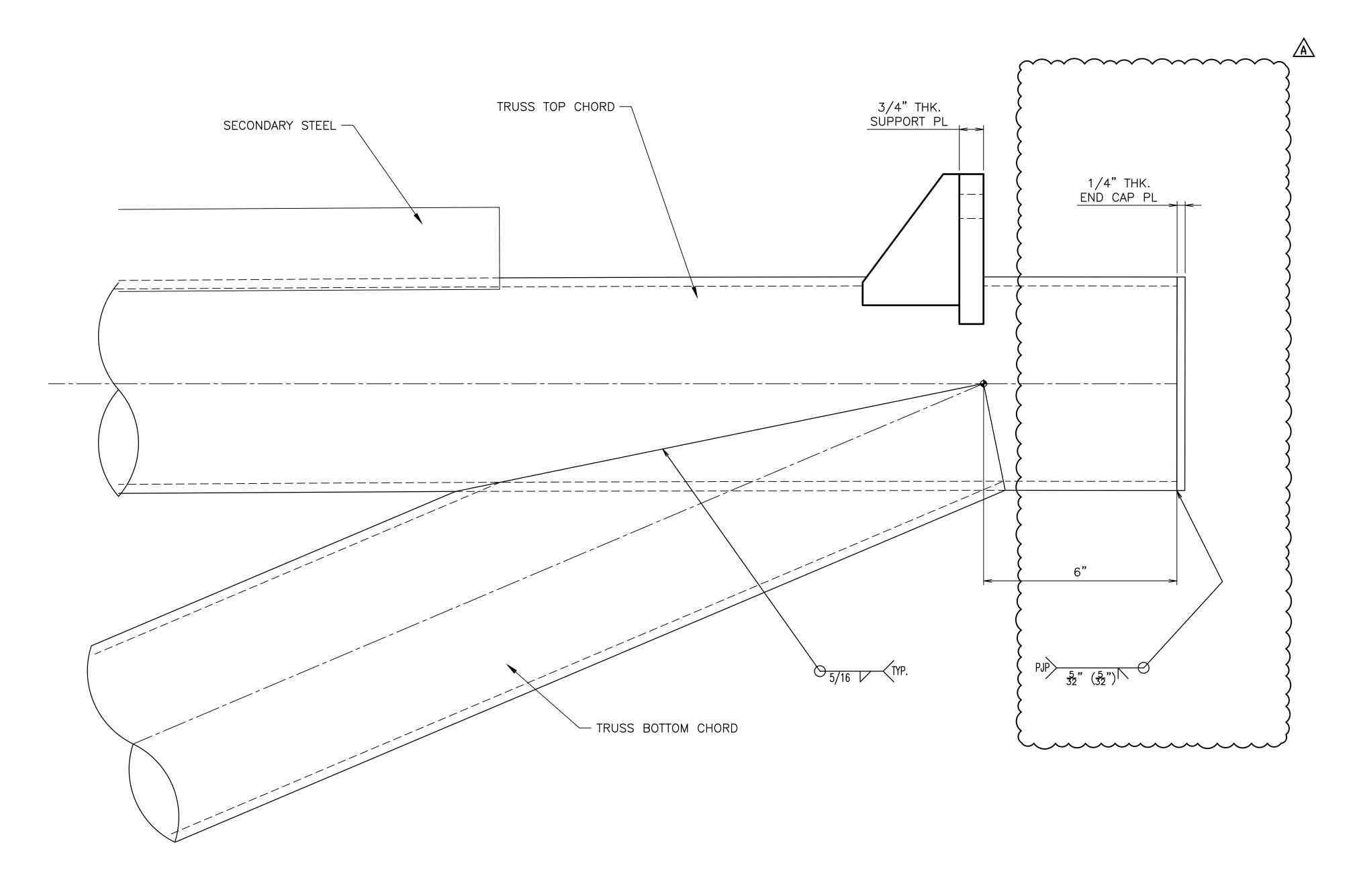


SECONDAY STEEL NOT SHOWN FOR CLARITY.

SECONDAY STEEL NOT SHOWN FOR CLARITY.





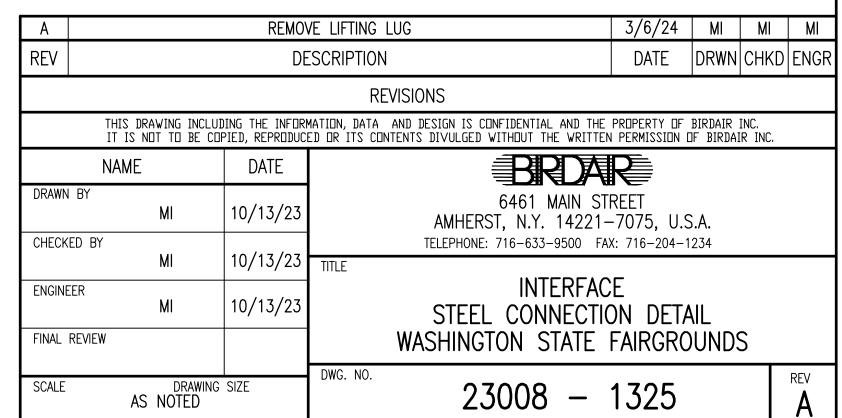


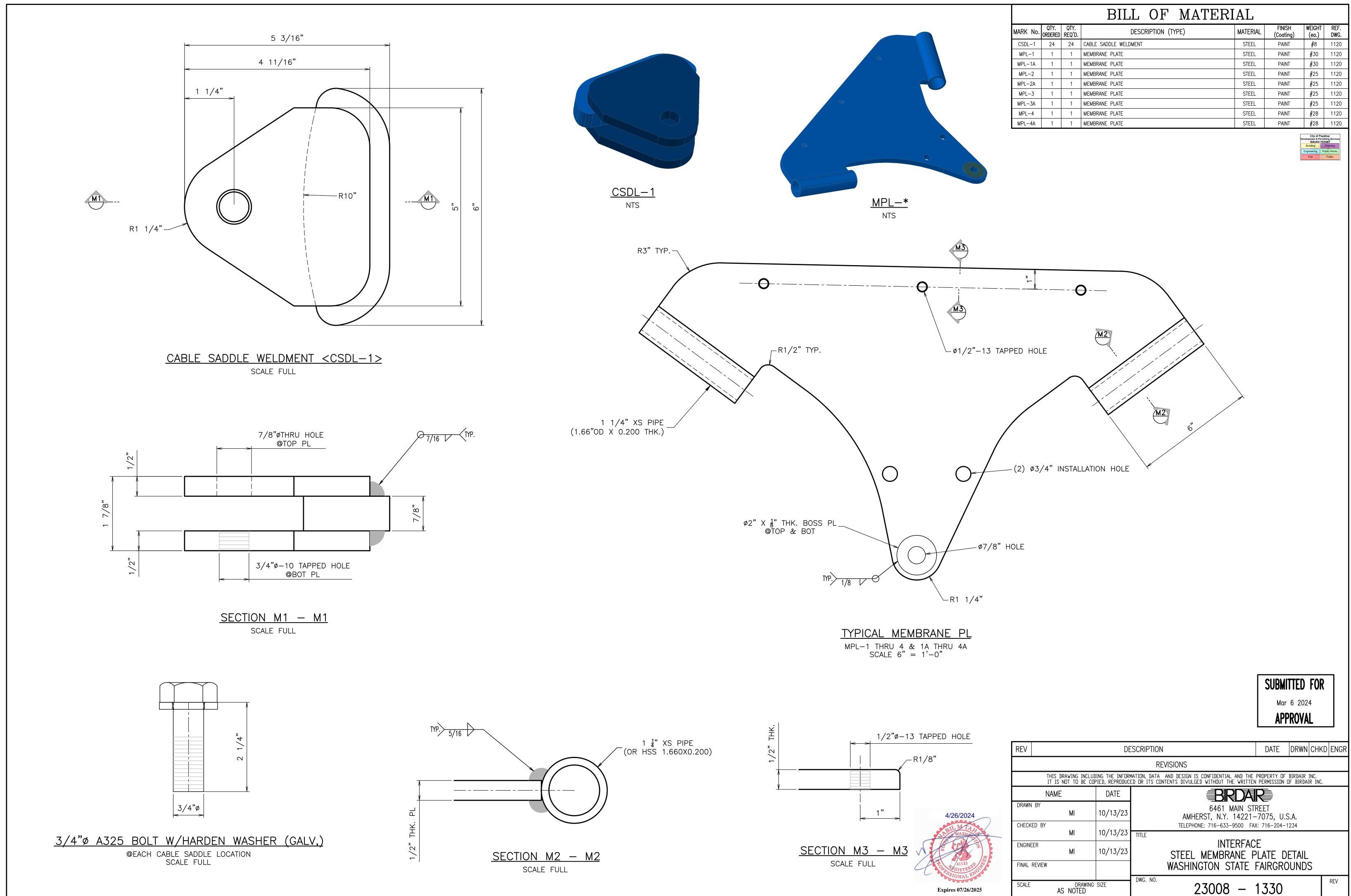
 $\frac{\text{VIEW E1} - \text{E1}}{\text{TYPICAL TRUSS END}}$  SCALE 6" = 1'-0"



SUBMITTED FOR Mar 6 2024

City of Puyallup velopment & Permitting Service ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic





## CABLE SPECS

1. GENERAL

1.1 ALL WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING AMER SPECIFICATIONS:

AMERICAN SOCIETY OF CIVIL ENGINEERS - ASCE 19, "STRUCTURAL APPLICATIO CABLES FOR BUILDINGS".

AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) - AS REFERENCED.

1.2 FULL CERTIFICATION FOR ALL CABLES IS REQUIRED, AND SHALL INCLUDE AS 1. CABLE PHYSICAL PROPERTIES, 2. MILL TEST REPORTS, 3. TEST REPORTS PRESTRETCHING WHERE NOTED, AND 4. TESTING OF FITTINGS WHERE NOTED.

### 2.0 MATERIALS

2.1 CABLE MATERIALS

- 2.1.1 STRUCTURAL WIRE ROPE CABLES SHALL CONFORM TO THE LATEST REVISION "STANDARD SPECIFICATION FOR ZINC-COATED STEEL STRUCTURAL WIRE R
- 2.1.2 STRUCTURAL STRAND CABLES SHALL CONFORM TO THE LATEST REVISION "STANDARD SPECIFICATION FOR ZINC-COATED PARALLEL AND HELICAL STE STRUCTURAL STRAND".
- 2.1.3 SEVEN WIRE PRE-STRESSING STRAND SHALL CONFORM TO THE LATEST R 416, "STANDARD SPECIFICATION FOR UNCOATED SEVEN WIRE STRESS REL PRESTRESSED CONCRETE", AND SHALL BE GRADE 270.
- 2.1.4 STRUCTURAL WIRE ROPE AND STRAND CABLES SHALL BE PVC COATED WH (WHITE IN COLOR UNLESS NOTED OR APPROVED OTHERWISE).

2.2 FITTINGS

- 2.2.1 ALL THREADED STUDS SHALL BE SUPPLIED USING MUNCY MACHINE & TO UNLESS NOTED OR APPROVED OTHERWISE. ALL THREADED STUDS SHALL THREADS AND SHALL INCLUDE A NUT, JAM NUT, AND WASHER UNLESS N WRENCH GRIPS SHALL BE PROVIDED UNLESS NOTED OTHERWISE.
- 2.2.2 ALL CABLE FITTINGS, OTHER THAN THREADED STUDS, SHALL BE CROSBY NUMBERS LISTED BELOW, UNLESS NOTED OR APPROVED OTHERWISE. TH SHALL MATCH THE CABLE DIAMETER ON EACH ASSEMBLY UNLESS NOTED

<u>FITTING</u>	<u>CROSBY_PART_NO.</u>
OPEN SWAGE SOCKET	S-501
CLOSED SWAGE SOCKET	S-502
OPEN SPELTER SOCKET	G-416
CLOSED SPELTER SOCKET	G-417
STEFL SLEEVE	S-505
EXTRA HEAVY WIRE ROPE THIMBLE	G-414

- 2.2.3 ALL OPEN SOCKETS AND CLEVISES SHALL INCLUDE A PIN AND STAINLESS PIN UNLESS NOTED OTHERWISE.
- 2.2.4 PINS 3 INCHES (75 mm) OR LARGER IN DIAMETER SHALL CONTAIN A 1/ CHAMFER UNLESS NOTED OTHERWISE.

### 3.0 EXECUTION

3.1 INSPECTION AND QUALITY CONTROL

CABLE MANUFACTURER SHALL PROVIDE EFFECTIVE QUALITY CONTROL OVER ALL ACTIVITIES. BIRDAIR OR ITS TESTING AGENCY MAY VISIT THE PLANT AT ANY TIM THAT A QUALITY CONTROL PROGRAM IS IN PLACE. THIS INSPECTION DOES NO FABRICATOR FROM MEETING THE QUALITY AND WORKMANSHIP REQUIREMENTS SPECIFICATION.

	3.2 WORKMANSHIP
RICAN STANDARD	3.2.1 CABLES
ONS OF STEEL	3.2.1.1 ALL CABLES SHALL BE THOROUGHLY COATED TO "CLASS A" ZINC COATING
	3.2.1.2 ALL CABLES SHALL BE PRESTRETCHED PER ASTM A 586 FOR STRUCTURAL AND ASTM A 603 FOR STRUCTURAL WIRE ROPE PRIOR TO FABRICATION. T CABLE TYPES SHALL HAVE THE SAME MODULUS OF ELASTICITY UNLESS NO OTHERWISE.
A MINIMUM: FOR CABLE	3.2.1.3 ALL CABLES SHALL BE MANUFACTURED TO A LENGTH TOLERANCE AS FOLLO
	LENGTH < 70 FEET TOLERANCE = 0.25 INCH (21.336 METERS) (6.35 MILLIMETERS)
ION OF ASTM A 603,	LENGTH 70 FEET TO 270 FEET TOLERANCE = 0.03% OF LEI (LENGTH 21.336 < L < 82.296 METERS)
OPE". OF ASTM A 586,	$\begin{array}{llllllllllllllllllllllllllllllllllll$
EEL WIRE	ALL LENGTHS SHALL BE MEASURED AT 70 DEGREES F.
REVISION OF ASTM A LIEVED STRAND FOR	3.2.1.4 ALL CABLES SHALL HAVE A CONTINUOUS LONGITUDINAL PAINT STRIPE, 1/4 (NOMINAL 6 MILLIMETERS) MAXIMUM ALONG THEIR TOP SURFACE, AND BLAC UNLESS NOTED OTHERWISE.
HERE NOTED	3.2.1.5 ALL MARKINGS, WHERE NOTED, SHALL BE A CIRCUMFERENTIAL PAINT STRIP WIDE (NOMINAL 6 MILLIMETERS) MAXIMUM. THE COLOR SHALL BE BLACK NOTED OTHERWISE.
DOL CO. FITTINGS	3.2.1.6 ALL CABLES SHALL BE FREE OF OIL, GREASE AND FOREIGN MATERIALS, AN CLEAN AND DRY TO THE TOUCH BY USE OF ONE OF THE FOLLOWING METI
BE RIGHT HAND NOTED OTHERWISE.	1. DRAW CABLE DRY. 2. DRAW CABLE WITH A CLEAR WATER SOLUABLE LUBRICANT. 3. ULTRASONIC CLEANING.
USING PART IE FITTING SIZES	3.2.1.7 TOUCH UP ALL DAMAGE TO ZINC COATING WITH GRAY ZINC-RICH PAINT.
OTHERWISE.	3.2.2 FITTINGS
	3.2.2.1 ALL END FITTINGS SHALL BE DESIGNED TO DEVELOP 110 PERCENT OF THE BREAKING STRENGTH OF EACH CABLE, AND AS ATTACHED AND INSTALLED, DEVELOP 100 PERCENT OF THE NOMINAL BREAKING STRENGTH OF EACH C AND PIN ASSEMBLIES SHALL BE DESIGNED TO DEVELOP THE AVAILABLE STF LISTED IN THE AISC STEEL CONSTRUCTION MANUAL, 14th EDITION, TABLE 1
S STEEL COTTER	3.2.2.2 SWAGED END FITTINGS, CLEVISES, PINS, NUTS AND WASHERS SHALL BE HC GALVANIZED PER ASTM A 153. ANY DAMAGE TO ZINC COATING SHALL BE PER SSPC—SP3 AND PAINTED WITH GRAY ZINC—RICH PAINT PER ASTM A 7
/4" (6 mm)	3.2.2.3 SPELTERED END FITTINGS SHALL BE HOT DIP GALVANIZED PER ASTM A 15 DAMAGE TO ZINC COATING SHALL BE CLEANED PER SSPC—SP3 AND PAINTE ZINC—RICH PAINT PER ASTM A 780.
	3.2.2.4 MARK ALL END FITTINGS WITH THE MARK NUMBER AND "X" OR "Y" END DI MARKING SHALL BE OF A PERMANENT NATURE.
L FABRICATION ME TO VERIFY	3.2.2.5 ATTACH A METAL TAG INDICATING MARK NUMBER, DIAMETER, AND LENGTH F CABLE ASSEMBLY.
T RELIEVE THE DF THIS	3.2.2.6 "FABRICATION LOAD" IS A LOAD WHICH "FABRICATION LENGTH" SHOWN IS TO MEASURED FOR CABLE ASSEMBLY.

3.2.2.7 "DESIGN LOAD", WHERE NOTED, IS A LOAD IN EACH CABLE UNDER DESIGN PRESTRESS CONDITION. THESE VALUES SHALL NOT BE USED FOR CABLE FABRICATION.



THROUGHOUT.

STRAND, THE SAME TED

OWS:

NGTH

INCH WIDE CK IN COLOR

1/4 INCH UNLESS

ND SHALL BE HODS:

NOMINAL SHALL CABLE. CLEVIS RENGTH AS 5-4. OT DIP CLEANED '80.

3. ANY ED WITH GRAY

ESIGNATION.

FOR EACH

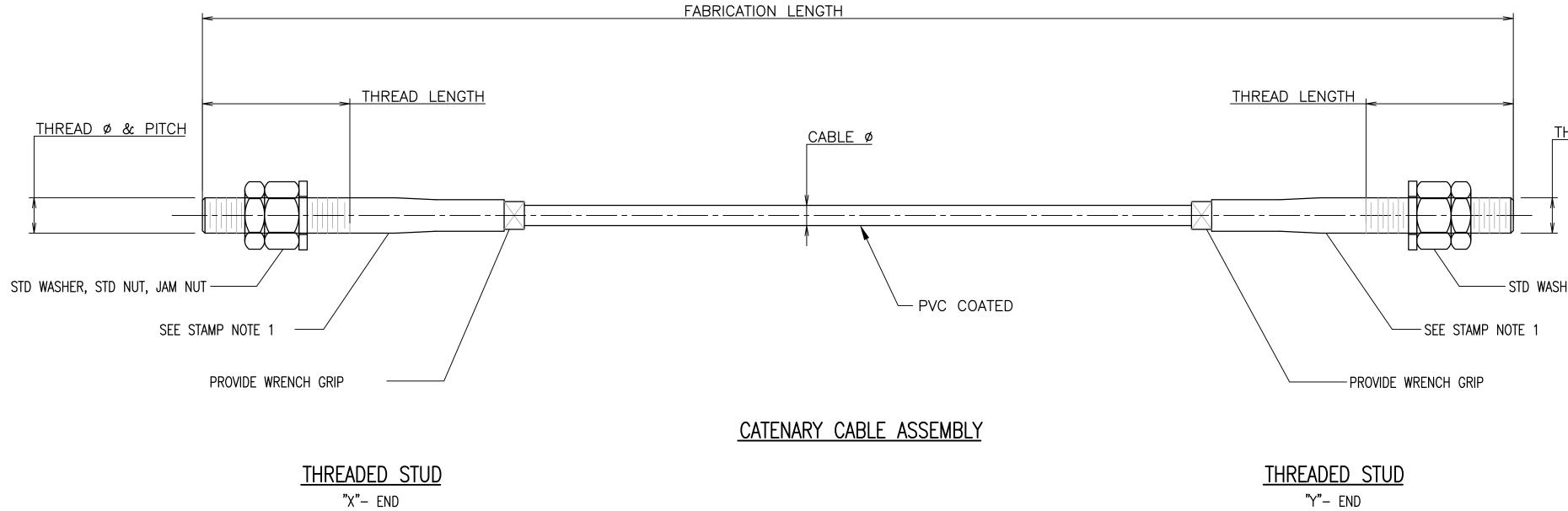
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# SUBMITTED FOR

City of Puyallup Development & Permitting Service ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic

Mar 6 2024 APPROVAL

DESCRIPTION DATE DRWNCHKDENG REV REVISIONS THIS DRAWING INCLUDING THE INFORMATION, DATA AND DESIGN IS CONFIDENTIAL AND THE PROPERTY OF BIRDAIR INC. IT IS NOT TO BE COPIED, REPRODUCED OR ITS CONTENTS DIVULGED WITHOUT THE WRITTEN PERMISSION OF BIRDAIR INC. BRDAR NAME DATE DRAWN BY 6461 MAIN STREET 10/13/23 MI 4/26/2024 AMHERST, N.Y. 14221-7075, U.S.A. TELEPHONE: 716-633-9500 FAX: 716-204-1234 CHECKED BY 10/13/23 TITLE MI INTERFACE ENGINEER 10/13/23 MI CABLE SPECIFICATION WASHINGTON STATE FAIRGROUNDS FINAL REVIEW DWG. NO. REV DRAWING SIZE SCALE 23008 - 1400 Expires 07/26/2025



"X"- END

## STAMP NOTES:

1. STAMP FITTINGS WITH MARK NO.

## PROVIDE EACH STUD END WITH:

- 1 STANDARD HEX NUT 1 JAM NUT
- 1 STANDARD HARDEN WASHER

С	ABLE	CHAR	Т	THREADED STUD			
MARK NO.	CABLE Ø	FABRICATION LENGTH (FT.)	FABRICATION LOAD (KIPS)	MANUFAC. PART NO.	THREAD Ø & PITCH (IN.)	THREAD LENGTH (IN.)	
C1	1/2"	79.979	3.5	TTS-16A	1-8	6	
C2	1/2"	11.163	3.5	TTS-16A	1-8	6	
C3	1/2"	41.978	3.5	TTS-16A	1-8	6	
C4	1/2"	9.650	3.5	TTS-16A	1-8	6	
C5	1/2"	47.608	3.5	TTS-16A	1–8	6	

(NOTE) FABRICATION LENGTH IS PRELIMINARY. FINAL LENGTH TO BE PROVIDED AFTER PATTERNING.



				BILL OF MATERI	[AL			
MARK	No.	QTY. Ordered	QTY. REQ'D.	DESCRIPTION (TYPE)	MATERIAL	FINISH (Coating)	WEIGHT (ea.)	REF. DWG.
C1		1	1	1/2"ø WIRE ROPE CABLE W/STUD ENS	A603	PVC		
C2		2	2	1/2"ø WIRE ROPE CABLE W/STUD ENS	A603	PVC		
C3		2	2	1/2"ø WIRE ROPE CABLE W/STUD ENS	A603	PVC		
C4		2	2	1/2"ø WIRE ROPE CABLE W/STUD ENS	A603	PVC		
C5		1	1	1/2"ø WIRE ROPE CABLE W/STUD ENS	A603	PVC		
C5		1	1	1/2"ø WIRE ROPE CABLE W/STUD ENS	A603	PVC	City of P	

City of Puyallup Development & Permitting Services /ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic

THREAD Ø & PITCH

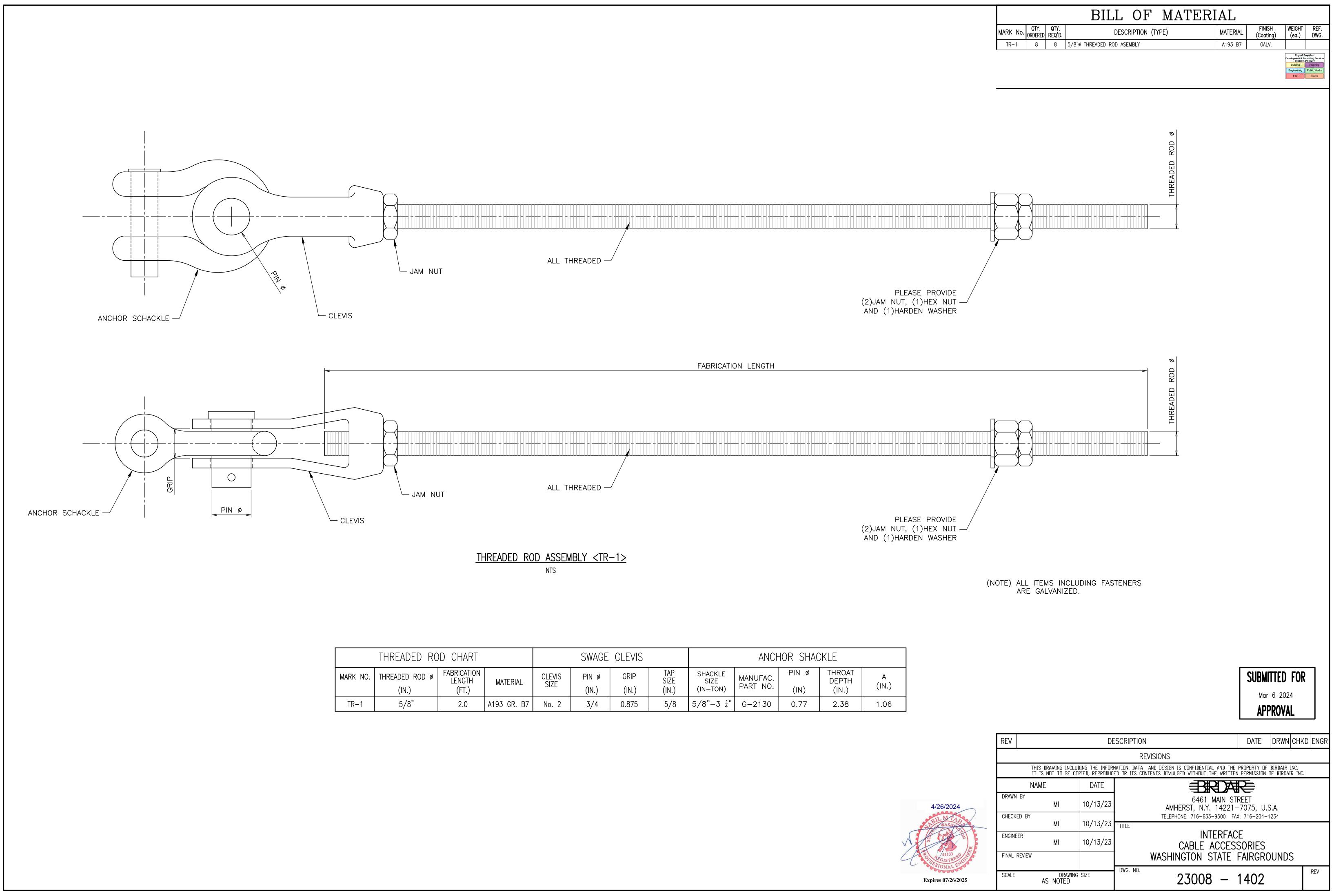
NOTES:

- 1. SEE DRAWING 4000 FOR CABLE SPECIFICATIONS.
- 2. ALL CABLES SUPPLIED COMPLETE WITH FITTINGS, NUTS, JAM NUTS, & WASHERS.
- 3. ALL END FITTINGS & HARDWARE TO BE HOT-DIP GALVANIZED.
- 4. ALL CABLES ON THIS DRAWING SHALL BE PRESTRETCHED, SEE 3.2.1.2 (DWG. 4000).

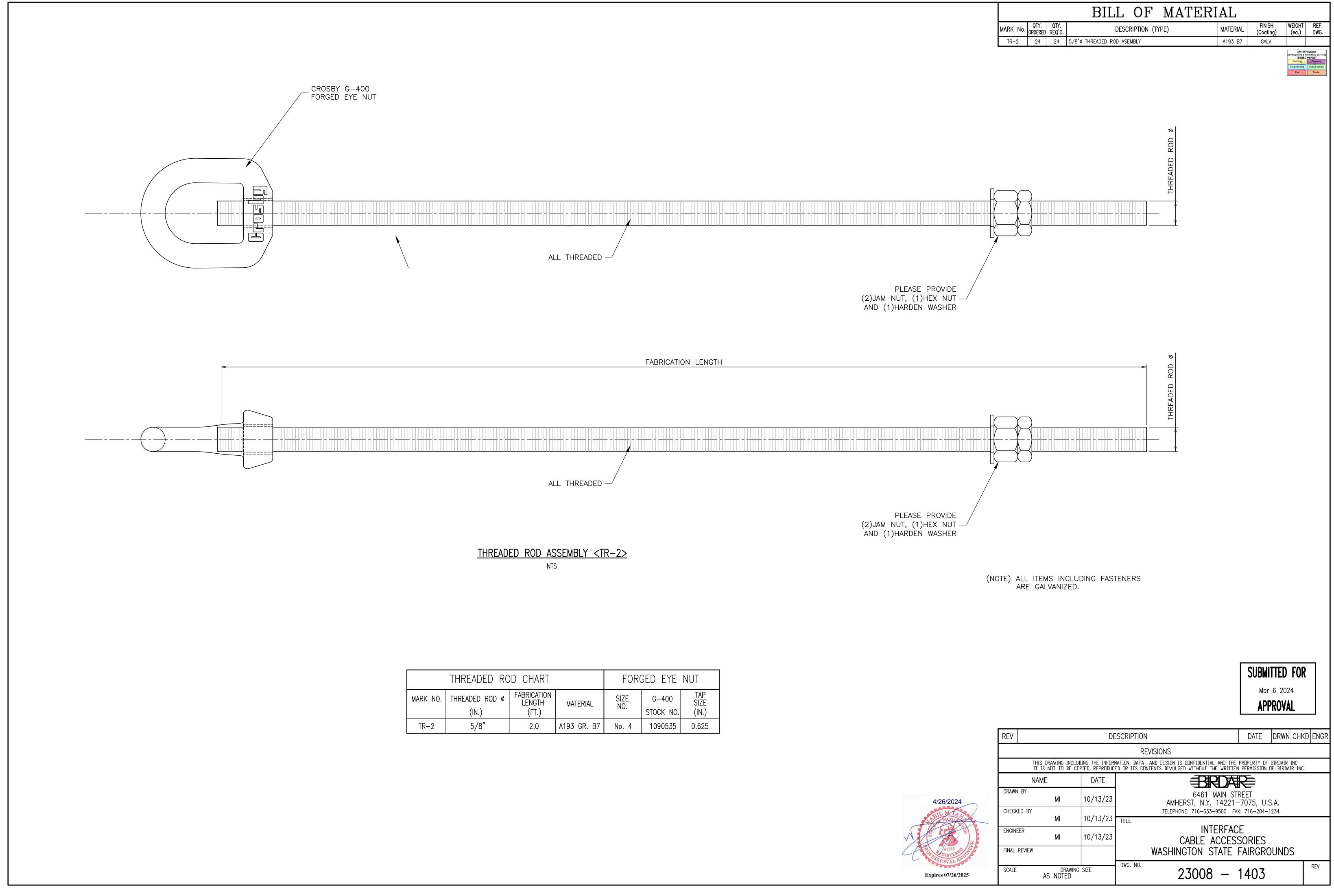
SUBMITTED FOR

Mar 6 2024

	REV		DE	DESCRIPTION DATE DRWN CHKD EN						
		REVISIONS								
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	NAME DATE				BRDA	R				
	DRAWN BY	MI	10/13/23	TELEPHONE: 716–633–9500 FAX: 716–204–1234						
1	CHECKED BY	МІ	10/13/23							
	ENGINEER	MI	10/13/23	INTERFACE						
	FINAL REVIEW				WASHINGTON STATE	FAIRGRC	UND	S		
	SCALE	DRAWING AS NOTED	I SIZE	DWG. NO.	23008 -	1401			REV	



RC	)D CHART			SWAGE	CLEVIS			ANCH	IOR SHAC	CKLE	
ø	FABRICATION LENGTH (FT.)	MATERIAL	CLEVIS SIZE	PIN Ø (IN.)	GRIP (IN.)	TAP SIZE (IN.)	SHACKLE SIZE (IN–TON)	MANUFAC. PART NO.	PIN Ø (IN)	THROAT DEPTH (IN.)	A (IN.)
	2.0	A193 GR. B7	No. 2	3/4	0.875	5/8	5/8"-3 ‡"	G-2130	0.77	2.38	1.06



	THREADED RC	D CHART	FORGED EYE NUT			
0.	THREADED ROD Ø (IN.)	FABRICATION LENGTH (FT.)	MATERIAL	SIZE NO.	G-400 STOCK NO.	TAP SIZE (IN.)
	5/8"	2.0	A193 GR. B7	No. 4	1090535	0.625

## ALUMINUM SPECIFICATIONS

<u>1.0 GENERAL</u>

- 1.1 ALL WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING AMERICAN STANDARD SPECIFICATIONS AND CODES WITH MODIFICATIONS AS SPECIFIED HEREIN:
  - ALUMINUM ASSOCIATION, INC. "SPECIFICATIONS FOR ALUMINUM STRUCTURES".

AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE" – ALUMINUM, AWS D1.2.

## 2.0 MATERIALS

- 2.1 ALL EXTRUDED STRUCTURAL ALUMINUM SECTIONS SHALL CONFORM TO ALLOY 6061-T6 WITH A MINIMUM TENSILE YIELD STRENGTH OF 35 KSI AND MINIMUM SHEAR YIELD STRENGTH OF 20 KSI, UNLESS NOTED OTHERWISE.
- 2.2 BENT PLATES SHALL BE FORMED FROM ALLOY 6061, THEN HEAT TREATED TO T6 REQUIREMENTS, UNLESS NOTED OTHERWISE.
- 2.3 SHEET ALUMINUM SHALL BE ALLOY 5052-H32 UNLESS NOTED OTHERWISE.
- 2.4 NON-STRUCTURAL ALUMINUM SHEET SHALL CONFORM TO 1100 SERIES ALLOY UNLESS NOTED OTHERWISE.
- 2.5 BIRDAIR SHALL BE SUPPLIED WITH CERTIFICATION OF ALLOY TYPE AND HEAT TREATMENT.
- 3.0 EXECUTION
- 3.1 INSPECTION AND QUALITY CONTROL

THE ALUMINUM FABRICATOR SHALL PROVIDE EFFECTIVE QUALITY CONTROL OVER ALL FABRICATION ACTIVITIES. BIRDAIR OR ITS TESTING AGENCY MAY VISIT THE PLANT AT ANY TIME TO VERIFY THAT A QUALITY CONTROL PROGRAM IS IN PLACE. THIS INSPECTION DOES NOT RELIEVE THE FABRICATOR FROM MEETING THE QUALITY AND WORKMANSHIP REQUIREMENTS OF THIS SPECIFICATION.

- 3.2 WORKMANSHIP
- 3.2.1 ALL STRUCTURAL ALUMINUM SHALL BE COATED USING THE FOLLOWING, AS NOTED:

"ANODIZED" AFTER MACHINING AND FABRICATION PER MIL-A-8625C TYPE 2, CLASS 1.

- 3.2.2 BREAK ALL EDGES AND REMOVE ALL BURRS UNLESS NOTED OTHERWISE. MINIMUM RADIUS OF BROKEN EDGES SHALL BE 1/16".
- 3.2.3 STAMP ALL PARTS WITH APPROPRIATE PART NUMBER.
- 3.2.4 ALL WORK SHALL BE FREE OF OIL, GREASE, AND MACHINING CHIPS.
- 3.2.5 TOLERANCES SHALL BE AS FOLLOWS:

CROSS SECTION DIMENSIONS SHALL HAVE A TOLERANCE OF PLUS OR MINUS 10% WITH A MAXIMUM OF 0.03 INCHES (0.8 MILLIMETERS) FROM THEORETICAL, UNLESS NOTED OTHERWISE.

ALL DIMENSIONS FOR LOCATING BOLT HOLES SHALL HAVE A TOLERANCE OF PLUS OR MINUS 0.03 INCH (0.8 MILLIMETERS) FROM THEORETICAL, UNLESS NOTED OTHERWISE.

OVERALL DIMENSIONS FOR PIECES SHALL HAVE A TOLERANCE OF PLUS OR MINUS 1/16 INCH (1.6 MILLIMETERS) FROM THEORETICAL, UNLESS NOTED OTHERWISE.

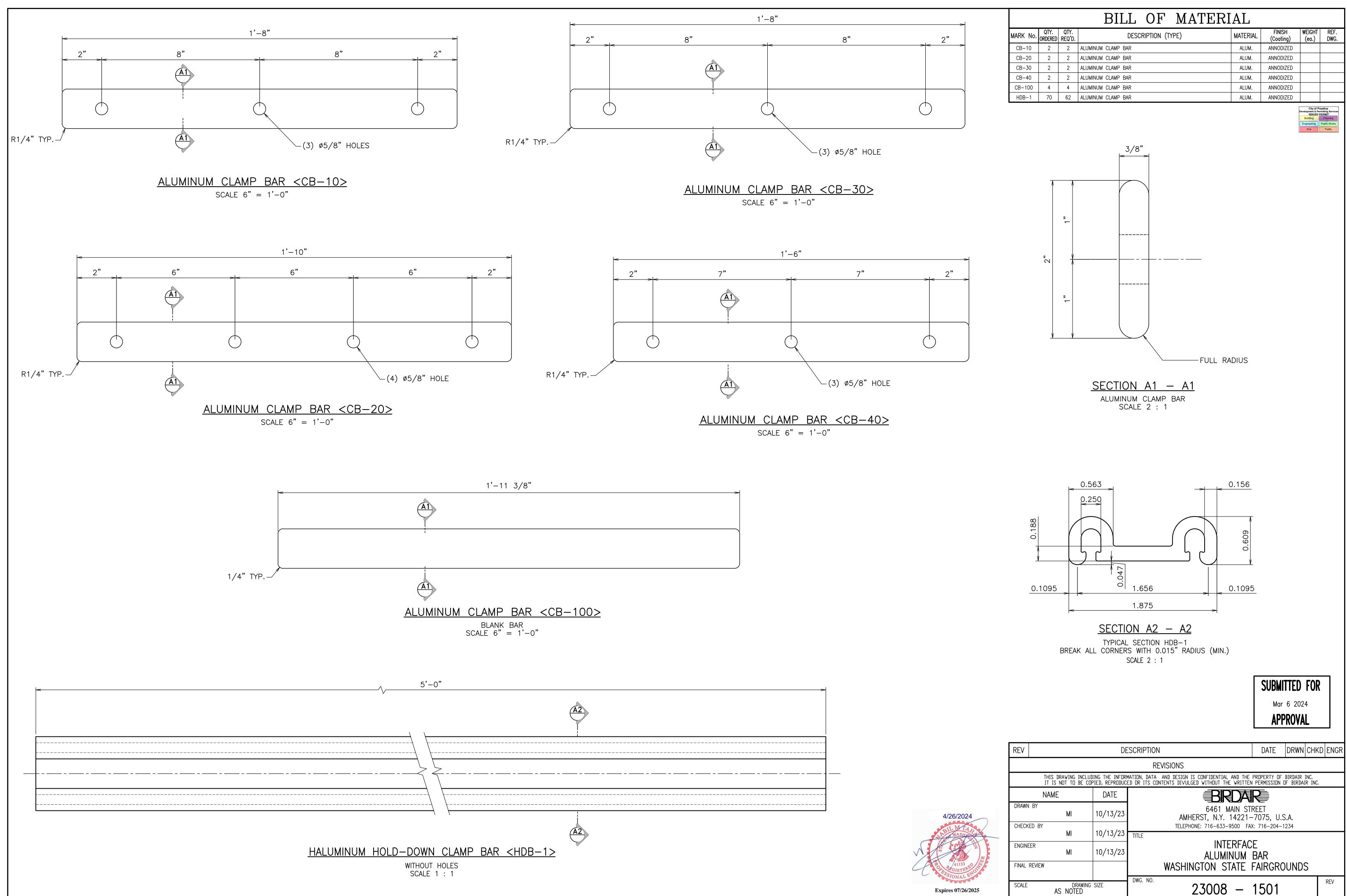
3.2.6 ALL WELDED JOINTS SHALL CONFORM TO AWS D1.2.



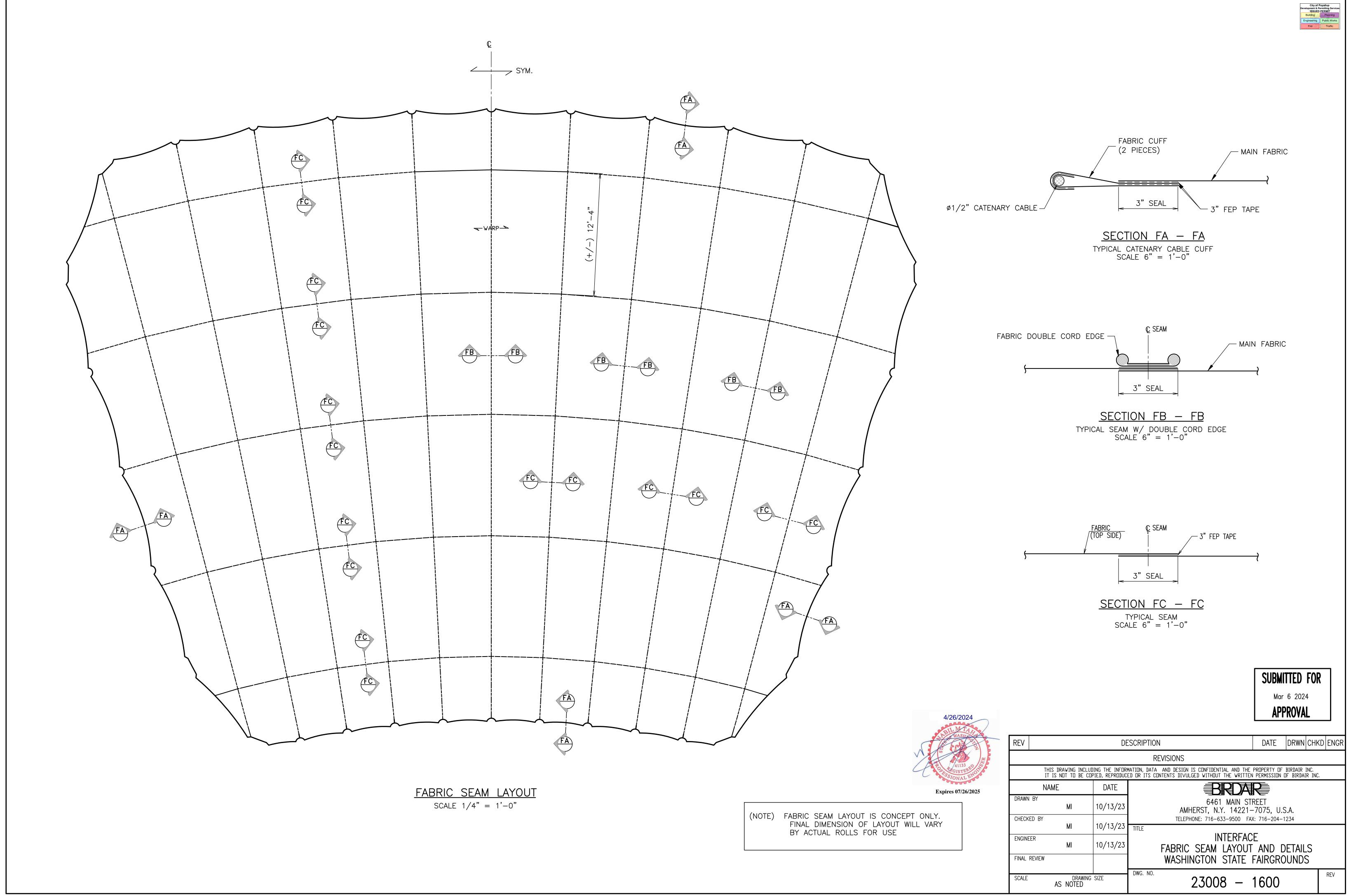
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	NAME	DATE	BRDAR						
DRAWN	BY MI	10/13/23	6461 MAIN STREET AMHERST, N.Y. 14221–7075, U.S.A.						
CHECK	ed by MI	10/13/23	INTERFACE						
ENGINE	er MI	10/13/23							
FINAL	REVIEW		WASHINGTON STATE FAIRGROUNDS						
SCALE	DRAWING AS NOTED	I SIZE	dwg. no. 23008 –	1500			REV		

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City of Puyallup Development & Permitting Services /ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic



Expires 07/26/2025



# SPONGE NEOPRENE SPECIFICATIONS

## <u>1.0 GENERAL</u>

ALL WORK MUST COMPLY WITH THE LATEST EDITION OF THE AMERICAN SOCIETY FOR TESTING AND MATERIALS AS REFERENCED.

## 2.0 MATERIAL

- 2.1 ALL SPONGE NEOPRENE SHALL BE OF A CELLULAR ELASTOMERIC COMPOUND OF A FIRM GRADE WHICH HAS BEEN MANUFACTURED IN PRE-FORMED SHAPES FOR USE AS GASKET AND SEALING MATERIAL, AS SPECIFIED IN ASTM SPECIFICATION C509.
- 2.2 CELLULAR ELASTOMERIC MATERIALS FURNISHED TO THIS SPECIFICATION SHALL BE MANUFACTURED FROM NATURAL OR SYNTHETIC RUBBER, OR MIXTURES OF THESE, WITH ADDED COMPOUNDS OF SUCH NATURE AND QUALITY THAT, WITH PROPER CURING, THE FINISHED PRODUCT WILL COMPLY WITH THIS SPECIFICATION.
- 2.3 THE CURED COMPOUNDS SHALL BE SUITABLE FOR USE WHERE RESISTANCE TO SUNLIGHT, WEATHERING OXIDATION, AND PERMANENT DEFORMATION UNDER LOAD ARE OF PRIME IMPORTANCE.
- 2.4 THE MANUFACTURING PROCESS SHALL BE SUCH AS WILL ENSURE A HOMOGENEOUS CELLULAR MATERIAL FREE OF DEFECTS THAT MAY AFFECT SERVICEABILITY.
- 2.5 THE PHYSICAL CHARACTERISTICS OF THE NEOPRENE MUST MEET OR EXCEED ASTM C509 – "STANDARD SPECIFICATION FOR ELASTOMERIC CELLULAR PRE FORMED GASKET AND SEALING MATERIALS"
- 2.6 CERTIFICATION OF MATERIAL SHALL BE PROVIDED THAT CONFORMS TO ASTM C509.

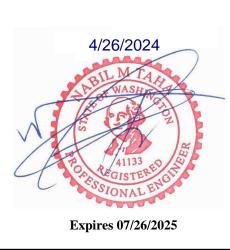
## <u>3.0 WORKMANSHIP</u>

- 3.1 NEOPRENE SHALL BE CLEAN AND FREE OF FOREIGN MATTER.
- 3.2 TOLERANCES SHALL BE AS FOLLOWS:

THE THICKNESS SHALL BE PLUS OR MINUS 0.02" (0.50 mm) FROM THEORETICAL, UNLESS NOTED OTHERWISE.

THE WIDTH SHALL BE PLUS OR MINUS 0.20" (5 mm) FROM THEORETICAL, UNLESS NOTED OTHERWISE.

THE HOLE SPACING SHALL BE PLUS OR MINUS 1% OF THE THEORETICAL, UNLESS NOTED OTHERWISE.



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NAME DATE			BRDAR					
DRAWN BY	МІ	10/13/23	6461 MAIN STREET AMHERST, N.Y. 14221–7075, U.S.A.					
CHECKED BY	МІ	10/13/23	TELEPHONE: 716–633–9500 FAX: 716–204–1234					
ENGINEER	MI	10/13/23	INTERFACE NEOPRENE GASKET SPECIFICATION					
FINAL REVIEW			WASHINGTON STATE FAIRGROUNDS					
SCALE	DRAWING AS NOTED	l SIZE	DWG. NO. 23008 - 1700	REV				

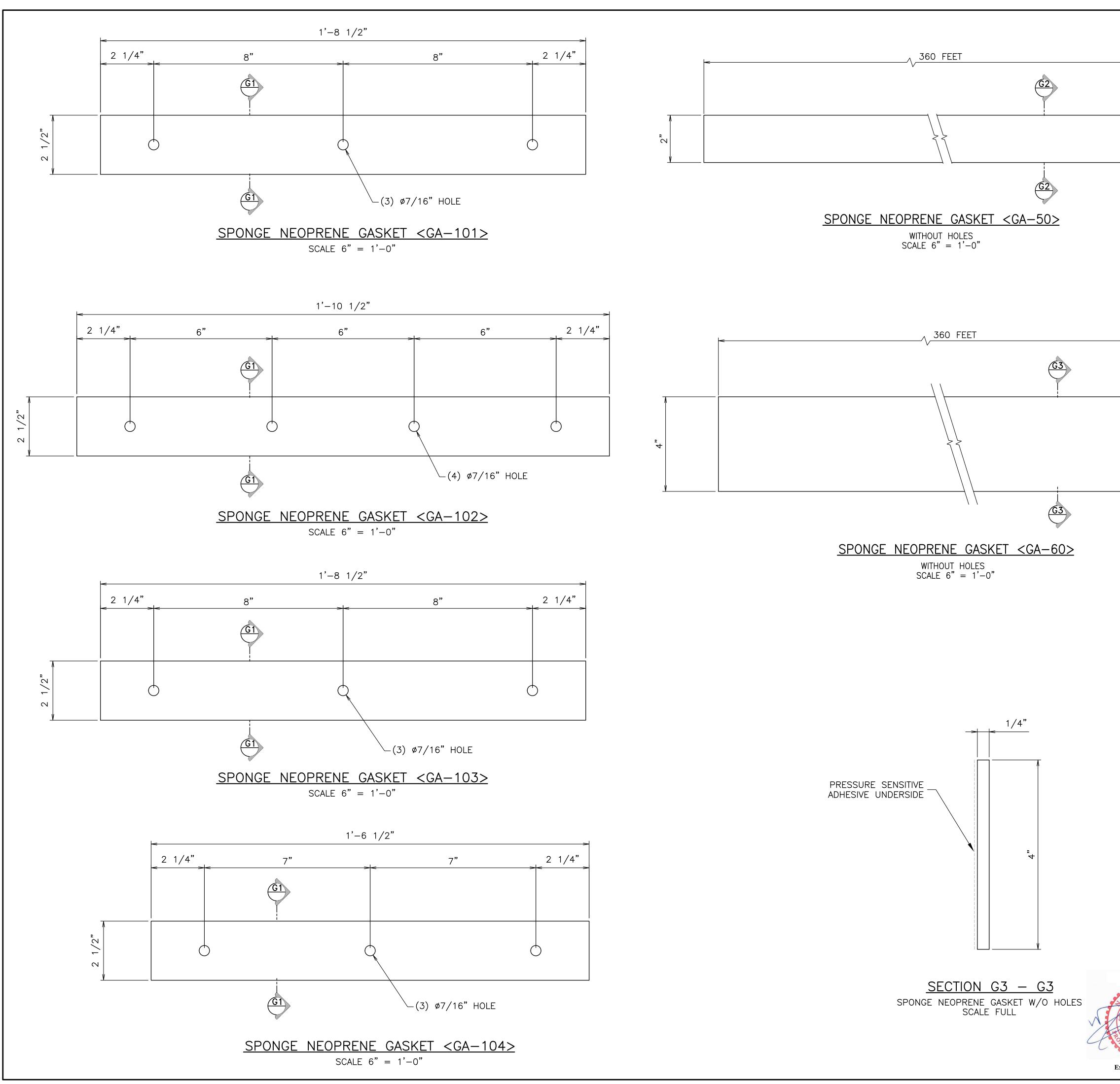
DESCRIPTION

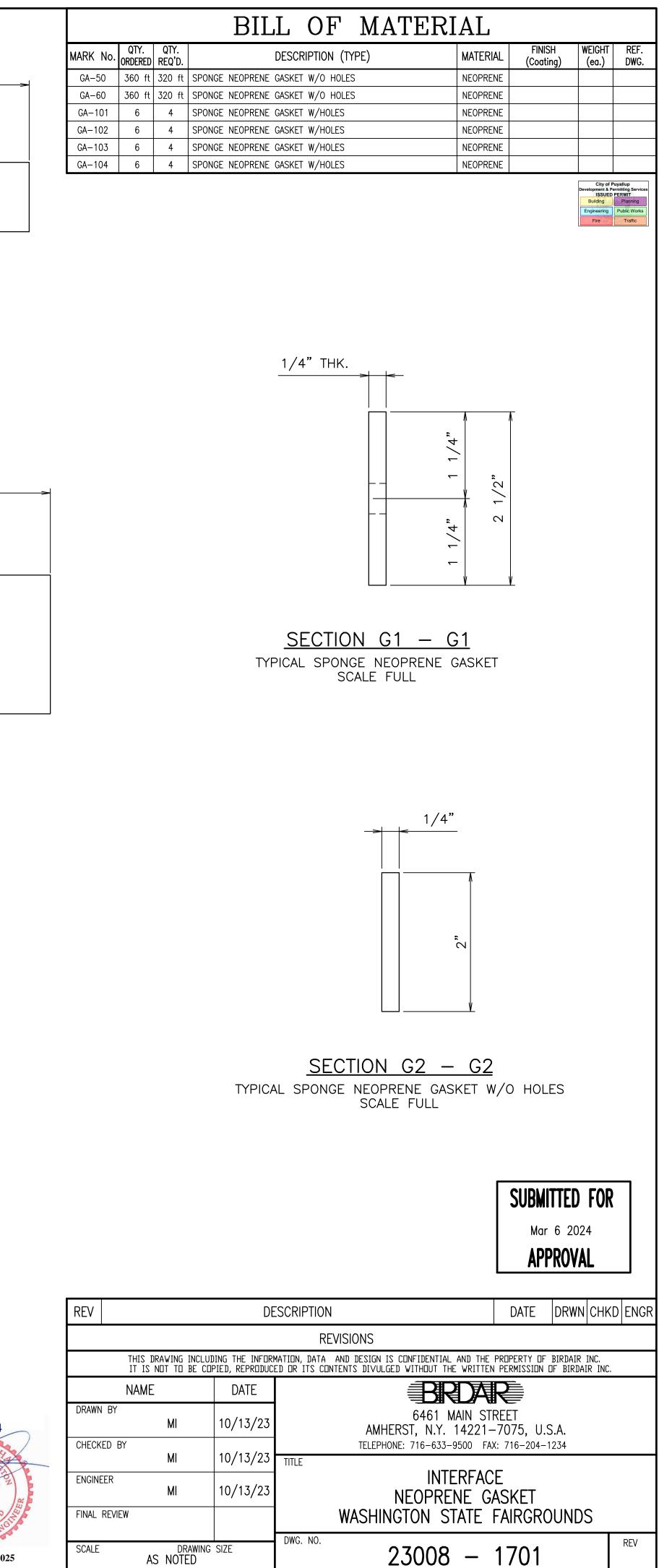
SUBMITTED FOR Mar 6 2024

APPROVAL

DATE DRWN CHKD ENGR

City of Puyallup Development & Permitting Services /ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic





## **IMPERIAL FASTENER SPECIFICATIONS**

### 1.0 <u>GENERAL</u>

1.1 ALL WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING AMERICAN STANDARD SPECIFICATIONS: AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) - AS REFERENCED.

AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI) - AS REFERENCED.

## 2.0 <u>MATERIALS</u>

- 2.1 STAINLESS STEEL STUDS SHALL CONFORM TO ASTM F 593, TYPE SS. THREADS SHALL BE ROLLED.
- 2.2 STAINLESS STEEL BOLTS SHALL CONFORM TO ASTM F 593, TYPE SS. THREADS SHALL BE ROLLED.
- 2.3 STAINLESS STEEL NUTS SHALL CONFORM TO ASTM F 594, TYPE SS, HEX.
- 2.4 STAINLESS STEEL WASHERS SHALL CONFORM TO TYPE SS.
- 2.5 STAINLESS STEEL LOCK WASHERS SHALL BE SPLIT-RING, TYPE SS.
- 2.6 STAINLESS STEEL SOCKET BUTTON HEAD CAP SCREW SHALL CONFORM TO ASTM F 879, TYPE SS.
- 2.7 HIGH STRENGTH STEEL BOLTS SHALL CONFORM TO ASTM A 325, TYPE 1, HEAVY HEX UNLESS NOTED
- 2.8 NUTS AND ASSOCIATED JAM NUTS, SUITED FOR HIGH STRENGTH BOLTS, SHALL CONFORM TO ASTM A 563, GRADE DH, HEAVY HEX UNLESS NOTED OTHERWISE.
- 2.9 HIGH STRENGTH HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F 436, TYPE 1, CIRCULAR.
- 2.10 THREADED ROD SHALL CONFORM TO ASTM A 36, ASTM A 572 GRADE 50, ASTM A 193 GRADE B7, OR ASTM F 1554 GRADE 36 OR 55, AS NOTED.
- 2.11 COMMON BOLTS SHALL CONFORM TO ASTM A 307, GRADE A, HEX UNLESS NOTED OTHERWISE.
- 2.12 COMMON NUTS AND ASSOCIATED JAM NUTS SUITED FOR ASTM A 307 BOLTS SHALL CONFORM TO ASTM A 563, GRADE A, HEX UNLESS NOTED OTHERWISE.
- 2.13 COMMON FLAT WASHERS SHALL CONFORM TO ASTM F 844, OR ANSI B18.22.1 TYPE A (USS).
- 2.14 COMMON SPRING LOCK WASHERS SHALL CONFORM TO ANSI B.18.21.1.

2.15 BIRDAIR SHALL BE SUPPLIED WITH CERTIFICATION THAT ALL FASTENERS COMPLY WITH THE ABOVE SPECIFICATIONS.

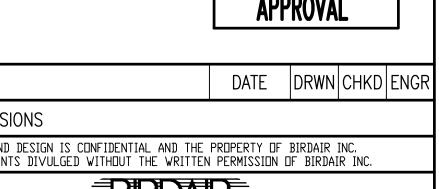
### 3.0 <u>EXECUTION</u>

3.1 INSPECTION AND QUALITY CONTROL

THE FASTENER FABRICATOR SHALL PROVIDE EFFECTIVE QUALITY CONTROL OVER ALL FABRICATION ACTIVITES. BIRDAIR OR ITS TESTING AGENCY MAY VISIT THE PLANT AT ANY TIME TO VERIFY THAT A QUALITY CONTROL PROGRAM IS IN PLACE. THIS INSPECTION DOES NOT RELIEVE THE FABRICATOR FROM MEETING THE QUALITY AND WORKMANSHIP REQUIREMENTS OF THIS SPECIFICATION.

- 3.2 WORKMANSHIP
  - 3.2.1 ALL WORK SHALL BE FREE OF OIL, GREASE AND MACHINING CHIPS.
  - 3.2.2 THREADS FOR BOLTS, STUDS, AND NUTS SHALL CONFORM TO ANSI/ASME B 1.1, RIGHT HAND, UNIFIED COARSE THREAD SERIES (UNC), AND SHALL HAVE CLASS 2A TOLERANCE UNLESS NOTED OTHERWISE.
  - 3.2.3 DIMENSIONS OF FASTENERS SHALL CONFORM TO THE REQUIREMENTS OF ANSI/ASME B 18.2.1, "SQUARE COARSE THREAD SERIES (UNC), AND SHALL HAVE CLASS 2A TOLERANCE UNLESS NOTED OTHERWISE.
  - 3.2.4 DIMENSIONS OF WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ANSI B18.22.1, "PLAIN WASHERS", TYPE A UNLESS NOTED OTHERWISE.
- 3.2.5 DIMENSIONS OF HEX NUTS SHALL CONFORM TO THE REQUIREMENTS OF ANSI B 18.2.2, "HEX NUTS AND JAM NUTS" UNLESS NOTED OTHERWISE.
- 3.2.6 DIMENSIONS OF LOCK WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ANSI B 18.21.1, "REGULAR HELICAL SPRING LOCK WASHERS" UNLESS NOTED OTHERWISE.
- 3.2.7 WHERE NOTED, ALL HOT DIP GALVANIZED, "GALVANIZED" FASTENERS SHALL CONFORM TO ASTM A 153, CLASS C.
- 3.2.8 WHERE NOTED, ALL MECHANICALLY DEPOSITED, "PLATED" FASTENERS SHALL CONFORM TO ASTM B 695, CLASS 50.



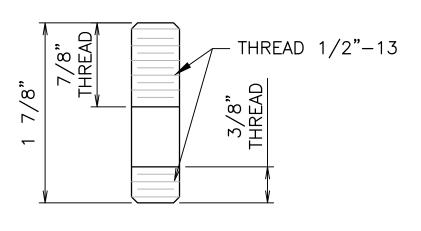


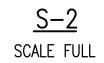
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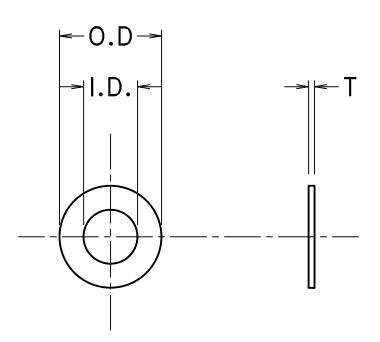
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Mar 6 2024 APPROVAL

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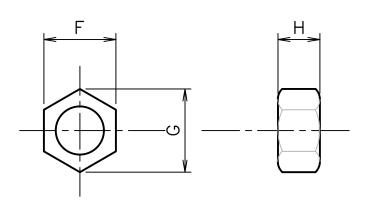






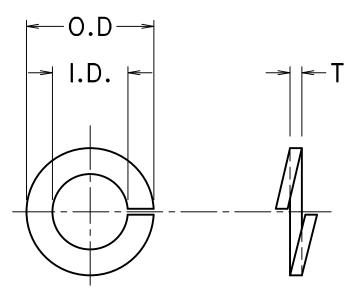
<u>FLAT ROUND WASHER</u>

MARK	DIA	I.D.	0.D.	Т
W-1	1/2	9/16	1 1/16	1/8



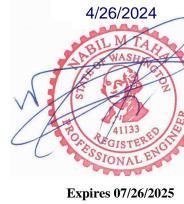
<u>HEX NUT</u>

MARK	DIA	F	G	Н	THRD./IN.
N-1	1/2	3/4	7/8	7/16	1/2 - 13



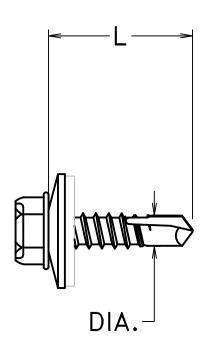
SPLITLOCK WASHER

MARK	DIA	I.D.	0.D.	Т
LW-1	1/2	1/2+	7/8	1/8



	BILL OF MATERIAL									
MARK No.	QTY. Ordered	QTY. REQ'D.	DESCRIPTION (TYPE)	MATERIAL	FINISH (Coating)	WEIGHT (ea.)	REF. DWG.			
S-2	40	26	STAINLESS 1/2" STUD	SS304	SS					
N-1	40	26	STAINLESS 1/2" HEX NUT	SS316	SS					
W-1	40	26	STAINLESS 1/2" FLAT WASHER	SS304	SS					
LW-1	40	40	STAINLESS 1/2" LOCK WASHER	SS304	SS					
SC-10	600	480	STAINLESS 1/4" SELF-DRILLING SCREW W/GASKET HEAD	SS304	SS					

City of Puyallup Development & Permitting Services /ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic



# SELF-DRILLING SCREW W/BONDED SEALING WASHER

HEX WASHER HEAD

MARK	DIA	L	THRD./IN.
SC-10	1/4"	2	1/4-20

SUBMITTED FOR

Mar 6 2024

