The Structure has been designed to resist code-required vertical and lateral forces after the construction of all structural elements have been completed. Stability of the structural elements prior to completion is the sole responsibility of the General Contractor. Builder is to verify all dimensions prior to starting work. All changes to structure must be reviewed and approved by Phillips Structural Engineering. All materials, methods, and workmanship shall be in accordance with the International Building Code (IBC) 2018 Edition. The Builder is responsible for using safe work practices and conforming to all safety ordinances.

Any construction observation performed by the Structural Engineer is for general conformance with design aspects per IBC 1704.6 and is not intended in any way to review the General Contractor's construction procedures. Structural observation does not waive the responsibility for special inspections per Section 1705.

STANDARDS:
All methods, materials and workmanship shall conform to the 2018 International Building Code (IBC) as amended and adopted by the local Building Official or applicable jurisdiction. STRUCTURAL DESIGN CRITERIA AND LOADING:

ROOF LOADING:

- DEAD LOAD = 15PSF (NO TILE WEIGHT INCLUDED)
 ROOF LIVE LOAD (Lr) = 20PSF
 TYPICAL FLAT ROOF SNOW LOAD (Pf) = 25PSF
 SNOW EXPOSURE FACTOR (Ce) = 1.0
 SNOW LOAD IMPORTANCE FACTOR (Is) = 1.0
 *TYPICAL ROOF SNOW LOAD SHALL NOT BE LESS THAN 25PSF

FLOOR LOADING:DEAD LOAD = DEAD LOAD = 12PSF (STANDARD WOOD FRAMING WITHOUT HEAVY FINISH) TYPICAL RESIDENTIAL OCCUPANCY LIVE LOADING (L) = 40PSF DECK LIVE LOAD (L) = 60PSF (1.5X FOR AREA SERVED) UPANCY LIVE LOA = (1.5X FOR AREA)

- WIND DESIGN DATA:

 BASIC DESIGN WIND SPEED (V) = 110 MPH

 ALLOWABLE STRESS DESIGN WIND SPEED (Vasd) = 85 MPH
- RISK CATEGORY = II WIND IMPORTANCE FACTOR (lw) = 1.0 WIND EXPOSURE = "B" TOPOGRAPHICAL EFFECT (Kzt) = 1.00

- EARTHQUAKE DEIGN DATA:
- SEISMIC IMPORTANCE FACTOR (Ie) = 1.0
 MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS
 Ss = 1.26
- Ss = 1.26
 S1 = 0.44
 SEISMIC DESIGN CATEGORY (SDC) = "D"
 BASIC SEISMIC FORCE-RESISTING SYSTEM = LIGHT-FRAM WALLS SHEATHED WITH WOOD STRUCTURAL PANEL RAT RESISTANCE
 RESPONSE MODIFICATION FACTOR (R) = 6.5
 ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FO = LIGHT-FRAMED (WOOD) AL PANEL RATED FOR SHEAR
- FORCE

SOIL INFORMATION: In the absence of a sc

In the absence of a soils report (prepared by a Licensed Geotechnical Engineer) an Allowable Soil Bearing Pressure (ASBP) of 1500psf has been assumed for design. It is the contractor's responsibility to verify that all footings bear on firm/unyielding, undisturbed earth or compacted "Structural Fill" that meets or exceeds the assumed allowable soil bearing pressure. Soils under footings and slabs to be a minimum of 95% compacted per the Modified Proctor Density Test (ASTM D1557). All construction on fill shall be reviewed by a licensed Geotechnical Engineer.

FOUNDATIONS:

Bottom of exterior footings shall be a minimum of 18" below finished grade for frost protection U.N.O. bearing on undisturbed native soils. Backfill with dry, free draining soils. Backfill next to any retaining walls with a minimum of 12" gravel or free draining soil per typical detailing. Appropriate drainage shall be provided adjacent to the base of exterior foundations draining to daylight per Soils Engineer requirements.

All concrete materials and placement shall conform to the current ACI 318 code. Concrete shall be made with Portland Cement ASTM C-150 Type II or Type I and shall be Ready-Mixed per ASTM C-94. Minimum compressive strength shall be 3000 PSI* at 28 days with a minimum water/cement ratio of 0.45. All concrete shall be air entrained $5 \pm 1\%$. Max aggregate size = $\frac{7}{8}$ ". *Special Inspection not required for concrete compressive strength. 3000 PSI compressive strength has been specified for weathering protection. Structural design of concrete has been based on 2500 PSI compressive strength. Concrete shall

PLAN VERACITY: Every attempt has been made to insure the accuracy of these engineered documents, site conditions, product availability, etc. All information listed above must be verified prior to construction and fabrication. Any changes deficiencies on or to the plan must be transmitted to Phillips Structural Engineering, PLLC for written approval. 으

PANEL EDGE BOUNDARY ———— NAILING (6" O.C. MAX. SPACING)

PANEL EDGE NAILING (STAGGER NAILING ()

JOIST/BLOCKING

2. Ownership of Engineering Calculations and Drawings: Phillips Structural Engineering, PLLC shall retain "Title 17 USC Rights and Ownership" of the Copyright Law of The United States of America of these Engineering Documents and all subsequent copies of the engineering. The Licensee is not permitted unlimited reuse of these documents without prior consent. LICENSE OF ENGINEERING DOCUMENTS:

1. Grant of License: Phillips Structural Engineering, PLLC grants the Licensee a nonexclusive Right of Use for the purpose of constructing a single structure (use) from this engineering package. Future uses of this drawing set are permitted when accompanied by an originally stamped "Site Specific Re-Use Letter" for each additional site and payment has been made to Phillips Structural Engineering, PLLC.

ROVED GLUE,

3/4" T&G PLYWOOD C SHEATHING AT FLOOR SHEATHING AT ROOF (

TYPICAL

HORIZONTAL (ROOF /

- PL

YWOOD D FLOOR)

DIAPHRAGM

LAYOUT

 \sim

PHILLIPS

STRUCTURAL

ENGINEERING

3. Copy and Transfer Restrictions: Unauthorized copying of the Engineering documents is strictly forbidden. The Engineering is permitted to be used by the Licensee only, and may not be transferred to a 3rd party without prior written consent of Phillips Structural Engineering, PLLC.

REINFORCING STEEL:

All reinforcing steel to be GRADE 60 PER ASTM A-615. Reinforcement lap splices shall be the greater of 32 bar diameters or 18". Lap horizontal steel at all corners and intersections in footings and walls with continuous corner bars. Footing reinforcement shall run continuous thru pad/expanded footings.

Minimum concrete cover over reinforcing steel shall be provided as follows:

INTERIOR FACES OF SLAB AND WALL BARS = 1½"

EXPOSED TO WEATHER OR EARTH = 1½" AT #5 AND SMALLER AND 2" AT #6 AND

- LARGER.
 CONCRETE CAST AGAINST SOIL
- ယ္န

WOOD FRAMING AND CARPENTRY:

General Requirements: Provide minimum nailing on the drawings. Do not notch or drill structural m Framing Connectors: Only ICC / IAPMO approved applications as manufactured by Simpson Strongquantity of fasteners shown in the manufacturer's Fasteners: ng per 2018 IBC table 2304 10.1 members, except as permitted k members, except as red connectors shall ole 2304.10.1 or as indicated as permitted by the Engineer. all be used in framing the Provide maximum size and size and

Bolts shall be per ASTM A-307 with standard cu Post-installed anchors require Engineer's appro ut washers or malleable iron washers.

val prior to installation. Contact Phillips

Structural Engineering for possible alternatives.
Nails shall be common wire nails or equivalent procanadian manufacturer only as indicated below. Financial manufacturer in the common wire nails or equivalent procanadian manufacturer only as indicated below. pneumatically drive nails (P-nails), American v. P-nails shall be installed per the

				· 1	1
	12d COMMON	N/A	10d COMMON	8d COMMON	COMMON WIRE NAIL
	0.148	0.131	0.148	0.131	(INCHES)
p-	31/4"	3"	21/2"	2½"	(INCHES)
	FRAMING	FRAMING	SHEATHING	SHEATHING	APPLICATION

Wood Sheathing (Structural):
Roof sheathing shall be ½" CDX or ¼6" OSB naile o.c. in field. Span index shall be 24/0. Plywood S (glued & nailed). Nailing shall be 10d @ 6"o.c. alc Span index shall be 48/24. Stagger all end laps p the grade trademark of the American Plywood As ailed w/ 8d @ 6" o.c. alor od Sub Flooring shall be 3 along panel edges, and os per typical detailing. A Association (APA). c. along panel edges, and 12" ll be ¾" T&G CDX or OSB s, and 12"o.c. in field (U.N.O.) ng. All sheathing shall bear

Studs:
All studs shall be kiln dried (KD) or surface dried (West Coast Lumber Inspection Bureau (WCLIB) c (WWPA) showing grade mark or approved equal. nominal minimum material and species shall be a following minimum strength properties: Fb= 675ps d (SD). d (SD). Each stud shall bear the stamp of) or Western Wood Products Association al. All studs and framing material shall be a minimum of Hem-Fir stud grade meetin psi, Fv=150psi, E=1,200,000psi. of the

DF#2 or (2)2x10 HF#2 with at least one cripple a 5'-0" and 8'-0" shall have headers as noted on p stud U.N.O. Spans greater that 8'-0" shall have Heavy Timbers:

All #:----oted on the plan with spans ≤5'-0" are to be 4x8 and one king stud at each end. Spans between olan and have at least two cripples and one king

All timbers above sizes listed above including (Typically 6x beams and posts). osts and beams shall be Doug-Fir #2 or better framing and support per plan.

Glu-Laminated Beams (GLB):

All GLB shall be in conformance with ANSI A190.1, American National Standards for Structural Glue-Laminated Timber. Grade 24F-V4 shall meet or exceed the following minimum structural properties: Fb=2400psi, Fv=240psi, E=1,800,000psi and shall be used at simple spans. Grade 24F-V8 shall be used at continuous and cantilevered spans. As an alternative at continuous/cantilevered spans an equivalent beam may be substituted

nd Lumber (PSL)

Parallel Strand Lumber (<u>PSL):</u>
All PSL shall be in conformance with ASTM D2559 and NER-292. PSL strength requirements shall meet or exceed the following minimum structural properties: Fb=2,900psi, Fv=290psi, E=2,000,000psi.

Laminated Veneer Lumber (LVL):

All LVL shall be in conformance with

neet or exceed the following minimum =1,900,000psi. reatment (P.T.): n ASTM D25 um structura 559. LVL shall be made of Doug-Fir (DF)and perties: Fb= 2,600psi, Fv=285psi,

All exposed framing including lumber, plywood an 0.25#/cf pentachlorophenol per AWPA specification and boring after pressure treatment shall be cared M-4. Exposed framing includes, but is not limited 1. Joists, girders and subfloor that is closer than 1. 2. Wood framing (including sheathing) that rest or the state of the state o and deck materials shall be pressure treated with ation P-5 or other approved treatment. All cutting red for in accordance with AWPA specification

less from exposed earth.

3. Any other wood product in direct contact with concrete or masonry.

Wood Connectors at P.T. Conditions:

Metal connectors which are in contact with pressure treated wood shall be protected with one of the following: Simpson "ZMAX" G185 Galvanization, Triple Zinc Coated, hot Dipped Galvanized or other approved method. 18" to exposed ground in crawl spaces. on exterior foundation stem walls and is 8" or

SITE FOR ALL INSPECTIONS (MIN. PLAN SIZE 24" X 36") C.T. S. P. P. O. S. F. F. E. C. S. P. S. P

All prefabricated wood roof and floor trusses:

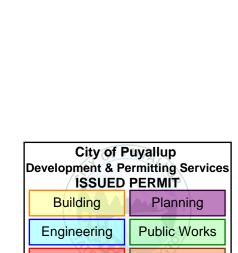
All prefabricated wood roof and floor trusses shall be designed by or under the direct supervision of a licensed Professional Engineer registered in the state where the structure is located. The truss shop drawings shall bear the stamp of that Engineer and shall be fabricated and installed per the latest Truss Plate Institute standards. Where trusses are not provided to complete the roof system, overframing members with positive connections shall be provided as directed by the Truss Engineer. Details on overframing shall be provided in the truss shop drawing package which shall be consistent with the modelling of the trusses. All necessary temporary and permanent bridging, blocking, pre-notched plates, hangers, etc. for the stability of the truss elements under gravity and lateral loads shall be designed, detailed/specified and furnished by the manufacturer. The Truss Manufacturer shall verify all setbacks, dimensions and bearing points prior to fabrication. Deviations from the layout shown on approved engineering plans shall be submitted to PSE for further evaluation. Maximum allowable deflections shall be as follows:

Roof Snow/Live Load = L/480 or $\frac{3}{4}$ " (whichever is less) Floor Total Load = L/360 or $\frac{3}{4}$ " (whichever is less) Floor Live Load = L/480 or $\frac{3}{2}$ " (whichever is less)

Trusses shall be designed for the spans and conditions shown and be constructed from a minimum of Hem-Fir timber and be furnished and installed in conformance with the manufacturer's published specifications. Additional concentrated loads from mechanical units and misc. equipment shall be accounted for/coordinated with sub-contractors, the Designer of Record and Truss Engineer. Framing has been designed assuming Hem-Fir plates w/ 405psi crushing capacity, truss engineer to confirm compatibility and add bearing enhancers or additional support as required.

Where trusses align with shear walls, truss engineer to design and provide a truss that has been designed to transfer lateral wind and seismic forces as shown on the plans. Loading indicated (100plf minimum) shall be designed by the truss engineer to transfer from roof or floor sheathing to shearwall below. Shop drawings including truss engineering shall be submitted to the Engineer of Record for review the builder is assuming responsibility and proceeding at their own risk.

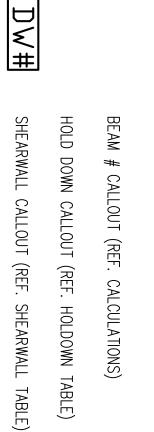
Alteration of the truss layout indicated on the plans may require supporting structural and foundation changes, therefore prior approval by the designer and Structural Engineer is required. Trusses shall not be field altered in any way without written approval from the Licensed Truss Engineer of Record.



City of Puyallup Building ACCEPTED

JMontgomery 07/19/2023 12:42:37 PM

SYMBOL Ω ABBREVIATIONS LEGEND:

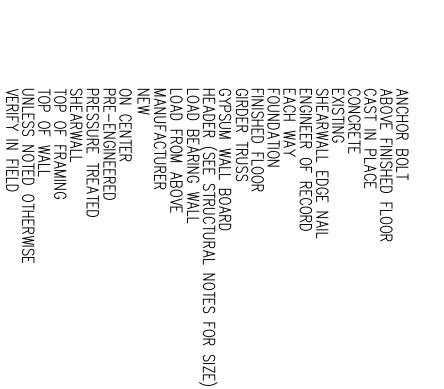


TERNATE ADHESIVE ANCHORS MAY BE AVAILABLE, CONTACT PHILLIPS STRUCTURAL ENGINEERING.
6" STEMWALLS ARE USED, THREADED ROD WITH DOUBLE NUT AND PLATE EXTENDED INTO THE FOOTING IS REQ'D.
FAILING. SB%"X24 & SB1X30 MAY ONLY BE USED IN 8" STEMWALLS.
1D14RJ HOLDOWNS ARE TYPICALLY TO BE INSTALLED WHERE A RIM JOIST OCCURS.
PSON HDU5-SDS2.5 MAY BE USED AS AN ALTERNATE TO STHD14(RJ).
RAPS MAY BE INSTALLED OVER SHEATHING

SEE ALTERNATE

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Traffic WHEN SHEATHING IS APPLIED OINTS ON OPPOSITE SIDES ARE OINTS ON OPPOSITE SIDES ARE OINTS ON OPPOSITE SIDES ARE DESIGNATED TO THE TOP OF THE AREATHING PANELLY THE AREALL SHEATHING PANELLY THE AREALL SHEATHING AT ALL MULTIPLE 2X STUDS). SEE TY ON THE AREALL STUDS SHALL BE CONTINUTED AND THE AREALL STUDS SHALL BE CONTINUTED AND SEE THE ALL ANCHOR BOLTS SHALL HAD SONCRETE. PLATE WASHERS SHAD SHALL TOOST INSTALLED ANCHORS MAY BE OUBLE STUDS ARE REQ'D ANCHORS TOGETHER W/ 164 AT STUDS TOGETHER W/ 165 AT STUDS TOGETHER W/ 165 AT STUDS TOGETHER W/ 165 AT STUDS TOGETHER W/ 166 AT STUDS TOGETHER W/ 16 Simpson Strong-Tie Product Label (SKU) (A) (B) (C) (F) (G) (H) YOWN ANCHORS AND STRAPS SHALL BE PER THE MANUFACTURE'S RECOMMENDATIONS
CONTACT WITH PRESSURE TREATED (P.T.) MATERIAL SHALL BE COATED WITH SIMPSON "Z-MAX" (G-185)
COATED FOR CORROSION PROTECTION. (2) 2X STUDS W/ (34) 0.162"\$x2½" NAILS (2) 2X STUDS W/ (46) 0.162"\$x2½" NAILS (2) 2x STUDS W/ (38) 0.148"\$x3" NAILS (2) 2x STUDS W/ (38) 0.148"\$x3" NAILS Required <u>NAILING</u> and Boundary Studs HOLDOWN AND STRAP

Strap/Holdown Attachment and Required

IMPORTANT FRAMING NOTE: GENERAL CONTRACTOR/FRAMER SHALL VERIFY ALL CONCENTRATED LOADS FROM ABOVE TO HAVE SOLID BEARING CONTINUOUS TO THE FOUNDATION (INCLUDING SQUASH BLOCKING IN CRAWLSPACE/FLOOR FRAMING). ANY CHANGES TO FOUNDATION AND STRUCTURE FROM ARCHITECTURAL PLANS SHALL BE NOTED AND ALL AFFECTED PARTIES SHALL BE NOTIFIED.

2018 IBC

E TOP PLATES TAGGERED .*-0" MIN	BEAM PER SIMPSON ST6224 W/ (28) 0.162ø × 2½" NAILS (CENTER ON SPLICE)
1-1/2" MIN ———————————————————————————————————	DOUBLE 2X TOP PLATE
SPLICE	(2) ZX CRIPPLE & (1) 2X KING MIN. TYPICAL WALL FRAMING
EATED BY BEAMS 1 ST6224 UNLESS ACED ON TOP OPTION.	80
CHORD SPLICE	3) BEAM FLUSH TO DBL TOP PLATE

ביו עוודמו שמוו נמטוד.
5. Where (2) shearwalls meet at a corner/intersection, the sheathing of each
edge nailed to the studs/post which the holdown is attached to.
6. Provide a solid rim joist where floor joist bear on exterior walls per typica
7. Where floor framing runs parallel to exterior walls, install 2x or I-joist bloom
between rim joist and first joist @ 48" o.c. ~ nail sheathing to blocking w/ 8c
(crawlspace framing where rim joist bears on concrete stemwalls are omitte
requirement).
8. Roof and floor joists/trusses are shown schematically on the plan and ar
show every location of every joist/truss.
9. Provide double joists under interior partition walls when running parallel
10. Top plates are assumed to be continuous and may be spliced per typic
sheet.
11. All columns not specified or shown on the plans are to be a minimum o
spike laminated together with 16d nails @ 6" o.c. (stagger).
12. All post-beam intersections shall contain positive connections to resist
and/or lateral displacement.
13. Anchorage of walls to the foundation shall be provided in accordance w
schedule.
14. Typical Walls shall be framed with Hem-Fir U.N.O.
15. Walls shall be anchored to the foundation per the minimum requiremen
 A B @ 48" o c embedded 7" and no more than 12" from ends of each sill a

ICALLY — FACE NAIL DOUBLE TOP PLATES 0.131″ø X 3″ @ 12″ O.C. STAGGERED L 4'—0″ MIN	BEAM PER SIMPSON ST6224 W/ (28) 0.1620 × 2½" NAILS (CENTER ON SPLICE)
1-1/2" MIN ———————————————————————————————————	DOUBLE 2X TOP PLATE
SPLICE SPLICE	(2) 2X CRIPPLE & (1) 2X KING MIN.
STUD STUD STUD STUD ® BOT SPLICE LOCATION (TYP) ATE DISCONTINUITIES ARE CREATED BY BEAMS ITC. STRAP W/ (1)-SIMPSON ST6224 UNLESS HERWISE. STRAP MAY BE PLACED ON TOP F PLATES ® CONTRACTOR'S OPTION.	TYPICAL WALL FRAMING
TYPICAL TOP CHORD SPLICE	3) BEAM FLUSH TO DBL TOP PLATE

A.B. @ 48" o.c. embedded 7" and no more than 12" from ends of each sill and 2 bolts minimum) with 3"x3"x¼" square washers firmly attached between plate and nut. 16. All walls over 10' tall shall have bracing/blocking at 48" o.c. (flat or on edge).	15. Walls shall be anchored to the foundation per the minimum requirements of SW6 ($\frac{5}{8}$ "Ø	14. Typical Walls shall be framed with Hem-Fir U.N.O.	schedule.		12. All post-beam intersections shall contain positive connections to resist against uplift	11. All columns not specified or shown on the plans are to be a minimum of (2) 2x studs spike laminated together with 16d nails @ 6" o.c. (stagger)	sheet.	Provide double joists under interior partition walls when running parallel to e	show every location of every joist/truss.	8. Roof and floor joists/trusses are shown schematically on the plan and are not intended to	requirement).	(crawlspace framing where rim joist bears on concrete stemwalls are omitted from this	between rim joist and first joist @ 48" o.c. ~ nail sheathing to blocking w/ 8d @ 3" o.c.	Where floor framing runs parallel to exterior walls, install 2x or I-joist blockir	6. Provide a solid rim joist where floor joist bear on exterior walls per typical details.	edge nailed to the studs/post which the holdown is attached to.	5. Where (2) shearwalls meet at a corner/intersection, the sheathing of each wall shall be	per shearwall table.	4. Exterior walls shall be nailed & detailed per SW6 U.N.O. All panel edges shall be blocked	Built-ups or posts shall run continuous to the foundation.	_	3. Provide solid built-up studs or posts under all girder trusses, roof beams and floor beams.	rafter & truss to framing with Simpson H2.5T or clip indicated in typical detailing.	2. Solid blocking shall be provided between the bearings of every rafter or truss. Attach every	1. Roof and floor diaphragm nailing per Wood Sheathing Notes.	I YPICAL FRAMING NOTES:	+\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
S #	DESIGNER:	PSE NUMBER:	SCALE:	ORIGINAL DRAWING DATE:	DRAWN BY:			GENER,				ROOF SNOW LOAD:	WIND EXPOSURE:	WIND STEED.			ı	Z PR	13							UI	P
SHEET NO:	RUEPPELL	PSE 21.047	N.T.S. (U.N.O.)	APR. 5, 2023	AMS			GENERAL NOTES				25 PSF	В		110 MBL		_[- 11	43	33	43 Y/	8R	D.	A۱		

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7-16

•			Fasteners	Strap/Holdown Attachment and Required Fasteners	rap/Holdown Atta	St	Stone Tie	Cimpo	
www.r.iiiiipaoE.coiii			TABLE	AND STRAP	HOLDOWN A	HOL			
Phone: (253) 344-1666					N.O.	: TABLE 2304.10.1 U.N.O	SHALL BE PER IBC	ALL NAILING S	≥
PO Box 108, Milton, WA 98354					OSB	APA RATED PLYWOOD MAY BE USED IN PLACE OF O	YWOOD MAY BE	PA RATED PI	≥
STRUCTURAL ENGINEERING		CONSULTATION.	STRUCTURAL ENGINEERING FOR	CONTACT PHILLIPS ST	ALTERNATIVE. CO	POST INSTALLED ANCHORS MAY NOT BE A VIABLE A	D ANCHORS MAY	OST INSTALLI	סַ
	FACE NAIL TO EACH OF	SING A SINGLE 3x. FAI	WHERE 3x STUDS ARE REQ'D AT ADJOINING PANEL EDGES (2) 2x STUDS MAY BE NAILED TOGETHER IN PLACE OF USING A SINGLE 3x. STUDS TOGETHER W/ 16d AT SAME SPACING SHOWN IN 2X BOTTOM PLATE ATTACHMENT COLUMN. PANEL EDGE NAILING IS REQUIRED THE DOUBLE STUDS AND SHALL BE STAGGERED	'UDS MAY BE NAILE PLATE ATTACHMEN	IN 2X BOTTOM	T ADJOINING PANEL E AME SPACING SHOWN BE STAGGERED	JDS ARE REQ'D A HER W/ 16d AT S STUDS AND SHALL	HERE 3x STU TUDS TOGETH HE DOUBLE S	∣⊉ળ≤
→		D MATERIALS.	PROVIDE "HOT-DIPPED" GALVANIZED NAILS/BOLTS AND METAL CONNECTORS WHEN ATTACHING TO PRESSURE TREATED MATERIALS.	ECTORS WHEN ATT	ND METAL CONN	IIZED NAILS/BOLTS AI	-DIPPED" GALVAN	ROVIDE "HOT	Ιœ
	INTO	ALL BE EMBEDDED 7"	W/ STANDARD 90' BEND SH	(3". ANCHOR BOLTS	HERS 0.229"x3")	ALL ANCHOR BOLTS SHALL HAVE STEEL PLATE WASHERS 0.229"×3"X3". ANCHO CONCRETE. PLATE WASHERS SHALL EXTEND TO WITHIN ½" OF SHEATHING EDGE.	ATE WASHERS SH	ONCRETE. PL	Ω ≥
	- <i< td=""><td>G.</td><td>SHEATHING SHALL BE CONTINUOUS OVER BOTTOM PLATE AND RIM OR SILL PLATE PER TYPICAL DETAILING WITH EDGE TWO ROWS OF NAILING ARE REQ'D AT DOUBLE SIDED SHEARWALK SEE TYPICAL DETAILING FOR FASTENERS AND BLK</td><td>OR SILL PLATE PER TYPICAL SEE TYPICAL DETAILING FOR</td><td>ATE AND RIM C</td><td>SHEATHING SHALL BE CONTINUOUS OVER BOTTOM PLATE AND RIM OR SILL PLATE PER TY TWO ROWS OF NAILING ARE REQ'D AT DOUBLE SIDED SHEARWALLS SEE TYPICAL DETAILING</td><td>SHALL BE CONTINUOUS OF NAILING ARE REQ'D</td><td>SHEATHING SHEATHING SHEATHING ROWS OF</td><td>님ㅋ의ㄷ</td></i<>	G.	SHEATHING SHALL BE CONTINUOUS OVER BOTTOM PLATE AND RIM OR SILL PLATE PER TYPICAL DETAILING WITH EDGE TWO ROWS OF NAILING ARE REQ'D AT DOUBLE SIDED SHEARWALK SEE TYPICAL DETAILING FOR FASTENERS AND BLK	OR SILL PLATE PER TYPICAL SEE TYPICAL DETAILING FOR	ATE AND RIM C	SHEATHING SHALL BE CONTINUOUS OVER BOTTOM PLATE AND RIM OR SILL PLATE PER TY TWO ROWS OF NAILING ARE REQ'D AT DOUBLE SIDED SHEARWALLS SEE TYPICAL DETAILING	SHALL BE CONTINUOUS OF NAILING ARE REQ'D	SHEATHING SHEATHING SHEATHING ROWS OF	님ㅋ의ㄷ
	O NG,	D DIRECTLY TO FRAMING, C CLIPS MAY BE USED	USE 0.131"0x2½" NAILS TYPICALLY FOR LTP CLIPS WHEN INSTALLING OVER SHEATHING. IF LTP CLIPS ARE INSTALLED 0.131"0x1½" NAILS MAY BE USED. LTP CLIPS MUST BE INSTALLED WITH LONG DIMENSION HORIZONTALLY. A35 OR RBOWHEN ATTACHING ROOF BLOCKING TO TOP PLATE.	OVER SHEATHING. TH LONG DIMENSIO	HEN INSTALLED W	TYPICALLY FOR LTP CLIPS WHEN INSTALLING OVER SHEATHING. BE USED. LTP CLIPS MUST BE INSTALLED WITH LONG DIMENSIC BLOCKING TO TOP PLATE.	NAILS TYPICAL AILS MAY BE USE ING ROOF BLOCKII	USE 0.131"øx2½" NAILS 0.131"øx1½" NAILS MAY WHEN ATTACHING ROOF	- ≤ o⊂
J	Š	I ≟	2½" FIELD NAILING AT 12" O.C. (WHERE :	×	VAILED WITH 0.1.	2X_ MINIMUM AND NAILED WITH 0.131"ø ; @ 6" O.C.)	INTERMEDIATE STUDS SHALL BE 2X_ N 16" O.C. PROVIDE FIELD NAILING @ 6"	INTERMEDIATE	= =
RE		I-UP HOLDOWN POST	TO EACH STUD USED IN A BUILT	REQUIRED	RTHER INFORMATION.	EDGE NAIL SHEATHING AT ALL HOLDOWN POST/STUDS. EDGE NAILING IS (MULTIPLE 2X STUDS). SEE TYPICAL DETAILS FOR FURTHER INFORMATION.	EATHING AT ALL I	DGE NAIL SH	
V. DELT#	E PLANS	ENDS OF FULL HEIGH D ON THE PLANS. SEI NAIL PATTERN).	SHEARWALL SHEATHING/NAILING SHALL BE PROVIDED THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. ENDS OF FULL HEIGHT SHEARWALLS ARE DESIGNATED BY EXTERIOR BUILDING CORNERS, CORRIDORS, WALL PENETRATIONS OR AS DESIGNATED ON THE PLANS. SEE & TABLE FOR REQUIRED HOLDOWNS (ALL SHEARWALLS REQUIRE SHEATHING ABOVE & BELOW OPENINGS WITH SAME NAIL PATTERN).	WALL BOVE	THE ENTIRE LE G CORNERS, COI LS REQUIRE SHE	SHEARWALL SHEATHING/NAILING SHALL BE PROVIDED THE ENTIRE LENGTH OF SHEARWALLS ARE DESIGNATED BY EXTERIOR BUILDING CORNERS, CORRIDORS, & TABLE FOR REQUIRED HOLDOWNS (ALL SHEARWALLS REQUIRE SHEATHING A	HEATHING/NAILING ARE DESIGNATED REQUIRED HOLDO	HEARWALL SI HEARWALLS , TABLE FOR	8 N N
A						EDGES	BLOCK ALL SHEATHING PANEL EDGES	LOCK ALL SH	lω
DA	THAT	SIDES OF WALL SUCH	EDGES ON OPPOSITE	& DW3), STAGGER PANEL S.	(DW4 STUD	WHEN SHEATHING IS APPLIED TO BOTH SIDES OF SHEARWALL JOINTS ON OPPOSITE SIDES ARE NOT LOCATED ON THE SAME	ING IS APPLIED TO	HEN SHEATH	≽≼
ATE		L PANEL EDGES.	16" O.C. SPACING. BLOCK AL	FRAMING EXCEEDS	ONTALLY WHERE	SHALL BE INSTALLED HORIZONTALLY WHERE	SHEATHING PANELS SHALL	ALL SHEATHIN	>
						S	MANDATORY NOTES	MANE	
REV	2x	%" @ 10" O.C.	(2) ROWS 16d @ 4" O.C. & LTP4 @ 16" OC AT BLK'G TO PLATE	LTP4 @ 6"OC	3×_	0.131×2½" @ 3" O.C.	%6™OSB Each Side	911/1274	۵ ا
	2x	%" @ 16" O.C.	(2) ROWS 16d @ 4" O.C. & LTP4 @ 16" OC AT BLK'G TO PLATE	LTP4 @ 8"OC	2×_	0.131×2½" @ 4" O.C.	¼6"OSB Each Side	700/990	4
I DE	2×	%" @ 18" O.C.	16d @ 3" O.C.	LTP4 @ 10"OC	3x_	0.131×2½" @ 2" O.C.	¾6"OSB (1) Side	595/832	2
SCF	2×	%" @ 24" O.C.	16d @ 3½" O.C.	LTP4 @ 12"0C	3x_	0.131×2½" @ 3" O.C.	¼6"OSB (1) Side	456/637	٦
RIPTION	2×	%" @ 32" O.C.	16d @ 4" O.C. OR 12d @ 3" O.C.	LTP4 @ 20"OC	2×_	0.131×2½" @ 4" O.C.	¾6"OSB (1) Side	350/495	4
N	2 _x	%" @ 48" O.C.	16d @ 6" O.C. OR 12d @ 4" O.C.	LTP4 @ 36"OC	2×_	0.131x2½" @ 6" O.C.	%"OSB (1) Side	242/339	၈၂
	Sill Plate Size © Fdn.	Spacing To Concrete Below (J) (K) (M)	Nailing To Wood Below (1)	(G) (H); (Does not apply to crawlspace)	© Adjoining Panel Edges (C) (F) (L)	© Sheathing Edges (D) (E) (K)	Sheathing (panel) (A) (B) (C) (D) (N)	Capacity (PLF) Seismic/Wind	<u>o</u>
	ment	Sill Plate Attachment	2x Bottom Plate Attachment	Rim Joist Or Block Connection To Top Plate	Stud & Blocking Size	Panel Edge Nailing	Minimum Required APA Rated	ASD S.W.	
		FIR) (O)	HEM-	HORAGE S	AND ANC	SHEAR WALL (SW) AND ANCHORAGE SCHEDULE	SHEAR		

I	N/A	N/A	N/A	N/A	I	SB1X30 (SEE NOTE E)	SB76X24 (SEE NOTE E)	MPSON SB%x24	MBEDDED STRAP	Anchorage to Foundation (D)
	4/5/2023		PROMAL EX		1 da Co 3 394 OF S	EE TO		NA PORTON		

ENGINEERING

2) 2X STUDS W/ (14) ¼"x2½" SDS SCREWS
(3) 2X STUDS W/ (20) ¼"x3" SDS SCREWS
3x6 DF POST W/ (24) ¼"x2½" SDS SCREWS

Required <u>SCREWS</u> and Boundary Studs

ALL VARIATIONS FROM DIMENSIONS AND CONDITIONS SHOWN ON THE DRAWINGS SHALL BE REPORTED TO THE DESIGNER FOR RESOLUTION WITH THE ENGINEER PRIOR TO PROCEEDING WITH WORK; FAILURE TO COMPLY BY THE CONTRACTOR SHALL BE THEIR SOLE RESPONSIBILITY FOR ANY COSTS NECESSARY FOR REMEDIAL WORK.

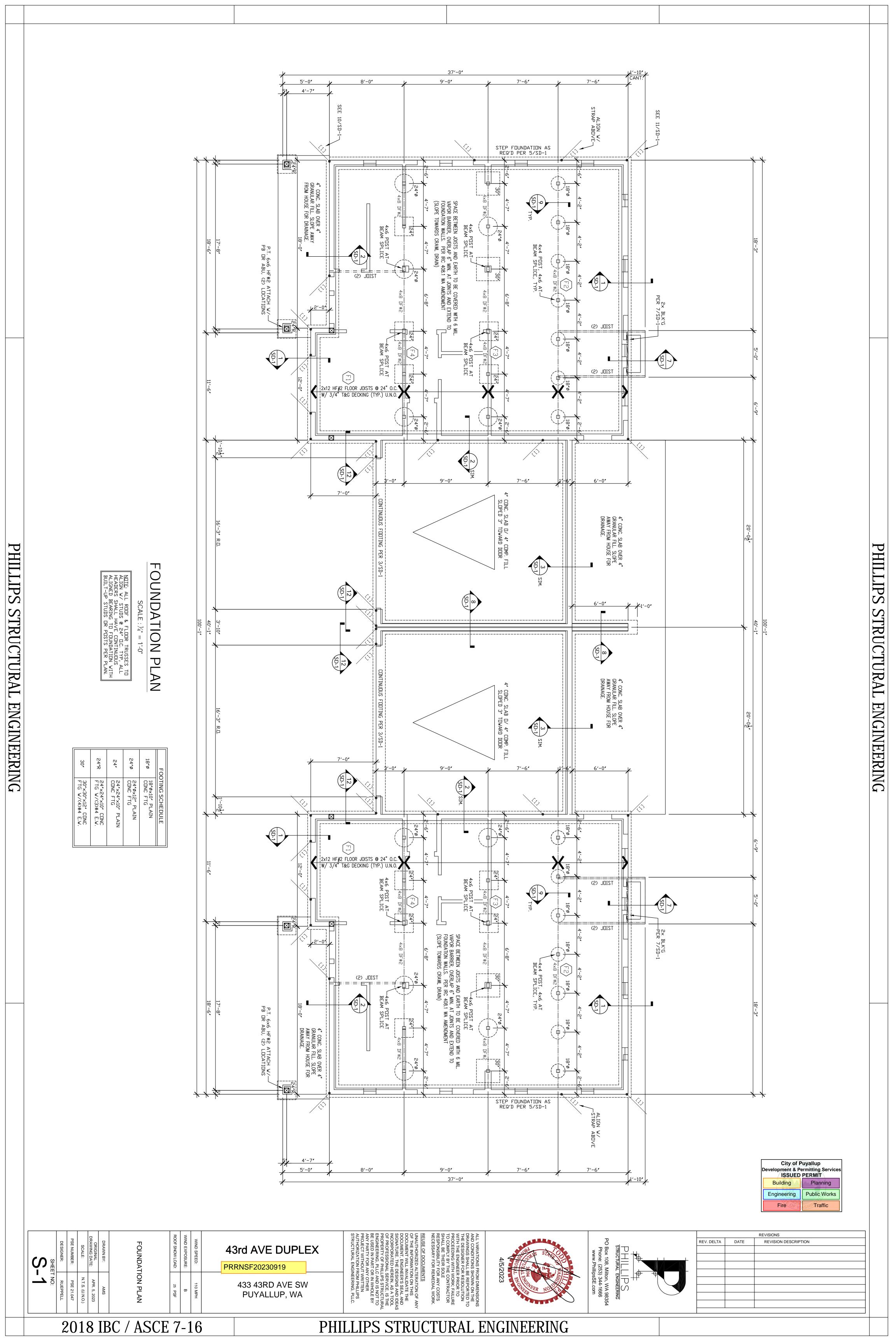
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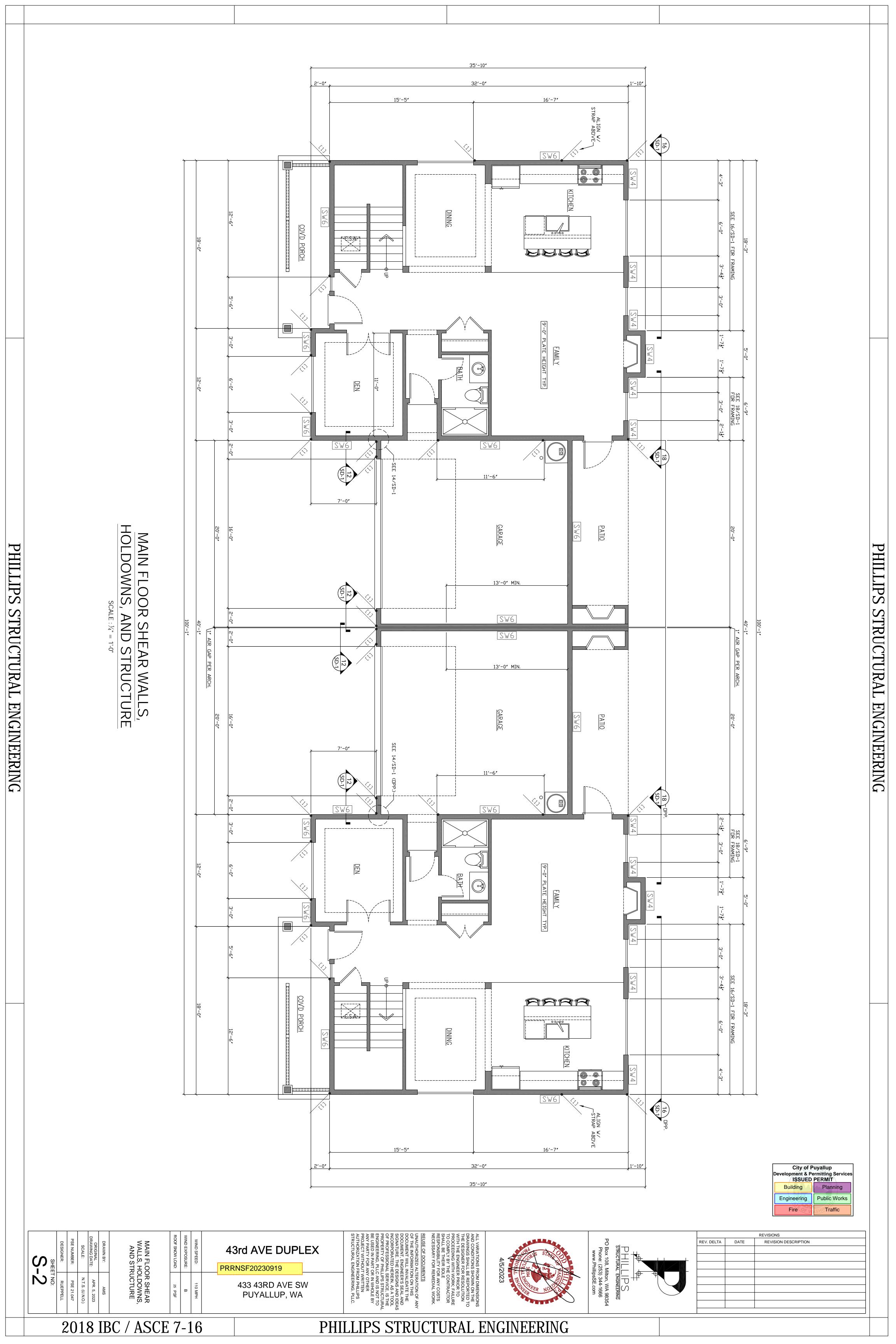
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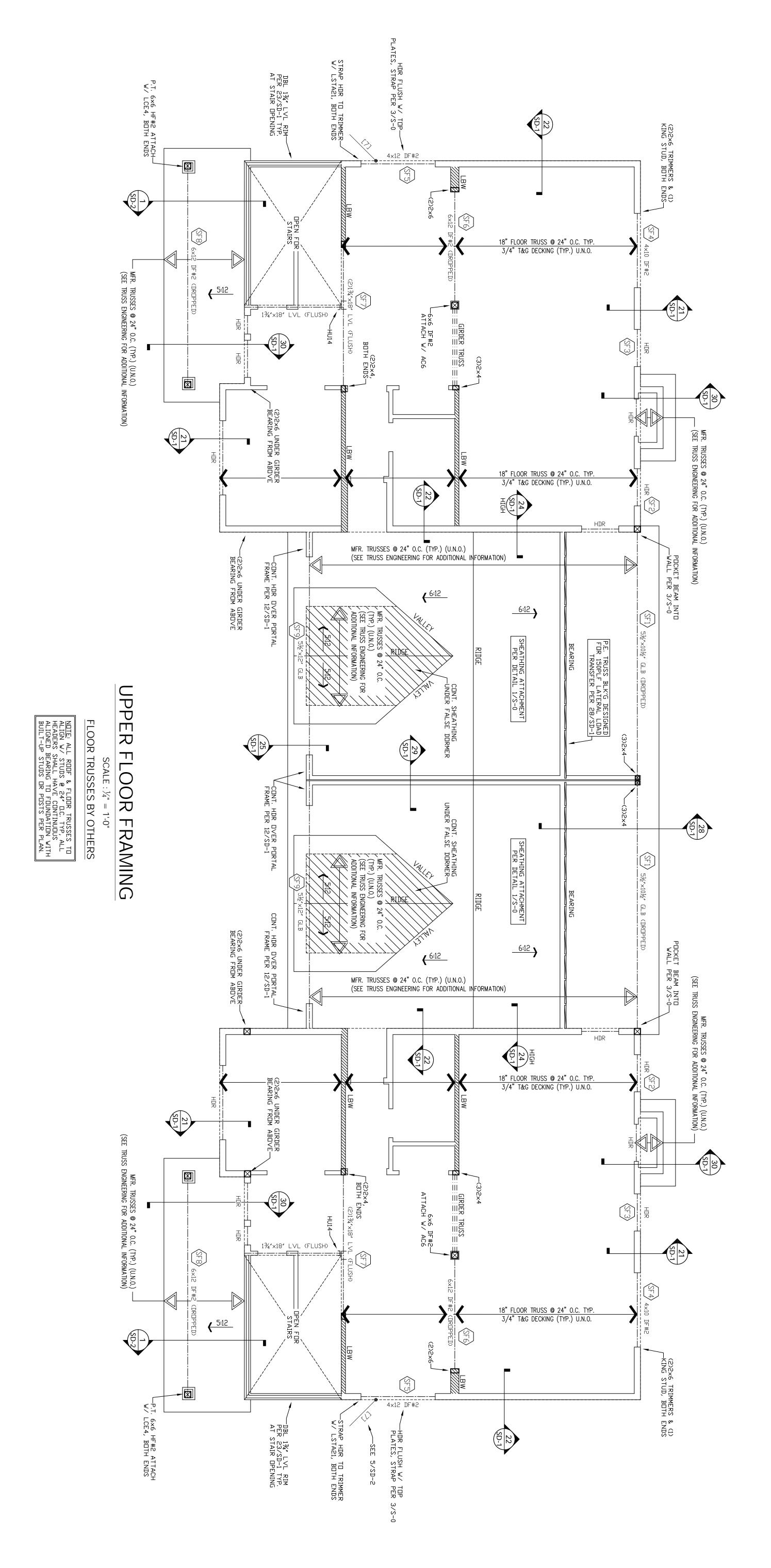
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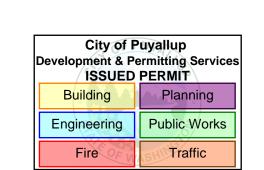
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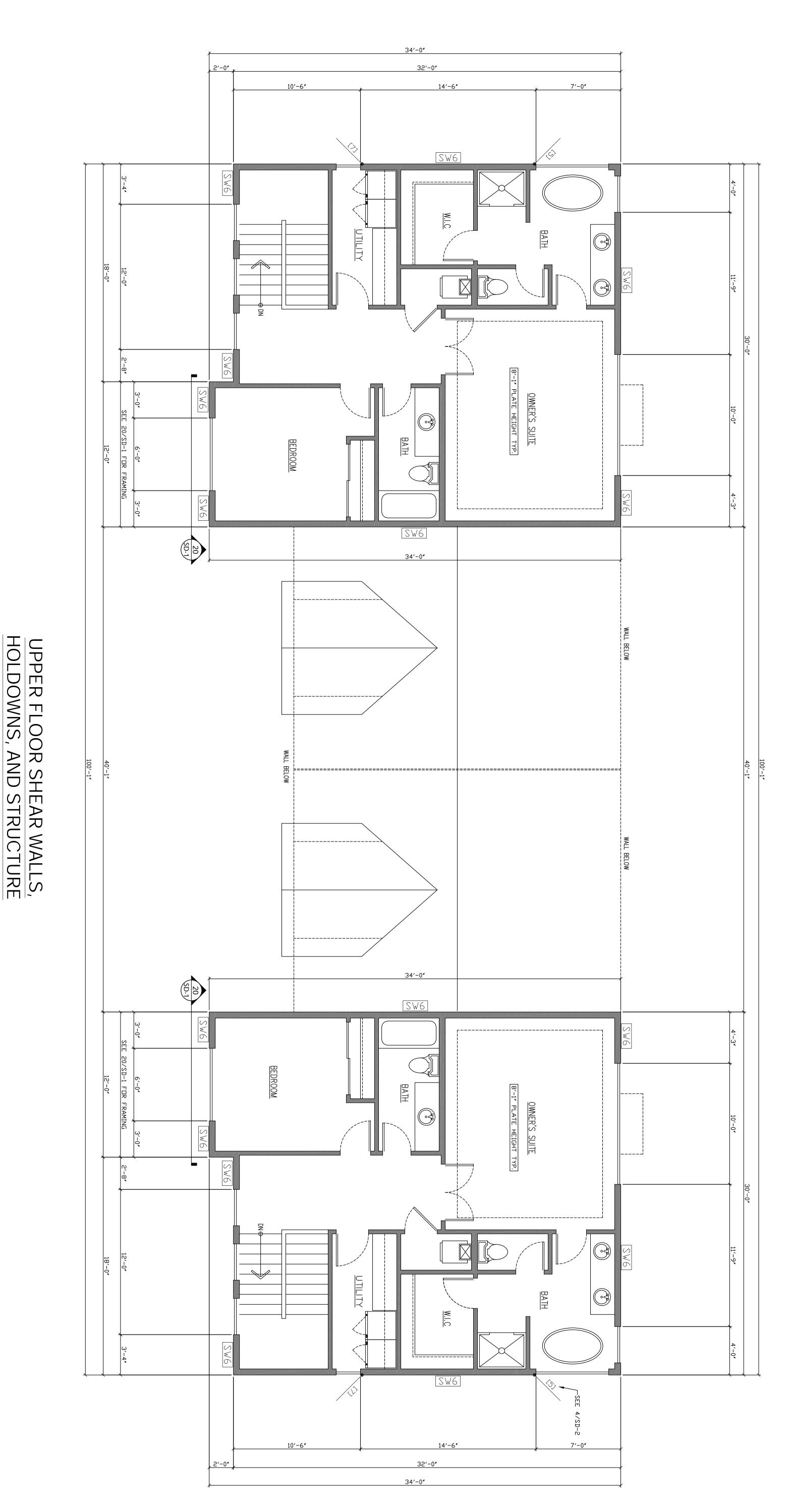






ALL VARIATIONS FROM DIMENSIONS AND CONDITIONS SHOWN ON THE DESIGNATIONS SHOWN ON THE DESIGNAS SHALL BE REPORTED TO THE DESIGNAR FOR RESOLUTION WITH THE BENGINEER PROPERTY OF PROCEEDING WITH WORK; FAILURE TO COMPLY BY THE CONTRACTOR SHALL BE THEIR SOLE. RESPONSIBILITY FOR ANY COST'S NECESSARY FOR REMEDIAL WORK; FAILURE TO COMPLY BY THE CONTRACTOR SHALL BE THEIR SOLE. RESPONSIBILITY FOR PREMEDIAL WORK; FAILURE THE DESIGNAS AND DEAS INCORPORATE HEREIN, AS A TOOL OF PROFESSIONAL SERVICE, IS THE PROPERTY OF PHILLIPS STRUCTURAL ENGINEER SEALAND DISAS IN PART OR IN WHOLE BY ANY PARTY FOR ANY OTHER PROPERTY OF PHILLIPS STRUCTURAL ENGINEER PROPERTY OF PHILLIPS STRUCTURAL ENGINEER OF ANY PARTY FOR ANY OTHER PROPERTY OF PHILLIPS STRUCTURAL ENGINEER OF ANY PARTY FOR ANY OTHER PROPERTY OF PHILLIPS STRUCTURAL ENGINEER OF ANY PARTY FOR ANY OTHER PROPERTY OF PHILLIPS STRUCTURAL ENGINEERING, PLLC. UNIND SPEED: U	PO Box 108, Milton, WA 98354 Phone: (253) 344-1666 www.PhillipsSE.com	REVISIONS REV. DELTA DATE REVISION DESCRIPTION
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2018 IBC / ASCE 7-16 PHILLIPS STRUCTURAL ENGINEERING



City of Puyallup
Development & Permitting Services
ISSUED PERMIT
Building Planning
Engineering Public Works
Fire Traffic

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AMS T.S. (U.N.O.) PSE 21.047 RUEPPELL O:	OOR (LLS, AND JRE	110 MPH B B 25 PSF	433 43RD AVE SW PUYALLUP, WA	DIMENSIONS WN ON THE EPORTED TO SOLUTION RIOR TO RK; FAILURE NTRACTOR NY COSTS DIAL WORK. DIAL WORK. DATE THE S SEAL AND NS AND IDEAS I, AS A TOOL VICE, IS THE STRUCTURAL D IS NOT TO WHOLE BY THEN THEN THEN THEN THEN THEN THEN THEN	CHITTER NO. 2	OS INEERING INA 98354 IA-1666 E.com		

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(2)2x6 TRIMMERS & (1) KING STUD, BOTH ENDS

SHEATHING ATTACHMENT PER DETAIL 1/S-0

MFR. TRUSSES @ 24" O.C. (TYP.) (U.N.O.)

(SEE TRUSS ENGINEERING FOR ADDITIONAL INFORMATION)

ATHING UNDER
E PER 3/SD-2

