1. STRUCTURAL NOTES

1.1. ANY DISCREPANCY FOUND AMONG THE DRAWINGS, SPECIFICATIONS, THESE NOTES, AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT AND THE STRUCTURAL ENGINEER, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S RISK. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION BRACING, FORMWORK AND TEMPORARY CONSTRUCTION SHORING.

1.2. BY THE ACT OF SUBMITTING A BID FOR THE PROPOSED CONTRACT, THE CONTRACTOR WARRANTS THAT:

1.2.1. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE (INCLUDING AGENTS AND SUPPLIERS) HAVE CAREFULLY AND THOROUGHLY REVIEWED THE DRAWINGS AND STRUCTURAL NOTES AND HAVE FOUND THEM COMPLETE AND FREE FROM AMBIGUITIES AND SUFFICIENT FOR THE PURPOSE INTENDED.

1.2.2. THE CONTRACTOR HAS CAREFULLY EXAMINED THE SITE OF THE WORK AND FROM THEIR OWN INVESTIGATIONS, THEY HAVE SATISFIED THEMSELF AS TO THE NATURE AND LOCATION OF THE WORK, AS TO THE CHARACTER, QUALITY, AND QUANTITIES OF MATERIAL AND DIFFICULTIES TO BE ENCOUNTERED, AS TO THE EXTENT OF EQUIPMENT AND OTHER FACILITIES NEEDED FOR THE PERFORMANCE OF THE WORK AND AS TO THE GENERAL AND LOCAL CONDITIONS, AND OTHER ITEMS WHICH MAY IN ANY WAY AFFECT THE WORK OR ITS PERFORMANCE.

1.2.3. THE CONTRACTOR AND ALL WORKERS THEY INTEND TO USE ARE SKILLED AND EXPERIENCED IN THE TYPE OF CONSTRUCTION REPRESENTED BY THE DRAWINGS AND DOCUMENTS BID UPON.

1.2.4. NEITHER THE CONTRACTOR NOR ANY OF THEIR EMPLOYEES, AGENTS, INTENDED SUPPLIERS, OR SUBCONTRACTORS HAVE RELIED UPON ANY VERBAL REPRESENTATIONS ALLEGEDLY AUTHORIZED OR UNAUTHORIZED FROM THE OWNER OR THEIR EMPLOYEES OR AGENTS, INCLUDING THE ARCHITECT OR ENGINEERS, IN ASSEMBLING THE BID FIGURES.

1.2.5. THE REQUIREMENTS CONTAINED WITHIN THIS SECTION SUPERSEDE REQUIREMENTS AND/OR RECOMMENDATIONS CONTAINED IN THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES", AS WELL AS CASE DOCUMENT 962-D "A GUIDELINE ADDRESSING COORDINATION AND COMPLETENESS OF STRUCTURAL CONSTRUCTION DOCUMENTS"

1.4. DESIGN CRITERIA

1.4.1. UNIFORM LOADS:

LOCATION	
ROOF SOLAR READINESS ZONE	
RESIDENTIAL (PRIVATE ROOMS AND C	
RESIDENTIAL (PUBLIC ROOMS ANDCOR	
STAIRS AND EXITS	
DECKS AND BALCONIES [1.5X OCCUPANCY SERV	
MECHANICAL ROOMS	
STORAGE	
PARKING GARAGE (PASSENGER VEHICLES)	
HANDRAILS AND GUARDS	
* THIS IS NOT A GROUND	
** SOLAR READINESS ZO COMMERCIAL PROVISION	
WHERE LIVE LOADS OF C EXCEED 50 PSF, SUCH D PART OF EACH STORY IN	

1.4.2. SNOW LOADS PER IBC SE

GROUND SNOW LOA

FLAT ROOF SNOW L

SNOW EXPOSURE F

3.13.3. ELECTRICAL CONDUIT AND PIPES EMBEDDED WITHIN THE POST TENSIONED SLAB SHALL SATISFY THE FOLLOWING REQUIREMENTS:

A. CONDUIT AND PIPES SHALL NOT BE LARGER THAN ONE THIRD THE OVERALL THICKNESS OF THE SLAB IN WHICH THEY ARE EMBEDDED.

B. CONDUIT AND PIPES SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. AT ELECTRICAL ROOMS OR CONGESTED AREAS WHERE MINIMUM SPACING CANNOT BE ACHIEVED, PROVIDE ADDITIONAL #5 REINFORCEMENT AT 12" OC EACH WAY EXTENDING A MINIMUM OF TWO FEET BEYOND CONGESTION. MORE THAN ONE LAYER OF CONDUIT IS NOT PERMITTED UNLESS APPROVED BY THE ENGINEER OF RECORD.

C. CONDUIT AND PIPES SHALL NOT BE PLACED WITHIN 2'-0" OF A COLUMN CAP, WITHIN 1'-6" OF A TENDON ANCHOR, OR WITHIN 2" OF A TENDON.

D. CONDUIT AND PIPES SHALL NOT BE RUSTING OR HAVE OTHER DETERIORATION.

E. CONDUIT AND PIPES SHALL BE UNCOATED OR GALVANIZED IRON OR STEEL, NOT THINNER THAN STANDARD SCHEDULE 40 STEEL PIPE.

3.14. GROUT FOR BEARING PLATES

THE NON-SHRINK GROUT SHALL MEET ASTM C1107 GRADE B OR EQUIVALENT (MASTERFLOW 928 BY BASF OR APPROVED EQUIVALENT). GROUT SHALL BE A PRE-PACKAGED HYDRAULIC CEMENT BASED MINERAL AGGREGATE GROUT, MIXED, PLACED AND CURED AS RECOMMENDED BY THE MANUFACTURER. COMPRESSIVE STRENGTH SHALL EXCEED 6000 PSI AT 28 DAYS.

3.15. SHOTCRETE

3.15.1. SHOTCRETE SHALL BE DEFINED AS MORTAR OR CONCRETE PNEUMATICALLY PROJECTED AT HIGH VELOCITY ONTO A SURFACE. EXCEPT AS SPECIFIED IN THIS SECTION, SHOTCRETE SHALL CONFORM TO THE REQUIREMENTS FOR PLAIN CONCRETE OR REINFORCED CONCRETE.

3.15.2. PROPORTIONS AND MATERIALS: SHOTCRETE PROPORTIONS SHALL BE SELECTED THAT ALLOW SUITABLE PLACEMENT PROCEDURES USING THE DELIVERY EQUIPMENT SELECTED AND SHALL RESULT IN FINISHED IN-PLACE HARDENED SHOTCRETE MEETING THE SPECIFIED STRENGTH REQUIREMENTS.

3.15.3. AGGREGATE: COARSE AGGREGATE, IF USED, SHALL NOT EXCEED 3/4 INCH

3.15.11. INSPECTIONS

A. DURING PLACEMENT
STRUCTURAL MEMB
IBC TABLE 1705.3. TH
CONTINUOUS INSPE
REINFORCEMENT AN
STATEMENT INDICAT
SPECIFICATIONS.

B. VISUAL EXAMINATIO
PLACE SHOTCRETE.
CHECKED VISUALLY
ROCK POCKETS, SA
EXAMINING A MINIM
CHOSEN BY THE DE
WORST CONGESTIO
THE PROJECT. EXTI
NON-CONGESTED A
CONGESTED AREAS
SHALL BE EXAMINED
SUBMITTED TO THE
APPROVAL OF THE S

C. TESTING EQUIPMEN
CONSTRUCTION TES
IN THE WORK REQU
EQUIPMENT IS APPR
BUILDING OFFICIAL.

3.16. ADHESIVE EXPANSIVE WATERSTOP

ADHESIVE EXPANSIVE WATERSTOP
MANUFACTURED BY CETCO), SWEL
OR APPROVED EQUIVALENT. INSTA
RECOMMENDATIONS.

3.17. CONCRETE COORDINATION DRAWI

PRIOR TO THE START OF CONCRET
THE CONTRACTOR SHALL SUBMIT
THE ARCHITECT/ENGINEER FOR RE
DRAWINGS SHALL INCLUDE DIMEN
DOOR AND WINDOW OPENINGS, MI
APPROPRIATE ITEMS.

4. MASONRY

11. STATEMENT OF SPECIAL INSPECTIONS

IBC	SI	SO	TITLE
1705.2	✓	✓	STEEL CONSTRUCTION (SEE TABLES 15A, 15B, 15C, AND 15D)
1705.3	✓	✓	CONCRETE CONSTRUCTION (SEE TABLE 13)
1705.4	✓	✓	MASONRY CONSTRUCTION (SEE TABLES 14A, 14B, 14C, 14D & 14E)
1705.6	✓	N/R	SOILS (SEE TABLE 12A)
1705.12.2	✓	✓	STRUCTURAL WOOD - SEISMIC FORCE RESISTING SYSTEM (SEE TABLE 18)

SI = SPECIAL INSPECTION

SO = STRUCTURAL OBSERVATION

✓ = ITEM IS REQUIRED

N/R = ITEM IS NOT REQUIRED

SPECIAL INSPECTIONS INDICATED ARE FOR STRUCTURAL ELEMENTS ONLY. SEE ARCH, MECH AND ELEC DRAWINGS FOR ADDITIONAL SPECIAL INSPECTIONS.

11.

11.1. INSPECTION/TESTING REQUIREMENTS:

SEE DRAWINGS, SPECIFICATIONS, AND IBC SECTIONS 110, AND CHAPTER 17.

11.2. INSPECTIONS BY THE BUILDING OFFICIAL (IBC SECTION 110):

11.2.1. FOOTING AND FOUNDATION INSPECTIONS SHALL BE MADE AFTER EXCAVATIONS ARE COMPLETE AND ANY REQUIRED REINFORCING IS IN PLACE. ANY REQUIRED FORMS SHALL BE IN PLACE PRIOR TO INSPECTION.

11.2.2. CONCRETE SLAB AND UNDER FLOOR INSPECTIONS SHALL BE MADE AFTER ALL IN SLAB OR UNDER FLOOR REINFORCING, CONDUIT, PIPING AND OTHER ANCILLARY EQUIPMENT ITEMS AND ACCESSORIES ARE IN PLACE BUT PRIOR TO CONCRETE PLACEMENT OR FLOOR SHEATHING INSTALLATION.

11.2.3. FRAMING INSPECTIONS SHALL BE MADE AFTER ALL SHEATHING, FRAMING, BLOCKING AND BRACING ARE COMPLETE AND ALL PIPES, DUCTS, ELECTRICAL, PLUMBING, ETC., ARE INSTALLED AND APPROVED PRIOR TO COVER.

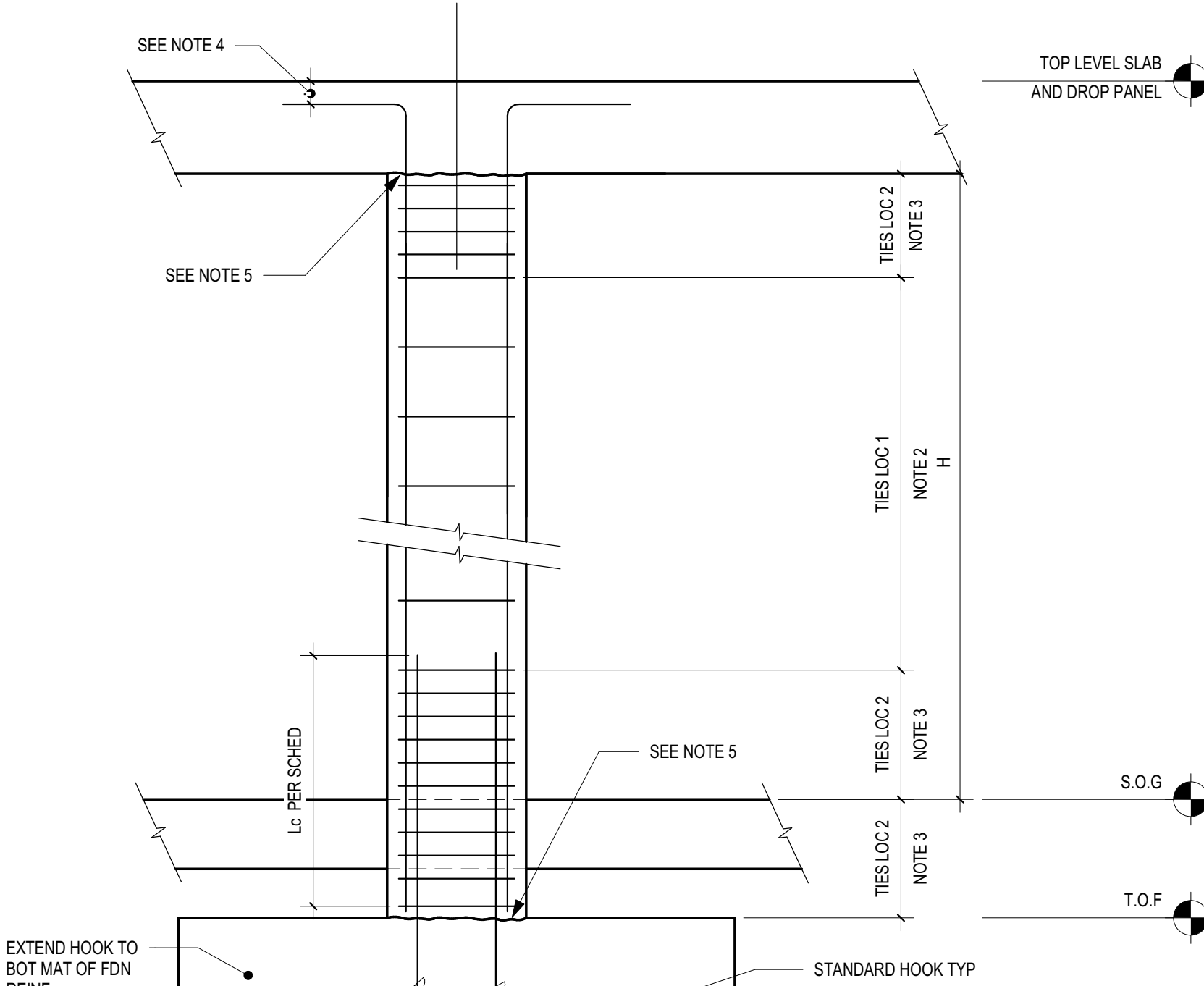
11.2.4. IN ADDITION TO THE INSPECTIONS SPECIFIED ABOVE, THE BUILDING OFFICIAL IS AUTHORIZED TO MAKE OR REQUIRE OTHER INSPECTIONS OF ANY CONSTRUCTION WORK TO ASCERTAIN COMPLIANCE WITH THE PROVISIONS OF THE IBC OR OTHER LAWS ENFORCED BY THE BUILDING OFFICIAL.

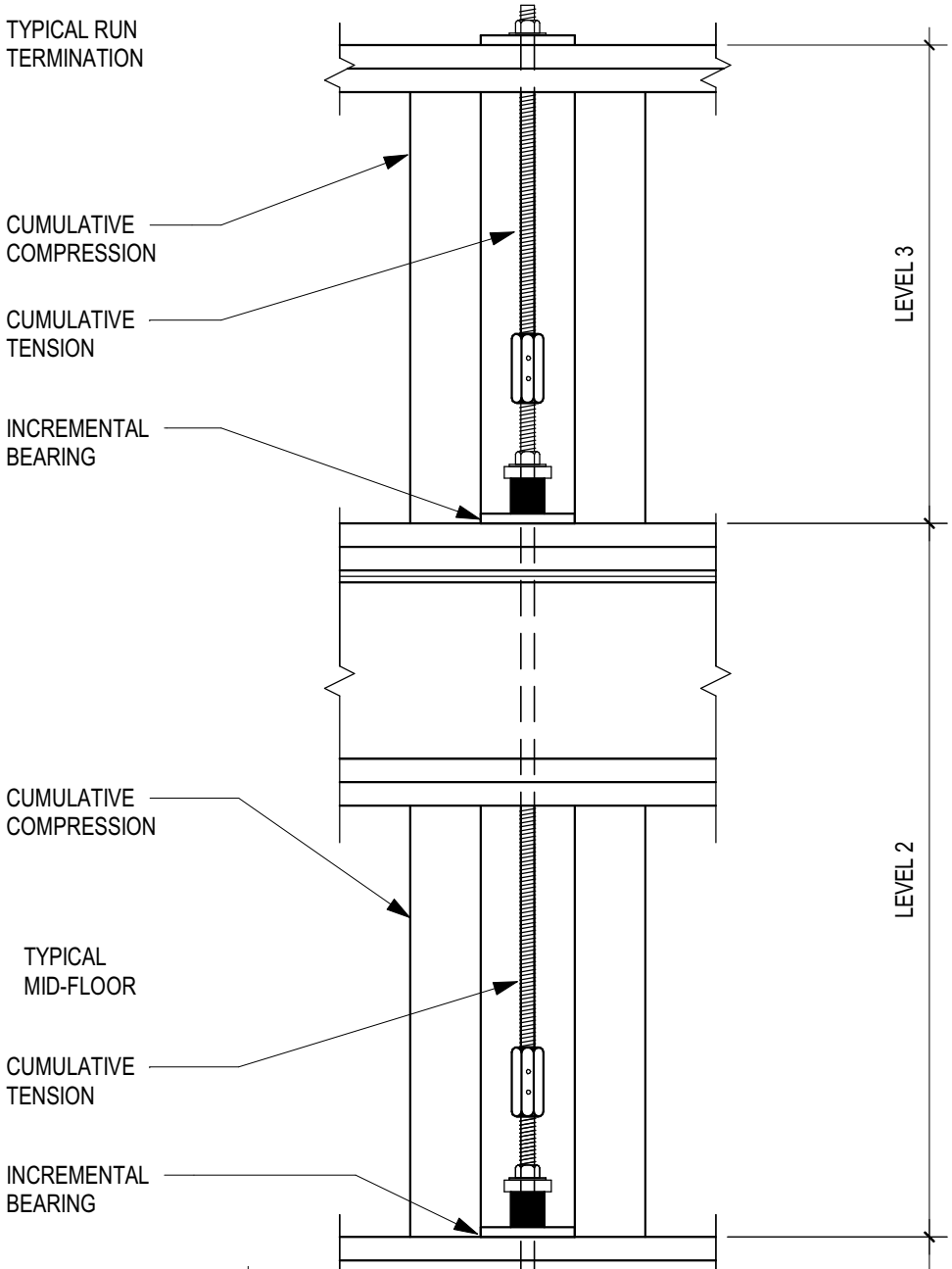
13. REQUI
CONS

		SPE
1.		INSPI PRES PLAC
2.		REINI
	A.	VERI OTHE
	B.	INSPI 5/16"
	C.	INSPI
3.		INSPI
4.		INSPI HARD
	A.	ADHE HORI ORIE LOAD
	B.	MECH NOT
5.		VERI
6.		PRIO SPEC SLUM DETE CONC
7.		INSPI PLAC TECH
8.		VERI TEMP

15.B REQUIRED SPECIAL INSPECTION AND TESTS OF STRUCTURAL STEEL CONSTRUCTION – INSPECTION OF BOLTING

SPECIAL INSPECTION OR TEST TYPE			CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
AISC 360 TABLE N5.6-1					
1.		PRIOR TO BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
	A.	MANUFACTURER'S CERTIFICATIONS FOR FASTENER MATERIALS	✓	N/R	
	B.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	N/R	✓	
	C.	PROPER FASTENER SELECTED FOR JOINT DETAIL	N/R	✓	AISC 360 A3.1
	D.	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	N/R	✓	
	E.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITIONS AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	N/R	✓	
	F.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	✓	N/R	
	G.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, AND OTHER FASTENER COMPONENTS	N/R	✓	
AISC 360 TABLE N5.6-2					
2.		DURING BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
	A.	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	N/R	✓	
	B.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	N/R	✓	
	C.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	N/R	✓	
	D.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	N/R	✓	
AISC 360 TABLE N5.6-3					
3.		AFTER BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
	A.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	✓	N/R	


SEE NOTE 4
TOP LEVEL SLAB
AND DROP PANEL
SEE NOTE 5
TIES LOC 2
NOTE 3
TIES LOC 1
NOTE 2
H
Lc PER SCHED
SEE NOTE 5
TIES LOC 2
NOTE 3
S.O.G
TIES LOC 2
NOTE 3
T.O.F
EXTEND HOOK TO
BOT MAT OF FDN
STANDARD HOOK TYP



1A. PROJECT DETAILS

FLOOR CONSTRUCTION	SHRIN
WOOD TRUSS FLOOR	0.5

ANCHOR TIEDOWN SYSTEM GENERAL NOTES:

1 SIMPSON STRONG-TIE SHALL PROVIDE THE ANCHOR TIED
FORCES AND ELONGATION LIMITS PROVIDED IN THE SIMP
TABLE AND ATS DETAILS PROVIDED ON THE STRUCTURAL
CALCULATIONS SHALL BE PROVIDED FOR REVIEW AND AP

2 SHEAR WALLS SHALL BE SUPPORTED WITH A BEARING PL
LEVEL. SKIPPING SHEAR WALL OVERTURNING RESTRAINT

3 SHRINKAGE COMPENSATION DEVICES SHALL BE USED TO
EACH LEVEL INDICATED IN THE PROJECT DETAILS TABLE.

4 ANCHOR BOLTS SHALL NOT BE IN CONTACT WITH PRESSU
PLATES SHALL HAVE OVERSIZE HOLES ¼ INCH MINIMUM A
ROD SIZE. AS AN ALTERNATE, THE ANCHOR SHALL BE GAL
ASTM A653.

5 DO NOT WELD PRODUCTS UNLESS THESE DRAWINGS SPE
DO NOT WELD PRODUCTS UNLESS THESE DRAWINGS SPE
SIMPSON STRONG-TIE. SOME STEELS HAVE POOR WELDA
WHEN WELDED. CRACKED STEEL WILL NOT CARRY LOAD
COUPLER SHALL NOT BE WELDED.

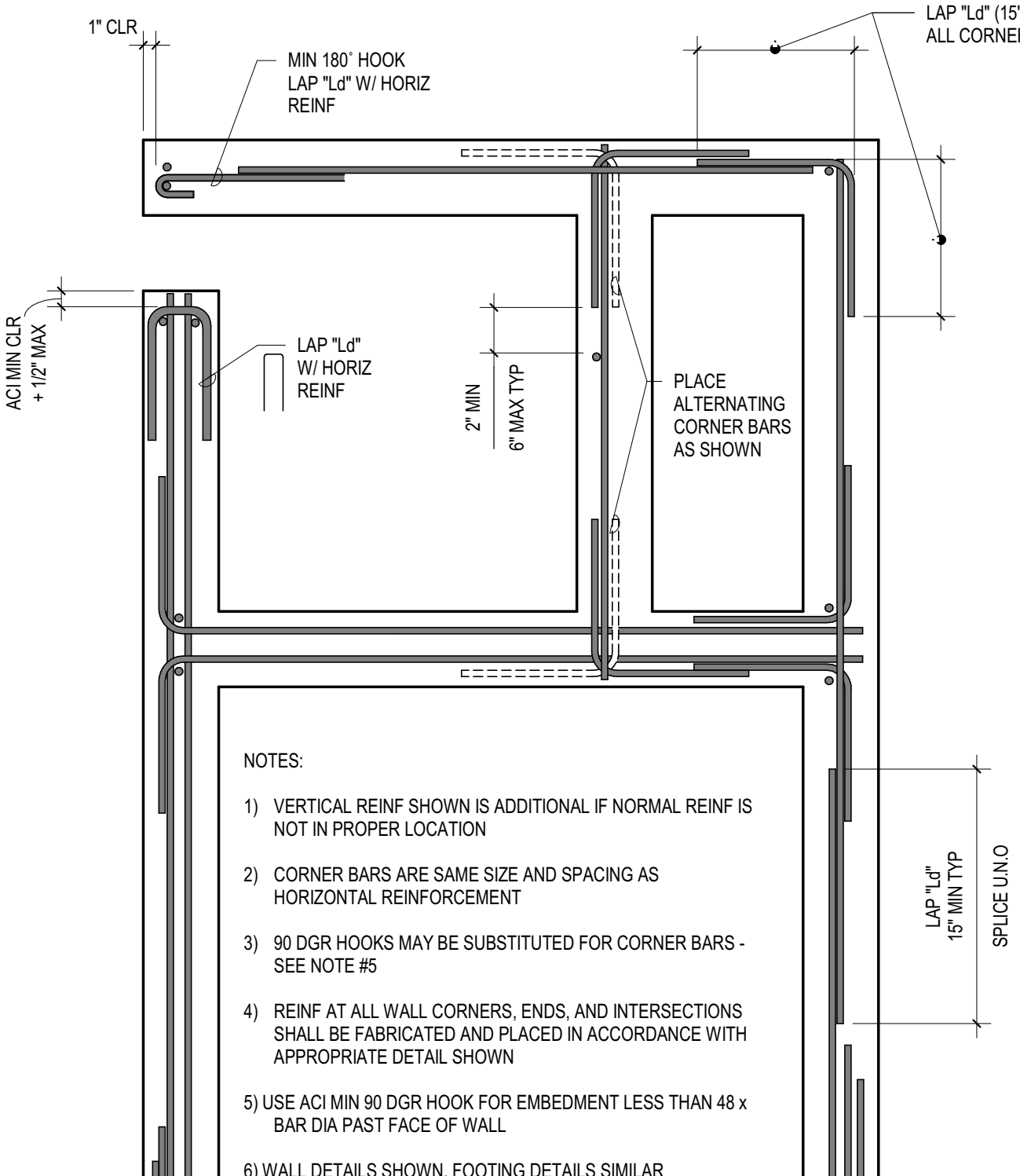
6 IN THE EVENT OF A DISCREPANCY BETWEEN THESE STRU
DRAWINGS, THE STRUCTURAL DRAWINGS ALWAYS GOVE

7 THESE DRAWINGS ARE SPECIFIC TO ATS AND ARE NOT AP
MANUFACTURER TIEDOWN SYSTEMS. CONTRACTOR'S PR
MANUFACTURER'S CONNECTORS SHALL BE SUBMITTED T
BUILDING JURISDICTION FOR REVIEW AND WRITTEN APPR
EXPENSE OF THE CONTRACTOR. REQUESTS FOR SUBSTI
ICC-ES EVALUATION REPORTS AND A LIST STATING THE P

WOOD STUD SHEARWALL SCHED

MARK	SHEATHING	NAILING		STUD SIZE AT ADJOINING PANEL EDGES	BLOCKING SIZE	FOUNDATION SILL PL ATTACHMENT	2x A
		SIZE	SPACING				
W 6	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	6" OC EDGES 12" OC FIELD	2x	2x FLAT OR 2x	3/4" DIA. AT 48" OC	
W 4	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	4" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 48" OC	
W 3	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	3" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 32" OC	
W 2	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	2" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 16" OC	
2W 6	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	6" OC EDGES 12" OC FIELD	2x	2x FLAT OR 2x	3/4" DIA. AT 32" OC	
2W 4	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	4" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 16" OC	
2W 3	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	3" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 16" OC	
2W 2	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	2" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 8" OC	

APA RATED SHEATHING SHEARWALL NOTES


1" CLR
MIN 180° HOOK
LAP "Ld" W/ HORIZ
REINF
LAP "Ld" (15" MIN) TYPICAL
ALL CORNER BARS
ACI MIN CLR
+ 1/2" MAX
LAP "Ld"
W/ HORIZ
REINF
2" MIN
6" MAX TYP
PLACE
ALTERNATING
CORNER BARS
AS SHOWN
NOTES:
1) VERTICAL REINF SHOWN IS ADDITIONAL IF NORMAL REINF IS NOT IN PROPER LOCATION
2) CORNER BARS ARE SAME SIZE AND SPACING AS HORIZONTAL REINFORCEMENT
3) 90 DGR HOOKS MAY BE SUBSTITUTED FOR CORNER BARS - SEE NOTE #5
4) REINF AT ALL WALL CORNERS, ENDS, AND INTERSECTIONS SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH APPROPRIATE DETAIL SHOWN
5) USE ACI MIN 90 DGR HOOK FOR EMBEDMENT LESS THAN 48 x BAR DIA PAST FACE OF WALL
6) WALL DETAILS SHOWN, FOOTING DETAILS SIMILAR
LAP "Ld"
15" MIN TYP
SPLICE U.N.O

FOUNDATION NOTES

1. SEE SHEETS S0.1-B - S0.2-B FOR STRUCTURAL NOTES, SEE SHEET S0.8-B FOR TYPICAL DETAILS, AND SHEETS S0.3-B - S0.4-B FOR TESTING AND INSPECTION NOTES.

2. SEE SHEET S0.5-B FOR FOOTING SCHEDULE AND FOR CONCRETE COLUMN SCHEDULE.

3. SEE ARCHITECTURAL / MECJANICAL DRAWINGS FOR DRAINS, SLOPES, AND OTHER FLOOR DEPRESSIONS NPT SHOWN.

4. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, AND WALLS NOT SHOWN.

5. VERIFY ALL WINDOW AND DOOR WIDTHS AND HEIGHTS WITH ARCHITECTURAL DRAWINGS.

6. SEE ARCHITECTURAL DRAWINGS FOR STUD SIZE, SPACING, AND CALLOUTS AT NON-STRUCTURAL WALLS.

7. FOR TYPICAL CONNECTION OF NON-LOAD BEARING WALLS TO SLAB, USE POWDER ACTUATED FASTENERS AT 16" OC.

8. SEE GEOTECHNICAL ENGINEERING REPORT FOR ALL FOUNDATION AND SLAB SUPPORT REQUIREMENTS. THIS INCLUDES ALL EXCAVATION, FILL AND FILL PLACEMENT REQUIREMENTS.

FLOOR FRAMING NOTES - PT

1. SEE SHEETS S4.1-B - S4.2-B FOR TYPICAL F

2. VERIFY ALL TOP OF SLAB AND TOP OF WAL

3. VERIFY ALL DOOR AND WINDOW WIDTHS A

4. VERIFY SIZE AND LOCATION OF ALL MECHA MECHANICAL DRAWINGS. GC SHALL SUBM

5. TOP = TOP MAT, BOT = BOTTOM MAT, MID

6. ALL TENDON PROFILES NOTED ON THE PLA MID-SPAN TO THE CENTER OF STRAND.

7. CONTRACTOR SHALL VERIFY ALL DIMENSI WINDOW WIDTHS AND HEIGHTS, WITH ARC ANY DISCREPANCIES.

8. SEE DETAIL 1 / S0.6-B FOR STUDRAIL REQU

9. SEE DETAIL 3 / S0.5-B FOR REQUIRED LAP L LENGTHS.

10. SEE DETAIL 2 / S4.2-B FOR REQUIREMENTS

11. SEE DETAIL 7 / S4.1-B FOR TYPICAL PT TEN RELATIONSHIPS.

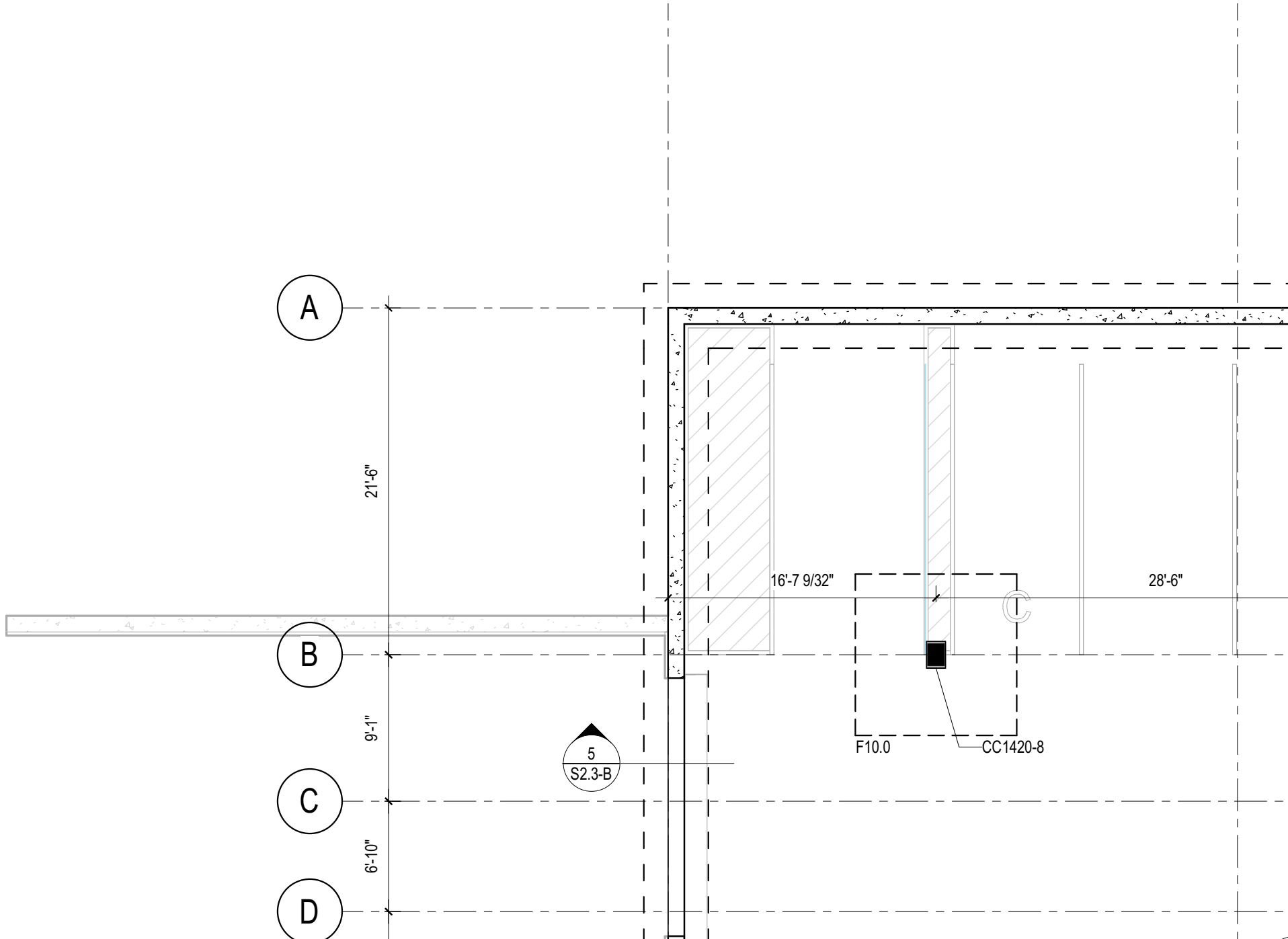
12. SEE DETAIL 5 / S4.2-B FOR METHOD OF MA

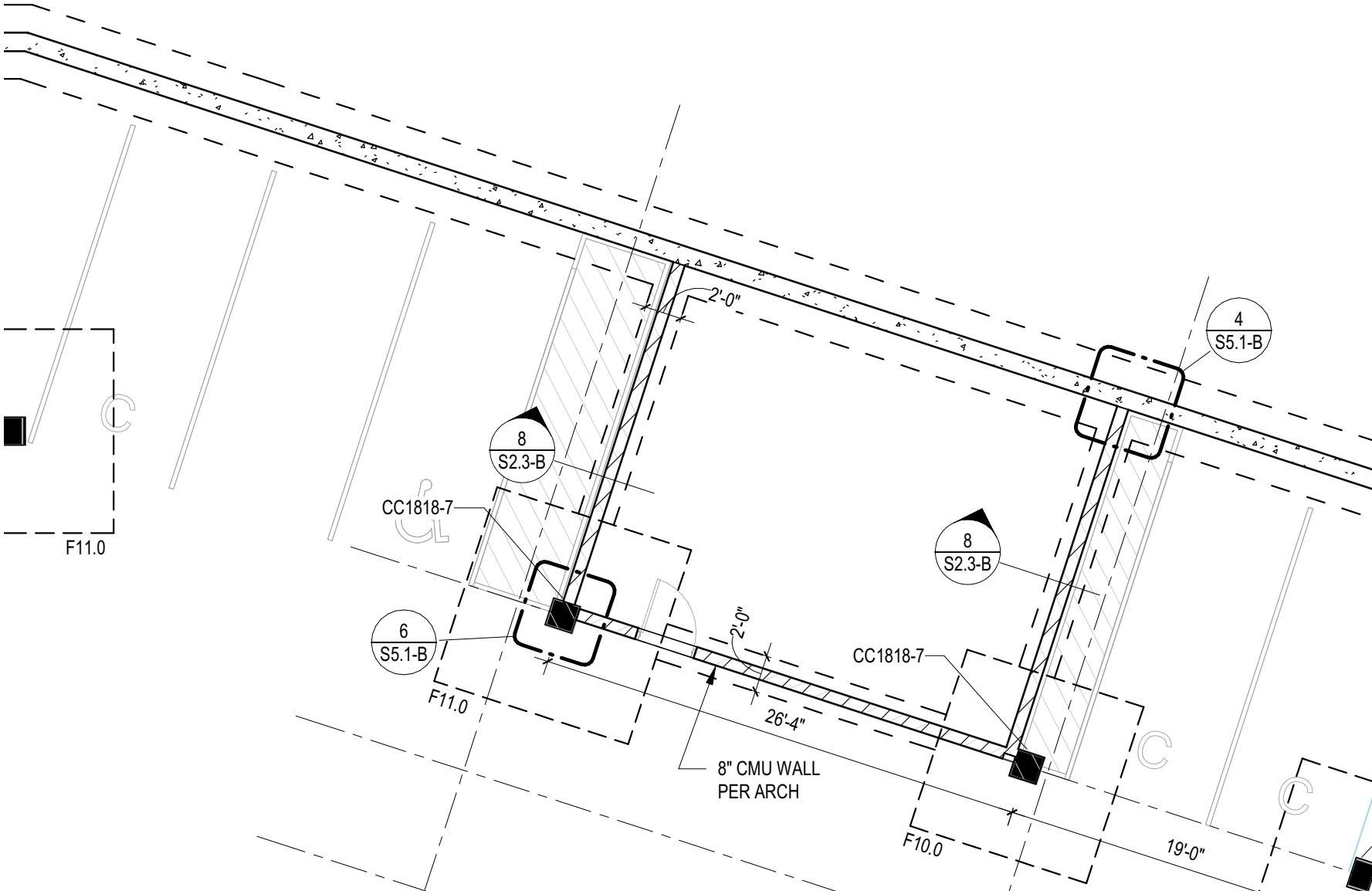
13. SEE SHEET 1 / S0.5-B FOR COLUMN TYPES

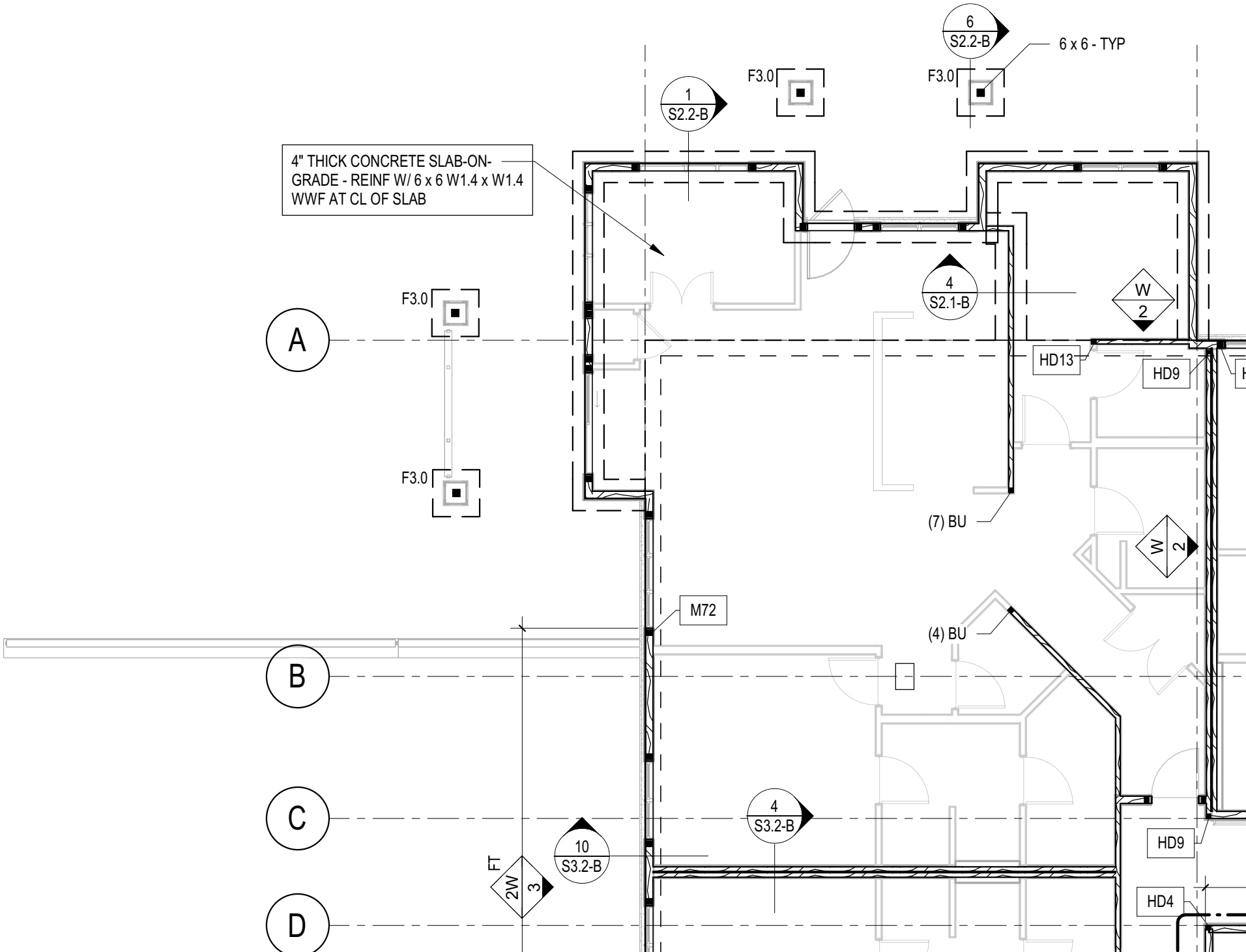
14. SEE DETAIL 1 / S4.1-B FOR PT ENCAPSULAT

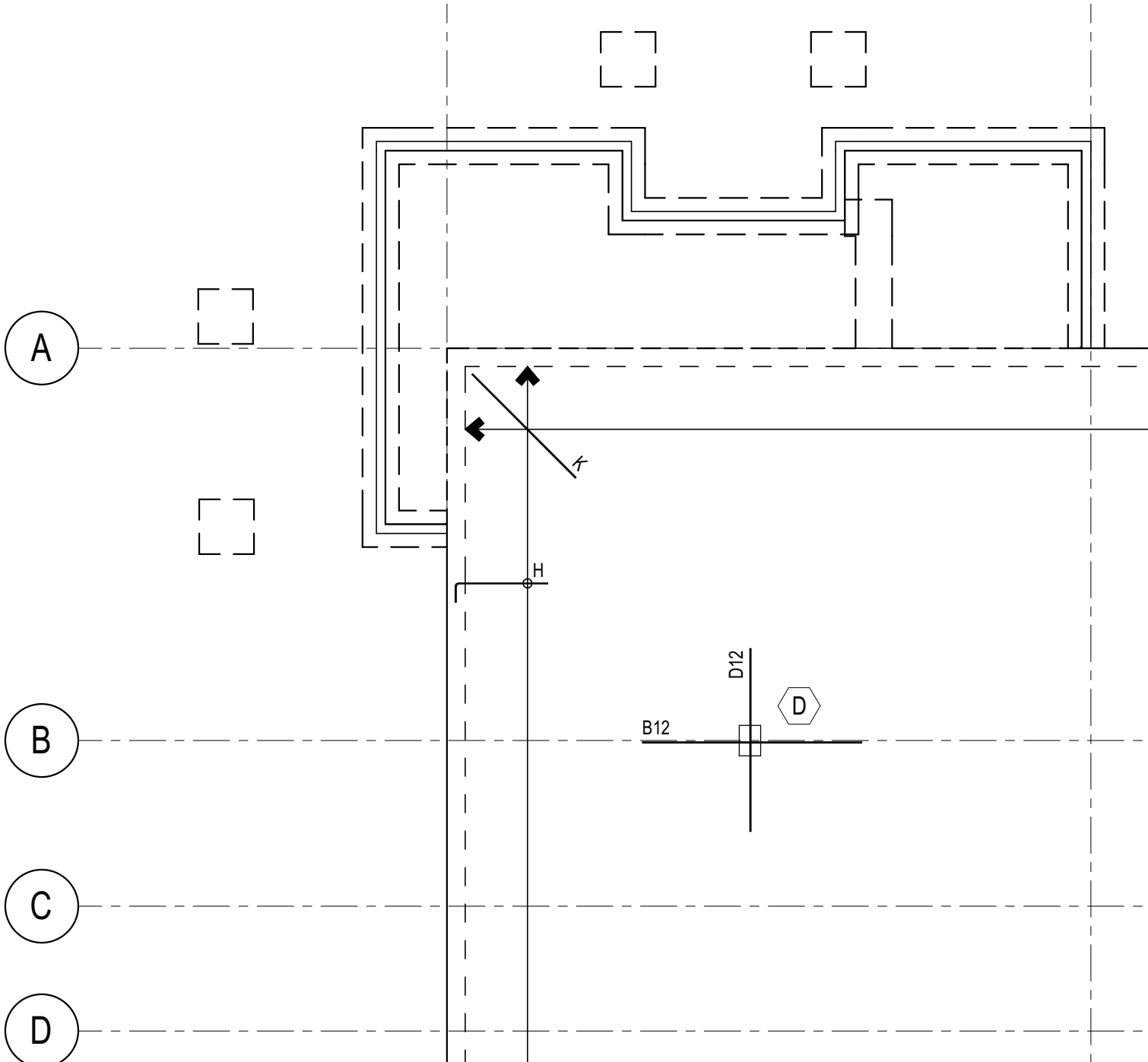
15. SEE DETAIL 4 / S4.2-B FOR PENETRATION R

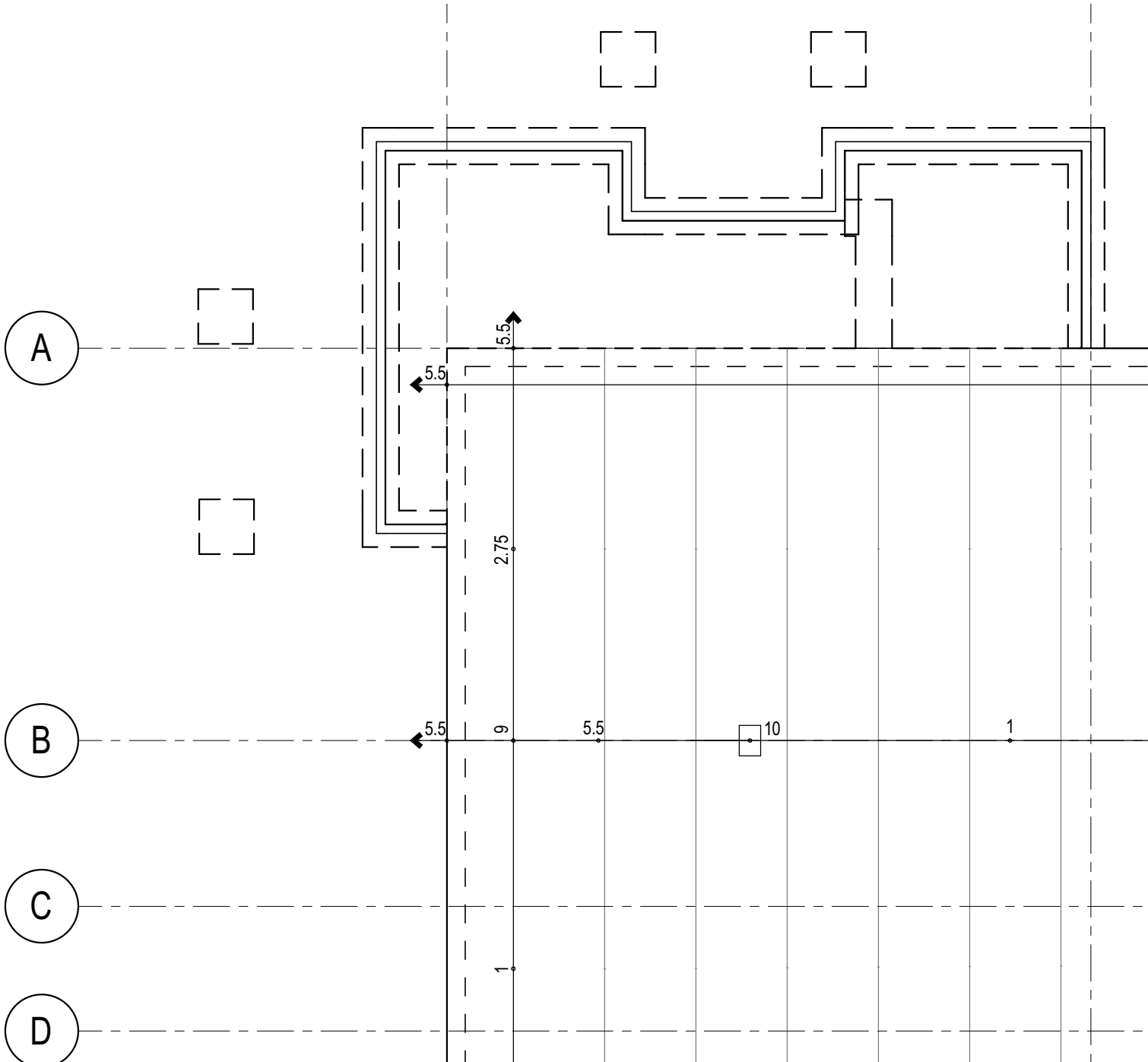
16. SEE DETAIL 3 / S4.2-B FOR HORIZONTAL AN ANCHORAGE.

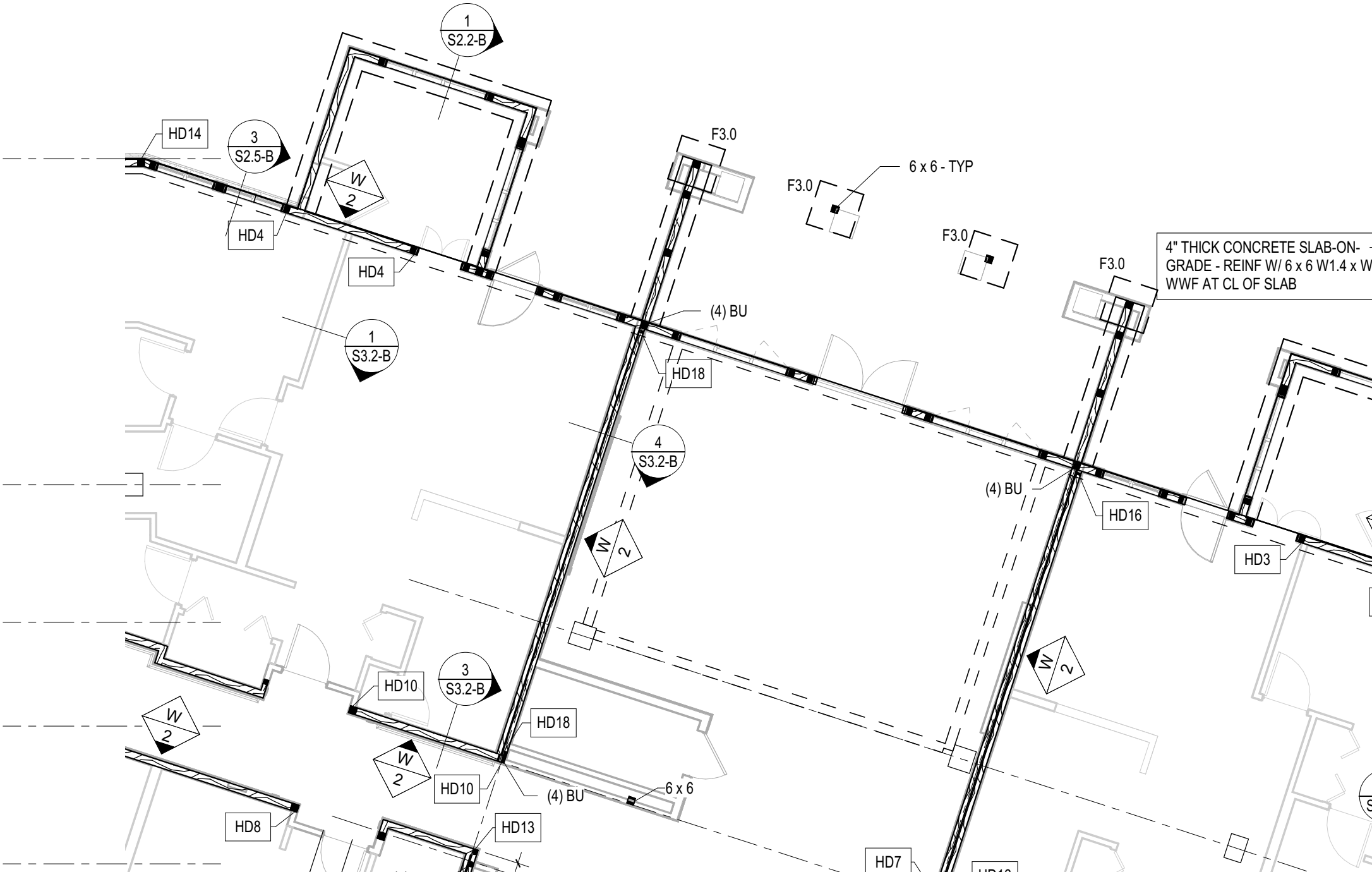

A
B
C
D
21'-6"
9'-1"
6'-10"
16'-7 9/32"
28'-6"
C
F10.0
CC1420-8
5
S2.3-B

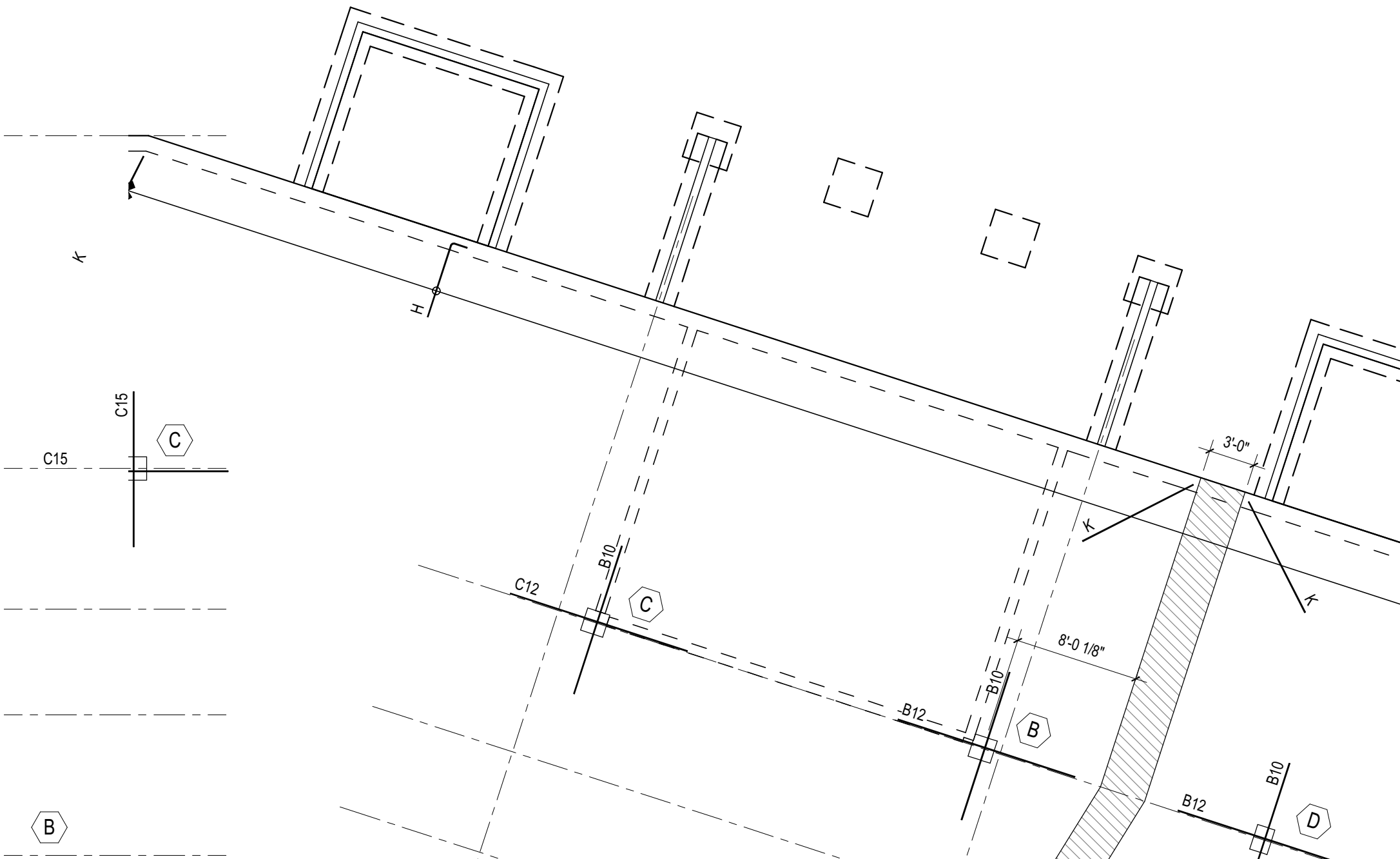

F11.0
8
S2.3-B
CC1818-7
6
S5.1-B
2'-0"
4
S5.1-B
8
S2.3-B
CC1818-7
2'-0"
F11.0
26'-4"
8" CMU WALL
PER ARCH
F10.0
19'-0"

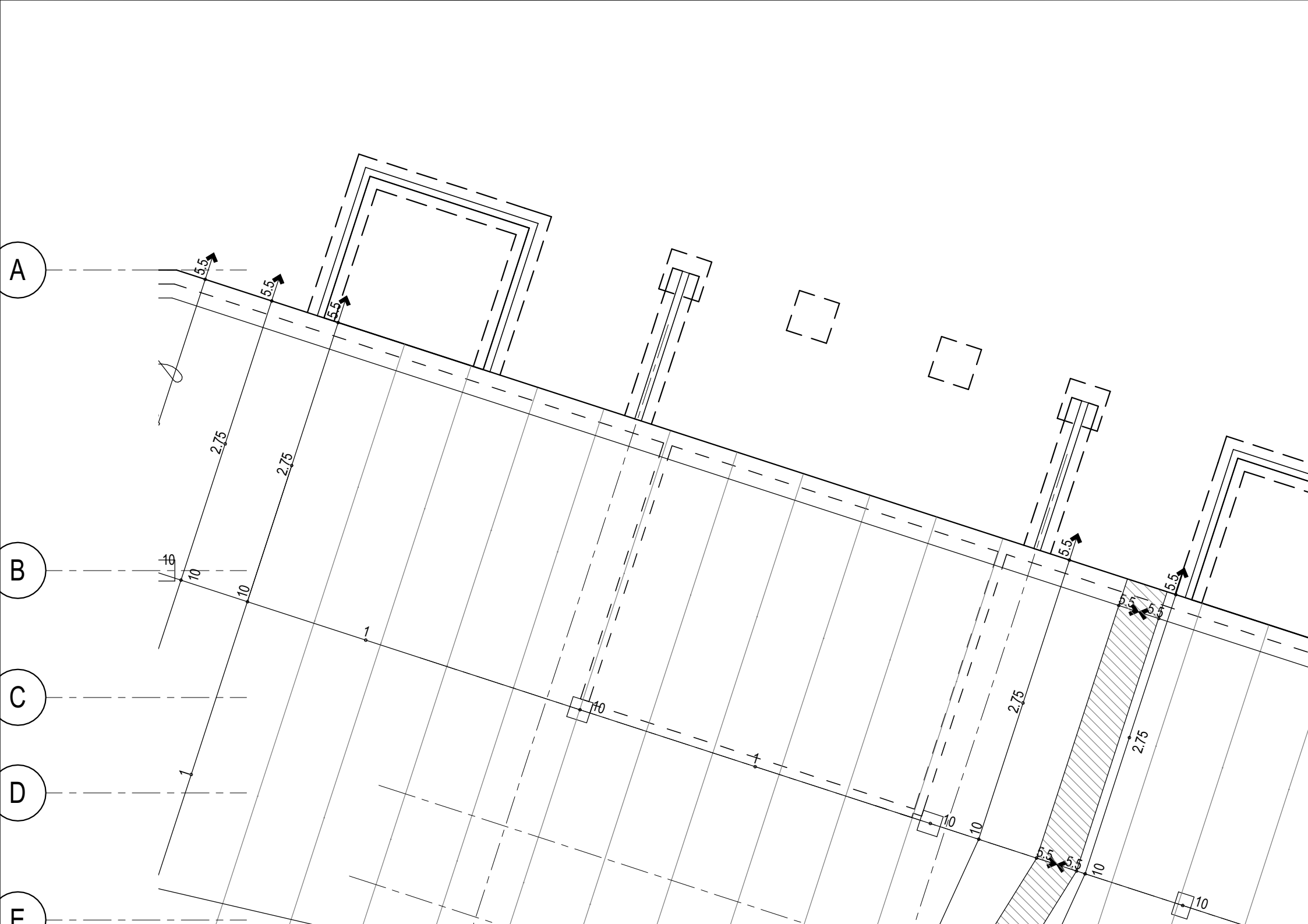

6
S2.2-B
6 x 6 - TYP
F3.0
F3.0
1
S2.2-B
4" THICK CONCRETE SLAB-ON-GRADE - REINF W/ 6 x 6 W1.4 x W1.4 WWF AT CL OF SLAB
4
S2.1-B
W
2
F3.0
A
HD13
HD9
F3.0
(7) BU
W
2
M72
(4) BU
B
C
4
S3.2-B
HD9
10
S3.2-B
FT
2W
3
HD4
D

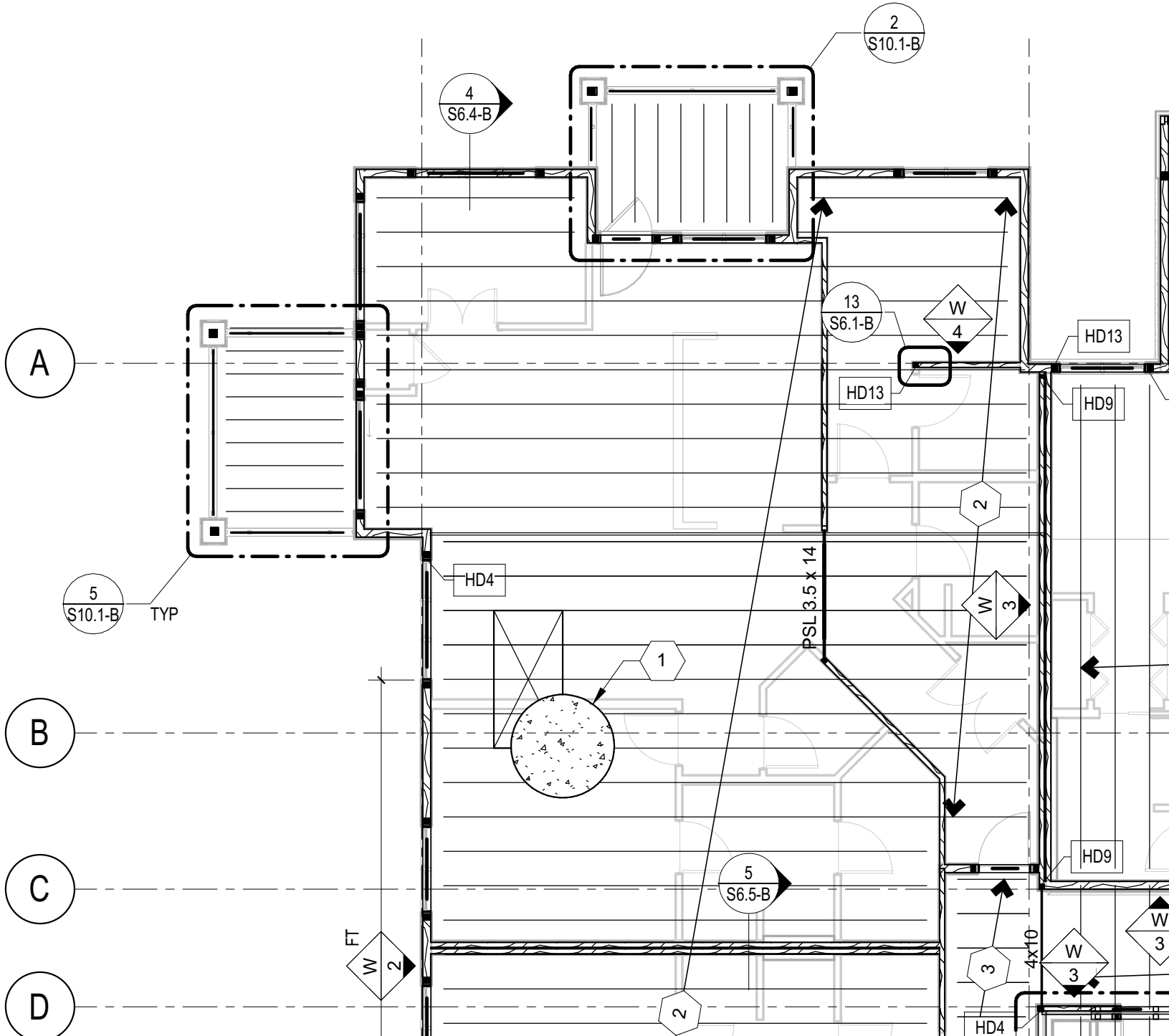

A
B
C
D
H
D12
D
B12

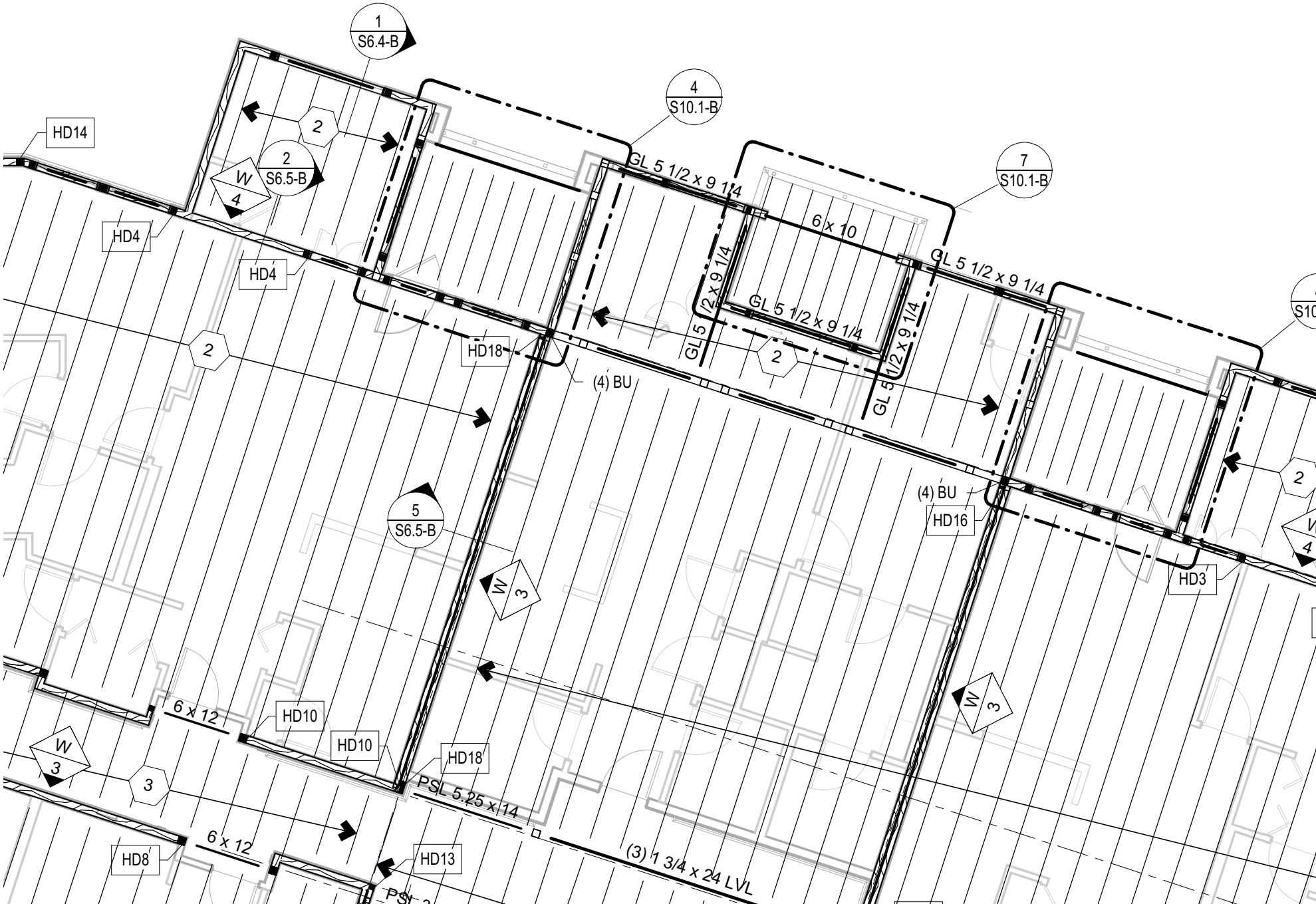

A
B
C
D
5.5
5.5
2.75
5.5
9
5.5
10
1
1

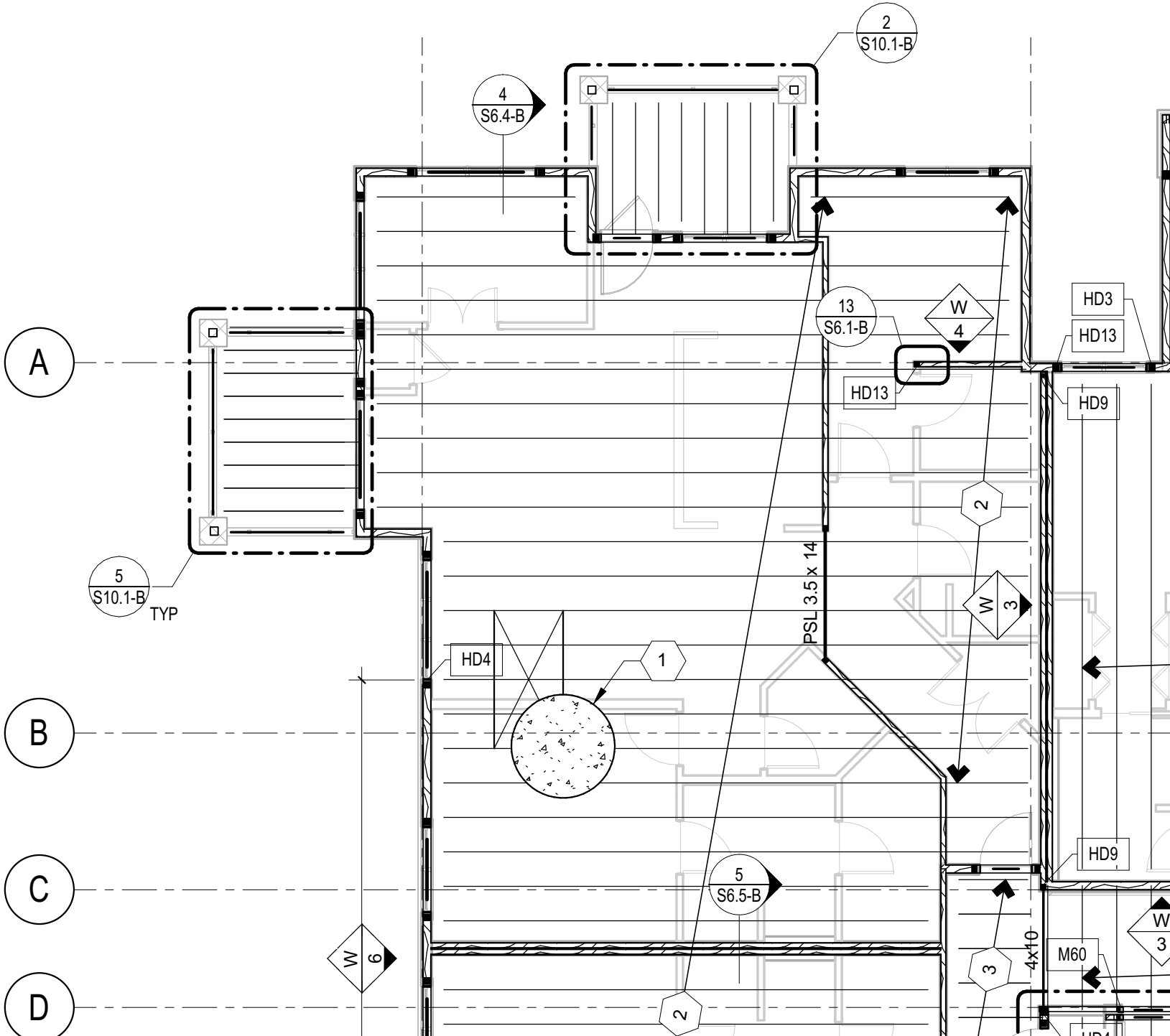

1
S2.2-B
HD14
3
S2.5-B
W
2
HD4
HD4
F3.0
F3.0
6 x 6 - TYP
F3.0
F3.0
4" THICK CONCRETE SLAB-ON-
GRADE - REINF W/ 6 x 6 W1.4 x W
WWF AT CL OF SLAB
(4) BU
1
S3.2-B
HD18
4
S3.2-B
(4) BU
HD16
HD3
W
2
W
2
W
2
HD10
3
S3.2-B
HD18
W
2
HD10
(4) BU
6 x 6
HD8
HD13
HD7


K
H
C15
C15
C
C12
B10
C
3'-0"
K
K
8'-0 1/8"
B10
B12
B
B12
B10
D
B

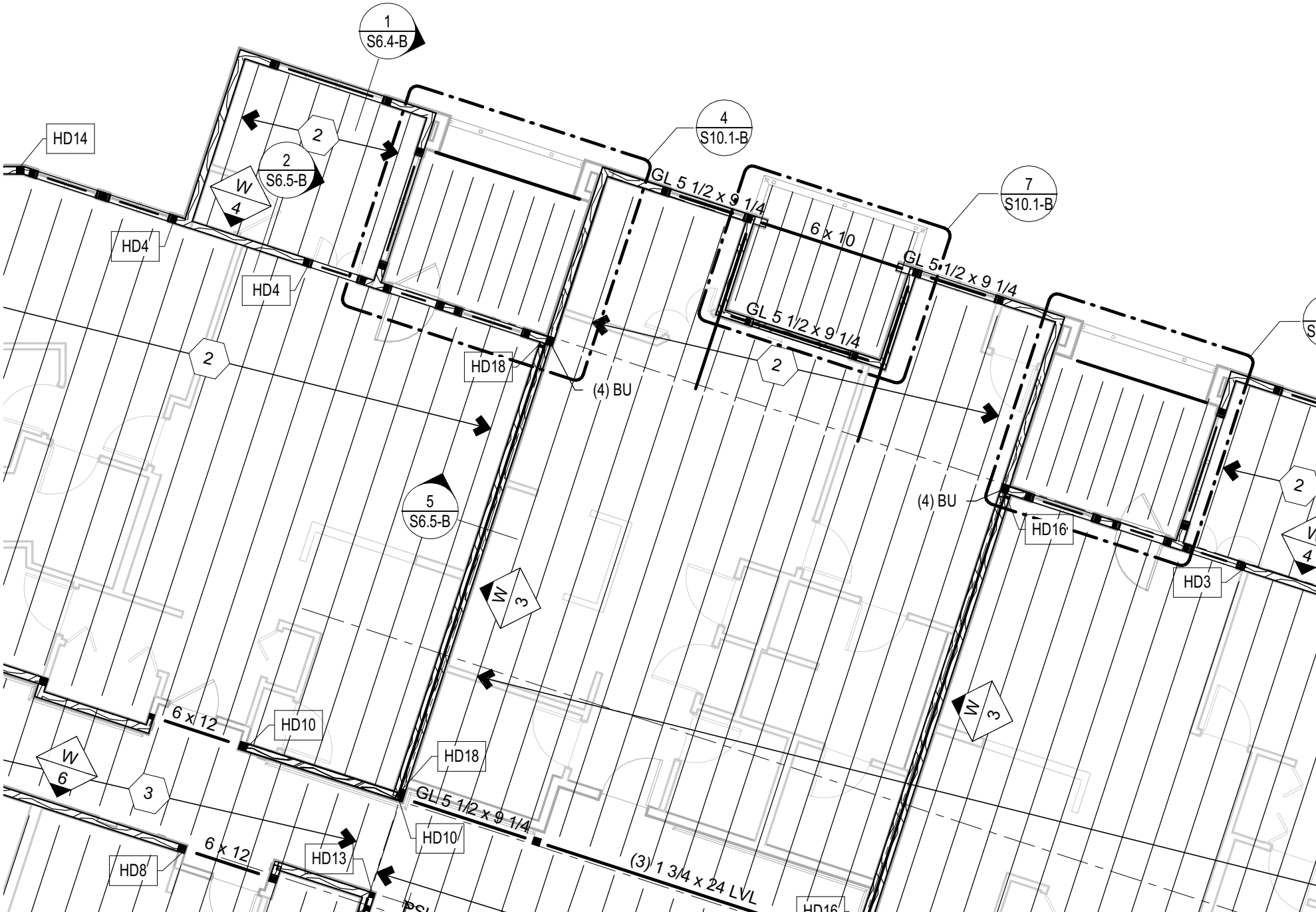

A
B
C
D
E
5.5
2.75
10
1

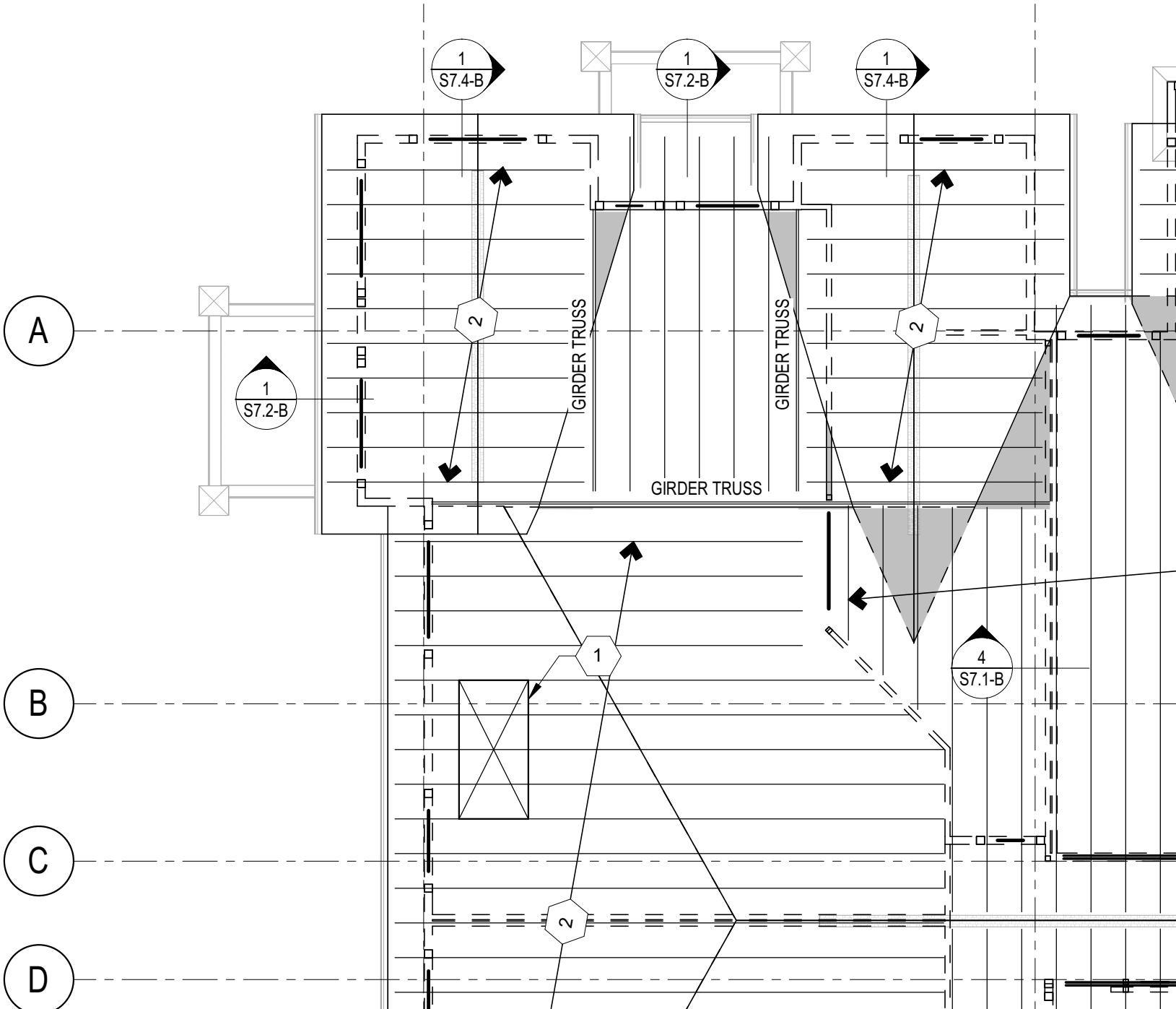

2
S10.1-B
4
S6.4-B
13
S6.1-B
W
4
HD13
HD13
HD9
A
2
HD4
5
S10.1-B
TYP
PSL 3.5 x 14
W
3
1
B
HD9
5
S6.5-B
C
FT
W
2
4x10
3
W
3
W
3
D
2
HD4

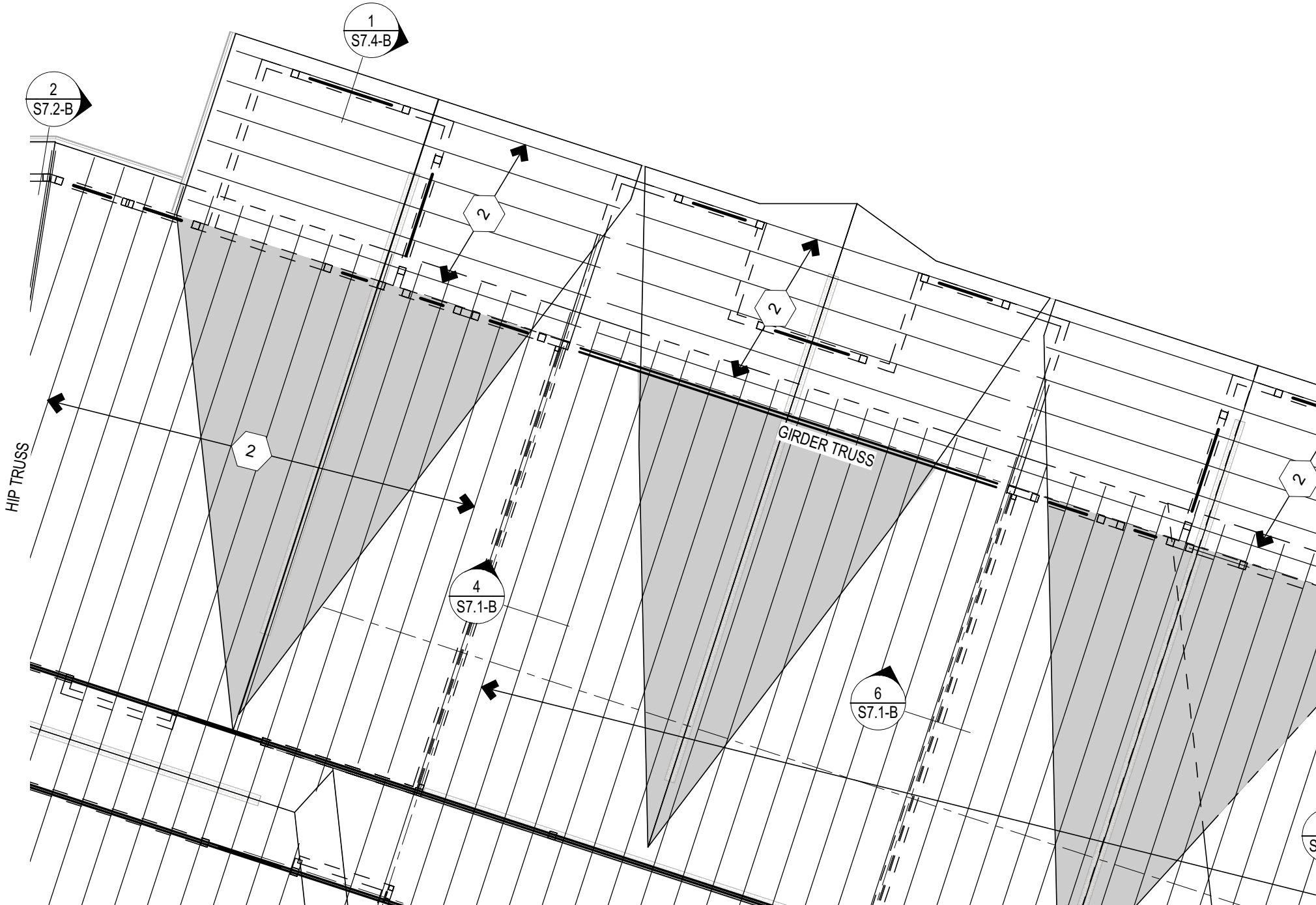

1
S6.4-B
4
S10.1-B
7
S10.1-B
HD14
HD4
HD4
2
S6.5-B
W
4
2
2
HD18
(4) BU
GL 5 1/2 x 9 1/4
6 x 10
GL 5 1/2 x 9 1/4
GL 5 1/2 x 9 1/4
GL 5 1/2 x 9 1/4
GL 5 1/2 x 9 1/4
2
5
S6.5-B
W
3
(4) BU
HD16
HD3
2
W
4
6 x 12
HD10
HD10
HD18
W
3
3
PSL 5.25 x 14
6 x 12
HD8
HD13
(3) 1 3/4 x 24 LVL
W
3

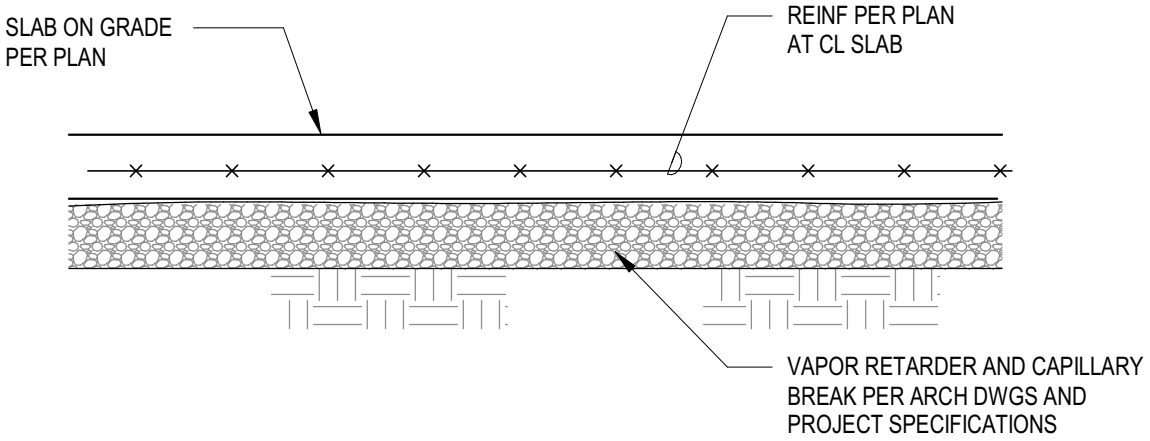

2
S10.1-B
4
S6.4-B
13
S6.1-B
W
4
HD3
HD13
HD13
HD9
A
5
S10.1-B
TYP
2
PSL 3.5 x 14
W
3
HD4
1
B
HD9
5
S6.5-B
C
W
6
3
4x10
M60
W
3
2
D

FT

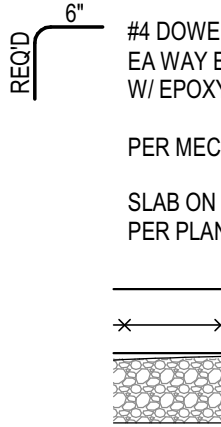

1
S6.4-B
HD14
2
2
S6.5-B
W
4
HD4
HD4
4
S10.1-B
GL 5 1/2 x 9 1/4
7
S10.1-B
6 x 10
GL 5 1/2 x 9 1/4
GL 5 1/2 x 9 1/4
2
HD18
(4) BU
2
2
5
S6.5-B
(4) BU
HD16
HD3
W
3
W
3
W
4
6 x 12
HD10
HD18
W
6
3
GL 5 1/2 x 9 1/4
HD10
6 x 12
HD8
HD13
(3) 1 3/4 x 24 LVL
HD16


A
B
C
D
1
S7.4-B
1
S7.2-B
1
S7.4-B
1
S7.2-B
2
2
2
GIRDER TRUSS
GIRDER TRUSS
GIRDER TRUSS
1
4
S7.1-B


1
S7.4-B
2
S7.2-B
2
2
2
HIP TRUSS
GIRDER TRUSS
4
S7.1-B
6
S7.1-B

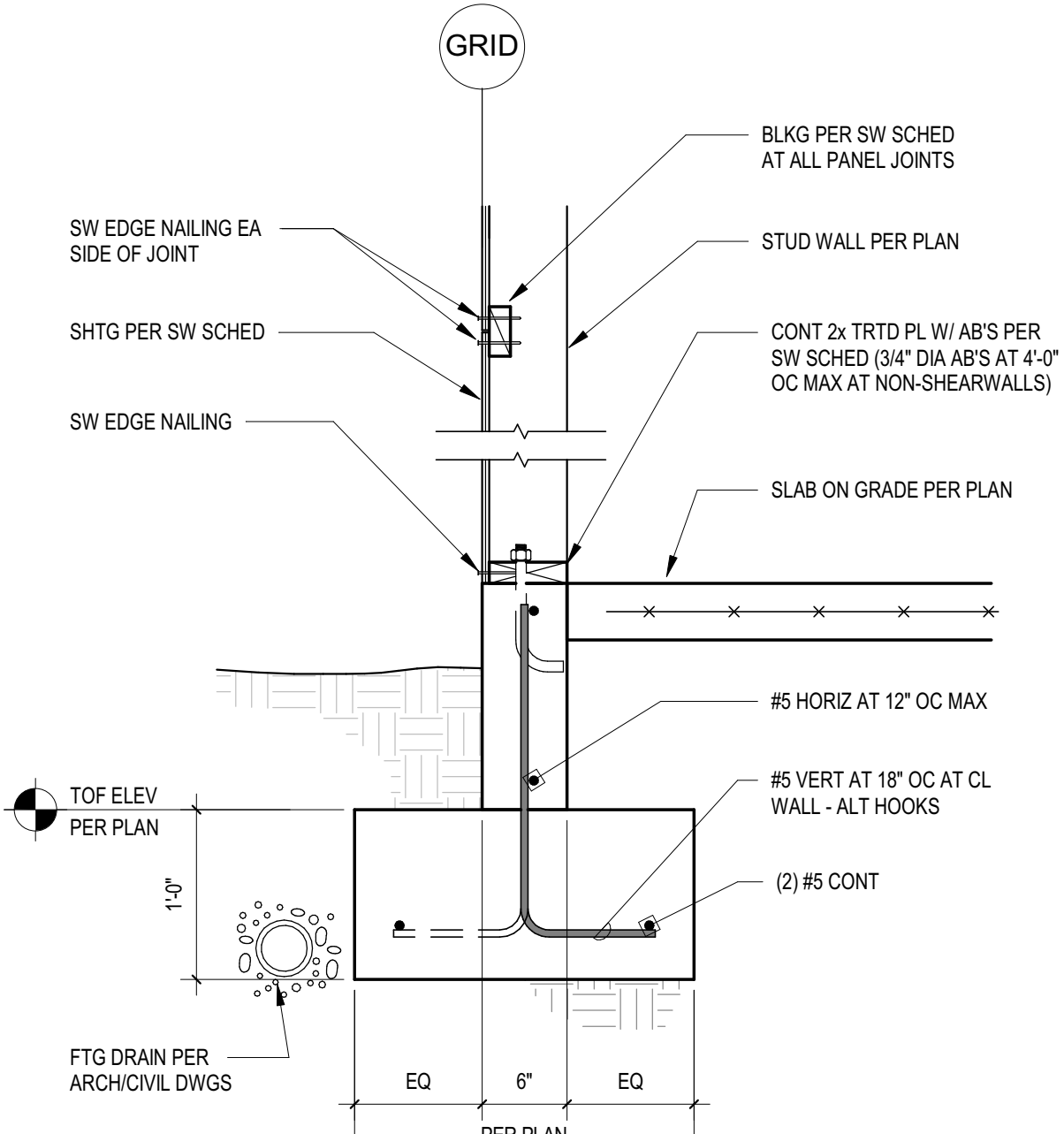


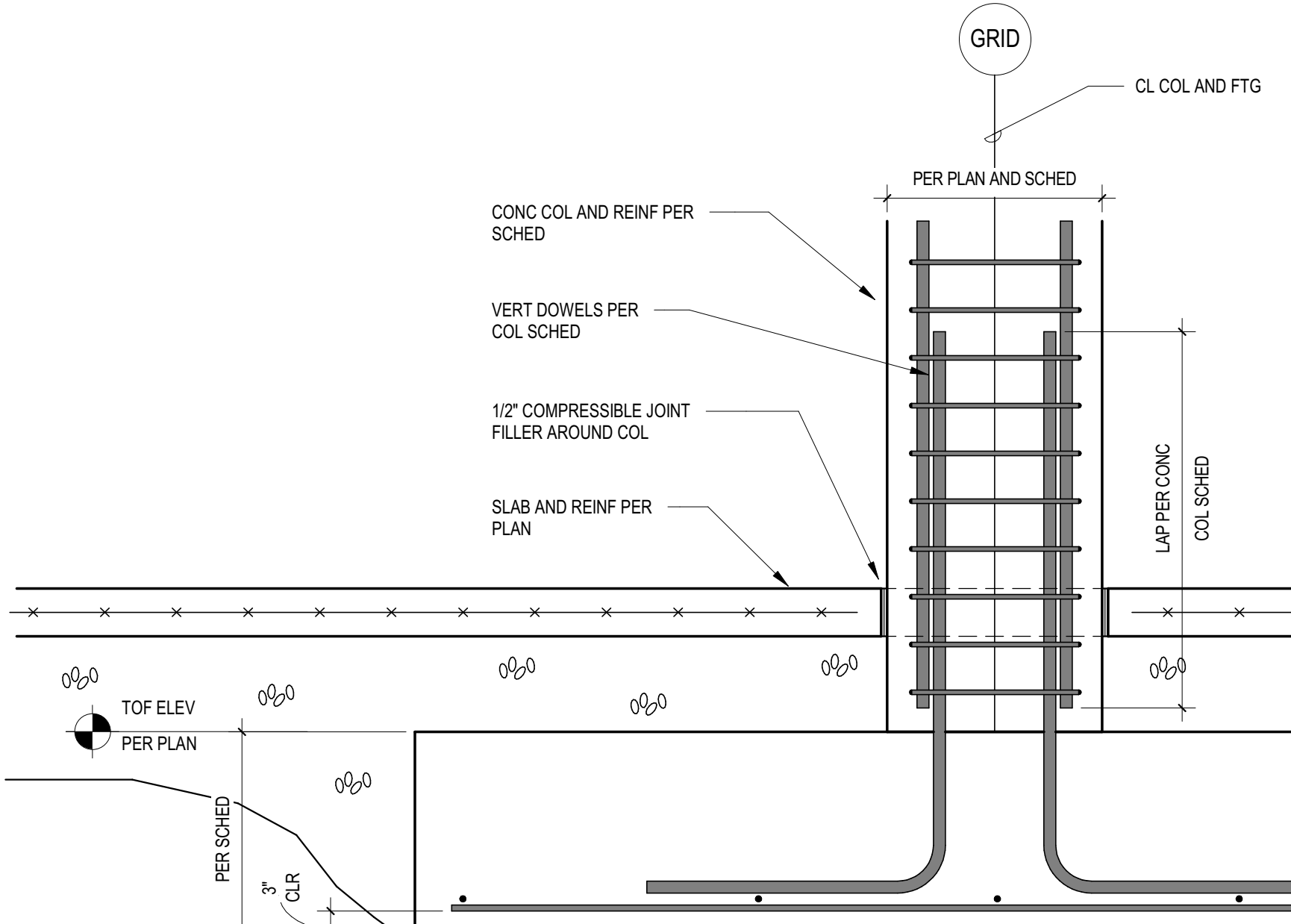
1 SECTION

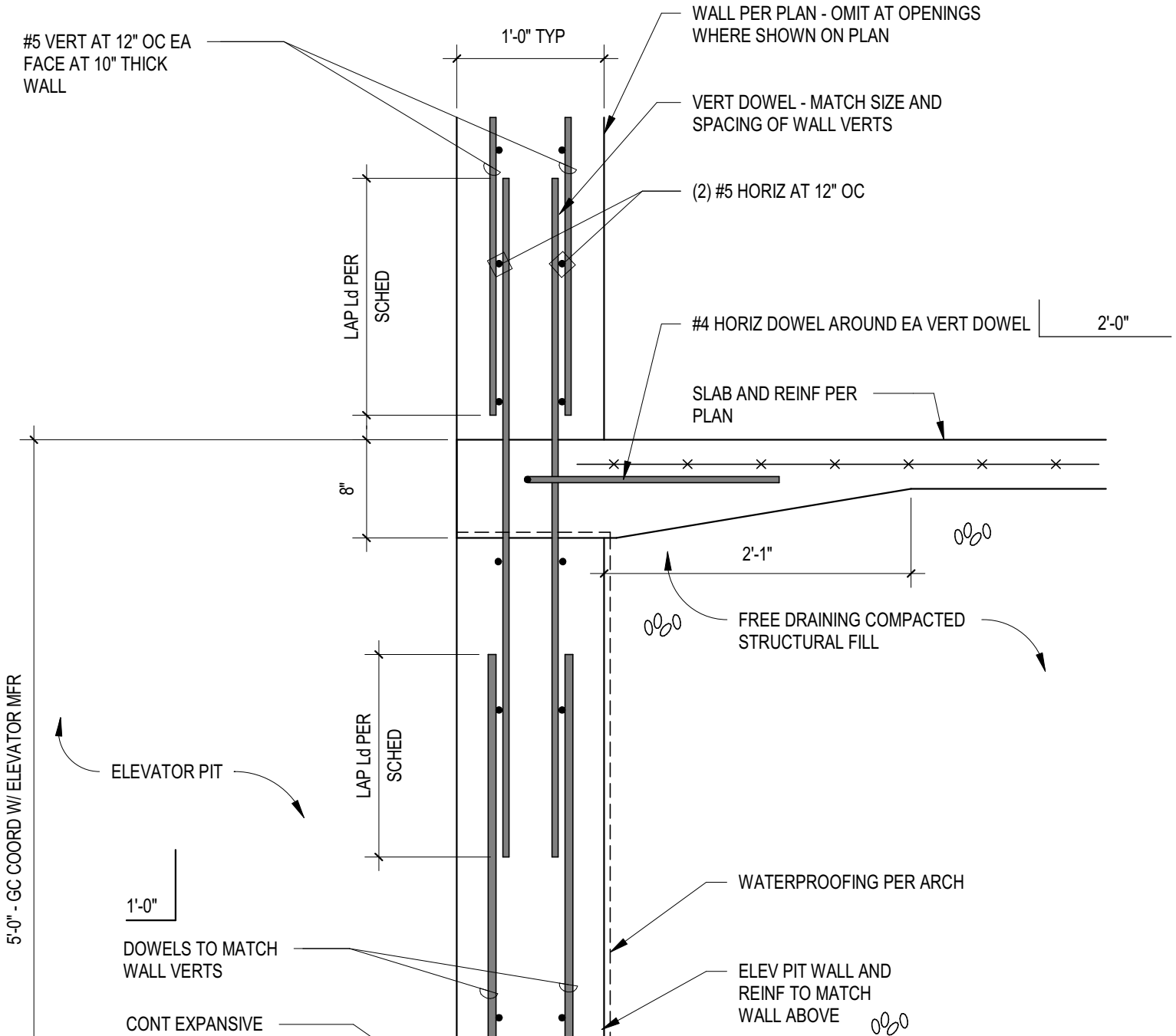


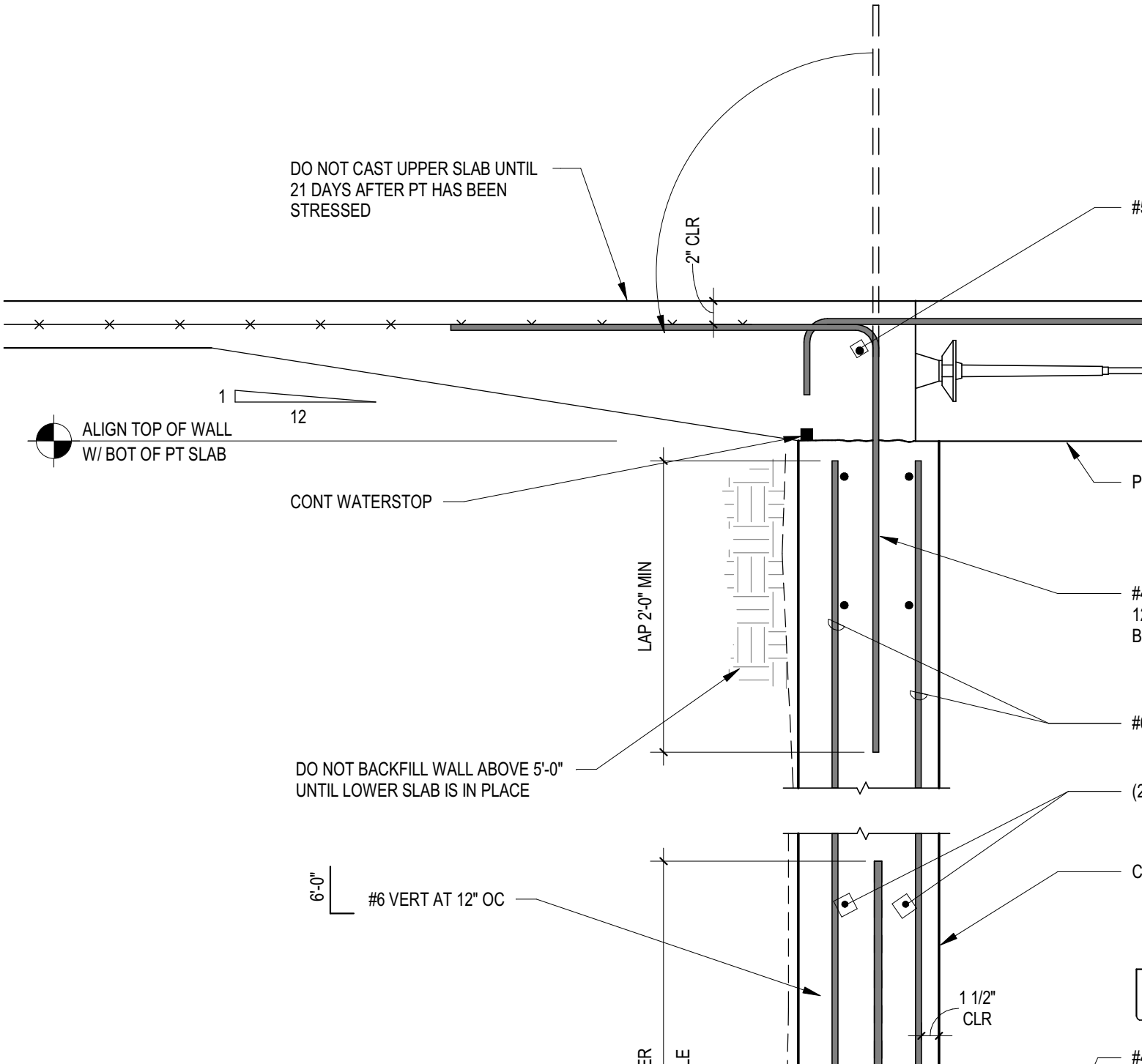
2 SEC

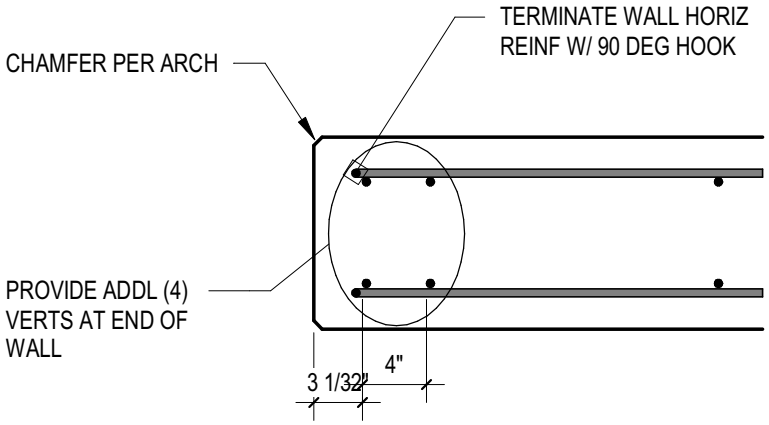
1" = 1'-0" 2 / S2


GRID
BLKG PER SW SCHED
AT ALL PANEL JOINTS
SW EDGE NAILING EA
SIDE OF JOINT
STUD WALL PER PLAN
SHTG PER SW SCHED
CONT 2x TRTD PL W/ AB'S PER
SW SCHED (3/4" DIA AB'S AT 4'-0"
OC MAX AT NON-SHEARWALLS)
SW EDGE NAILING
SLAB ON GRADE PER PLAN
#5 HORIZ AT 12" OC MAX
TOF ELEV
PER PLAN
#5 VERT AT 18" OC AT CL
WALL - ALT HOOKS
(2) #5 CONT
1'-0"
FTG DRAIN PER
ARCH/CIVIL DWGS
EQ
6"
EQ


GRID
CL COL AND FTG
PER PLAN AND SCHED
CONC COL AND REINF PER SCHED
VERT DOWELS PER COL SCHED
1/2" COMPRESSIBLE JOINT FILLER AROUND COL
SLAB AND REINF PER PLAN
LAP PER CONC
COL SCHED
TOF ELEV
PER PLAN
PER SCHED
3" CLR


#5 VERT AT 12" OC EA FACE AT 10" THICK WALL
1'-0" TYP
WALL PER PLAN - OMIT AT OPENINGS WHERE SHOWN ON PLAN
VERT DOWEL - MATCH SIZE AND SPACING OF WALL VERTS
(2) #5 HORIZ AT 12" OC
LAP Ld PER SCHED
#4 HORIZ DOWEL AROUND EA VERT DOWEL
2'-0"
SLAB AND REINF PER PLAN
8"
2'-1"
FREE DRAINING COMPACTED STRUCTURAL FILL
5'-0" - GC COORD W/ ELEVATOR MFR
ELEVATOR PIT
LAP Ld PER SCHED
1'-0"
WATERPROOFING PER ARCH
DOWELS TO MATCH WALL VERTS
ELEV PIT WALL AND REINF TO MATCH WALL ABOVE
CONT EXPANSIVE

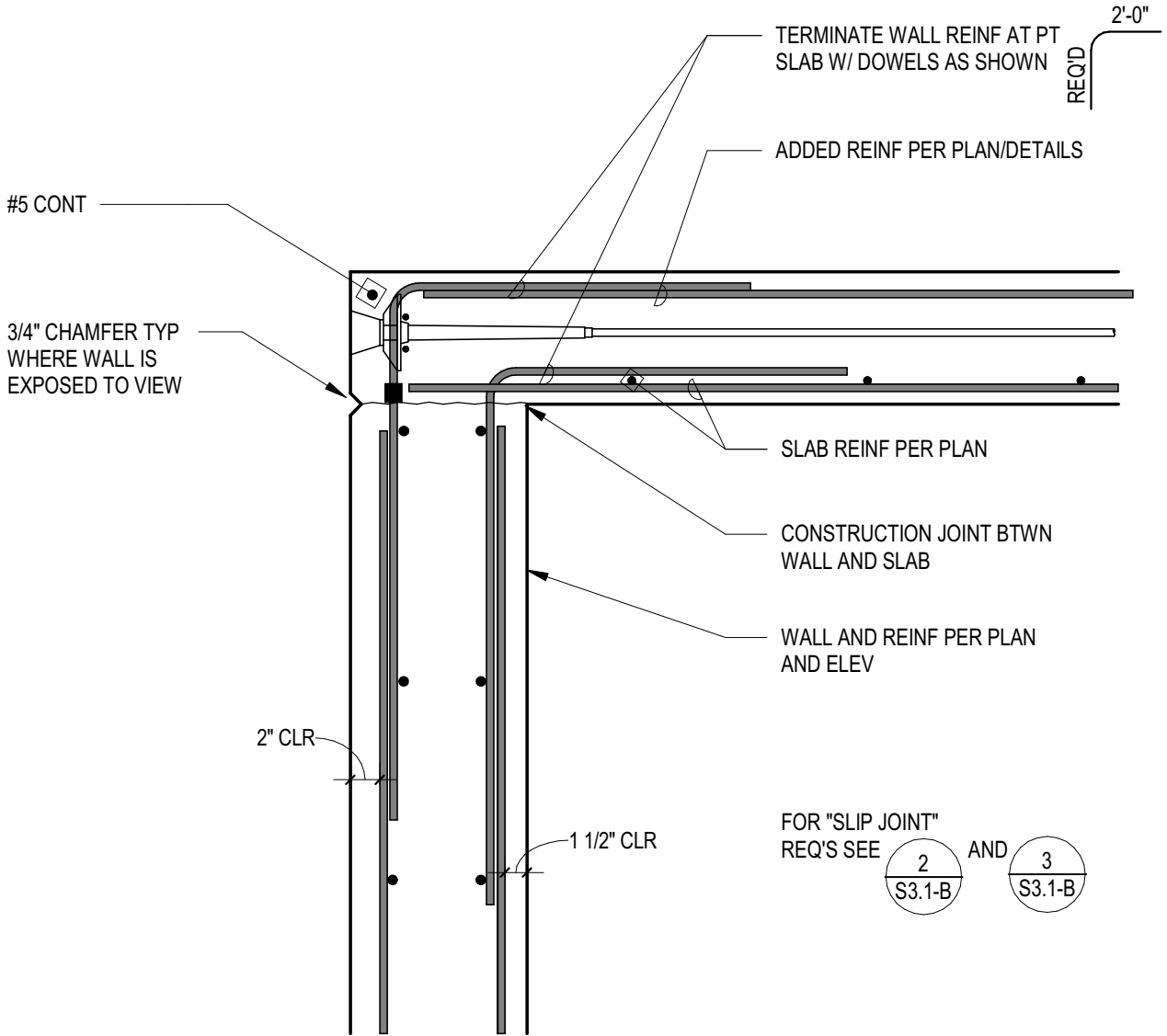

DO NOT CAST UPPER SLAB UNTIL
21 DAYS AFTER PT HAS BEEN
STRESSED
2" CLR
1
12
ALIGN TOP OF WALL
W/ BOT OF PT SLAB
CONT WATERSTOP
LAP 2'-0" MIN
DO NOT BACKFILL WALL ABOVE 5'-0"
UNTIL LOWER SLAB IS IN PLACE
6'-0"
#6 VERT AT 12" OC
1 1/2"
CLR

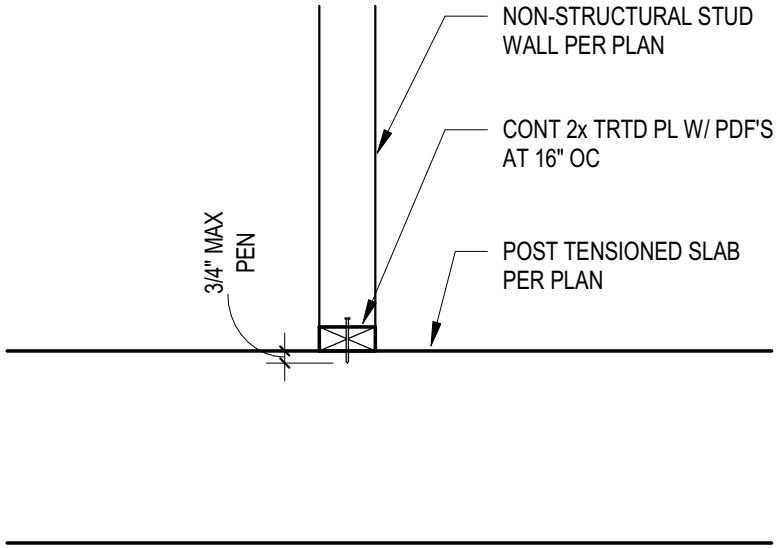


TYPICAL ENDWALL CONDITION

1 SECTION

2 SEC


TERMINATE WALL REINF AT PT SLAB W/ DOWELS AS SHOWN
2'-0" REQ'D
ADDED REINF PER PLAN/DETAILS
#5 CONT
3/4" CHAMFER TYP WHERE WALL IS EXPOSED TO VIEW
SLAB REINF PER PLAN
CONSTRUCTION JOINT BTWN WALL AND SLAB
WALL AND REINF PER PLAN AND ELEV
2" CLR
1 1/2" CLR
FOR "SLIP JOINT" REQ'S SEE 2 S3.1-B AND 3 S3.1-B

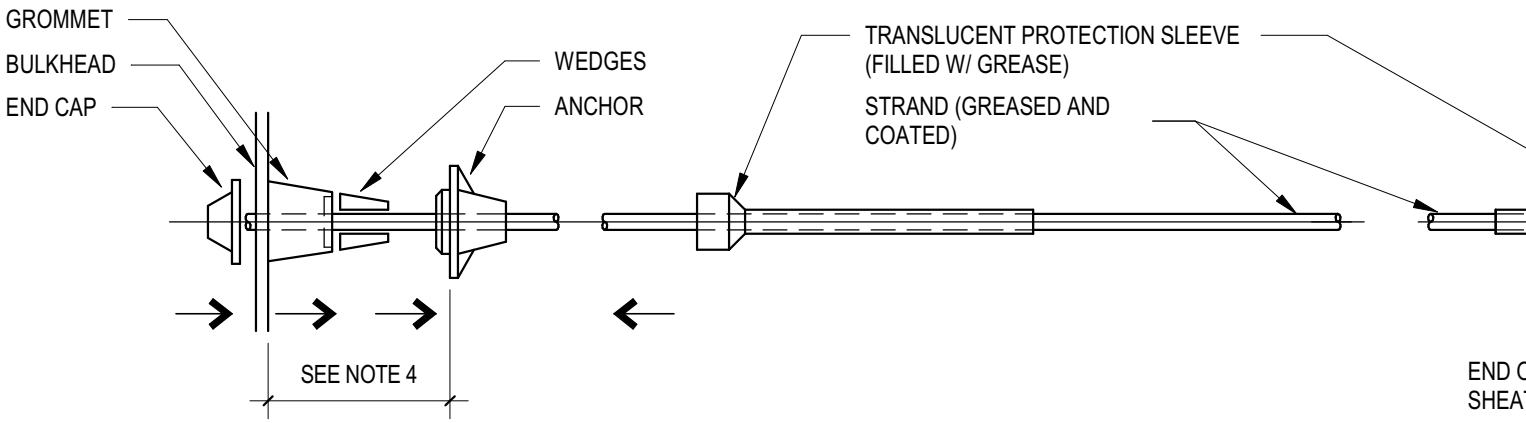


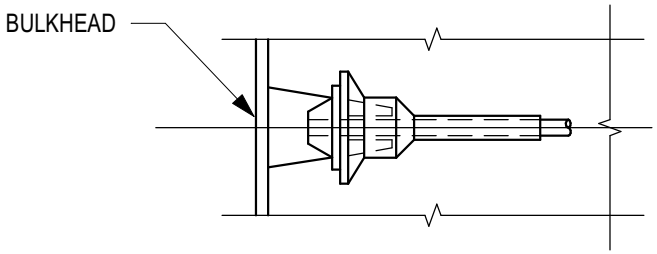
1 SECTION

1" = 1'-0" 1 / S3.2-B

2 SECTION

1" = 1'-0" 2 / S3.2-B





AT STRESSING END

NOTES:

1 LOCATE ANCHOR AT BULKHEAD PER FRAMING PLANS.

2 INSTALL GROMMET FLUSH BETWEEN BULKHEAD AND ANCHOR FOR TIGHT SEAL.

3 SLIDE GREASE-FILLED PROTECTION SLEEVE TIGHT AGAINST ANCHOR PROVIDING 4" MIN OVERLAP BETWEEN SLEEVE AND END OF SHEATHING.

4 AFTER POURING, AT TIME OF STRESSING, REMOVE GROMMET AND INSERT WEDGES.

5 AFTER STRESSING AND ENGINEER'S APPROVAL OF STRESSING RECORD, CUT STRAND 1 1/2" - 1" BEYOND WEDGES PER SHEET S0.1 AND GREASE END CAP PRIOR TO INSERTING IT TIGHT AGAINST ANCHOR. SEE STRUCTURAL

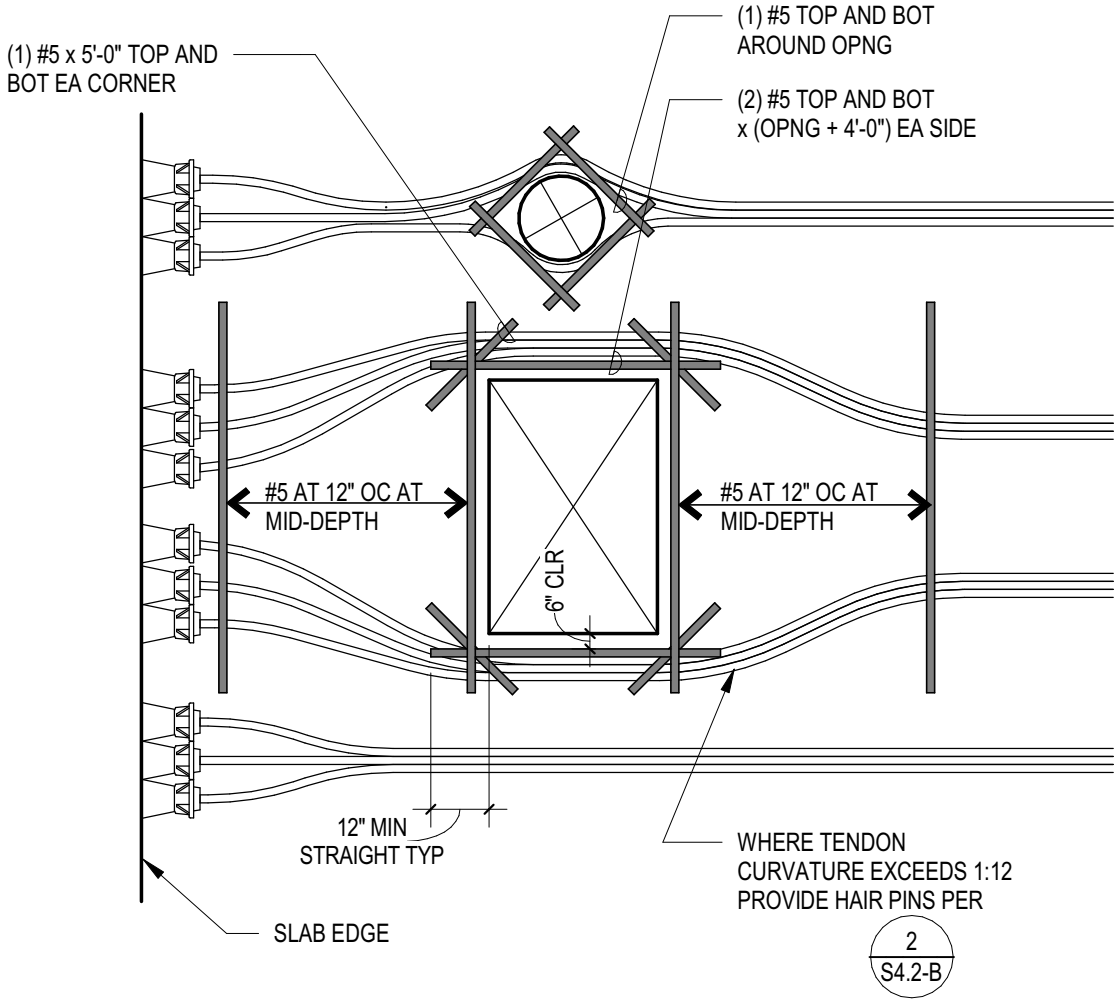
AT D

NOTES:

1 LOCATE ANCHOR AT BULKHEA

2 IF FABRICATED IN SHOP INCLU

3 IF FIELD SEATING IS REQUIRED
ANCHOR.

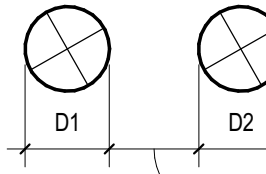


NOTES:

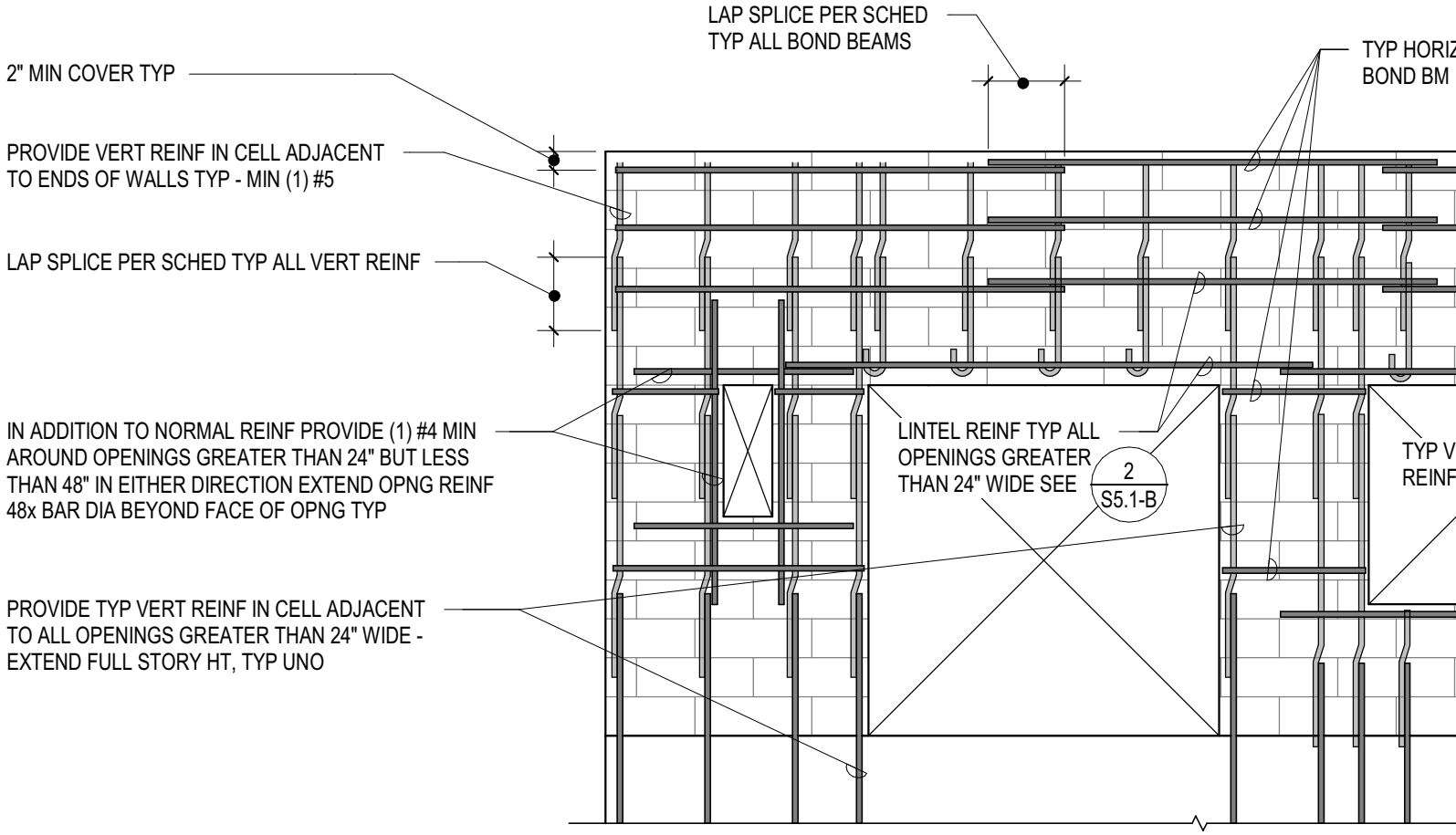
1. OPENING SHALL NOT BE PL
COLUMN UNLESS EXACT LOCAT
COORDINATED WITH STRUCTUR

2. ROUND OPENINGS MAY BE
REINFORCED SIMILAR TO RECT

3. SPACING OF CIRCULAR OPE

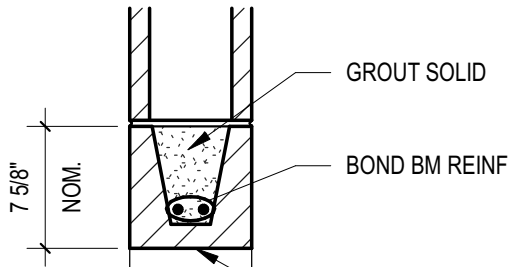


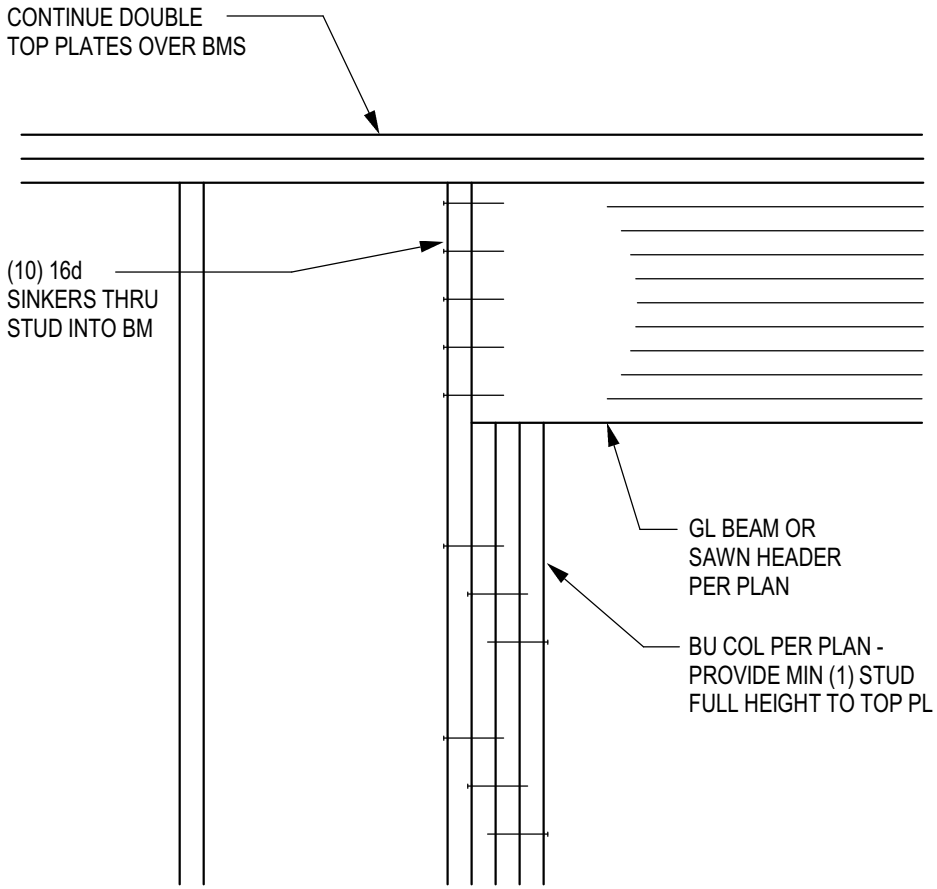
4. MAX OPENING SIZE LARGER
ROUND: 48" DIA RECTANGULAR

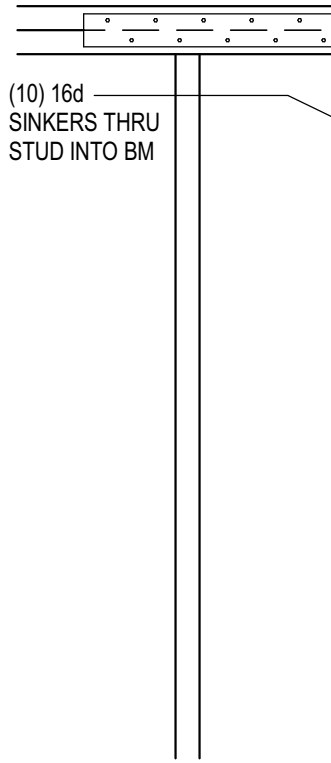


CMU WALL ELEVATION NOTES:

1. TYPICAL REINFORCEMENT SHOWN. PROVIDE MORE IF REQUIRED BY DETAILS.
2. PROVIDE CONTINUOUS BOND BM AT TOP OF WALL.
3. FOR CORNER BARS USE SAME SIZE AND SPACING OF TYPICAL HORIZONTAL REINFORCING. LAP CORNER BARS WITH TYPICAL HORIZONTAL REINFORCING WITH A LAP SPLICE PER SCHEDULE.
4. HOOK ALL REINFORCING THAT CANNOT BE EXTENDED.
5. GROUT ALL CELLS CONTAINING REINFORCING, ANCHOR BOLTS OR OTHER EMBEDDED ITEMS.





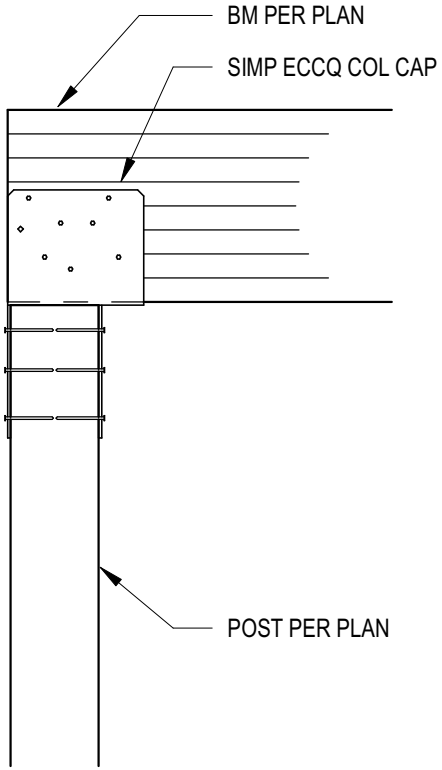


1 SECTION

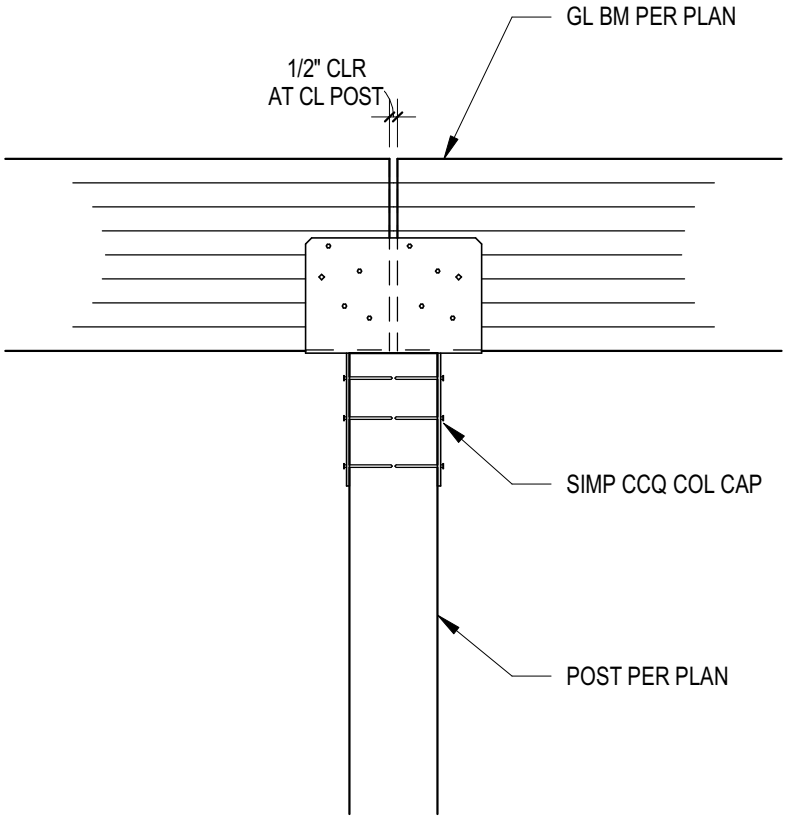
1" = 1'-0" 1 / S6.1-B

2 SECTIO

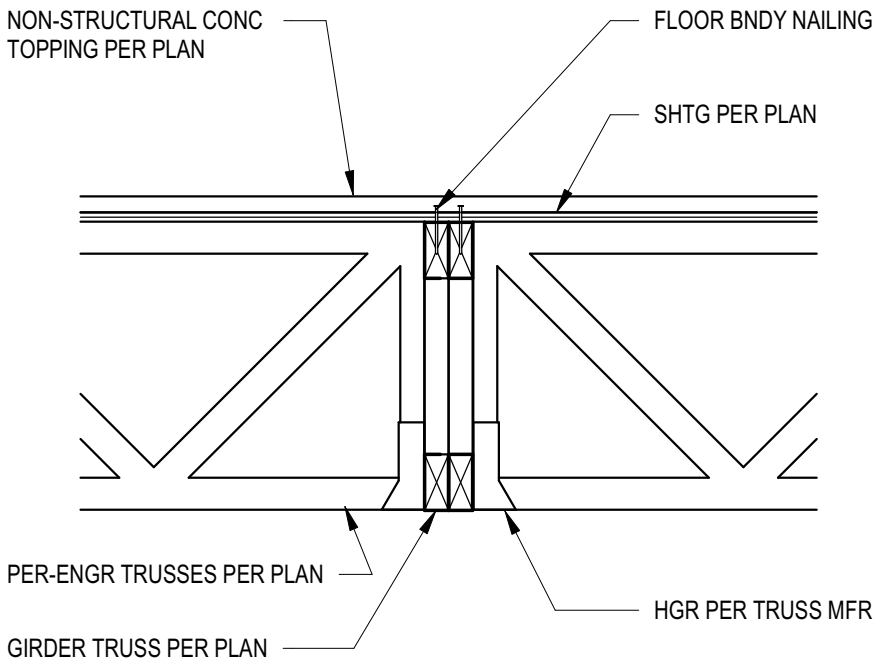
1" = 1'-0" 2 / S6.1-B



1 SECTION

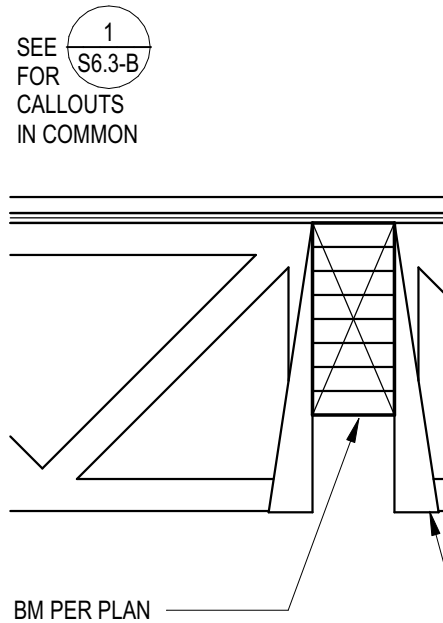


2 SECTION



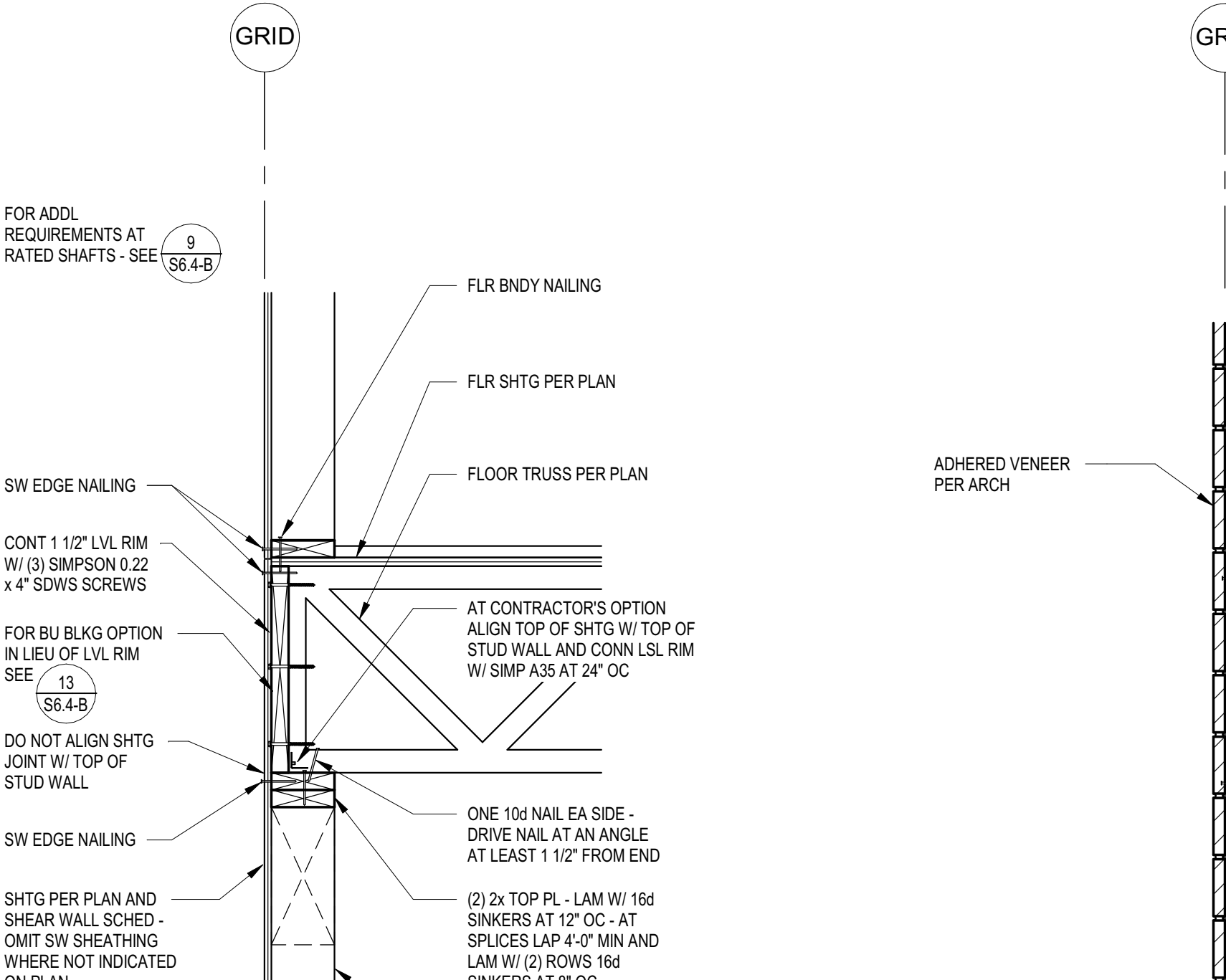
1 SECTION

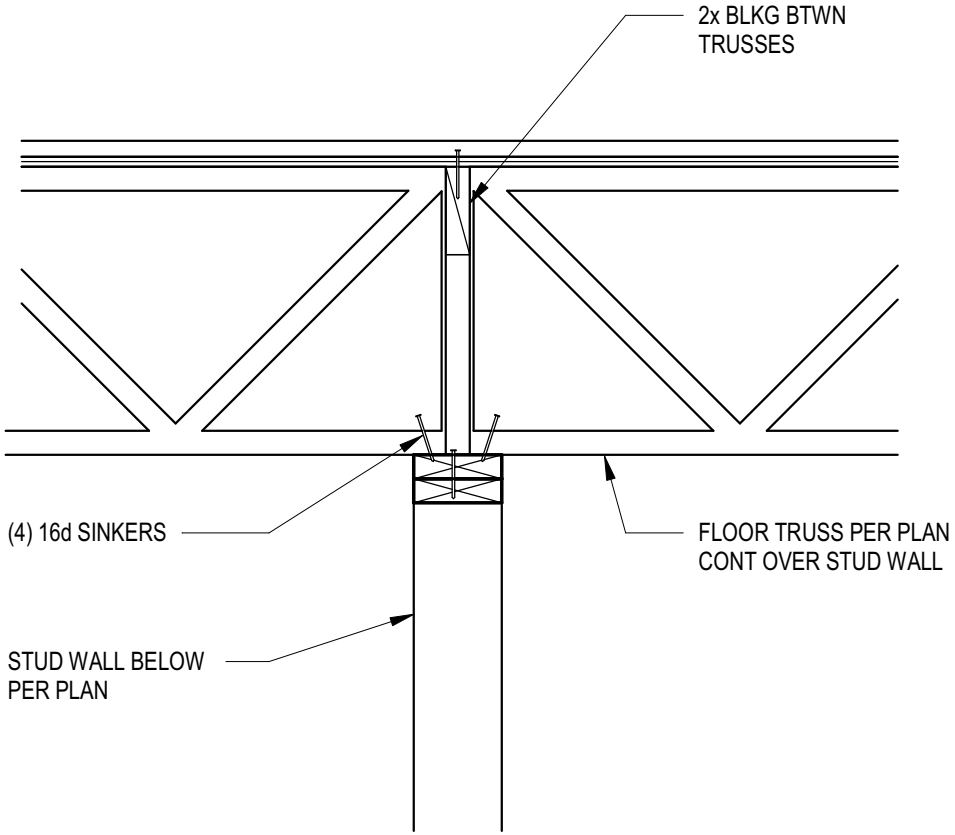
1" = 1'-0" 1 / S6.3-B



2 SECTION

1" = 1'-0" 2 / S6.3-B


GRID
GR
FOR ADDL REQUIREMENTS AT RATED SHAFTS - SEE 9 S6.4-B
FLR BNDY NAILING
FLR SHTG PER PLAN
FLOOR TRUSS PER PLAN
ADHERED VENEER PER ARCH
SW EDGE NAILING
CONT 1 1/2" LVL RIM W/ (3) SIMPSON 0.22 x 4" SDWS SCREWS
FOR BU BLKG OPTION IN LIEU OF LVL RIM SEE 13 S6.4-B
AT CONTRACTOR'S OPTION ALIGN TOP OF SHTG W/ TOP OF STUD WALL AND CONN LSL RIM W/ SIMP A35 AT 24" OC
DO NOT ALIGN SHTG JOINT W/ TOP OF STUD WALL
SW EDGE NAILING
ONE 10d NAIL EA SIDE - DRIVE NAIL AT AN ANGLE AT LEAST 1 1/2" FROM END
SHTG PER PLAN AND SHEAR WALL SCHED - OMIT SW SHEATHING WHERE NOT INDICATED
(2) 2x TOP PL - LAM W/ 16d SINKERS AT 12" OC - AT SPLICES LAP 4'-0" MIN AND LAM W/ (2) ROWS 16d



1 SECTION

1" = 1'-0" 1 / S6.5-B

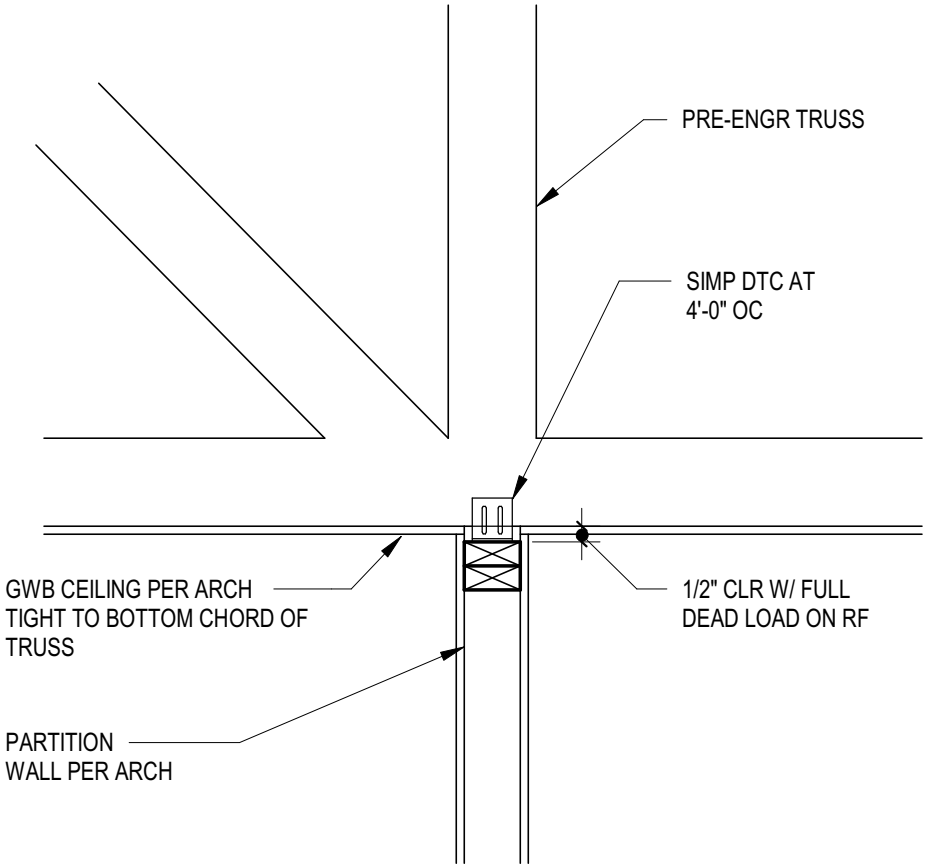
SEE 1 / S6.5-B FOR CALLOUTS IN COMMON

16d SINKER TOENAILS AT 6" OC AT BLKG

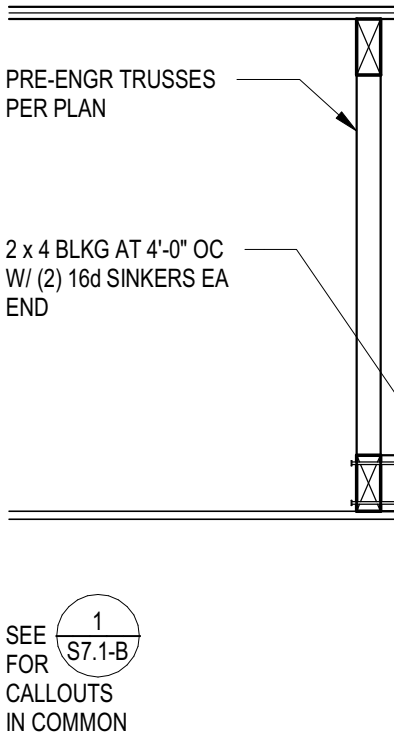
2 SECTI

1" = 1'-0" 2 / S6.5-B

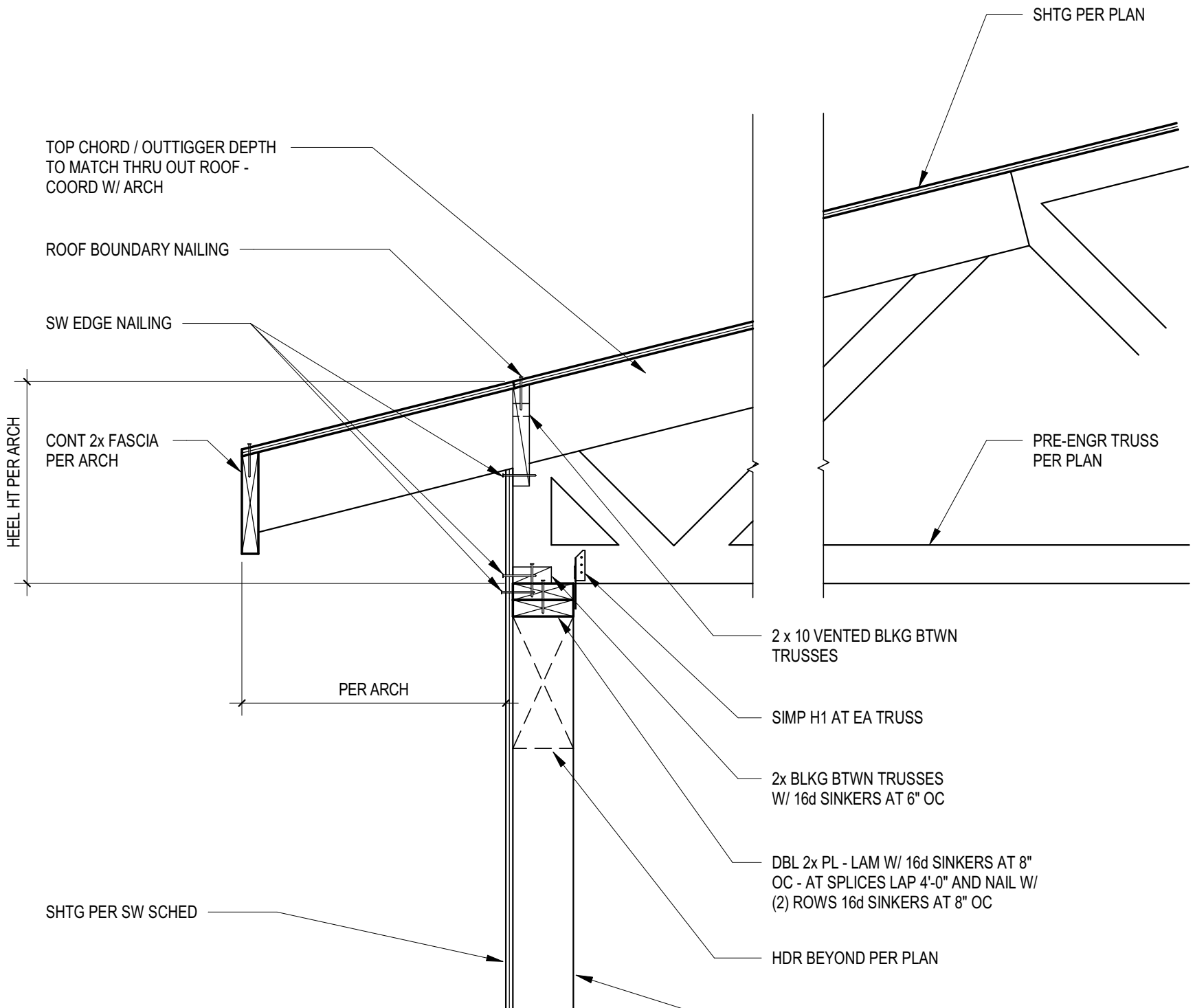
SEE 4 / S6.5-B AND 5 / S6.5-B

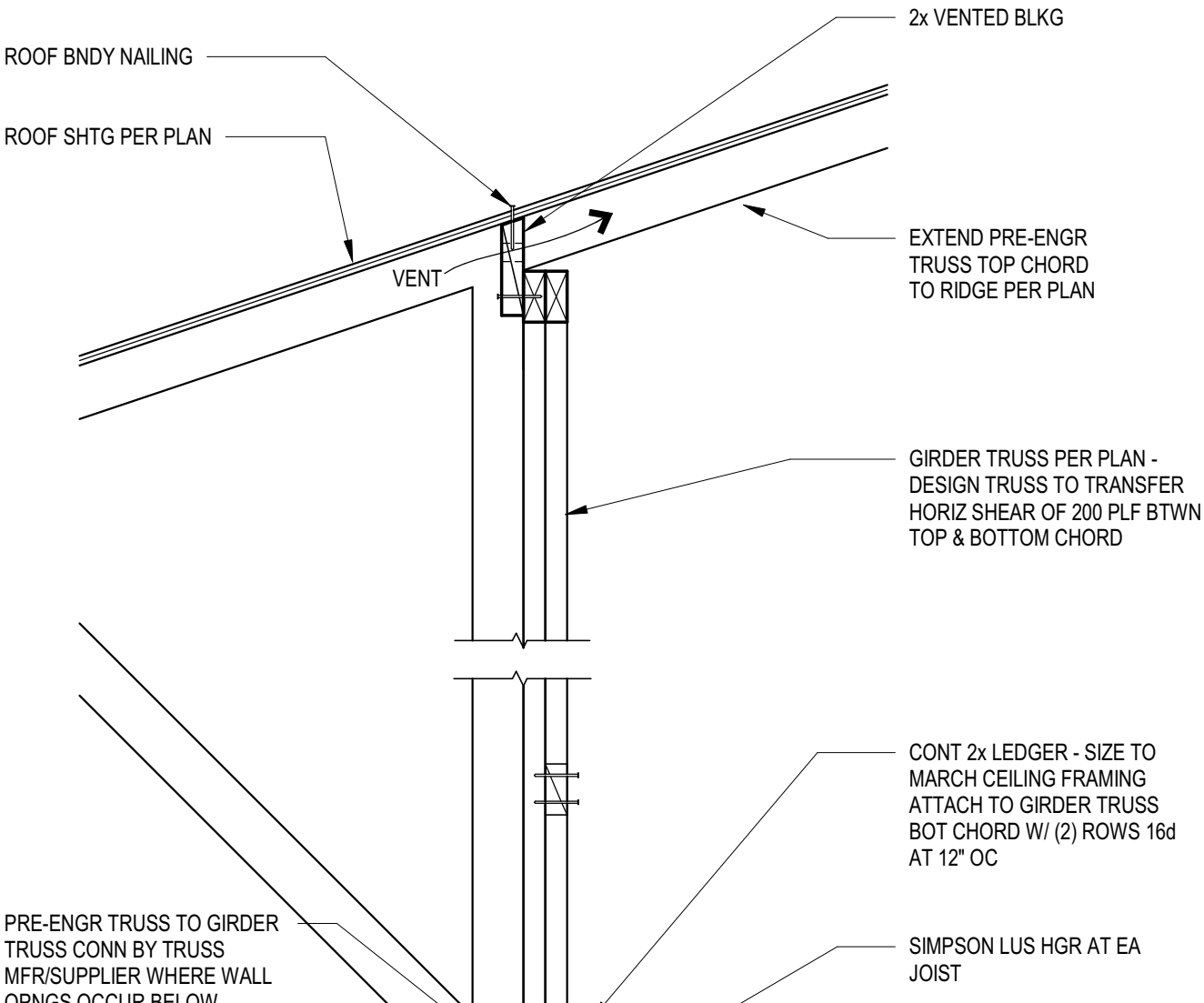


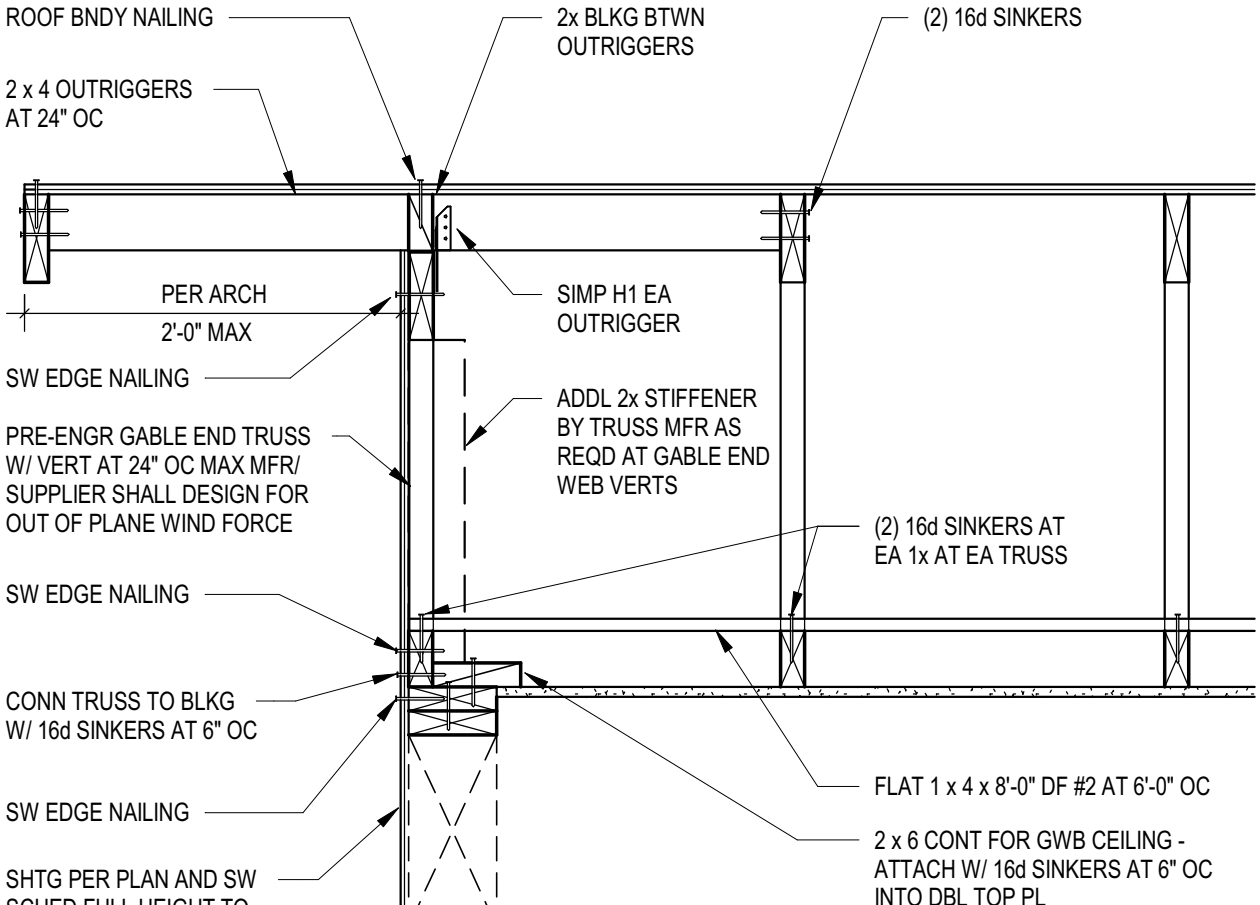
1 SECTION

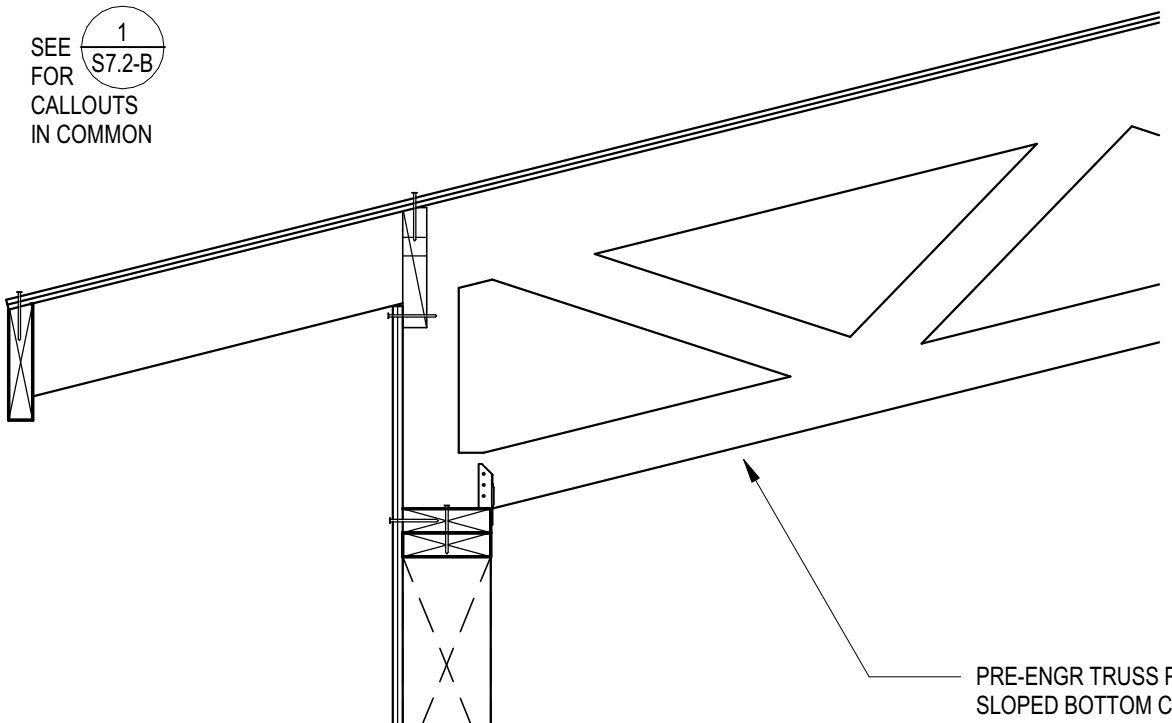


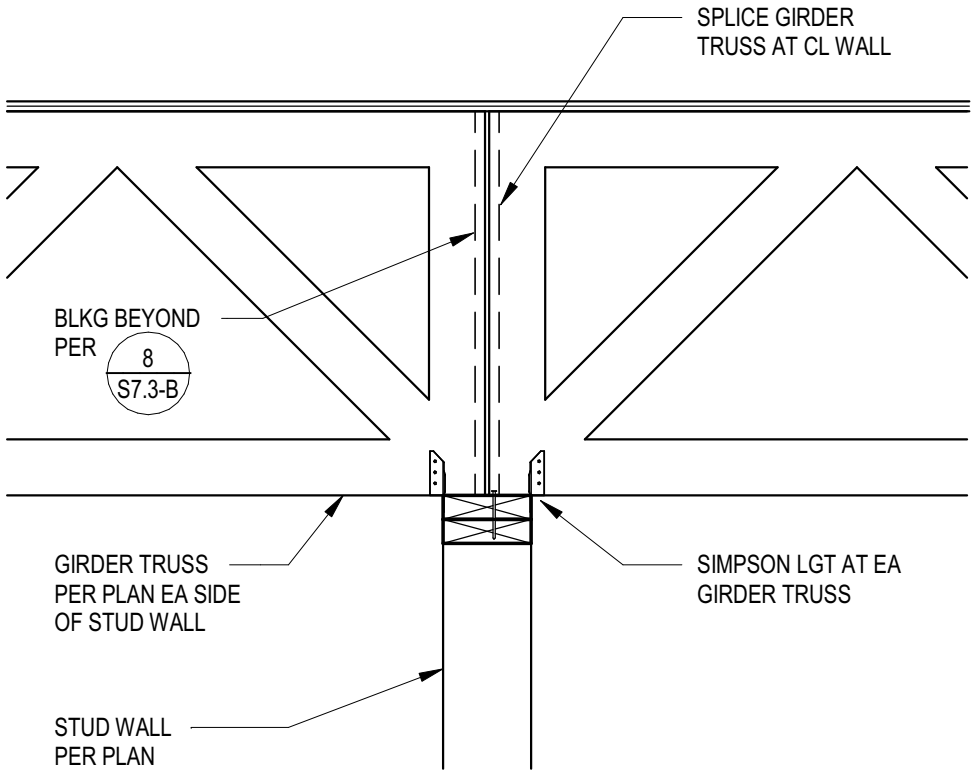
SECTIO

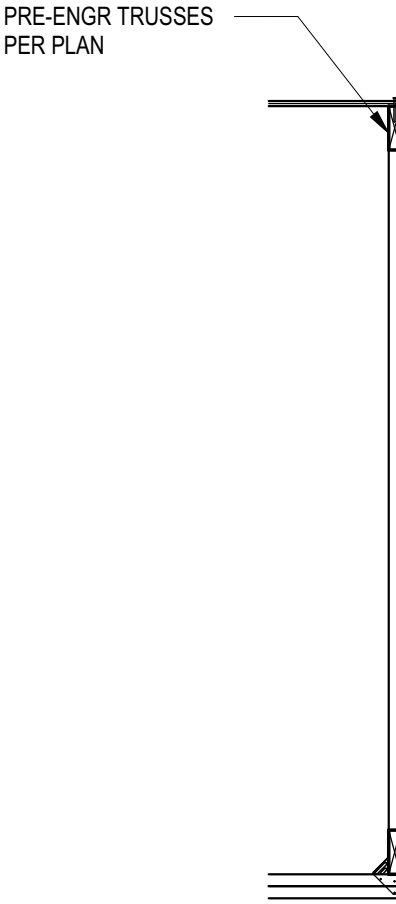

SHTG PER PLAN
TOP CHORD / OUTTIGGER DEPTH
TO MATCH THRU OUT ROOF -
COORD W/ ARCH
ROOF BOUNDARY NAILING
SW EDGE NAILING
HEEL HT PER ARCH
CONT 2x FASCIA
PER ARCH
PRE-ENGR TRUSS
PER PLAN
2 x 10 VENTED BLKG BTWN
TRUSSES
PER ARCH
SIMP H1 AT EA TRUSS
2x BLKG BTWN TRUSSES
W/ 16d SINKERS AT 6" OC
DBL 2x PL - LAM W/ 16d SINKERS AT 8"
OC - AT SPLICES LAP 4'-0" AND NAIL W/
(2) ROWS 16d SINKERS AT 8" OC
SHTG PER SW SCHED
HDR BEYOND PER PLAN

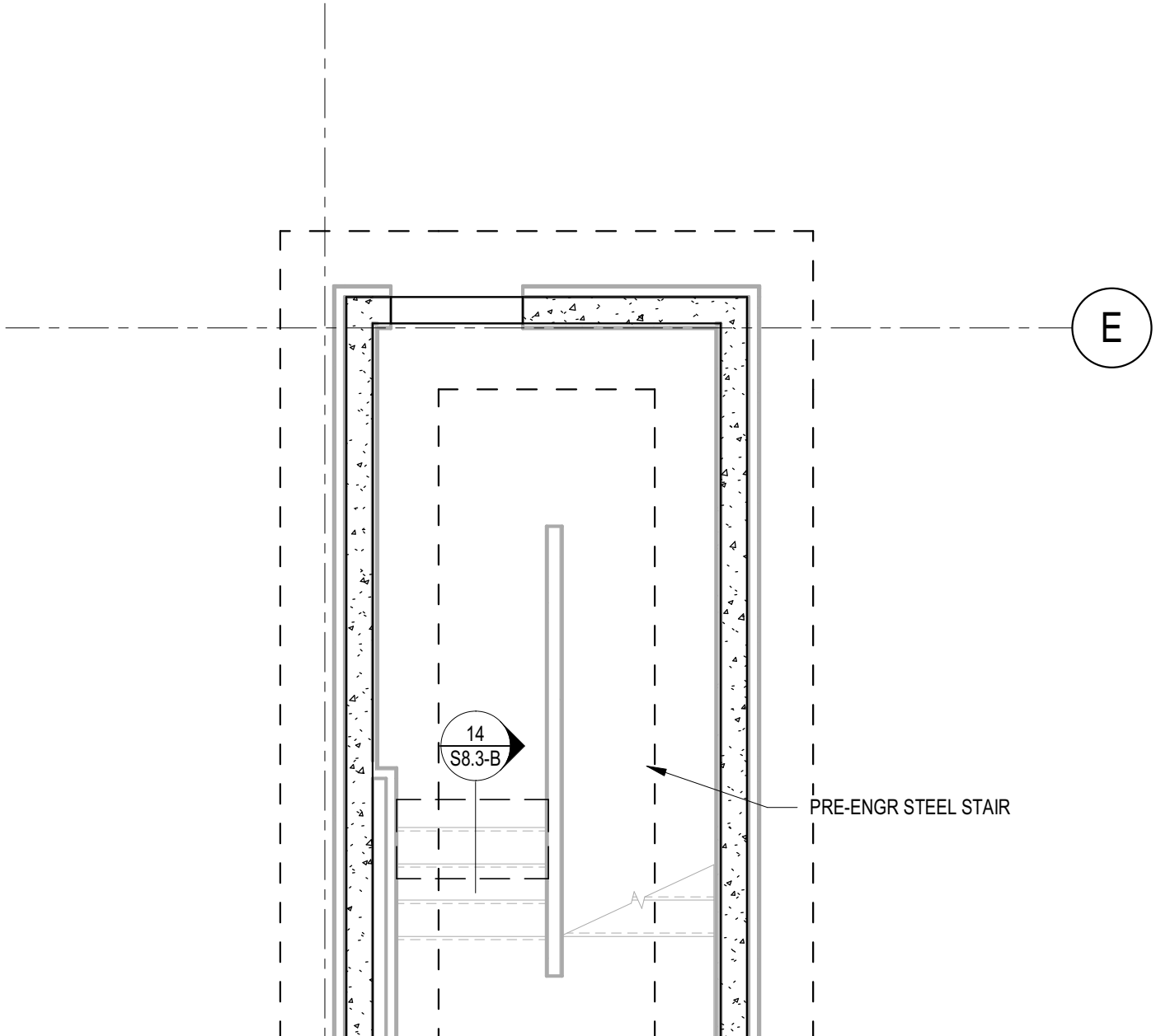

2x VENTED BLKG
ROOF BNDY NAILING
ROOF SHTG PER PLAN
VENT
EXTEND PRE-ENGR
TRUSS TOP CHORD
TO RIDGE PER PLAN
GIRDER TRUSS PER PLAN -
DESIGN TRUSS TO TRANSFER
HORIZ SHEAR OF 200 PLF BTWN
TOP & BOTTOM CHORD
CONT 2x LEDGER - SIZE TO
MARCH CEILING FRAMING
ATTACH TO GIRDER TRUSS
BOT CHORD W/ (2) ROWS 16d
AT 12" OC
PRE-ENGR TRUSS TO GIRDER
TRUSS CONN BY TRUSS
MFR/SUPPLIER WHERE WALL
SIMPSON LUS HGR AT EA
JOIST

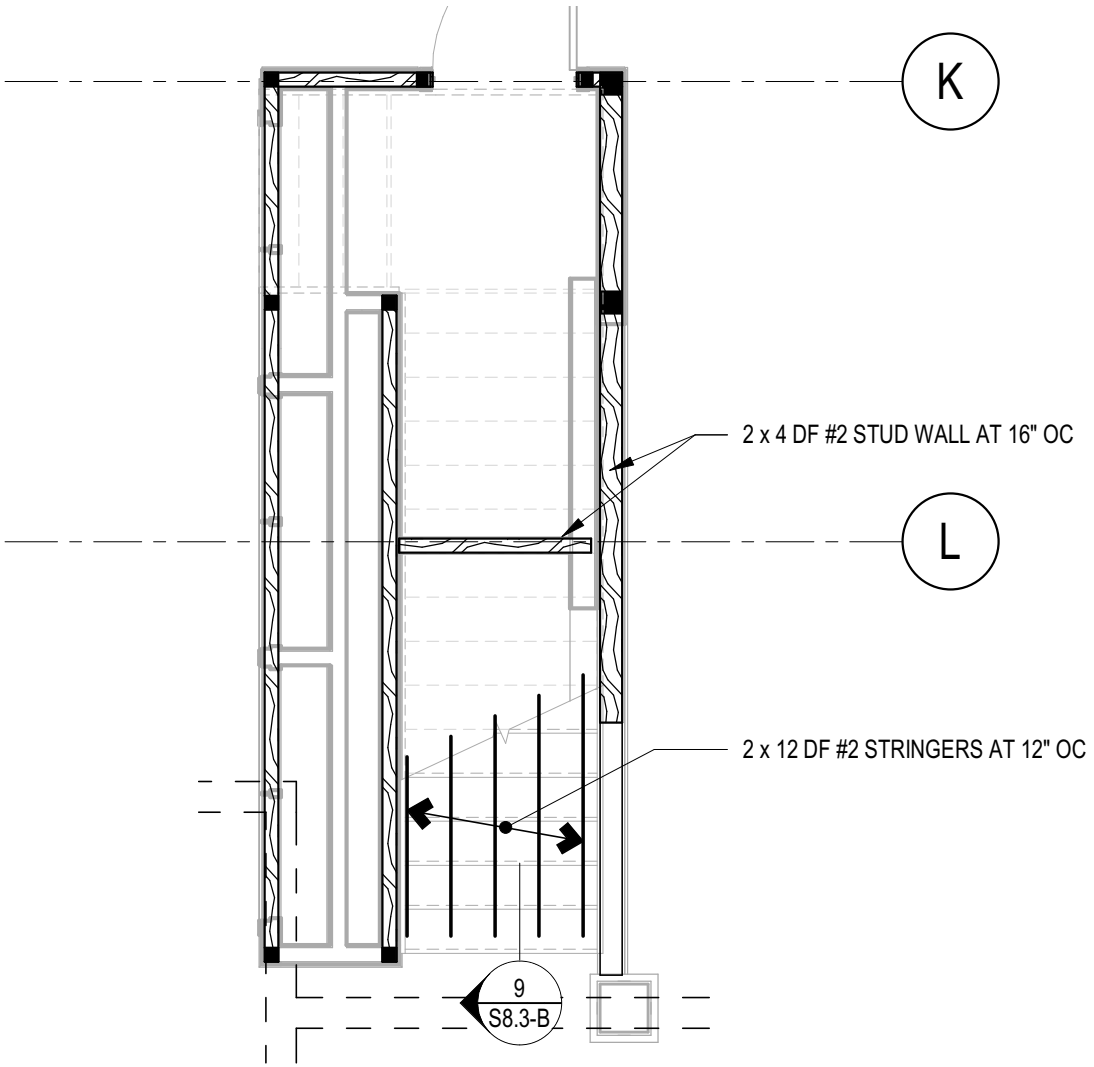

ROOF BNDY NAILING
2x BLKG BTWN OUTRIGGERS
(2) 16d SINKERS
2 x 4 OUTRIGGERS AT 24" OC
PER ARCH
2'-0" MAX
SIMP H1 EA OUTRIGGER
SW EDGE NAILING
ADDL 2x STIFFENER BY TRUSS MFR AS REQD AT GABLE END WEB VERTS
PRE-ENGR GABLE END TRUSS W/ VERT AT 24" OC MAX MFR/ SUPPLIER SHALL DESIGN FOR OUT OF PLANE WIND FORCE
(2) 16d SINKERS AT EA 1x AT EA TRUSS
SW EDGE NAILING
CONN TRUSS TO BLKG W/ 16d SINKERS AT 6" OC
FLAT 1 x 4 x 8'-0" DF #2 AT 6'-0" OC
SW EDGE NAILING
2 x 6 CONT FOR GWB CEILING - ATTACH W/ 16d SINKERS AT 6" OC INTO DBL TOP PL
SHTG PER PLAN AND SW


SEE 1/S7.2-B FOR CALLOUTS IN COMMON
PRE-ENGR TRUSS PER PLAN W/ SLOPED BOTTOM CHORD - SEE

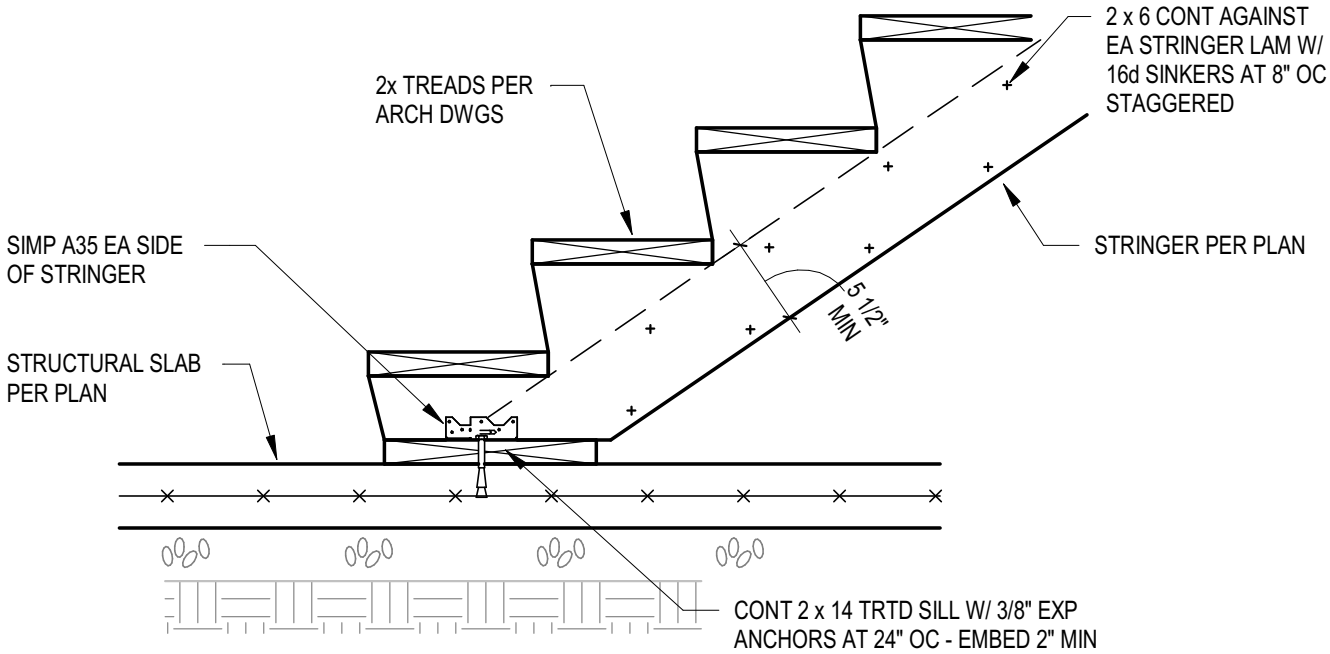

SPLICE GIRDER
TRUSS AT CL WALL
BLKG BEYOND
PER
8
S7.3-B
GIRDER TRUSS
PER PLAN EA SIDE
OF STUD WALL
SIMPSON LGT AT EA
GIRDER TRUSS
STUD WALL
PER PLAN


PRE-ENGR TRUSSES
PER PLAN


E
14
S8.3-B
PRE-ENGR STEEL STAIR



LOBBY STAIR L1 PLAN



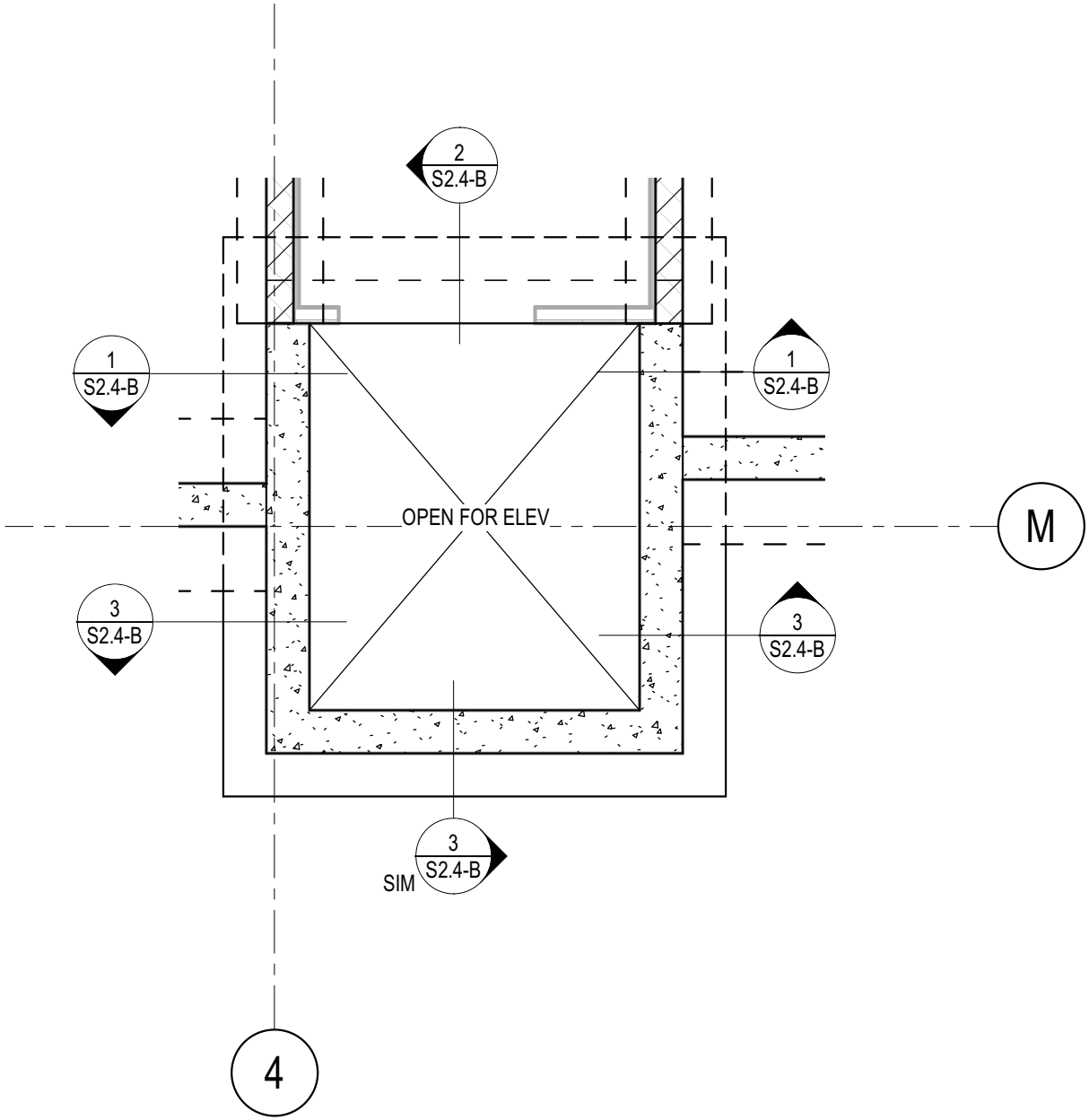
TYP STRINGER TO SLAB CONNECTION

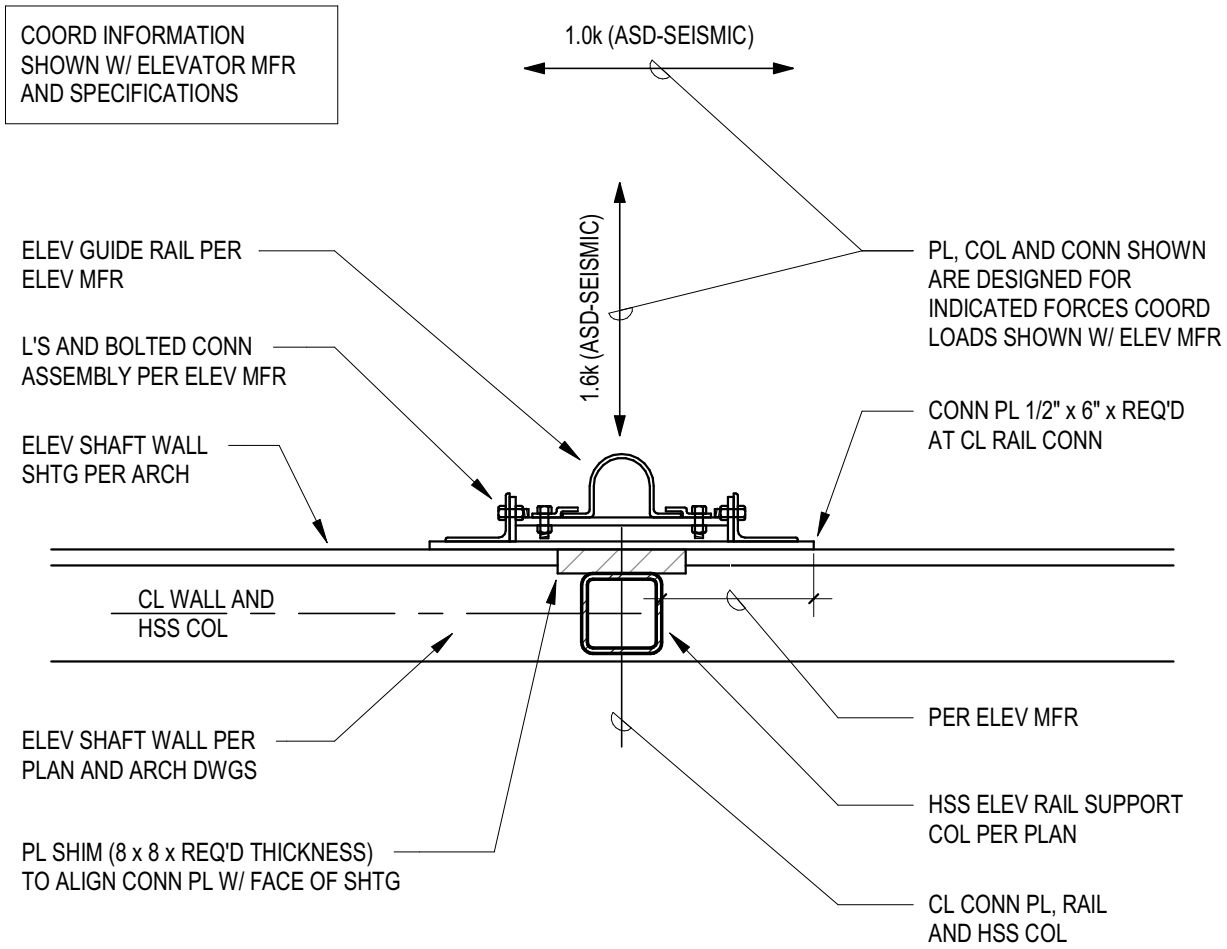
1

SECTION

1" = 1'-0" 1 / S8.3B

2

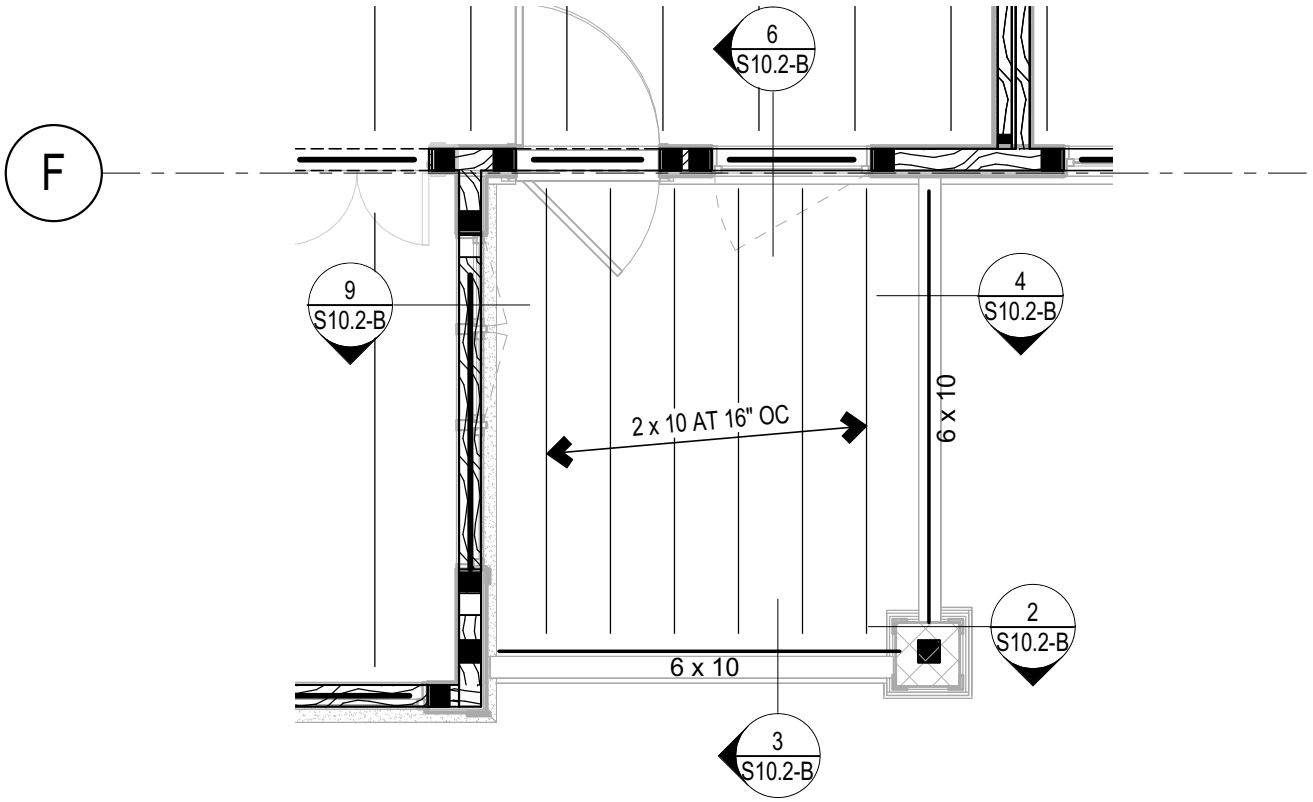

2
S2.4-B
1
S2.4-B
1
S2.4-B
OPEN FOR ELEV
M
3
S2.4-B
3
S2.4-B
3
S2.4-B
SIM
4

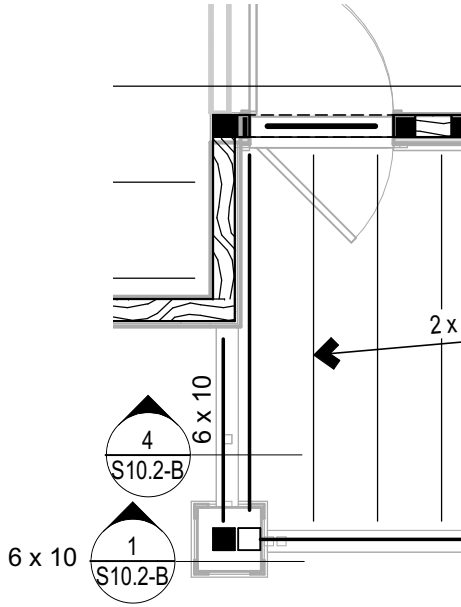


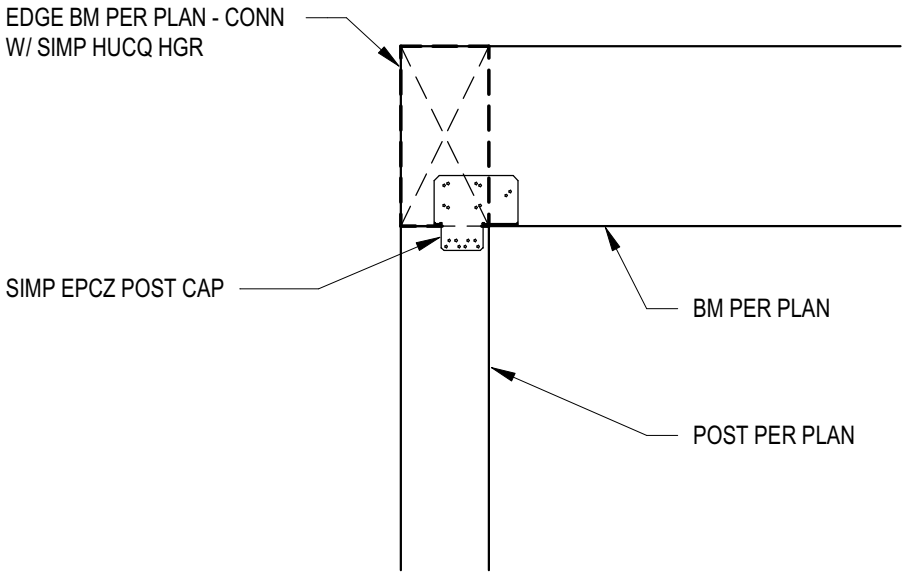
TYP ELEV RAIL CONN

1 SECTION

1" = 1'-0" 1 / S9.2B

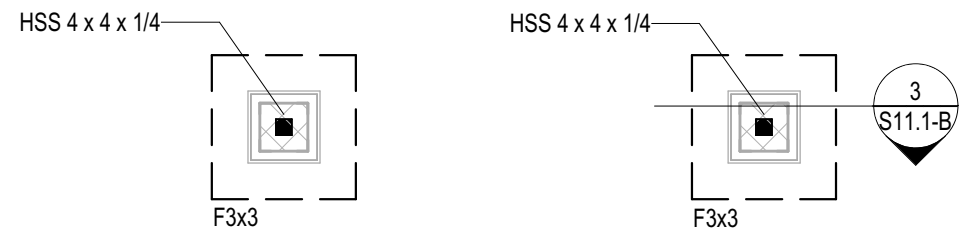

F
6
S10.2-B
9
S10.2-B
4
S10.2-B
2 x 10 AT 16" OC
6 x 10
2
S10.2-B
6 x 10
3
S10.2-B

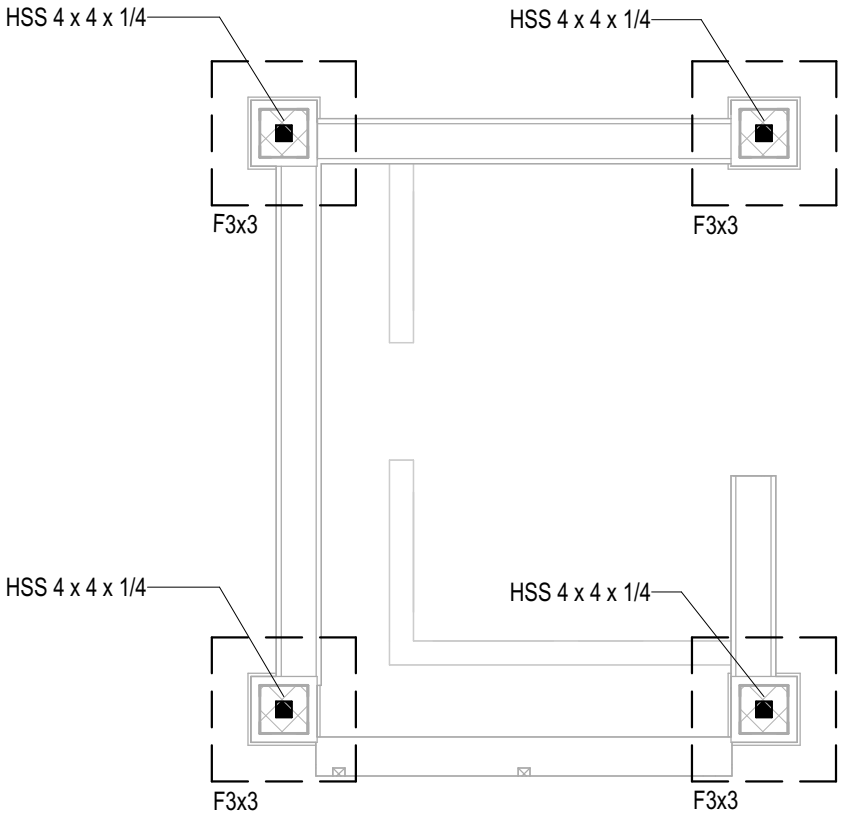

2 x
6 x 10
4
S10.2-B
6 x 10
1
S10.2-B

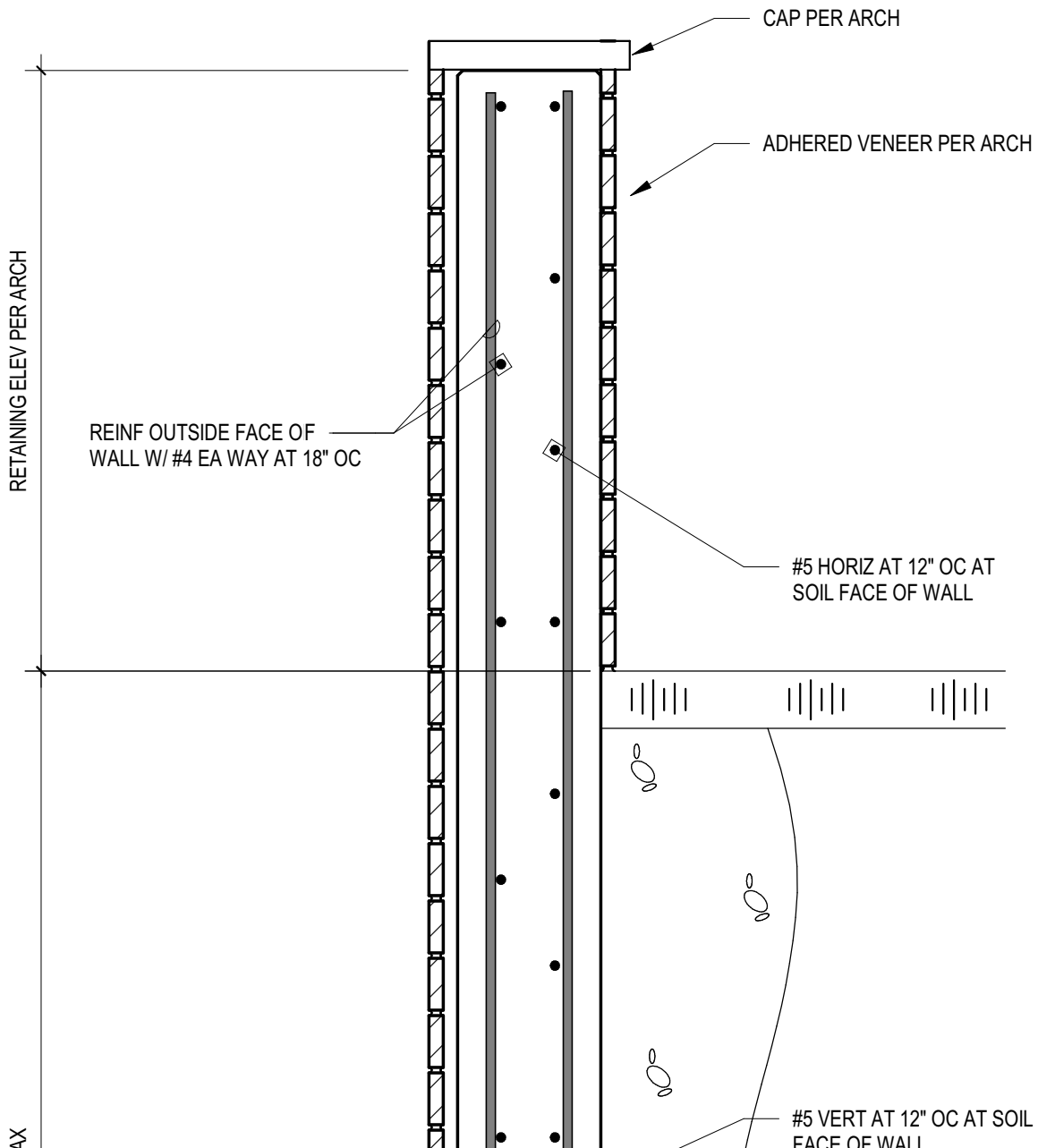


EDGE BM PER PLAN - CON
W/ SIMP HUCQ HGR

SIMP EPCZ POST CAP


HSS 4 x 4 x 1/4
F3x3
HSS 4 x 4 x 1/4
F3x3
3
S11.1-B


HSS 4 x 4 x 1/4
F3x3
HSS 4 x 4 x 1/4
F3x3
HSS 4 x 4 x 1/4
F3x3
HSS 4 x 4 x 1/4
F3x3


CAP PER ARCH
ADHERED VENEER PER ARCH
RETAINING ELEV PER ARCH
REINF OUTSIDE FACE OF
WALL W/ #4 EA WAY AT 18" OC
#5 HORIZ AT 12" OC AT
SOIL FACE OF WALL
#5 VERT AT 12" OC AT SOIL
FACE OF WALL
MAX