ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, THE 2021 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC).	ΓI	. FOUNDATION VALUES LISTE
DESIGN LOADING CRITERIA		THEREFORE N NOTIFY THE S
ROOF SNOW LOAD25 PSFFLOOR LIVE LOAD125 PSF OR 2,000 LBSSTAIR AND EXITS LIVE LOAD100 PSF		FOOTINGS SH OR BOTH) AT SHOWN ON PL
GUARDRAILS/BALCONY RAILS (EXIT FACILITY)50 PLF OR 200 LBS.GUARDRAILS/BALCONY RAILS (OTHER THAN EXIT FACILITY)20 PLF OR 200 LBS.MECHANICAL UNITSWEIGHTS FURNISHED BY MANUFACTURER		OF FOOTINGS FOOTINGS SH BACKFILL BE
<u>WIND</u> : ANALYSIS PROCEDURE: ASCE 7-16 CHAPTER 27 "PART I - BUILDINGS OF ALL HEIGHTS" RISK CATEGORY II 98 MPH		SUBSURFACE THE STRUCTUR REPORT:
EXPOSURE "B" TOPOGRAPHIC FACTOR Kzt = 1.0		ALLOWABLE S
EARTHQUAKE : ANALYSIS PROCEDURE: IBC "EQUIVALENT LATERAL FORCE PROCEDURE" SEISMIC DESIGN CATEGORY (SDC) = D RISK CATEGORY = II SEISMIC SITE CLASS = D IMPORTANCE FACTOR Ie = I.0 MAPPED MCE Ss = I.26; SI = 0.43 DESIGN ACCELERATION Sds = I.01; SdI = 0.54		SEISMIC SURC PASSIVE SOIL SOIL COEFFIC SOIL DENSITY
<u>LATERAL LOADS</u> ARE TRANSFERRED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR WALLS OR BRACED FRAMES. MOMENTS, SHEARS AND ROTATIONAL FORCES ARE BASED ON THE RIGIDITY OF EACH SHEAR WALL OR BRACED FRAME AND ARE CARRIED BY THE SHEAR WALLS OR BRACED FRAMES TO THE FOUNDATION.	8	DEMOLITION: SHALL BE INS TO THE WORK PLANS. SAW SAVED. DEM
<u>STRUCTURAL DRAWINGS</u> SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.		STRUCTURE. SYSTEMS TO A. ALL NEM
<u>CONTRACTOR</u> SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.		SAW CUT B. VERIFY C. SMALL F
<u>CONTRACTOR</u> SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.		D. WHERE N EXISTING OTHERW
<u>CONTRACTOR</u> SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THEIR WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.	19	. <u>CONCRETE</u> SI CONSTRUCTIC 28-DAY STRI SLABS-ON-GI
<u>CONTRACTOR-INITIATED</u> CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.	SLABS-ON-G SECTION 1904 CEMENT PER (BEFORE THE	
DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. WHERE INFORMATION ON THE DRAWINGS IS IN CONFLICT WITH THE SPECIFICATIONS, THE MORE STRINGENT SHALL APPLY, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. DO NOT SCALE THE DRAWINGS.		FOOTINGS AN FOR FOOTING THE MINIMUM PERFORMANG APPROVAL 1 THE PERFORI COARSE AGO
<u>ALL STRUCTURAL SYSTEMS</u> WHICH ARE COMPOSED OF FIELD ERECTED COMPONENTS SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.		CONCRETE Y ADMIXTURES PERCENTAGE PERFORMANC
<u>SHOP DRAWINGS</u> FOR REINFORCING STEEL (FOR BOTH CONCRETE AND MASONRY CONSTRUCTION), STRUCTURAL STEEL, AND METAL DECKING, SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.		THE ATTENTION ONLY THAT IN CONTRACTOR
CONTRACTOR SHALL SUBMIT WALL ELEVATION DRAWINGS OF AT LEAST 1/8" = 1'-0" SCALE INDICATING LOCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS FOR REVIEW PRIOR TO CONSTRUCTION. CONTRACTOR SHALL COORDINATE WITH REINFORCEMENT SHOP DRAWINGS.		ALL CONCRE AIR-ENTRAIN CONCRETE S CONCRETE TO
<u>SHOP DRAWING REVIEW</u> : DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.		0. <u>REINFORCING</u> DETAILED (IN CONTINUOUS 1 BARS AT ALI DIAMETERS (
SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND	2	318, CLASS E SLABS EXTEN . <u>CONCRETE P</u>
THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.		FOOTINGS AN FORMED SUR (#6 BARS OR (#5 BARS OR
DEFERRED SUBMITTALS OF DESIGN BUILD COMPONENTS SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON		COLUMN TIES
CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE AND SHALL INCLUDE DESIGN CALCULATIONS WITH THE ENGINEER'S STAMP.	2:	2. <u>CAST-IN-PLA</u> OF DOOR AN AND LOCATIO
THE FOLLOWING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT: STAIRS, RAILINGS, AND METAL BUILDINGS		ARCHITECTUR TEXTURE, ANI AND PRECAS
MECHANICAL UNIT CONNECTIONS TO THE BUILDING SHALL BE DESIGNED BY THE MANUFACTURER FOR THE DESIGN CRITERIA AND CONDITIONS SHOWN ON THE STRUCTURAL DRAWINGS. MANUFACTURER SHALL SUBMIT DETAIL DRAWINGS AND CALCULATIONS, BOTH OF WHICH BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER. MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR DESIGN, CODE CONFORMANCE, AND CONNECTION OF THE UNIT TO THE BASIC STRUCTURE. ALL NECESSARY BRACING, TIES, ANCHORAGE, DISTRIBUTION MEMBERS, AND SIMILAR	2:	3. <u>NON-SHRINK</u> APPROVED N MANUFACTUR THE MATERIA
ELEMENTS SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH SUBMITTED DRAWINGS AND CALCULATIONS.	24	4. <u>POLYSTYREN</u> SHALL BE RI MINIMUM COM RESISTANCE
SPECIAL INSPECTION: CONCRETE CONSTRUCTION, MASONRY CONSTRUCTION, STRUCTURAL STEEL FABRICATION AND ERECTION (INCLUDING FIELD WELDING AND HIGH-STRENGTH FIELD BOLTING), METAL DECK INSTALLATION, EXPANSION BOLTS AND THREADED EXPANSION INSERTS, SCREW ANCHORS, AND EPOXY GROUTED INSTALLATIONS SHALL BE SUPERVISED IN ACCORDANCE WITH IBC SECTIONS 1704 & 1705 AND THE PROJECT SPECIFICATIONS BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE OWNER. THE TESTING AGENCY AND INSPECTOR SHALL BE REGISTERED WITH WABO AND SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE OWNER, ARCHITECT, STRUCTURAL ENGINEER, CONTRACTOR AND BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET		RESISTANCE BLOCK JOIN RECOMMEND

### GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

#### <u>GEOTECHNICAL</u>

5: ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE BASED N ON PREVIOUSLY PERMITTED BUILDING PLANS DATED DECEMBER 22, 1995 AND BE VERIFIED IN THE FIELD. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED. TURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

BEAR ON FIRM, UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL ST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS IT BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD. UNLESS OTHERWISE NOTED, BE CENTERED UNDER COLUMNS OR WALLS ABOVE.

ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR INAGE.

DESIGN IS BASED ON THE FOLLOWING VALUES FROM THE REFERENCED GEOTECHNICAL

BEARING PRESSURE	2,000 PSF
PRESSURE (RESTRAINED/UNRESTRAINED)	55 PCF/35 PCF
GE PRESSURE (RESTRAINED/UNRESTRAINED)	8H PSF/5H PSF
ESSURE	350 PCF
OF FRICTION	0.35
	120 PCF

#### <u>RENOVATION</u>

RIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING LED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE QUENCES. EXISTING REINFORCING SHALL BE SAVED WHERE AND AS NOTED ON THE FING, IF AND WHERE USED, SHALL NOT CUT EXISTING REINFORCING THAT IS TO BE ION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR

ENINGS THROUGH EXISTING WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY WHEREVER POSSIBLE

EXISTING CONDITIONS AND LOCATION OF MEMBERS PRIOR TO CUTTING ANY OPENINGS. D OPENINGS SHALL BE ACCOMPLISHED BY CORE DRILLING, IF POSSIBLE REINFORCING TERMINATES AT EXISTING CONCRETE, REBAR DOWELS EPOXIED INTO THE NCRETE SHALL BE PROVIDED TO MATCH HORIZONTAL REINFORCING, UNLESS OTED ON PLANS.

#### CONCRETE

BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301. PLERANCES SHALL NOT EXCEED THOSE LISTED IN ACI 117. CONCRETE SHALL ATTAIN A H OF F'C = 3,000 PSI. ALL CONCRETE EXPOSED TO THE WEATHER AND ALL GARAGE SHALL ATTAIN A 28-DAY STRENGTH I'C OF 3,500 PSI IN ACCORDANCE WITH IBC ND ACI 318 TABLE 19.3.2.1. MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF C YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS PITION OF ADMIXTURES). THE WATER/CEMENT RATIO SHALL NOT EXCEED 0.55 FOR 45 FOR ALL SLABS AND EXPOSED CONCRETE UNLESS OTHERWISE NOTED. EXCEPT ND SLAB ON GRADE, AGGREGATE SIZE SHALL NOT EXCEED 3/4".

UNT OF CEMENT AND THE MAXIMUM SLUMP MAY BE CHANGED IF A CONCRETE IX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT FOR NEEKS PRIOR TO PLACING ANY CONCRETE. (THE W/C RATIO LIMITS STILL APPLY). E MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND ATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. CHEMICAL FLY ASH SHALL CONFORM TO ASTM C494 AND C618 RESPECTIVELY. FLY ASH TOTAL CEMENTITIOUS MATERIAL SHALL NOT EXCEED 20%. THE USE OF A X REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE BROUGHT TO THE OWNER. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES MATION PRESENTED CONFORMS GENERALLY TO CONTRACT DOCUMENTS. INTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

ITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AGENT CONFORMING TO ASTM C260. TOTAL AIR CONTENT FOR FROST-RESISTANT BE IN ACCORDANCE WITH ACI 318 TABLE 19.3.2.1 AND IBC SECTION 1904. ALL CEIVE A STEEL TROWELED FINISH SHALL NOT BE AIR-ENTRAINED.

EL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT SI), AND SHALL BE PING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 AND 318. LAP ALL FORCEMENT #5 AND SMALLER 60 BAR DIAMETERS, 2'-0" MINIMUM. PROVIDE CORNER L AND FOOTING INTERSECTIONS. LAP CORNER BARS #5 AND SMALLER 60 BAR -O" MINIMUM. LAPS OF LARGER BARS SHALL BE MADE IN ACCORDANCE WITH ACI 2.0 VIDE (2) #5 MIN. U.N.O. TRIM BARS AROUND ALL OPENINGS IN CONCRETE WALLS OR 5 2'-6" PAST CORNERS, TYPICAL.

<u>CTION (COVER) FOR REINFORCING STEEL</u> SHALL BE AS FOLLOWS:

HER UNFORMED SURFACES CAST AGAINST EARTH	3"
IS EXPOSED TO EARTH (I.E. WALLS BELOW GROUND) OR WEATHER GER) LLER)	2"  - /2"
PIRALS AND BEAM STIRRUPS	- /2"

GREATER OF (BAR DIAMETER PLUS 1/8") OR 3/4" S (INTERIOR FACE)

ONCRETE: SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS NDOW OPENINGS IN ALL CONCRETE WALLS. SEE MECHANICAL DRAWINGS FOR SIZE MISCELLANEOUS MECHANICAL OPENINGS THROUGH CONCRETE WALLS. SEE DRAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, HER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES, BOTH CAST-IN-PLACE

IT SHALL BE NON-METALLIC CONFORMING TO ASTM CIIOT AND BE FURNISHED BY AN FACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO I WHICH IT IS PLACED (5000 PSI MINIMUM).

IGID INSULATION) LIGHTWEIGHT STRUCTURAL FILL PLACED BELOW CONCRETE SLABS ELLULAR POLYSTYRENE CONFORMING TO ASTM D6817 OR ASTM C578, WITH A SSIVE RESISTANCE OF 5 PSI @ 1% DEFORMATION AND A MINIMUM COMPRESSIVE 5 PSI @ 10 % DEFORMATION, U.O.N. MAXIMUM DENSITY SHALL BE 2.0 PCF. OFFSET TWEEN ADJACENT LAYERS AND ATTACH BLOCKS PER THE MANUFACTURER'S

25. EXPANSION BOLTS INTO CONCRETE SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-2302 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

<u>ANCHORAGE</u>

- 26. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-1385 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 27. EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3037 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR JAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 28. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH IAPMO UES REPORT NO. ER-240 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 29. SCREW ANCHORS INTO CONCRETE SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3027 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 30. SCREW ANCHORS INTO GROUT FILLED CMU SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3056 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 31. DRIVE PINS, SHOT PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE LOW VELOCITY TYPE FASTENERS AS MANUFACTURED BY HILTI CORPORATION. WHEN CALLED FOR IN THE DRAWINGS, PROVIDE THE APPROPRIATE FASTENER AS NOTED IN THE TABLE BELOW FOR EACH GIVEN APPLICATION. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORTS NO. ESR-2269 FOR THE X-U FASTENERS AND ESR-2379 FOR THE X-CP FASTENERS. MINIMUM EMBEDMENT IN CONCRETE SHALL BE I" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3" TO NEAREST CONCRETE EDGE AND 4" CENTER TO CENTER SPACING. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES.

ALLOWABLE APPLICATION	ALLOWABLE <u>FASTENER TYPE</u>	SHEAR CAF
2X TREATED LUMBER TO CONCRETE (2000 PSI MIN.)	X-CP 72 P8 S23 w/ 1.33" EMBED	250
LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN. TO CONCRETE (2000 PSI MIN.)	X-U 27 P8 SI5	190
2X LUMBER TO STRUCTURAL STEEL (3/16" MIN., 36 OR 50 KSI)	X-U 52 MX PLUS R-23 WASHERS	250
LIGHT GAUGE STEEL 43 & 33 MILS (18 & 20 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	445
LIGHT GAUGE STEEL 97, 68 \$ 54 MILS (12, 14 \$ 16 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	720
LIGHT GAUGE STEEL (ALL GA.) TO STRUCTURAL STEEL (3/4" AND GREATER)	X-U 19 P8 TH	350
2X TREATED LUMBER TO GROUTED CMU	X-CP 72 P8 523	105
	X-11 37 PB 615	220

LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN.	X-U 32 P8 515	220
TO GROUTED CMU		

- 32. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO CONCRETE SHALL BE INSTALLED USING "HIT-RE 500 V3" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3814, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.
- 33. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO GROUT FILLED CMU SHALL BE INSTALLED USING "HIT HY 270" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-4143, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

#### MASONRY

34. CONCRETE MASONRY UNIT WALLS SHALL BE CONSTRUCTED OF MEDIUM WEIGHT UNITS CONFORMING TO ASTM C90, LAID IN A RUNNING BOND. CONTACT ENGINEER FOR RE-DESIGN OF REINFORCING WHERE STACK BOND LAYOUT IS REQUIRED. LINEAR SHRINKAGE SHALL NOT EXCEED 0.065%. MORTAR SHALL BE TYPE "S" IN ACCORDANCE WITH ASTM C270. GROUT SHALL CONFORM TO IBC REQUIREMENTS AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS, DESIGN F'M = 2,000 PSI. FULL STRESSES ARE REQUIRED. STRENGTH SHALL BE VERIFIED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH TMS 602-16.

PROVIDE (2) #5 ((2)#4 @ 6" AND 4" WALLS) VERT. AT EACH SIDE OF OPENINGS, AT WALL CORNERS AND INTERSECTIONS AND AT FREE ENDS OF WALLS AND (2) #4 HORIZ. AT ELEVATED FLOOR AND ROOF LEVELS, AT TOPS OF WALLS AND ABOVE AND BELOW ALL OPENINGS. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAMS. EXTEND REINFORCEMENT AROUND OPENINGS 2'-O" BEYOND FACE OF OPENING. IF 2'-O" IS UNAVAILABLE, EXTEND AS FAR AS POSSIBLE AND HOOK. PROVIDE CORNER BARS TO LAP HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS. UNLESS NOTED OTHERWISE, LAP ALL REINFORCING STEEL IN CMU 48 BAR DIAMETERS, 2'-O" MINIMUM.

ALL CELLS ARE TO BE SOLID GROUTED UNLESS NOTED AS PARTIAL GROUTING. FOR PARTIAL GROUTING FILL ALL CELLS CONTAINING REINFORCEMENT OR EMBEDDED ITEMS AND ALL CELLS BELOW GRADE WITH GROUT. ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING AND SHALL BE HELD AT TOP, BOTTOM AND 192 BAR DIAMETERS (MAX.) O.C. PER TMS 602 SPECIFICATION 3.5, GROUT POURS SHALL NOT EXCEED 5'-4" IN HEIGHT UNLESS A TEST PANEL IS CONSTRUCTED BY THE MASON AND APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE CLEANOUT HOLES AT BOTTOM OF ALL CELLS CONTAINING REINFORCEMENT FOR ALL GROUT POURS OVER 5'-4" IN HEIGHT. PROVIDE | 1/2 IN. GROUT KEYS BETWEEN EACH POUR.

APACITY (LBS) TENSION CAPACITY (LBS)

17	5	
le	5	
T.	5	
3	60	
5	35	
З	75	
IC	00	
2	25	

35. STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE BASED ON THE LATEST EDITIONS OF THE A.I.S.C. SPECIFICATIONS AND CODES:

A. AISC - STEEL CONSTRUCTION MANUAL, 15<sup>TH</sup> EDITION

B. AISC 303-16 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. C. 2014 RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS.

36. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

#### TYPE OF MEMBER

		-
A. WIDE FLANGE (W AND WT) SHAPES	A992	50 KSI
B. ALL OTHER SHAPES	A36	36 KSI
C. PLATE	A36 OR A572	36 KSI (MIN)
D. PIPE SECTIONS	A53 (TYPE E OR S, GRADE B)	35 KSI
E. STRUCTURAL TUBING (SQUARE OR RECTANGULAR)	A500 (GRADE C)	50 KSI
F. ANCHOR BOLTS AND THREADED RODS	F1554 (GRADE 36) OR	36 KSI
(EMBEDDED IN MASONRY OR CONCRETE)	FI554 (GRADE 55, SUPP. SI)	55 KSI
G. CONNECTION BOLTS	F3125 GRADE A325-N	92 KSI
(7/8" ROUND, UNLESS SHOWN OTHERWISE)		
H. HEADED SHEAR STUDS	A29	49 KSI
I. THREADED RODS	A36	36 KSI
J. STAINLESS STEEL	AISI 316L	30 KSI

ASTM SPECIFICATION

Fy

SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE SHALL NOT BE ALLOWED WITHOUT PRIOR APPROVAL OF THE ENGINEERALL STEEL TO BE FIREPROOFED SHALL BE LEFT UNPAINTED. ALL OTHER STEEL SHALL HAVE ONE COAT OF APPROVED SHOP PAINT.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER OR EARTH SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A123. GALVANIZE BOLTS AND SIMILAR THREADED FASTENERS EXPOSED TO WEATHER OR EARTH IN ACCORDANCE WITH ASTM A153. ALL FIELD WELDS EXPOSED TO WEATHER OR EARTH SHALL BE COATED WITH BRUSH APPLIED ZINC RICH PAINT COMPLYING WITH ASTM A780 (Z.R.C. OR EQUIVALENT).

A MINIMUM OF TWO BOLTS ARE REQUIRED FOR ALL CONNECTIONS. ALTERNATE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ENGINEER.

ALL MEMBERS ARE TO BE ERECTED WITH THE NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM CAMBER ON THE DRAWINGS IS THE UPWARD CAMBER REQUIRED IN THE BEAM AS DELIVERED TO THE JOBSITE. CONTRACTOR TO CONSIDER CAMBER LOSS, IF ANY, DUE TO SHIPPING AND HANDLING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES AND UNEQUAL PARTS

37. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL SHALL CONFORM TO SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

38. ALL A325 CONNECTION BOLTS SHALL BE INSTALLED TO THE SNUG-TIGHT CONDITION PER RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. ALL NUTS SHALL CONFORM TO ASTM A563. ALL WASHERS SHALL CONFORM TO ASTM F436 OR ASTM F959 TYPE 325. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS OTHERWISE NOTED.

39. ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.S.C. AND A.W.S. STANDARDS AND SHALL BE PERFORMED BY W.A.B.O. CERTIFIED WELDERS USING ETO XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.W.S.) SHALL BE USED. ALL WELDING OF STAINLESS STEEL SHALL USE E309 ELECTRODES WITH A GMAW PROCESS. ALL WELDING SHALL BE PERFORMED BY WELDERS WITH AWS / W.A.B.O. CERTIFICATION WITH THE MATERIAL AND METHOD REQUIRED.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. WELDS SHOWN ON DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON PLATE THICKNESS. MINIMUM WELDING SHALL BE 3/16-INCH. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD ARROWS ARE SHOWN WHERE A FIELD WELD IS REQUIRED BY THE STRUCTURAL DESIGN; THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL DELIVERY AND ERECTION. SEE THE SPECIFICATIONS AND DRAWINGS FOR ADDITIONAL WELDING REQUIREMENTS, ESPECIALLY AT SPECIAL MOMENT RESISTING FRAMES AND OTHER SEISMIC CRITICAL WELDS.

40. WELDING OF LATERAL FORCE RESISTING MEMBERS SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS DI.I (INCLUDING AWS DI.8 SEISMIC SUPPLEMENT) AND APPROVED BY THE STRUCTURAL ENGINEER BEFORE WORK BEGINS. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER METAL MANUFACTURER. WELDING ELECTRODES SHALL BE ETOTT-K2 OR ETOT-6 WITH A MINIMUM SPECIFIED CHARPY V-NOTCH (CVN) OF 20 ft-lbs AT -20 DEGREES FAHRENHEIT AND 40 ft-lbs AT 70 DEGREES FAHRENHEIT. REMOVE BOTTOM FLANGE WELD TAB AT MOMENT FRAME CONNECTIONS AND REINFORCE WITH 5/16" FILLET WELD IN CONFORMANCE WITH FEMA-353 RECOMMENDATIONS. WELD ACCESS HOLE DETAILING AT MOMENT FRAME CONNECTIONS SHALL CONFORM WITH FEMA-350 AND FEMA-353 RECOMMENDATIONS.

41. METAL FLOOR AND ROOF DECKING - PROVIDE SIZE, TYPE, GAUGE, AND ATTACHMENT TO THE SUPPORTING STRUCTURE AS SHOWN ON THE PLANS. ALTERNATES MUST BE CONNECTED ACCORDING TO PUBLISHED I.C.C. OR IAPMO UES CRITERIA FOR DIAPHRAGM SHEARS SHOWN. PROVIDE SHORING WHERE REQUIRED PER MANUFACTURER'S PUBLISHED CRITERIA. ALL DECKING SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.

42. HEADED STUDS FOR COMPOSITE CONNECTION OF STRUCTURAL STEEL TO CAST-IN-PLACE CONCRETE SHALL BE MANUFACTURED FROM MATERIAL CONFORMING TO ASTM A29 AND SHALL BE WELDED IN CONFORMANCE WITH A.W.S. REQUIREMENTS.

43. DEFORMED BAR ANCHORS (DBA) SHALL BE TYPE D2L ANCHORS BY NELSON STUD WELDING DIVISION, TRW ASSEMBLIES AND FASTENERS GROUP (OR EQUIVALENT). ANCHORS SHALL BE MADE FROM COLD ROLLED, DEFORMED STEEL CONFORMING TO ASTM A1064.

See Slab Construction permits for Building, Planning and Engineering review

Separate approval by L & I is required for factory built eFRAME<sup>™</sup> Enclosures. Contact L & I Factory Assembled Structures and provide verification for inspections.

THE APPROVED CONSTRUCTION PLANS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

Approval of submitted plans is not an approval of omissions or oversight by this office or noncompliance with any applicable regulations of local government The contractor is responsible for making sure that the building complies with all applicable building codes and regulations of the local government.



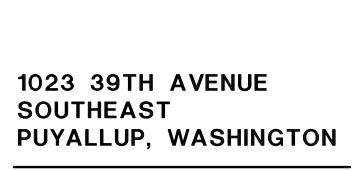
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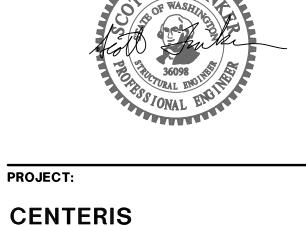
## GENERAL STRUCTURAL NOTES SHEET NO.

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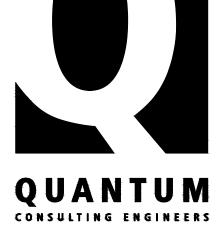




Centers for Information Systems

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SUITE 323

ΡLΑ	. COLD-FORMED STEEL FRAMING MEMBERS SHALL BE OF THE SHAPE, SIZE, AND GAUGE SHOWN ON
	THE PLANS. ALL FRAMING MEMBERS SHALL COMPLY WITH I.C.C. REPORT NO. ESR-3064P. NOTATIONS ON THE DRAWINGS, RELATING TO MEMBER TYPES AND SIZES OR MISCELLANEOUS FRAMING ITEMS, REFER TO CATALOG NUMBERS OF MEMBERS MANUFACTURED BY THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA). PRODUCTS BY OTHER MANUFACTURERS MAY BE SUBSTITUTED FOR FRAMING SHOWN, PROVIDED THEY ARE EQUIVALENT IN SHAPE, SIZE, STIFFNESS, AND STRENGTH. ALTERNATE FRAMING SHALL BE SUBJECT TO REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION. ALL COLD-FORMED STEEL FRAMING SHALL CONFORM TO THE A.I.S.I. "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
В	. MATERIAL:
	METAL FRAMING SHALL BE GALVANIZED UNLESS OTHERWISE NOTED, CONFORMING AS FOLLOWS: ASTM A653 SS GRADE 50, CLASS I OR 3 $F_{\rm V}$ = 50 KSI II8, 97, 68, AND 54 MIL
	ASTM A653 SS GRADE 33 $Fy = 33$ KSI 43 AND 33 MIL WHERE NOTED, PAINTED STUDS SHALL CONFORM TO:
	ASTM AIOII SS GRADE 50 Fy = 50 KSI 118, 97, 68, AND 54 MIL
C	. <u>WELDING</u> OF COLD-FORMED METAL FRAMING SHALL CONFORM TO AWS DI.3 AND SHALL BE PERFORMED BY WELDERS QUALIFIED TO PRODUCE THE SPECIFIED CLASSES OF WELD.
	MALL FRAMING: ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 400S162-43 ( 16" O.C. AT INTERIOR WALLS AND 600S162-43 AT 16" O.C. AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS. TWO 800S162-54 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS UNLESS OTHERWISE NOTED JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS. SOLID BLOCKING FOR MULTI-STUD OR STEEL COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW. PROVIDE CONTINUOUS FULL WIDTH BLOCKING AT 1/3 POINTS OF ALL STUD WALLS UNLESS NOTED OTHERWISE MAXIMUM GAP BETWEEN STUD AND TRACK AT ANY POINT SHALL NOT EXCEED 1/16-INCH. NO SPLICES ARE PERMITTED IN STUDS.
	ALL STUD WALLS SHALL HAVE THEIR BOTTOM TRACKS ATTACHED TO FRAMING BELOW WITH #10 SCREWS AT 16" O.C. OR ATTACHED TO CONCRETE WITH 5/32" DIAMETER DRIVE-PINS @ 16" O.C. UNLESS INDICATED OTHERWISE. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE WELDED OR SCREWED TO EACH OTHER IN ACCORDANCE WITH THE DETAILS. REFER TO THE PLANS AND SHEAT WALL SCHEDULE FOR REQUIRED SHEATHING AND STRAP BRACING. WHEN NOT OTHERWISE NOTED, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES AND GYPSUM SHEATHING ON EXTERIOR SURFACES SCREWED TO ALL STUDS, TOP AND BOTTOM TRACKS, AND BLOCKING WITH SCREWS AT 12" O.C. ALL SCREWS SHALL BE "GRABBER" TYPE FASTENERS COMPLYING WITH I.C.C. REPORT NO ESR-12TI ALL SPECIFIED PNEUMATIC FASTENERS SHALL BE ET&F, COMPLYING WITH I.C.C. REPORT NO. ESR-1777
	ALL BEARING STUDS SHALL BE LATERALLY SUPPORTED TO PREVENT WEAK AXIS BUCKLING WITH CENTER U-CHANNEL AT 1/3 POINTS AS SHOWN IN THE DETAILS AND CONNECTING EACH FLANGE TO GYPSUM WALLBOARD PER IBC SECTION 2508.1.
	TRACK SECTIONS SHALL BE UNPUNCHED AND HAVE AT LEAST I" FLANGES AND MATCH STUD THICKNESS.
	WALLS WHICH HAVE SHEATHING CONNECTED ON ONE SIDE ONLY SHALL HAVE UNSHEATHED FLANGES LATERALLY SUPPORTED IN ACCORDANCE WITH THE DETAILS.
15. <u>Met</u>	AL BUILDING SYSTEM (MBS)
م	. BUILDING MANUFACTURER SHALL DESIGN BUILDINGS FOR THE LOADS, SPANS AND CONDITIONS SHOWN ON THESE DRAWINGS.
B	. ALL COLUMNS SHALL BE DESIGNED ASSUMING THEY ARE FREE TO ROTATE, DO NOT FIX COLUMN BASES.
С	. DESIGN AND PROVIDE SUPPORTS AROUND OVERHEAD DOORS. A MINIMUM COLLATERAL LOADING OF IO PSF SHALL BE INCLUDED IN THE DESIGN OF THE ROOF TO ACCOUNT FOR MISCELLANEOUS DEAD LOAD. THE LATERAL DRIFT OF THE BUILDING SHALL BE LIMITED TO 0.02H, WHERE H IS THE HEIGHT OF THE BUILDING. ALL ROOF MEMBERS SHALL BE LIMITED TO A MAXIMUM TOTAL DEFLECTION OF L/180.
	COORDINATE ALL DETAILS WHICH ARE SHOWN ON THESE DRAWINGS WITH THE PREFABRICATED BUILDING DESIGN.
E	. COORDINATE THE FINAL FOUNDATION LOADING AND BASE PLATE CONFIGURATION WITH THE STRUCTURAL ENGINEER. FOUNDATION DESIGN SHOWN IN THESE PLANS MAY NEED TO BE REVISED BASED UPON THE FINAL DESIGN AND/OR COLUMN LOCATIONS.
F	. SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. DESIGN SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER. THE BUILDING MANUFACTURER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON THE BASIC STRUCTURE.
	STRUCTURAL OBSERVATION
STR BUT THE SPE	NOTED IN IBC SECTION 1704.6, STRUCTURAL OBSERVATION IS REQUIRED FOR THIS PROJECT. UCTURAL OBSERVATION MEANS THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM, INCLUDING NOT LIMITED TO, THE ELEMENTS AND CONNECTIONS AT SIGNIFICANT CONSTRUCTION STAGES AND COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND CIFICATIONS. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY OF INSPECTIONS REQUIRED BY IBC SECTIONS 110 AND 1704.
AS SPE ACC INDI	UR STRUCTURAL OBSERVATION, WE WILL SELECT PORTIONS OF WORK TO REVIEW CLOSELY AS WEL OBSERVE THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND CIFICATIONS. SUCH REVIEW PROCEDURES WILL BE CONDUCTED IN ACCORDANCE WITH COMMONLY CEPTED STANDARDS OF PRACTICE. THE BUILDING OFFICIAL UNDERSTANDS THAT SUCH PROCEDURE CATE ACTUAL CONDITIONS ONLY WHERE THE REVIEW IS PERFORMED AND THAT THE RESULTS WILL NFERRED TO EXIST IN OTHER AREAS NOT REVIEWED.
MIN	BUILDING OFFICIAL ALSO RECOGNIZES THAT STRUCTURAL REVIEW IS A TECHNIQUE EMPLOYED TO MIZE THE RISK OF PROBLEMS ARISING DURING CONSTRUCTION. STRUCTURAL OBSERVATION BY THE IGN PROFESSIONAL DOES NOT CONSTITUTE WARRANTY OR GUARANTEE OF ANY TYPE. IN ALL

# GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

	TIONS	
t	L	Angle
,)	LB.	Pound
r		Live Load
5		Long Leg Horizontal
er	LLV LONGIT.	Long Leg Vertical Longitudinal
	LT. WT.	Lightweight
e		
t	MAX.	Maximum
al	MECH.	Mechanical
e e	MEZZ. MF	Mezzanine Moment Frame
t	MFR.	Manufacturer
n	MIN.	Minimum
	MISC.	Miscellaneous
N	MK.	Mark
ef O	(N)	New
9	N.	North
ģ	N.S.	Near Side
9 m	NOM.	Nominal
'n	NTS	Not to Scale
9 n	O.C.	On Center
	0.D.	Outside Diameter
8	<i>O</i> .F.	Outside Face
r	О.Н.	Overhang
9 1	OPNG.	Opening
n n	OPP.	Opposite
9	PAF	Powder Actuated Fastener
r	PC	Precast
it	PERM.	Permanent
n	PERP.	Perpendicular Bastial Joint Popotastion
8 6	PJP PL or PL	Partial Joint Penetration Plate
s n	PLF	Pounds per linear Foot
5	PLYWD	Plywood
k	PREFAB.	Prefabricated
	PSF	Pounds per Square Foot
r	PSI	Pounds per Square Inch
8	P.T. or PT P/T	Post-Tensioning Pressure-Treated
e h	1 / 1	1 1 85501 8-11 BOLEO
r	RAD.	Radius
al	REF.	Reference
n	REINF.	Reinforce or Reinforcement
n n	REQD. REV.	Required Revise
0	R.O.	Rough Opening
il		
9	S.	South
~		
9	SCH. or SCHEI	
	SECT.	Section
	SECT. SHT.	Section Sheet
9 9 t h	SECT.	Section
9	SECT. SHT. SIM. SOG SPEC.	Section Sheet Similar
g t h e n	SECT. SHT. SIM. SOG SPEC. SQ.	Section Sheet Similar Slab On Grade Specification Square
gith n r	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT.	Section Sheet Similar Slab On Grade Specification Square Square Feet
gth on rh	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es)
gith n r	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT.	Section Sheet Similar Slab On Grade Specification Square Square Feet
9 then rhr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard
gth on rh ra yn	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener
9thønrhra y	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel
gthenrhrdynr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural
gth on rh ra yn	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel
9 the nrhrd ynr nhr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical
9 the nrhrd ynr nhrr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical
9 the nrhrd ynr nhrre	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom
9 the nrhrd ynr nhrret	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue \$ Groove
9 the nrhrd ynr nhrre	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue \$ Groove Temporary
9 the nrhrd ynr nhrret 9 e	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue \$ Groove Temporary Through Top of Concrete
9thenrhrd ynr nhrretg ed	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel
9thønrhra ynr nhrrøt 9 ødd	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall
9thenrhrd ynr nhrretg ed	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall
9thønrhra ynr nhrrøt 9 ødd	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall
9thønrhragnr nhrrøt9 øddd dr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TS TYP.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Steel Top of Steel Top of Steel Top of Steel Top of Vall
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9theorhrd yn rohrret 9 eddd drrr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TS TYP. U.O.N.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
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9 the nrhrd yn r'n hrret 9 eddd drrrd	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. VERT. VIF	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
9thenrhrd ynr nhrretg eddd drrrdnt	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TS TYP. U.O.N. VERT. VIF W.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Vall Transverse Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field
9theorhrayor chrreto eddd drrrant r	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. VERT. VIF W. W/ or w/	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field
9thenrhrd ynr nhrretg eddd drrrdnt	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TS TYP. U.O.N. VERT. VIF W.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Vall Transverse Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field
9 the nrhrdynr nhrretg eddd drrrdnt re	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.P.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without
9thønrhragnr nhrrøtg ødda arrrant røh	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.M. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.P. W.T.S.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Nork Point
9theorhrayor chrreton eddd drrrant rehrr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.P.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without
9 the nrhrdynr nhrretg eddd drrrdnt rehn	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.M. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.P. W.T.S.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Work Point
9theorhrayor chrreton eddd drrrant rehrr	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TYP. U.O.N. VERT. VIF W. W/ or w/ W.F. WWF	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Wall Transverse Tube Steel Typical Unless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Nork Point Welded Threaded Stud Welded Wire Fabric
9thenrhrd ynr nhrretg eddd drrrdnt rehnr t st	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. VERT. VIF W. W/ or w/ W.F. WMF X SECT.	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Steel Steel Steel Steel Steel Steel Steel Steel Steel St
9 the nrhraynr nhrreta eddd drrrant rehnr t s	SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TMP. THRU T.O.C. T.O.S. TO.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P. W.T.S. WWF X SECT. X-STR	Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical Uhless Otherwise Noted Vertical Verify in Field West With Welded Headed Stud Without Work Point Welded Threaded Stud Welded Mire Fabric

City of Puyallup Development & Permitting Services ISSUED PERMIT		
Building	Planning	
Engineering	Public Works	
Fire OF W	Traffic	

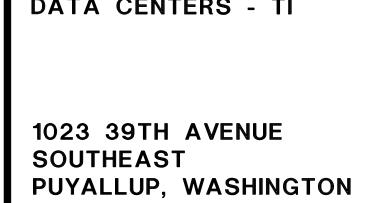
	ABBRE
)	At
k –	Penny (Nails)
<b>)</b>	Diameter Degrees
#	Pounds
ŧ	Number
A)	Above
<b>\.</b> В.	Anchor Bolt
ADD'L	Additional
NLT.	Alternate
APPROX	
ARCH.	Architect
4.S.D.	Allowable Stress Design
B)	Below
B) 3/	Below Bottom of
3F	Braced Frame
3LKG.	Blocking
BLDG.	Building
BM.	Beam
30T.	Bottom
BRG.	Bearing
BTMN.	Between
L or E	
	Camber Cast In Place
717 J.J.	Cast in Place Construction Joint or Control Joint
,JP	Construction Joint or Control Joint Complete Joint Penetration
LG.	Complete Joint Penetration Ceiling
LC.	Clear
	Concrete Masonry Unit
COL.	Column
CONC.	Concrete
ONN.	Connections
CONST.	Construction
CONT.	Continuous
SK.	Countersink
DBA DBL.	Deformed Bar Anchor Daubla
DEL. DEG.	Double
DF	Degree Doug Fir-Larch
NA.	Diameter
DIAG.	Diagonal
DIAPH.	Diaphragm
NM.	Dimension
DN.	Down
00	Ditto
DTL.	Detail
PTΡ	Double Top Plate
DWG.	Drawing
=)	Existing
E) I. EA.	East
A.	Each
F.	Each Face
L.	Elevation
LEV.	Elevator
MBED.	Embedment Length
NGR.	Engineer
Q.	Equal
.W.	Each May
XP.	Expansion
XT.	Exterior
DN.	Foundation
IN.	Finish
LR.	Floor
RP	Fiber Reinforced Polymer
.S.	Far Side
T.	Foot or Feet
TG.	Footing
A.	69400
SALV.	Gauge Galvanized
	Glue Laminated
SMB	Gypsum Wall Board
ÐG	Hot Dipped Gal∨anized
IDR.	Header
lf I s n	Hem Fir
IGR.	Hanger
IORIZ.	Horizontal
199 17	Hollow Structural Section
IT.	Height
.D.	Inside Diameter
	Inside Diameter Inside Face
., . N.	Inch
NFO.	Information
NT.	Interior
I <del></del>	
IT.	Joint

INI. Interior JT. Joint K Kips KSF Kips per Square Foot KSI Kips per Square Inch



# GENERAL STRUCTURAL NOTES SHEET NO.

	GENERATOR ANCHORA	GE	4/12/24	
NO.	DESCRIPTION		DATE	BY
ISSU	IES:	RE	VISIONS:	$\Delta$
P.M.		SHT		
P.E.		тум		
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	NO.	23444.(		



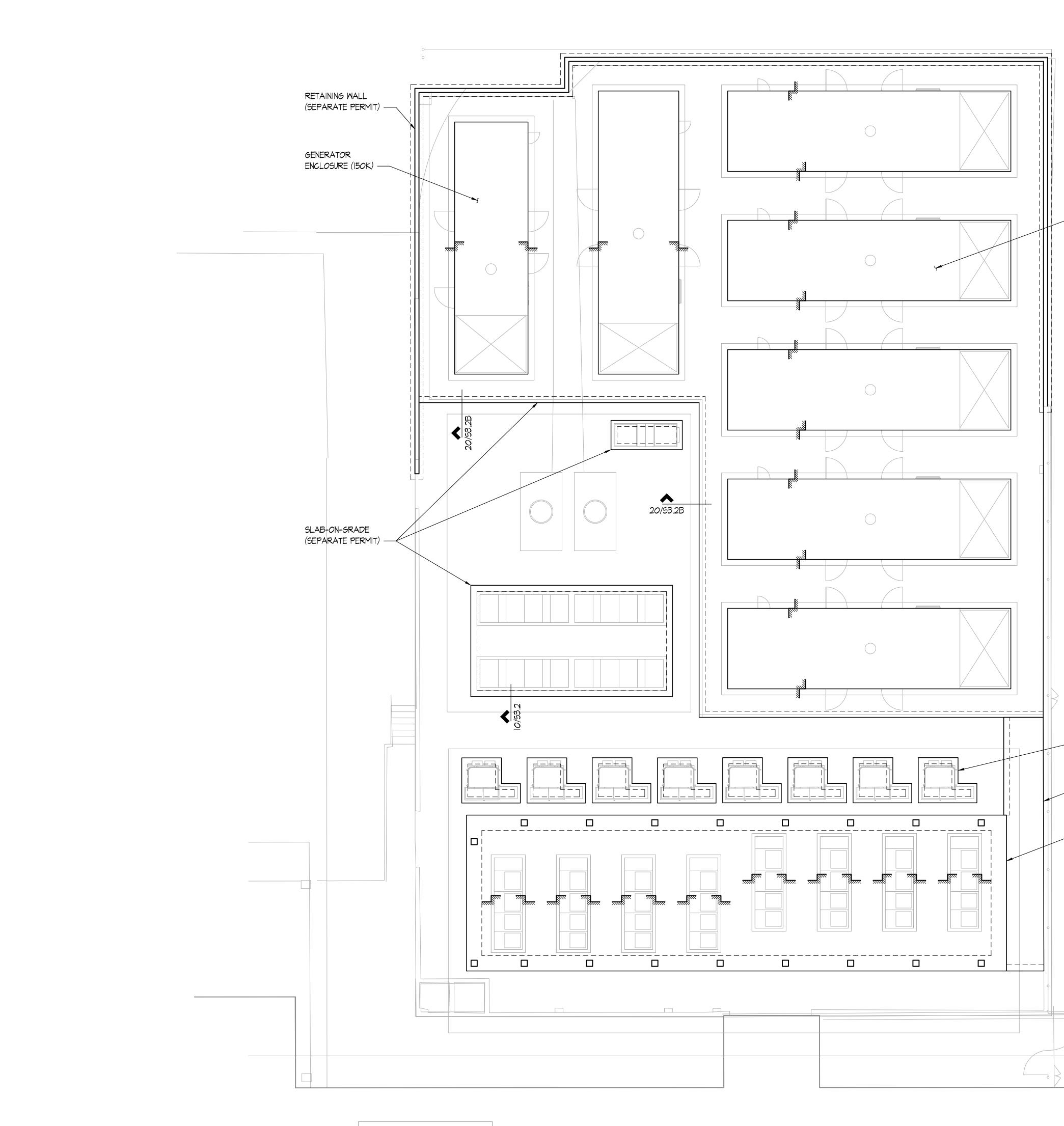
APPROVAL:

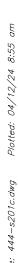












EXISTING DATA CENTER BUILDING



 NORTH YARD GENERATOR PLAN (EAST)

 SCALE: 1/8" = 1'-0"

#### - GENERATOR ENCLOSURE (157K), TYP. U.O.N.

### GENERATOR ANCHORAGE PLAN NOTES:

- I. ALL DIMENSIONS AND ELEVATIONS ON THE STRUCTURAL PLANS ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED BY THE CONTRACTOR WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND MANUFACTURER'S DRAWINGS BEFORE CONSTRUCTION BEGINS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- 2. ALL EXISTING INFORMATION IS ASSUMED AND SHALL BE FIELD VERIFIED. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- 3. FOR STRUCTURAL GENERAL NOTES AND ABBREVIATIONS SEE SHEETS SI.O TO SI.I.
- 4. FOR TYPICAL CONCRETE FOUNDATION AND ANCHORAGE DETAILS SEE SHEET S3.2B.

- SLAB-ON-GRADE (SEPARATE PERMIT)

- SLAB-ON-GRADE (SEPARATE PERMIT)

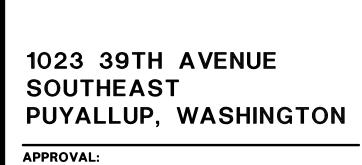
- SLAB-ON-GRADE (SEPARATE PERMIT)

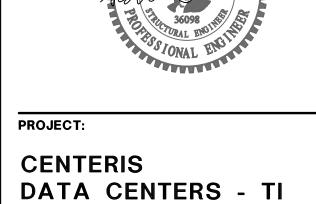
City of Puyallup Development & Permitting Services ISSUED PERMIT					
Building	Planning				
Engineering	Public Works				
Fire	Traffic				

# S2.1C

# NORTH YARD GENERATOR PLAN (EAST) SHEET NO.

	GENERATOR ANCHOR	AGE	4/12/24		
NO.	DESCRIPTION		DATE	BY	
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P.M.		SHT			
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DAT	E:	4/12/24			
JOB	NO.	01			
	ET TITLE:				







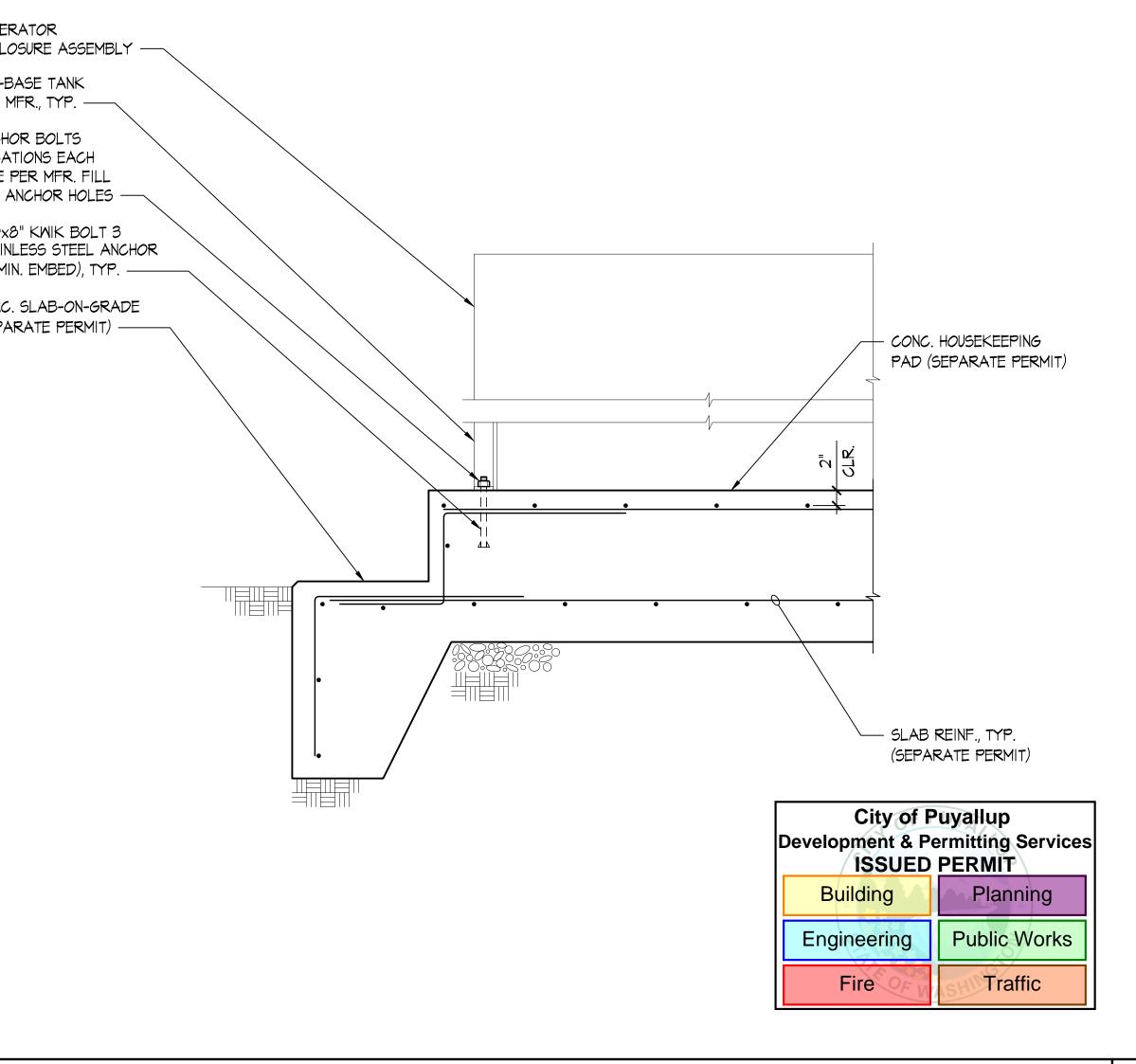
Centers for Information Systems

SEAL:



DETAIL SCALE: I"=1"-0"	DETAIL	SCALE: 1"=1-0" 2 DETAIL	SCALE: I*=1*-0* 3 DETAIL
DETAIL SCALE: I*="-0"	6 DETAIL	SCALE: I"=I"-0"	SCALE. I"=I-O" 8 DETAIL GENER ENCLO SUB-B PER M ANCHO LOCA' SIDE F ALL A 3/4"0x0 STAIN (4" MI CONC. (SEPA
DETAIL SCALE: I*=I*-0*	DETAIL	SCALE: I'=I'-0"  2 DETAIL	SCALE: I"=1-0" 3

SCALE:  "= '-0"	4	DETAIL	SCALE:  "=1'-0"	5
	•			
SCALE: I*=1*-0*	9	DETAIL	SCALE:  "=1'-0"	0

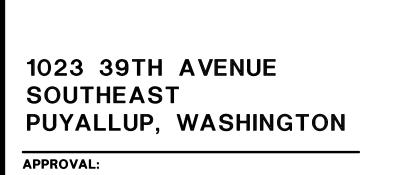


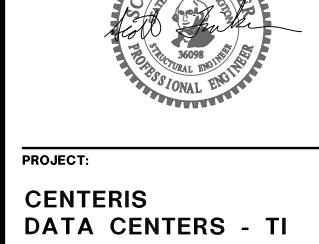


SHEET NO.

# DETAILS

	GENERATOR ANCHOR	AGE	4/12/24		
NO.	DESCRIPTION		DATE	BY	
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P.M.		SHT			
P.E.		TVM			
DRA	WN BY:	sc			
SCA	LE:	AS SHOWN			
DAT	E:	4/12/24			
JOB	NO.	23444.01			
SHE	ET TITLE:				





centeris

Centers for Information Systems

SEAL:

