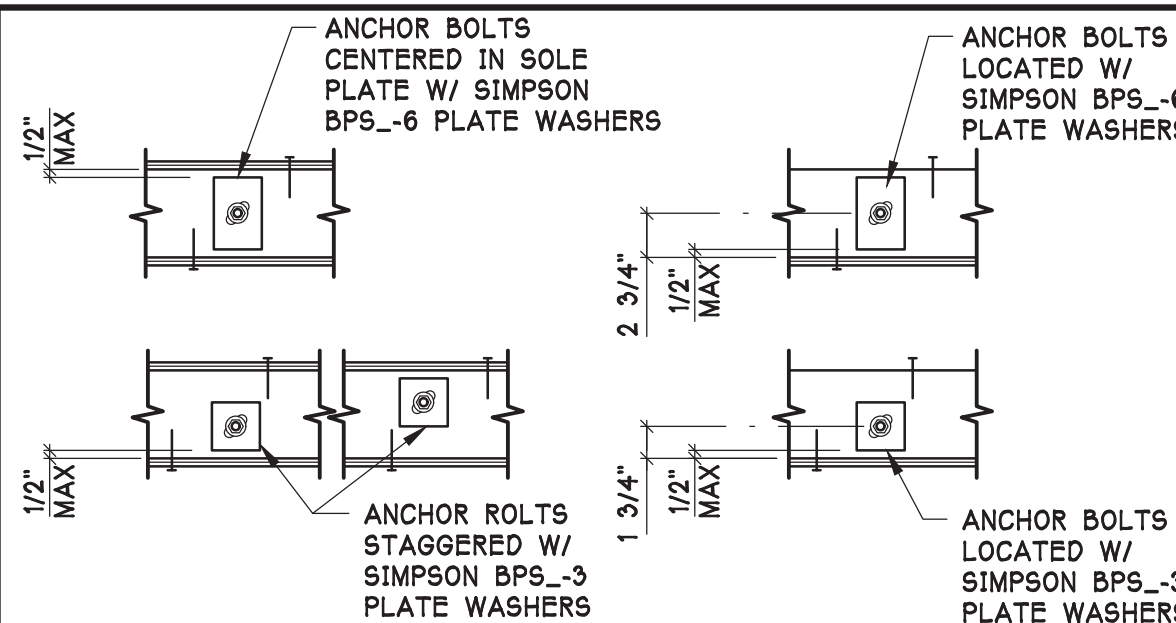


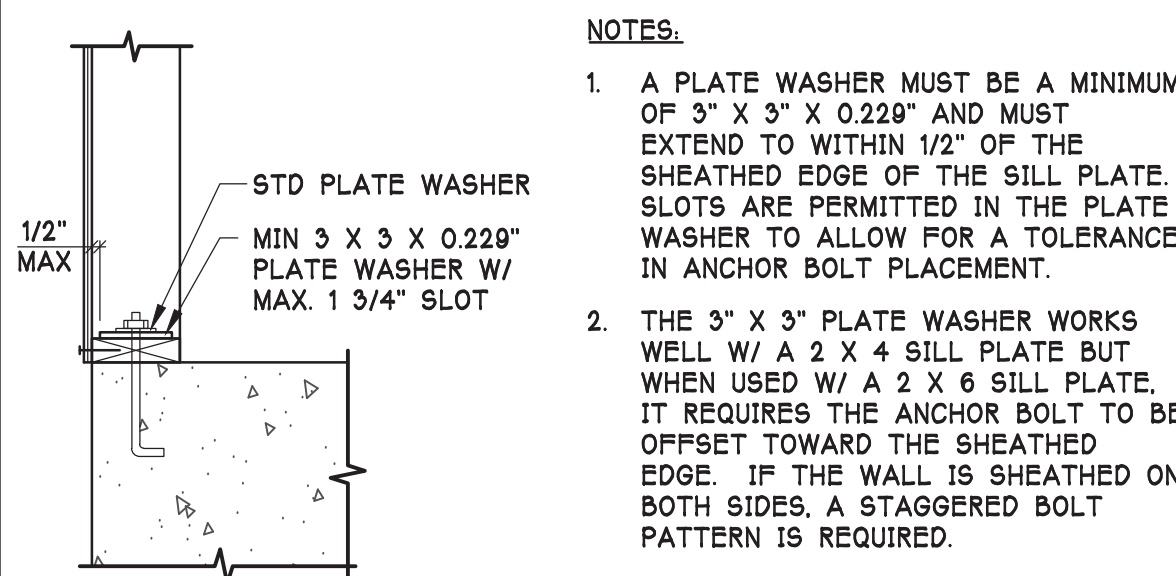
SHEARWALL SCHEDULE							
TYPE	CAPACITY EQ / WIND [PLF]	SHEATHING	BLKG REQ'D	① NAILING #10	② RIM JOIST/BLOCKING TO DBL TOP PL	③ WOOD CONNECTION AT SILL	④ CONCRETE
P-1	225 / 315	1/2" PLYWOOD ONE SIDE	YES #5	8d @ 6" o.c. EDGE, 12" o.c. FIELD	SIMPSON A35 @ 16" o.c.	16d @ 6" o.c.	1/2" ANCHOR BOLTS #2 @ 2'-6" o.c.
P-2	325 / 455	1/2" PLYWOOD ONE SIDE	YES #5	8d @ 4" o.c. EDGE, 12" o.c. FIELD	SIMPSON A35 @ 12" o.c.	{2} ROWS 16d @ 6" o.c. #4 STAGGER 3"	1/2" ANCHOR BOLTS #2 @ 1'-8" o.c.
P-3	650 / 910	1/2" PLYWOOD BOTH SIDES	YES #5	8d @ 4" o.c. EDGE, #10 12" o.c. FIELD	SIMPSON L500 @ 8" o.c.	{2} ROWS 1/4" x 0'-4" LAG #3 SCREWS @ 6" o.c. STAGGER 3"	1/2" ANCHOR BOLTS #2 @ 0'-10" o.c.

TYPICAL SHEAR WALL NOTES

- WOOD STRUCTURAL PANEL SHEAR WALLS:
 - 8d NAILS SHALL BE COMMON (2 1/2" X 0.113" DIAMETER) WITH 1 3/8" MINIMUM PENETRATION INTO FRAMING. NO. 14 GAGE STAPLES WITH A MINIMUM 7/16" OD CROWN AND 1 1/2" LENGTH MAY BE USED ONE FOR ONE IN LIEU OF 8d NAILS. FASTENERS EXPOSED TO WEATHER SHALL BE ZINC COATED BY HOT DIP GALVANIZING, MECHANICALLY DEPOSITED, OR ELECTRO DEPOSITED.
 - WHERE PLYWOOD IS APPLIED BOTH SIDES OF SHEAR WALL, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING MEMBERS SHALL BE 3" WIDE (NOMINAL) AND NAILS ON EACH SIDE SHALL BE STAGGERED.
 - FRAMING AT ADJOINING PANEL EDGES AND BOTTOM PLATES OF SHEAR WALLS SHALL BE 3-INCH NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED WHERE THE REQUIRED SHEAR CAPACITY EXCEEDS 700 pif.
 - ALLOWABLE SHEAR VALUES IN TABLE ABOVE ARE FOR HEM-FIR FRAMING MEMBERS (GROUP III). NO SUBSTITUTION OF LESSER GROUPS WILL BE ALLOWED.
- PLATE WASHERS A MINIMUM OF 3 INCH X 3 INCH X 0.229 INCH THICK SHALL BE USED ON EACH ANCHOR BOLT. SEE DETAIL A/S2.1.
- ALL BLOCKING SHALL BE 2X HEM-FIR STUD GRADE OR BETTER. (USE 3 X HF BLOCKING WHERE REQUIRED BY NOTE #1b & 1c.)
- PROVIDE DOUBLE BLOCKING OR RIM JOIST UNDER WOOD SILL WHERE (2) ROWS OF FASTENERS ARE REQUIRED. PROVIDE TRIPLE BLOCKING OR RIM JOIST UNDER WOOD SILL WHERE (3) ROWS OF FASTENERS ARE REQUIRED. (CLOSEST ALLOWABLE SPACING FOR 16d NAILS IN 1 1/4" "TIMBERSTRAND" LSL RIM IS 6" o.c.)
- "④" INDICATES CONNECTION TYPE AS DESCRIBED IN SCHEDULE ABOVE - LOCATE PER DIAGRAM AT RIGHT.



SHEAR ANCHOR OPTIONS FOR 2 X 6 WALL SHEATHED BOTH SIDES



SILL PLATE ANCHORAGE

GENERAL NOTE
 CODE: INTERNATIONAL BUILDING CODE --- 2021 EDITION
 ALL ASTM'S CALLED OUT ARE TO BE THE LATEST EDITION

LIVE LOADS
 BUILDING RISK CATEGORY II (IBC TABLE 1604.5)

GRAVITY LOADS:
 SNOW GROUND SNOW LOAD Pg = 36 psf
 FLAT ROOF SNOW LOAD Pf = 25 psf
 SNOW EXPOSURE FACTOR Ce = 1.0
 SNOW IMPORTANCE FACTOR Is = 1.0
 FLOOR THERMAL FACTOR Ct = 1.0
 DECK 60 PSF

LATERAL LOADS:
 WIND Vult = 110 MPH Vasd = 85 MPH
 EXPOSURE "B" Kzt = 1.00
 SEISMIC SITE CLASS "C"
 SEISMIC DESIGN CATEGORY "D"
 IMPORTANCE FACTOR Ie = 1.0
 Ss = 1.271g Si = 0.439g
 Fa = 1.2 Fv = NULL
 Sps = 1.017g Sop = NULL
 R = 6.5 (PLYWOOD SHEARWALLS)

FOUNDATION
 FOUNDATION DESIGN WAS BASED ON THE FOLLOWING ASSUMED ALLOWABLE VALUES:
 FOOTING BEARING PRESSURE: 1500 PSF ON DENSE NATIVE MATERIAL OR COMPACTED STRUCTURAL FILL (33% INCREASE FOR WIND OR SEISMIC)
 LATERAL EARTH PRESSURE: 35 PCF EQUIVALENT FLUID PRESSURE (ACTIVE)
 250 PCF EQUIVALENT FLUID PRESSURE (PASSIVE)
 COEFFICIENT OF FRICTION: 0.35

ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 1'-6" BELOW FINISH GRADE. SLABS AND FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR STRUCTURAL FILL COMPACTED TO 95% MAXIMUM DRY DENSITY PER ASTM D-1557. CONTRACTOR SHALL PROVIDE POSITIVE PERMANENT DRAINAGE OF BUILDING PERIMETER.

CONCRETE
 fc = 2500 psi FOR FOOTINGS & SLABS ON GRADE (UNINSPECTED - 5 1/2 SACK MIX)
 WORKING STRESS DESIGN METHOD USED. MIXING AND PLACING OF ALL CONCRETE AND SELECTION OF MATERIALS SHALL BE IN ACCORDANCE WITH THE IBC AND ACI CODE 318. PROPORTIONING OF AGGREGATE TO CEMENT SHALL BE SUCH AS TO PRODUCE A DENSE WORKABLE MIX WITH 5" MAXIMUM SLUMP WHICH CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER. FOR ADMIXTURES, SEE SPECIFICATIONS. 1/2" CHAMFER ALL EXPOSED EDGES, UNLESS INDICATED OTHERWISE ON ARCHITECTURAL DRAWINGS. WATER CURING SHALL BE USED. AIR ENTRAIN ALL CONCRETE EXPOSED TO WEATHER WITH 3% TO 6% AIR BY VOLUME.

REINFORCING STEEL
 ALL CONCRETE REINFORCING STEEL SHALL BE DEFORMED PER ASTM A615, GRADE 60 (fy=60,000 psi) EXCEPT ALL #4 SLAB DOWELS SHALL BE GRADE 40 (fy=40,000 psi). LAP CONTINUOUS REINFORCING BARS 30 BAR DIAMETERS, 1'-7" MINIMUM UNLESS NOTED OTHERWISE. CORNER BARS (1'-7" BEND) WILL BE PROVIDED FOR ALL HORIZONTAL REINF. DETAIL STEEL IN ACCORDANCE WITH "ACI MANUAL OF STANDARD PRACTICE OF DETAILING REINFORCED CONCRETE STRUCTURES". WELDED WIRE FABRIC (WWF) TO CONFORM WITH ASTM A185. REINFORCING HOOKS TO COMPLY WITH STANDARD ACI HOOKS.

UNLESS NOTED OTHERWISE, COVER TO MAIN REINFORCEMENT TO BE:
 CONCRETE CAST AGAINST & PERMANENTLY EXPOSED TO EARTH 3 INCHES
 CONCRETE EXPOSED TO EARTH OR WEATHER 1 1/2 INCHES (#5 BARS & SMALLER)
 CONCRETE NOT EXPOSED TO WEATHER 2 INCHES (#6 THRU #8 BARS)
 OR IN CONTACT WITH GROUND 3/4 INCHES (#11 BAR & SMALLER)
 1 1/2 INCHES (#14 & #18 BARS)

MISCELLANEOUS STEEL PLATES
 MISCELLANEOUS STEEL PLATES SHALL CONFORM TO ASTM A36 (fy=36,000 PSI). WELDS NOT SPECIFIED SHALL BE 1/4" CONTINUOUS FILLET MINIMUM. ALL WELDS TO BE BY WABO CERTIFIED WELDERS --- USE FRESH E70 ELECTRODES. MISCELLANEOUS HANGERS TO BE SIMPSON OR I.C.C. APPROVED EQUAL. NAIL ALL HOLES WITH NAILS AS SPECIFIED BY MANUFACTURER UNLESS NOTED OTHERWISE ON DRAWINGS. MACHINE BOLTS TO BE A-307.

TIMBER
 2X MEMBERS H.F.#2 (Fb=1270 psi REP.) (2 X 6 OR LESS)
 4X MEMBERS H.F.#2 (Fb=1035 psi REP.) (2 X 8 OR LARGER)
 D.F.#2 (Fb=1080 psi) (4 X 10 OR LESS)
 6X MEMBERS D.F.#1 (Fb=990 psi) (4 X 12 OR LARGER)
 LUMBER NOT NOTED TO BE D.F.#2 (Fb=1350 psi)
 BOLTS IN WOOD CONFORM WITH ASTM A307

ALL GRADES SHALL CONFORM TO "WPA GRADING RULES FOR WESTERN LUMBER --- LATEST EDITION." BOLT HEADS AND NUTS BEARING AGAINST WOOD SHALL BE PROVIDED WITH STANDARD CUT WASHERS. PLATE WASHERS A MINIMUM OF 3" X 3" X 1/4" SHALL BE USED AT ALL SILL PLATE ANCHOR BOLTS. ALL NEW FRAMING LUMBER SHALL HAVE 19% MAXIMUM MOISTURE CONTENT AT TIME OF INSTALLATION. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED. ALL NAILS ARE "COMMON" UNLESS INDICATED OTHERWISE. MINIMUM NAILING PER IBC TABLE 2304.10.1. S4S TYPICAL UNLESS NOTED OTHERWISE. SUBSTITUTION OF OTHERS SPECIES WITHOUT WRITTEN APPROVAL OF THE ENGINEER IS PROHIBITED. MISCELLANEOUS HANGERS TO BE SIMPSON OR I.C.C. APPROVED EQUAL. ALL CONNECTORS FOR PRESSURE TREATED LUMBER AND ALL NAILS IN EXTERIOR SHEATHING SHALL BE HOT-DIPPED GALVANIZED. NAIL ALL HOLES WITH NAILS AS SPECIFIED BY MANUFACTURER UNLESS NOTED OTHERWISE ON DRAWINGS.

NAILS
 NAILS SHALL BE COMMON, AMERICAN OR CANADIAN MANUFACTURED ONLY WITH MINIMUM DIAMETERS AS FOLLOWS:

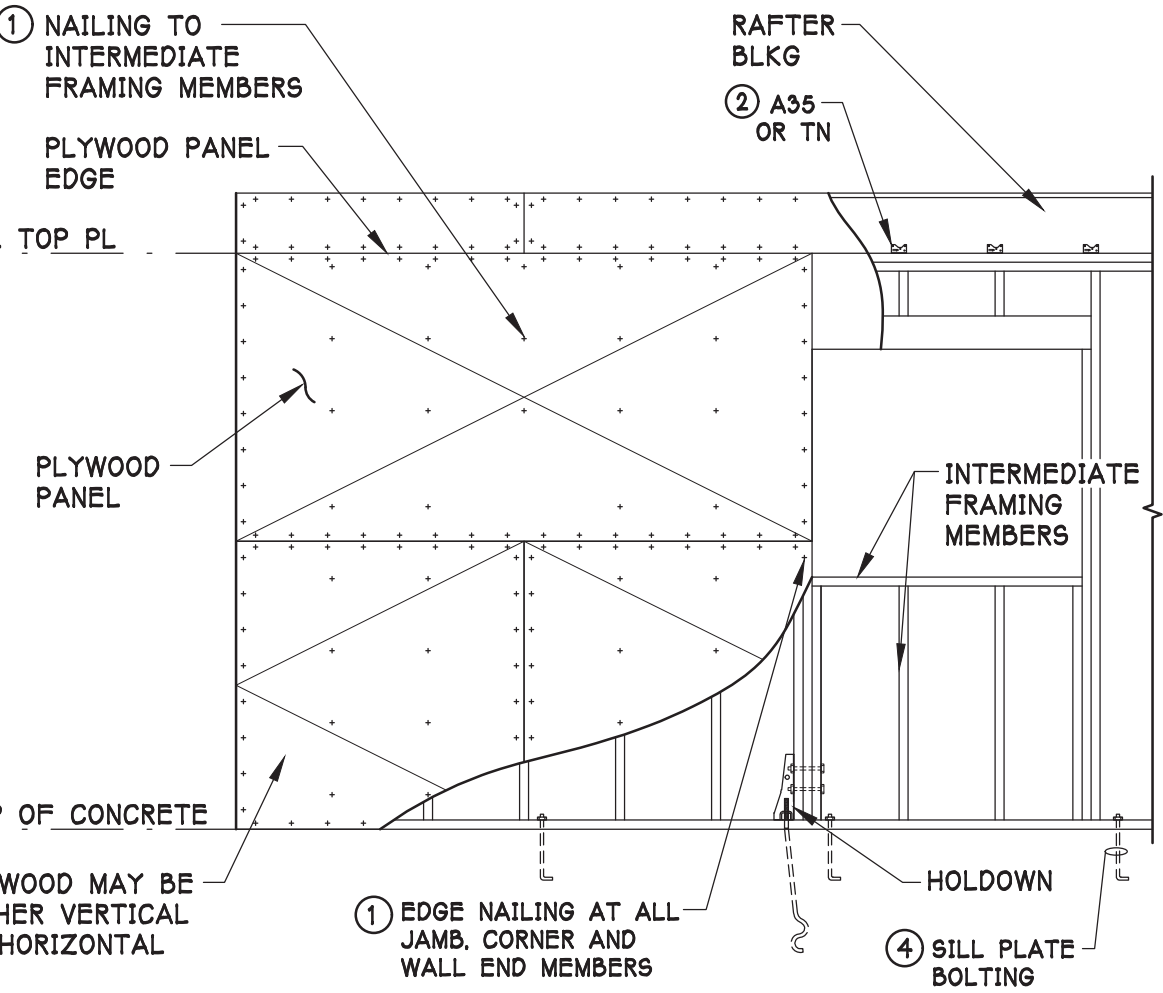
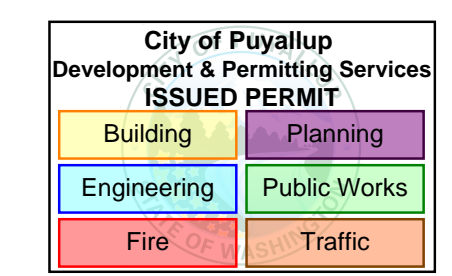
NAIL DESIGNATION	MINIMUM NAIL SHANK DIAMETER (IN)	MINIMUM NAIL LENGTH
8d	0.131"	2 1/2"
10d	0.148"	3"
16d SINKER OR 12d	0.148"	3 1/4"
16d	0.162"	3 1/2"
20d	0.192"	4"

SIMPSON STRONG-TIE HARDWARE
 ALL FRAMING HARDWARE SHALL BE SIMPSON STRONG-TIE OR ENGINEER APPROVED EQUAL. CONTRACTOR SHALL INSTALL ALL FRAMING HARDWARE WITH SIZE, TYPE AND NUMBER OF FASTENERS SPECIFIED BY THE MANUFACTURER.

GLU-LAMINATED WOOD MEMBERS
 TRUSSES SHALL BE FABRICATED IN ACCORDANCE WITH IBC CHAPTER 2303.4 AND INSPECTED IN ACCORDANCE WITH IBC CHAPTER 1704.2. IN ADDITION, THE MANUFACTURER SHALL MAINTAIN A RECORD BY A SEPARATE IN-HOUSE STAFF WHICH CHECKS AMONG OTHER THINGS, LUMBER GRADES, PLATE SIZE, PLATE LOCATION AND FABRICATION QUALITY FOR ALL TRUSSES. RECORDS SHALL BE SUBMITTED TO ARCHITECT UPON REQUEST. CERTIFIED, FULL SCALE LOAD TEST RESULTS RUN ON SIMILAR TRUSSES AND IN ACCORDANCE WITH IBC CHAPTER 1704.2 SHALL BE SUBMITTED TO THE ARCHITECT UPON REQUEST.
 PLATE MANUFACTURER MUST BE A MEMBER OF THE TRUSS PLATE INSTITUTE (TPI) AND HAVE A CURRENT ICC REPORT FOR ITS METAL GUSSET PLATES.
 ALL LUMBER TO BE KILN DRIED (TO A MAX. OF 19% MC) HEM FIR OR DOUG FIR. TOP CHORDS MUST BE DF. SHOP DRAWINGS TO INCLUDE TRUSS CALCULATIONS, LAYOUT AND PLACEMENT PLANS, INDICATE ALL BRIDGING AND BLOCKING, AND SHOW ALL BEARING CONDITIONS INCLUDING ADDITIONAL BEARING CLIPS TO INSURE ADEQUATE BEARING AREA. SHOP DRAWINGS TO BE PREPARED UNDER THE DIRECT SUPERVISION AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF WASHINGTON, AND EMPLOYED BY TRUSS MANUFACTURER. ONE STRUCTURAL ENGINEER MUST STAMP ALL SHEETS INCLUDING ERECTION DRAWINGS AND CALCULATIONS.

ROOF & SHEARWALL SHEATHING
 ROOF SHEATHING SHALL BE 1/2" (NOMINAL) APA RATED SHEATHING 24/0, EXPOSURE 1, SIZED FOR SPACING. INSTALL PANELS WITH 1/8" SPACING AT END JOINTS AND 1/4" SPACING AT EDGE JOINTS. NAILING SHALL BE 10d (COMMON) AT 6" o.c. AT PANEL EDGES AND 12" o.c. AT INTERMEDIATE SUPPORTS U.N.O. ON PLANS.
 SHEARWALL SHEATHING SHALL BE 1/2" (NOMINAL) APA RATED SHEATHING WALL-16, EXPOSURE 1, SIZED FOR SPACING. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER. BLOCK ALL PANEL EDGES AND NAIL PER SHEARWALL SCHEDULE ON THIS SHEET.

SHOP DRAWINGS
 SUBMIT THREE SETS OF SHOP DRAWINGS TO THE ENGINEER AND ONE SET TO THE BUILDING DEPARTMENT FOR APPROVAL PRIOR TO FABRICATION FOR:
 REINFORCING STEEL, MISC. STEEL, METAL GUSSET WOOD TRUSSES & GLU-LAMINATED WOOD MEMBERS.
SPECIAL INSPECTIONS
 INSPECTIONS ARE TO BE PER IBC CHAPTER 17, SECTIONS 1704 AND 1705 AND ARE TO BE BY AN INDEPENDENT TESTING LAB AND APPROVED BY THE OWNER AND BUILDING DEPARTMENT AND ENGAGED BY AND PAID FOR BY THE OWNER PRIOR TO STARTING CONSTRUCTION.
ROOF DIAPHRAGM:
 NOTIFY BUILDING DEPARTMENT AND ENGINEER OF RECORD FOR INSPECTION 48 HOURS PRIOR TO COVERING.
SPECIAL CONDITIONS
 CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN FIELD AND SHALL PROVIDE ADEQUATE SHORING AND BRACING OF ALL STRUCTURAL MEMBERS DURING CONSTRUCTION. CONTRACTOR SHALL NOTIFY ENGINEER OF ALL FIELD CHANGES PRIOR TO INSTALLATION.



- SHEAR WALL SYSTEMS:**
- PANELS SHALL NOT BE LESS THAN 4' X 8', EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING. ALL EDGES OF ALL PANELS SHALL BE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING.
 - NAILS SHALL BE LOCATED AT LEAST 3/8" FROM THE PANEL EDGES. MAXIMUM NAIL SPACING AT PANEL EDGES SHALL BE 6" ON CENTER.
 - NAILS ALONG INTERMEDIATE FRAMING MEMBERS SHALL BE THE SAME SIZE AS NAILS SPECIFIED FOR PANEL EDGE NAILING. AT INTERMEDIATE FRAMING MEMBERS, THE MAXIMUM NAIL SPACING SHALL BE 6" ON CENTER.
 - THE WIDTH OF THE NAILED FACE OF FRAMING MEMBERS AND BLOCKING SHALL BE 2" NOMINAL OR GREATER AT ADJOINING PANEL EDGES EXCEPT THAT A 3" NOMINAL OR GREATER WIDTH AT ADJOINING PANEL EDGES AND STAGGERED NAILING AT ALL PANEL EDGES ARE REQUIRED WHERE:
 - NAIL SPACING OF 2" ON CENTER OR LESS AT ADJOINING PANEL EDGES IS SPECIFIED, OR
 - 10d COMMON NAILS HAVING PENETRATION INTO FRAMING MEMBERS AND BLOCKING OF MORE THAN 1-1/2" ARE SPECIFIED AT 3" ON CENTER, OR LESS AT ADJOINING PANEL EDGES, OR
 - REQUIRED NOMINAL UNIT SHEAR CAPACITY ON EITHER SIDE OF THE SHEAR WALL EXCEEDS 700 pif IN SEISMIC DESIGN CATEGORY D, E, OR F.
 - MAXIMUM STUD SPACING SHALL BE 24" ON CENTER.
 - WOOD STRUCTURAL PANELS SHALL CONFORM TO THE REQUIREMENTS FOR ITS TYPE IN DOC PS 1 OR PS 2.

REQUIRED SPECIAL INSPECTIONS AND TEST OF CONCRETE CONSTRUCTION (TABLE 1705.3 IBC 2021)

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD ^a	IBC REFERENCED
1. INSPECTION OF REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	ACI 318 CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: <ol style="list-style-type: none"> VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND INSPECT ALL OTHER WELDS. 	-	X	AWS D1.4 ACI 318 26.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE.	-	X	ACI 318 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: ^b <ol style="list-style-type: none"> ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a. 	X	-	ACI 318 17.8.2.4	-
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X	ACI 318 CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C31 ASTM C172 ACI 318: 26.5, 26.12	-
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318 26.5	-
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318 26.5.3-26.5.5	-
9. INSPECT PRESTRESSED CONCRETE FOR: <ol style="list-style-type: none"> APPLICATION OF PRESTRESSING FORCES; AND GROUTING OF BONDED PRESTRESSING TENDONS. 	X	-	ACI 318 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	X	ACI 318 CH. 26.9	-
11. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENTS @ JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR NDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E, OR F, INSPECT SUCH CONNECTIONS & REINFORCEMENT IN THE FIELD FOR: <ol style="list-style-type: none"> INSTALLATION OF THE EMBEDDED PARTS COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS COMPLETION OF CONNECTIONS IN THE FIELD 	X	-	ACI 318: 26.13.1.3 ACI550.5	-
12. INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR COMPLIANCE WITH ACI 550.5	-	X	ACI 318: 26.13.1.3	-
13. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	ACI 318: 26.11.2	-
14. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 26.11.1.2(b)	-

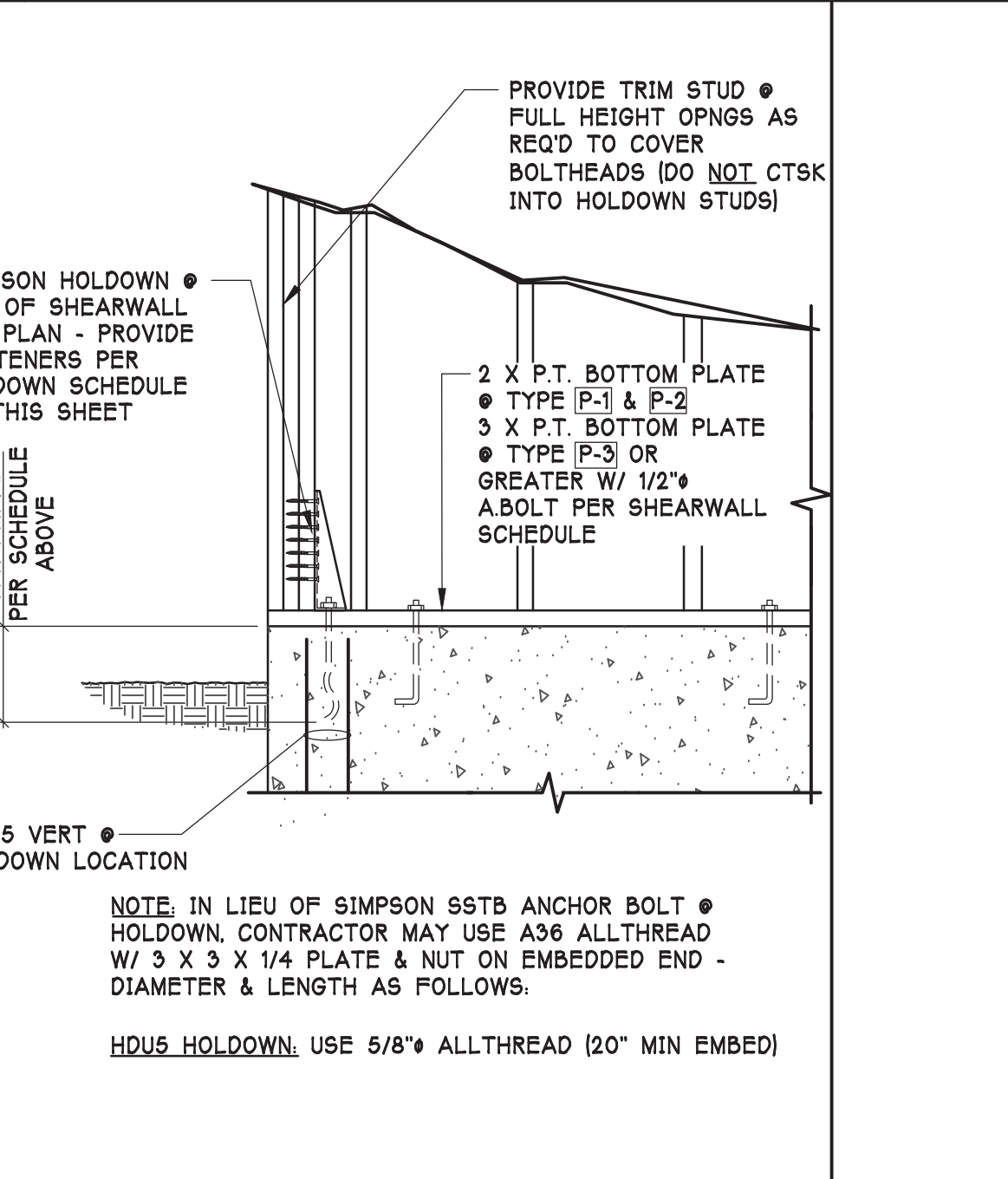
- NOTES:**
- WHERE APPLICABLE, SEE SECTION 1705.13.
 - SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

GENERAL NOTES

REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS (TABLE 1705.6 IBC 2021)

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X
4. DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS & PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X

SHEARWALL SCHEDULE



HD HOLDOWN DETAIL AT CONCRETE

PRGA20241682

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STANDARD SCHEDULES, NOTES & DETAILS

PROJECT TITLE

SHEET TITLE

PROJECT: 24056
 DATE: 10/17/24
 DRAWN: NH
 CHKD: JT

SHEET :

S2.1

OF

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