1. STRUCTURAL NOTES

1.1. ANY DISCREPANCY FOUND AMONG THE DRAWINGS, SPECIFICATIONS, THESE NOTES, AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT AND THE STRUCTURAL ENGINEER, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S RISK. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION BRACING, FORMWORK AND TEMPORARY CONSTRUCTION SHORING.

1.2. BY THE ACT OF SUBMITTING A BID FOR THE PROPOSED CONTRACT, THE CONTRACTOR WARRANTS THAT:

1.2.1. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE (INCLUDING AGENTS AND SUPPLIERS) HAVE CAREFULLY AND THOROUGHLY REVIEWED THE DRAWINGS AND STRUCTURAL NOTES AND HAVE FOUND THEM COMPLETE AND FREE FROM AMBIGUITIES AND SUFFICIENT FOR THE PURPOSE INTENDED.

1.2.2. THE CONTRACTOR HAS CAREFULLY EXAMINED THE SITE OF THE WORK AND FROM THEIR OWN INVESTIGATIONS, THEY HAVE SATISFIED THEMSELF AS TO THE NATURE AND LOCATION OF THE WORK, AS TO THE CHARACTER, QUALITY, AND QUANTITIES OF MATERIAL AND DIFFICULTIES TO BE ENCOUNTERED, AS TO THE EXTENT OF EQUIPMENT AND OTHER FACILITIES NEEDED FOR THE PERFORMANCE OF THE WORK AND AS TO THE GENERAL AND LOCAL CONDITIONS, AND OTHER ITEMS WHICH MAY IN ANY WAY AFFECT THE WORK OR ITS PERFORMANCE.

1.2.3. THE CONTRACTOR AND ALL WORKERS THEY INTEND TO USE ARE SKILLED AND EXPERIENCED IN THE TYPE OF CONSTRUCTION REPRESENTED BY THE DRAWINGS AND DOCUMENTS BID UPON.

1.2.4. NEITHER THE CONTRACTOR NOR ANY OF THEIR EMPLOYEES, AGENTS, INTENDED SUPPLIERS, OR SUBCONTRACTORS HAVE RELIED UPON ANY VERBAL REPRESENTATIONS ALLEGEDLY AUTHORIZED OR UNAUTHORIZED FROM THE OWNER OR THEIR EMPLOYEES OR AGENTS, INCLUDING THE ARCHITECT OR ENGINEERS, IN ASSEMBLING THE BID FIGURES.

1.2.5. THE REQUIREMENTS CONTAINED WITHIN THIS SECTION SUPERSEDE REQUIREMENTS AND/OR RECOMMENDATIONS CONTAINED IN THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES", AS WELL AS CASE DOCUMENT 962-D "A GUIDELINE ADDRESSING COORDINATION AND COMPLETENESS OF STRUCTURAL CONSTRUCTION DOCUMENTS"

1.2.6. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE ARE AWARE OF AND ACKNOWLEDGE THAT CLOSE COORDINATION AMONG ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS IS REQUIRED.

1.2.7. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE SHALL RECOGNIZE THAT THE PROJECT CONTRACT DOCUMENTS INCLUDE THE ARCHITECTURAL, STRUCTURAL, MECHANICAL AND ELECTRICAL AND OTHER TRADE DRAWINGS AND SPECIFICATIONS

1.2.8. CONTRACTOR AND ALL SUBCONTRACTORS ACKNOWLEDGE THAT CLOSE COORDINATION BETWEEN DISCIPLINES INCLUDED WITHIN THE CONTRACT DOCUMENTS IS NECESSARY. ELEMENTS THAT WILL REQUIRE CLOSE COORDINATION BY THE CONTRACTOR INCLUDE (BUT ARE NOT LIMITED TO):

A. VERIFICATION OF ALL DIMENSIONS INDICATED ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS
B. DETERMINATION OF ALL COLUMN LOCATIONS
C. DETERMINATION OF TOP OF FLOOR, TOP OF STEEL, WALL PLATE AND/OR TOP OF BEAM ELEVATIONS
D. DETERMINATION OF TOP OF FOOTING ELEVATIONS AND FOOTING STEP LOCATIONS
E. MECHANICAL/ELECTRICAL EQUIPMENT LOCATIONS AND WEIGHTS
F. LOCATION AND SIZE OF ALL MECHANICAL/ ELECTRICAL PENETRATIONS THROUGH WALLS AND FLOORS/ ROOFS
G. COORDINATION WITH DESIGNERS/ SUPPLIERS OF PRE-ENGINEERED COMPONENTS (JOISTS, TRUSSES, STAIRS, ETC.)

1.2.9. THE CONTRACTOR ACKNOWLEDGES THAT TEMPORARY SHORING AND/OR BRACING MAY BE REQUIRED TO COMPLETE THE PROJECT. DESIGN AND IMPLEMENTATION OF TEMPORARY SHORING AND/OR BRACING DURING CONSTRUCTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

1.2.10. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE SHALL MAKE CONSIDERATION FOR, AND INCLUDE MONIES FOR THE ABOVE IN THE PREPARATION OF THEIR BIDS.

1.2.11. THE CONTRACTOR SHALL NOT SCALE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR LOCATIONS OF ELEMENTS NOTED ABOVE.

1.2.12. ELECTRONIC COPIES OF THE STRUCTURAL DRAWINGS (PDF'S, CAD DRAWINGS OR BIM MODELS) MAY BE PROVIDED TO THE CONTRACTOR FOR THEIR USE. THESE FILES MAY BE PROVIDED AT THE REQUEST OF THE CONTRACTOR FOR THEIR CONVENIENCE ONLY. THE CONTRACTOR AGREES THAT THESE FILES SHALL NOT SUPERSEDE INFORMATION SHOWN ON THE ORIGINAL BID/ CONSTRUCTION DOCUMENTS. THE CONTRACTOR AGREES TO HOLD THE STRUCTURAL ENGINEER HARMLESS FOR ANY ERRORS OR DISCREPANCIES CONTAINED WITHIN THESE ELECTRONIC FILES.

1.2.13. THE BID FIGURE IS BASED SOLELY UPON THE CONSTRUCTION CONTRACT DOCUMENTS AND PROPERLY ISSUED WRITTEN OR VERBAL REPRESENTATIONS.

1.3. CODES

1.3.1. ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2018 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING AUTHORITY.

1.3.2. ALL REFERENCES TO OTHER CODES, STANDARDS AND SPECIFICATIONS, (ACI, ASTM, ETC.), SHALL BE FOR THE EDITION CURRENTLY REFERENCED BY IBC AS AMENDED AND ADOPTED BY THE LOCAL BUILDING AUTHORITY.

1.4. DESIGN CRITERIA

1.4.1. UNIFORM LOADS:

LOCATION	LIVE LOAD	DEAD LOAD
ROOF SOLAR READINESS ZONE **	25 PSF (SNOW*)	ACTUAL +4 PSF +175 PSF (INVERTER)
RESIDENTIAL (PRIVATE ROOMS AND CORRIDORS)	40 PSF	ACTUAL
RESIDENTIAL (PUBLIC ROOMS AND CORRIDORS)	100 PSF	ACTUAL
STAIRS AND EXITS	100 PSF	ACTUAL
DECKS AND BALCONIES [1.5X OCCUPANCY SERVED, NOT EXCEEDING 100 PSF]	60 PSF	ACTUAL
MECHANICAL AREAS	50 PSF	ACTUAL
STORAGE	125 PSF	ACTUAL
HANDRAILS AND GUARDS	50 PLF OR 200# CONCENTRATED LOAD	

* THIS IS NOT A GROUND SNOW LOAD

** SOLAR READINESS ZONE PER WA STATE ENERGY CODE, COMMERCIAL PROVISIONS, CHAPTER 51-11C WAC.

WHERE LIVE LOADS OF COMMERCIAL OR INDUSTRIAL BUILDINGS EXCEED 50 PSF, SUCH DESIGN LOADS SHALL BE POSTED IN THAT PART OF EACH STORY IN WHICH THEY APPLY

1.4.2. SNOW LOADS PER IBC SECTION 1608 AND CHAPTER 7 OF ASCE 7:

GROUND SNOW LOAD (P_g):	25.0 PSF
FLAT ROOF SNOW LOAD (P_f):	25.0 PSF
SNOW EXPOSURE FACTOR (C_e):	1.0
SNOW IMPORTANCE FACTOR (I_s):	1.0
THERMAL FACTOR (C_t):	1.0

1.4.3. CONCENTRATED LOADS: ALL MANUFACTURERS OF PRE-ENGINEERED COMPONENTS OR SYSTEMS SHALL LOCATE, COORDINATE, VERIFY WEIGHTS, ETC., OF MECHANICAL UNITS OR OTHER CONCENTRATED LOADS AND DESIGN THEIR SYSTEM FOR THESE LOADS.

1.4.4. WIND LOADS (PER IBC SECTION 1609 AND ASCE 7 CHAPTERS 26 THRU 30):

BASIC WIND SPEED (V):	98 MPH
RISK CATEGORY	II
WIND EXPOSURE:	B
APPLICABLE INTERNAL PRESSURE COEFFICIENT:	+/-0.18
ENCLOSED STRUCTURE	
TOPOGRAPHIC FACTOR (K_{zt})	1.0

COMPONENTS AND CLADDING: ULTIMATE DESIGN WIND PRESSURES TO BE USED FOR THE DESIGN OF EXTERIOR COMPONENT AND CLADDING MATERIALS IS AS FOLLOWS:

ZONE:1	+16.0/-27.5 PSF (10 SQ FT)
ZONE:1'	+16.0/-16.0 PSF (10 SQ FT)
ZONE:2	+16.0/-36.3 PSF (10 SQ FT)
ZONE:3	+16.0/-36.3 PSF (10 SQ FT)
ZONE:4	+16.0/-17.1 PSF (10 SQ FT)
ZONE:5	+16.0/-21.1 PSF (10 SQ FT)

1.4.5. SEISMIC LOADS (PER IBC SECTION 1613 AND ASCE 7 CHAPTERS 11 THRU 13):

RISK CATEGORY:	II
SEISMIC IMPORTANCE FACTOR (I_e):	1.00
S_s:	1.257
S_1:	0.434
SITE CLASS:	C
S_{DS}:	1.006
S_{D1}:	0.434
SEISMIC DESIGN CATEGORY:	D
DESIGN BASE SHEAR:	$V = C_s \times W$
SEISMIC RESPONSE COEFFICIENT (C_s):	0.154 (WOOD SW) 0.200 (CONC SW)
ANALYSIS PROCEDURE USED:	EQUIVALENT LATERAL FORCE PROCEDURE

SEISMIC FORCE-RESISTING SYSTEM	RESPONSE MODIFICATION COEFFICIENT, R	OVERSTRENGTH FACTOR, Ω_0
A. BEARING WALL SYSTEMS:		
1. SPECIAL REINFORCED CONCRETE SHEAR WALLS	5	2 ½
16. LIGHT-FRAMED (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE	6 ½	2 ½

NOTE: TABULATED OVERSTRENGTH FACTOR HAS BEEN REDUCED IN ACCORDANCE WITH ASCE 7 TABLE 12.2-1 FOOTNOTE B FOR STRUCTURES WITH FLEXIBLE DIAPHRAGMS.

1.5. STATEMENT OF SPECIAL INSPECTIONS

SEE STATEMENT OF SPECIAL INSPECTION AND TESTING SHEETS S0.3 AND S0.4.

1.6. SHOP DRAWINGS

1.6.1. SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR THE FOLLOWING:

A. CONCRETE MIX DESIGN SUBMITTALS
B. REINFORCING STEEL
C. STRUCTURAL AND MISCELLANEOUS STEEL INCLUDING WELD INSERTS AND ANCHORS
D. GLUED-LAMINATED MEMBERS/PARALLAL/LSL MEMBERS
E. PRE-ENGINEERED WOOD TRUSSES*

* DEFERRED SUBMITTALS: PRE-ENGINEERED ITEMS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AFTER REVIEW BY THE ENGINEER OR RECORD AS A DEFERRED SUBMITTAL.

1.6.2. SHOP DRAWING REVIEW NOTES

A. ENGINEER OF RECORD SHALL REVIEW SHOP DRAWINGS FOR GENERAL CONFORMANCE WITH THE PROJECT CONSTRUCTION DOCUMENTS (PLANS AND SPECIFICATIONS).
B. ENGINEER OF RECORD REVIEW OF SHOP DRAWINGS SHALL NOT RELIEVE THE GENERAL CONTRACTOR OF THEIR RESPONSIBILITY FOR REVIEW OF THE SHOP DRAWINGS FOR COMPLIANCE WITH THE PROJECT REQUIREMENTS.
C. APPROVAL OF THE SHOP DRAWINGS BY THE ENGINEER OF RECORD SHALL NOT BE CONSIDERED AS A GUARANTEE BY THE ENGINEER THAT THE SHOP DRAWINGS COMPLY WITH ALL PROJECT REQUIREMENTS.
D. CONCURRENT SHOP DRAWING REVIEW SHALL ONLY BE PERMITTED IF APPROVED BY THE ARCHITECT/ENGINEER OF RECORD PRIOR TO THE START OF SHOP DRAWING REVIEW.

1.7. MISCELLANEOUS

1.7.1. VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD.

1.7.2. VERIFY SIZE AND LOCATION OF ALL OPENINGS IN THE FLOORS, ROOF AND WALLS WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.

1.7.3. CONSTRUCTION DETAILS NOT SPECIFICALLY SHOWN ON THE DRAWINGS SHALL FOLLOW SIMILAR DETAILS OF SECTIONS OF THIS PROJECT AS APPROVED BY THE ARCHITECT/ ENGINEER.

1.7.4. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR DIMENSIONS AND LOCATIONS OF OPENINGS NOT DIMENSIONED OR SHOWN ON STRUCTURAL PLANS.

1.7.5. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND WEIGHTS OF ALL MECHANICAL AND ELECTRICAL EQUIPMENT INCLUDING HOUSEKEEPING PADS.

1.7.6. FOR PIPES, CONDUITS, DUCTS AND MECHANICAL EQUIPMENT SUPPORTED OR BRACED FROM STRUCTURE: CONFORM TO SHEET METAL AND AIR CONDITIONING CONTRACTORS NATIONAL ASSOCIATION, INC., PUBLICATION "APPENDIX E: SEISMIC RESTRAINT MANUAL GUIDELINES FOR MECHANICAL SYSTEMS." ALL BRACING AND SUPPORTS SHALL BE DESIGNED FOR SEISMIC HAZARD LEVEL (SHL) B. SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA PAMPHLET 13.

1.7.7. THE STRUCTURE HAS BEEN DESIGNED TO RESIST CODE REQUIRED VERTICAL AND LATERAL FORCES AFTER THE CONSTRUCTION OF ALL STRUCTURAL ELEMENTS HAS BEEN COMPLETED. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO JOB SITE SAFETY: ERECTION MEANS, METHODS, AND SEQUENCES; TEMPORARY SHORING, FORMWORK, AND BRACING; USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES.

2. SITE PREPARATION/SOIL REMEDIATION

2.1. SOIL DATA

ALLOWABLE SOIL PRESSURE 3,000 PSF. ALLOW 33-1/3% INCREASE FOR LOADS FROM WIND OR SEISMIC ORIGIN. SEE GEOTECHNICAL ENGINEERING REPORT BY TERRA ASSOCIATES, INC. DATED NOVEMBER 14, 2016. SEE GEOTECH REPORT FOR ALL SUBGRADE PREPARATION REQUIREMENTS AS WELL AS CAPILLARY BREAK AND VAPOR BARRIER RECOMMENDATIONS.

2.1.1. RETAINING WALL DESIGN CRITERIA:

A. ACTIVE EARTH PRESSURE:	35 PCF
B. AT-REST EARTH PRESSURE:	100 PSF (UNIFORM)
C. SEISMIC EARTH PRESSURE:	8 x "H" PSF
D. PASSIVE EARTH PRESSURE:	350 PCF *
E. FRICTION COEFFICIENT:	0.35 *

* INCLUDES FACTOR OF SAFETY OF 1.5

2.2. EXCAVATION

EXCAVATE TO DEPTH SHOWN AND TO FIRM UNDISTURBED MATERIAL. OVER-EXCAVATIONS SHALL BE BACKFILLED WITH LEAN CONCRETE (f'_c=500-1200 PSI) OR STRUCTURAL FILL AT THE CONTRACTOR'S EXPENSE. EXERCISE EXTREME CARE DURING EXCAVATION TO AVOID DAMAGE TO BURIED LINES, TANKS, AND OTHER CONCEALED ITEMS. UPON DISCOVERY, DO NOT PROCEED WITH WORK UNTIL RECEIVING WRITTEN INSTRUCTIONS FROM THE ARCHITECT. A COMPETENT REPRESENTATIVE OF THE OWNER SHALL INSPECT ALL FOOTING EXCAVATIONS FOR SUITABILITY OF BEARING SURFACES PRIOR TO PLACEMENT OF REINFORCING STEEL. PROVIDE DRAINAGE AS NECESSARY TO AVOID WATER-SOFTENED SUBGRADE.

2.3. FILL, BACKFILL AND COMPACTION

BACKFILL AGAINST WALLS SHALL NOT BE PLACED UNTIL AFTER THE REMOVAL OF ALL MATERIAL SUBJECT TO ROT OR CORROSION. ALL FILL PLACED AGAINST RETAINING WALLS OR BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL. STRUCTURAL FILL OTHER THAN PEA GRAVEL SHALL BE GRANULAR PLACED IN 6-INCH LIFTS AND COMPACTED TO AT LEAST 95% OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557 (MOD PROCTOR). PEA GRAVEL FILL SHALL HAVE A MAXIMUM PARTICLE SIZE OF 3/8" DIAMETER.

3. STRUCTURAL CONCRETE

3.1. GENERAL

ALL CONCRETE SHALL BE HARD ROCK CONCRETE MEETING THE REQUIREMENTS OF ACI-301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS." PROPORTIONING OF INGREDIENTS FOR EACH CONCRETE MIX SHALL BE BY METHOD 2 OR THE ALTERNATE PROCEDURE GIVEN IN ACI-301. PLACE CONCRETE PER ACI-304 AND CONFORM TO ACI-604 (306) FOR WINTER CONCRETING AND ACI-605 (305) FOR HOT WEATHER CONCRETING. USE INTERIOR MECHANICAL VIBRATORS WITH 7,000 RPM MINIMUM FREQUENCY. DO NOT OVER-VIBRATE. CONCRETE SHALL BE PLACED MONOLITHICALLY BETWEEN CONSTRUCTION OR CONTROL JOINTS. PROTECT ALL CONCRETE FROM PREMATURE DRYING, EXCESSIVE HOT OR COLD TEMPERATURE FOR SEVEN DAYS AFTER PLACING.

3.2. STRENGTH

TWENTY-EIGHT DAY COMPRESSIVE STRENGTHS (f'_c) SHALL BE AS FOLLOWS WITH EXPOSURE CATEGORY AND CLASS PER ACI TABLE 19.3.1.1 GIVEN IN PARENTHESIS:

SLABS ON GRADE (F0/S0/W0/C0)	4000 PSI
FOOTINGS (F0/S0/W0/C1)	3000 PSI
VERTICALLY FORMED WALLS (F1/S0/W0/C0)	4000 PSI *

* MAXIMUM W/C RATIO SHALL BE 0.55

CONCRETE SUPPLIER TO PROVIDE TEST RECORDS PER SECTION 26.4 OF ACI 318. WHEN NO PRIOR EXPERIENCE OR TRIAL MIXTURE DATA ARE AVAILABLE, THE WATER/CEMENT RATIO FROM THE TABLE BELOW MAY BE USED, BUT ONLY WHEN SPECIAL PERMISSION IS GIVEN BY ENGINEER.

MAXIMUM ABSOLUTE WATER/CEMENT RATIO BY WEIGHT FOR CONCRETE MIXES WITHOUT TEST RECORDS SHALL BE AS FOLLOWS:

SPECIFIED COMPRESSIVE STRENGTH	NON-AIR ENTRAINED CONCRETE	AIR-ENTRAINED CONCRETE
3000 PSI	0.58	0.46
4000 PSI	0.44	0.35

3.3. MATERIALS

3.3.1. CEMENT: ASTM C150, TYPE I OR TYPE II. ENGINEER'S APPROVAL IS NEEDED FOR USE OF TYPE III CEMENT.

3.3.2. COARSE AND FINE AGGREGATE: ASTM C33.

3.3.3. WATER SHALL BE CLEAN AND POTABLE.

3.3.4. FLYASH: ASTM C618 CLASS C (CLASS F MAY BE ALLOWED IF APPROVED BY THE STRUCTURAL ENGINEER)

3.3.5. GROUND GRANULATED BLAST FURNACE SLAG (GGBFS): ASTM C989 GRADE 100 OR 120. GGBFS SHALL NOT BE PERMITTED UNLESS REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER. MIX DESIGNS SUBMITTED INCLUDING GGBFS SHALL INCLUDE SHRINKAGE TEST RESULTS AT 28 DAYS.

3.4. ADMIXTURES

3.4.1. WATER REDUCING ADMIXTURE: ASTM C494. ADMIXTURES SHALL BE USED IN EXACT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

3.4.2. WATER REDUCING ADMIXTURES SHALL BE USED AT ALL HEAVILY CONGESTED AREAS (I.E. CONCRETE BEAMS, COLUMNS AND WALLS WITH REINFORCING SPACING OF 4" OR LESS)

3.4.3. CONCRETE USING ADMIXTURES TO PRODUCE FLOWABLE CONCRETE MAY BE USED SUBJECT TO ENGINEER'S APPROVAL.

3.4.4. AIR ENTRAINMENT: ASTM C260 AND ASTM C494 ENTRAIN 5% PLUS/MINUS 1.5% BY VOLUME IN ALL CONCRETE EXPOSED TO WEATHER.

3.4.5. NO OTHER ADMIXTURES PERMITTED UNLESS APPROVED BY THE ENGINEER.

3.5. FORMWORK AND SHORING

3.5.1. FOLLOW RECOMMENDED PRACTICE FOR CONCRETE FORMWORK (ACI-347).

3.5.2. WHILE RESHORING OPERATIONS ARE UNDERWAY, NO CONSTRUCTION LOADS WILL BE PERMITTED ON THE NEW CONSTRUCTION.

3.5.3. ALL SHORING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. FORMWORK SUPPORTS AND SHORING SHALL BE DESIGNED TO PROVIDE FINISHED CONCRETE SURFACES AT ALL FACES LEVEL, PLUMB AND TRUE TO THE DIMENSIONS AND ELEVATIONS SHOWN. TOLERANCES AND VARIATIONS SHALL BE AS SPECIFIED.

3.6. REINFORCING STEEL:

3.6.1. DETAIL, FABRICATE, AND PLACE PER ACI-315 AND ACI-318. SUPPORT REINFORCEMENT WITH APPROVED CHAIRS, SPACERS, OR TIES.

3.6.2. DEFORMED BAR REINFORCEMENT: ASTM A615 GR 60

3.6.3. WELDABLE DEFORMED BAR REINFORCEMENT: ASTM A706 GR 60 WHERE NOTED ON STRUCTURAL DRAWINGS

3.6.4. LONGITUDINAL (VERTICAL) REINFORCEMENT RESISTING SEISMIC MOMENT AND/OR AXIAL FORCES IN SPECIAL MOMENT FRAMES, SPECIAL STRUCTURAL WALLS INCLUDING BOUNDARY ELEMENTS, COUPLING BEAMS AND WALL PIERS SHALL BE ASTM A706 GR 60. ASTM A615 GR 60 MAY BE USED IF:

A. THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED THE SPECIFIED YIELD BY MORE THAN 18 KSI; AND
B. THE RATIO OF THE ACTUAL ULTIMATE TENSILE STRENGTH TO THE ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25; AND
C. MINIMUM ELONGATION IN 8-INCH SHALL BE AT LEAST 14% FOR #3 THRU #6 BARS, AT LEAST 12% FOR #7 THRU #11 BARS, AND AT LEAST 10% FOR #14 THRU #18.

3.6.5. WELDED WIRE FABRIC: ASTM 1064 GR 65

3.6.6. DEFORMED BAR ANCHORS: ASTM A496

3.6.7. EXCEPT AS NOTED SPECIFICALLY ON THE DRAWINGS, ALL CONCRETE REINFORCEMENT SHALL BE LAP-SPLICED AS INDICATED ON THE REINFORCING BAR DEVELOPMENT AND SPLICE LENGTH SCHEDULE PROVIDED ON THE STRUCTURAL DRAWINGS. NO MORE THAN 50% OF HORIZONTAL OR VERTICAL REINFORCING BARS SHALL BE SPLICED AT ANY ONE LOCATION.

3.6.8. EXCEPT AS NOTED SPECIFICALLY ON THE DRAWINGS, PROVIDE CORNER BARS TO MATCH QUANTITY AND DIAMETER OF HORIZONTAL REINFORCEMENT AND LAP WITH SPECIFIED HORIZONTAL REINFORCEMENT FOR "L_d" PER REINFORCING BAR DEVELOPMENT AND SPLICE LENGTH TABLES PROVIDED ON THE STRUCTURAL DRAWINGS. THESE CORNER BARS SHALL BE PLACED AT ALL CORNERS AND INTERSECTIONS IN CONCRETE FOOTINGS AND WALLS.

3.6.9. LAP WELDED WIRE FABRIC 12" OR ONE SPACING PLUS 2", WHICHEVER IS MORE.

3.7. CONCRETE COVER ON REINFORCING SHALL BE AS FOLLOWS (UNLESS SHOWN OTHERWISE):

BOTTOM OF FOOTINGS	3"
FORMED EARTH FACE AND SLAB ON GRADE	2"
WALLS, WEATHER FACE	1-1/2"
WALLS, INSIDE FACE	1"

3.8. CONSTRUCTION OR CONTROL JOINTS

3.8.1. UNLESS NOTED OTHERWISE, LOCATION OF THE CONSTRUCTION OR CONTROL JOINTS IN SLAB ON GRADE SHALL NOT EXCEED THE DISTANCES NOTED BELOW. JOINTS SHALL BE LOCATED ON COLUMN GRIDS OR UNDER PERMANENT PARTITIONS TO THE GREATEST EXTENT POSSIBLE. ADDITIONAL JOINTS SHALL BE REQUIRED AT REENTRANT CORNERS AND CORNERS OF SLAB DEPRESSIONS OR PENETRATIONS. SEE ARCHITECTURAL DRAWINGS FOR JOINT LAYOUT AT EXPOSED CONCRETE CONDITIONS. PROVIDE JOINT SEALANT PER SPECIFICATIONS - INSTALL PER MANUFACTURER RECOMMENDATIONS.

4" SLAB ON GRADE	12'-0" OC

3.8.2. CONSTRUCTION OR CONTROL JOINT SPACING IN WALLS SHALL NOT EXCEED 50' ON CENTER EXCEPT AS DIRECTED BY THE ARCHITECT/ENGINEER.

3.8.3. HORIZONTAL CONSTRUCTION JOINTS IN BEAMS AND GIRDERS SHALL NOT BE PERMITTED EXCEPT WHERE INDICATED ON STRUCTURAL DRAWINGS. VERTICAL CONSTRUCTION JOINTS IN BEAMS AND SLABS SHALL BE LOCATED BETWEEN THE MIDPOINT AND THE THIRD POINT OF THE SPAN.

3.8.4. PROVIDE CONSTRUCTION OR CONTROL JOINTS IN NON-STRUCTURAL TOPPING SLABS AT 10'-0" OC EACH WAY, MAXIMUM.

3.9. CONDUIT AND PIPING EMBEDDED IN CONCRETE

3.9.1. ELECTRICAL CONDUIT SHALL NOT BE PLACED WITHIN A SLAB ON GRADE BUT PLACED BELOW THE SLAB IN THE SUB-BASE.

3.10. GROUT FOR BEARING PLATES

THE NON-SHRINK GROUT SHALL MEET ASTM C1107 GRADE B OR EQUIVALENT (MASTERFLOW 928 BY BASF OR APPROVED EQUIVALENT). GROUT SHALL BE A PRE-PACKAGED HYDRAULIC CEMENT BASED MINERAL AGGREGATE GROUT, MIXED, PLACED AND CURED AS RECOMMENDED BY THE MANUFACTURER. COMPRESSIVE STRENGTH SHALL EXCEED 6000 PSI AT 28 DAYS.

3.11. SHOTCRETE

3.11.1. SHOTCRETE SHALL BE DEFINED AS MORTAR OR CONCRETE PNEUMATICALLY PROJECTED AT HIGH VELOCITY ONTO A SURFACE. EXCEPT AS SPECIFIED IN THIS SECTION, SHOTCRETE SHALL CONFORM TO THE REQUIREMENTS FOR PLAIN CONCRETE OR REINFORCED CONCRETE.

3.11.2. PROPORTIONS AND MATERIALS: SHOTCRETE PROPORTIONS SHALL BE SELECTED THAT ALLOW SUITABLE PLACEMENT PROCEDURES USING THE DELIVERY EQUIPMENT SELECTED AND SHALL RESULT IN FINISHED IN-PLACE HARDENED SHOTCRETE MEETING THE SPECIFIED STRENGTH REQUIREMENTS.

3.11.3. AGGREGATE: COARSE AGGREGATE, IF USED, SHALL NOT EXCEED 3/4 INCH.

3.11.4. REINFORCEMENT: LAP SPLICES IN REINFORCING BARS SHALL BE BY THE NON-CONTACT LAP SPLICE METHOD WITH AT LEAST 2 INCHES CLEARANCE BETWEEN BARS. THE BUILDING OFFICIAL MAY PERMIT THE USE OF CONTACT LAP SPLICES WHEN NECESSARY FOR THE SUPPORT OF THE REINFORCING PROVIDED IT CAN BE DEMONSTRATED BY MEANS OF PRE-CONSTRUCTION TESTING THAT ADEQUATE ENCASEMENT OF THE BARS AT THE SPLICE CAN BE ACHIEVED, AND PROVIDED THAT THE SPLICES ARE PLACED SO THAT A LINE THROUGH THE CENTER OF THE TWO SPLICED BARS IS PERPENDICULAR TO THE SURFACE OF THE SHOTCRETE WORK.

3.11.5. PRE-CONSTRUCTION TESTS: WHEN REQUIRED BY THE SPECIFICATIONS OR BUILDING OFFICIAL, A TEST PANEL SHALL BE SHOT, CURED, CORED OR SAWN, EXAMINED AND TESTED PRIOR TO COMMENCEMENT OF THE PROJECT. THE SAMPLE PANEL SHALL BE REPRESENTATIVE OF THE PROJECT AND SIMULATE JOB CONDITIONS AS CLOSELY AS POSSIBLE. THE PANEL THICKNESS AND REINFORCING SHALL REPRODUCE THE THICKEST AND MOST CONGESTED AREA SPECIFIED IN THE STRUCTURAL DESIGN. IT SHALL BE SHOT AT THE SAME ANGLE, USING THE SAME NOZZLEMAN AND WITH THE SAME CONCRETE MIX DESIGN THAT WILL BE USED ON THE PROJECT.

3.11.6. REBOUND: ANY REBOUND OR ACCUMULATED LOOSE AGGREGATE SHALL BE REMOVED FROM THE SURFACES TO BE COVERED PRIOR TO PLACING THE INITIAL OR ANY SUCCEEDING LAYERS OF SHOTCRETE. REBOUND SHALL NOT BE REUSED AS AGGREGATE.

3.11.7. JOINTS: EXCEPT WHERE PERMITTED HEREIN, UNFINISHED WORK SHALL NOT BE ALLOWED TO STAND FOR MORE THAN 30 MINUTES UNLESS ALL EDGES ARE SLOPED TO A THIN EDGE. BEFORE PLACING ADDITIONAL MATERIAL ADJACENT TO PREVIOUSLY APPLIED WORK, SLOPING AND SQUARE EDGES SHALL BE CLEANED AND WETTED WITH A CONCRETE BONDING AGENT AS APPROPRIATE.

3.11.8. DAMAGE: IN-PLACE SHOTCRETE WHICH EXHIBITS SAGS OR SLOUGHS, SEGREGATION, HONEYCOMBING, SAND POCKETS OR OTHER OBVIOUS DEFECTS SHALL BE REMOVED AND REPLACED. SHOTCRETE ABOVE SAGS AND SLOUGHS SHALL BE REMOVED AND REPLACED WHILE STILL PLASTIC.

3.11.9. CURING: DURING THE CURING PERIODS SPECIFIED HEREIN, SHOTCRETE SHALL BE MAINTAINED ABOVE 40° F, AND IN MOIST CONDITION. IN INITIAL CURING, SHOTCRETE SHALL BE KEPT CONTINUOUSLY MOIST FOR 24 HOURS AFTER PLACEMENT IS COMPLETE. FINAL CURING SHALL CONTINUE FOR SEVEN DAYS AFTER SHOTCRETING, FOR THREE DAYS IF HIGH-EARLY-STRENGTH CEMENT IS USED, OR UNTIL THE SPECIFIED STRENGTH IS OBTAINED. FINAL CURING SHALL CONSIST OF A FOG SPRAY OR AN APPROVED MOISTURE-RETAINING COVER OR MEMBRANE. IN SECTIONS WITH A DEPTH IN EXCESS OF 12", FINAL CURING SHALL BE THE SAME AS THAT FOR INITIAL CURING.

3.11.10. STRENGTH TEST: STRENGTH TEST FOR SHOTCRETE SHALL BE MADE BY AN APPROVED AGENCY ON SPECIMENS WHICH ARE REPRESENTATIVE OF WORK AND WHICH HAVE BEEN WATER SOAKED FOR AT LEAST 24 HOURS PRIOR TO TESTING. WHEN THE MAXIMUM SIZE AGGREGATE IS LARGER THAN 3/8" SPECIMENS SHALL CONSIST OF NOT LESS THAN (3) 3" DIAMETER CORES OR 3-INCH CUBES. WHEN THE MAXIMUM SIZE AGGREGATE IS 3/8" OR SMALLER, SPECIMENS SHALL CONSIST OF NOT LESS THAN (3) 2" DIAMETER CORES OR 2" CUBES. SPECIMENS SHALL BE TAKEN IN ACCORDANCE WITH ONE OF THE FOLLOWING:

A. FROM THE IN-PLACE WORK: TAKEN AT LEAST ONCE EACH SHIFT BUT NOT LESS THAN ONE FOR EACH 50 CUBIC YARDS OF SHOTCRETE; OR
B. FROM TEST PANELS: MADE NOT LESS THAN ONCE EACH SHIFT OR NOT LESS THAN ONE FOR EACH 50 CUBIC YARDS OF SHOTCRETE PLACED. WHEN THE MAXIMUM SIZE AGGREGATE IS LARGER THAN 3/8", THE TEST PANELS SHALL HAVE A MINIMUM DIMENSION OF 18" X 18". WHEN THE MAXIMUM SIZE AGGREGATE IS 3/8" OR SMALLER, THE TEST PANELS SHALL HAVE A MINIMUM DIMENSION OF 12" X 12". PANELS SHALL BE GUNNED IN THE SAME POSITION AS THE WORK, DURING THE COURSE OF THE WORK AND BY NOZZLEMEN DOING THE WORK. THE CONDITION UNDER WHICH THE PANELS ARE CURED SHALL BE THE SAME AS THE WORK.

THE AVERAGE OF THREE CORES FROM A SINGLE PANEL SHALL BE EQUAL TO OR EXCEED 0.85 f'_c WITH NO SINGLE CORE LESS THAN 0.75 f'_c. THE AVERAGE OF THREE CUBES TAKEN FROM A SINGLE PANEL MUST EQUAL OR EXCEED f'_c WITH NO INDIVIDUAL CUBE LESS THAN 0.88 f'_c TO CHECK TESTING ACCURACY. LOCATIONS REPRESENTED BY ERRATIC CORE STRENGTHS MAY BE RETESTED.

3.11.11. INSPECTIONS

A. DURING PLACEMENT. WHEN SHOTCRETE IS USED FOR STRUCTURAL MEMBERS, A SPECIAL INSPECTOR IS REQUIRED BY IBC TABLE 1705.3. THE SPECIAL INSPECTOR SHALL PROVIDE CONTINUOUS INSPECTION OF THE PLACEMENT OF THE REINFORCEMENT AND SHOTCRETING AND SHALL SUBMIT A STATEMENT INDICATING COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.
B. VISUAL EXAMINATION FOR STRUCTURAL SOUNDNESS OF IN-PLACE SHOTCRETE. COMPLETED SHOTCRETE WORK SHALL BE CHECKED VISUALLY FOR REINFORCING BAR EMBEDMENT, VOIDS, ROCK POCKETS, SAND STREAKS AND SIMILAR DEFICIENCIES BY EXAMINING A MINIMUM OF (3) 3" CORES TAKEN FROM (3) AREAS CHOSEN BY THE DESIGN ENGINEER WHICH REPRESENT THE WORST CONGESTION OF REINFORCING BARS OCCURRING IN THE PROJECT. EXTRA REINFORCING BARS MAY BE ADDED TO NON-CONGESTED AREAS TO REPRESENT THE MOST HEAVILY CONGESTED AREAS ELSEWHERE ON THE PROJECT. THE CORES SHALL BE EXAMINED BY THE SPECIAL INSPECTOR AND A REPORT SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO FINAL APPROVAL OF THE SHOTCRETE.
C. TESTING EQUIPMENT. THE EQUIPMENT USED IN PRE-CONSTRUCTION TESTING SHALL BE THE SAME EQUIPMENT USED IN THE WORK REQUIRING SUCH TESTING, UNLESS SUBSTITUTE EQUIPMENT IS APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING OFFICIAL.

3.12. ADHESIVE EXPANSIVE WATERSTOPS

ADHESIVE EXPANSIVE WATERSTOP SHALL BE VOLCLAY WATERSTOP-RX (AS MANUFACTURED BY CETCO), SWELLSTOP OR HYDROTIGHT (GREENSTREAK), OR APPROVED EQUIVALENT. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.

3.13. CONCRETE COORDINATION DRAWINGS

PRIOR TO THE START OF CONCRETE WALL OR ELEVATED SLAB CONSTRUCTION THE CONTRACTOR SHALL SUBMIT CONCRETE COORDINATION DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL. COORDINATION DRAWINGS SHALL INCLUDE DIMENSIONS AND SIZES FOR EMBED LOCATIONS, DOOR AND WINDOW OPENINGS, MECHANICAL PENETRATIONS, AND OTHER APPROPRIATE ITEMS.

4. MASONRY

4.1. MORTAR

CMU:

ASTM C270, TYPE S, f'_c = 1800 PSI AT 28 DAYS

4.2. GROUT

ASTM C476, f'_c = 2500 PSI AT 28 DAYS, 5-1/2 SACK MIX (MINIMUM), 3/8" MAX AGGREGATE SLUMP 8" TO 11"

4.3. REINFORCEMENT: SEE STRUCTURAL CONCRETE MATERIALS SECTION OF THESE NOTES.

4.4. CONCRETE MASONRY UNITS (CMU)

CONFORM TO ASTM C90, MINIMUM FACE SHELL THICKNESS OF 1-1/4", GRADE N-1. MINIMUM COMPRESSIVE STRENGTH OF MASONRY (f'_m) SHALL BE 2000 PSI, UNLESS NOTED OTHERWISE.

4.5. INSTALLATION OF MASONRY UNITS.

PER THE IBC SECTION 2104 FOR UNIT MASONRY CONSTRUCTION REQUIREMENTS.

4.6. MASONRY COORDINATION DRAWINGS:

PRIOR TO THE START OF MASONRY CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT MASONRY COORDINATION DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL. COORDINATION DRAWINGS SHALL INCLUDE DIMENSIONS AND SIZES FOR EMBED LOCATIONS, DOOR AND WINDOW OPENINGS, MECHANICAL PENETRATIONS, AND OTHER APPROPRIATE ITEMS.

4.7. CONDUIT OR PIPING EMBEDDED IN MASONRY:

4.7.1. NO MASONRY LINTELS SHALL BE SLEEVED FOR PIPING OR CONDUIT EXCEPT AS NOTED ON THE STRUCTURAL DRAWINGS OR AS APPROVED BY THE ENGINEER.

4.7.2. CONDUIT SHALL NOT BE PLACED WITHIN CELLS CONTAINING REINFORCING UNLESS APPROVED BY THE ENGINEER.

4.8. MASONRY VENEER (BRICK, CMU, OR STONE UNITS)

4.8.1. MATERIALS: SEE ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS.

4.8.2. ADHERED MASONRY VENEER

ADHERED UNITS SHALL NOT EXCEED 2-5/8" IN SPECIFIED THICKNESS, 36" IN ANY FACE DIMENSION, OR MORE THAN 5 SQUARE FEET OF FACE DIMENSION AND SHALL NOT WEIGH MORE THAN 15 PSF. ADHERED MASONRY VENEER SHALL CONSIST OF A FULLY ENGINEERED SYSTEM (BACKING BOARD, GROUT AND LATHE/REINFORCING) COMPLYING WITH SEISMIC DESIGN CATEGORY NOTED IN THE DESIGN CRITERIA SECTION OF THESE NOTES. THE ADHERED MASONRY VENEER SYSTEM SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL.

City of Puyallup
Building
REVIEWED
FOR
COMPLIANCE
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Approval of submitted plans is not an approval of omissions or oversight by this office or noncompliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable building codes and regulations of the local government.

THE APPROVED CONSTRUCTION PLANS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.
PRINT in COLOR and to SCALE.

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WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER
707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2,
2024

ORIGINAL ISSUE: 08/17/16

REVISIONS

No.	Description	Date

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PROJECT NUMBER

KJK
DRAWN BY

ADM
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WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER

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STRUCTURAL NOTES

S0.1

5. METALS

5.1. STRUCTURAL STEEL

5.1.1. STEEL W SHAPES AND C & MC SHAPES 8" OR LARGER SHALL BE ASTM A992 (F_y=50 KSI).

5.1.2. STEEL M, S, HP AND L SHAPES SHALL BE ASTM A572 Gr. 50 (F_y=50 KSI).

5.1.3. STEEL PLATES THAT ARE PART OF THE SEISMIC FORCE RESISTING SYSTEM SHALL BE ASTM A572 Gr. 50 (F_y=50 KSI).

5.1.4. OTHER STEEL PLATES AND C & MC SHAPES SMALLER THAN 8" SHALL BE ASTM A36 (F_y=36 KSI).

5.1.5. STEEL PIPE SECTIONS (PIPE) SHALL BE ASTM A53 Gr. B (F_y=35 KSI).

5.1.6. RECTANGULAR AND ROUND HOLLOW STEEL SECTIONS (HSS) OR TUBE STEEL SECTIONS (TS) SHALL BE ASTM A500, GR. C (F_y=50 KSI).

5.1.7. STRUCTURAL TEES SHALL BE CUT FROM W, M OR S SHAPES TO MAKE WT, MT AND ST SHAPES.

5.1.8. BOLTS

A. MACHINE BOLTS NOT SPECIFIED AS HIGH STRENGTH SHALL BE ASTM A307 GRADE A.

B. HIGH STRENGTH BOLTS SHALL BE ASTM F3125 GRADE A325 OR GRADE A490 AS INDICATED ON STRUCTURAL DRAWINGS. ALL BOLTS SHALL BE CONSIDERED BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE (CONNECTION TYPE N) UNLESS NOTED OTHERWISE. ALL HIGH STRENGTH BOLTED CONNECTIONS SHALL BE INSTALLED WITH NUTS CONFORMING TO ASTM A563 AND HARDENED WASHERS CONFORMING TO ASTM F436.

C. HIGH STRENGTH BOLTS WITH TWIST OFF TYPE TENSION CONTROL MAY BE SUBSTITUTED FOR CONVENTIONAL BOLTS AND SHALL BE ASTM F3125 GRADE F1852 OR GRADE F2280, AND MAY BE USED FOR GRADE A325 OR GRADE A490 RESPECTIVELY.

D. ALL HIGH STRENGTH BOLTS SHALL BE INSTALLED PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS (LATEST EDITION) BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (WWW.BOLTCOUNCIL.ORG).

5.1.9. STEEL ANCHORAGE ELEMENTS:

A. THREADED RODS SHALL BE ALL-THREAD ASTM A36 (F_y=36 KSI) UNLESS NOTED OTHERWISE.

B. WELDED HEADED STUDS: "NELSON STUDS" SHALL BE BY NELSON STUD WELDING, INC. OR APPROVED EQUIVALENT COMPLYING WITH ASTM A108. STUDS SHALL HAVE A MINIMUM F_u OF 65 KSI.

C. ANCHOR RODS: ANCHOR RODS SHALL BE ASTM F1554, F_y=36 KSI WITH HOOKED, HEADED OR THREADED AND NUTTED ENDS AS INDICATED. AT COLUMN LOCATIONS ANCHOR RODS SHALL BE ASTM F1554, F_y=36 KSI WITH HEADED OR THREADED/NUTTED END. TACK WELD NUT TO ANCHOR ROD UNLESS NOTED OTHERWISE. WHERE NOTED, HIGH STRENGTH ANCHOR RODS SHALL BE ASTM F1554, F_y=105 KSI WITH DOUBLE NUTTED PLATE WASHER.

D. EXPANSION ANCHORS SHALL BE CARBON STEEL AS NOTED IN THE FOLLOWING TABLE. ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND/OR ICC-ES AC193 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. ANCHORS SHALL HAVE A CURRENT CODE REPORT THAT COMPLIES WITH THE CURRENT EDITION OF THE IBC AND SHALL BE RATED FOR USE IN THE SEISMIC DESIGN CATEGORY NOTED IN THE DESIGN CRITERIA SECTION OF THESE NOTES.

EXPANSION ANCHORS IN CONCRETE	CODE REPORT
HILTI KWIK BOLT TZ	ICC ESR-1917
SIMPSON STRONG-BOLT 2	ICC ESR-3037
DEWALT POWER-STUD+ SD2	ICC ESR-2502

EXPANSION ANCHORS IN GROUT FILLED CONCRETE MASONRY	CODE REPORT
HILTI KWIK BOLT 3	ICC ESR-1385
SIMPSON STRONG-BOLT 2	IAPMO ER-240
DEWALT POWER-STUD+ SD1	ICC ESR-2966

E. HEAVY DUTY CONCRETE/MASONRY SCREW ANCHORS SHALL BE USED IN DRY INTERIOR CONDITIONS AND SHALL BE AS NOTED IN THE FOLLOWING TABLE:

HEAVY DUTY CONCRETE/ MASONRY SCREW ANCHORS	CODE REPORT
HILTI KWIK HUS-EZ	ICC ESR-3027(CONC) ICC ESR-3056 (CMU)
SIMPSON TITEN HD	ICC ESR-2713 (CONC) ICC ESR-1056 (CMU)
DEWALT SCREW BOLT+	ICC ESR-3889 (CONC) ICC ESR-4042 (CMU)

F. ADHESIVE ANCHORS SHALL BE THREADED ANCHOR RODS OR REBAR DOWELS USING AN INJECTABLE ADHESIVE AS NOTED IN THE FOLLOWING TABLE. ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND/OR ICC-ES AC-308 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. ANCHORS SHALL HAVE A CURRENT CODE REPORT THAT COMPLIES WITH THE CURRENT EDITION OF THE IBC AND SHALL BE RATED FOR USE IN THE SEISMIC DESIGN CATEGORY NOTED IN THE DESIGN CRITERIA SECTION OF THESE NOTES.

ADHESIVE ANCHORS IN CONCRETE (1) (2)	CODE REPORT
HILTI HIT HY-200 SAFE SET	ICC ESR-3187
SIMPSON AT-XP (3)	IAPMO ER-263
DEWALT AC200+ DUST-X	ICC ESR-4027

ADHESIVE ANCHORS IN GROUT FILLED CONCRETE MASONRY	CODE REPORT
HILTI HIT HY-270	ICC ESR-4143
SIMPSON AT-XP *	IAPMO ER-281
DEWALT AC100+ GOLD	ICC ESR-3200

(1) ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI, OR AN APPROVED ALTERNATE WHEN SUBMITTED AND APPROVED BY THE ENGINEER. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.

(2) ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS.

(3) SIMPSON SET-XP MAY BE USED WHERE BASE MATERIAL TEMPERATURE IS ABOVE 50 DEGREES FAHRENHEIT OR FOR EMBEDMENT GREATER THAN 12-INCHES FOR LONGER GEL TIME. SEE ICC ESR-2508 (CONC) AND IAPMO ER-265 (MASONRY).

G. POWDER ACTUATED FASTENERS: PDF'S OR PAF'S SHALL BE A MINIMUM 0.157" DIA KNURLED SHANK FASTENER AS NOTED IN THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. FASTENERS DRIVEN INTO STEEL SHALL BE DRIVEN SO THAT THE POINT OF THE FASTENER COMPLETELY PENETRATES THE STEEL BASE MATERIAL. AT TOPPING SLABS, PT SLABS OR SLABS WITH RADIANT HEAT TUBES EMBEDDED WITHIN THE SLAB, LIMIT THE PDF PENETRATION TO 3/4" MAXIMUM AND COORDINATE WITH TENDON/TUBE PLACEMENT AND COVER.

POWDER ACTUATED FASTENERS	CODE REPORT
HILTI X-U	ICC ESR-2269
SIMPSON PDPA	ICC ESR-2138
DEWALT CSI PIN	ICC ESR-2024

H. CONCRETE/MASONRY SCREWS SHALL BE AS NOTED IN THE FOLLOWING TABLE:

CONCRETE/MASONRY SCREWS	CODE REPORT
HILTI KWIK CON II+	-
SIMPSON TITEN	-
DEWALT TAPPER+	ICC ESR-3068 (CONC) ICC ESR-3196 (MAS)

5.1.10. METAL PROTECTION: ALL STEEL EXPOSED TO WEATHER, MOISTURE, SOIL, OR AS NOTED SHALL BE GALVANIZED PER ASTM A123 OR A153 AS APPLICABLE. ALL OTHER STEEL SURFACES SHALL BE SHOP PRIMED AFTER FABRICATION.

REPAIR ALL DAMAGED AREAS OF GALVANIZED PARTS SUCH AS FIELD WELDS, ETC. APPLY REPAIR COATING THICKNESS GREATER THAN OR EQUAL TO ORIGINAL ZINC COATING THICKNESS.

5.1.11. STEEL COLUMNS: ALL VERTICAL LOAD CARRYING MEMBERS HAVE BEEN NOTED AS "COLUMNS" ON THE STRUCTURAL DRAWINGS. THIS NOTATION DOES NOT IDENTIFY THESE MEMBERS AS "POSTS" OR "COLUMNS" AS DEFINED BY THE LATEST OSHA RULES REGARDING COLUMN ANCHORAGE REQUIREMENTS (OSHA 29 CFR PARTS 1926.751 AND 1926.755). THE GENERAL CONTRACTOR, STEEL DETAILER, AND STEEL ERECTOR SHALL BE RESPONSIBLE TO DETERMINE THE CORRECT OSHA DESIGNATION OF EACH MEMBER REGARDLESS OF THE NOTATION SHOWN ON THE STRUCTURAL DRAWINGS.

5.1.12. LIGHT GAGE STEEL SHEATHING

LIGHT GAGE STEEL SHEATHING SHALL BE FROM STEEL SHEETS OF A THICKNESS INDICATED ON SHEARWALL SCHEDULE. STEEL SHEETS SHALL BE ASTM A1003 GRADE 33 TYPE H UNLESS NOTED OTHERWISE.

A. AT SHEARWALLS, WALL STUDS SHALL HAVE A MINIMUM FLANGE WIDTH OF 1 5/8" WITH A 3/8" MINIMUM EDGE STIFFENER.

B. STEEL SHEETS SHALL BE ATTACHED TO FRAMING WITH MINIMUM #8 OR #10 SCREWS AT SPACINGS INDICATED ON SHEARWALL SCHEDULE. SEE SHEARWALL SCHEDULE FOR PERMITTED SCREW LENGTH, HEAD PROFILE AND HEAD DIAMETER.

5.2. WELDING

5.2.1. ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE," AWS D1.1, AWS D1.4 AND AWS D1.8 AS APPROPRIATE.

5.2.2. ALL WELDING SHALL BE BY CERTIFIED WELDERS; USE 70 KSI LOW HYDROGEN FILLER METAL AND SHALL BE PROTECTED PER AWS D1.1 UNTIL USE. FOR ALL FULL PENETRATION WELDS, FILLER METAL SHALL BE NOTCH TOUGH TO MEET CHARPY V-NOTCH OF 20 FOOT-POUND AT -20°F.

5.2.3. NO WELDING OF REINFORCING STEEL SHALL BE ALLOWED EXCEPT WHERE SHOWN. ALL WELDING OF REINFORCEMENT SHALL BE PER ANSI/AWS D1.4. THE FOLLOWING FILLER METAL SHALL BE USED WHEN WELDING REINFORCEMENT:

A. FOR WELDING OF ASTM A706 GR 60 REBAR, 80 KSI FILLER METAL.

B. FOR WELDING OF ASTM A615 GR 60 REBAR, NOT PERMITTED.

C. FOR WELDING OF ASTM A615 GR 40 REBAR, NOT PERMITTED.

5.2.4. ALL FULL PENETRATION FIELD AND SHOP WELDS SHALL BE FULL TIME INSPECTED AND TESTED BY NON-DESTRUCTIVE PROCEDURES. RESULTS OF TESTS SHALL BE SUBMITTED FOR REVIEW BY THE STRUCTURAL ENGINEER.

5.3. WELDING PROCEDURE SPECIFICATION (WPS)

5.3.1. FOR ALL WELDING OF REINFORCING STEEL, NON-PREQUALIFIED WELDS AND ALL WELDING OF COMPONENTS WHICH ARE PART OF THE SEISMIC FORCE RESISTING SYSTEM, CONTRACTOR SHALL SUBMIT A WELDING PROCEDURE SPECIFICATION (WPS) TO ENGINEER FOR APPROVAL. PRIOR TO WELDING, EACH WPS SHALL INCLUDE ALL NECESSARY INFORMATION REQUIRED BY AWS D1.1, AWS D1.4 AND AWS D1.8 AND AS FOLLOWS:

A. APPLICABLE BASE METAL TYPES AND THICKNESSES.

B. SKETCH OF JOINT INDICATING APPLICABLE DIMENSIONS. INDIVIDUAL PASSES SHALL BE IDENTIFIED AND NUMBERED TO IDENTIFY THE SEQUENCE. THE SKETCH SHALL IDENTIFY THE MAXIMUM THICKNESS AND BEAD WIDTH. IN NO CASE SHALL THE LAYER THICKNESS EXCEED 1/4" NOR THE BEAD WIDTH EXCEED 5/8."

C. PREHEAT REQUIREMENTS.

D. ELECTRICAL CHARACTERISTICS (I.E., CURRENT, VOLTAGE, TRAVEL SPEED, ETC.).

E. ELECTRODE REQUIREMENTS SHALL MEET THE REQUIREMENTS OF AWS A5.1, AWS A5.5, AWS A5.17, AWS A5.23, AWS A5.18, AWS A5.20, AWS A5.28, AND AWS A5.29, AS APPLICABLE FOR WELDING METHOD USED.

5.4. COLD FORMED STEEL FRAMING

5.4.1. ALL STUD AND/OR JOIST FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE AND GAGE AS SHOWN AND SHALL BE MANUFACTURED BY A CURRENT MEMBER OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA).

5.4.2. ALL STRUCTURAL STEEL STUDS AND JOISTS HAVE BEEN SPECIFIED PER THE GUIDELINES GIVEN IN THE STEEL STUD MANUFACTURER'S ASSOCIATION ICC ESR-3064P. THE IDENTIFICATION CODE IDENTIFIES THE MEMBER DEPTH (IN MILS), SECTION TYPE, FLANGE WIDTH (IN MILS), AND MATERIAL THICKNESS (IN MILS). THE ICC REPORT IS AVAILABLE AT WWW.SSMA.COM.

FOR EXAMPLE, 600 S 125 - 54 [50]:

600 REFERS TO MEMBER DEPTH (600 X 1/100 = 6")

S REFERS TO SECTION TYPE (S=STUD, T= TRACK)

125 REFERS TO FLANGE WIDTH (125 X 1/100 = 1.25")

54 REFERS TO MEMBER THICKNESS (54 MILS OR 16 Ga)

50 REFERS TO YIELD STRENGTH (KSI) IF OTHER THAN NOTED BELOW

5.4.3. ALL STUDS AND/OR JOISTS, 12, 13, 14 AND 16 GAGE, SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF THE LATEST EDITION OF THE AISI (AMERICAN IRON AND STEEL INSTITUTE) STANDARDS WITH A MINIMUM YIELD STRENGTH OF 50 KSI.

5.4.4. ALL 18 AND 20 GAGE STUDS AND/OR JOISTS, AND ALL TRACK, BRIDGING, END CLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF THE LATEST EDITION OF THE AISI (AMERICAN IRON AND STEEL INSTITUTE) STANDARDS WITH A MINIMUM YIELD STRENGTH OF 33 KSI.

5.4.5. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING IN ACCORDANCE WITH ASTM A525.

5.4.6. ALL FRAMING COMPONENTS SHALL BE CUT FOR A TIGHT FIT AGAINST ABUTTING MEMBERS, UNLESS SHOWN OTHERWISE. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.

5.4.7. AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB, PRIOR TO STUD AND TRACK ATTACHMENT.

5.4.8. NON-AXIALLY LOADED WALL STUDS SUCH AS INTERIOR PARTITION WALLS AND EXTERIOR INFILL WALLS SHALL HAVE VERTICAL DEFORMATION CAPABILITY BETWEEN WALL AND ROOF/FLOOR STRUCTURE UNLESS SHOWN OTHERWISE.

5.5. STEEL DECKING

5.5.1. ALL STEEL DECKING SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A653, F_y = 38 KSI, AS A MINIMUM. THE GALVANIZED COATING SHALL CONFORM TO ASTM A653, G-60 OR G-90 SPECIFICATIONS AS APPLICABLE.

5.5.2. ALL STEEL DECKING SHALL BE PLACED ON SUPPORTING FRAMEWORK WITH A MINIMUM END LAP OF TWO INCHES CENTERED OVER SUPPORTS. THE DECK SHALL BE ATTACHED TO SUPPORTS AND FASTENED AT SIDE LAPS PER THE DIAPHRAGM SCHEDULE.

5.5.3. DECKING SHALL BE AS MANUFACTURED BY ASC PROFILES, INC. OR EQUIVALENT. ALTERNATE MANUFACTURERS SHALL BE PERMITTED PROVIDED THE DECK MEETS OR EXCEEDS THE SECTION PROPERTIES OF THE SPECIFIED DECK AND THE ALLOWABLE SHEAR CAPACITY SPECIFIED IN THE DECK ATTACHMENT SCHEDULE.

5.5.4. CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE WITH ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ANY SPECIAL REQUIREMENTS.

5.5.5. CONTRACTOR SHALL INSTALL DECKING AS REQUIRED TO PREVENT MARRING/DAMAGING DECKING AT LOCATIONS EXPOSED TO VIEW FROM BELOW.

6. CARPENTRY

DIMENSION LUMBER SHALL BE DF No.2. SAWN LUMBER BEAMS, HEADERS AND COLUMNS SHALL BE DF No.1 OR AS SHOWN ON THE DRAWINGS. ALL 2" NOMINAL LUMBER SHALL BE KILN DRIED (KD). EACH PIECE OF LUMBER SHALL BEAR STAMP OF WEST COAST LUMBER INSPECTION BUREAU (WCLIB) AND/OR WESTERN WOOD PRODUCTS ASSOCIATION (WWPA) SHOWING GRADE MARK.

6.1. PRESSURE-PRESERVATIVE TREATMENT IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1, LATEST EDITION TO THE USE CATEGORY AS FOLLOWS:

6.1.1. TREAT ALL WOOD IN CONTACT WITH CONCRETE, MORTAR, GROUT, MASONRY AND WITHIN 12" OF EARTH TO THE REQUIREMENTS OF USE CATEGORY UC2 (INTERIOR/DAMP).

6.1.2. TREAT ALL WOOD EXPOSED TO WEATHER BUT PROTECTED BY PAINT OR COVER TO THE REQUIREMENTS OF USE CATEGORY UC3A (ABOVE GROUND PROTECTED).

6.1.3. TREAT ALL WOOD EXPOSED TO WEATHER SUCH AS EXTERIOR DECKING, JOISTS, BEAMS, RAILINGS, ETC TO THE REQUIREMENTS OF USE CATEGORY UC3B (ABOVE GROUND EXPOSED).

6.1.4. TREAT ALL WOOD IN CONTACT WITH THE GROUND, SOIL OR FRESH WATER TO THE REQUIREMENTS OF USE CATEGORY UC4A (GROUND CONTACT GENERAL USE).

6.1.5. TREAT ALL LUMBER NOTED AS FIRE TREATED TO THE REQUIREMENTS OF USE CATEGORY UCFA (FIRE RETARDANT INTERIOR).

6.1.6. WHERE POSSIBLE, PRECUT MATERIAL PRIOR TO TREATMENT. ALL FIELD CUTS AND DRILLED HOLES SHALL BE FIELD TREATED IN ACCORDANCE WITH AWPA M-4.

6.2. CARPENTRY HARDWARE

6.2.1. MACHINE BOLTS SHALL BE ASTM A307.

6.2.2. PROVIDE MALLEABLE IRON WASHERS (MIW) OR HEAVY PLATE CUT WASHERS WHERE BOLT HEADS, NUTS OR LAG SCREWS BEAR ON WOOD.

6.2.3. NAILS SHALL BE COMMON, AMERICAN OR CANADIAN MANUFACTURER ONLY WITH MIN. DIAMETERS AS FOLLOWS:

NAIL SIZE	MINIMUM NAIL SHANK DIAMETER	MINIMUM NAIL LENGTH
8d	0.131"	2 1/2"
10d	0.148"	3"
12d	0.148"	3 1/4"
16d SINKER	0.148"	3 1/4"
16d	0.162"	3 1/2"
20d	0.192"	4"

6.2.4. LAG SCREWS SHALL MEET THE REQUIREMENTS OF ANSI/ASME B18.2.1. WOOD SCREWS SHALL MEET THE REQUIREMENTS OF ANSI/ASME B18.6.1.

6.2.5. ANCHORS AND CONNECTIONS SHALL BE SIMPSON, USP, OR ICC (INTERNATIONAL CODE COUNCIL) APPROVED. ALL FASTENERS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS UNLESS OTHERWISE SHOWN. SUBSTITUTED CONNECTIONS SHALL HAVE A TABULATED CAPACITY EQUAL TO OR GREATER THAN THE SPECIFIED CONNECTOR.

6.2.6. CORROSION RESISTANT HARDWARE AND FASTENERS:

A. FASTENERS AND HARDWARE EXPOSED TO WEATHER OR IN UNHEATED PORTIONS OF THE BUILDING SHALL BE MECHANICALLY OR HOT DIPPED GALVANIZED PER ASTM B695 - CLASS 55 OR ASTM A153 - CLASS D. HARDWARE IN CONTACT WITH TREATED WOOD SHALL CONFORM TO A MINIMUM GALVANIZED COATING OF G185 OR AS NOTED BELOW.

B. IF PRESERVATIVE TREATMENT USED IS ACZA (AMMONIACAL COPPER ZINC ARSENATE), IF THE CHEMICAL RETENTION LEVEL IS AWPA USE CATEGORY UC4A OR GREATER, OR IF THE PRESERVATIVE TREATMENT USED IS NOT KNOWN, HARDWARE SHALL BE TYPE 316L STAINLESS STEEL. FASTENERS SHALL BE TYPE 304 OR 305 STAINLESS STEEL.

C. HARDWARE IN MARINE ENVIRONMENT SHALL BE TYPE 316L STAINLESS STEEL. FASTENERS SHALL BE TYPE 316 STAINLESS STEEL, HOT DIPPED GALVANIZED TO ASTM A153 - CLASS C, SILICON BRONZE, OR COPPER.

D. IN THE EVENT OF A CONFLICT BETWEEN THE HARDWARE MANUFACTURER'S RECOMMENDATIONS FOR SELECTING CORROSION-RESISTANT HARDWARE AND FASTENERS, THESE NOTES, AND THE SPECIFICATIONS, THE MOST STRINGENT REQUIREMENT SHALL BE USED UNLESS APPROVED BY THE ENGINEER.

6.3. MINIMUM NAILING: PER IBC TABLE 2304.10.1 FASTENING SCHEDULE.

6.4. COORDINATION AT HOLES IN WOOD STUD WALLS

6.4.1. PIPES IN INTERIOR NONBEARING WALLS: STUD PARTITIONS CONTAINING PIPES SHALL BE FRAMED, AND THE JOISTS SHALL BE SPACED, SO AS TO GIVE PROPER CLEARANCE FOR THE PIPING. WHERE A PARTITION CONTAINING PIPING RUNS PARALLEL TO THE JOISTS, THE JOISTS SHALL BE DOUBLED AND SPACED SO AS TO PERMIT THE PASSAGE OF SUCH PIPING AND SHALL BE BRIDGED. WHERE PIPES ARE PLACED IN, OR PARTIALLY IN, A PARTITION NECESSITATING THE CUTTING OF THE SOLES OR PLATES, A SIMPSON RPS STRAP SHALL BE FASTENED TO EACH PLATE ACROSS AND TO EACH SIDE OF THE OPENING WITH NOT LESS THAN SIX 16d NAILS.

6.4.2. CUTTING AND NOTCHING SAWN LUMBER: IN EXTERIOR WALLS AND BEARING PARTITIONS, ANY WOOD STUD IS PERMITTED TO BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 15 PERCENT OF ITS WIDTH. CUTTING OR NOTCHING OF STUDS TO A DEPTH NOT GREATER THAN 40 PERCENT OF THE WIDTH OF THE STUD IS PERMITTED IN NONBEARING PARTITIONS SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE PARTITION.

6.4.3. CUTTING AND NOTCHING ENGINEERED LUMBER: CUTTING AND NOTCHING SHALL NOT BE PERMITTED IN ENGINEERED LUMBER (LSL) STUDS WITHOUT APPROVAL FROM THE ENGINEER OF RECORD.

6.4.4. BORED HOLES IN SAWN LUMBER: A HOLE NOT GREATER IN DIAMETER THAN 33 PERCENT OF THE STUD WIDTH IS PERMITTED TO BE BORED IN ANY WOOD STUD WITHOUT ENGINEERING VERIFICATION. BORED HOLES NOT GREATER THAN 60 PERCENT OF THE WIDTH OF THE STUD ARE PERMITTED IN NONBEARING PARTITIONS, PROVIDED NOT MORE THAN ANY TWO ADJACENT STUDS ARE SO BORED. IN NO CASE SHALL THE EDGE OF THE BORED HOLE BE NEARER THAN 5/8-INCH FROM THE EDGE OF THE STUD. BORED HOLES SHALL NOT BE LOCATED AT THE SAME SECTION OF STUD AS A NOTCH OR CUT AND SHALL NOT BE LOCATED WITHIN 8-INCHES OF THE END OF THE STUD.

6.4.5. BORED HOLES IN ENGINEERED LUMBER: BORED HOLES SHALL NOT BE PERMITTED IN ENGINEERED LUMBER (LSL) STUDS WITHOUT APPROVAL FROM THE ENGINEER OF RECORD.

6.5. SHEATHING (WOOD STRUCTURAL PANEL SHEATHING)

EACH PANEL SHALL BE IDENTIFIED WITH THE APPROPRIATE TRADEMARK OF APA, AND SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF VOLUNTARY PRODUCT STANDARD PS1, VOLUNTARY PRODUCT STANDARD PS2 OR ANSI/APA PRP-210. PANEL PERFORMANCE CATEGORY, GRADE AND GROUPNUMBER OR SPAN RATING SHALL BE AT LEAST EQUAL TO THAT SHOWN ON THE DRAWINGS. APPLICATION SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF APA. ALL PLYWOOD SHALL BE C-D INTERIOR WITH EXTERIOR GLUE OR AS NOTED ON THE DRAWINGS AND SHALL BE GROUP I OR II SPECIES. EXCEPT AS OTHERWISE SHOWN, PROVIDE THE FOLLOWING MINIMUM NAILING: PANEL EDGES 10d AT 6" ON CENTER, INTERMEDIATE SUPPORT 10d AT 12" ON CENTER. GAP SHEETS 1/8" FOR 4'x8' SHEETS AND 1/4" FOR 8'x8' AND LARGER SHEETS, UNLESS OTHERWISE INDICATED BY PANEL MANUFACTURER. THE MOISTURE CONTENT SHALL NOT BE GREATER THAN 15% AT TIME OF ROOFING.

6.6. GLUED-LAMINATED TIMBER

MATERIALS, MANUFACTURE AND QUALITY CONTROL PER ANSI/AITC A190.1 "STANDARD FOR WOOD PRODUCTS - STRUCTURAL GLUED LAMINATED TIMBER." CAMBER 1-1/2 TIMES DEAD LOAD DEFLECTION WHERE NOT INDICATED ON DRAWINGS. ALL BEAM MEMBERS SHALL BE COMBINATION 24F-V4 FOR SIMPLE SPANS AND 24F-V8 FOR CONTINUOUS OR CANTILEVERED SPANS AND HAVE EXTERIOR GLUE. ALL COLUMN MEMBERS SHALL BE 24F-V8 UNLESS NOTED OTHERWISE. ALL MEMBERS EXPOSED TO VIEW SHALL BE ARCHITECTURAL APPEARANCE GRADE UNLESS NOTED OTHERWISE. ALL MEMBERS CONCEALED FROM VIEW SHALL BE INDUSTRIAL APPEARANCE UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

6.6.1. ADHESIVES SHALL MEET THE REQUIREMENTS FOR WET-USE SERVICE CONDITIONS.

6.6.2. MEMBERS SHALL BE MARKED WITH THE APA TRADEMARK INDICATING CONFORMANCE WITH ANSI/AITC A190.1. IN ADDITION, A CERTIFICATE OF SUCH CONFORMANCE SHALL BE PROVIDED TO THE BUYER, IF REQUESTED.

6.7. MANUFACTURED STRUCTURAL WOOD MEMBERS

6.7.1. TIMBERSTRAND LSL MEMBERS SHALL BE TIMBERSTRAND 1.55E AS MANUFACTURED BY WEYERHAEUSER OR APPROVED EQUIVALENT.

6.7.2. MICROLLAM LVL MEMBERS SHALL BE MICROLLAM 2.0E AS MANUFACTURED BY WEYERHAEUSER OR APPROVED EQUIVALENT.

6.8. PRE-ENGINEERED METAL-PLATE-CONNECTED WOOD TRUSSES

6.8.1. GEOMETRY AND SPACING SHALL BE AS SHOWN. THE MANUFACTURER SHALL PROVIDE ADDITIONAL FRAMING MEMBERS AS SHOWN OR AS NECESSARY TO SUPPORT MECHANICAL EQUIPMENT, WALLS AND/OR PARTITIONS, SNOW DRIFT LOADS, ETC.

6.8.2. WHERE NOTED, PRECUT BLOCKING, BRIDGING, BRACING AND/OR FILLER PIECES SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PROVIDED BY THE TRUSS MANUFACTURER.

6.8.3. TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE ALL PERMANENT INDIVIDUAL MEMBER RESTRAINT/BRACING, INCLUDING WIND UPLIFT BRACING.

6.8.4. GENERAL CONTRACTOR SHALL PROVIDE TEMPORARY INSTALLATION RESTRAINT/BRACING IN ACCORDANCE WITH BCSI-2013 (UPDATE MARCH 2015) BUILDING COMPONENT SAFETY INFORMATION - GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.

6.8.5. FOR TRUSSES SPANNING 35-FEET OR GREATER TRUSS MANUFACTURER SHALL DESIGN THE HEEL PLATES CONSIDERING THE EFFECT OF ECCENTRIC LOADING.

6.8.6. DESIGN LOADS SHALL BE AS STATED IN THE DESIGN CRITERIA SECTION OF THESE NOTES PLUS ANY SPECIAL LOADS INDICATED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, MINIMUM DESIGN LOADS SHALL INCLUDE:

A. TOTAL DEAD LOAD OF 21 PSF (INCLUDING TRUSS SYSTEM DEAD LOADS).

B. BOTTOM CHORD SUPERIMPOSED DEAD LOAD OF 6 PSF (INCLUDED IN TOTAL DEAD LOAD NOTED ABOVE).

C. MINIMUM NET UPLIFT LOAD OF 10 PSF OR AS DETERMINED USING SITE SPECIFIC "COMPONENTS AND CLADDING" WIND FORCES IN ACCORDANCE WITH THE CRITERIA NOTED IN THE DESIGN CRITERIA SECTION OF THESE NOTES, WHICHEVER IS GREATER.

6.8.7. PROPRIETARY COMPONENTS SHALL HAVE ICC (INTERNATIONAL CODE COUNCIL) APPROVAL.

6.8.8. SHOP DRAWINGS SHALL INCLUDE A TRUSS PLACEMENT DIAGRAM AND TRUSS DESIGN DRAWINGS. TRUSS DESIGN DRAWINGS SHALL INCLUDE DETAILS OF ALL PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING (INCLUDING BOTTOM CHORD AND WEB BRACING REQUIRED TO RESIST WIND UPLIFT FORCES). TRUSS DESIGN DRAWINGS SHALL INCLUDE STRUCTURAL CALCULATIONS THAT INDICATE MEMBER STRESSES, SPECIES/GRADES AND APPLICABLE ICC APPROVAL. TRUSS DESIGN DRAWINGS SHALL BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

6.8.9. UNLESS NOTED OTHERWISE, THE TRUSS MANUFACTURER SHALL SPECIFY AND FURNISH CONNECTION HARDWARE NECESSARY FOR INSTALLATION OF THEIR SYSTEM.

6.8.10. DESIGN AND MANUFACTURE OF METAL-PLATE-CONNECTED TRUSSES SHALL CONFORM TO CURRENT STANDARDS OF THE TRUSS PLATE INSTITUTE (TPI): TPI 1-2014: NATIONAL DESIGN STANDARDS FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION.

6.8.11. DELIVERED COMPONENTS SHALL BE ACCOMPANIED BY FABRICATOR'S CERTIFICATE OF CONFORMANCE TO THE REFERENCED STANDARDS, AND BY USER ADVISORY NOTICES EQUIVALENT TO BUILDING COMPONENT SAFETY INFORMATION, BCSI (CURRENT EDITION) - GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING, AND BRACING OF METAL-PLATE-CONNECTED WOOD TRUSSES.

6.9. TONGUE AND GROOVED LUMBER DECKING

6.9.1. SHALL CONFORM TO MOST RECENT AITC-112 "STANDARD FOR TONGUE-AND-GROOVE HEAVY TIMBER ROOF DECKING."

6.9.2. SPECIES SHALL BE DOUGLAS FIR-LARCH F_b = 1,450 PSI MINIMUM. SEE SPECIFICATIONS FOR GRADE OF MATERIAL AND FINISH.

6.9.3. THICKNESS SHALL BE 3" NOMINAL (NET 2.5" MINIMUM). WIDTH SHALL BE 6" NOMINAL UNLESS NOTED OTHERWISE.

6.9.4. MOISTURE CONTENT SHALL BE 15% AVERAGE MEASURED THROUGHOUT THE PIECE.

6.9.5. DECK LAY-UP SHALL BE CONTROLLED RANDOM LAY-UP. SEE THE STRUCTURAL DRAWINGS FOR DECK LAY-UP NOTES. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR APPEARANCE REQUIREMENTS AND LIMITATIONS ON EXPOSED DECKING JOINT LOCATIONS.

6.9.6. AT EXPOSED DECKING LOCATIONS, NAIL LENGTHS SHALL BE ADJUSTED IN ORDER TO PREVENT PENETRATION OF NAILS COMPLETELY THROUGH DECKING. COORDINATE NAIL LENGTHS WITH STRUCTURAL ENGINEER.

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WESLEY BRADLEY PARK
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707 39TH AVENUE SE
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PERMIT SET
DECEMBER 2,
2024

ORIGINAL ISSUE: 08/17/16

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STRUCTURAL NOTES

S0.2

11. STATEMENT OF SPECIAL INSPECTIONS

IBC	SI	SO	TITLE
1705.2	✓	✓	STEEL CONSTRUCTION (SEE TABLES 15A, 15B, AND 15C)
1705.3	✓	✓	CONCRETE CONSTRUCTION (SEE TABLE 13)
1705.4	✓	✓	MASONRY CONSTRUCTION (SEE TABLES 14A AND 14B)
1705.6	✓	N/R	SOILS (SEE TABLE 12A)
1705.13.3	✓	✓	SEISMIC RESISTAINCE – STRUCTURAL WOOD (SEE TABLE 18)
1705.13.5 1705.14.2	✓	N/R	ARCHITECTURAL COMPONENTS (INCLUDING ACCESS FLOORS) (SEE TABLE 18)

SI = SPECIAL INSPECTION

SO = STRUCTURAL OBSERVATION

✓ = ITEM IS REQUIRED

N/R = ITEM IS NOT REQUIRED

SPECIAL INSPECTIONS INDICATED ARE FOR STRUCTURAL ELEMENTS ONLY. SEE ARCH, MECH AND ELEC DRAWINGS FOR ADDITIONAL SPECIAL INSPECTIONS.

11.

11.1. INSPECTION/TESTING REQUIREMENTS:

SEE DRAWINGS, SPECIFICATIONS, AND IBC SECTIONS 110, AND CHAPTER 17.

11.2. INSPECTIONS BY THE BUILDING OFFICIAL (IBC SECTION 110):

11.2.1. FOOTING AND FOUNDATION INSPECTIONS SHALL BE MADE AFTER EXCAVATIONS ARE COMPLETE AND ANY REQUIRED REINFORCING IS IN PLACE. ANY REQUIRED FORMS SHALL BE IN PLACE PRIOR TO INSPECTION.

11.2.2. CONCRETE SLAB AND UNDER FLOOR INSPECTIONS SHALL BE MADE AFTER ALL IN SLAB OR UNDER FLOOR REINFORCING, CONDUIT, PIPING AND OTHER ANCILLARY EQUIPMENT ITEMS AND ACCESSORIES ARE IN PLACE BUT PRIOR TO CONCRETE PLACEMENT OR FLOOR SHEATHING INSTALLATION.

11.2.3. FRAMING INSPECTIONS SHALL BE MADE AFTER ALL SHEATHING, FRAMING, BLOCKING AND BRACING ARE COMPLETE AND ALL PIPES, DUCTS, ELECTRICAL, PLUMBING, ETC., ARE INSTALLED AND APPROVED PRIOR TO COVER.

11.2.4. IN ADDITION TO THE INSPECTIONS SPECIFIED ABOVE, THE BUILDING OFFICIAL IS AUTHORIZED TO MAKE OR REQUIRE OTHER INSPECTIONS OF ANY CONSTRUCTION WORK TO ASCERTAIN COMPLIANCE WITH THE PROVISIONS OF THE IBC OR OTHER LAWS ENFORCED BY THE BUILDING OFFICIAL.

11.3. STRUCTURAL TESTS AND SPECIAL INSPECTIONS (IBC CHAPTER 17):

11.3.1. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

11.3.2. STRUCTURAL TESTS AND SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE IBC AS WELL AS ANY ADDITIONAL REQUIREMENTS OF THE BUILDING OFFICIAL. OMISSION FROM THE LIST BELOW OF TESTING AND INSPECTION REQUIREMENTS SHALL NOT RELIEVE THE CONTRACTOR FROM PROVIDING TESTING AND INSPECTION REQUIRED BY THE SPECIFICATIONS, THE IBC AND THE BUILDING OFFICIAL.

11.3.3. TESTING AND SPECIAL INSPECTIONS SHALL BE COMPLETED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE IBC FOR THE ITEMS LISTED IN THIS SECTION.

11.4. STRUCTURAL OBSERVATION

11.4.1. STRUCTURAL OBSERVATION MAY BE PERFORMED DURING CONSTRUCTION IN A MANNER AS REQUIRED TO BECOME GENERALLY FAMILIAR WITH THE IN-PLACE CONSTRUCTION.

11.4.2. STRUCTURAL OBSERVATION EXTENT SHALL BE AS INDICATED ABOVE. TIMING AND DURATION OF OBSERVATIONS SHALL BE COORDINATED WITH THE GENERAL CONTRACTOR DURING CONSTRUCTION.

11.4.3. CONSTRUCTION OBSERVATION REPORTS AND FINDINGS SHALL NOT BE VIEWED AS A WARRANTY OR GUARANTEE BY THE STRUCTURAL ENGINEER.

11.5. SPECIAL INSPECTOR: SHALL BE CURRENTLY WABO CERTIFIED.

11.5.1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.

11.5.2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, ENGINEER OF RECORD, ARCHITECT OF RECORD, AND OTHER DESIGNATED PERSONS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE GENERAL CONTRACTOR FOR CORRECTION, THEN, IF NOT IN CONFORMANCE, TO THE PROPER DESIGN AUTHORITY AND BUILDING OFFICIAL.

11.5.3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.

12A. REQUIRED SPECIAL INSPECTIONS AND TEST OF SOILS

IBC TABLE 1705.6

SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	N/R	✓
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	N/R	✓
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIAL	N/R	✓
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	✓	N/R
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	N/R	✓

12.

12.1. SPECIAL INSPECTIONS AND TESTS FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT, AND LOAD-BEARING REQUIREMENTS PER IBC 1705.6., AS NOTED IN TABLE 12A.

12.1.1. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONALS SHALL BE USED TO DETERMINE COMPLIANCE.

13. REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

IBC TABLE 1705.3

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	IBC REFERENCE
1.	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	N/R	✓	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2.	REINFORCING BAR WELDING:				
A.	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	N/R	✓	AWS D1.4 ACI 318:26.6.4	
B.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	N/R	✓		
C.	INSPECT ALL OTHER WELDS	✓	N/R		
3.	INSPECT ANCHORS CAST IN CONCRETE	N/R		ACI 318: 17.8.2	
4.	INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS				
A.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	✓	N/R	ACI 318: 17.8.2.4	
B.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4A	N/R	✓	ACI 318: 17.8.2	
5.	VERIFY USE OF REQUIRED DESIGN MIX	N/R	✓	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6.	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	✓	N/R	ASTM C 172 ASTM C 31 ACI318:26.4, 26.12	1908.10
7.	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	✓	N/R	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8.	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	N/R	✓	ACI 318: 26.5.3-26.5.5	1908.9
9.	INSPECT PRESTRESSED CONCRETE FOR:				
A.	APPLICATION OF PRESTRESSING FORCES	✓	N/R	ACI 318: 26.10	
B.	GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE RESISTING SYSTEM	✓	N/R		
10.	INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	N/R	✓	ACI 318: 26.9	
11.	VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	N/R	✓	ACI 318: 26.10.2	
12.	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	N/R	✓	ACI 318: 26.11.1.2(b)	

13.

13.1. CONCRETE: SPECIAL INSPECTION AND TESTING PER IBC TABLE 1705.3 AS NOTED IN TABLE 13, INCLUDING:

13.1.1. CONTINUOUS SPECIAL INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.

13.1.2. CONTINUOUS SPECIAL INSPECTION OF BOLTS INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.

13.1.3. SHOTCRETE: SEE STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

13.1.4. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IAPMO, ETC.).

13.1.5. CONTINUOUS SPECIAL INSPECTION FOR CONCRETE REINFORCING BARS, CONCRETE MATERIALS OR PLACEMENT OF CONCRETE FOR COMPOSITE MEMBERS.

13.2. SPECIAL INSPECTIONS AND TESTS SHALL NOT BE REQUIRED FOR THE FOLLOWING:

13.2.1. NON-STRUCTURAL CONCRETE SLABS ON GRADE.

14.A REQUIRED SPECIAL INSPECTION AND TEST OF MASONRY CONSTRUCTION – MINIMUM VERIFICATION REQUIREMENTS

TMS 602 TABLE 3

MINIMUM VERIFICATION REQUIREMENTS	REQUIRED FOR QUALITY ASSURANCE		REFERENCE FOR CRITERIA
	LEVEL 1	LEVEL 2	TMS 602
1. PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS.	✓	✓	ART. 1.5
2. PRIOR TO CONSTRUCTION, VERIFICATION OF f'_m AND f'_{acc}, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE.	N/R	✓	ART. 1.4 B
3. DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF- CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE.	N/R	✓	ART. 1.5 & 1.6.3
4. DURING CONSTRUCTION, VERIFICATION OF f'_m AND f'_{acc}, FOR EVERY 5,000 SQUARE FEET	N/R	N/R	ART. 1.4 B
5. DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.	N/R	N/R	ART. 1.4 B

14.B REQUIRED SPECIAL INSPECTION AND TEST OF MASONRY CONSTRUCTION – MINIMUM SPECIAL INSPECTION REQUIREMENTS

TMS 602 TABLE 4

	INSPECTION TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCE FOR CRITERIA	
		LEVEL 2	LEVEL 2	TMS 402	TMS 602
1.	AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
A.	PROPORTIONS OF SITE-PREPARED MORTAR	N/R	✓		ART. 2.1, 2.6 A, & 2.6 C
B,	GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	N/R	✓		ART. 2.4 B & 2.4 H
C.	GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	N/R	✓		Art. 3.4 & 3.6 A
D.	PRESTRESSING TECHNIQUE	N/R	✓		Art. 3.6 B
E.	PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	REQUIRED FOR FIRST 5000 SF	REQUIRED AFTER FIRST 5000 SF		Art. 2.1 C.1
F.	SAMPLE PANEL CONSTRUCTION	N/R	✓		Art. 1.6 D
2.	PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
A.	GROUT SPACE	N/R	✓		Art. 3.2 D & 3.2 F
B.	PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES	N/R	✓	Sec. 10.8 & 10.9	Art. 2.4 & 3.6
C.	PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS	N/R	✓	Sec. 6.1, 6.3.1, 6.3.6 & 6.3.7	Art. 3.2 E & 3.4
D.	PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	N/R	✓		Art. 2.6 B & 2.4 G.1.b
3.	VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:				
A.	MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS	N/R	✓		Art. 1.5
B.	PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION	N/R	✓		Art. 3.3 B
C.	SIZE AND LOCATION OF STRUCTURAL MEMBERS	N/R	✓		Art. 3.3 F
D.	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAME, OR OTHER CONSTRUCTION	N/R	ü	Sec. 1.2.1(E), 6.2.1, & 6.3.1	
E.	WELDING OF REINFORCEMENT	✓	N/R	Sec.6.1.6.1.2	
F.	PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURES ABOVE 90°F)	N/R	✓		Art. 1.8 C & 1.8 D
G.	APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	✓	N/R		Art. 3.6 B
H.	PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	✓	N/R		Art. 3.5 & 3.6 C
I.	PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	REQUIRED FOR FIRST 5000 SF	REQUIRED AFTER FIRST 5000 SF		Art. 3.3 B.9 & 3.3 F.1.b
4.	OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	N/R	✓		Art. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4

NOTE: SPECIAL INSPECTION PER TABLE 14.B NOT REQUIRED FOR QUALITY ASSURANCE LEVEL 1

14.

14.1. SPECIAL INSPECTION AND VERIFICATION OF MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH TMS 402 AND TMS 602 QUALITY ASSURANCE REQUIREMENTS, AS NOTED IN THE TABLES ABOVE INCLUDING:

14.1.1. COMPRESSIVE STRENGTH OF MASONRY SHALL BE CONSIDERED SATISFACTORY IF THE COMPRESSIVE STRENGTH OF EACH MASONRY WYTHE AND GROUTED COLLAR JOINT MEETS OR EXCEEDS THE SPECIFIED f'_m.

14.1.2. COMPRESSIVE STRENGTH OF MASONRY SHALL BE DETERMINED IN ACCORDANCE WITH THE PROVISIONS OF TMS 602 USING THE UNIT STRENGTH METHOD.

14.1.3. FOR RISK CATEGORY I, II, OR III, MINIMUM QUALITY ASSURANCE LEVEL FOR STRUCTURAL MASONRY SHALL BE LEVEL 2 AS NOTED IN TABLES 14A AND 14B.

15.A REQUIRED SPECIAL INSPECTION AND TESTS OF STRUCTURAL STEEL CONSTRUCTION – INSPECTION OF WELDING

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
	AISC TABLE N5.4-1			
1.	PRIOR TO WELDING, VERIFY AND INSPECT THE FOLLOWING:	N/R	✓	
A.	WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	✓	N/R	
B.	WELDING PROCEDURE SPECIFICATIONS (WPS)	✓	N/R	
C.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES	✓	N/R	AISC 360 A3.5
C.	MATERIAL IDENTIFICATION OF STRUCTURAL STEEL MEMBERS	N/R	✓	AISC 360 A3.1
E.	WELDER IDENTIFICATION SYSTEM	N/R	✓	
F.	FIT-UP OF GROOVE WELDS, INCLUDING JOINT GEOMETRY			
	1) JOINT PREPARATION	N/R	✓	
	2) DIMENSIONS: ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL	N/R	✓	
	3) CLEANLINESS: CONDITION OF STEEL SURFACES	N/R	✓	
	4) TACKING: TACK WELD QUALITY AND LOCATION	N/R	✓	
	5) BACKING TYPE AND FIT (IF APPLICABLE)	N/R	✓	
G.	FIT-UP OF CJP GROOVE WELDS OF HSS T-,Y- AND K-JOINTS WITHOUT BACKING, INCLUDING JOINT GEOMETRY.			
	1) JOINT PREPARATION	✓	N/R	
	2) DIMENSIONS: ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL	✓	N/R	
	3) CLEANLINESS: CONDITION OF STEEL SURFACES	✓	N/R	
	4) TACKING: TACK WELD QUALITY AND LOCATION	✓	N/R	
H.	CONFIGURATION AND FINISH OF ACCESS HOLES	N/R	✓	
H.	FIT-UP OF FILLET WELDS			
	1) DIMENSIONS: ALIGNMENT, GAPS AT ROOT	N/R	✓	
	2) CLEANLINESS: CONDITION OF STEEL SURFACES	N/R	✓	
	3) TACKING: TACK WELD QUALITY AND LOCATION	N/R	✓	
I.	CHECK WELDING EQUIPMENT	N/R	✓	
	AISC 360 TABLE N5.4-2			
2.	DURING WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	USE OF QUALIFIED WELDERS	N/R	✓	
B.	CONTROL AND HANDLING OF WELDING CONSUMABLES			
	1) PACKAGING	N/R	✓	
	2) EXPOSURE CONTROL	N/R	✓	
C.	NO WELDING OVER CRACKED TACK WELDS	N/R	✓	
D.	ENVIRONMENTAL CONDITIONS			
	1) WIND SPEED WITHIN LIMITS	N/R	✓	
	2) PRECIPITATION AND TEMPERATURE	N/R	✓	
E.	WELDING PROCEDURE SPECIFICATIONS FOLLOWED			
	1) SETTINGS ON WELDING EQUIPMENT	N/R	✓	
	2) TRAVEL SPEED	N/R	✓	
	3) SELECTED WELDING MATERIALS	N/R	✓	
	4) SHIELDING GAS TYPE AND FLOW RATE	N/R	✓	
	5) PREHEAT APPLIED	N/R	✓	
	6) INTERPASS TEMPERATURE MAINTAINED	N/R	✓	
	7) PROPER POSITION	N/R	✓	
F.	WELDING TECHNIQUES			
	1) INTERPASS AND FINAL CLEANING	N/R	✓	
	2) EACH PASS WITHIN PROFILE LIMITATIONS	N/R	✓	
	3) EACH PASS MEETS QUALITY REQUIREMENTS	N/R	✓	
G.	PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	✓	N/R	
	AISC 360 TABLE N5.4-3			
3.	AFTER WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	WELDS CLEANED	N/R	✓	
B.	SIZE, LENGTH, AND LOCATION OF WELDS	✓	N/R	
C.	WELDS MEET VISUAL ACCEPTANCE CRITERIA			
	1) CRACK PROHIBITION	✓	N/R	
	2) WELD TO BASE METAL FUSION	✓	N/R	
	3) CRATER CROSS SECTION	✓	N/R	
	4) WELD PROFILES	✓	N/R	
	5) WELD SIZE	✓	N/R	
	6) UNDERCUT	✓	N/R	
	7) POROSITY	✓	N/R	
D.	ARC STRIKES	✓	N/R	
E.	k-AREA	✓	N/R	
F.	BACKING REMOVED AND WELD TABS REMOVED, IF REQUIRED	✓	N/R	
G.	REPAIR ACTIVITIES	✓	N/R	
H.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	✓	N/R	
I.	NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE ENGINEER OF RECORD	N/R	✓	

PROVIDE COPY OF ALL SPECIAL INSPECTION REPORTS TO THE BUILDING INSPECTION.

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DECEMBER 2, 2024

ORIGINAL ISSUE: 08/17/16

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PROJECT NUMBER

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QUALITY ASSURANCE NOTES

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15.B REQUIRED SPECIAL INSPECTION AND TESTS OF STRUCTURAL STEEL CONSTRUCTION – INSPECTION OF BOLTING

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
	AISC 360 TABLE N5.6-1			
1.	PRIOR TO BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
A.	MANUFACTURER'S CERTIFICATIONS FOR FASTENER MATERIALS	✓	N/R	
B.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	N/R	✓	
C.	PROPER FASTENER SELECTED FOR JOINT DETAIL	N/R	✓	AISC 360 A3.1
D.	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	N/R	✓	
E.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITIONS AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	N/R	✓	
F.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	✓	N/R	
G.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, AND OTHER FASTENER COMPONENTS	N/R	✓	
	AISC 360 TABLE N5.6-2			
2.	DURING BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
A.	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	N/R	✓	
B.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	N/R	✓	
C.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	N/R	✓	
D.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	N/R	✓	
	AISC 360 TABLE N5.6-3			
3.	AFTER BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
A.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	✓	N/R	

15.C REQUIRED SPECIAL INSPECTION AND TESTS OF COLD FORMED STEEL DECK

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
	SDI QA/QC TABLE 1.1			
1.	PRIOR TO DECK PLACEMENT VERIFY AND INSPECT THE FOLLOWING:			
A.	COMPLIANCE OF MATERIALS (DECK AND ALL ACCESSORIES) WITH CONSTRUCTION DOCUMENTS INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS	✓	N/R	
B.	DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	✓	N/R	
	SDI QA/QC TABLE 1.2			
2.	AFTER DECK PLACEMENT VERIFY AND INSPECT THE FOLLOWING:			
A.	COMPLIANCE OF DECK AND ALL ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	✓	N/R	
B.	DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	✓	N/R	
C.	DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND ACCESSORIES	✓	N/R	
	SDI QA/QC TABLE 1.3			
3.	PRIOR TO WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	WELDING PROCEDURE SPECIFICATION (WPS)	N/R	✓	
B.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES	N/R	✓	
C.	MATERIAL IDENTIFICATION (TYPE/GRADE)	N/R	✓	
D.	CHECK WELDING EQUIPMENT	N/R	✓	
	SDI QA/QC TABLE 1.4			
4.	DURING WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	USE OF QUALIFIED WELDERS	N/R	✓	
B.	CONTROL AND HANDLING OF WELDING CONSUMABLES	N/R	✓	
C.	ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE)	N/R	✓	
D.	WPS FOLLOWED	N/R	✓	
	SDI QA/QC TABLE 1.5			
5.	AFTER WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	SIZE AND LOCATION OF WELDS INCLUDING SUPPORT, SIDE LAP AND PERIMETER	✓	N/R	
B.	WELDS MEET VISUAL ACCEPTANCE CRITERIA	✓	N/R	
C.	VERIFY REPAIR ACTIVITIES	✓	N/R	
D.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	✓	N/R	
	SDI QA/QC TABLE 1.6			
6.	PRIOR TO MECHANICAL FASTENING, VERIFY AND INSPECT THE FOLLOWING:			
A.	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	N/R	✓	
B.	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	N/R	✓	
C.	PROPER STORAGE FOR MECHANICAL FASTENERS	N/R	✓	
	SDI QA/QC TABLE 1.7			
7.	DURING MECHANICAL FASTENING, VERIFY OR INSPECT THE FOLLOWING:			
A.	FASTENERS ARE POSITIONED AS REQUIRED	N/R	✓	
B.	FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	N/R	✓	
	SDI QA/QC TABLE 1.8			
8.	AFTER MECHANICAL FASTENING, VERIFY OR INSPECT THE FOLLOWING:			
A.	CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS	✓	N/R	
B.	CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS	✓	N/R	
C.	CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS	✓	N/R	
D.	VERIFY REPAIR ACTIVITIES	✓	N/R	
E.	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	✓	N/R	

15.

15.1. STRUCTURAL STEEL CONSTRUCTION:

SPECIAL INSPECTION AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS SHALL BE IN ACCORDANCE WITH THE QUALITY CONTROL AND QUALITY ASSURANCE REQUIREMENTS OF AISC 360, AS NOTED IN TABLES 15A, 15B, AND AWS D1.1, INCLUDING:

15.1.1. INSPECTION OF ERECTED STEEL SYSTEM.

15.1.2. REVIEW OF MATERIAL TEST REPORTS AND CERTIFICATIONS FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.

15.1.3. OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROCESS AND COMPLETED WELDS SHALL BE AS FOLLOWS:

A. VERIFY THAT WELD FILLER MATERIAL AND MANUFACTURER'S CERTIFICATE OF COMPLIANCE CONFORM TO AWS SPECIFICATION SPECIFIED. VERIFY WELDERS ARE CERTIFIED BY WABO, THAT PROPER ELECTRODES IN OVEN DRY CONDITIONS ARE USED, AND THAT PROPER METHODS AND PREPARATIONS ARE USED.

B. PERIODIC SPECIAL INSPECTION OF WELDING SHALL BE PERFORMED FOR SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16" AND FLOOR AND DECK WELDS.

C. CONTINUOUS SPECIAL INSPECTION OF WELDING SHALL BE PERFORMED ON COMPLETE AND PARTIAL PENETRATION GROOVE WELDS AND FILLET WELDS GREATER THAN 5/16".

D. ALL WELDS SHALL BE CHECKED VISUALLY.

E. ALL SHOP AND FIELD WELDING SHALL BE SUBJECT TO INSPECTION BY A WABO CERTIFIED WELDING INSPECTOR EMPLOYED BY THE OWNER. THE INSPECTOR SHALL UTILIZE RADIOGRAPHIC, ULTRASONIC, OR MAGNETIC PARTICLE TESTING AND ANY OTHER AID TO VISUAL INSPECTION THAT MAY BE DEEMED NECESSARY TO ASSURE THE ADEQUACY OF WELDING. THE OWNER SHALL CARRY OUT TESTING AND INTERPRETATION AT ANY STAGE AFTER WELDING.

F. 10% OF ALL FILLET WELDS SHALL BE CHECKED BY MAGNETIC PARTICLE TESTING.

G. 100% OF ALL COMPLETE PENETRATION WELDS SHALL BE CHECKED BY ULTRASONIC TESTING.

H. ALL WELDS FOUND DEFECTIVE AND REPAIRED SHALL BE REINSPECTED BY THE SAME METHOD ORIGINALLY USED. THE COST OF REPAIR AND REINSPECTION SHALL BE BORNE BY THE CONTRACTOR.

I. STANDARDS FOR ACCEPTANCE SHALL BE AS GIVEN IN AWS D1.1.

15.1.4. OBSERVATION OF BOLTING OPERATIONS.

15.1.5. WHERE CONTINUOUS SPECIAL INSPECTION IS NOTED, IT SHALL BE PERFORMED FOR EACH JOINT OR MEMBER. WHERE PERIODIC SPECIAL INSPECTION IS NOTED, IT SHALL BE PERFORMED ON ITEMS ON A RANDOM BASIS. PERIODIC SPECIAL INSPECTION NEED NOT DELAY FABRICATION OR ERECTION OPERATIONS.

15.1.6. COLD FORMED STEEL DECK:

SPECIAL INSPECTIONS AND QUALIFICATION FOR WELDING SPECIAL INSPECTORS SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF SDI QA/QC, AS NOTED IN TABLE 15C.

15.1.7. EPOXY ANCHORS: SPECIFIC REQUIREMENTS FOR INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE OR MASONRY SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IAPMO, ETC.).

15.1.8. EXPANSION ANCHORS: SPECIFIC REQUIREMENTS FOR INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE OR MASONRY SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IAPMO, ETC.).

18. REQUIRED SPECIAL INSPECTION AND TESTS FOR SEISMIC RESISTANCE

		SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1.		STRUCTURAL WOOD IN SEISMIC DESIGN CATEGORY C, D, E, OR F:		
	B.	NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE MAIN SEISMIC FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLDOWNS.	N/R	✓
2.		COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION IN SEISMIC DESIGN CATEGORY C, D, E OR F:		
	A.	WELDING OF ELEMENTS ON THE SEISMIC FORCE-RESISTING SYSTEM.	N/R	✓
	B.	SCREW ATTACHMENT, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC FORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, BRACES, DIAPHRAGMS, COLLECTORS, AND HOLDOWNS.	N/R	✓
3.		ARCHITECTURAL COMPONENTS IN SEISMIC DESIGN CATEGORY D, E OR F:		
	A.	THE ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NON-BEARING WALLS, AND INTERIOR AND EXTERIOR VENEER	N/R	✓

18.

18.1. SPECIAL INSPECTIONS AND TESTING FOR SEISMIC RESISTANCE:

18.1.1. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE PER IBC 1705.12 SHALL BE REQUIRED FOR SEISMIC FORCE-RESISTING SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E OR F PER TABLE 18 AND THE FOLLOWING:

A. SPECIAL INSPECTIONS OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.

18.1.2. TESTING AND QUALIFICATION FOR SEISMIC RESISTANCE PER IBC 1705.13 SHALL BE REQUIRED FOR SEISMIC FORCE-RESISTING SYSTEM IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F FOR THE FOLLOWING:

A. NONDESTRUCTIVE TESTING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.

18.1.3. SPECIAL INSPECTION IS NOT REQUIRED FOR THE FOLLOWING:

A. STRUCTURAL WOOD WHERE THE FASTENER SPACING OF THE SHEATHING IS GREATER THAN 4 INCHES ON CENTER.

B. COLD-FORMED STEEL LIGHT-FRAMED CONSTRUCTION WHERE THE SHEATHING IS STEEL SHEETS ON ONLY ONE SIDE OF THE SHEAR WALL, SHEAR PANEL OR DIAPHRAGM ASSEMBLY AND THE FASTENER SPACING OF THE SHEATHING IS MORE THAN 4 INCHES ON CENTER.

C. SPECIAL INSPECTION IS NOT REQUIRED FOR ARCHITECTURAL COMPONENTS WHERE:

a. EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS AND INTERIOR AND EXTERIOR VENEER ARE 30 FEET OR LESS IN HEIGHT ABOVE GRADE OR WALING SURFACE.

b. EXTERIOR CLADDING AND INTERIOR AND EXTERIOR VENEERS WEIGHTING 5 PSF OR LESS.

c. INTERIOR NONBEARING WALLS WEIGHING 15 PSF OR LESS.

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QUALITY ASSURANCE NOTES

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FOOTING SCHEDULE

MARK	SIZE	REINFORCING	REMARKS
F3.0	3'-0" x 3'-0" x 1'-0"	(3) #5 EACH WAY AT BOTTOM OF FOOTING	
F4.0	4'-0" x 4'-0" x 1'-0"	(4) #5 EACH WAY AT BOTTOM OF FOOTING	
F6.0	6'-0" x 6'-0" x 2'-6"	(7) #7 EACH WAY AT TOP AND BOTTOM OF FOOTING	

FOOTINGS SCHEDULE NOTES:

1. TOP OF FOOTING ELEVATION = -1'-0" UNLESS NOTED OTHERWISE ON PLAN.
2. FOOTING DESIGN BASED ON 3000 PSF ALLOWABLE SOIL BEARING PRESSURE.
3. EQUALLY SPACE REINFORCING IN EACH DIRECTION.
4. PROVIDE 3" CLEAR TO REINFORCING AT BOTTOM OF FOOTING.

1 SCHEDULE

1" = 1'-0"

F'c = 3000 PSI

BAR SIZE	Ld	Lt	Lsb	Lsbt
#3	17	23	23	30
#4	22	29	29	38
#5	28	37	37	49
#6	33	43	43	56
#7	48	63	63	82
#8	55	72	72	94
#9	62	81	81	106
#10	70	91	91	119
#11	78	102	102	133
#14	93	121	-	-
#18	124	162	-	-

F'c = 4000 PSI

BAR SIZE	Ld	Lt	Lsb	Lsbt
#3	15	20	20	26
#4	19	25	25	33
#5	24	32	32	42
#6	29	38	38	50
#7	42	55	55	72
#8	48	63	63	82
#9	54	71	71	93
#10	61	80	80	104
#11	67	88	88	115
#14	81	106	-	-
#18	108	141	-	-

F'c = 5000 PSI

BAR SIZE	Ld	Lt	Lsb	Lsbt
#3	13	17	17	23
#4	17	23	23	30
#5	22	29	29	38
#6	26	34	34	45
#7	38	50	50	65
#8	43	56	56	73
#9	48	63	63	82
#10	54	71	71	93
#11	60	78	78	102
#14	72	94	-	-
#18	96	125	-	-

ALL CONCRETE STRENGTHS

BAR SIZE	Lb	Lc	Lcs	-
#3	9	12	12	
#4	11	15	12	
#5	14	19	15	
#6	17	23	17	
#7	20	27	20	
#8	22	30	23	
#9	25	34	26	
#10	28	39	29	
#11	31	43	32	
#14	38	-	-	
#18	50	-	-	

NOTES:

1. REINFORCEMENT DEVELOPMENT AND SPLICE LENGTHS ARE IN ACCORDANCE WITH ACI 318.
2. NOTATIONS:
   - db: NOMINAL BAR DIAMETER (IN)
   - Ld: TENSION DEVELOPMENT LENGTH (IN) FOR REINFORCEMENT SATISFYING THE FOLLOWING REQUIREMENTS: SLABS AND WALLS: CLEAR SPACING GREATER THAN 2db, AND CONCRETE CLEAR COVER GREATER THAN db BEAMS AND COLUMNS: CLEAR SPACING GREATER THAN db, AND CONCRETE CLEAR COVER GREATER THAN db
   - Lt: DEVELOPMENT LENGTH OF TOP BARS IN TENSION = 1.3 X Ld (IN)
   - Lb: DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = 22 X db (IN)
   - Lc: TIED COLUMN LAP SPLICE IN COMPRESSION = 30 X db (IN)
   - Lcs: SPIRAL COLUMN LAP SPLICE IN COMPRESSION = 22.5 X db (IN)
   - Lsb: TENSION LAP SPLICE LENGTH FOR OTHER THAN TOP BARS = 1.3 X Ld (IN)
   - Lsbt: TENSION LAP SPLICE LENGTH OF TOP BARS = 1.69 X Ld (IN)
3. MULTIPLY VALUES IN THE TABLE BY 1.5 IF CLEAR SPACING OR CONCRETE COVER DO NOT MEET THE REQUIREMENTS FOR Ld IN NOTE 2.
4. TOP BARS: HORIZONTAL BEAM REINFORCING WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW.
5. THE DEVELOPMENT AND SPLICE LENGTHS ARE BASED ON REINFORCEMENT STRENGTH Fy = 60 KSI. 6. #14 AND #18 BARS SHALL NOT BE LAP SPLICED. SEE GENERAL NOTES.

REINFORCING BAR DEVELOPMENT AND SPLICE LENGTH TABLES

2 SCHEDULE

1" = 1'-0"

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PERMIT SET
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2220236.20
PROJECT NUMBER

KJK
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PHASE 2 - CARE CENTER

CONCRETE SCHEDULES

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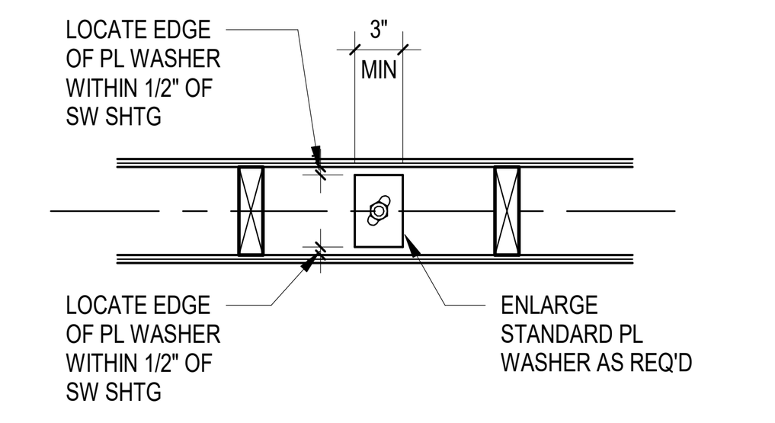
WOOD STUD SHEARWALL SCHEDULE

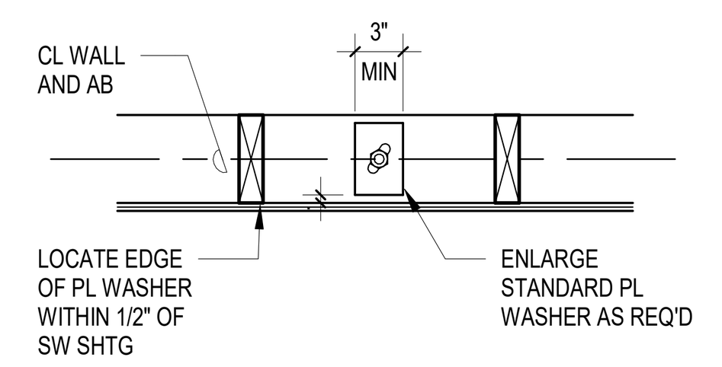
MARK	SHEATHING	NAILING		STUD SIZE AT ADJOINING PANEL EDGES	BLOCKING SIZE	FOUNDATION SILL PL ATTACHMENT	2x BOTTOM PLATE ATTACHMENT TO WOOD BELOW	CLIP SPACING RIM/BLKG TO TOP PLATE	ASD ALLOWABLE UNIT SHEAR - SEISMIC	ASD ALLOWABLE UNIT SHEAR - WIND
		SIZE	SPACING							
W 6	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	6" OC EDGES 12" OC FIELD	2x	2x FLAT OR 2x	3/4" DIA. AT 48" OC	16d AT 6" OC STAGGERED	SIMP A35 OR LTP4 AT 24"OC	310 PLF	435 PLF
W 4	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	4" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 48" OC	(2) ROWS 16d AT 8" O.C. STAGGERED	SIMP A35 OR LTP4 AT 16"OC	460 PLF	645 PLF
W 3	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	3" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 32" OC	(2) ROWS 16d AT 6" OC STAGGERED	SIMP A35 OR LTP4 AT 12"OC	600 PLF	840 PLF
W 2	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4 MIN)	2" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 16" OC	(3) ROWS 16d AT 8" OC STAGGERED	SIMP A35 OR LTP4 AT 10"OC	770 PLF	1078 PLF
2W 6	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	6" OC EDGES 12" OC FIELD	2x	2x FLAT OR 2x	3/4" DIA. AT 32" OC	(2) ROWS 16d AT 6" OC STAGGERED	SIMP A35 OR LTP4 AT 24"OC EA FACE	620 PLF	870 PLF
2W 4	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	4" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 16" OC	(2) ROWS 16d AT 4" OC STAGGERED	SIMP A35 OR LTP4 AT 16"OC EA FACE	920 PLF	1290 PLF
2W 3	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	3" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 16" OC	SEE DETAIL 2/S0.7	SIMP A35 OR LTP4 AT 12"OC EA FACE	1200 PLF	1680 PLF
2W 2	15/32" APA RATED SHEATHING TWO SIDES OF WALL	10d COMMON (0.148" DIA x 2 1/4 MIN)	2" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA. AT 8" OC	SEE DETAIL 2/S0.7	SIMP A35 OR LTP4 AT 10"OC EA FACE	1540 PLF	2155 PLF

APA RATED SHEATHING SHEARWALL NOTES

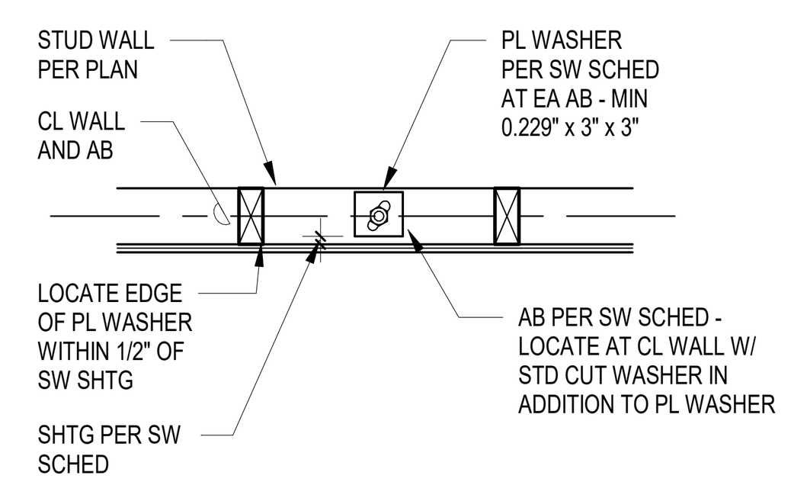
1. NAILS SHALL BE COMMON FROM AN AMERICAN OR CANADIAN MFR ONLY. MINIMUM NAIL PENETRATION INTO WOOD FRAMING SHALL BE 1 1/2" FOR 10d NAILS. UNLESS NOTED OTHERWISE, NAIL DIAMETERS AND LENGTHS SHALL BE PER NOTE 6.2.3 OF THE STRUCTURAL NOTES. GALVANIZED NAILS SHALL BE HOT DIPPED OR TUMBLED.
2. APA RATED SHEATHING MATERIAL MAY BE EITHER PLYWOOD OR ORIENTED STRAND BOARD CONFORMING TO DOC PS 1 OR PS 2. SHEATHING MAY BE ORIENTED EITHER HORIZONTALLY OR VERTICALLY.
3. SHEATHING PANELS SHALL NOT BE LESS THAN 4' x 8' EXCEPT AT SHEARWALL BOUNDARIES AND CHANGES IN FRAMING. ALL EDGES OF ALL PANELS SHALL BE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING.
4. ALL INTERIOR SHEAR WALLS HAVE BEEN DESIGNATED. ALL EXTERIOR WALLS WITHOUT DESIGNATION SHALL BE TYPE W6. WHERE THE SHEARWALL HAS BEEN DESIGNATED ON THE PLANS TO EXTEND ALONG LENGTHS OF WALLS WITH PENETRATIONS, SHEATHING AND NAILING OF THAT TYPE SHALL BE REQUIRED ABOVE AND BELOW WALL OPENINGS. OTHERWISE, SHEATHING AND NAILING ABOVE AND BELOW OPENINGS MAY BE TYPE W6.
5. UNLESS NOTED OTHERWISE, THE SHEARWALL DESIGNATION APPLIES TO FULL EXTENT OF WALL BETWEEN CORNERS OF WALLS.
6. SHEARWALLS SHALL RUN CONTINUOUS THROUGH BREAKS CAUSED BY INTERSECTING WALLS.
7. WHEN SHEATHING IS REQUIRED ON ONE SIDE ONLY, PLACE ON THE SIDE OF THE SYMBOL. WHERE THE SHEATHING IS NOTED ON TWO SIDES OF THE WALL, STAGGER VERTICAL PANEL JOINTS SUCH THAT JOINTS ON OPPOSITE SIDES OF THE WALL DO NOT FALL ON THE SAME FRAMING MEMBER.
8. NAIL SPACING INDICATED ON SCHEDULE APPLIES TO ALL STUDS, TOP AND BOTTOM PLATES AND BLOCKING. NAIL SPACINGS OF 3" ON CENTER OR LESS AT ADJOINING PANEL EDGES SHALL BE STAGGERED. NAILS SHALL BE LOCATED AT LEAST 3/8" FROM PANEL EDGES.
9. PROVIDE SHEATHING EDGE NAILING TO ALL COLUMNS WITH HOLDOWNS AND STUDS ATTACHED TO STEEL TUBE COLUMNS.
10. HOT DIPPED GALVANIZED FASTENERS SHALL BE USED TO ATTACH TO ALL TREATED WOOD MEMBERS. ELECTROPLATED FASTENERS ARE NOT ACCEPTABLE.
11. SPACING OF WALL STUDS SHALL BE AS NOTED ON THE PLANS. SPACING OF STUDS SHALL NOT EXCEED 24" O.C.
12. WHERE NOTED, THE WIDTH OF THE NAILED FACE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES SHALL BE 3" NOMINAL. TWO 2" NOMINAL FRAMING MEMBERS SHALL BE PERMITTED TO BE USED IN LIEU OF A SINGLE 3" NOMINAL MEMBER PROVIDED THE 2" NOMINAL MEMBERS ARE LAMINATED TOGETHER WITH NAILS OR BOLTS AS NOTED IN "2x BOTTOM PLATE ATTACHMENT TO WOOD BELOW" COLUMN IN SCHEDULE ABOVE.
13. ANCHOR BOLTS SHALL NOT BE SPACED GREATER THAN 48" OC, AND SHALL HAVE 7" MIN. EMBED. EXPANSION BOLTS SHALL HAVE 5" MIN EMBED. SEE DETAILS FOR TYPE OF CONNECTION REQUIRED. PROVIDE A MINIMUM OF (2) ANCHOR BOLTS PER PIECE, WITH ONE ANCHOR LOCATED NOT MORE THAN 12" OR LESS THAN 4" FROM EACH END OF EACH PIECE. AT NON-SHEAR WALLS, PROVIDE SPECIFIED ANCHOR BOLTS AT 48" OC MAX, UNLESS NOTED OTHERWISE.
14. FOUNDATION ANCHOR BOLTS SHALL HAVE A STEEL PLATE WASHER AT EA ANCHOR BOLT NO LESS THAN 0.229" x 3" x 3" IN SIZE. THE HOLE IN THE PLATE WASHER SHALL BE PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1 - 3/4", PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. THE PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE FOUNDATION SILL PLATE. SLOTTED PLATE WASHERS SHALL BE A MINIMUM 4" x 4" FOR 2x6 WALLS, AND 6" x 6" FOR 2x8 WALLS.
15. STANDARD CUT WASHERS MAY BE SUBSTITUTED IN LIEU OF PLATE WASHERS FOR ALL TYPE W6 WALLS LONGER THAN 10 FEET.
16. SIMPSON A35 CLIPS MAY BE OMITTED PER ALTERNATE CONNECTION SCHEDULE 2/S0.7.



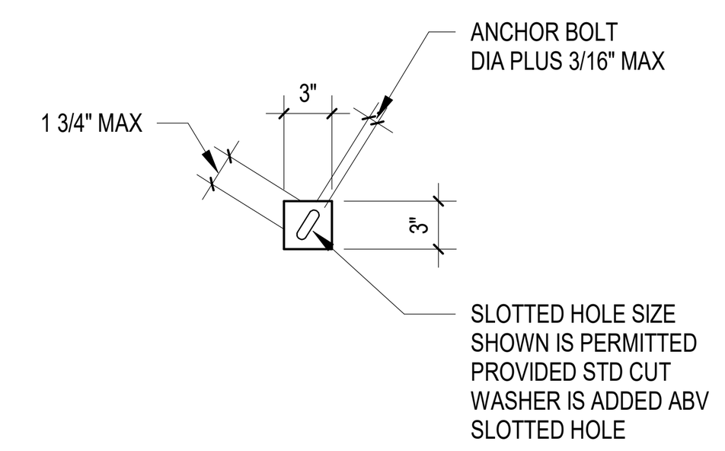
CONDITION AT WALLS SHEATHED BOTH SIDES



CONDITION AT 2x6 AND LARGER WALLS



TYPICAL CONDITION



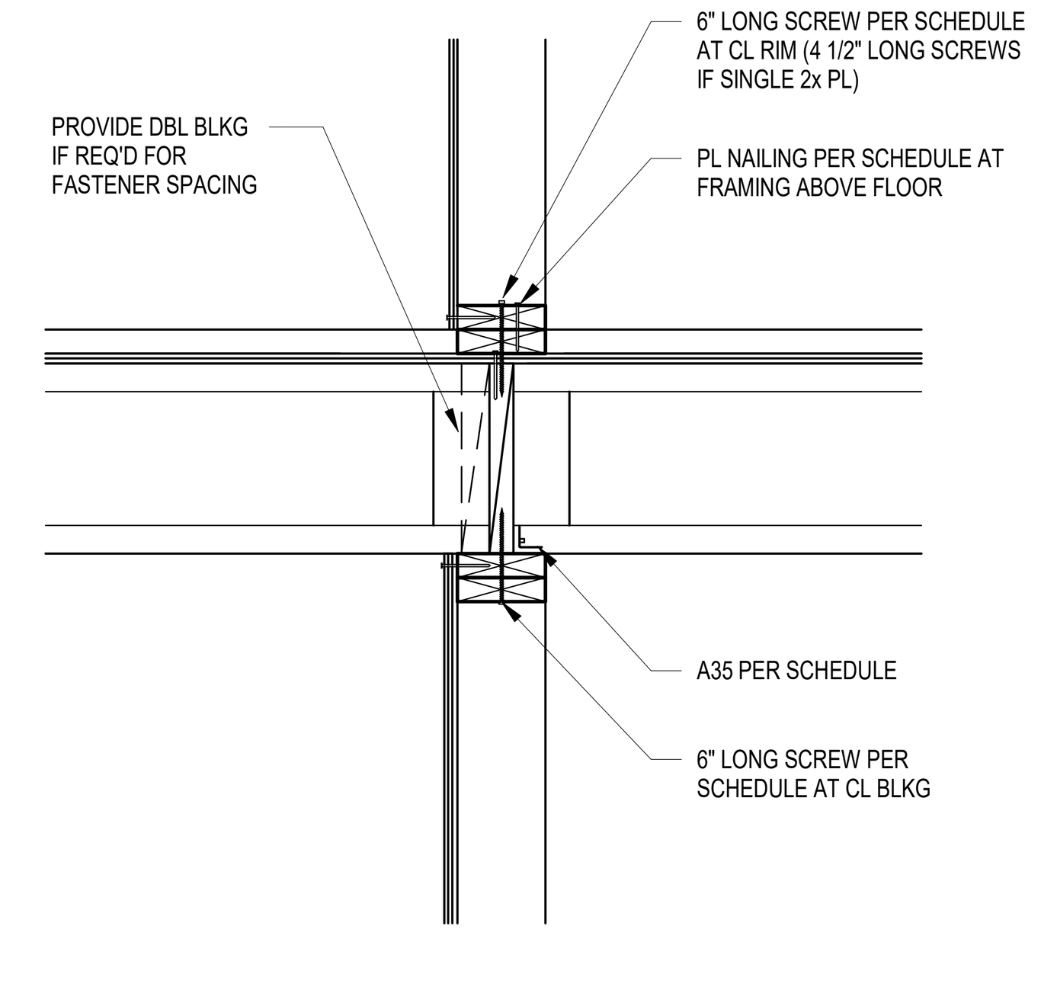
STANDARD (MINIMUM) PL WASHER

1 SCHEDULE

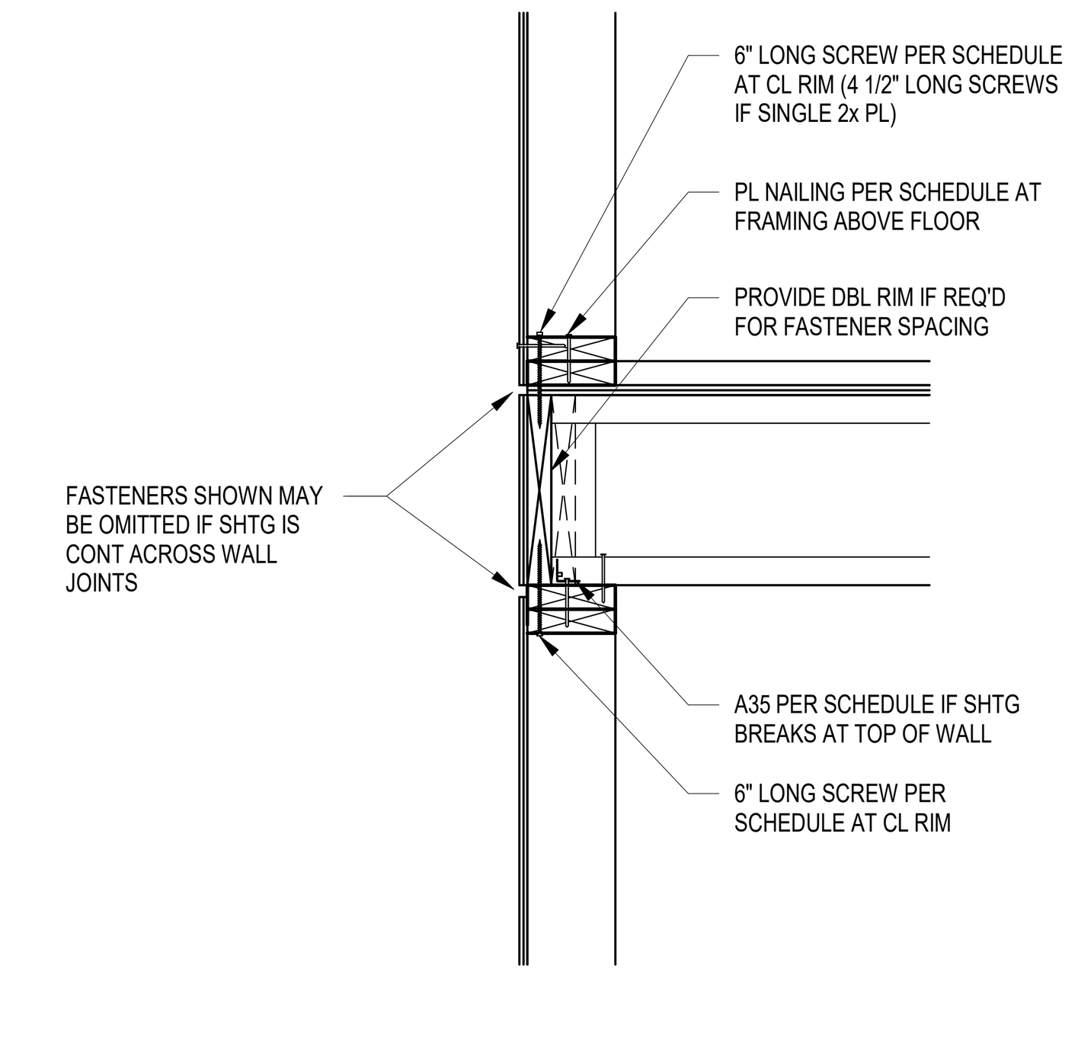
1" = 1'-0"

SHEARWALL CONNECTION SCHEDULE

MARK	SIMPSON A35 OPTION	SIMPSON SDWC OPTION	SIMPSON SDWH OPTION
W 6	A35 AT 24"OC	0.152 DIA AT 5" OC	0.276 DIA AT 12" OC
W 4	A35 AT 16"OC	0.152 DIA AT 3" OC	0.276 DIA AT 10" OC
W 3	A35 AT 12"OC	0.152 DIA AT 2 1/2" OC	0.276 DIA AT 8" OC
W 2	A35 AT 10"OC	0.152 DIA AT 2" OC	0.276 DIA AT 6" OC
2W 6	A35 AT 12"OC	0.152 DIA AT 2 1/2" OC	0.276 DIA AT 6" OC
2W 4	A35 AT 9"OC	0.152 DIA AT 1 1/2" OC	0.276 DIA AT 5" OC
2W 3	A35 AT 6"OC	N/A	0.276 DIA AT 4" OC
2W 2	N/A	N/A	0.276 DIA AT 3" OC



INTERIOR SHEARWALL CONDITION



EXTERIOR SHEARWALL CONDITION

2 SCHEDULE

1" = 1'-0"

TYPICAL WOOD STUD WALL SCHEDULE

		LEVEL 1
CARE CENTER	EXTERIOR WALL	2 x 6 DF #2 AT 16" OC
	CORRIDOR WALL	2 x 6 DF #2 AT 16" OC
	INTERIOR BEARING WALL	2 x 6 DF #2 AT 12" OC
	INTERIOR SHEAR WALL	2 x 6 DF #2 AT 16" OC

WOOD WALL SCHEDULE NOTES:

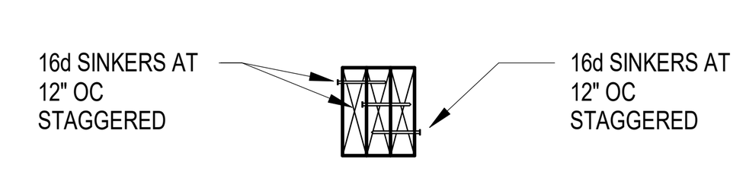
1. SEE PLANS FOR WALL TYPE AND LOCATIONS.
2. SEE DETAIL 9 / S0.8-B FOR TYPICAL WOOD BEARING WALL ELEVATION.

3 SCHEDULE

1" = 1'-0"

TYPICAL BUILT-UP STUD COL SCHEDULE

	LOCATION	OPENING SIZE	LEVEL 1
CARE CENTER	EXTERIOR	4'-0" OR LESS	(2) FULL HT (1) BRG
		6'-0" OR LESS	(2) FULL HT (2) BRG
		8'-0" OR LESS	(3) FULL HT (2) BRG
		14'-0" OR LESS	(4) FULL HT (2) BRG
	INTERIOR	4'-0" OR LESS	(1) FULL HT (1) BRG
		6'-0" OR LESS	(1) FULL HT (2) BRG
		8'-0" OR LESS	(2) FULL HT (2) BRG
		14'-0" OR LESS	(2) FULL HT (2) BRG



CONDITION AT BU COL

TYPICAL BUILT-UP COLUMN CONSTRUCTION

4 SCHEDULE

1" = 1'-0"

TYPICAL WOOD HEADER SCHEDULE

	LOCATION	OPENING SIZE	ROOF
CARE CENTER	EXTERIOR	4'-0" OR LESS	(2) 2 x 8 DF #2
		6'-0" OR LESS	(2) 2 x 12 DF #2
		8'-0" OR LESS	(3) 2 x 12 DF #2
		14'-0" OR LESS	GL 5 1/2 x 12
	INTERIOR	4'-0" OR LESS	(2) 2 x 8 DF #2
		6'-0" OR LESS	(2) 2 x 12 DF #2
		8'-0" OR LESS	(3) 2 x 12 DF #2
		14'-0" OR LESS	GL 5 1/2 x 12

5 SCHEDULE

1" = 1'-0"

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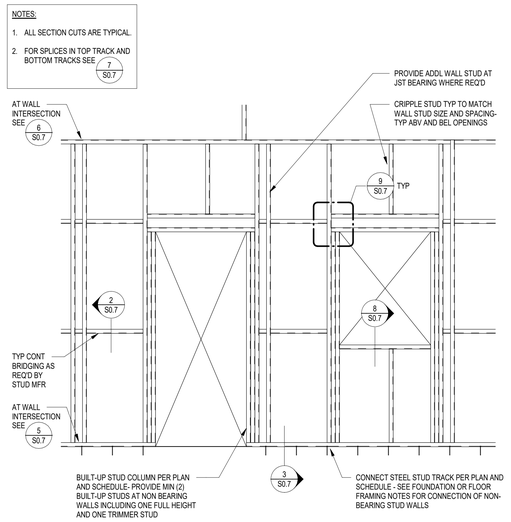
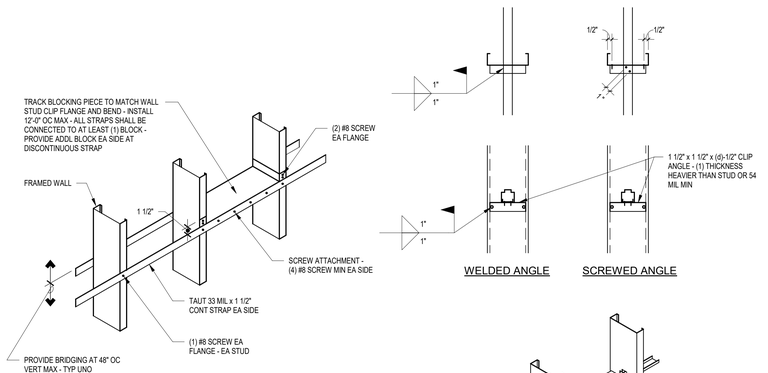
WOOD SCHEDULES

S0.6

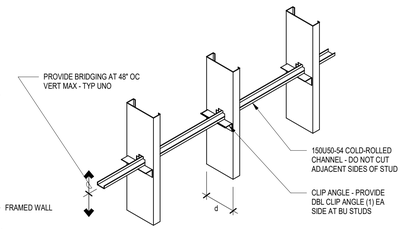


1 TYPICAL

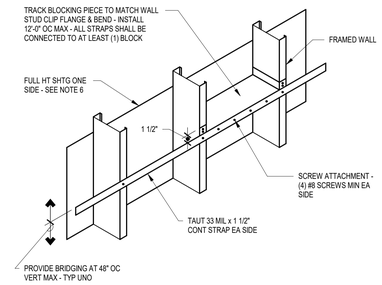
N.T.S. 1 / S0.9



TYPE 1 BRIDGING - DOUBLE FLAT STRAP WITH BLOCKING



TYPE 2 BRIDGING - COLD-ROLLED CHANNEL WITH CLIP ANGLE



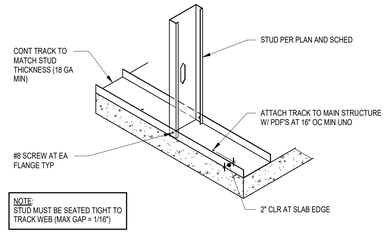
TYPE 3 BRIDGING - SHEATHING WITH SINGLE FLAT STRAP WITH BLOCKING

NOTES:

1. PROVIDE BRIDGING ON ALL LOAD BEARING AND CURTAIN WALL STUDS.
2. USE ONLY TYPE 1 AND TYPE 2 BRIDGING FOR LOAD BEARING STUD WALLS (SUPPORTING ROOFS, FLOORS, MEZZANINE, ETC.)
3. INSTALL ALL BRIDGING PRIOR TO INSTALLATION OF SUPPORTED FRAMING.
4. NO BRIDGING REQUIRED AT WALLS WITH FULL HEIGHT SHEATHING EACH SIDE.
5. TYPE 2 BRIDGING NOT ALLOWED ON STUDS GREATERS THAN 6" DEEP.
6. QUALIFIED SHEATHING SHALL BE GWB, GYPSHEATHING, PW, OR OSB ONLY.
7. TYPICAL MINIMUM EDGE DISTANCE = 3x SCREW DIAMETER.

2 TYPICAL

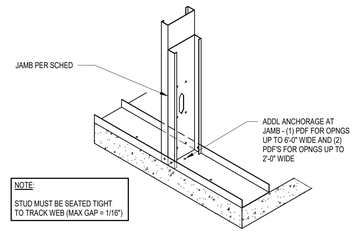
N.T.S. 2 / S0.9



STUD WALL BOTTOM TRACK ATTACHMENT

3 TYPICAL

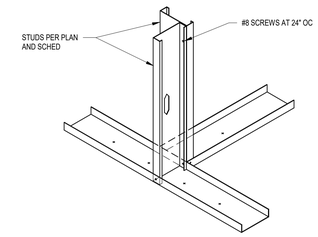
N.T.S. 3 / S0.9



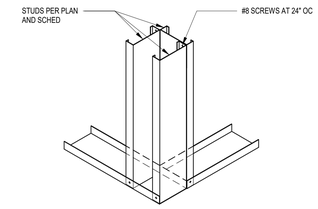
JAMB STUD ATTACHMENT

4 TYPICAL

N.T.S. 4 / S0.9



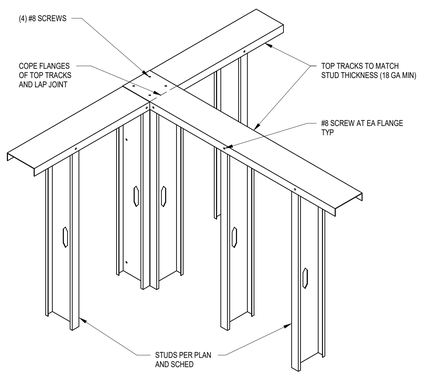
WALL INTERSECTION - BOTTOM TRACK



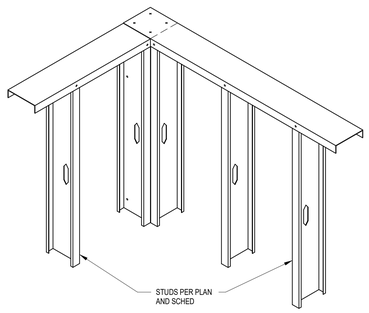
WALL CORNER - BOTTOM TRACK

5 TYPICAL

N.T.S. 5 / S0.9



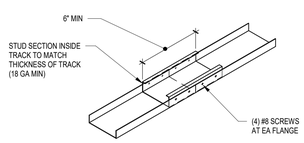
WALL INTERSECTION - TOP TRACK



WALL CORNER - TOP TRACK

6 TYPICAL

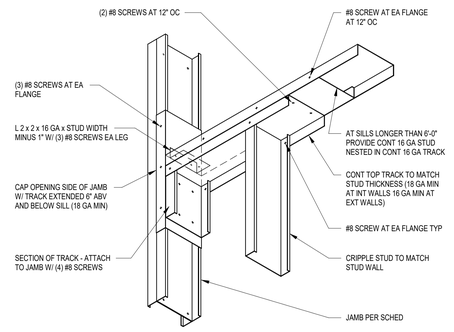
N.T.S. ELEVATION



TYPICAL BOTTOM / TOP TRACK SPLICE

7 TYPICAL

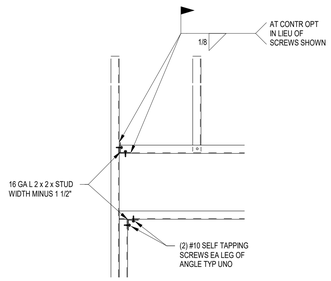
N.T.S. 7 / S0.9



TYPICAL WINDOW SILL

8 TYPICAL

N.T.S. 8 / S0.9



TYPICAL BUILT-UP BEARING HEADER

9 ELEVATION

N.T.S.

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PHASE 2 - CARE CENTER

STEEL STUD DETAILS

S0.7

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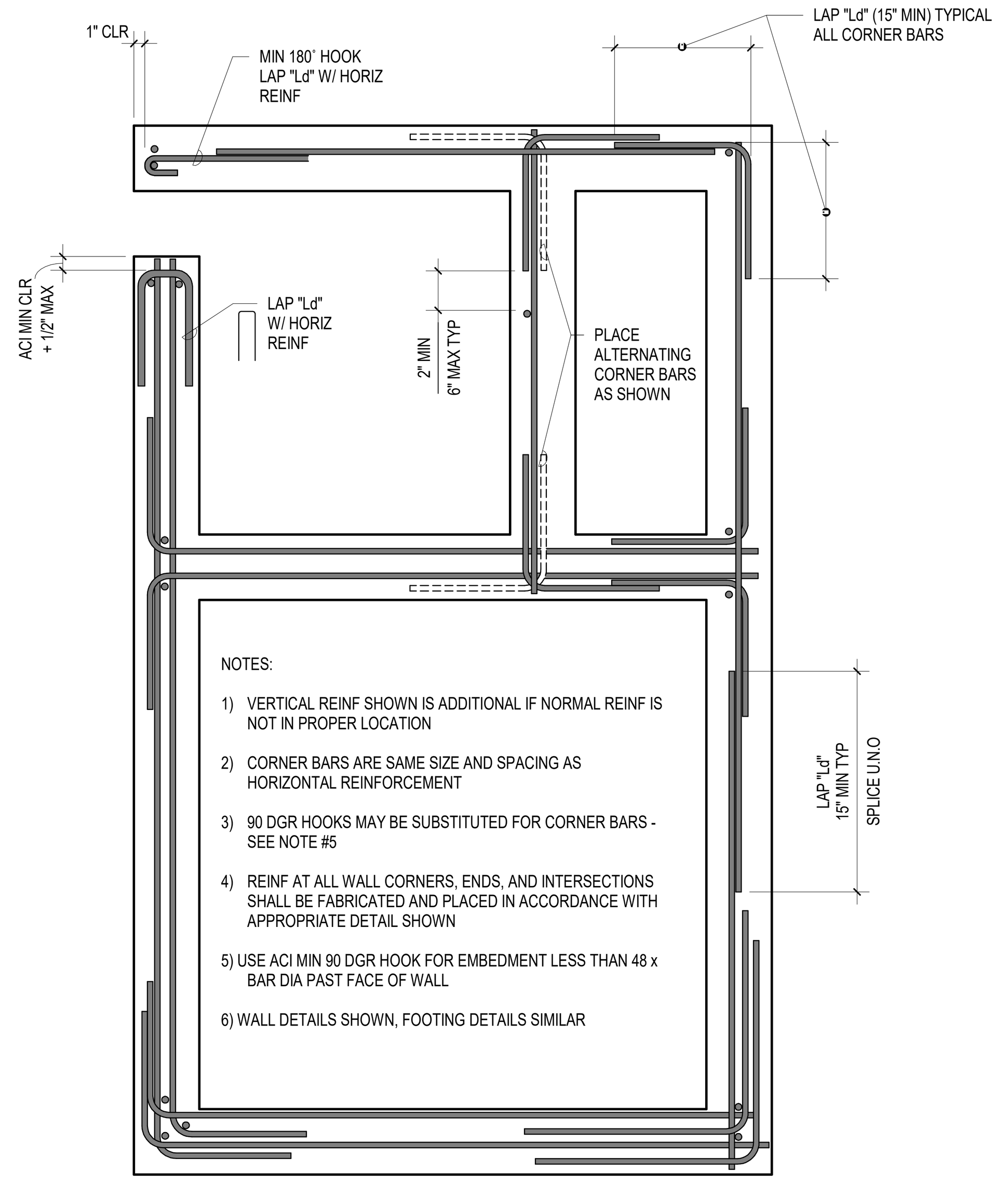
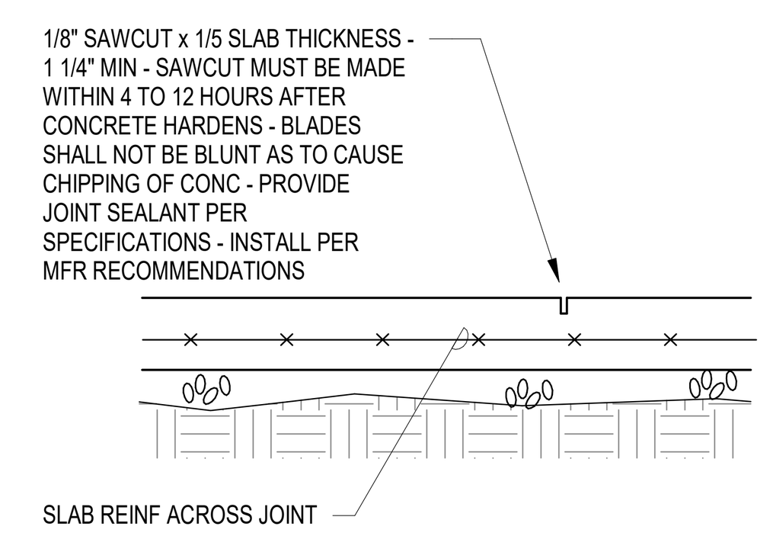


NOTES:

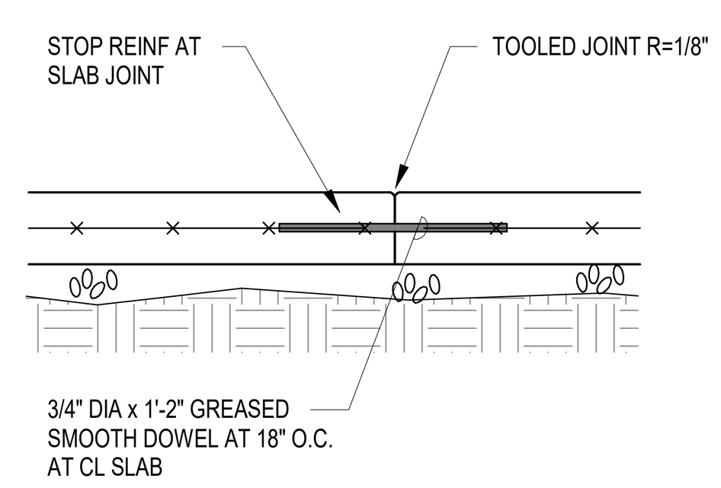
1) VERTICAL REINF SHOWN IS ADDITIONAL IF NORMAL REINF IS NOT IN PROPER LOCATION
2) CORNER BARS ARE SAME SIZE AND SPACING AS HORIZONTAL REINFORCEMENT
3) 90 DGR HOOKS MAY BE SUBSTITUTED FOR CORNER BARS - SEE NOTE #5
4) REINF AT ALL WALL CORNERS, ENDS, AND INTERSECTIONS SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH APPROPRIATE DETAIL SHOWN
5) USE ACI MIN 90 DGR HOOK FOR EMBEDMENT LESS THAN 48 x BAR DIA PAST FACE OF WALL
6) WALL DETAILS SHOWN, FOOTING DETAILS SIMILAR

TYPICAL REINFORCEMENT PLACING DETAIL

1 TYPICAL
1" = 1'-0" 1 / S0.8-B

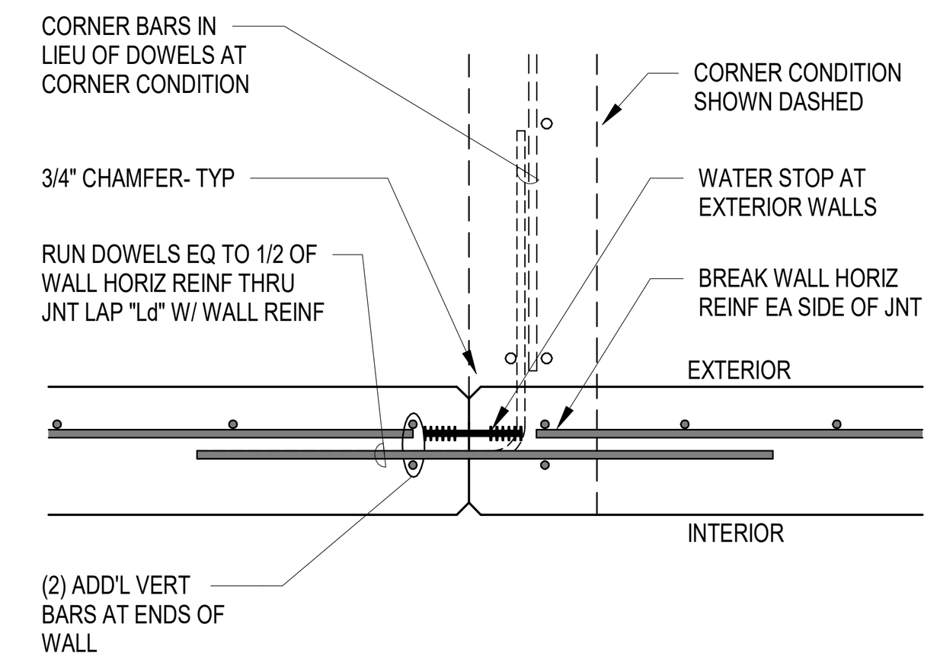


TYPICAL CONTROL JOINT

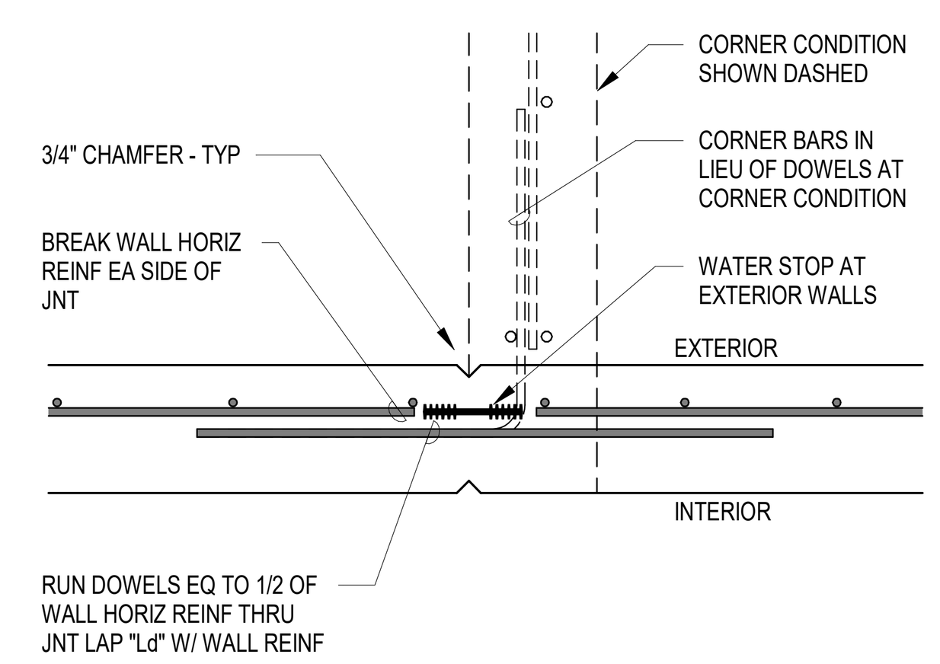


TYPICAL CONSTRUCTION JOINT

2 TYPICAL
1" = 1'-0" 2 / S0.8-B

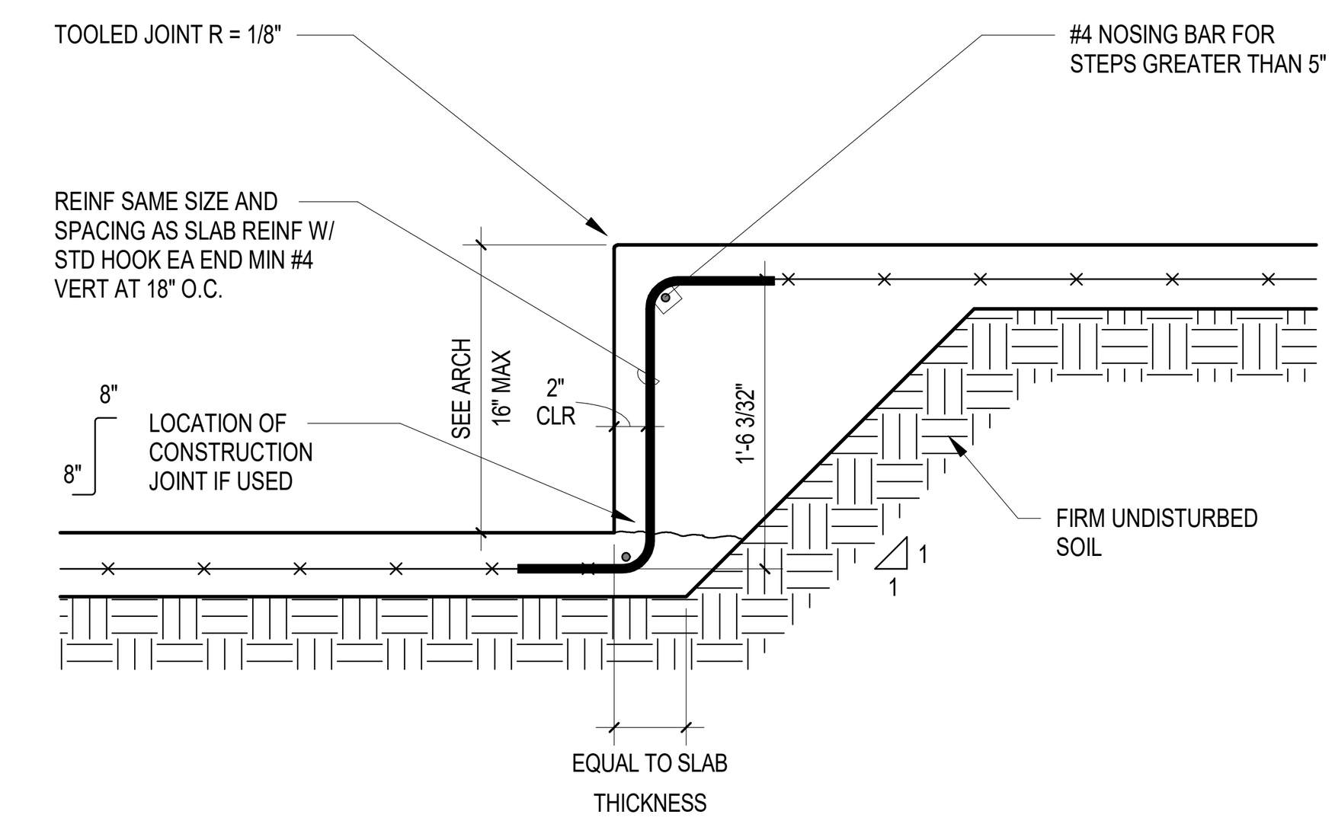


TYPICAL WALL CONSTRUCTION JOINT



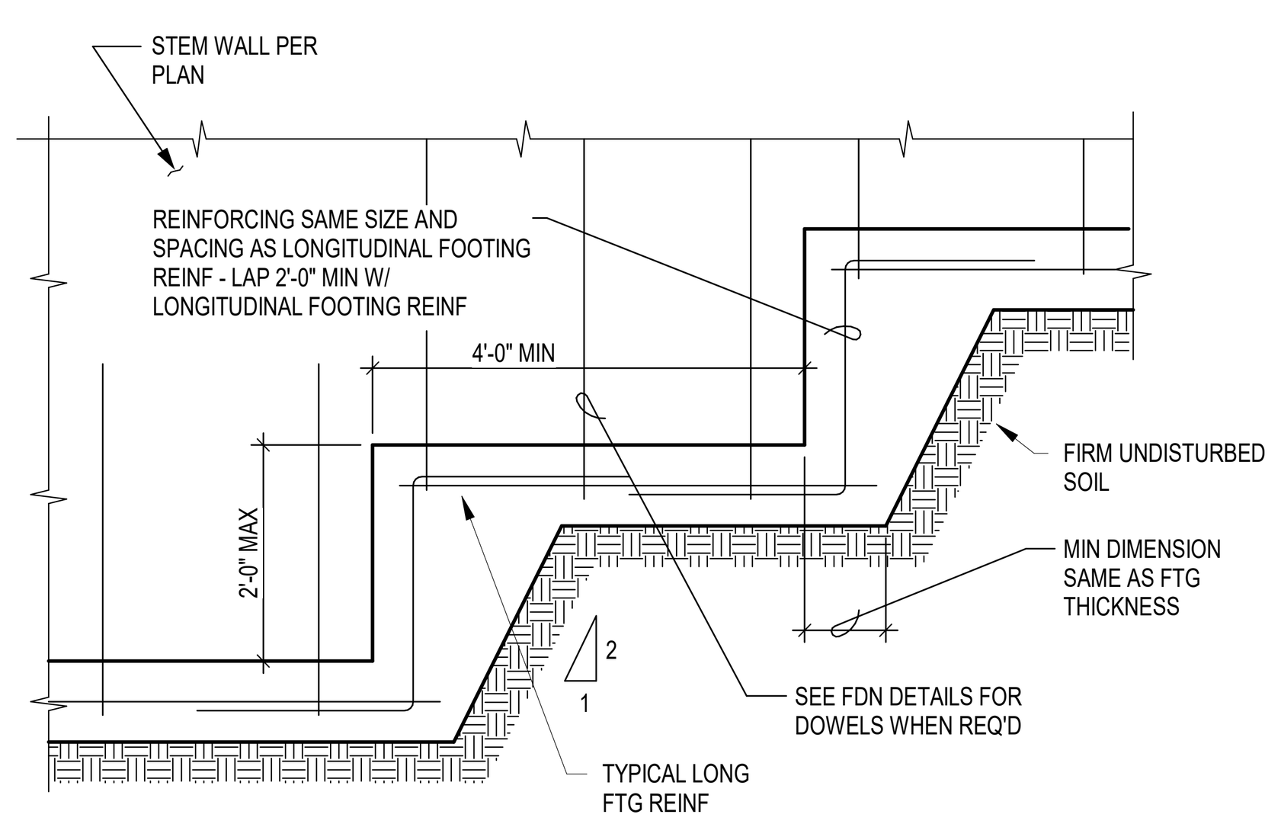
TYPICAL WALL CONTROL JOINT

3 TYPICAL
1" = 1'-0" 3 / S0.8-B



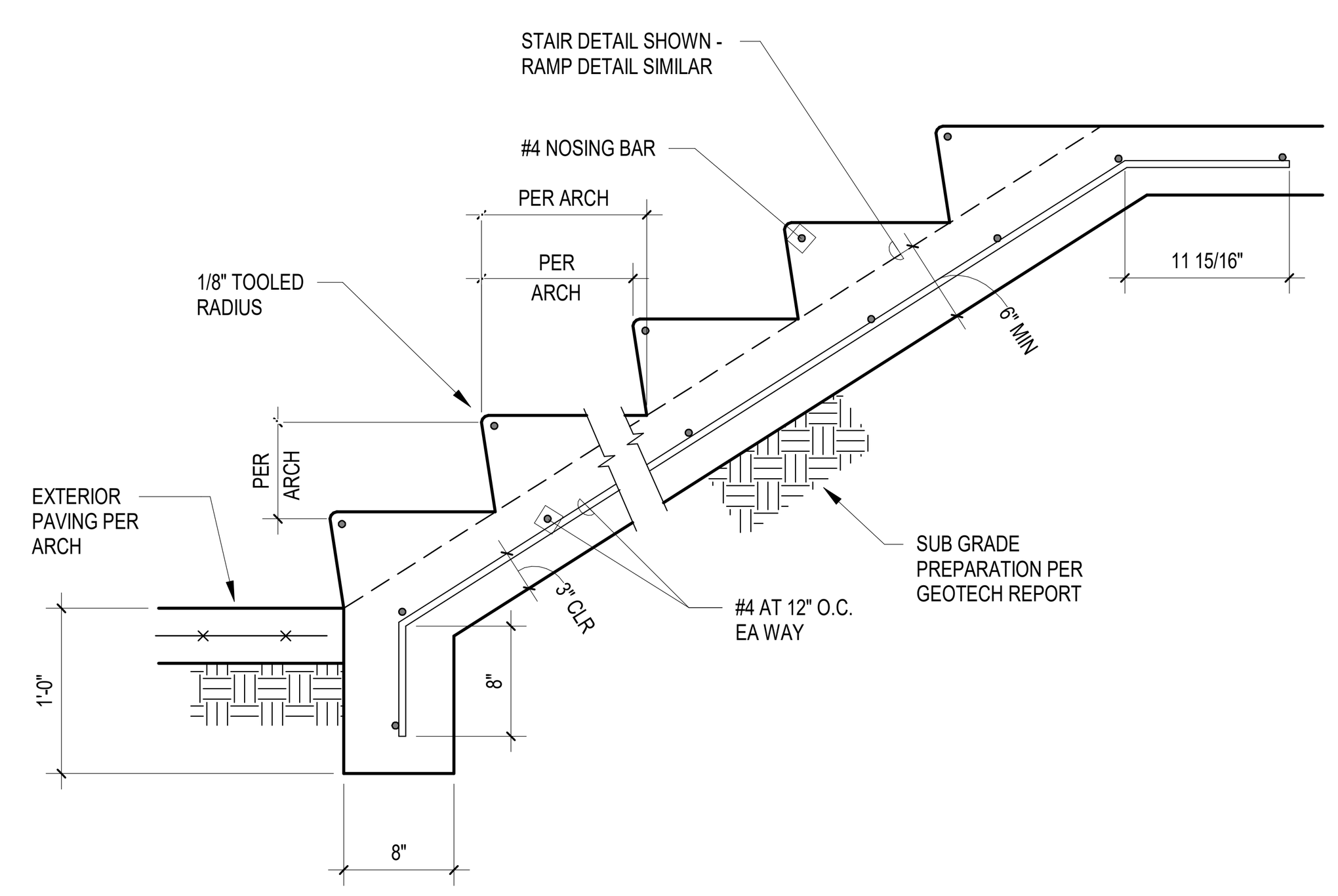
TYPICAL AT STEPS IN CONC SLAB ON GRADE

4 TYPICAL
1" = 1'-0" 4 / S0.8-B



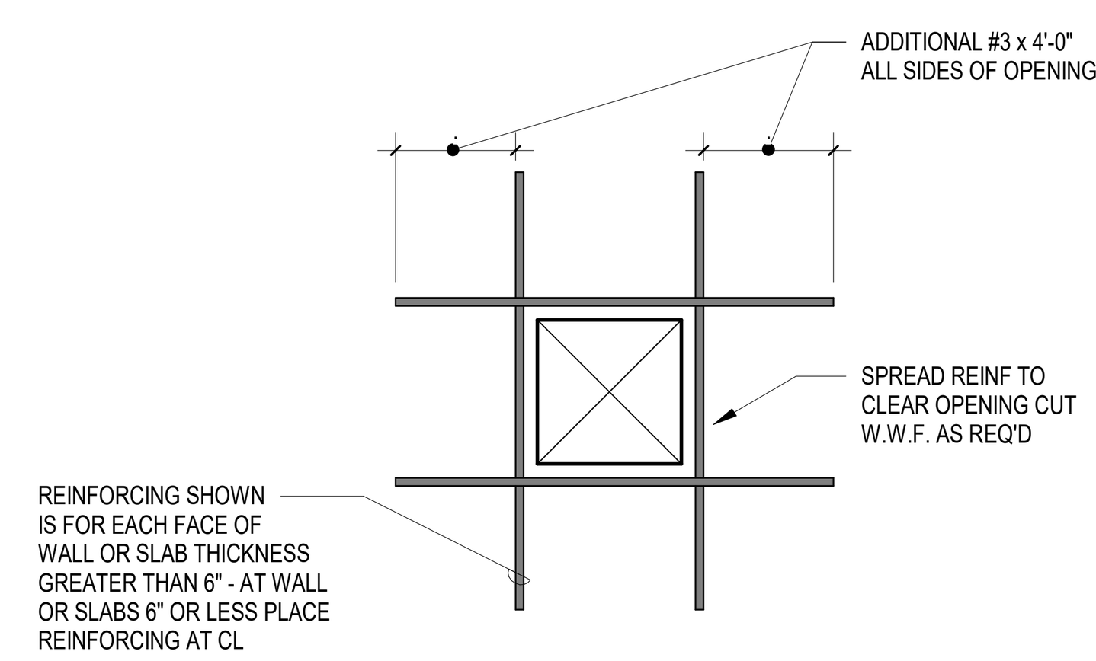
TYPICAL STEPPED FOOTING

5 TYPICAL
1" = 1'-0" 5 / S0.8-B

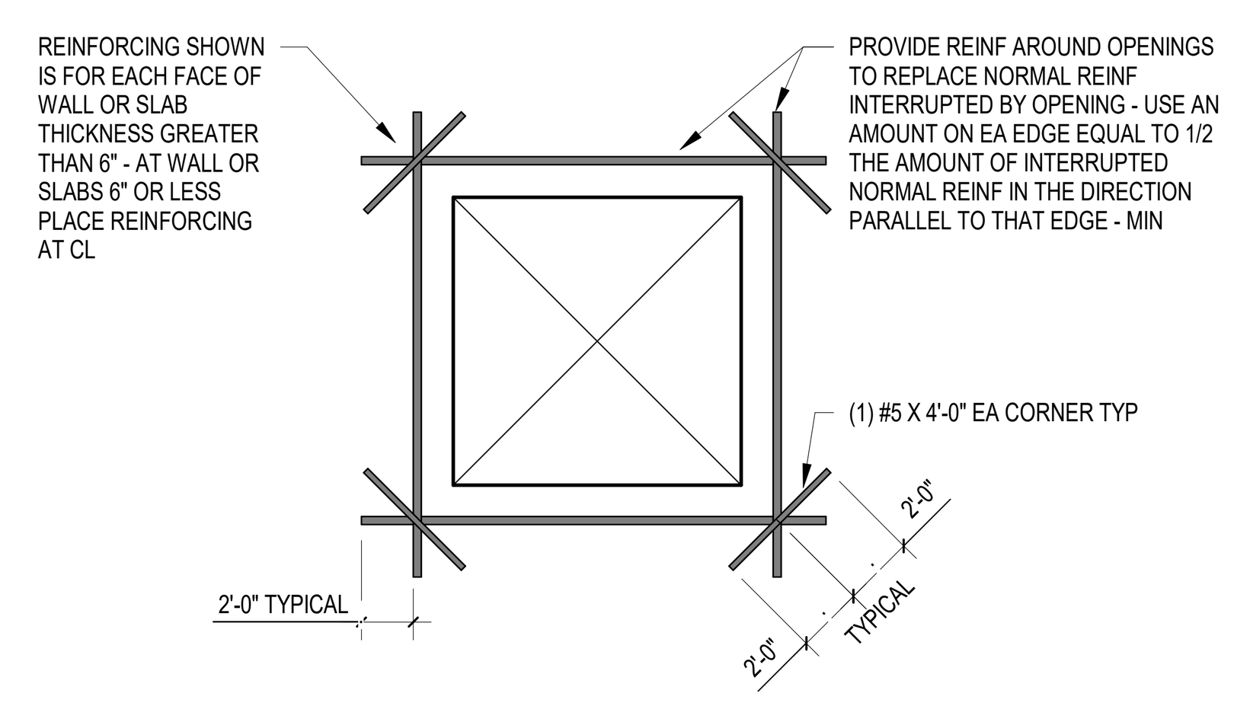


TYP CONCRETE STAIR CONSTRUCTION

6 TYPICAL
1" = 1'-0" 6 / S0.8-B

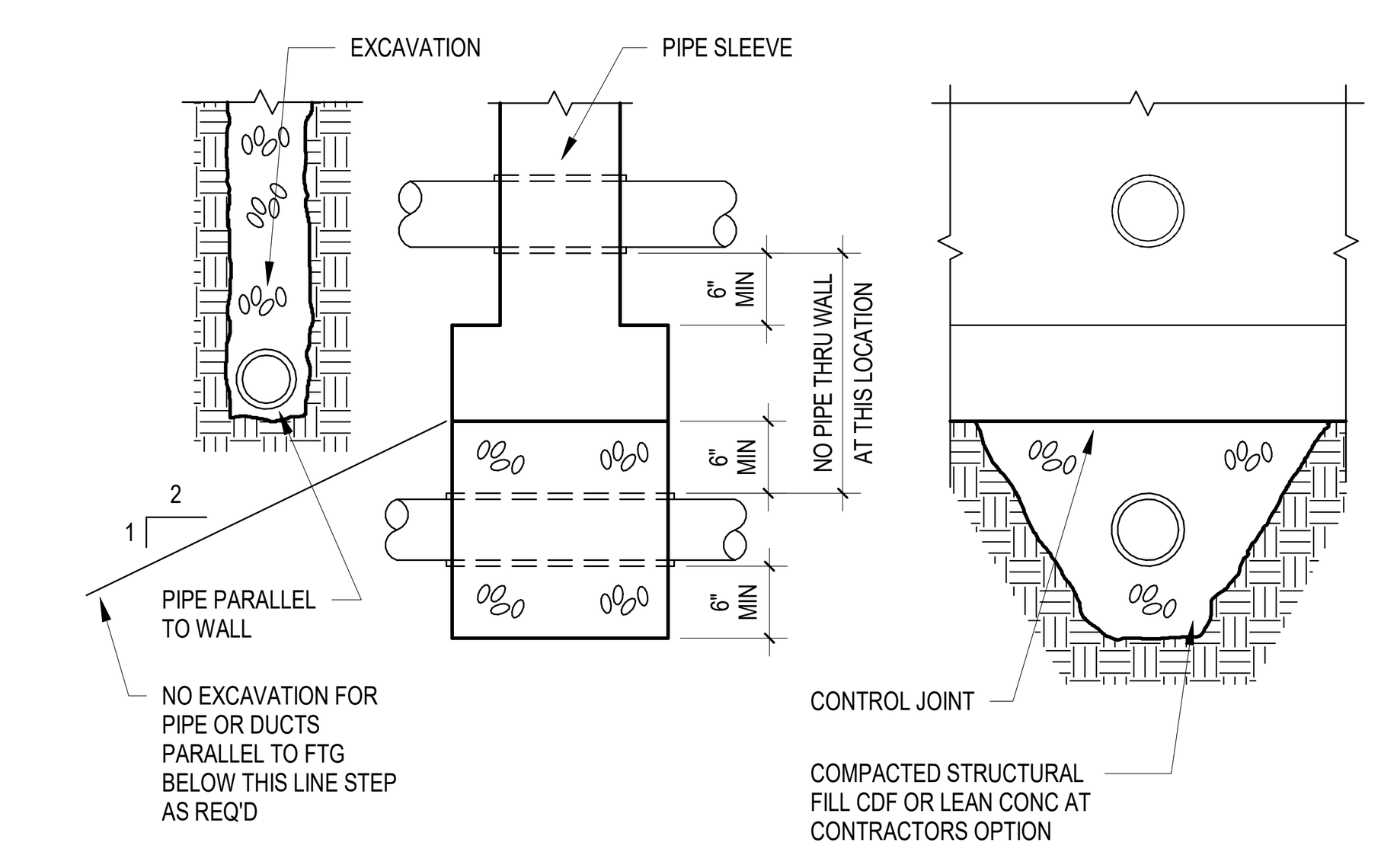


TYPICAL AT OPENINGS LESS THAN 12" IN CONC WALL OR SLAB



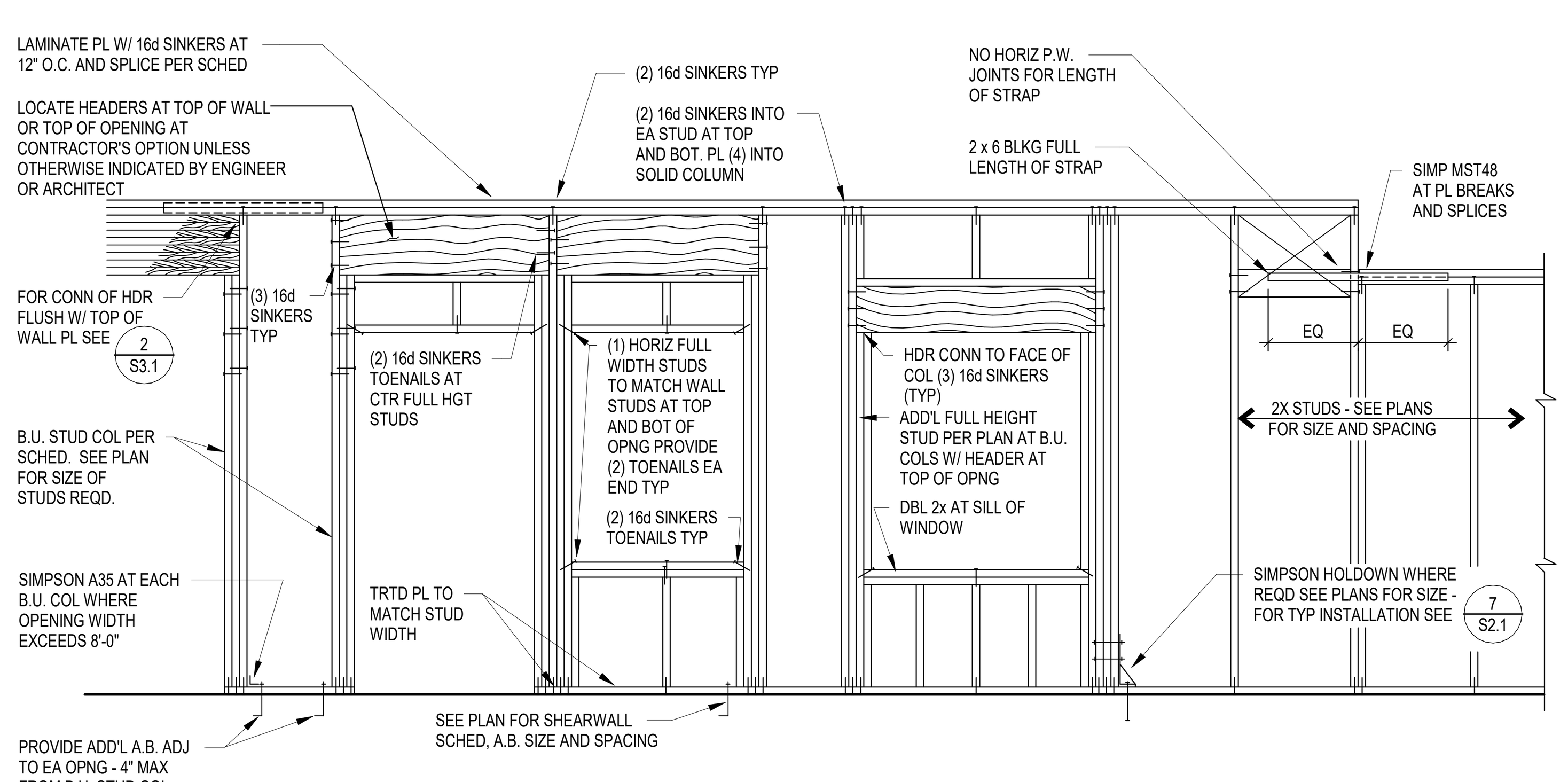
TYPICAL AT OPENINGS GREATER THAN 12" IN CONC WALL OR SLAB

7 TYPICAL
1" = 1'-0" 7 / S0.8-B



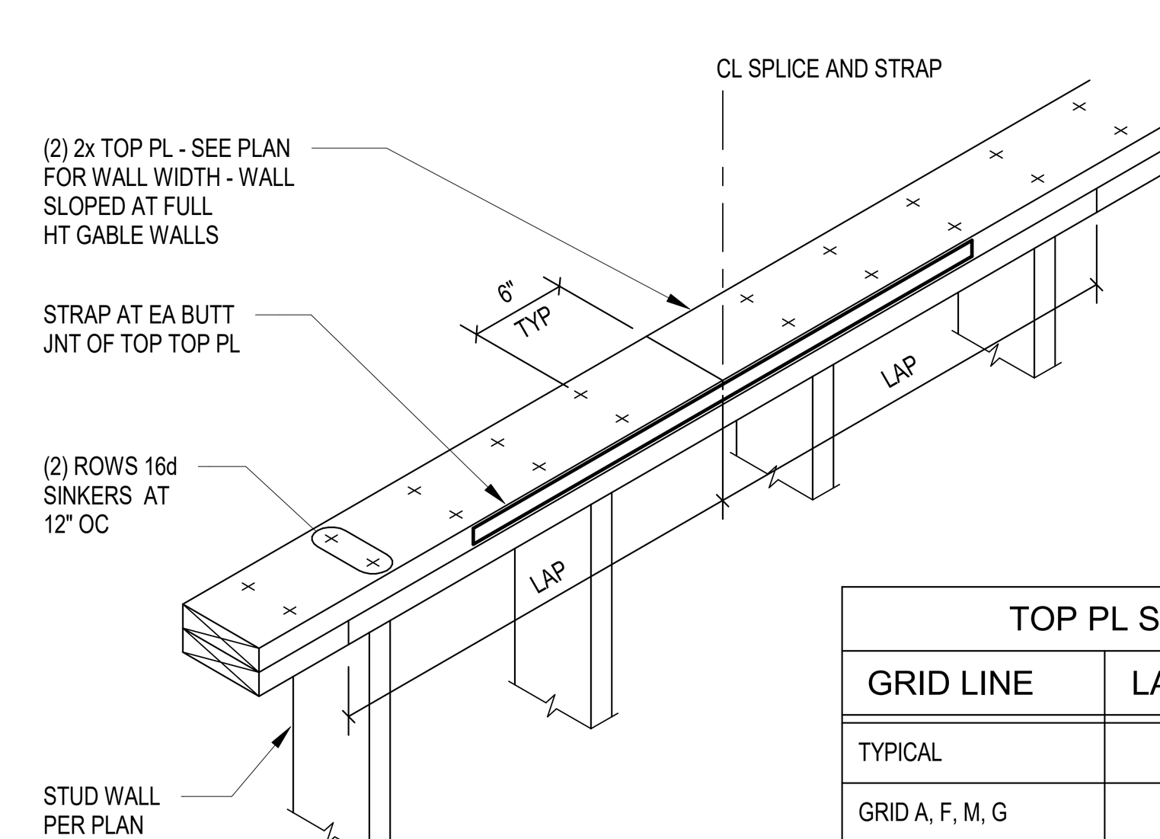
TYP DETAIL OF PIPE AT CONCRETE FTG

8 TYPICAL
1" = 1'-0" 8 / S0.8-B



TYPICAL BEARING WALL ELEVATION

9 TYPICAL
1" = 1'-0" 9 / S0.8-B



TOP PL SPLICE SCHEDULE		
GRID LINE	LAP LENGTH	STRAP
TYPICAL	4'-0" MIN	NONE
GRID A, F, M, G	8'-0" MIN	SIMP MSTA 36
NONE	8'-0" MIN	(2) SIMP MSTA 36

TYPICAL TOP PL SPLICE SCHEDULE

10 TYPICAL
1" = 1'-0" TYPICAL

12/4/2024 10:56:06 AM

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NOTICE:

WESLEY BRADLEY PARK PHASE 2 - CARE CENTER
707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 01/07/19

REVISIONS

No.	Description	Date

2220236.20
PROJECT NUMBER

KJK DRAWN BY
ADM CHECKED BY

WESLEY BRADLEY PARK
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TYPICAL DETAILS

S0.8

FOUNDATION NOTES

1. SEE SHEETS S0.1 - S0.2 FOR STRUCTURAL NOTES, SHEET S0.8 FOR TYPICAL DETAILS, AND SHEETS S0.3 AND S0.4 FOR TESTING AND INSPECTION NOTES.
2. SEE SHEET S0.5 FOR FOOTING SCHEDULE AND FOR CONCRETE COLUMN SCHEDULE.
3. SEE ARCHITECTURAL/MECHANICAL DRAWINGS FOR DRAINS, SLOPES, AND OTHER FLOOR DEPRESSIONS NOT SHOWN.
4. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, AND WALLS NOT SHOWN.
5. VERIFY ALL WINDOW AND DOOR WIDTHS AND HEIGHTS WITH ARCHITECTURAL DRAWINGS.
6. SEE ARCHITECTURAL DRAWINGS FOR STUD SIZE, SPACING, AND CALLOUTS AT NON-STRUCTURAL WALLS.
7. FOR TYPICAL CONNECTION OF NON-LOAD BEARING WALLS TO SLAB, USE POWDER ACTUATED FASTENERS AT 16" OC.
8. SEE GEOTECHNICAL ENGINEERING REPORT FOR ALL FOUNDATION AND SLAB SUPPORT REQUIREMENTS, THIS INCLUDES ALL EXCAVATION, FILL AND FILL PLACEMENT REQUIREMENTS.

FLOOR FRAMING NOTES - WOOD TRUSS CONSTRUCTIION

1. SEE S0.6 FOR WOOD FRAMING SCHEDULES.
2. ALIGN BEAMS SHALL HAVE 0" CAMBER UNLESS NOTED OTHERWISE.
3. ALIGN TRUSSES WITH STUDS BELOW WHERE SPACINGS ARE EQUAL
4. VERIFY ALL TOP OF BEAM AND TOP OF WALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
5. VERIFY ALL DOOR AND WINDOW WIDTHS AND HEIGHTS WITH ARCHITECTURAL DRAWINGS.
6. VERIFY SIZE AND LOCATION OF ALL MECHANICAL PENETRATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. ALL SAWN HEADERS SHOWN SHALL BE DF No. 1 UNLESS NOTED OTHERWISE.
8. .ALL PRE-ENGINEERED JOIST SPACINGS SHALL BE 2'-0" EXCEPT AS SHOWN OR NOTED.
9. TRUSS MANUFACTURER SHALL SUBMIT CERTIFICATION THAT TRUSES DESIGNED AND INSTALLED AS INDICATED IN THE ARCHITECTURAL, STRUCTURAL, AND SHOP DRAWINGS RESULT IN A FLOOR SYSTEM WITH AN ACCEPTABLE VIBRATION PERCEPTIBILITY PERFORMANCE.
10. TRUSS MANUFACTURER SHALL SUBMIT CERTIFICATION THAT TRUSSES DESIGNED AND INSTALLED AS INDICATED IN THE ARCHITECTURAL, STRUCTURAL, AND SHOP DRAWINGS RESULT IN A FLOOR SYSTEM WITH AN ACCEPTABLE VIBRATION PERCEPTIBILITY PERFORMANCE.
11. ATTACH NON STRUCTURAL WALLS TO FLOOR PER DETAIL 4 /S3.1 AND 5 /S3.1.
12. UNLESS NOTED OTHERWISE, SHEATHING SHALL BE UNBLOCKED AND ORIENTED WITH LONG EDGE OF PANEL (OR FACE GRAIN IF PLYWOOD IS USED) PERPENDICULAR TO SUPPORTS. PANELS SHALL BE STAGGERED WITH OFFSET JOINTS OCCURRING OVER SUPPORTS. MINIMUM SHEATHING DIMENSION PERPENDICULAR TO SUPPORTS SHALL BE 24" UNLESS EDGES OF PANEL ARE BLOCKED.
13. GYPCRETE (OR EQUIVALENT) TOPPING IS A NON-STRUCTURAL FLOOR FINISH PRODUCT, AND HAS NOT BEEN SPECIFIED OR DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD. THE MATERIAL IS SHOWN ON THESE DRAWINGS SOLELY FOR THE PURPOSE OF ITS INCLUSION IN THE DESIGN OF FLOOR JOISTS. THE ENGINEER OF RECORD ACCEPTS NO RESPONSIBILITY FOR THE APPROPRIATENESS, DESIGN, OR PROPER INSTALLATION OF THE TOPPING.

14 SEE THE SHEARWALL SCHEDULE FOR SHEATHING, NAILING AND ANCHOR BOLT REQUIREMENTS AT ALL WALLS INDICATED AS SHEARWALLS, EXTENT OF THE SHEARWALL REQUIREMENTS INCLUDE THE TOTAL LENGTH OF THE WALL INCLUDING ABOVE AND BELOW WINDOWS AND DOORS UNLESS NOTED OTHERWISE.

15 ALL LOAD BEARING WALL STUDS SHALL BE COVERED WITH A MIN. OF 1/2" SHEATHING (EITHER GWB, PW, OR OSB AS APPLICABLE) (1) SIDE OF STUDS. SEE ARCHTIECTURAL DRAWINGS FOR ADDITIONAL WALL COVERING REQUIREMENTS.

16 ALL 2x STUDS SHALL BE CONTINUOUS BETWEEN DETAIL CUTS. POSITION BUILT-UP STUDS TO ALIGN WITH THE TRUSSES ABOVE.

17 FOR TYPICAL CONNECTION OF NON-LOAD BEARING WALLS TO SLAB, USE POWDER ACTUATED FASTENERS AT 16" OC.

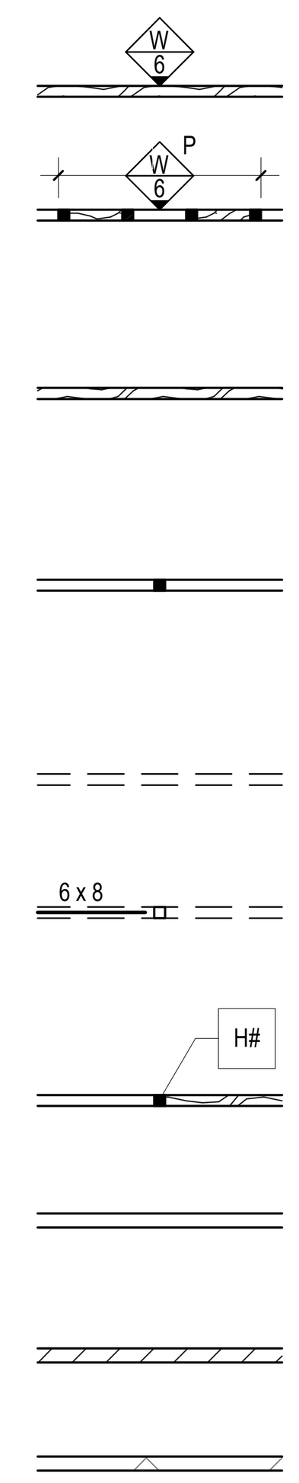
ROOF FRAMING NOTES - WOOD TRUSS CONSTRUCTION

1. ALL BEAMS SHALL HAVE 0" CAMBER UNLESS NOTED OTHERWISE.
2. VERIFY ALL TOP OF BEAM AND TOP OF WALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
3. VERIFY ALL DOOR AND WINDOW WIDTHS AND HEIGHTS WITH ARCHITECTURAL DRAWINGS.
4. VERIFY SIZE AND LOCATION OF ALL MECHANICAL PENETRATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
5. ALL SHADED AREAS SHALL BE OVERFRAMING AT 24" OC BY TRUSS MANUFACTURER.
6. BOTTOM CHORD ELEVATIONS MAY VARY. SEE ARCHITECTURAL DRAWINGS.
7. ALL SAWN HEADERS SHOWN SHALL BE DF No. 1 UNLESS NOTED OTHERWISE.
8. ALIGN WOOD TRUSSES WITH STUDS BELOW WHERE SPACINGS ARE EQUAL.
9. ATTACH NON-STRUCTURAL WALLS TO ROOF PER SHEET S4.1.
10. FOR SPECIAL NOTES REGARDING PRE-ENGINEERED METAL-PLATE-CONNECTION WOOD TRUSS DESIGN, COORDINATION AND FABRICATION, SEE "PRE-ENGINEERED METAL-PLATE-CONNECTION WOOD TRUSS NOTES."
11. ALL PRE-ENGINEERED WOOD TRUSS SPACINGS SHALL BE 2'-0" OC UNLESS NOTED OTHERWISE.

PRE-ENGINEERED METAL-PLATE-CONNECTED WOOD TRUSS NOTES

1. THE TRUSS ENGINEER SHALL BE A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.
2. THE TRUSS SHOP DRAWINGS SHALL INCLUDE A TRUSS PLACEMENT DIAGRAM AND TRUSS DESIGN DRAWINGS. THE TRUSS PLACEMENT DIAGRAM SHALL SHOW EACH TRUSS, TEMPORARY AND PERMANENT BRACING REQUIREMENTS INCLUDING PLACEMENT AND CONNECTION DETAILS, TRUSS TO TRUSS CONNECTION DETAILS AND REQUIRED HARDWARE, AND OVERFRAMING PLACEMENT AND CONNECTION DETAILS. TRUSS DESIGN DRAWING SHALL BE SEALED BY THE TRUSS ENGINEER AND SHALL INCLUDE SLOPE, DEPTH, SPAN AND SPACING; LOCATION OF JOINTS AND SUPPORTS; NUMBER OF PILES; REQUIRED BEARING WIDTHS; DESIGN LOADS; DESIGN ADJUSTMENT FACTORS; REACTIONS; CONNECTOR NUMBER, TYPE AND SIZE; SIZE, SPECIES AND GRADE FOR EACH MEMBER; TRUSS TO TRUSS CONNECTIONS; MAXIMUM DEFLECTIONS FOR LIVE AND TOTAL LOAD; MAXIMUM AXIAL TENSION AND COMPRESSION FORCES IN EACH MEMBER; AND REQUIRED PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING AND THE METHOD AND DETAILS OF RESTRAINT/BRACING. SHOP DRAWINGS MAY CONTAIN THE MANUFACTURER'S ENGINEERING RESPONSIBILITY LIMITATIONS. HOWEVER, THE SHOP DRAWINGS SHALL MAKE NO STATEMENT AS TO ENGINEER OF RECORD RESPONSIBILITIES.
3. ALL ROOF TRUSSES SHALL BE DESIGNED UNDER THE DIRECT SUPERVISION OF THE THE PRE-ENGINEERED TRUSS ENGINEER.
4. ROOF TRUSSES SHALL BE PROVIDED TO COMPLETE THE ROOF FRAMING FROM THE ROOF SHEATHING TO THE SUPPORTING STRUCTURE BELOW.
5. WHERE TRUSSES ARE NOT PROVIDED TO COMPLETE THE ROOF SYSTEM, OVERFRAMING MEMBERS AND THEIR CONNECTIONS SHALL BE PROVIDED. OVERFRAMING DETAILS SHALL BE INCLUDED IN THE TRUSS DESIGN DRAWINGS. IN ORDER TO PROVIDE LOADING CONDITIONS CONSISTENT WITH THE MODELING OF THE TRUSSES, THE OVERFRAMING AND RELATED DETAILS SHALL BE DESIGNED UNDER THE DIRECTION OF THE TRUSS ENGINEER.
6. TRUSS LOCATIONS ARE SCHEMATICALLY SHOWN ON THE PLANS. IT IS NOT THE INTENT OF THE PLANS TO GRAPHICALLY LOCATE ALL FRAMING MEMBERS EXCEPT WHERE SPECIFICALLY INDICATED. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING THE TRUSS SHOP DRAWINGS FOR MEMBER LAYOUT, CONTRACTIBILITY, AND QUANTITY TAKEOFFS.
7. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE TRUSS ENGINEER AND SHOWN IN THE TRUSS DESIGN DRAWINGS,
8. THE TRUSS ENGINEER SHALL VERIFY TRUSS BEARING CAPACITY ON HEM-FIR NO. 2 PLATES.
9. WHERE TRUSSES ALIGN WITH SHEARWALLS, A SPECIAL TRUSS SHALL BE PROVIDED THAT HAS BEEN DESIGNED BY THE TRUSS ENGINEER TO TRANSFER THE SPECIFIC WIND OR SEISMIC LOAD SHOWN ON THE PLANS. THE TRUSS SHALL BE DESIGNED TO TRANSFER THE LOAD BETWEEN THE ROOF SHEATHING OR DECKING AND THE SHEARWALL BELOW. THE TRUSS SHALL BE DESIGNED TO TRANSFER A MINIMUM OF 150 PLF ALONG THE LENGTH OF THE TRUSS. THE SPECIAL TRUSS SHALL BE DESIGNED CONSIDERING THE ACTUAL SUPPORT CONDITIONS AS SHOWN ON THE PLANS - HORIZONTAL REACTIONS SHALL BE RESISTED ONLY BY LATERAL FORCE RESISTING ELEMENTS SUCH AS SHEARWALLS.
10. ALL PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING REQUIRED FOR THE STABILITY OF THE TRUSS ELEMENTS UNDER GRAVITY LOADS, IN-PLANE WIND OR SEISMIC LOADS, AND WIND UPLIFT LOADS SHALL BE DESIGNED BY THE TRUSS ENGINEER. WHERE THE TOP CHORD IS NOT DIRECTLY ATTACHED TO THE ROOF SHEATHING, THE TRUSS ENGINEER SHALL DESIGN AND SHOW THE PLACEMENT OF ALL REQUIRED TOP CHORD BRACING AND CONNECTIONS ON THE TRUSS SHOP DRAWINGS. ANY BRACING LOADS TRANSFERRED TO THE MAIN BUILDING SYSTEM SHALL BE IDENTIFIED AND SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW.
11. THE GENERAL CONTRACTOR SHALL PROVIDE TEMPORARY INSTALLATION, RESTRAINT/BRACING IN ACCORDANCE WITH BCSI-2008 BUILDING COMPONENT SAFETY INFORMATION - GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF METAL-PLATE-CONNECTED WOOD TRUSSES.
12. FOR TRUSSES SPANNING 60-FEET OR GREATER, THE GENERAL CONTRACTOR SHALL CONTRACT WITH A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT FOR THE DESIGN OF BOTH TEMPORARY INSTALLATION RESTRAING/BRACING AND PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING.

WALL LEGEND:



SHEARWALL - REFER TO SHEARWALL SCHEDULE ON SHEET S0.6.

PERFORATED SHEARWALL - SHEATHING, NAILING AND BLOCKING SPECIFIED IN THE SHEARWALL SCHEDULE SHALL BE PROVIDED ABOVE AND BELOW OPENINGS FOR LENGTH INDICATED.

WOOD STUD FRAMED WALL ORIGINATING ON FOUNDATION OR FLOOR FRAMING PLAN ON WHICH IT IS NOTED. SEE DETAIL 9 / S0.8-B FOR REQUIREMENTS, SEE WOOD WALL SCHEDULE ON SHEET S0.7-B FOR STUD SIZE AND SPACING.

COLUMN ORIGINATING ON FOUNDATION OR FLOOR FRAMING PLAN ON WHICH IT IS NOTED. COLUMNS SHALL BE (3) BUILT-UP STUDS UNLESS NOTED OTHERWISE. SEE BUILT-UP STUD COLUMN SCHEDULE FOR STUDS REQUIRED EACH SIDE OF OPENINGS.

WALL BELOW FLOOR OR ROOF FRAMING PLAN ON WHICH IT IS NOTED - SEE PLAN BELOW FOR REQUIREMENTS.

COLUMN BELOW WITH HEADER SUPPORTING FLOOR OR ROOF FRAMING ON THE LEVEL ON WHICH IT IS NOTED - SEE PLAN BELOW FOR COLUMN REQUIREMENTS.

INDICATES HOLDOWN TYPE - REFER TO HOLDOWN SCHEDULE AND DETAILS ON SHEET S2.1.

CONC WALL - SEE PLAN AND DETAILS FOR REINFORCING REQUIREMENTS

CMU WALL PER PLAN - SEE SHEET S5.1-B FOR DETAILS.

STEEL STUD WALL PER PLAN - SEE SHEET S0.7 FOR DETAILS.

KEY TO ABBREVIATIONS

AB	ANCHOR BOLT	L	ANGLE
ABV	ABOVE	LLH	LONG LEG HORIZONTAL
ADDL	ADDITIONAL	LLV	LONG LEG VERTICAL
ADJ	ADJACENT	LOC	LOCATION
AFF	ABOVE FINISH FLOOR	LONGIT	LONGITUDINAL
ALT	ALTERNATE	MAX	MAXIMUM
ARCH	ARCHITECTURAL, ARCHITECT	MB	MACHINE BOLT
ASD	ALLOWABLE STRESS DESIGN	MECH	MECHANICAL
BEL	BELOW	MFR	MANUFACTURER
BLKG	BLOCKING	MIN	MINIMUM
BM	BEAM	MIW	MALLEABLE IRON WASHER
BNDY	BOUNDARY	NS	NEAR SIDE
BOT	BOTTOM	NTS	NOT TO SCALE
BRG	BEARING	NWT	NORMAL WEIGHT
BS	BOTH SIDES	O/	OVER
BTWN	BETWEEN	OC	ON CENTER
BU	BUILT UP	O.F.	OUTSIDE FACE
CIP	CAST IN PLACE	OPP	OPPOSITE HAND
CJ	CONSTRUCTION/CONTROL JOINT	OPNG	OPENING
CL	CENTERLINE	OSB	ORIENTED STRAND BOARD
CLG	CEILING	PC	PRE-CAST
CLR	CLEAR	PDF	POWER DRIVEN FASTENERS, PAF
CMU	CONCRETE MASONRY UNIT	PAF	POWER ACTUATED FASTENERS, PDF
COL	COLUMN	PERP	PERPENDICULAR
CONC	CONCRETE	PL	PLATE
CONN	CONNECT, CONNECTION	PLF	POUNDS PER LINEAR FOOT
CONT	CONTINUOUS	PNL	PANEL
COORD	COORDINATE	PRE-ENGR	PRE-ENGINEERED
CSK	COUNTERSINK	PROV	PROVIDE
CTR	CENTER	PT	POST TENSIONED
CVR	COVER	PW	PLYWOOD
DEG	DEGREE	REF	REFERENCE
DIA	DIAMETER	REINF	REINFORCE, REINFORCEMENT
DBL	DOUBLE	REQ'D	REQUIRED
EA	EACH	RF	ROOF
EF	EACH FACE	SCHED	SCHEDULE
ELEV	ELEVATION, ELEVATOR	SFRS	SEISMIC FORCE RESISTING SYSTEM
EMB	EMBEDMENT	SHTG	SHEATHING
ENGR	ENGINEER	SIM	SIMILAR
EQ	EQUAL/EQUIVALENT	SIMP	SIMPSON STRONG-TIE
EQUIV	EQUIVALENT	SOG	SLAB ON GRADE
ES	EACH SIDE	SPCG	SPACING
EW	EACH WAY	SQ	SQUARE
(E)	EXISTING	STD	STANDARD
EXP	EXPANSION	STIFF	STIFFENER
EXT	EXTERIOR	SW	SHEARWALL
FDN	FOUNDATION	T&G	TONGUE AND GROOVE
FF	FINISH FLOOR	THK	THICK
FFE	FINISH FLOOR ELEVATION	THRD	THREADED
FOC	FACE OF CONCRETE	T.O.	TOP OF
FOM	FACE OF MASONRY	TOC	TOP OF CONCRETE
FOS	FACE OF STUD	TOF	TOP OF FOOTING
FS	FAR SIDE	TOPL	TOP OF PLATE
FTG	FOOTING	TOS	TOP OF STEEL
GA	GAGE	T.O.W.	TOP OF WALL
GALV	GALVANIZED	TRANSV	TRANSVERSE
GC	GENERAL CONTRACTOR	TRTD	TREATED
GL	GLUE LAMINATED	TYP	TYPICAL
GWB	GYPSUM WALL BOARD	UNO	UNLESS NOTED OTHERWISE
HGR	HANGER	VFY	VERIFY
HORIZ	HORIZONTAL	VERT	VERTICAL
HSS	HOLLOW STEEL SECTION	W/	WITH
HT	HEIGHT	W/O	WITHOUT
I.F.	INSIDE FACE	WF	WIDE FLANGE
INT	INTERIOR	WHS	WELDED HEADED STUD
JNT	JOINT	WP	WORK POINT
JST	JOIST	WTS	WELDED THREADED STUD
K, KIPS	KIPS=1000 LBS	WWF	WELDED WIRE FABRIC

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PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 08/17/16

REVISIONS

No.	Description	Date

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PROJECT NUMBER

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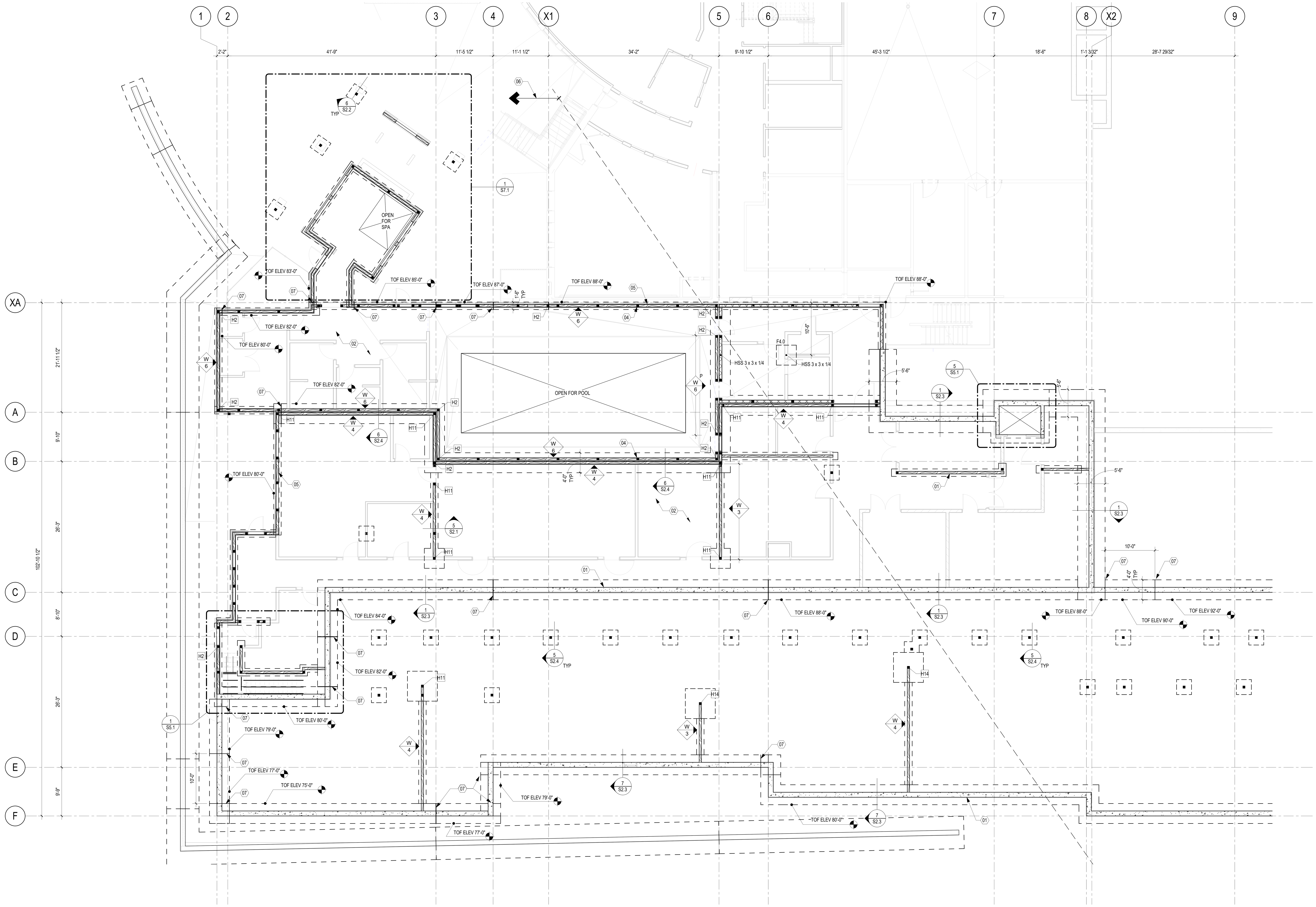
PHASE 2 - CARE CENTER

GENERAL NOTES

12/4/2024 10:56:08 AM

KEYNOTES

01	12" THICK CONC WALL - REINF W/ #6 VERT AT 12" OC EA FACE AND (2) #5 HORIZ AT 12" OC
02	4" CONC SOG - REINF W/ 6x6 W1.4xW1.4 WWF AT CL OF SLAB
04	6 x 6 DF No. 1 POSTS AT EACH GLULAM BEAM ABOVE
05	2 x 6 DF No. 2 STUDS AT 16" OC - TYP
06	REMOVE AND REPLACE EXIST FILL W/ STRUCT FILL - COORD LOCATION AND EXTENTS W/ GEOTECH
07	STEP FOOTING PER 5 / S0.8



1 BASEMENT FOUNDATION PLAN - NORTH

1/8" = 1'-0"

SCALE IN FEET

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1000 university ave. w. suite 130
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WESLEY BRADLEY PARK
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BASEMENT FOUNDATION PLAN - NORTH

S1.0-N

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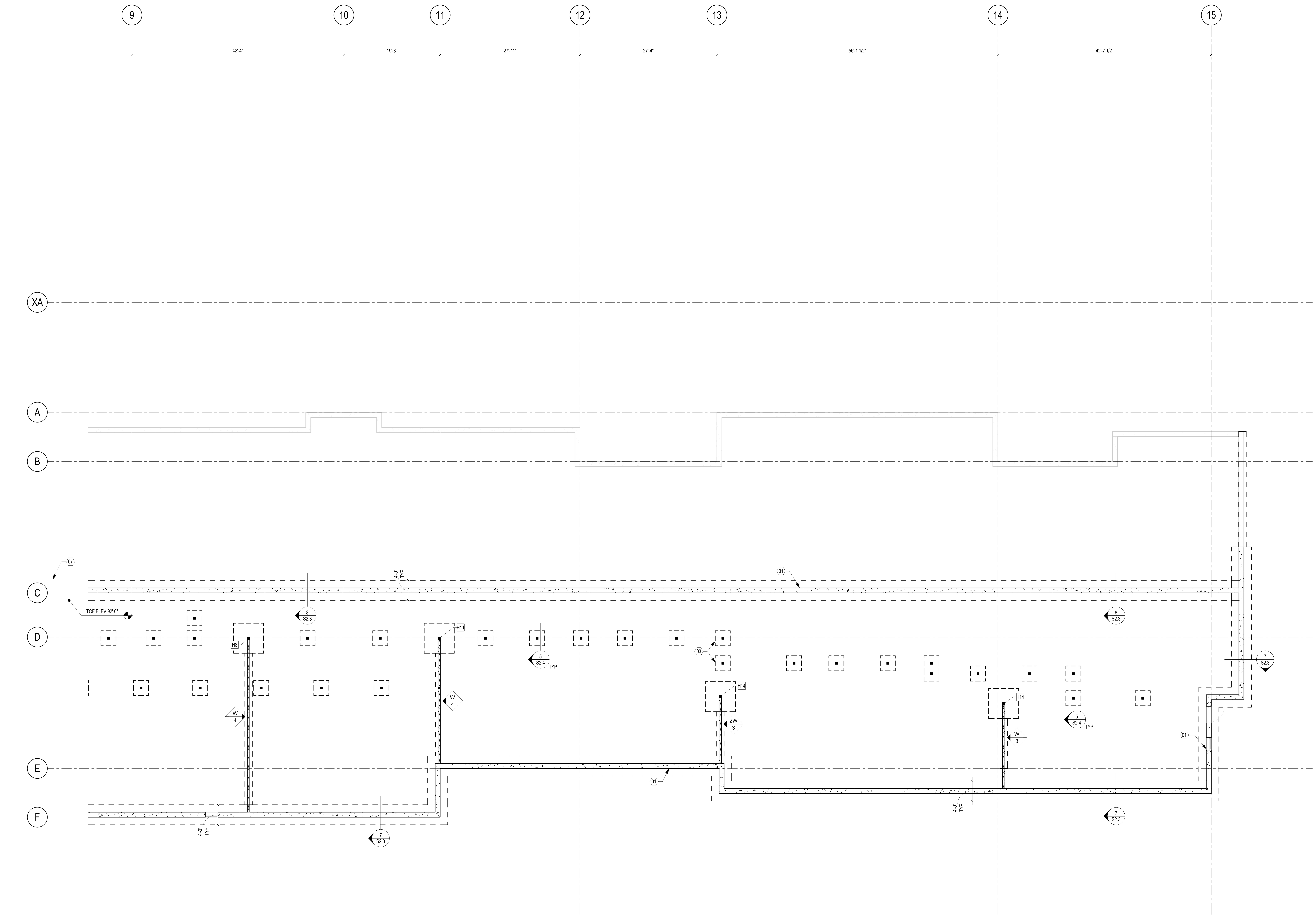
KEYNOTES

01	12" THICK CONC WALL - REINF W/ #6 VERT AT 12" OC EA FACE AND (2) #5 HORIZ AT 12" OC
03	6 x 6 DF No. 1 POSTS ON F3.0 FOOTING AT 12'-0" OC - TYP AT CRAWL SPACE
07	STEP FOOTING PER 5 / S0.8



1 BASEMENT FOUNDATION PLAN - SOUTH

1/8" = 1'-0"

0 4' 8' 12'

SCALE IN FEET

in site architects
1000 university ave. w. suite 130
st. paul, minnesota 55104
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NOTICE:

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ADM
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BASEMENT FOUNDATION PLAN - SOUTH

S1.0-S

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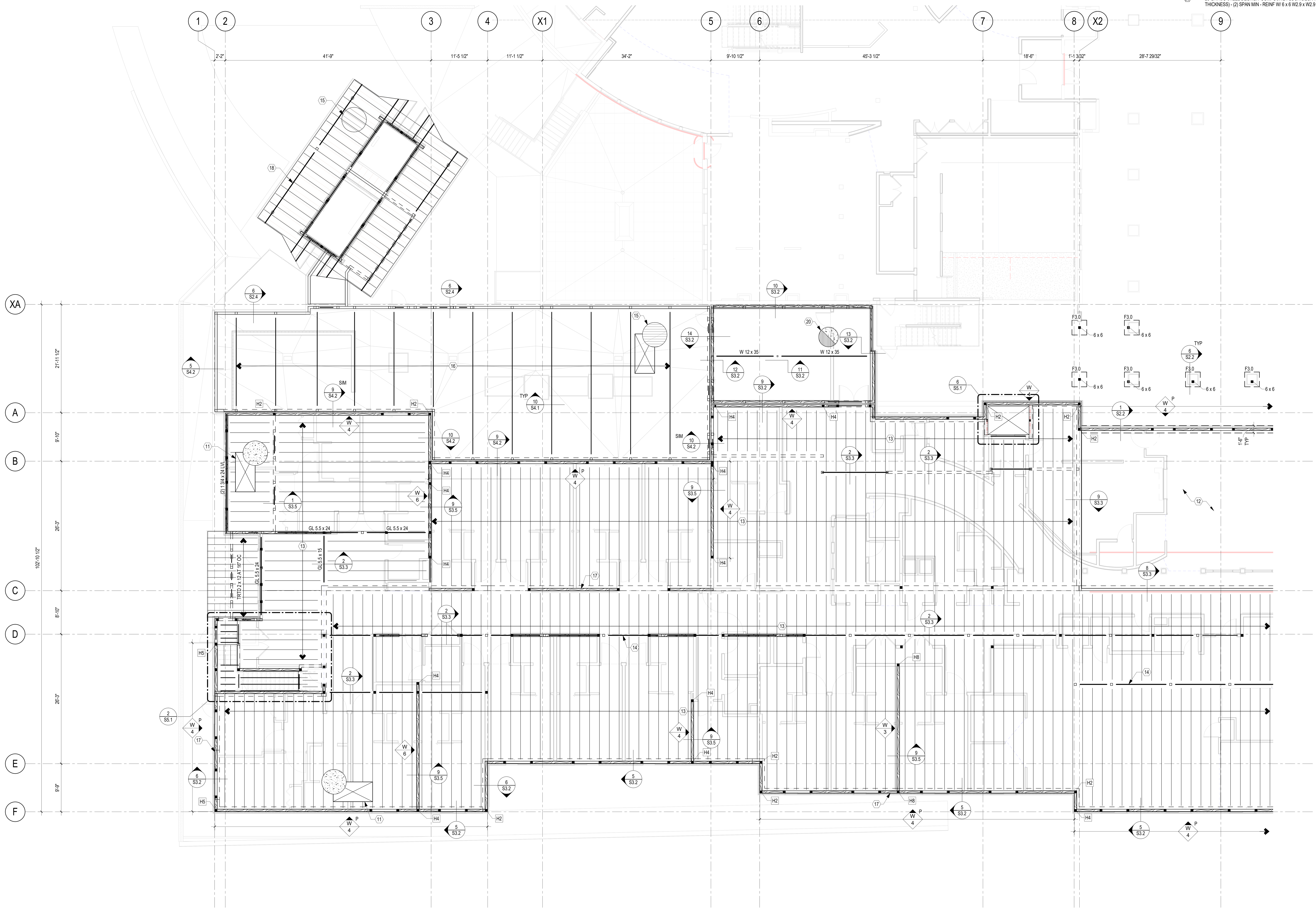
KEYNOTES

- (11) 1 1/4" CONC TOPPING OVER 3/4" APA RATED 48/24 SHTG W/ LONG EDGE PERP TO SUPPORTS - ATTACH W/ 10d AT 6" OC AT EDGES AND 12" OC IN FIELD
- (12) 4" CONC SOG - REINF W/ 6x6 W1.4xW1.4 WWF AT CL SLAB
- (13) 24" DEEP PRE-ENGR WOOD TRUSSES AT 24" OC MAX
- (14) TYP FLOOR BEAM: GL 5.5 x 15 24F-V4
- (15) 2" NOMINAL T&G TIMBER DECKING W/ 3/8" APA RATED SHTG
- (16) GL 5.125 x 18 20F-V12 AT 8'-0" MAX
- (17) 2 x 6 DF No. 2 STUDS AT 16" OC - TYP
- (18) GLULAM POST AND BEAM FRAMING AT HOT TUB AREA / OUTDOOR PAVILION
- (20) 20 GA ASC STEEL DECK 2W COMPOSITE FLOOR DECK W/ 5 1/4" NWT CONC TOPPING (7 1/4" TOTAL THICKNESS) - (2) SPAN MIN - REINF W/ 6 x 6 W2.9 x W2.9 WWF AT CL OF TOPPING - 3 HOUR RATED



1 LEVEL 1 FRAMING PLAN - NORTH

1/8" = 1'-0"

0 4' 8' 12'
SCALE IN FEET

in site architects
1000 university ave. w. suite 130
st. paul, minnesota 55104
612-252-4820

WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 05/22/15

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2220236.20
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LEVEL 1 FRAMING PLAN - NORTH

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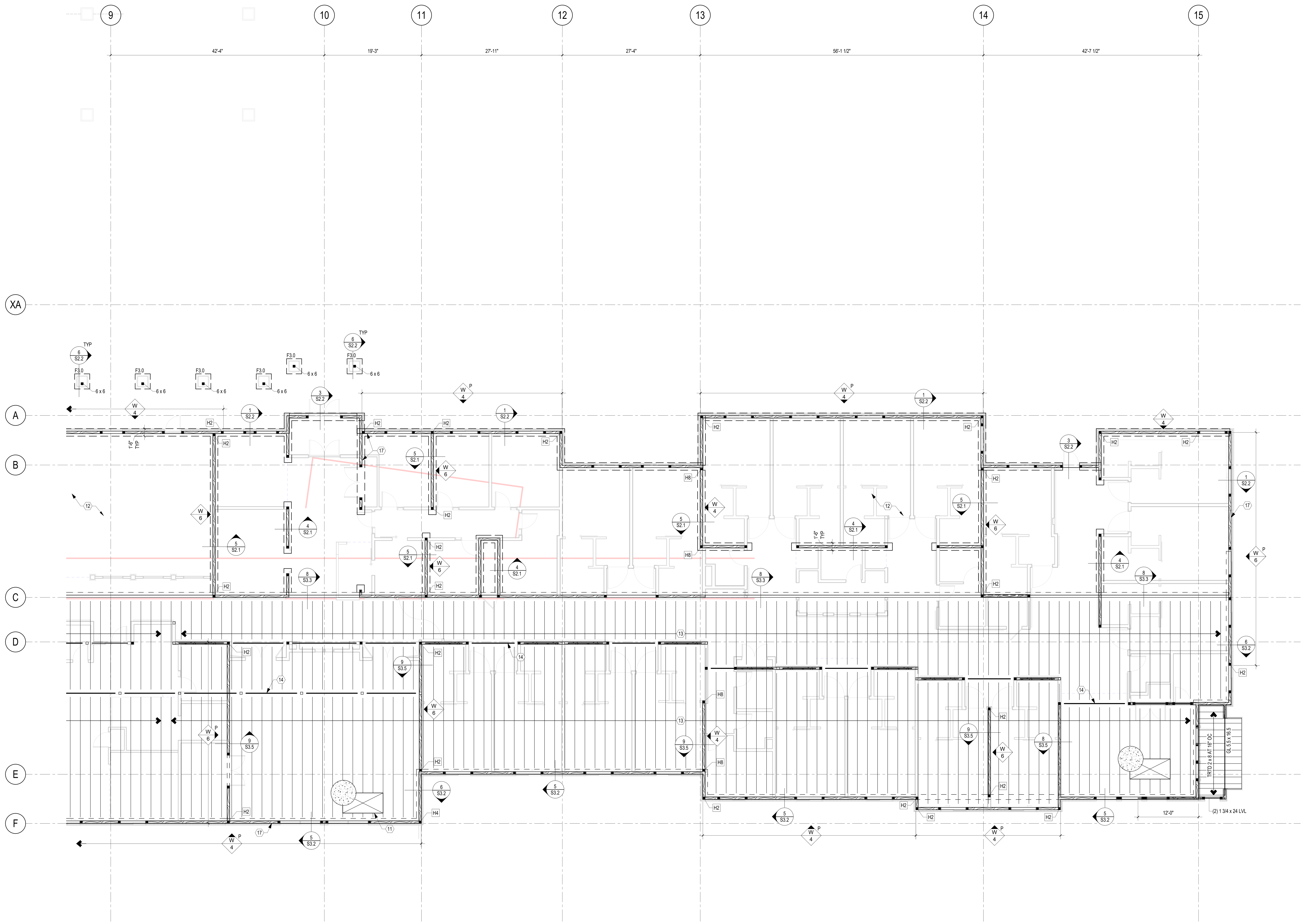
S1.1-N

12/4/2024 10:56:12 AM

KEYNOTES

- 11 — 1 1/4" CONC TOPPING OVER 3/4" APA RATED 48/24 SHTG W/ LONG EDGE PERP TO SUPPORTS - ATTACH W/ 10d AT 6" OC AT EDGES AND 12" OC IN FIELD
- 12 — 4" CONC SOG - REINF W/ 6x6 W1.4xW1.4 WWF AT CL SLAB
- 13 — 24" DEEP PRE-ENGR WOOD TRUSSES AT 24" OC MAX
- 14 — TYP FLOOR BEAM: GL 5.5 x 15 24F-V4
- 17 — 2 x 6 DF No. 2 STUDS AT 16" OC - TYP



1 LEVEL 1 FRAMING PLAN - SOUTH

1/8" = 1'-0"

0 4' 8' 12'

SCALE IN FEET

in site architects
1000 university ave. w. suite 130
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612-252-4820

ANDREW DEAN McEACHERN — STATE OF WASHINGTON — 38722 — STRUCTURAL — PROFESSIONAL ENGINEER

WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

707 39TH AVENUE SE
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PERMIT SET
DECEMBER 2, 2024

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PHASE 2 - CARE CENTER

LEVEL 1 FRAMING PLAN - SOUTH

S1.1-S

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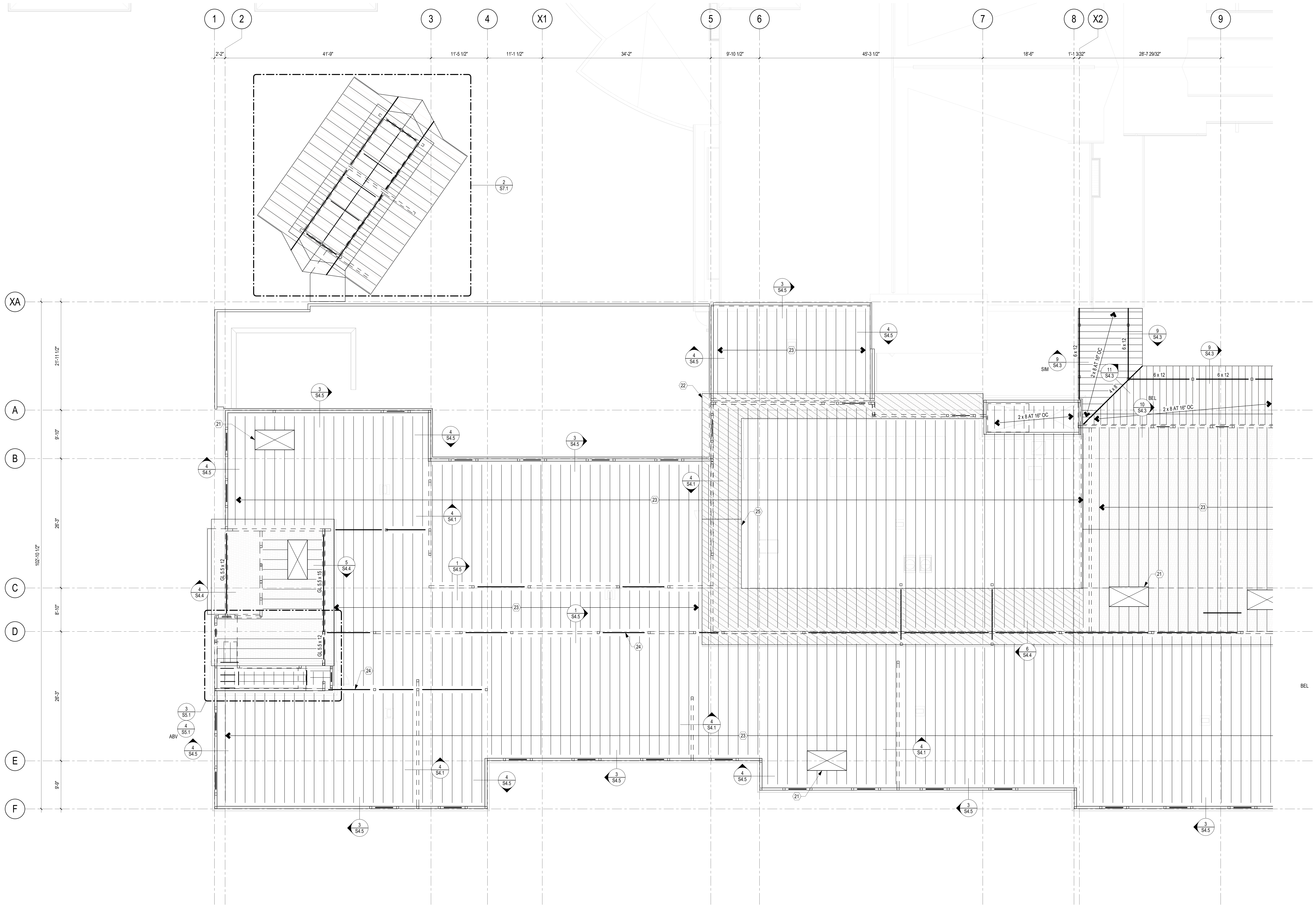
KEYNOTES

- (21) 19/32" APA RATED 40/20 SHTG W/ LONG EDGE PERP TO SUPPORTS - ATTACH W/ 10d AT 6" OC AT EDGES AND 12" OC IN FIELD
- (22) SHADED AREA INDICATES OVERFRAMING BY TRUSS MFR
- (23) PRE-ENGR WOOD TRUSSES AT 24" OC MAX
- (24) TYP ROOF BEAM: 6 x 12 DF No. 1
- (25) DIAGONAL HATCHED ARE INDICATED MANSARD ROOF OVERFRAMING BY TRUSS MFR - SEE ROOF DETAILS



1 ROOF FRAMING PLAN - NORTH

1/8" = 1'-0"

0 4' 8' 12'
SCALE IN FEET

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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

707 39TH AVENUE SE
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PERMIT SET
DECEMBER 2, 2024

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ROOF FRAMING PLAN - NORTH

S1.2-N

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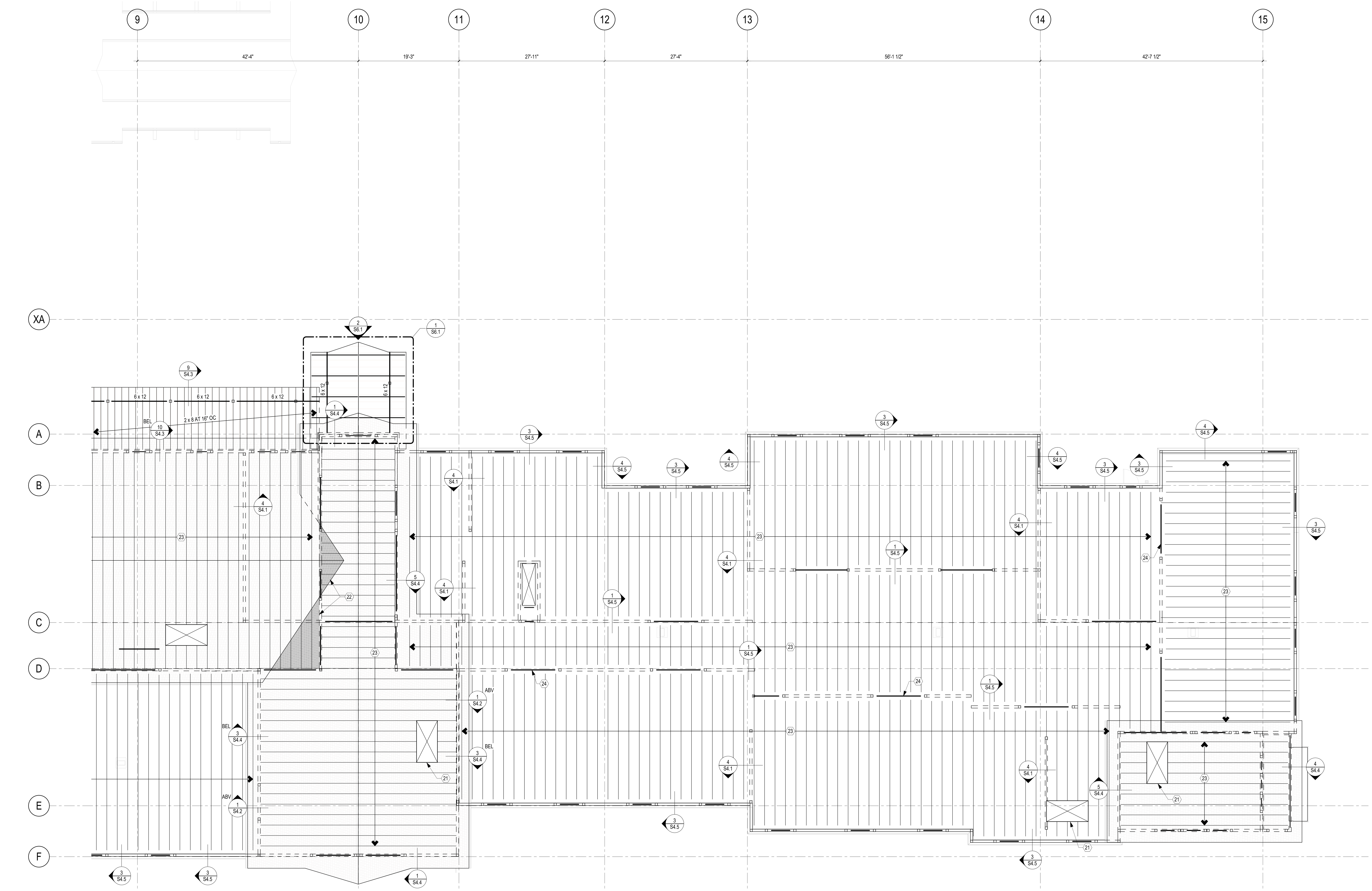
KEYNOTES

- (21) 19/32" APA RATED 40/20 SHTG W/ LONG EDGE PERP TO SUPPORTS - ATTACH W/ 10d AT 6" OC AT EDGES AND 12" OC IN FIELD
- (22) SHADED AREA INDICATES OVERFRAMING BY TRUSS MFR
- (23) PRE-ENGR WOOD TRUSSES AT 24" OC MAX
- (24) TYP ROOF BEAM: 6 x 12 DF No. 1



1 ROOF FRAMING PLAN - SOUTH

1/8" = 1'-0"

0 4' 8' 12'
SCALE IN FEET

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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

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ROOF FRAMING PLAN - SOUTH

S1.2-S

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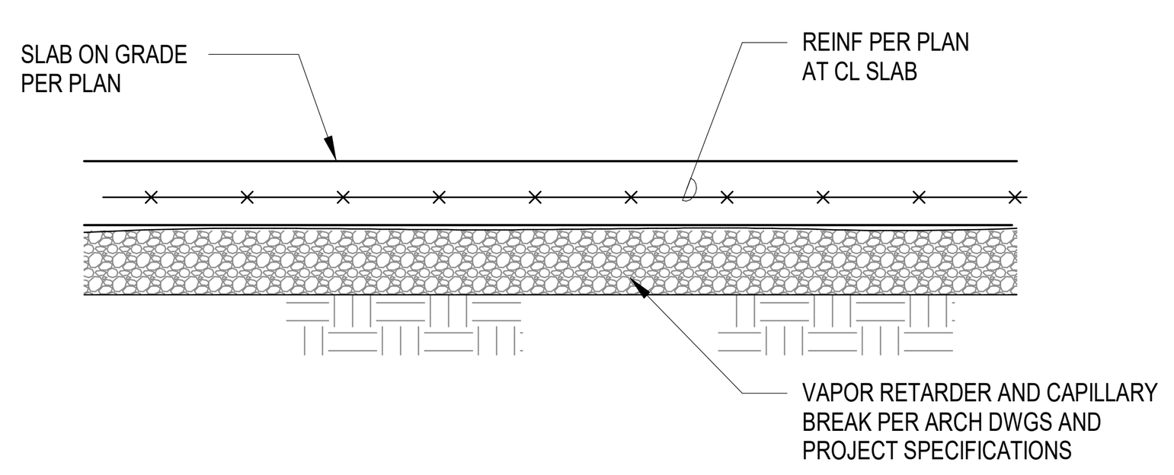

1 SECTION

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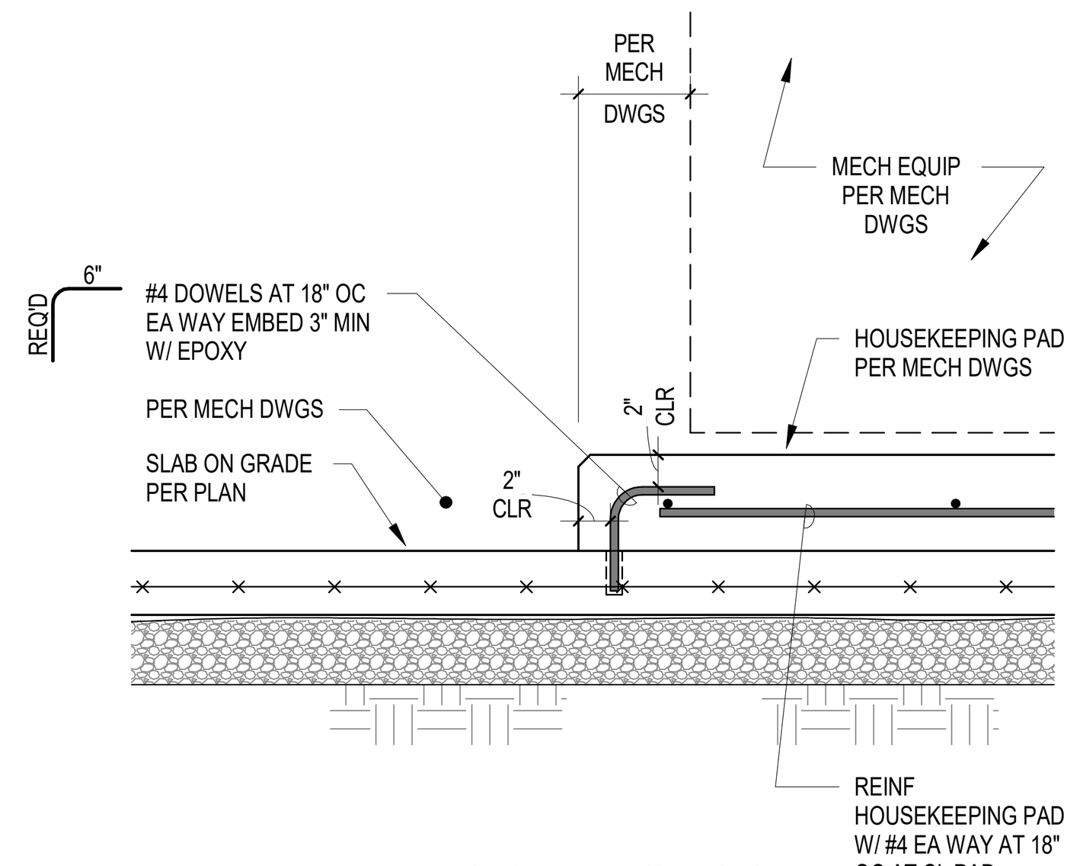
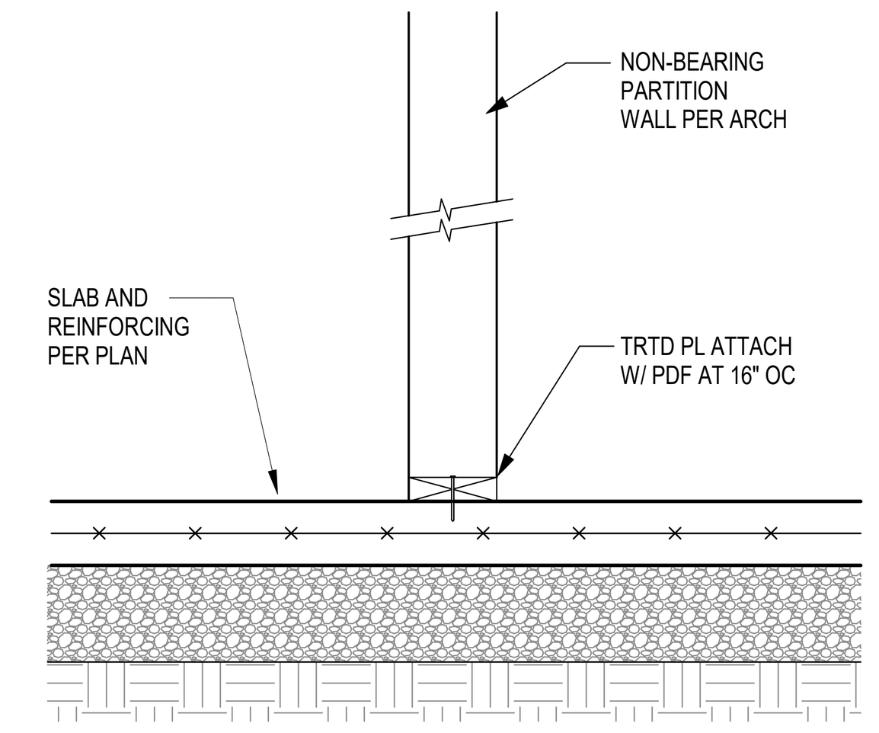


TYP HOUSEKEEPING PAD

2 SECTION

1" = 1'-0" 2 / S2.1



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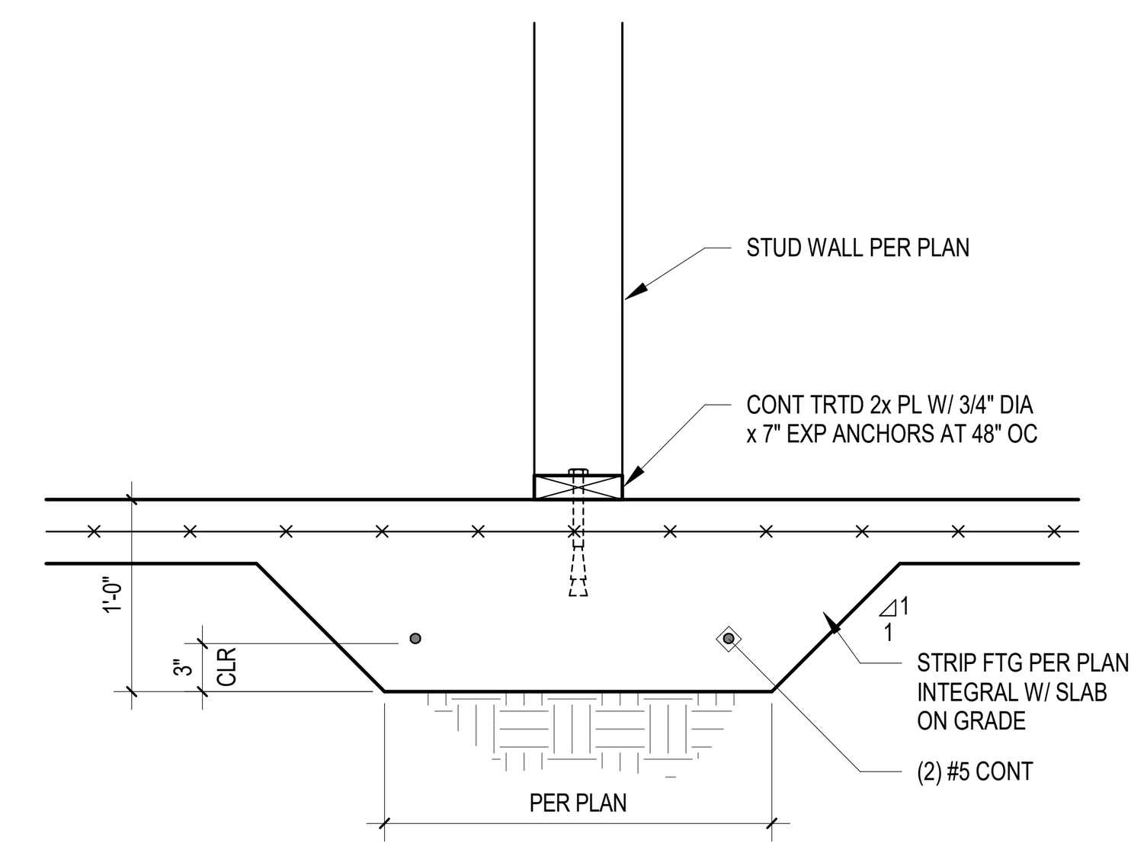

4 SECTION

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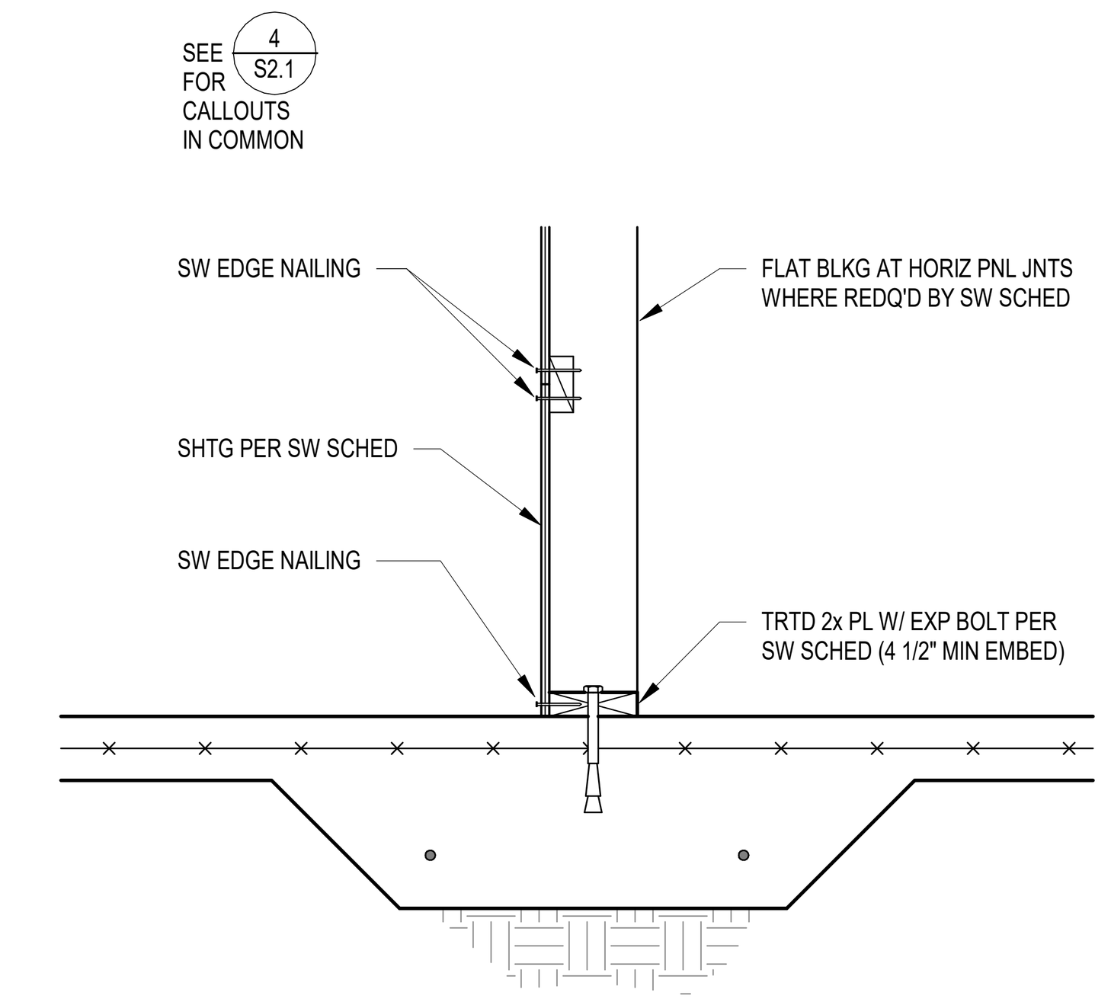
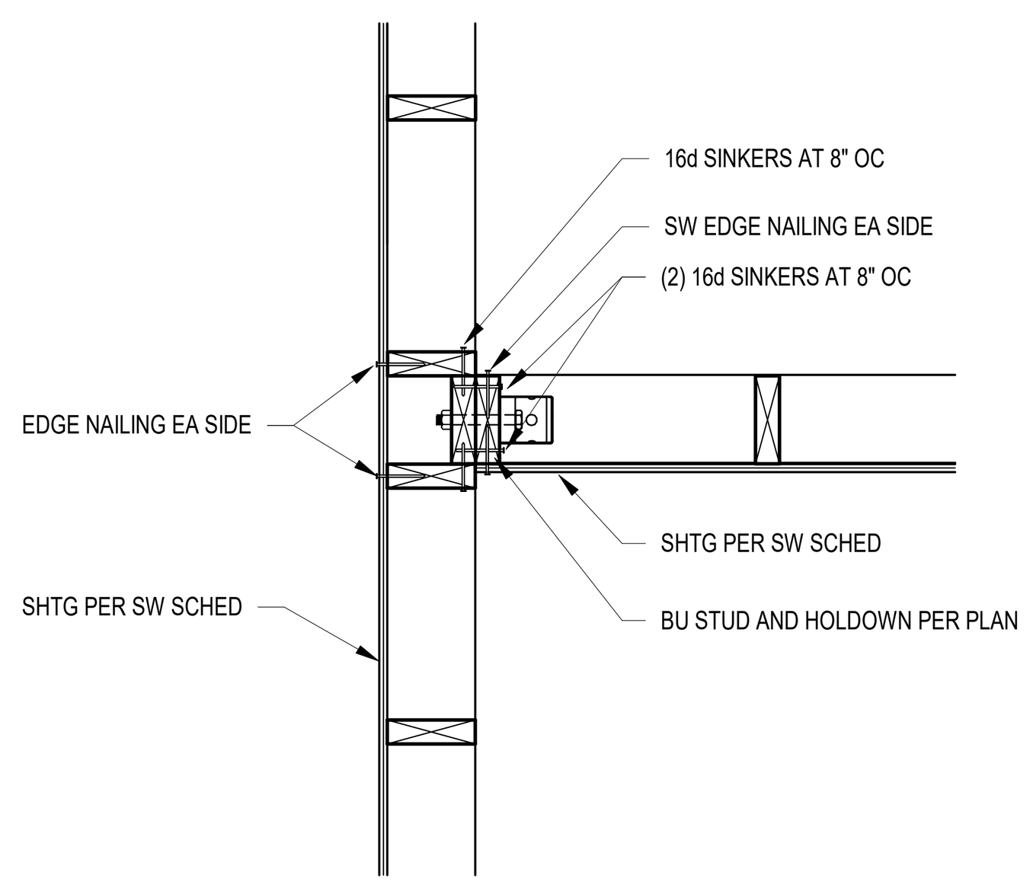


5 SECTION

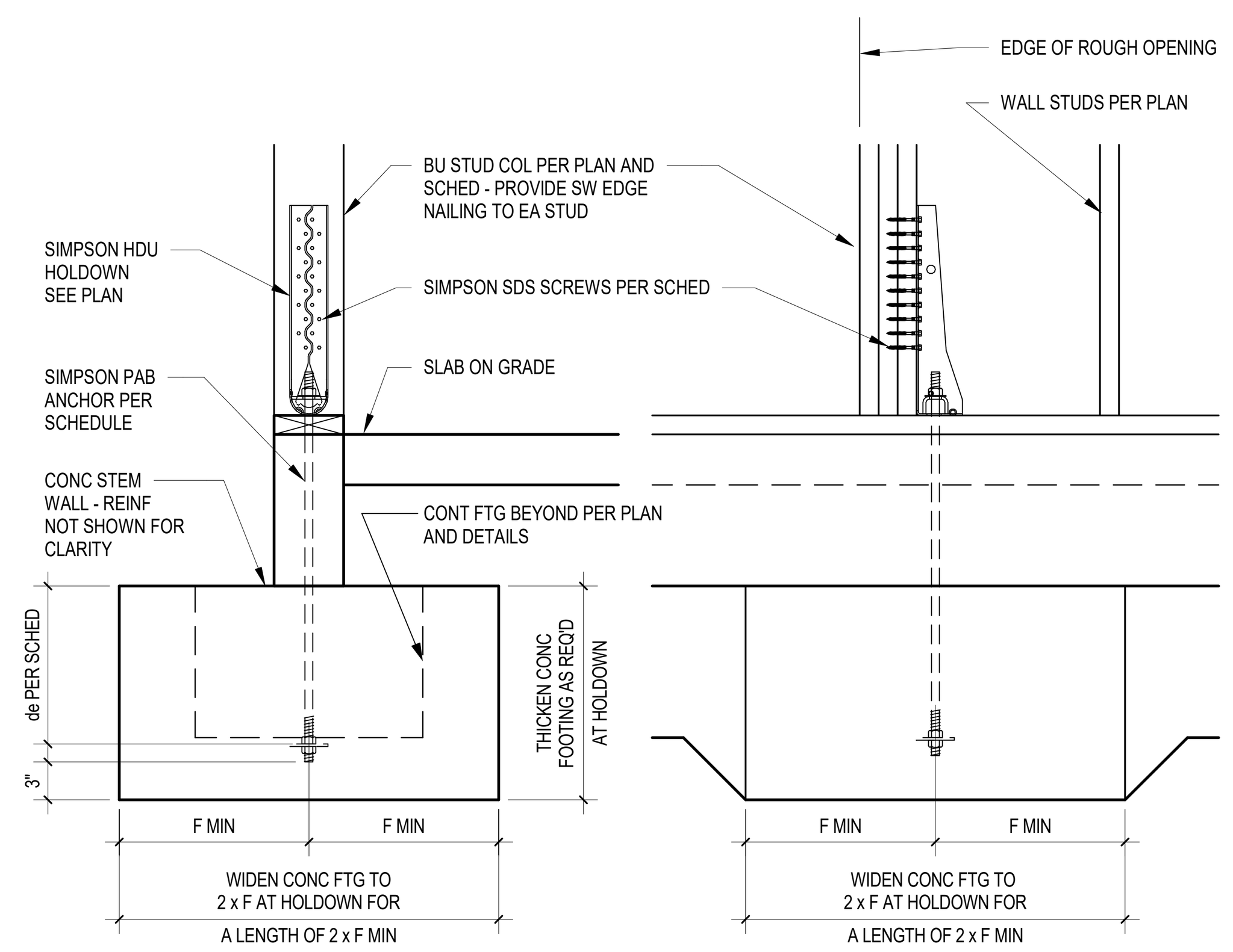
1" = 1'-0" 5 / S2.1



TYPICAL SHEAR WALL INTERSECTION

6 PLAN

1" = 1'-0" 6 / S2.1



HOLDOWN SCHEDULE

MARK	HOLDOWN	FOUNDATION ANCHOR	de	F	FASTENERS	MINIMUM WOOD MEMBER THICKNESS	ALLOWABLE TENSION LOAD
H2	SIMP HDU2	SIMP PAB5	6"	9"	(6) SIMP SDS 1/4" x 2 1/2"	3"	3075#
H4	SIMP HDU4	SIMP PAB5	6"	9"	(10) SIMP SDS 1/4" x 2 1/2"	3"	4565#
H5	SIMP HDU5	SIMP PAB5	6"	9"	(14) SIMP SDS 1/4" x 2 1/2"	3"	5645#
H8	SIMP HDU8	SIMP PAB7	10"	15"	(20) SIMP SDS 1/4" x 2 1/2"	4 1/2"	7870#
H11	SIMP HDU11	SIMP PAB8	12"	18"	(30) SIMP SDS 1/4" x 2 1/2"	5 1/2"	9535#
H14	SIMP HDU14	SIMP PAB8	12"	18"	(36) SIMP SDS 1/4" x 2 1/2"	6x6	14445#

HOLDOWN SCHEDULE NOTES:

1. ALLOWABLE LOADS ARE VALID FOR HOLDOWN FLUSH OR RAISED OFF SILL PLATE.
2. TABULATED LOADS MAY BE DOUBLED WHEN THE HDU IS INSTALLED ON OPPOSITE SIDES OF THE WOOD MEMBER PROVIDED EITHER THE POST IS LARGE ENOUGH TO PREVENT OPPOSING HOLDOWN SCREW INTERFERENCE, OR THE HOLDOWNS ARE OFFSET TO ELIMINATE SCREW INTERFERENCE.
3. SIMP PAB8 SHALL USE A HEAVY HEX ANCHOR NUT.
4. AT POST-TENSIONED SLAB SEE 6 / S3.1-B AND 7 / S3.1-B.
5. AT CONCRETE WALL SEE 8 / S3.1-B.

TYPICAL HOLDOWN REQUIREMENTS

7 SECTION

1" = 1'-0" 7 / S2.1

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WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER

707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
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FOUNDATION DETAILS

S2.1

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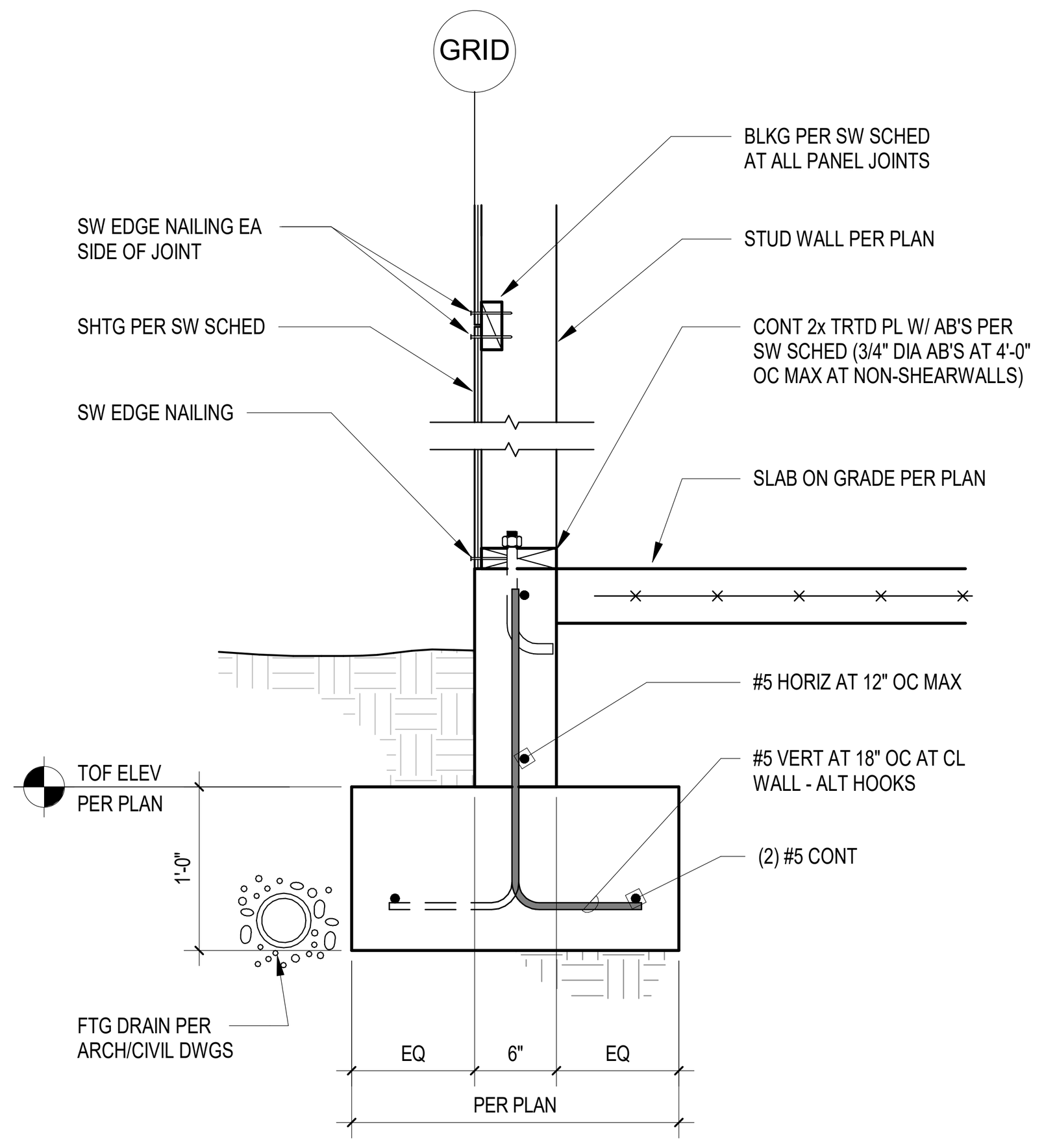

1 SECTION

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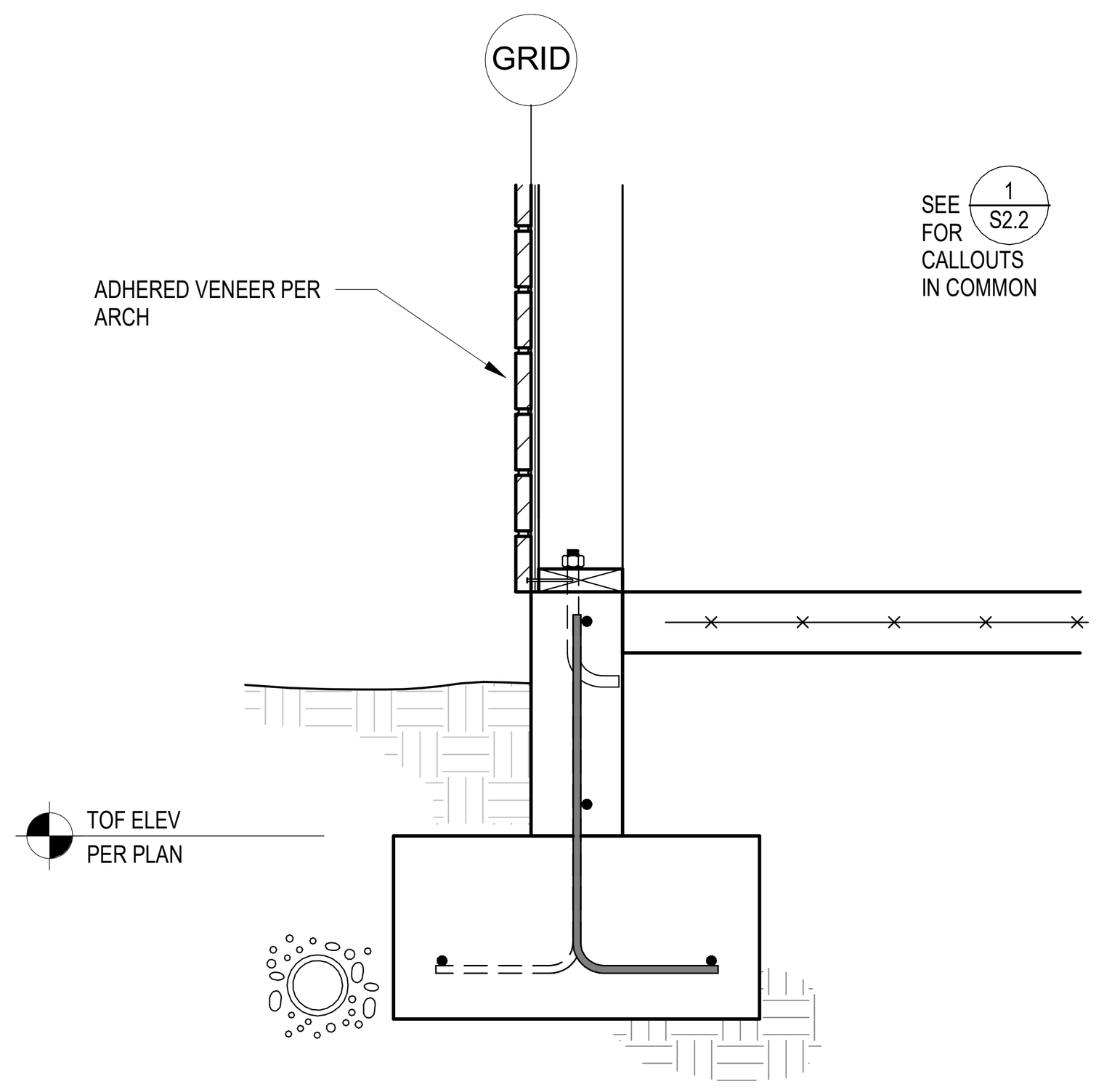

2 SECTION

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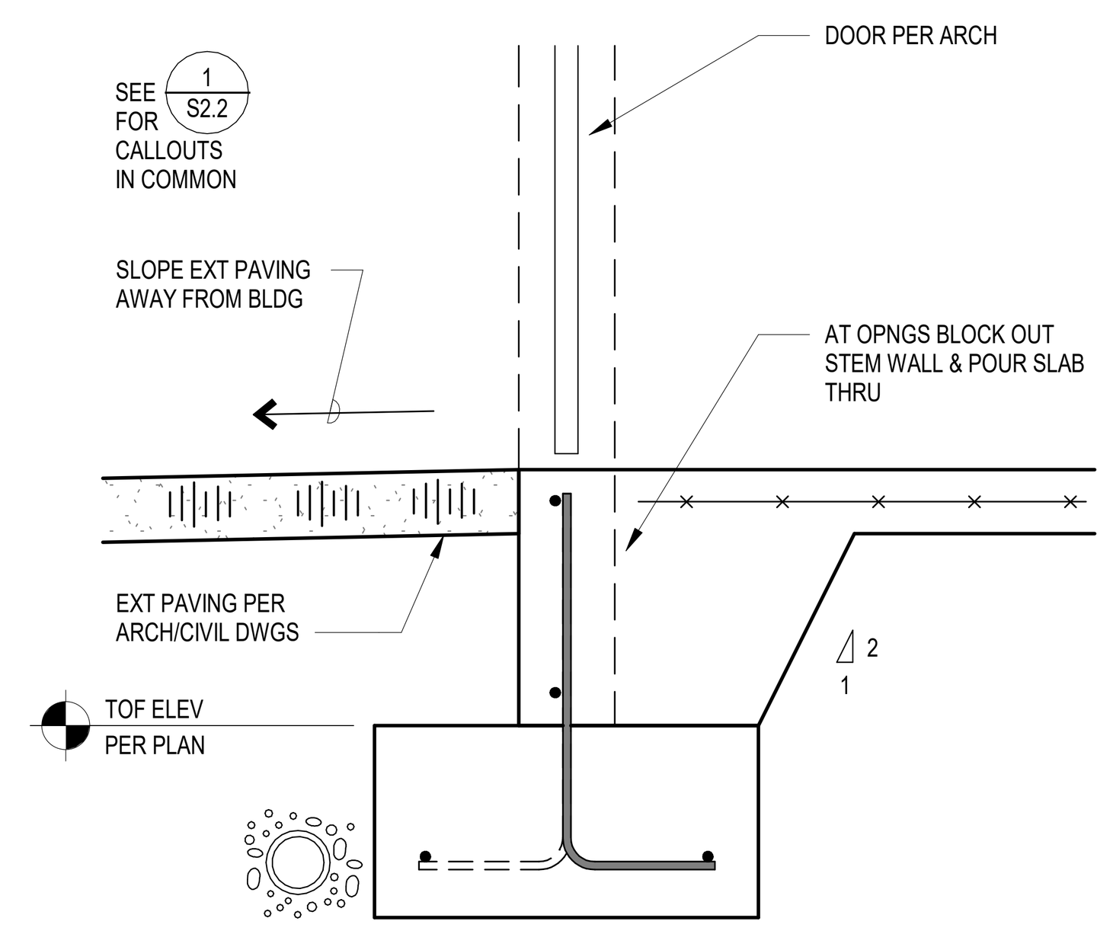

3 SECTION

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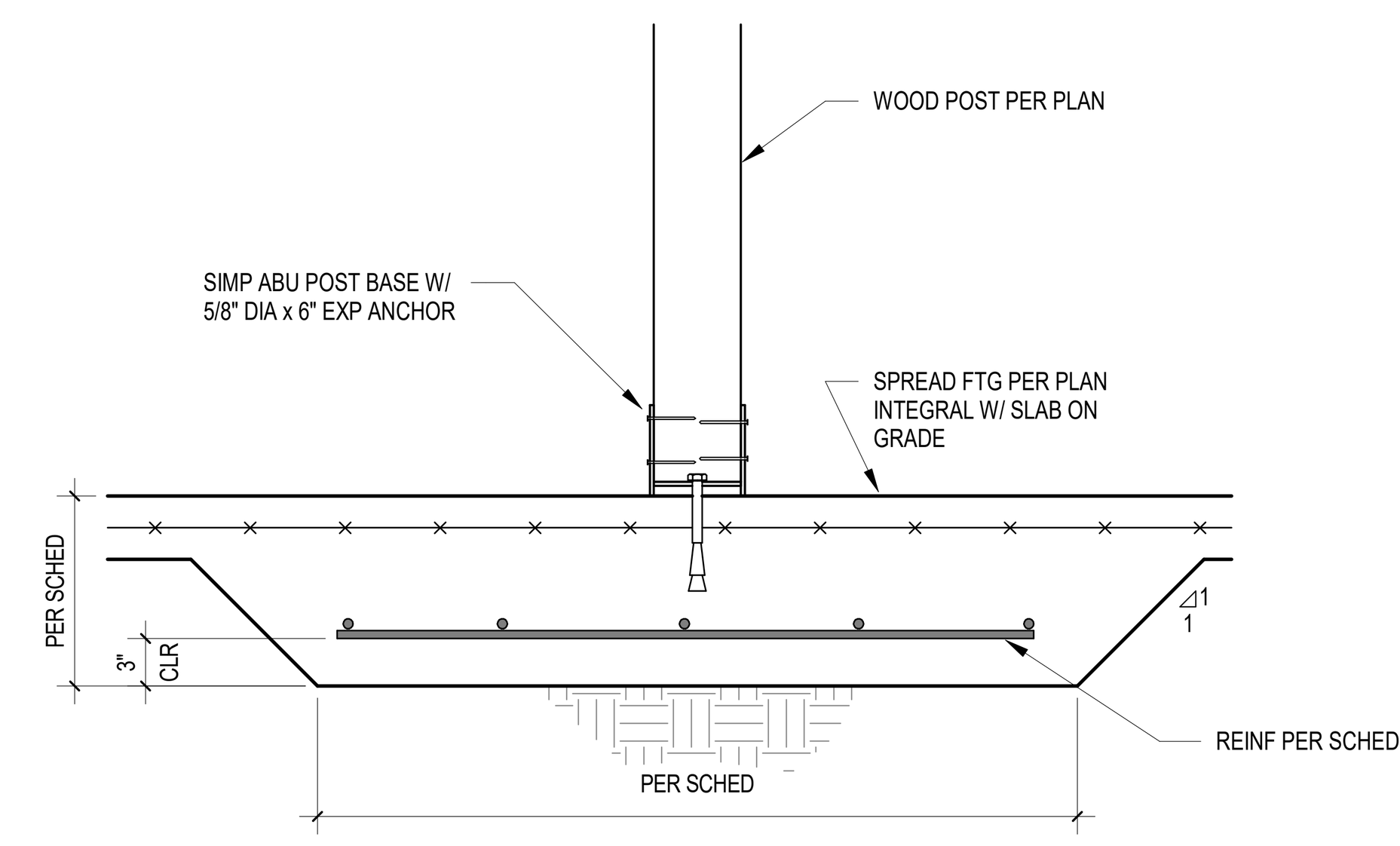
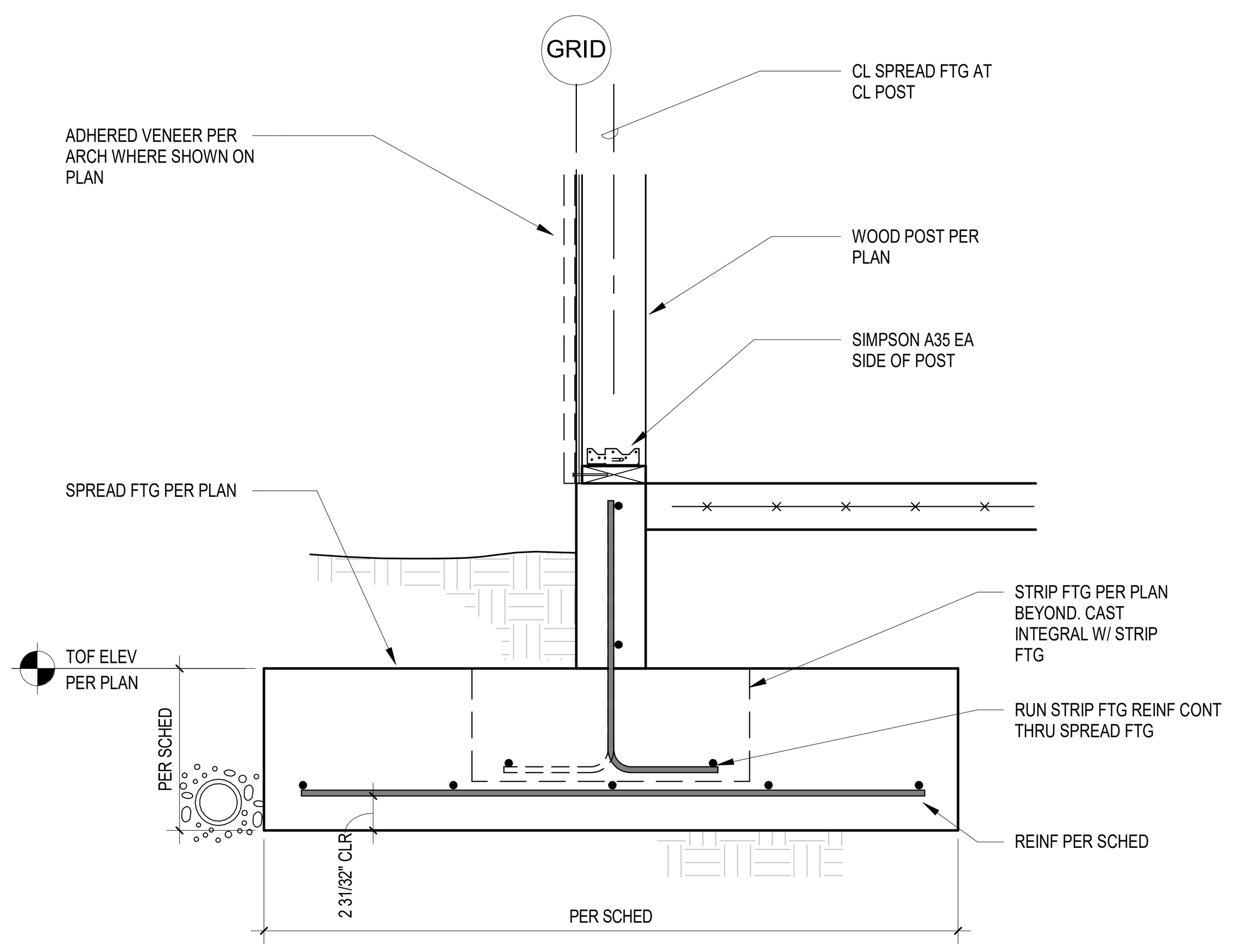


4 SECTION

1" = 1'-0" 4 / S2.2



5 SECTION

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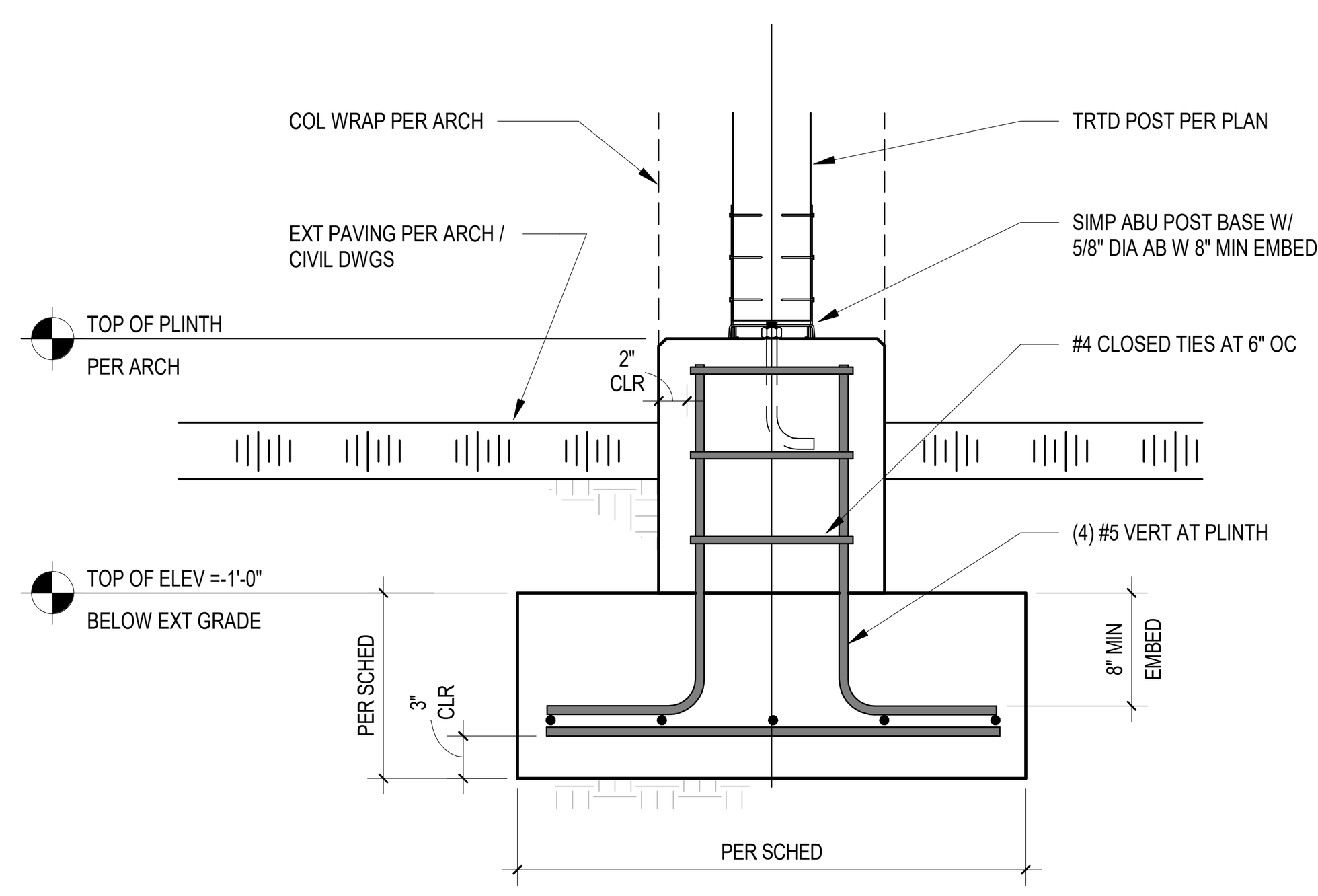

6 SECTION

1" = 1'-0" 6 / S2.2

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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
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PHASE 2 - CARE CENTER

FOUNDATION DETAILS

S2.2

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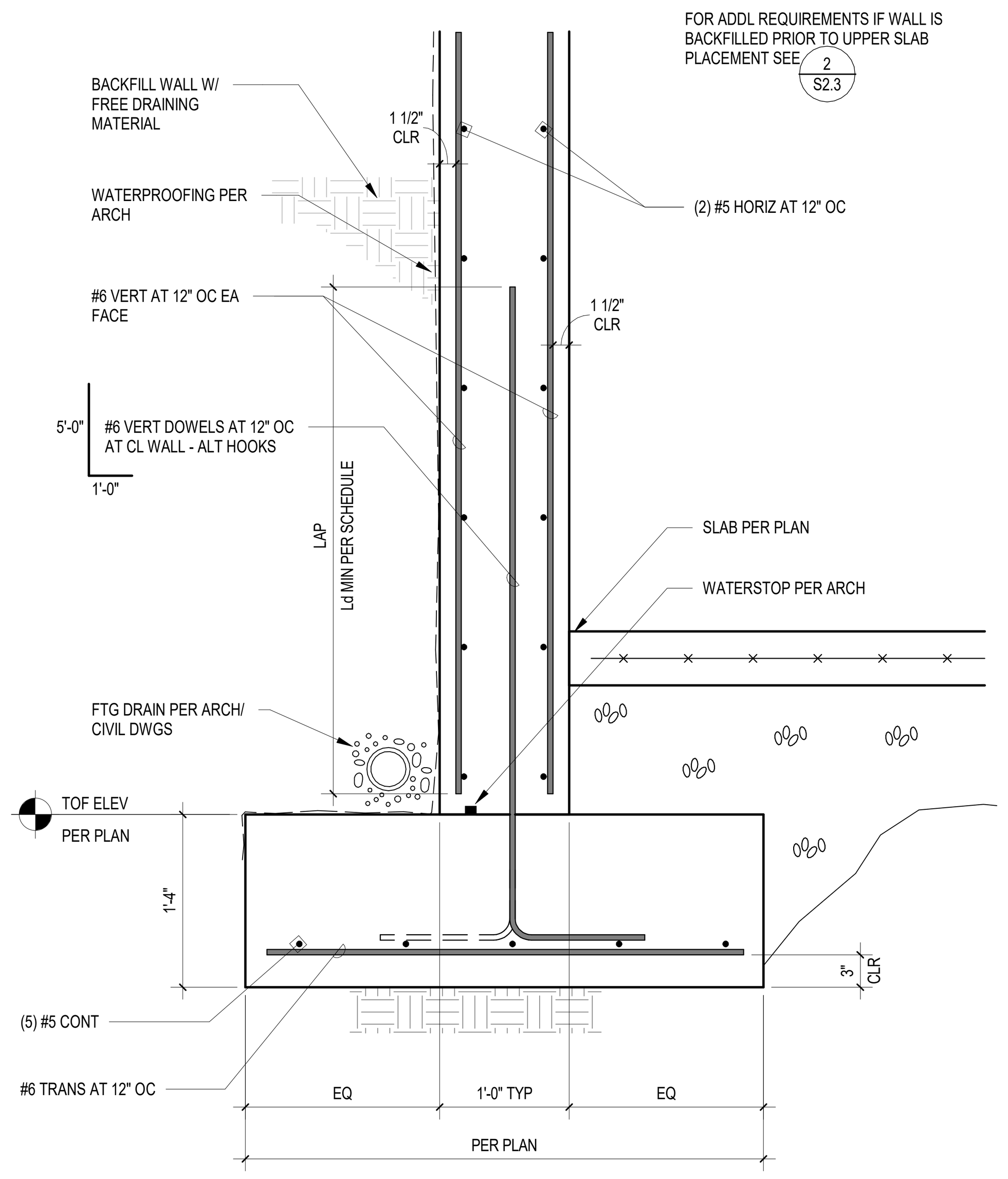

1 SECTION
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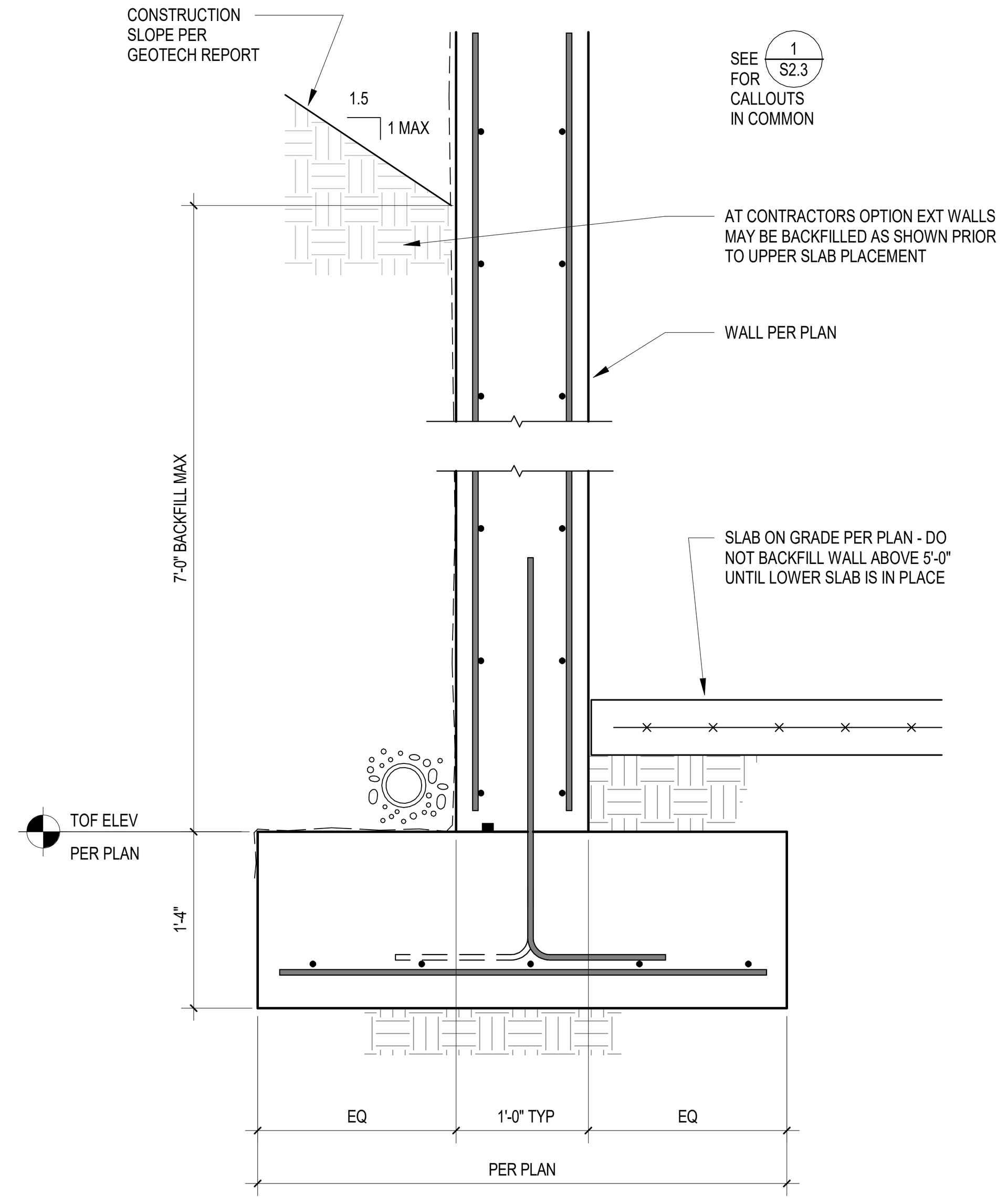

2 SECTION
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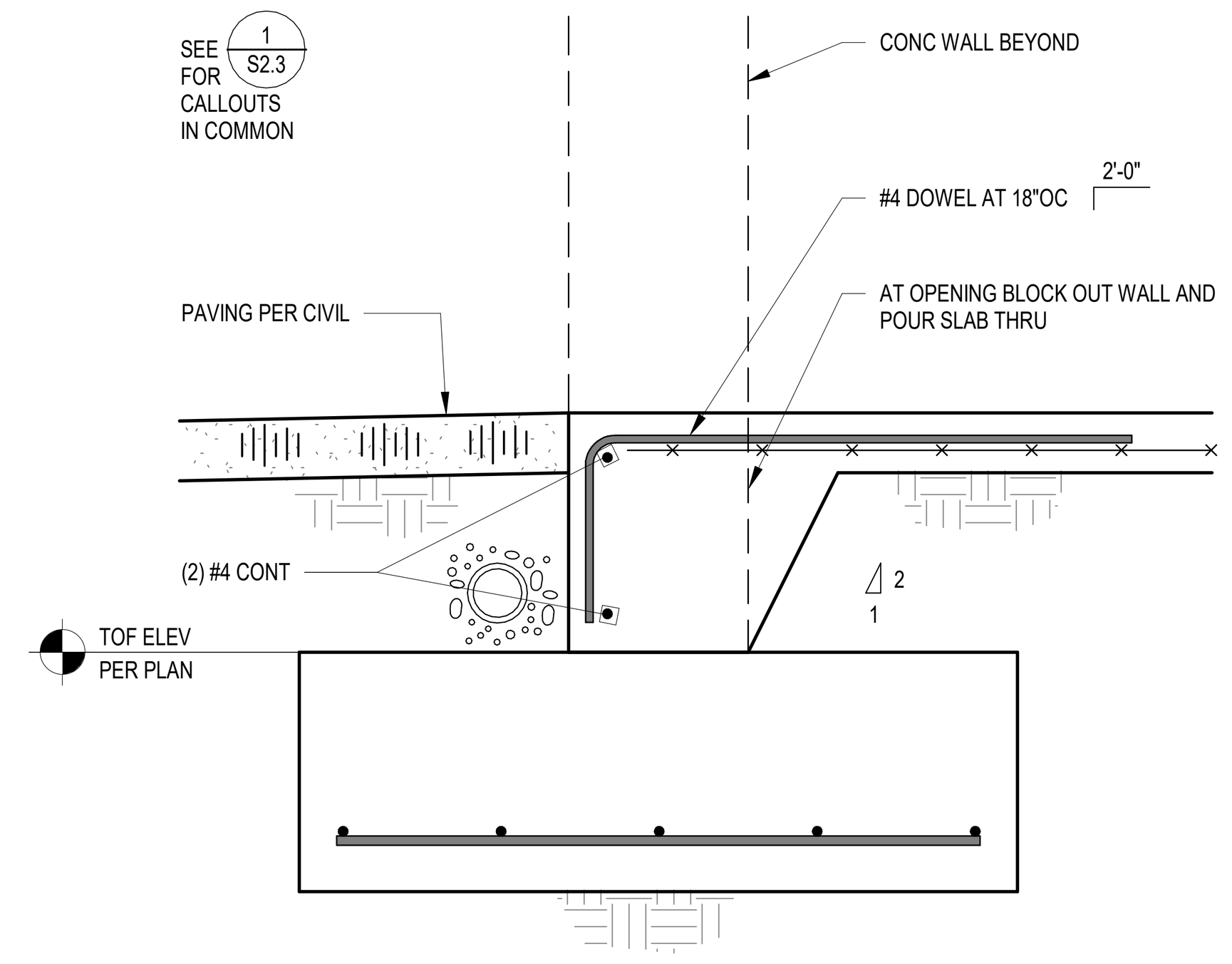

3 SECTION
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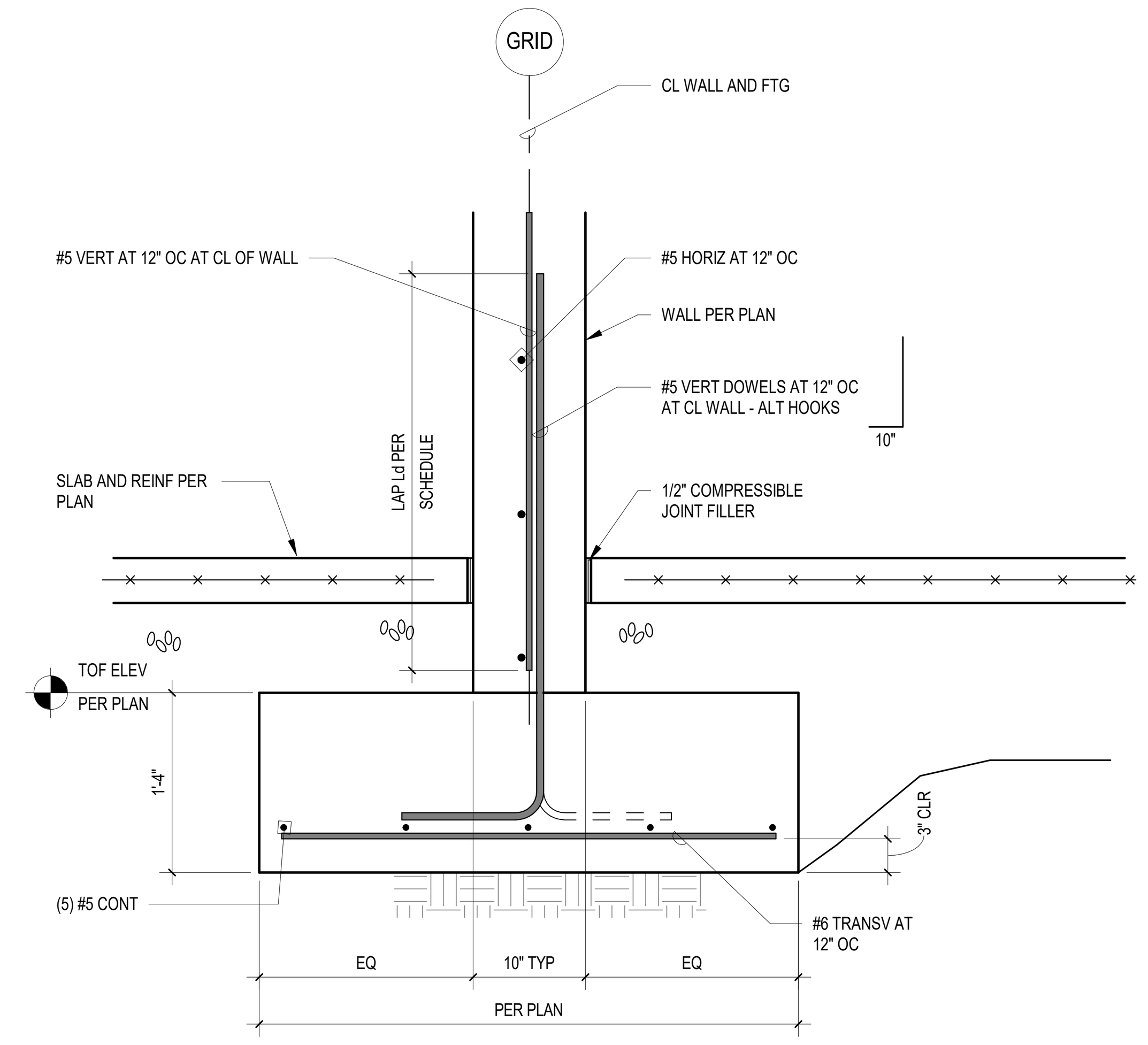

4 SECTION
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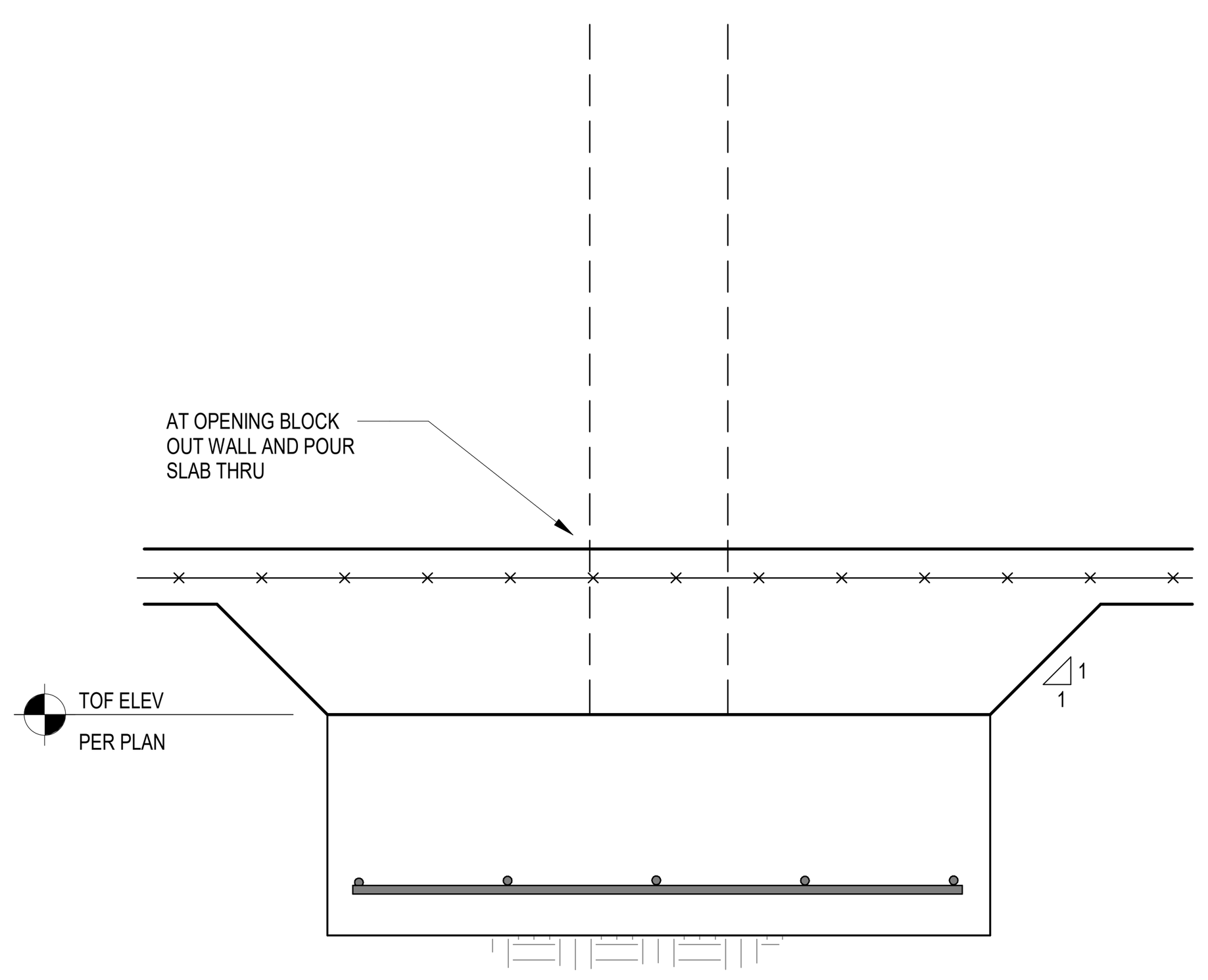

5 SECTION
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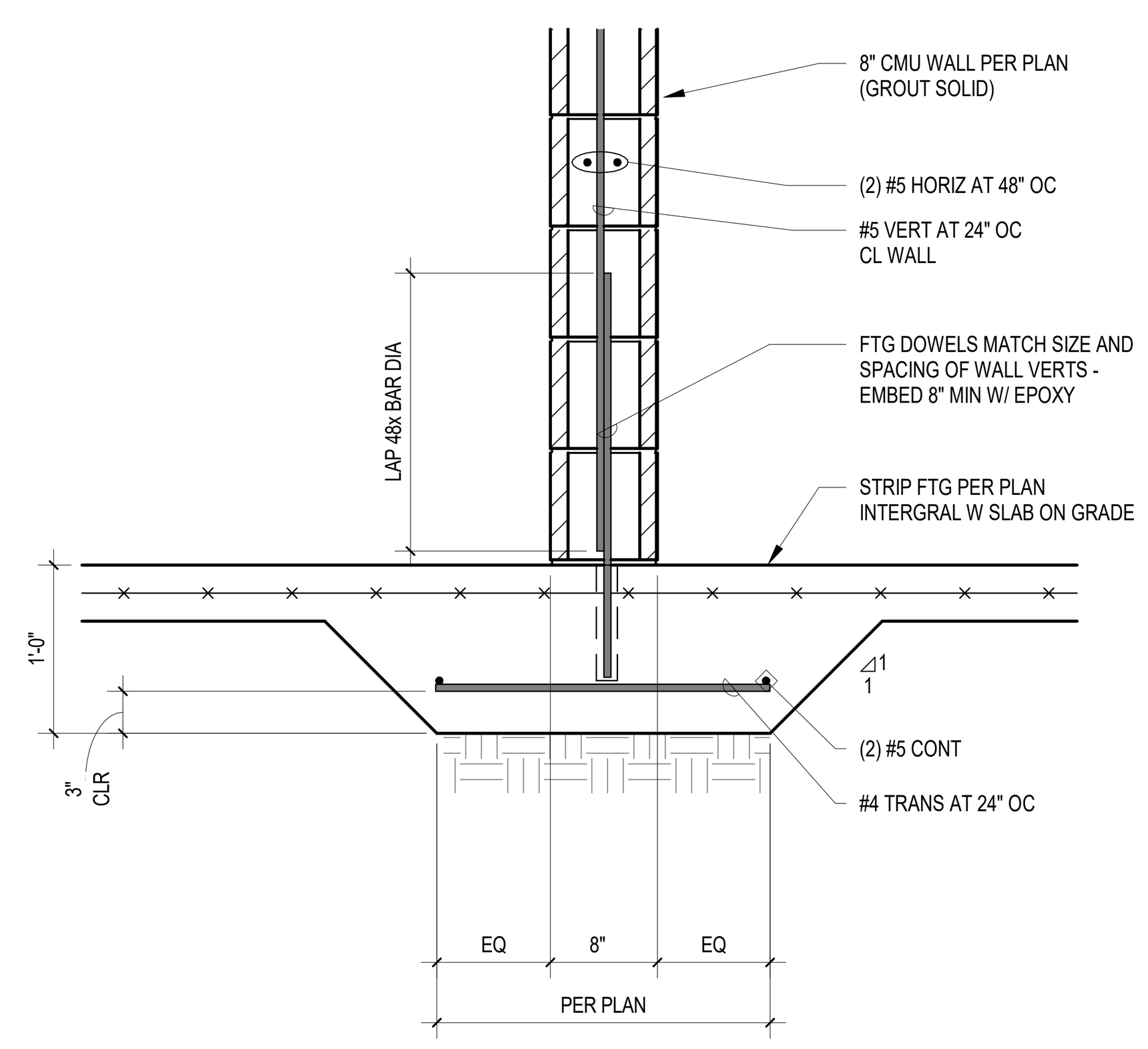

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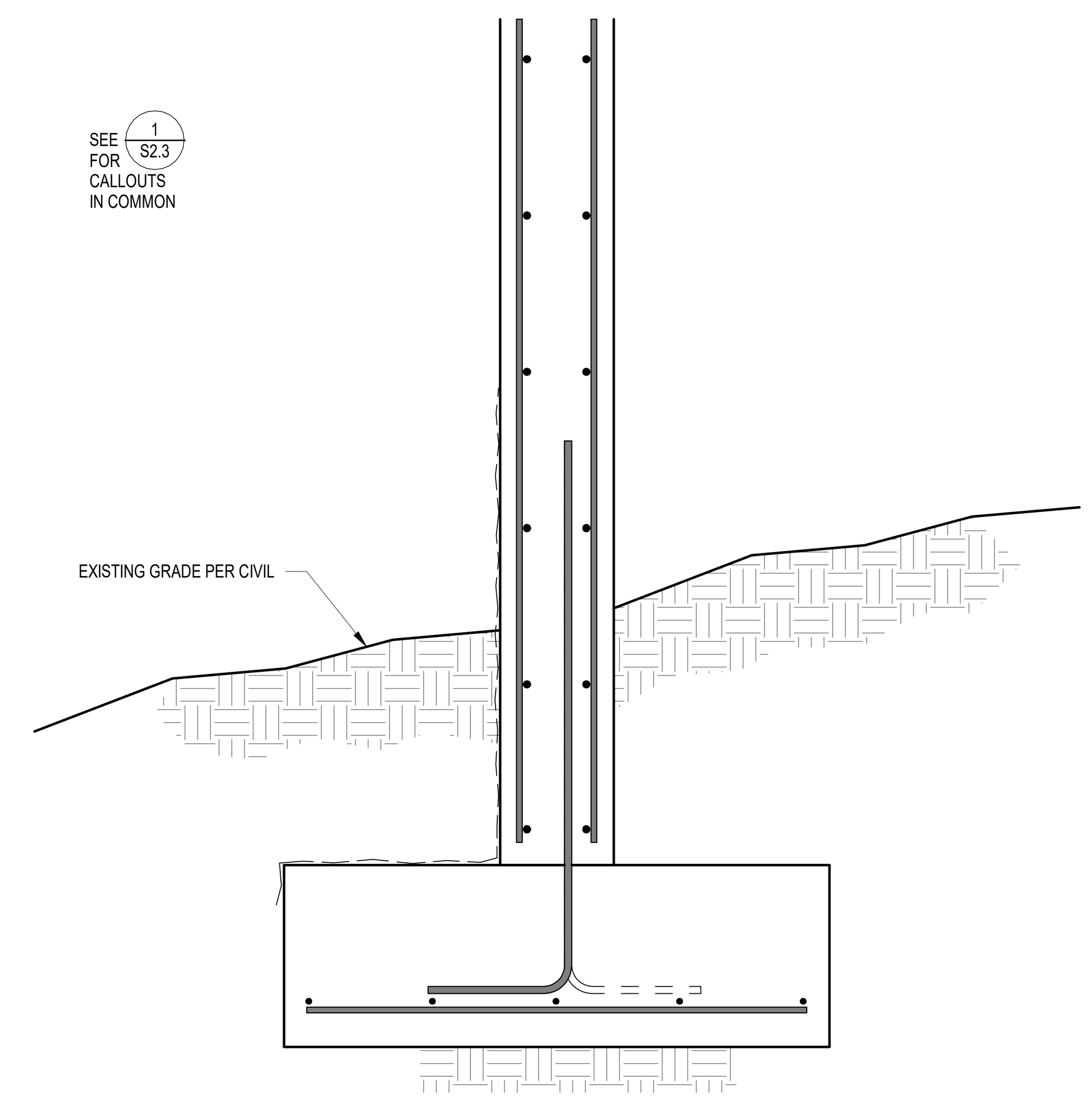

7 TYPICAL
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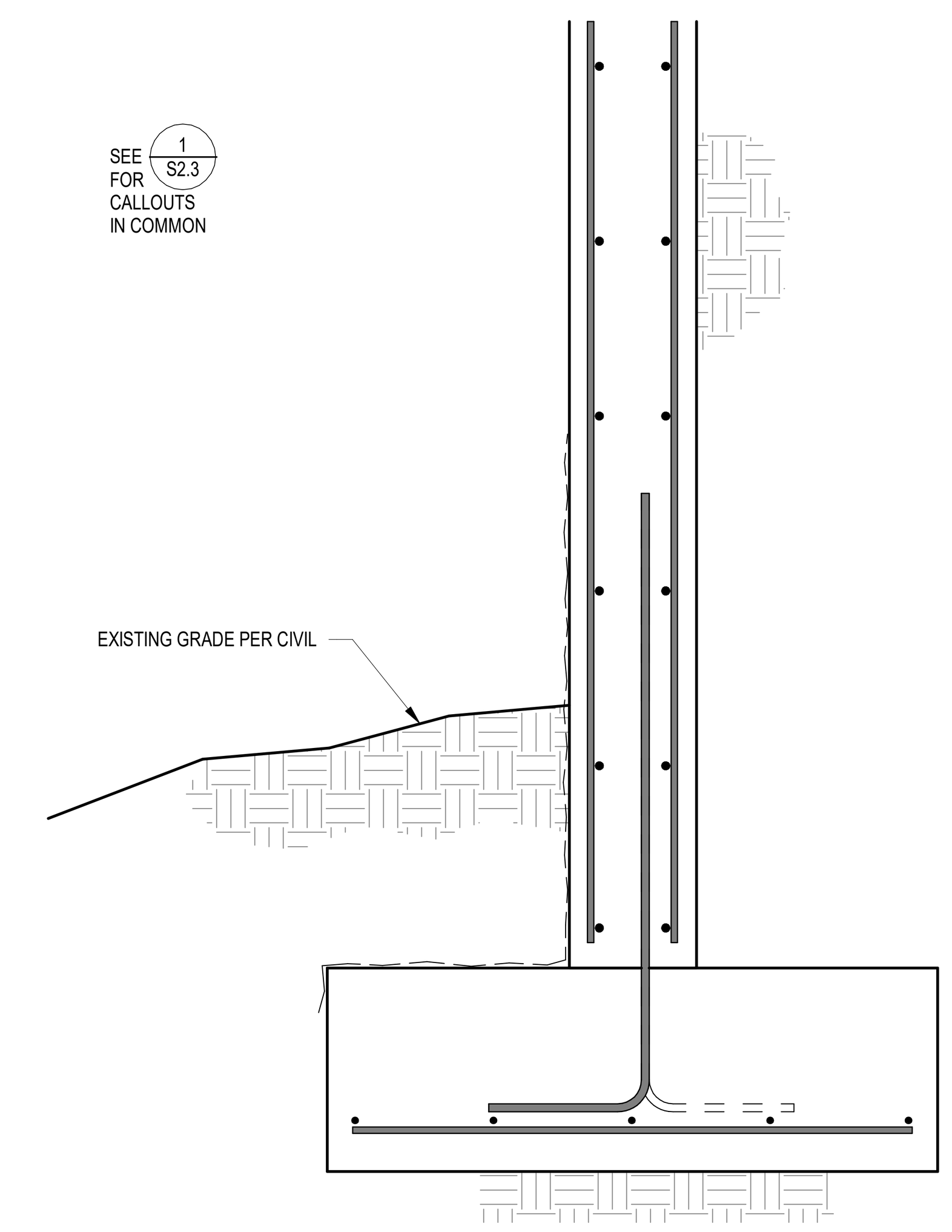

8 SECTION
1" = 1'-0" 8 / S2.3

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ANDREW DEAN McEACHERN — STATE OF WASHINGTON — STRUCTURAL — PROFESSIONAL ENGINEER

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PERMIT SET
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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

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S2.3

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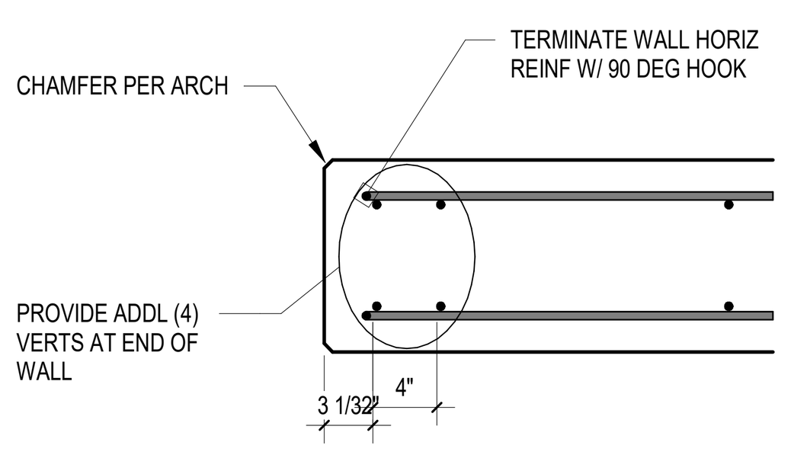
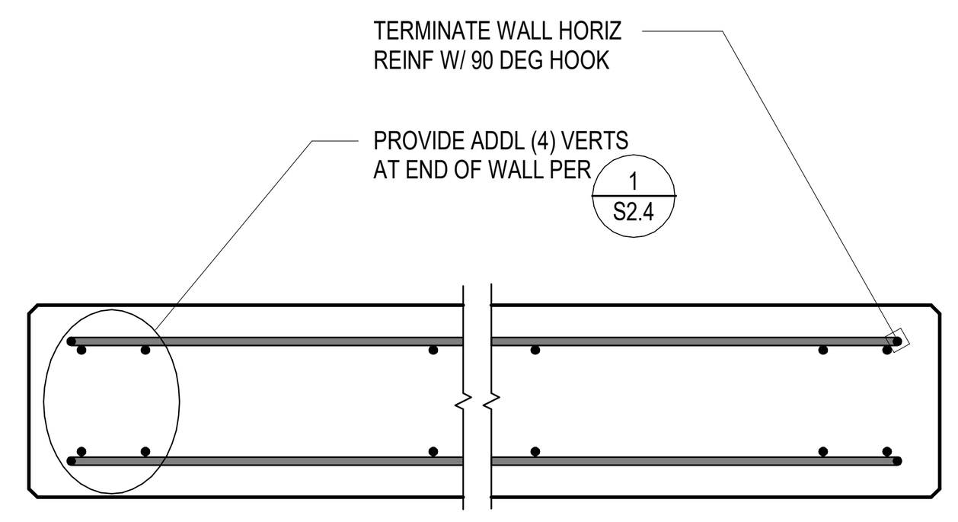
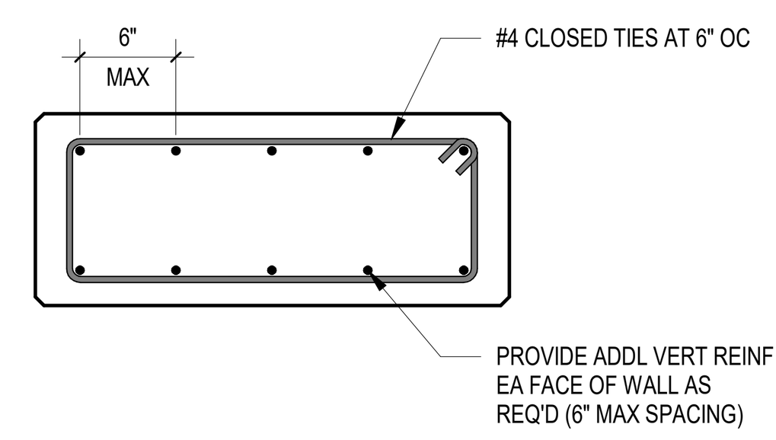


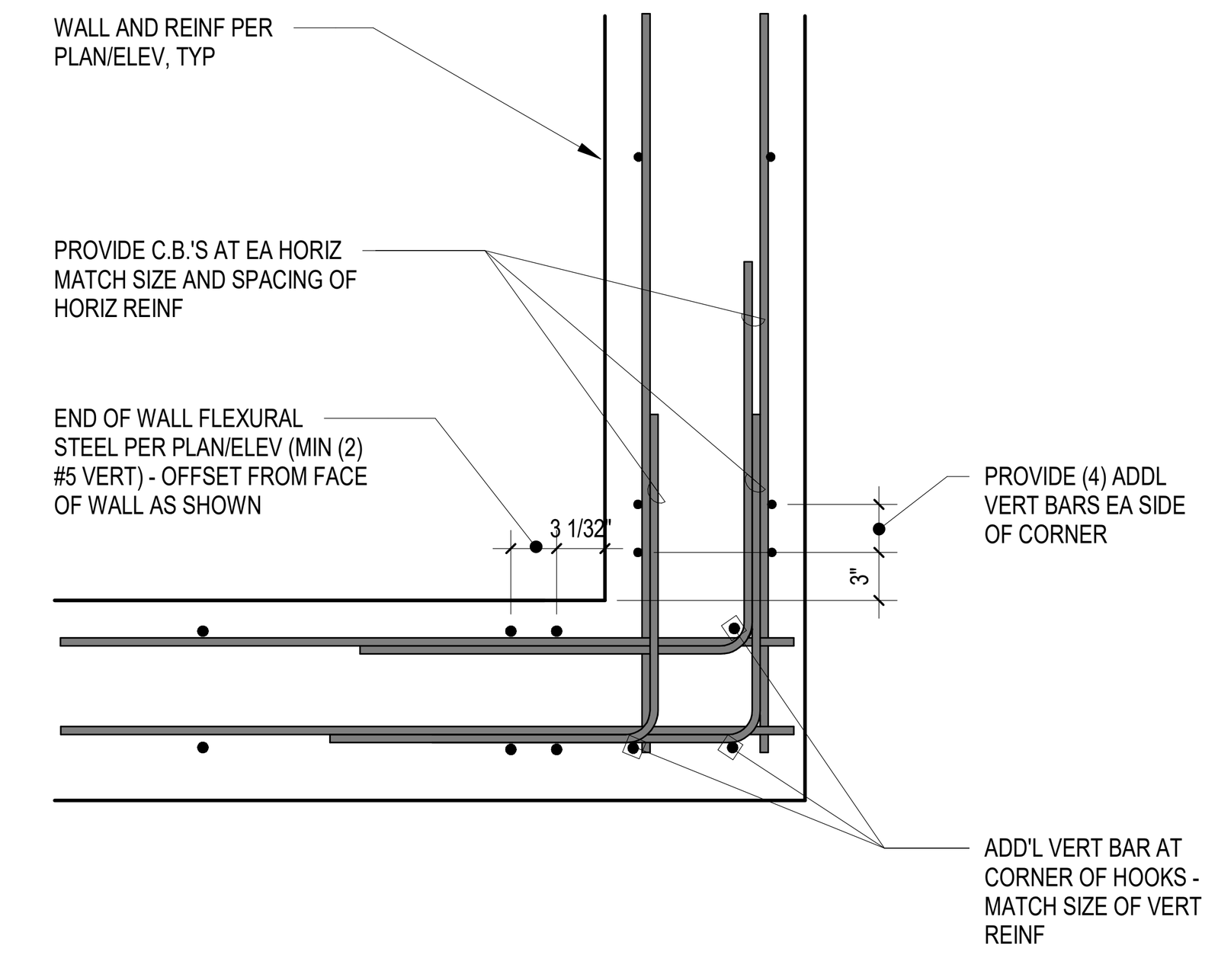
TYPICAL ENDWALL CONDITION



WALL PIERS 6'-0" OR LESS



WALL PIERS 2'-0" OR LESS

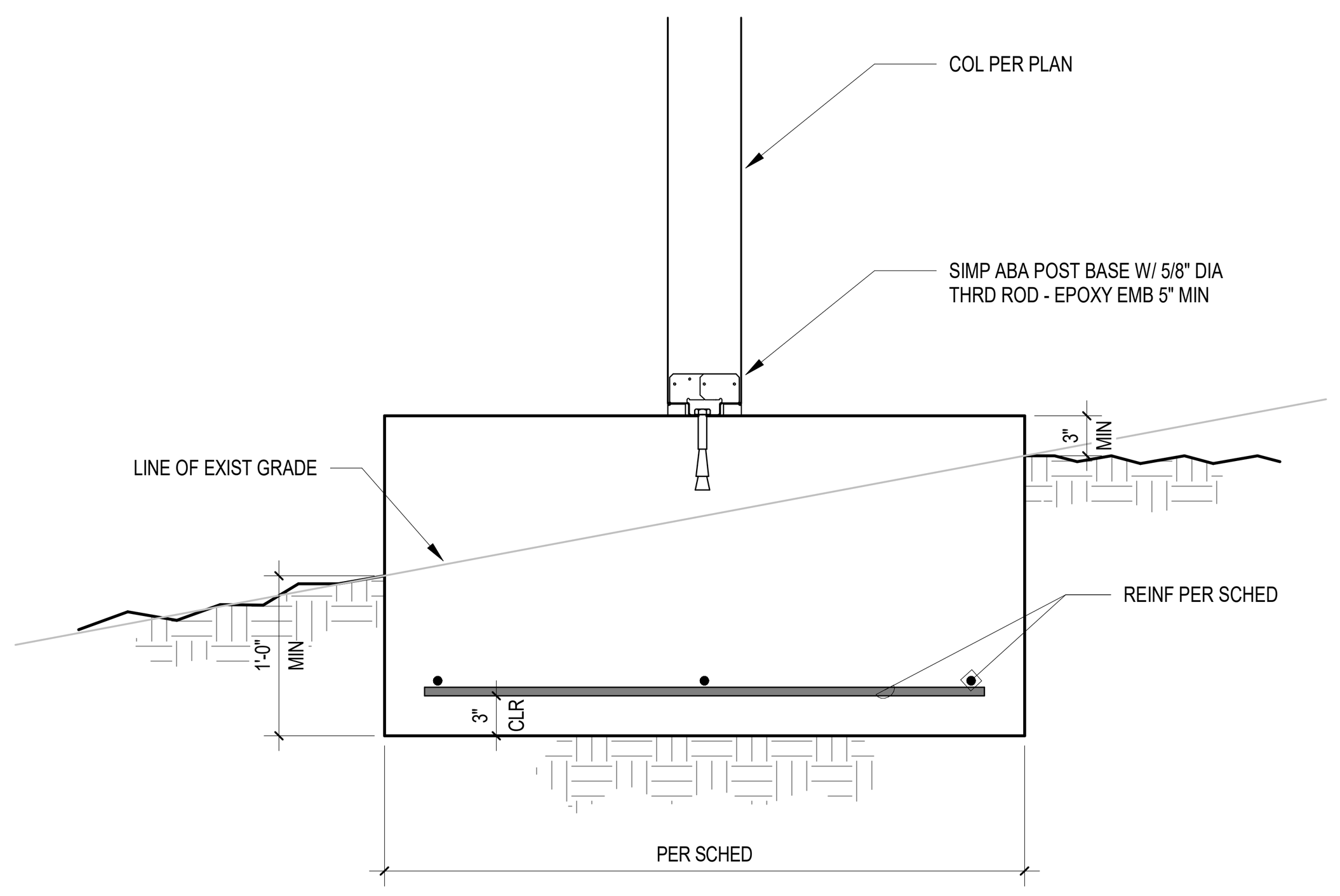


1 SECTION
1" = 1'-0" 1 / S2.4

2 SECTION
1" = 1'-0" 2 / S2.4

3 SECTION
1" = 1'-0" 3 / S2.4

4 SECTION
1" = 1'-0" 4 / S2.4



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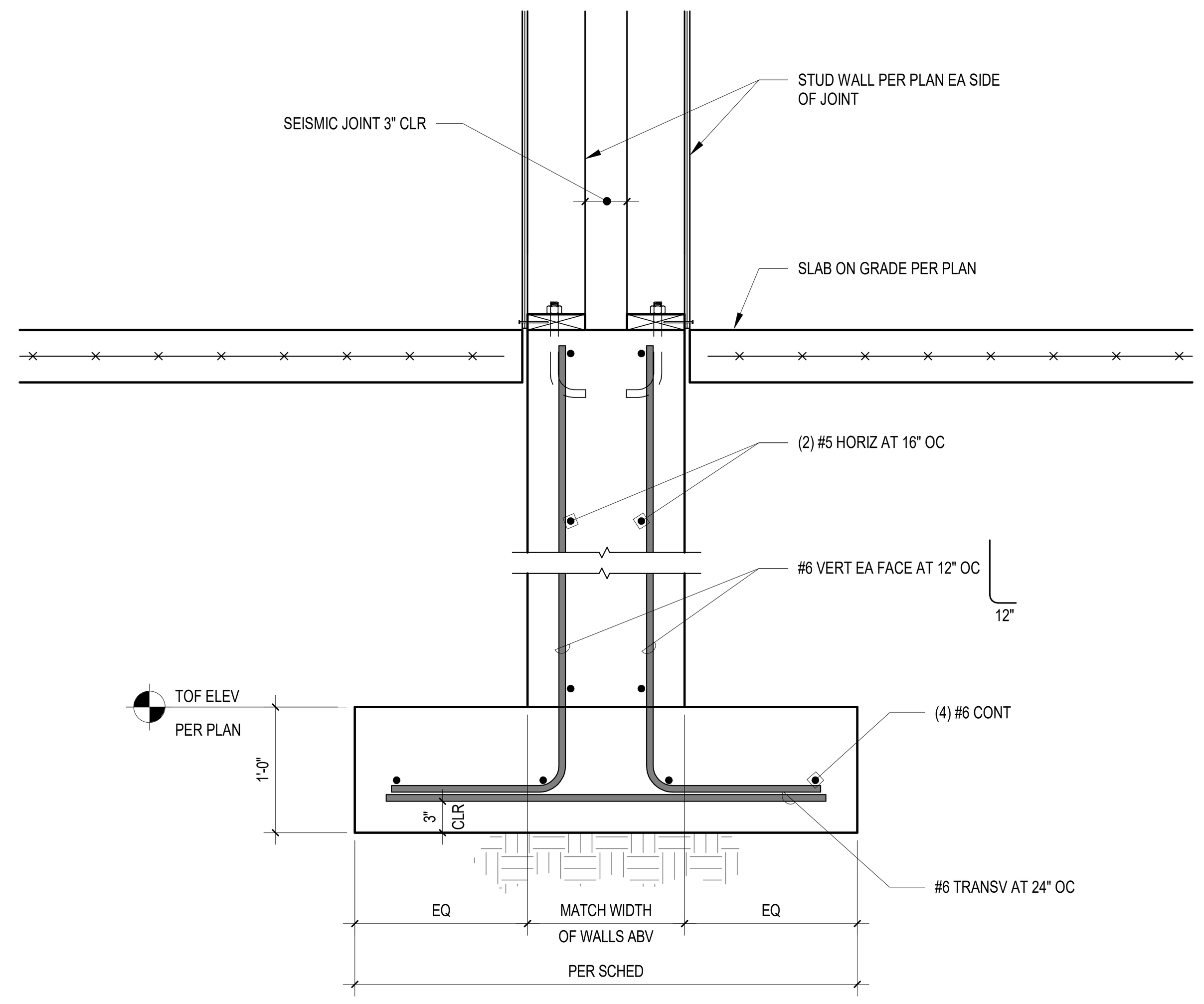

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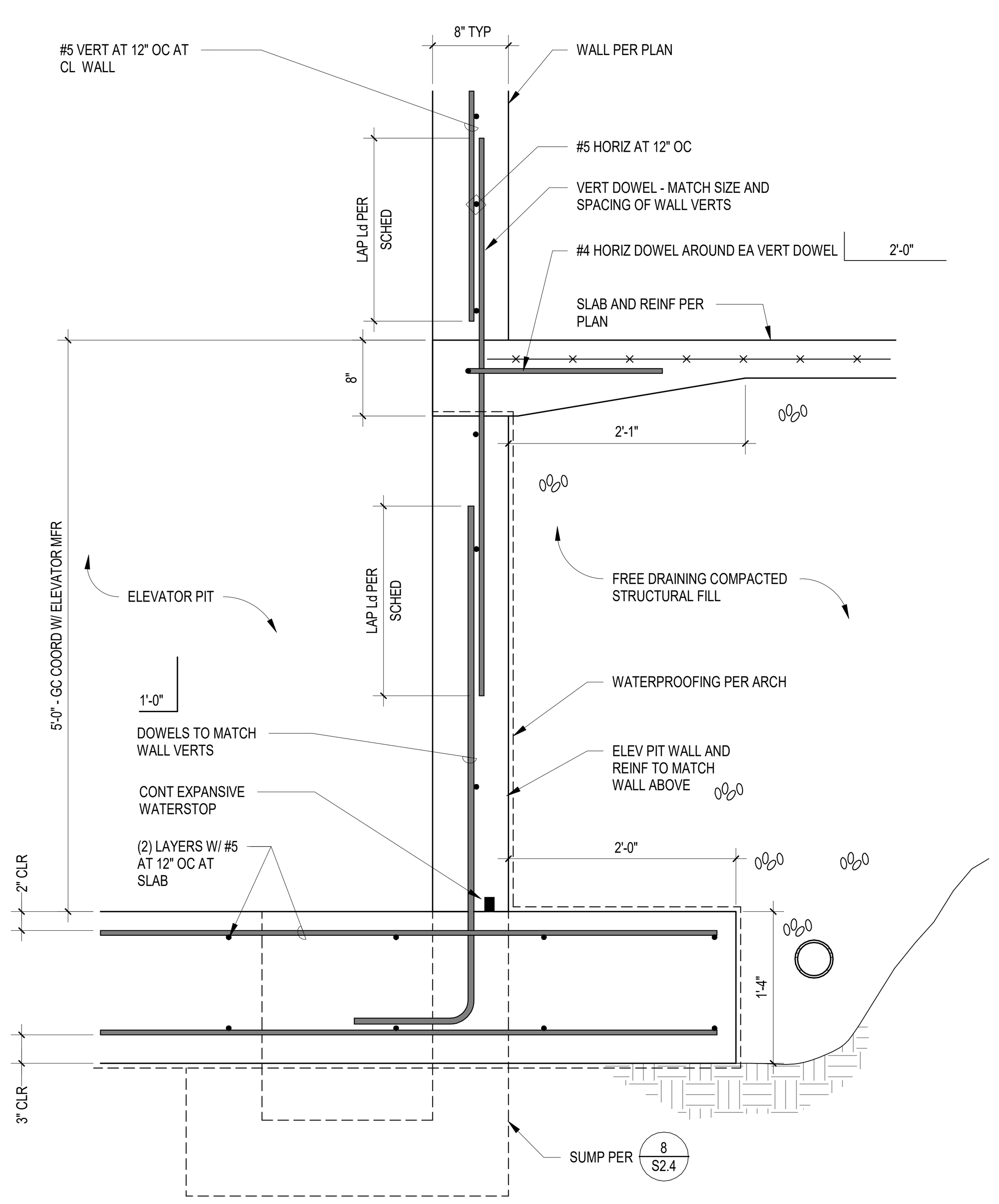

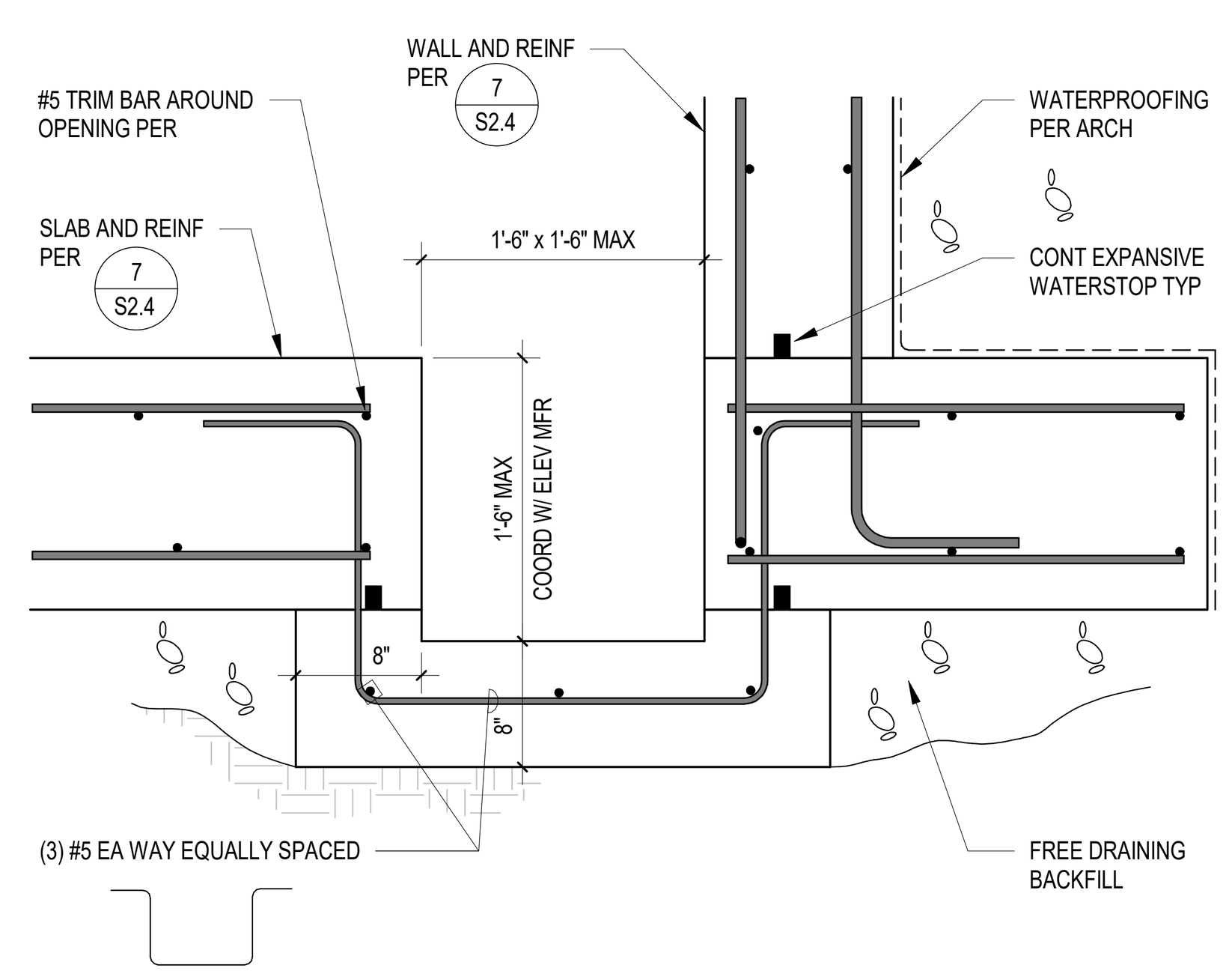

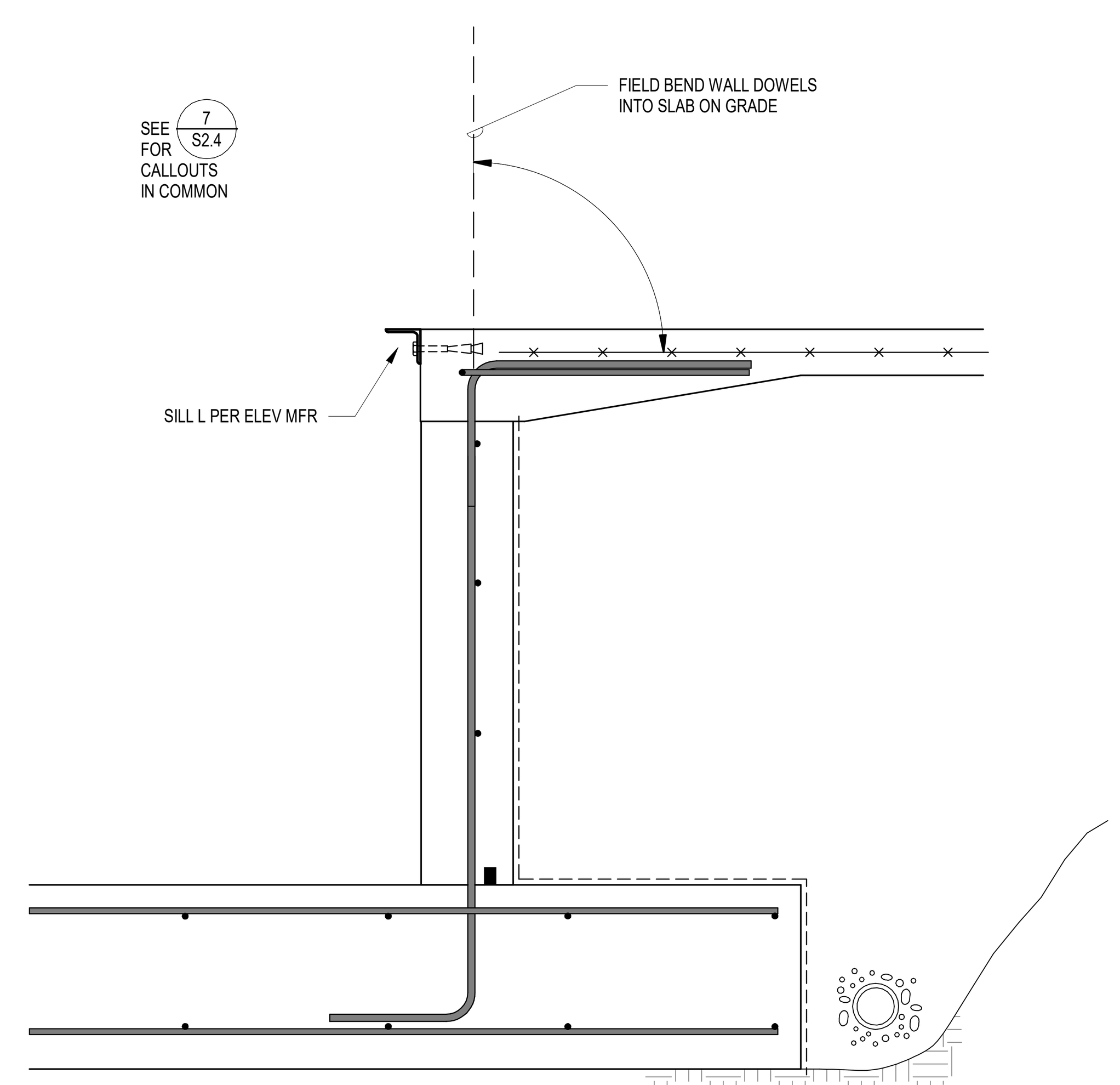

7 SECTION
1" = 1'-0" S2.4-7

8 SECTION
1" = 1'-0" S2.4-8

9 SECTION
1" = 1'-0" S2.4-9

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ANDREW DEAN McEACHERN
STATE OF WASHINGTON
38722
STRUCTURAL
PROFESSIONAL ENGINEER

WESLEY BRADLEY PARK PHASE 2 - CARE CENTER
707 39TH AVENUE SE
PUYALLUP, WA 98374

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DECEMBER 2, 2024

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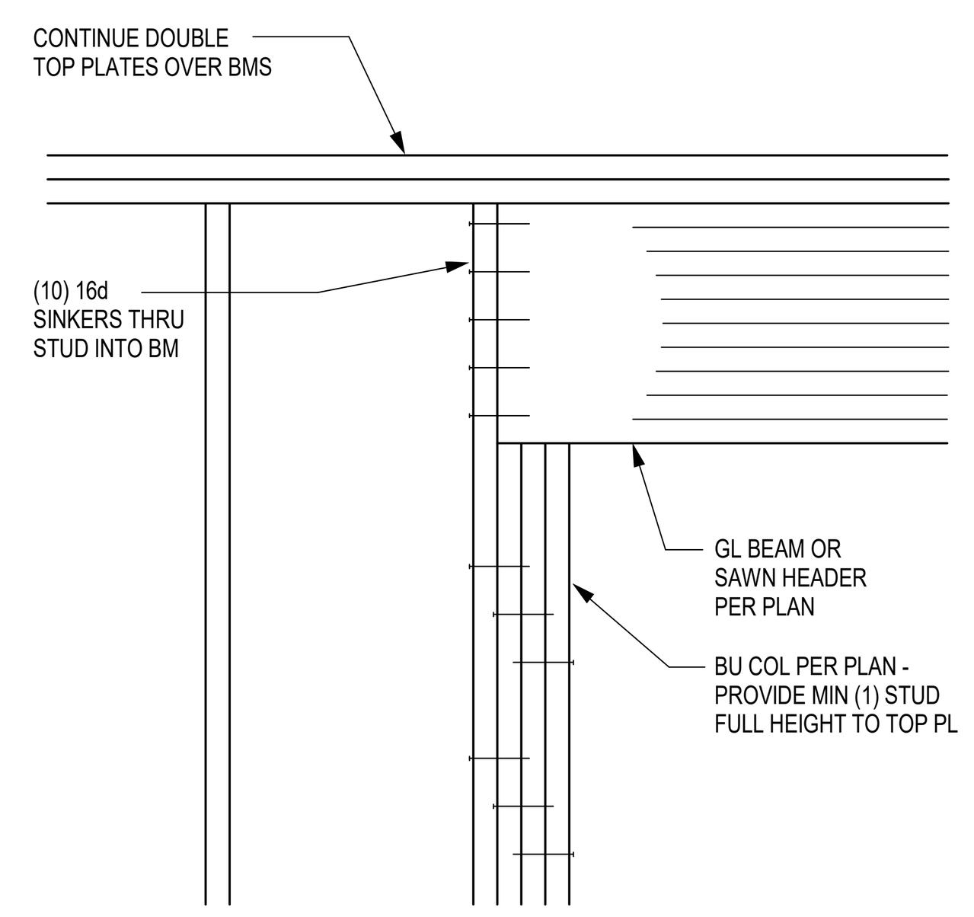
FOUNDATION DETAILS

S2.4



1 **SECTION**
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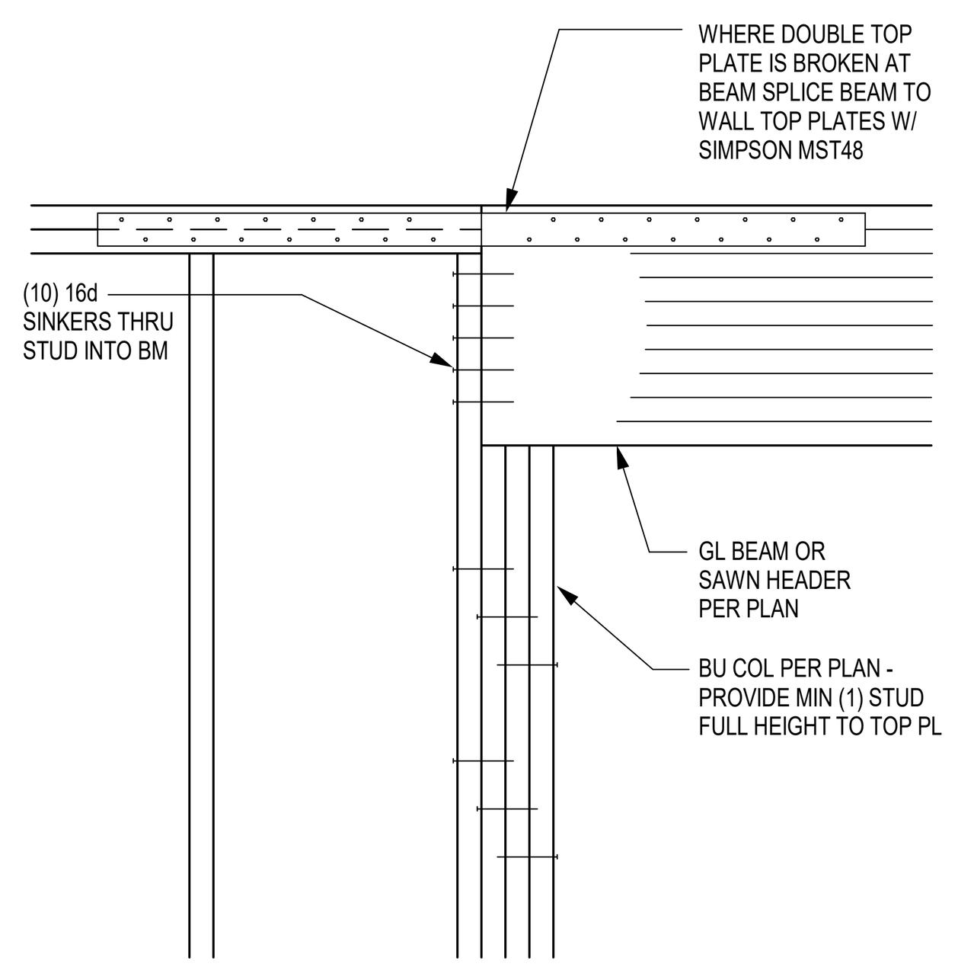
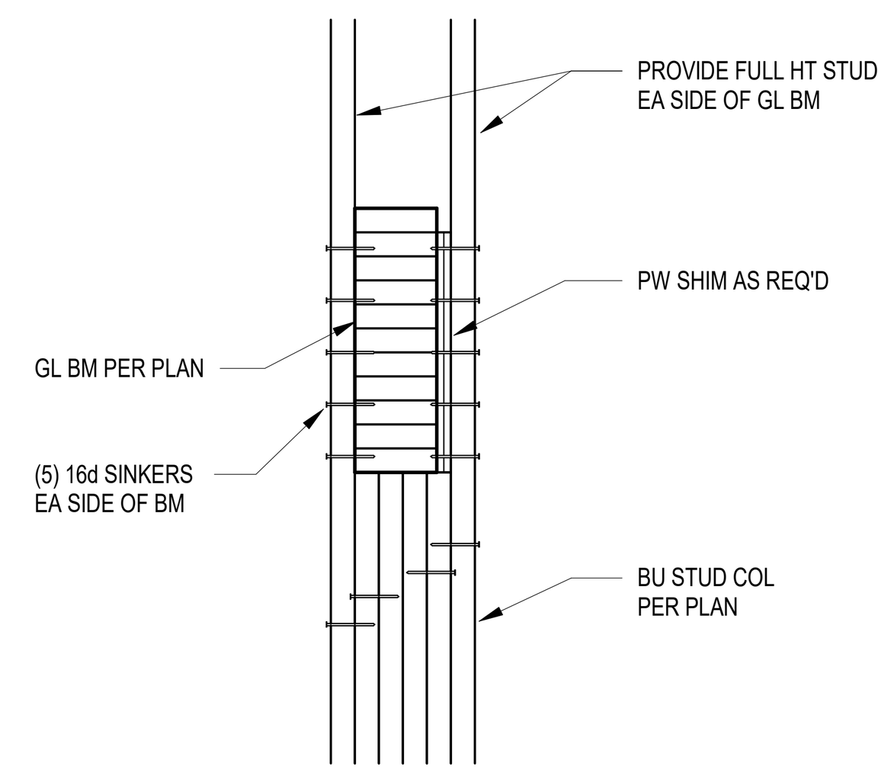


2 **SECTION**
1" = 1'-0" 2 / S3.1



3 **SECTION**
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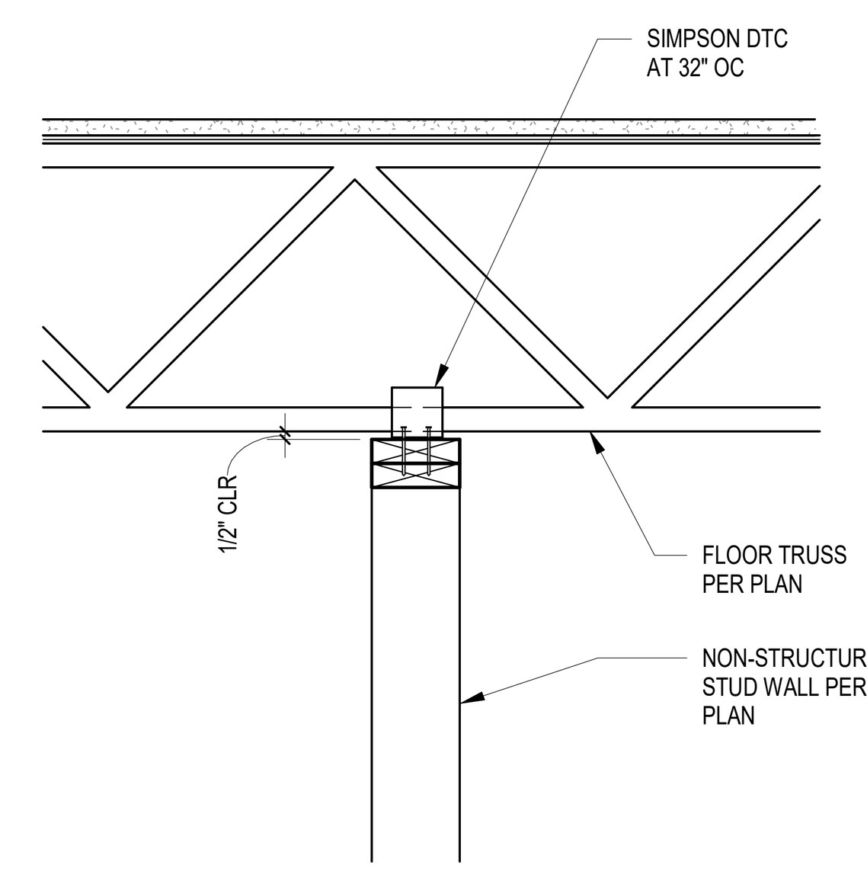
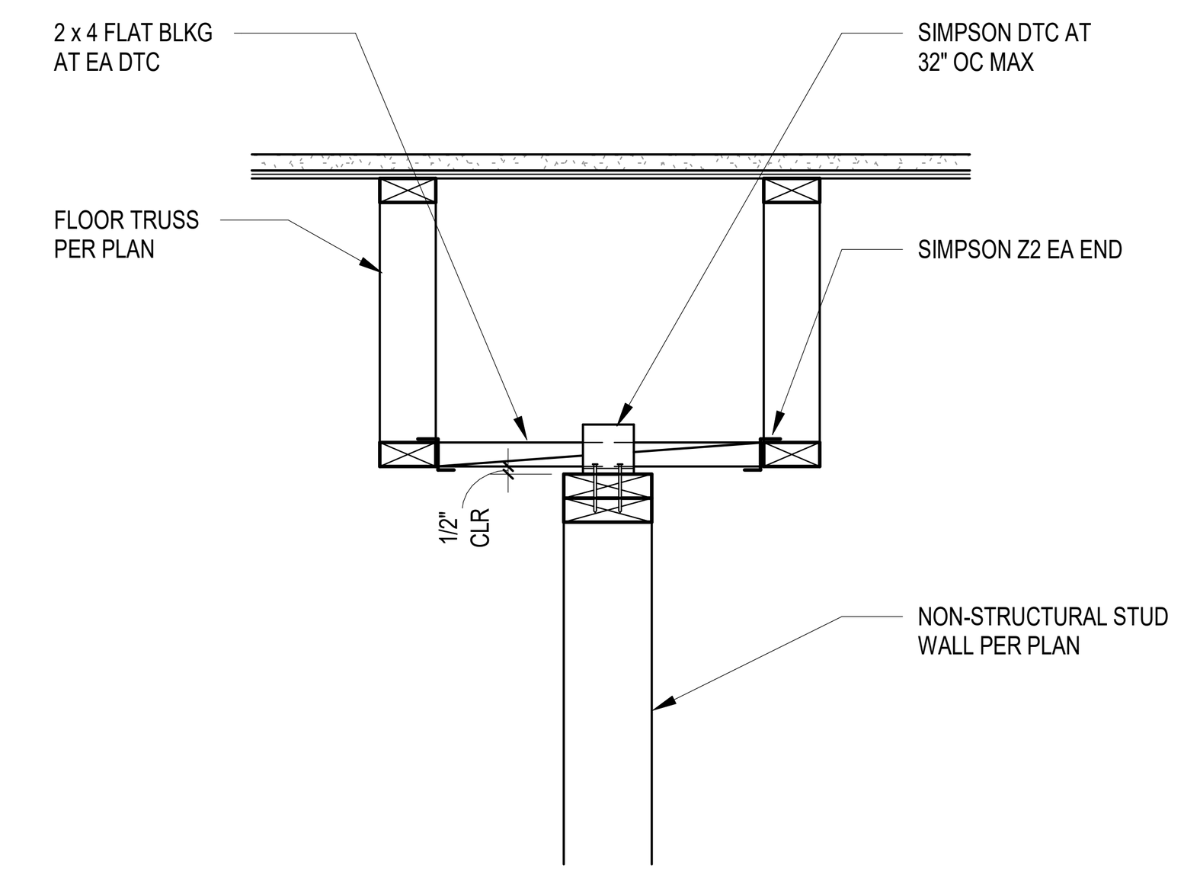


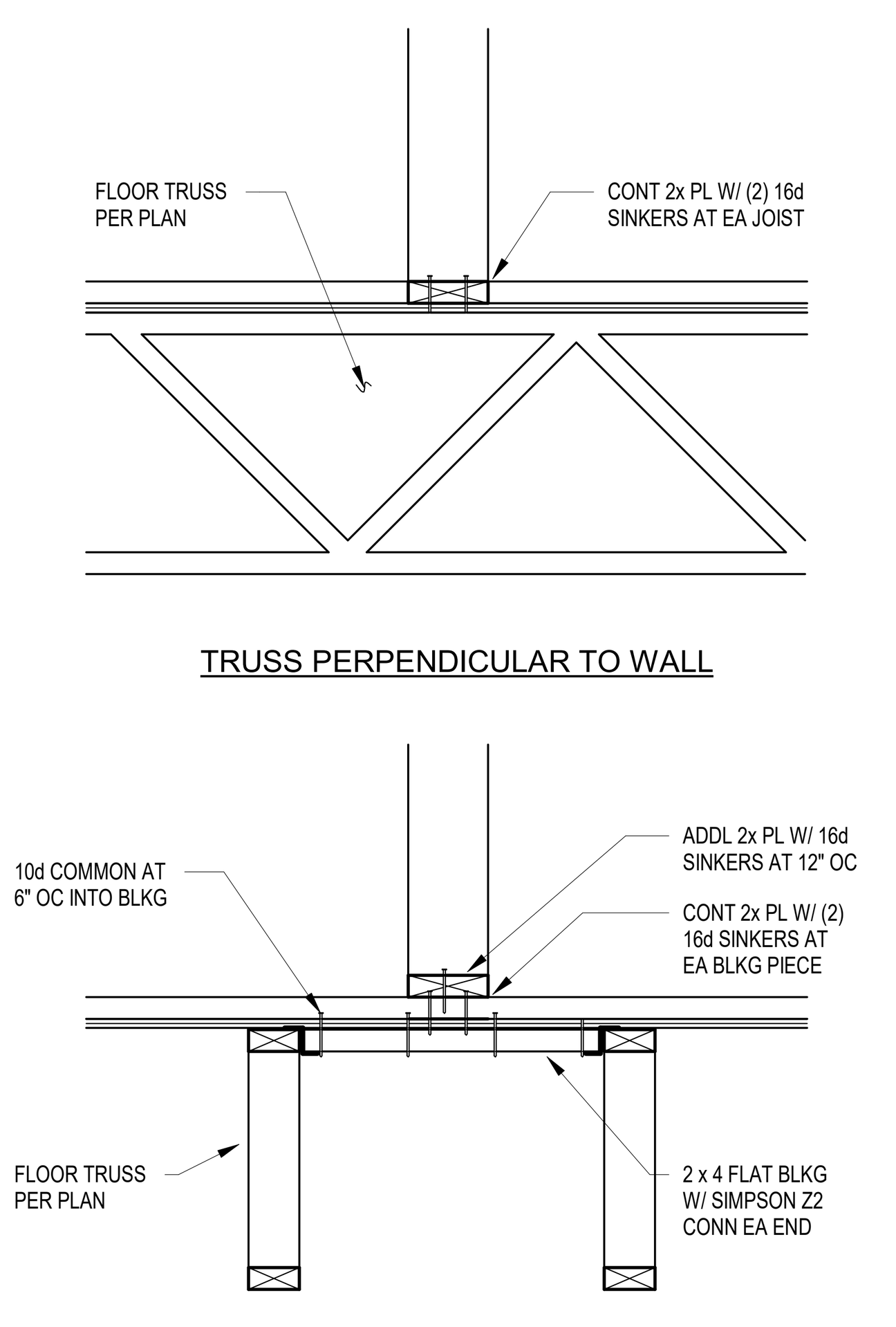
TYPICAL INTERIOR NON-STRUCTURAL WALL CONNECTION

4 **SECTION**
1" = 1'-0" 4 / S3.1



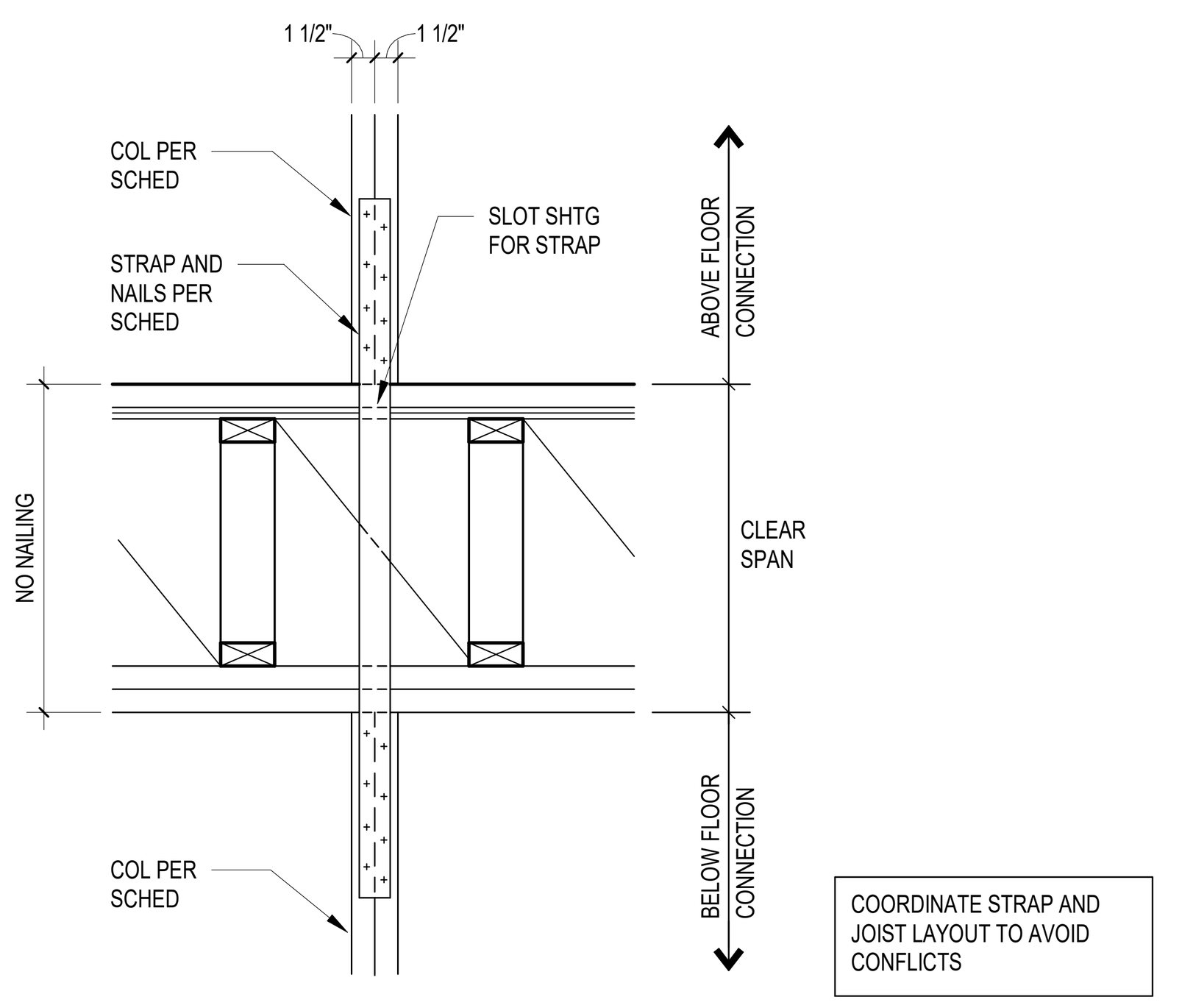
TYPICAL INTERIOR NON-STRUCTURAL WALL CONNECTION

5 **SECTION**
1" = 1'-0" 5 / S3.1



INTERIOR NON-STRUCTURAL WALL CONN AT 2ND FLOOR

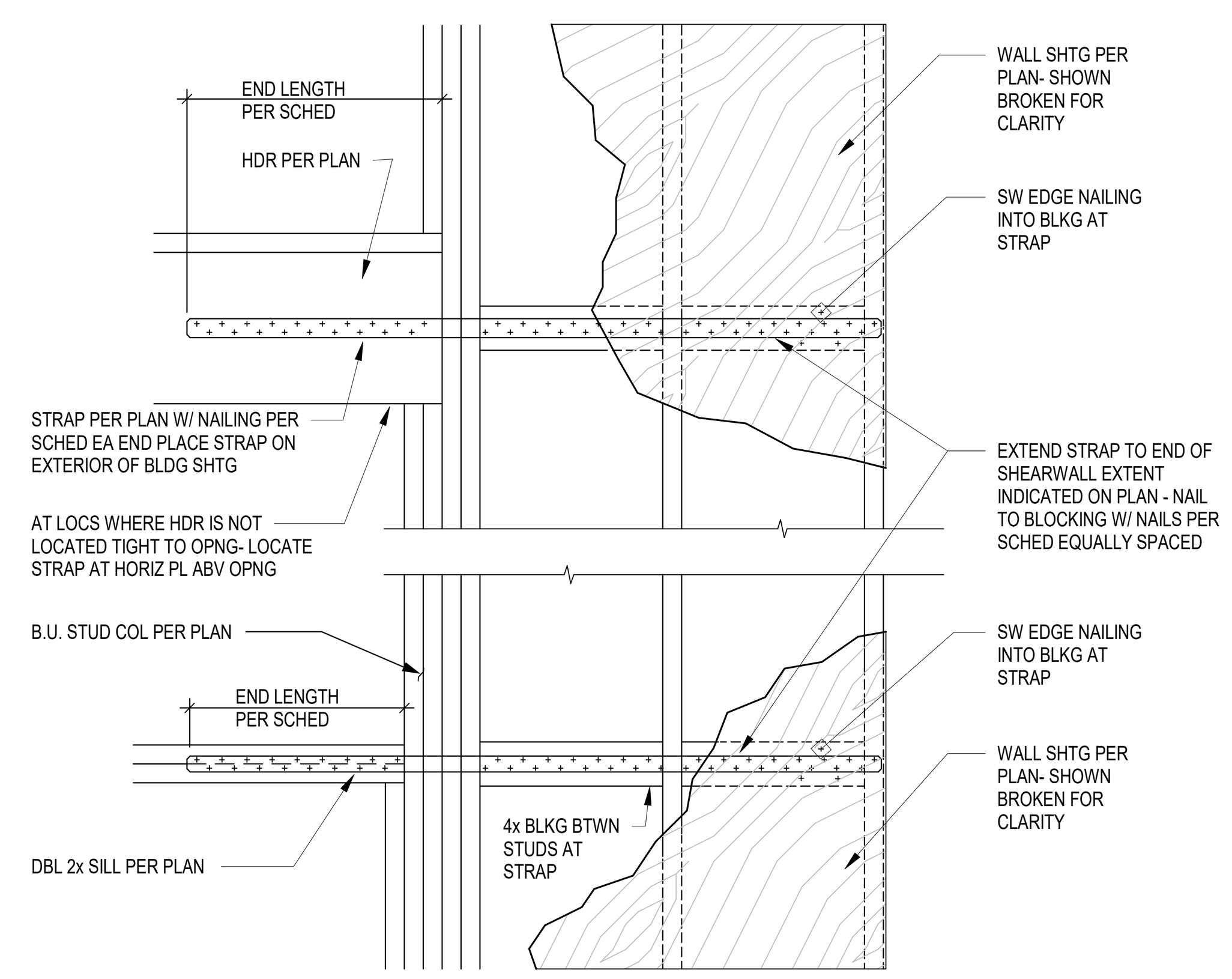
6 **SECTION**
1" = 1'-0" 6 / S3.1



STRAP ATTACHMENT DETAIL

STRAP SCHEDULE			
DESIGNATION	STRAP	CONNECTION ABOVE AND BELOW FLOOR	ALLOWABLE TENSION LOAD
M60	MST60	(17) 16d x 2 1/2" NAILS TO (2) 2x STUDS	4605#
M72	MST72	(24) 16d x 2 1/2" NAILS TO (2) 2x STUDS	6505#

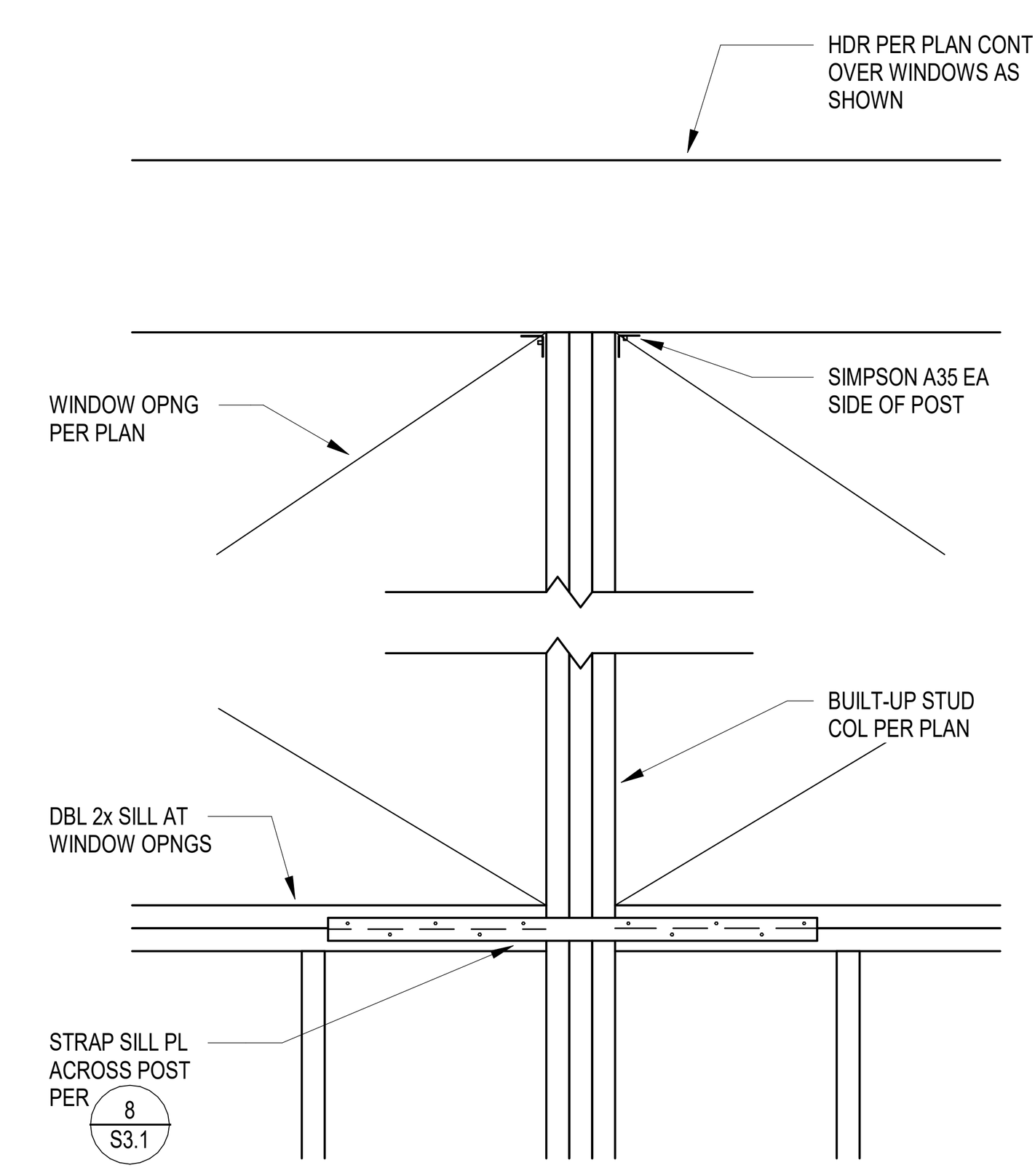
7 **SECTION**
1" = 1'-0" 7 / S3.1



STRAP DESIGNATION	END LENGTH	NAILING
CS16	13"	(11) 10d EA END

NOTE:
NAIL SPACING SHALL BE 2 1/16" OC PER ROW

8 **SECTION**
1" = 1'-0" 8 / S3.1



9 **SECTION**
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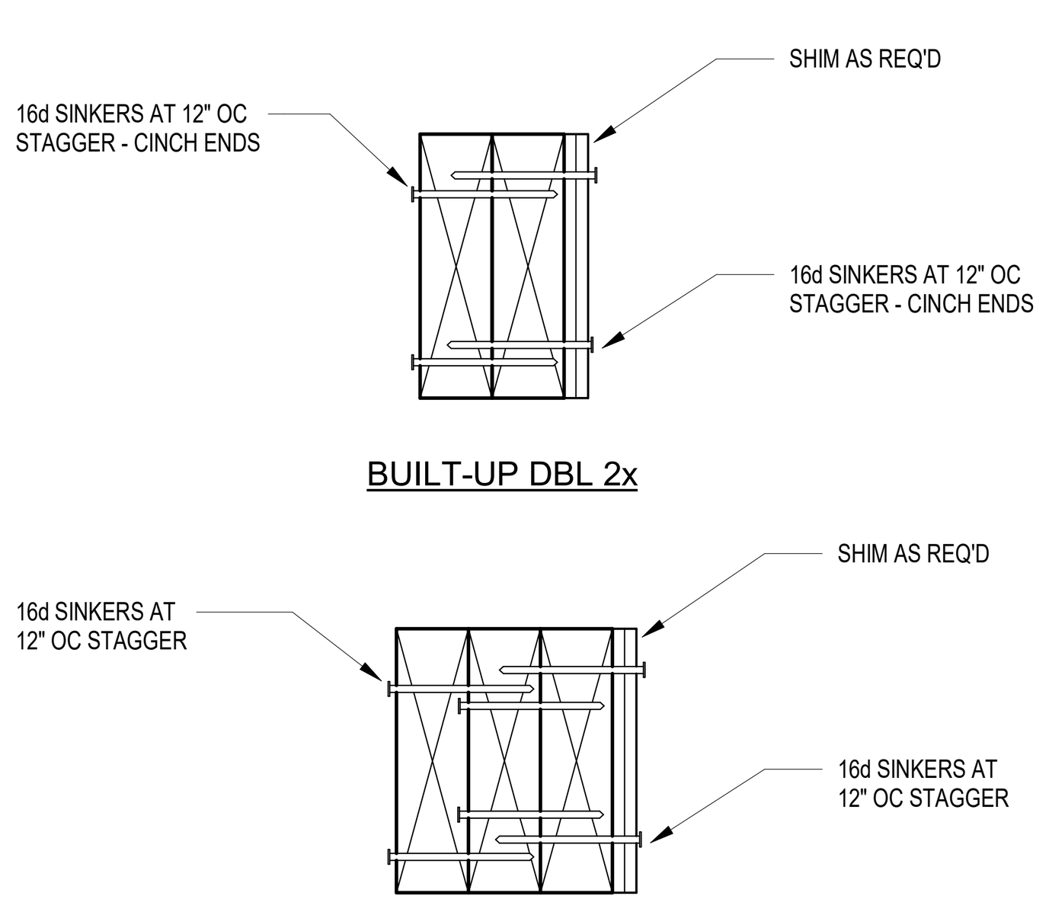

10 **SECTION**
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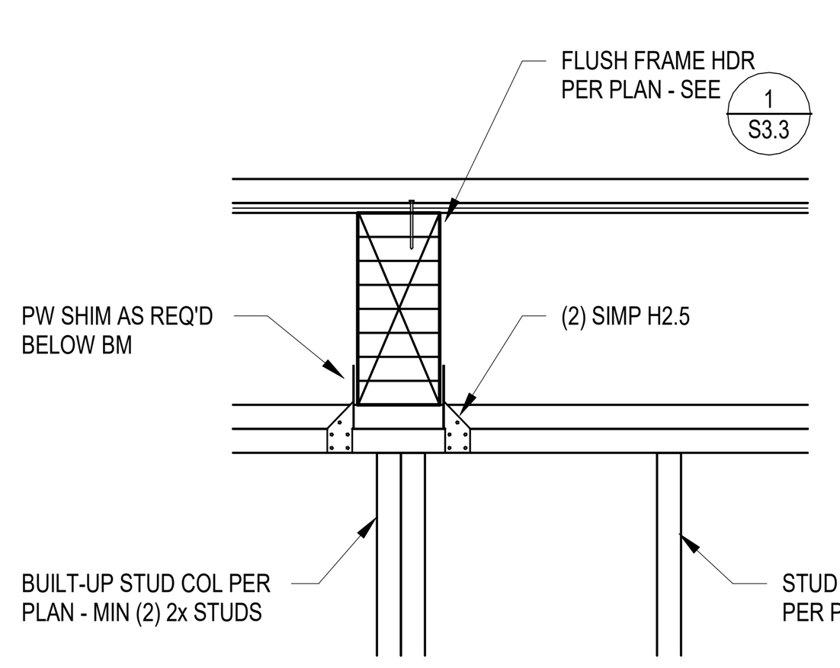

11 **SECTION**
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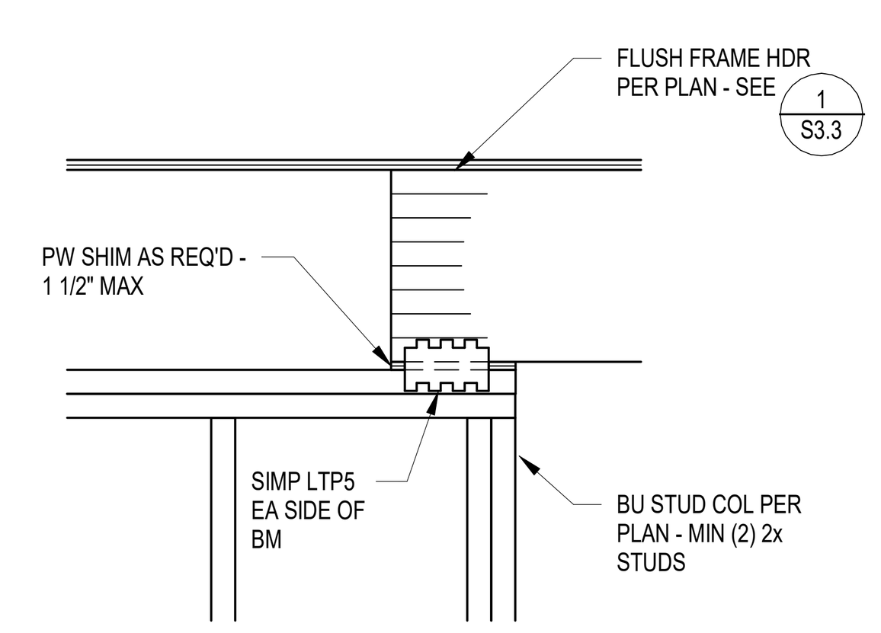

12 **SECTION**
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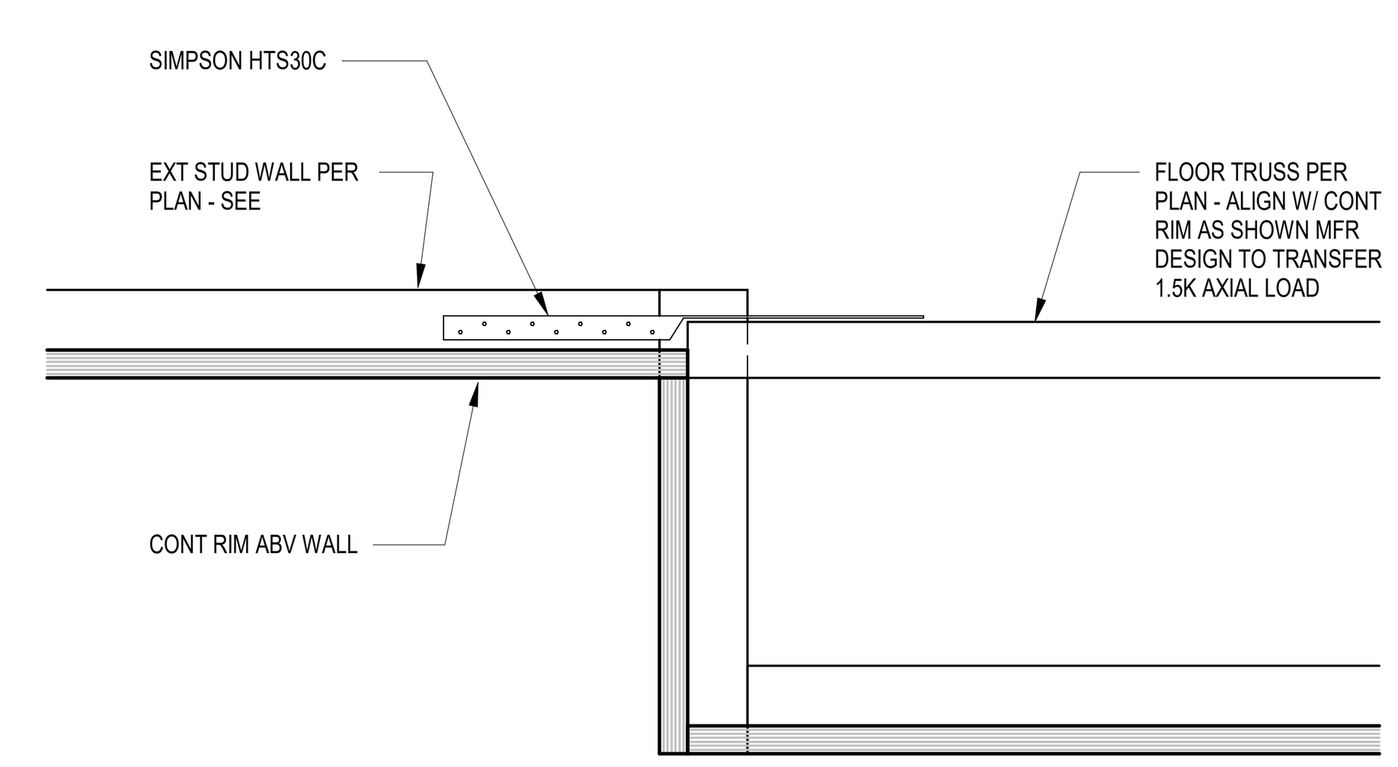

TYPICAL DIAPHRAGM CHORD SPLICE

13 **PLAN**
1" = 1'-0" 13 / S3.1

12/4/2024 10:56:19 AM

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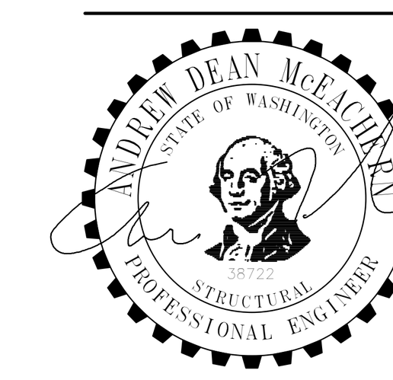
1000 university ave. w. suite 130
st. paul, minnesota 55104
612-252-4820



NOTICE:

WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER
707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 12/17/15

REVISIONS

No.	Description	Date

2220236.20
PROJECT NUMBER

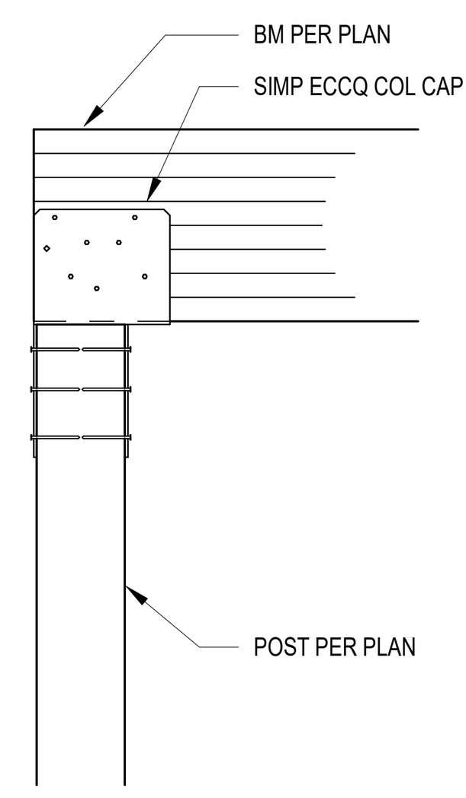
KJK DRAWN BY
ADM CHECKED BY

WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER

AHBL
TACOMA SEATTLE SPOKANE TRI-CITIES
2215 North 30th Street, Suite 300, Tacoma,WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

FLOOR FRAMING DETAILS

S3.1



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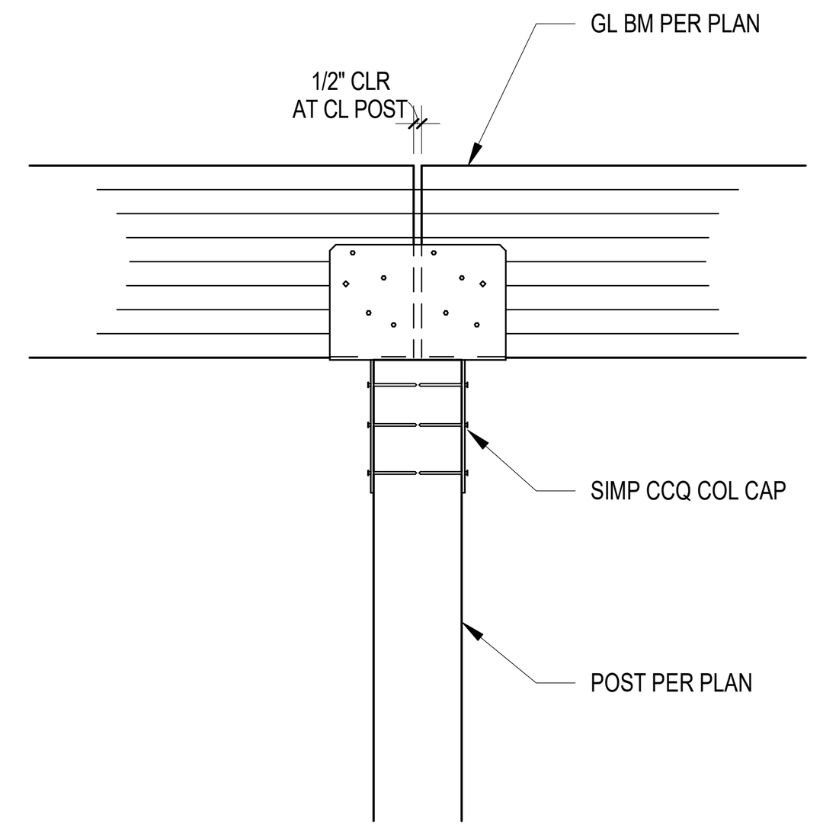

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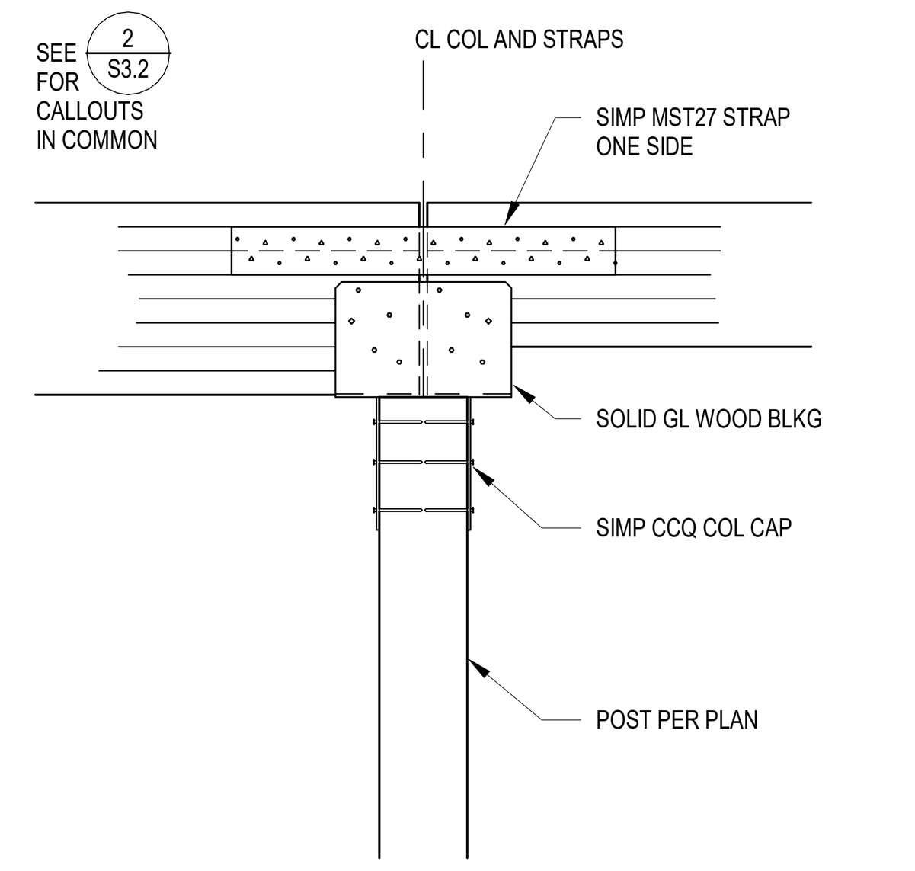

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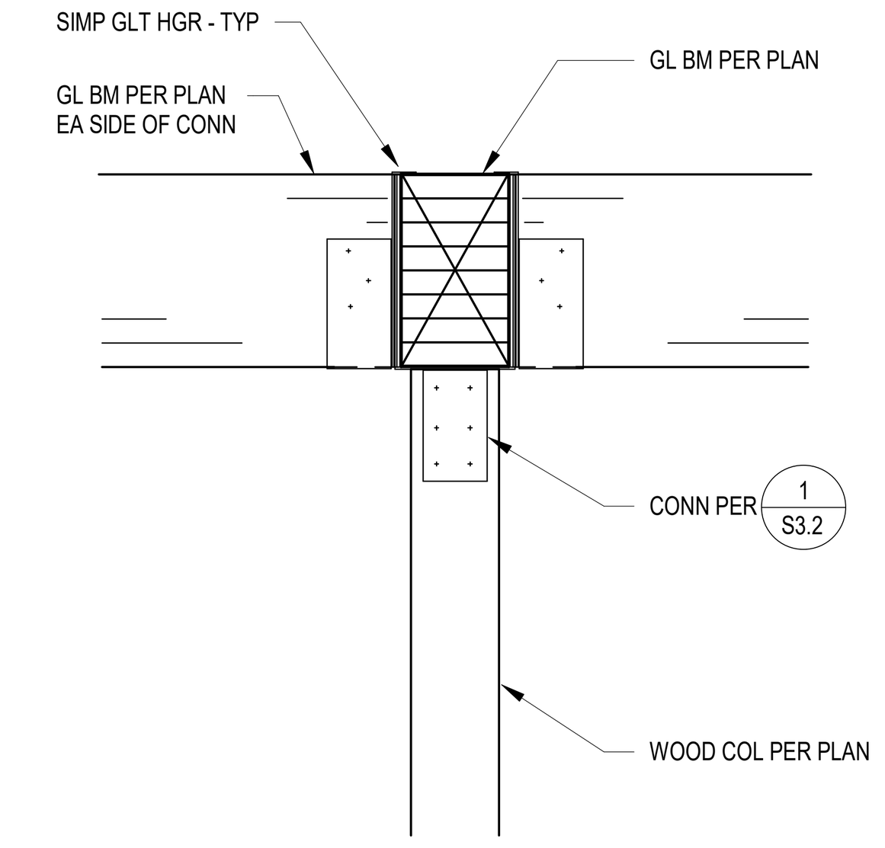

4 SECTION
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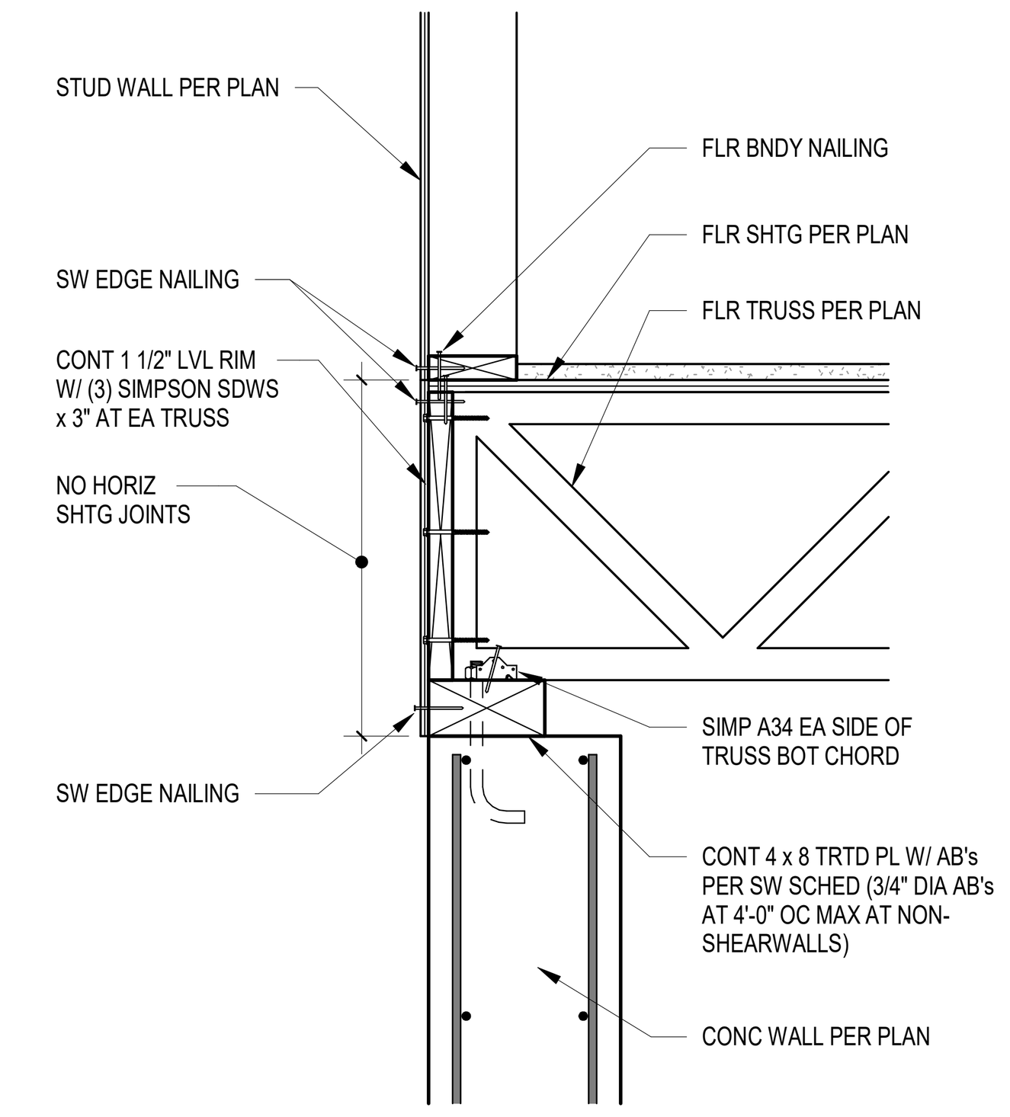
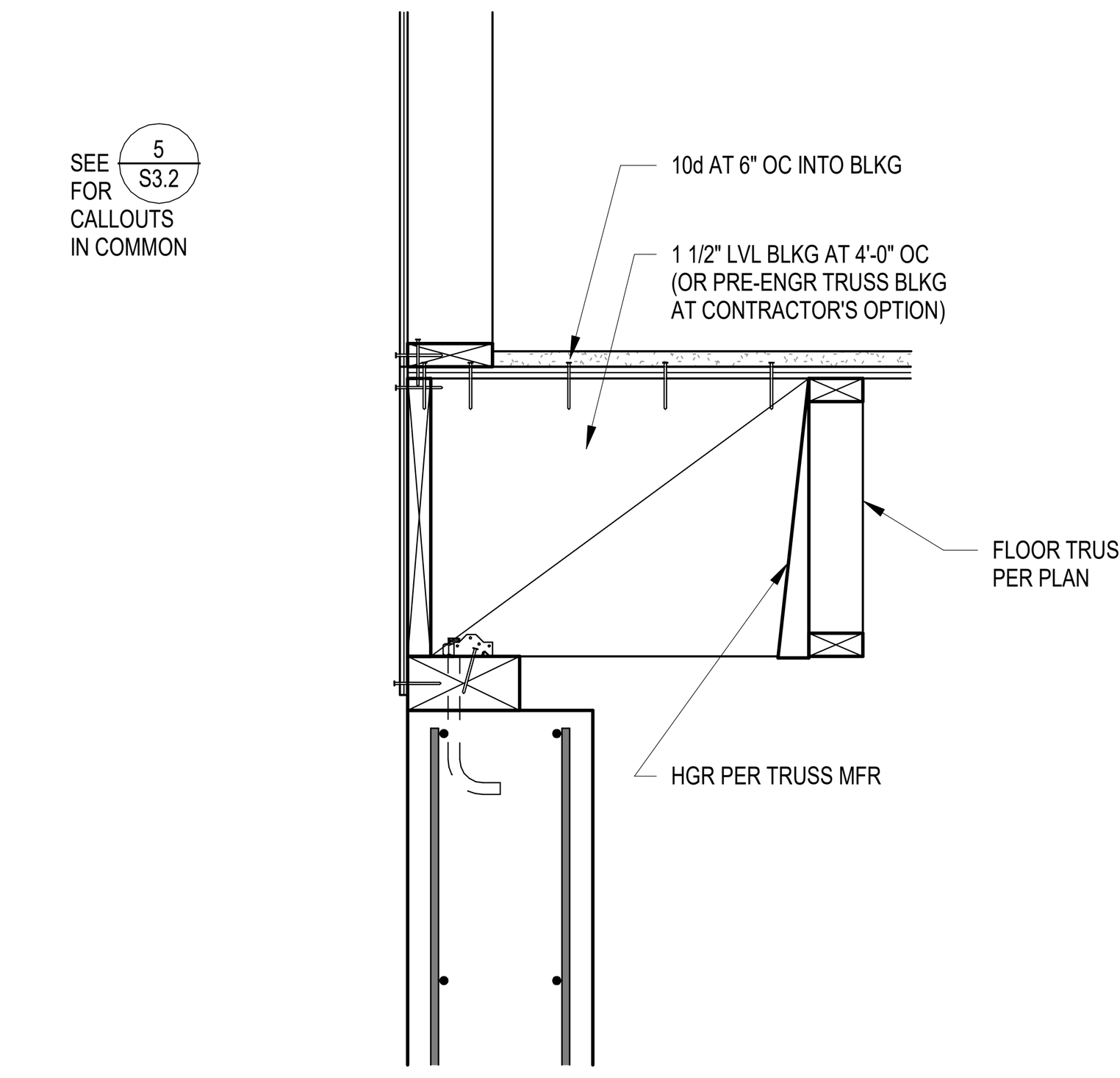
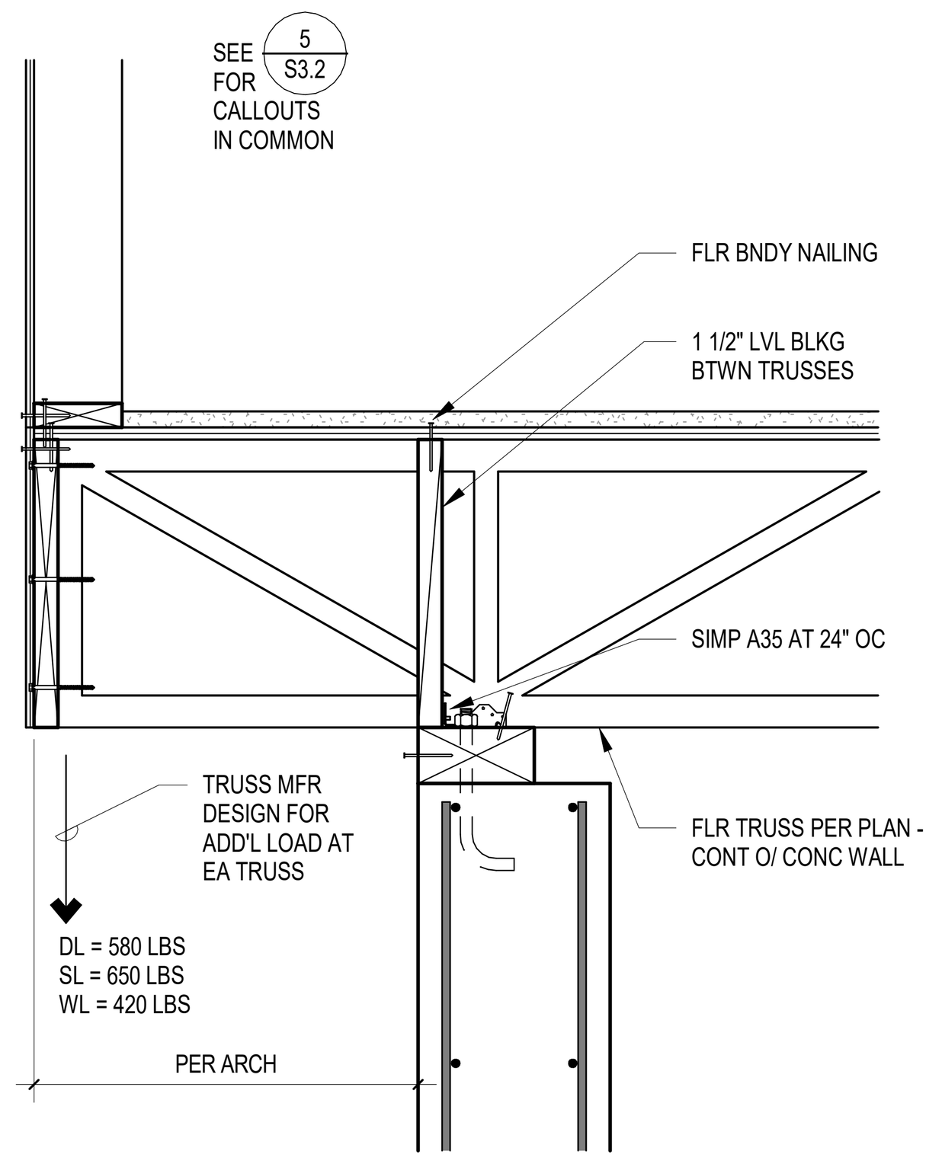


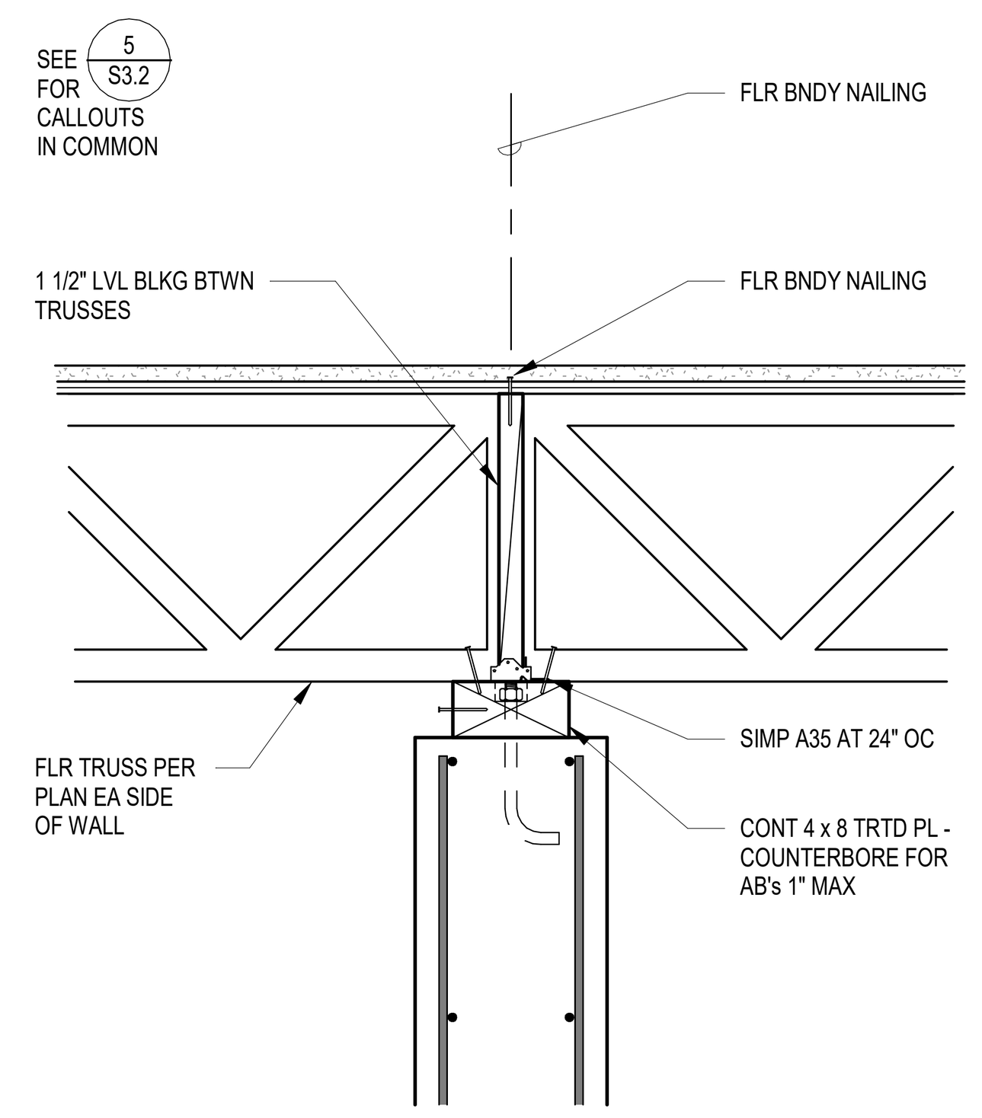
5 SECTION
1" = 1'-0" 5 / S3.2



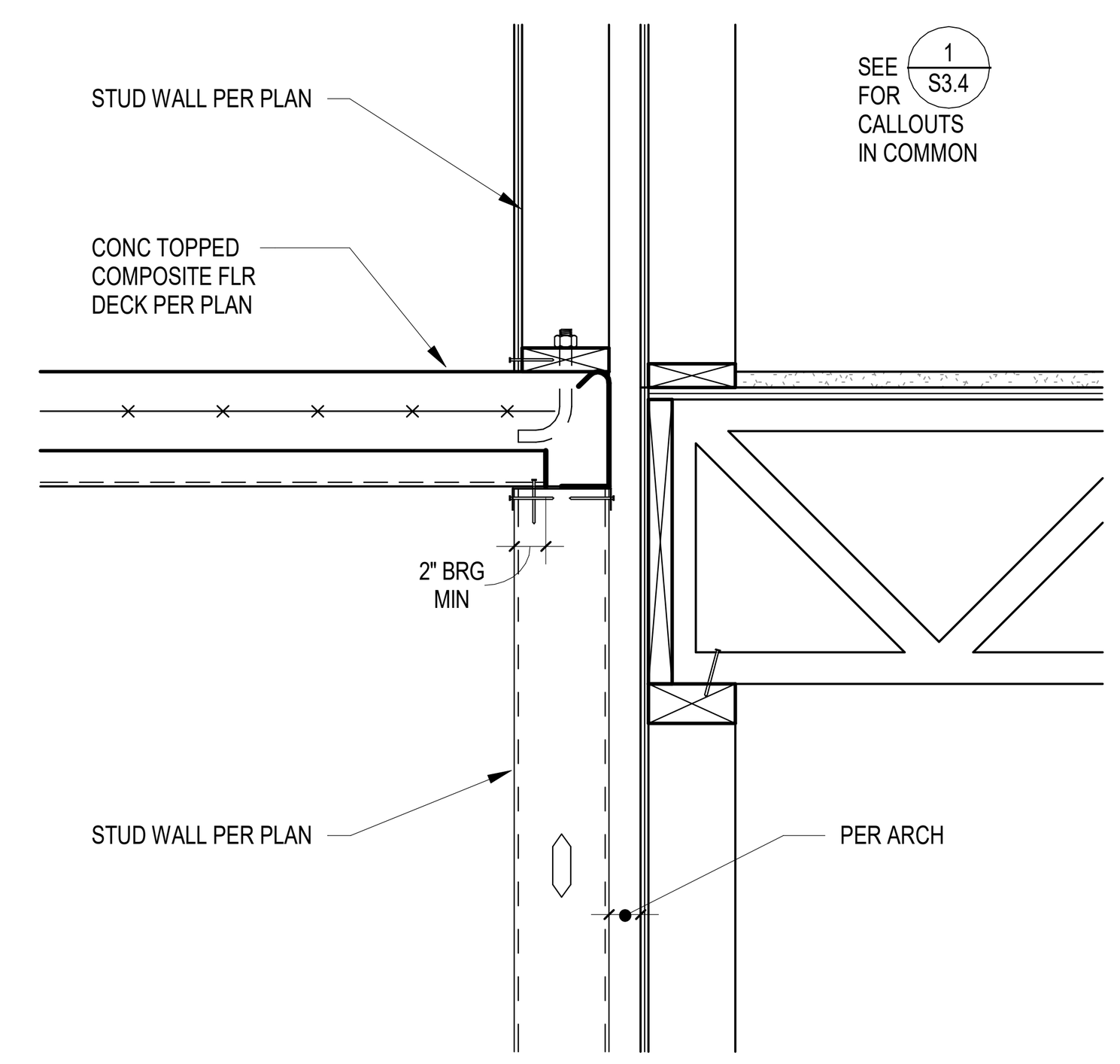
6 SECTION
1" = 1'-0" 6 / S3.2



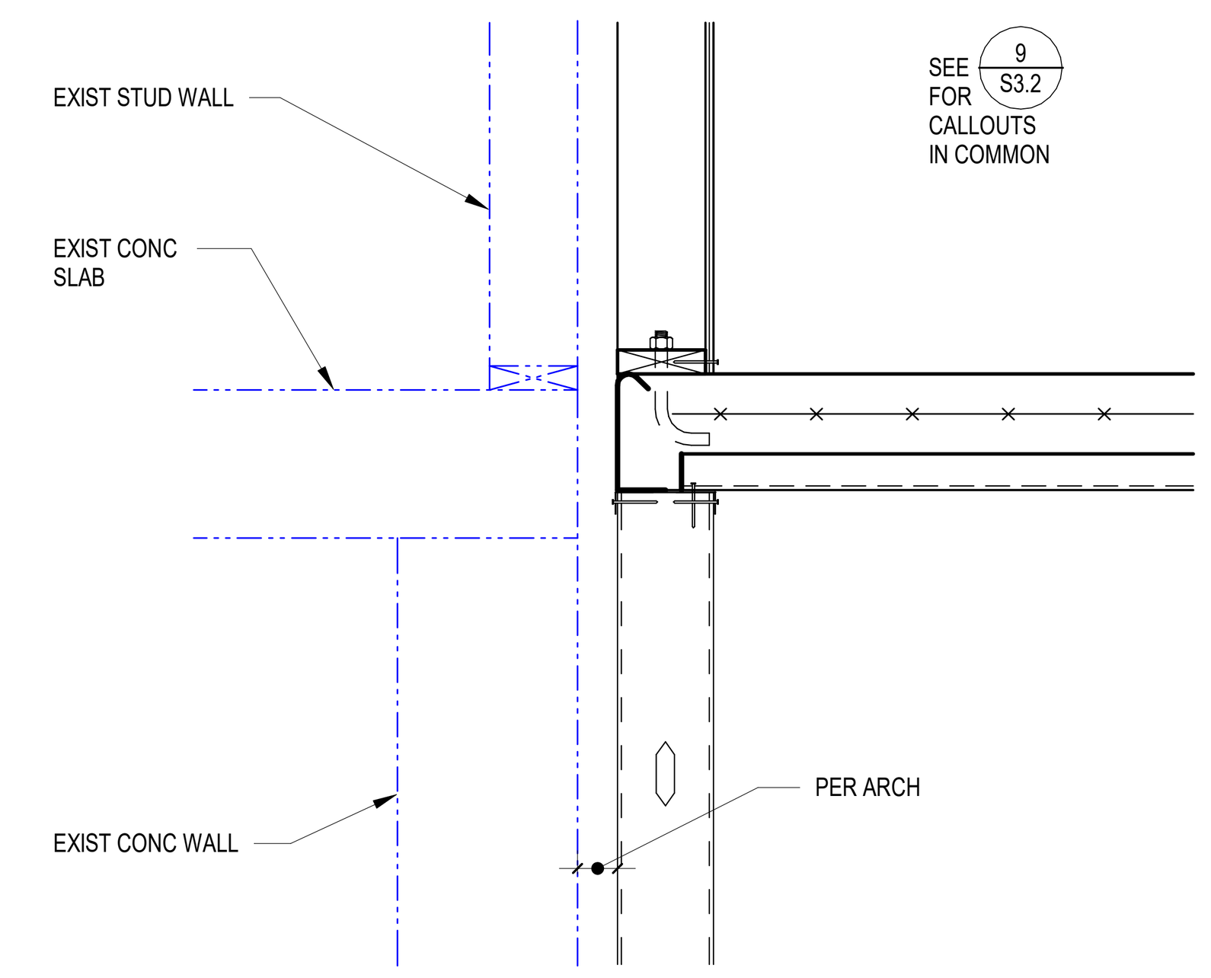
7 SECTION
1" = 1'-0" 7 / S3.2



8 SECTION
1" = 1'-0" 8 / S3.2



9 SECTION
1" = 1'-0" 9 / S3.2



10 SECTION
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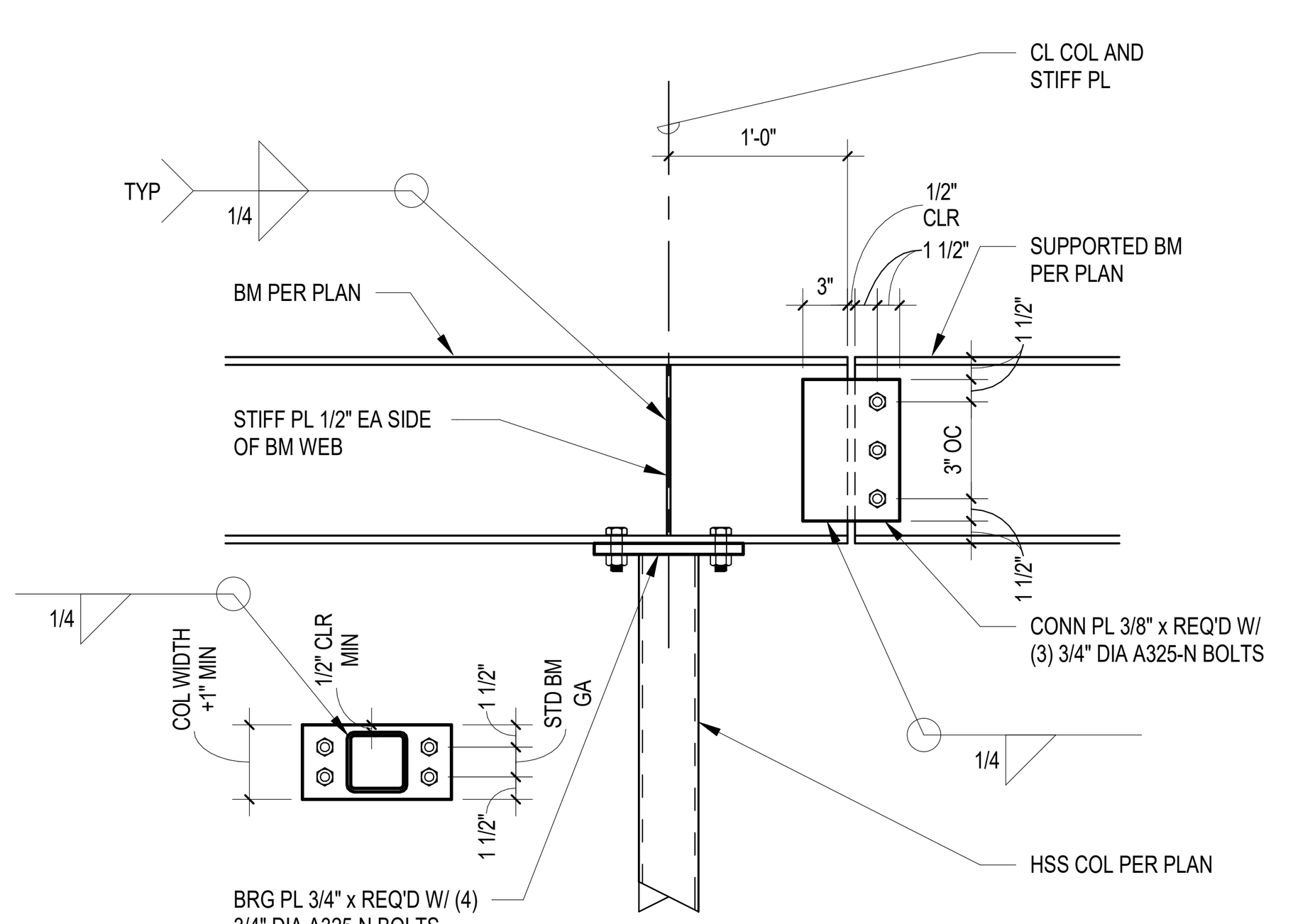

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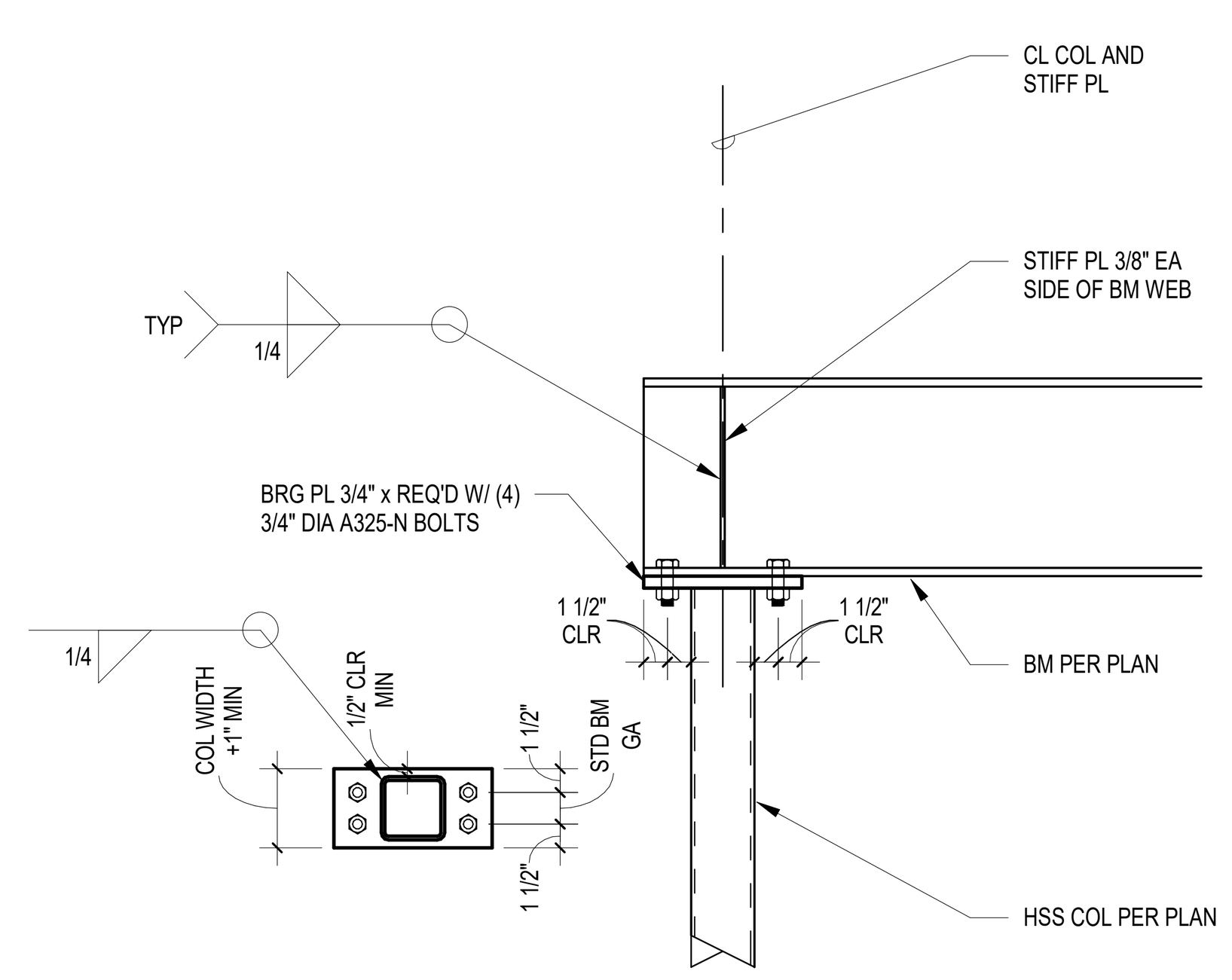

12 SECTION
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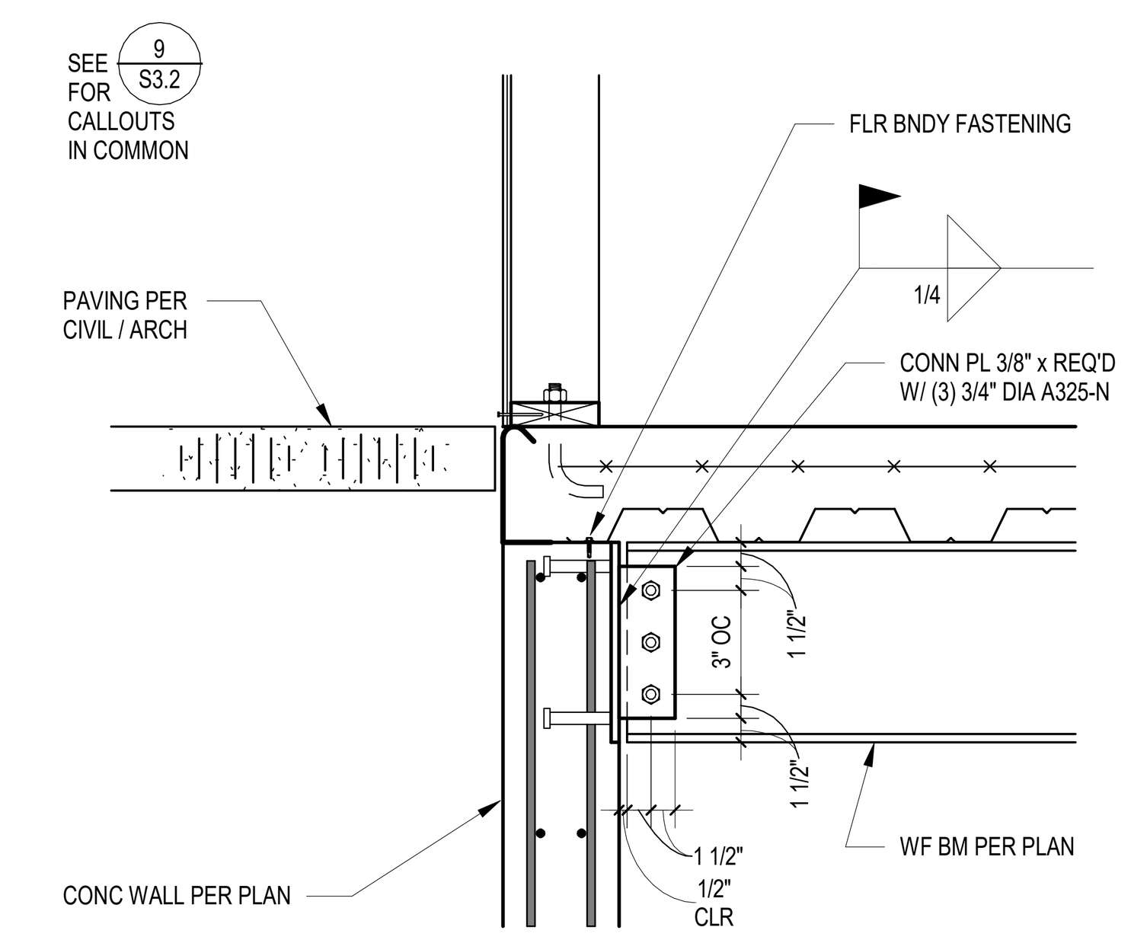

13 SECTION
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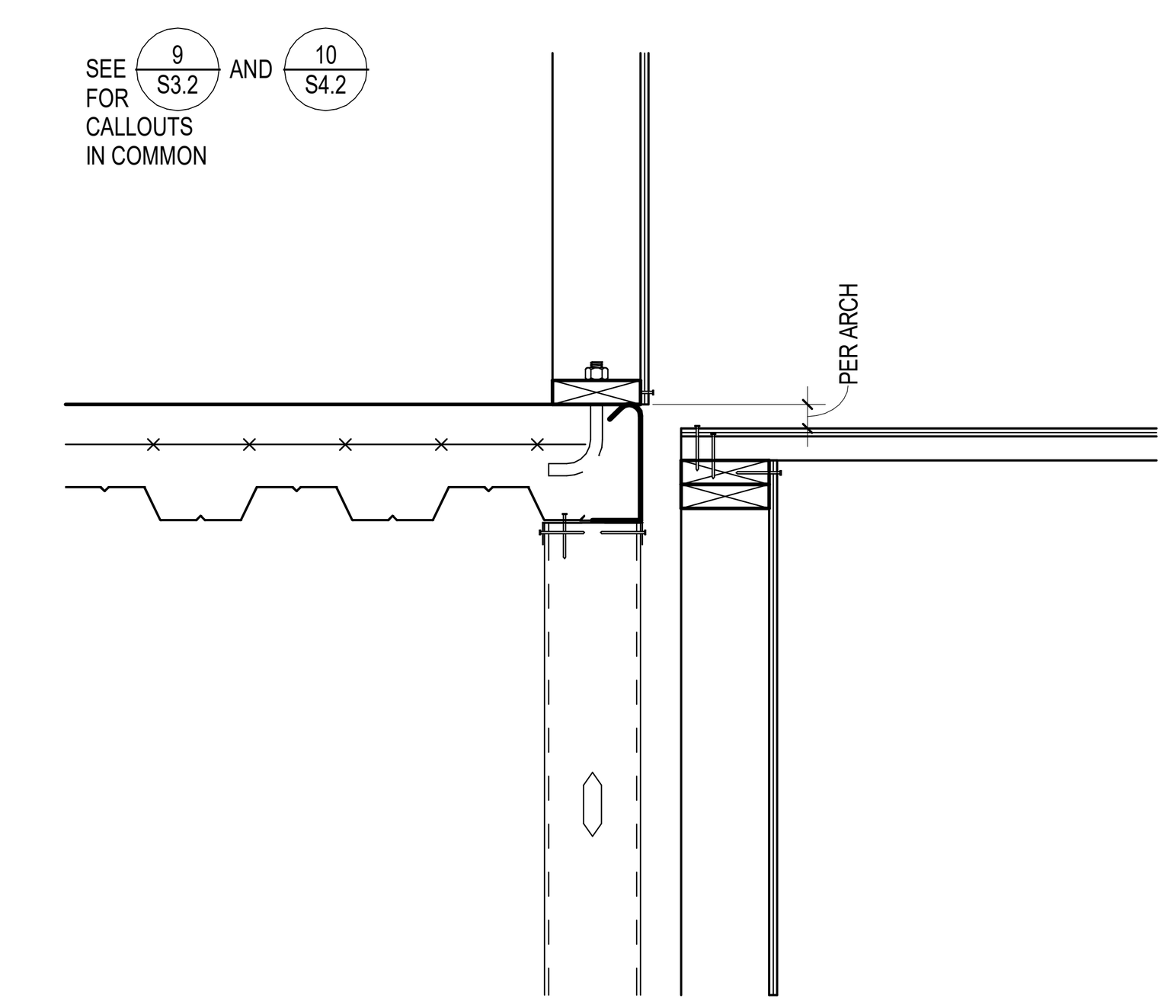

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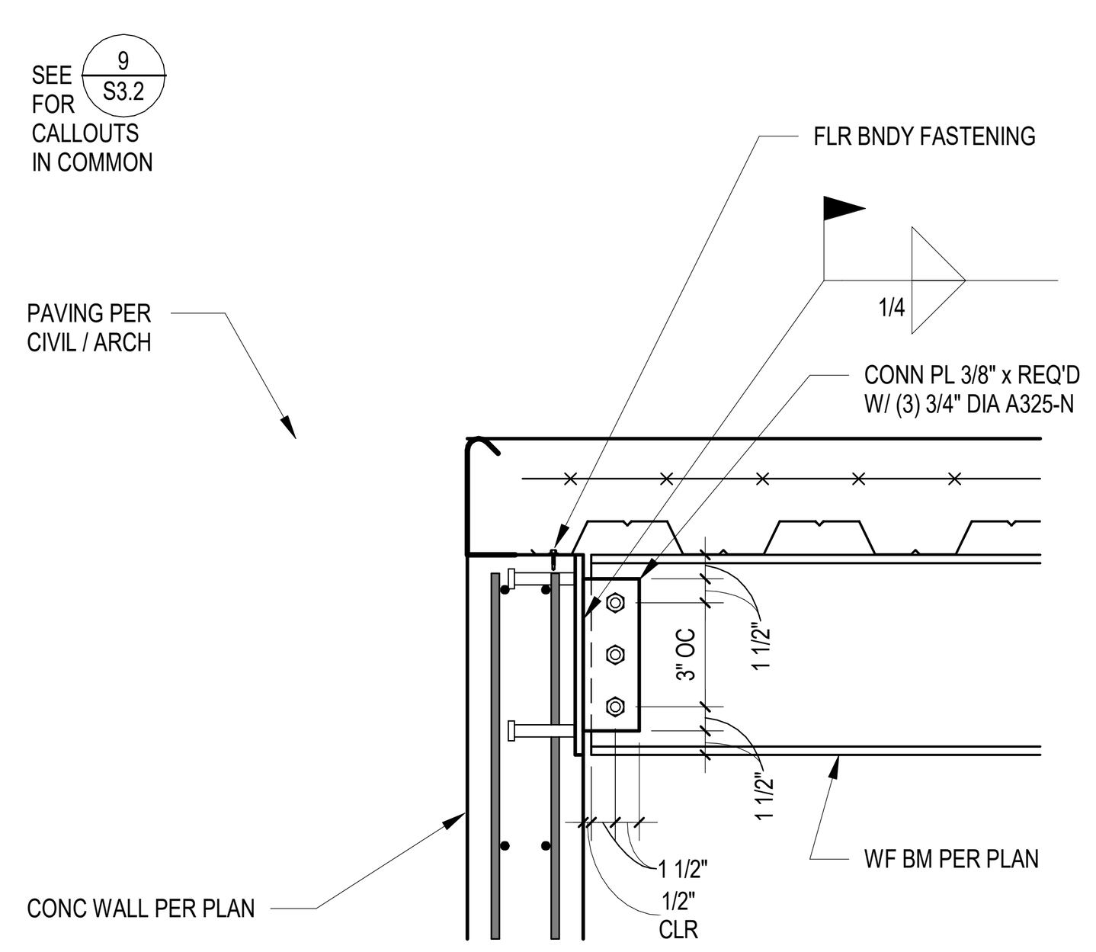

15 SECTION
1" = 1'-0" 15 / S3.2

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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER
707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 01/24/18

REVISIONS

No.	Description	Date

2220236.20
PROJECT NUMBER

KJK DRAWN BY
ADM CHECKED BY

WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

AHBL
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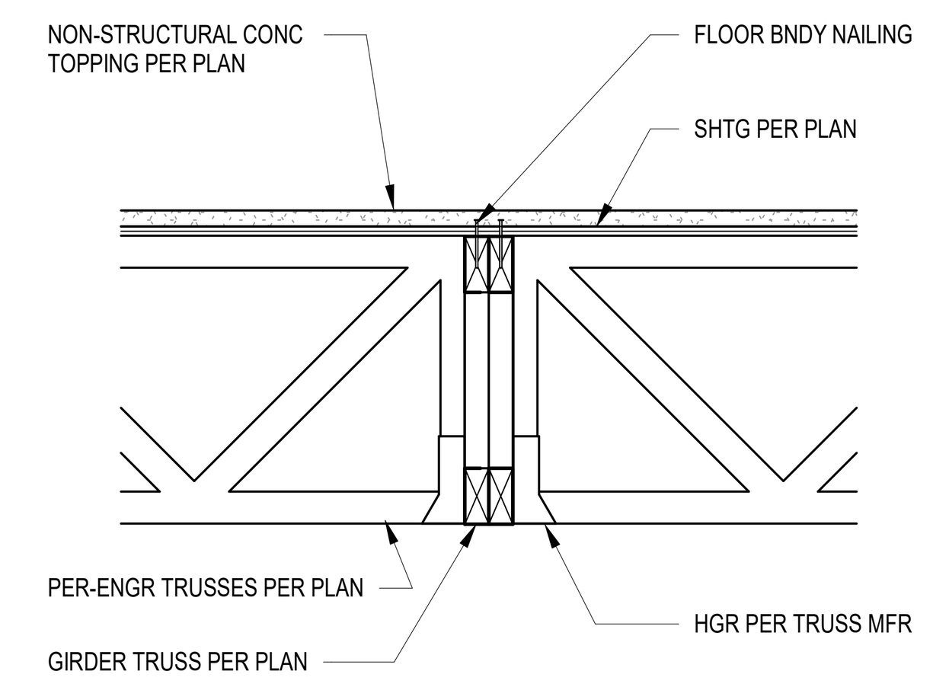
2215 North 30th Street, Suite 300, Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

FLOOR FRAMING DETAILS

S3.2



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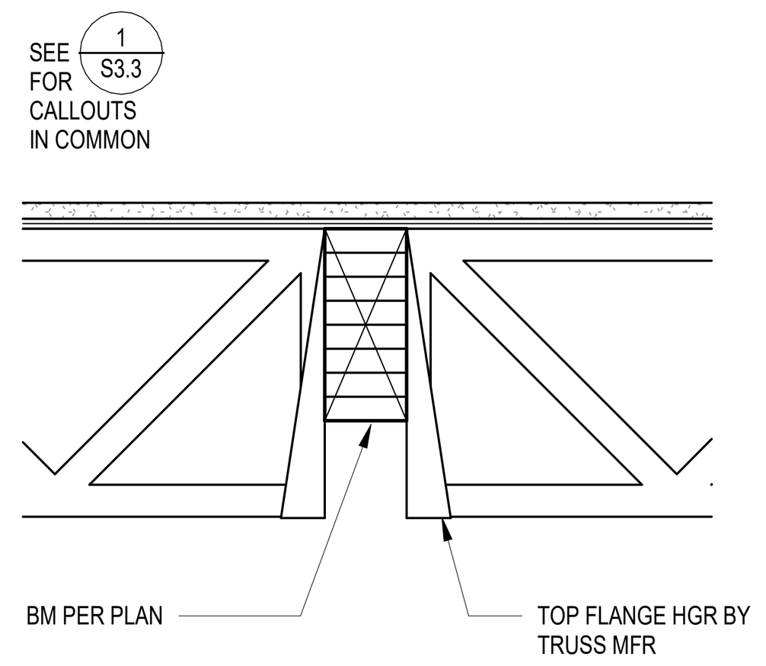

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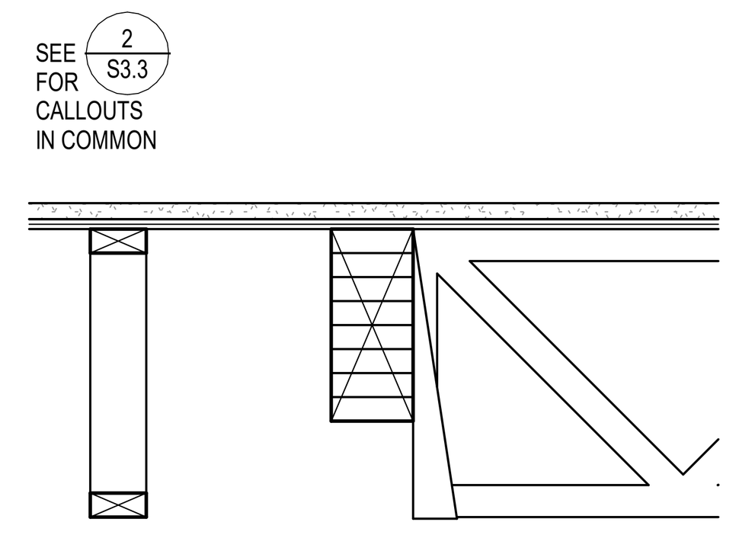

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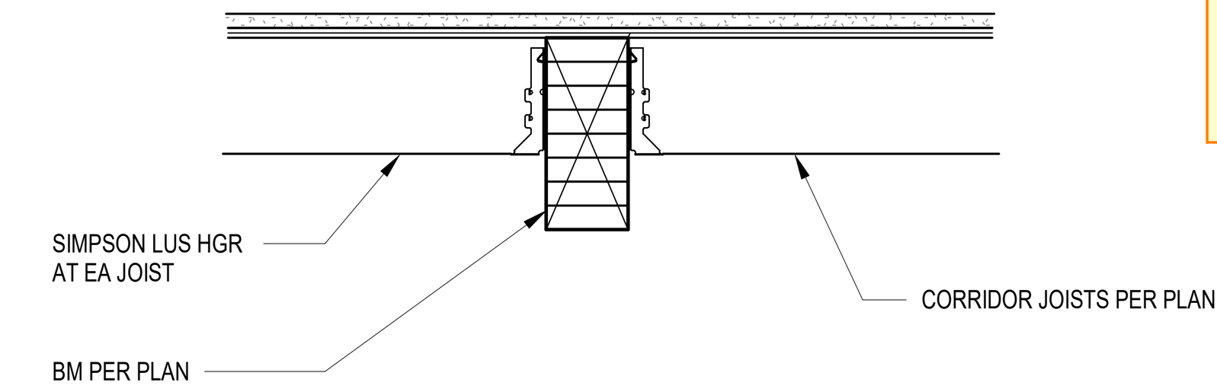

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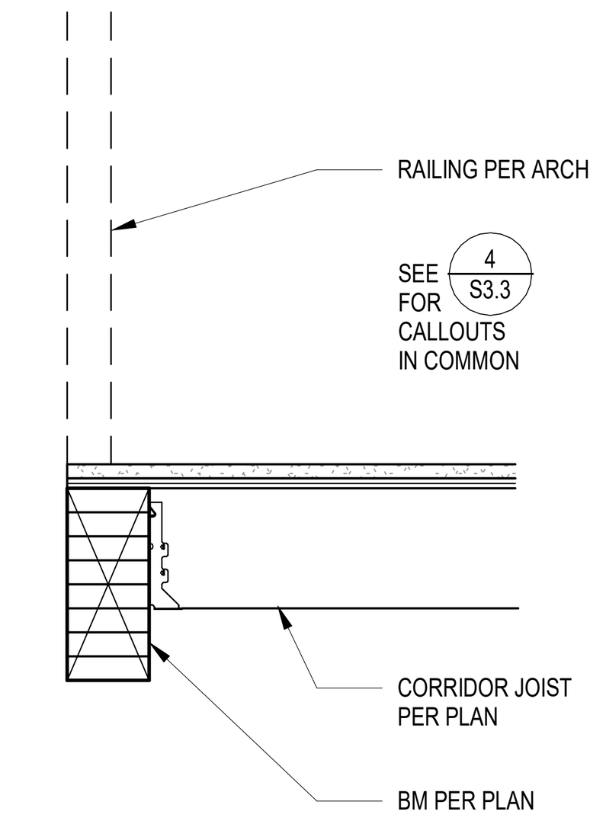

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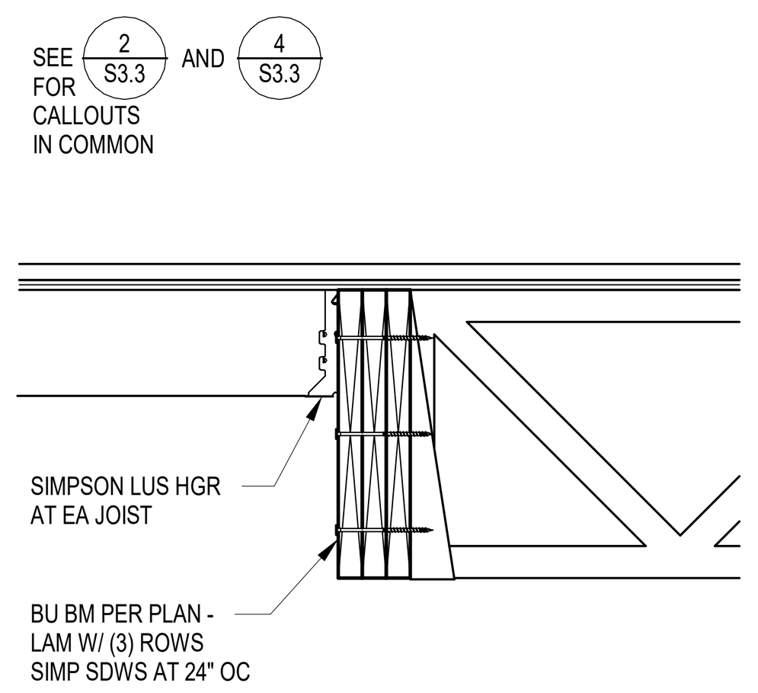
1" = 1'-0"



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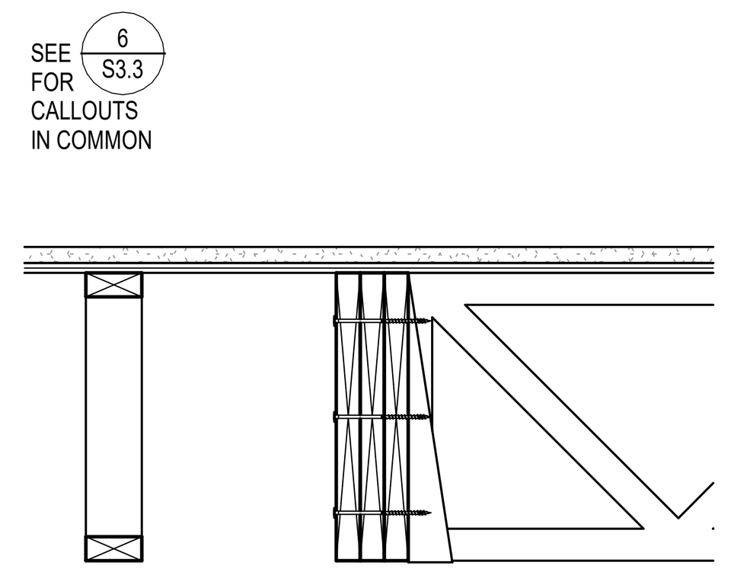

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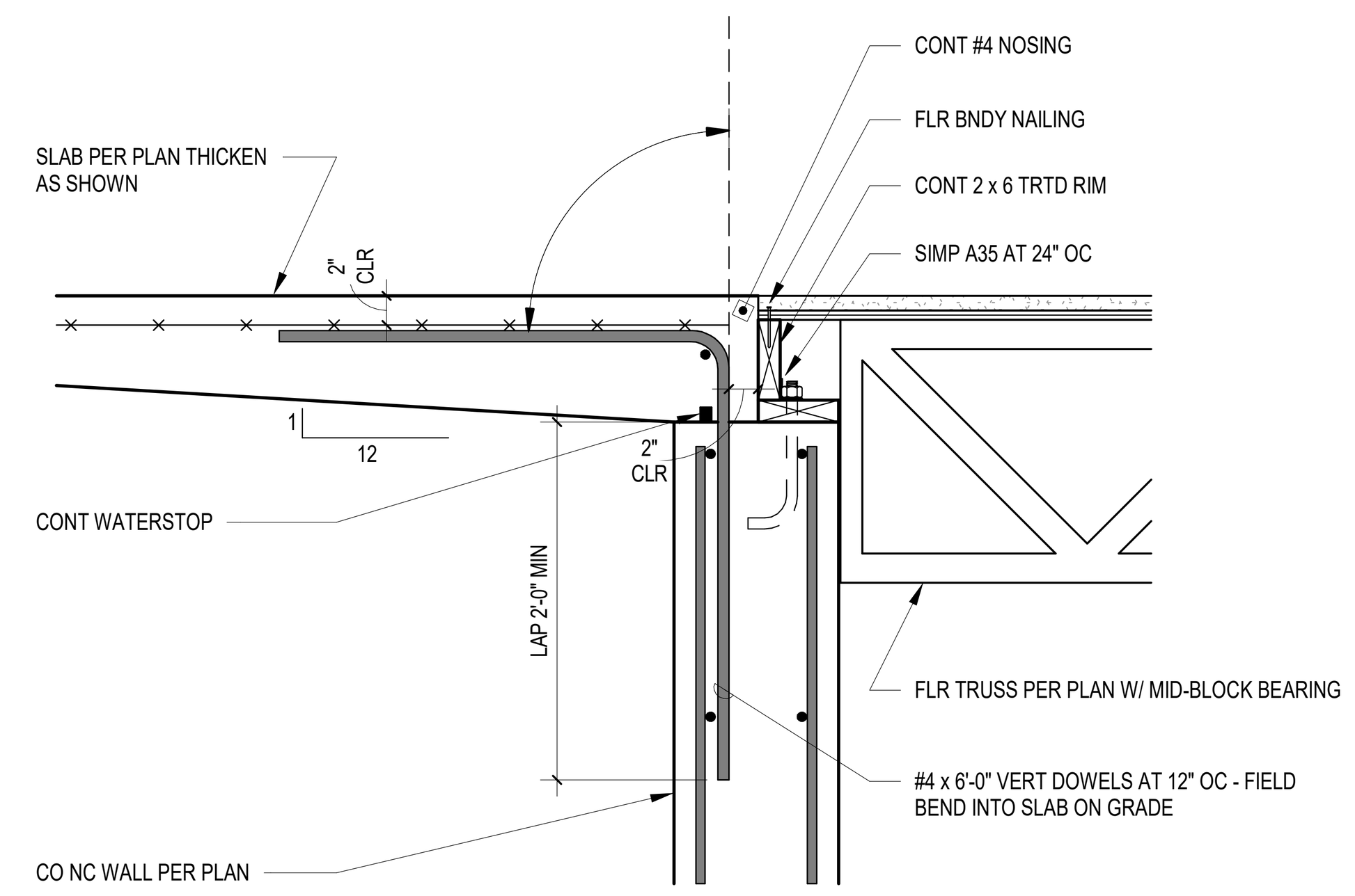
1" = 1'-0"



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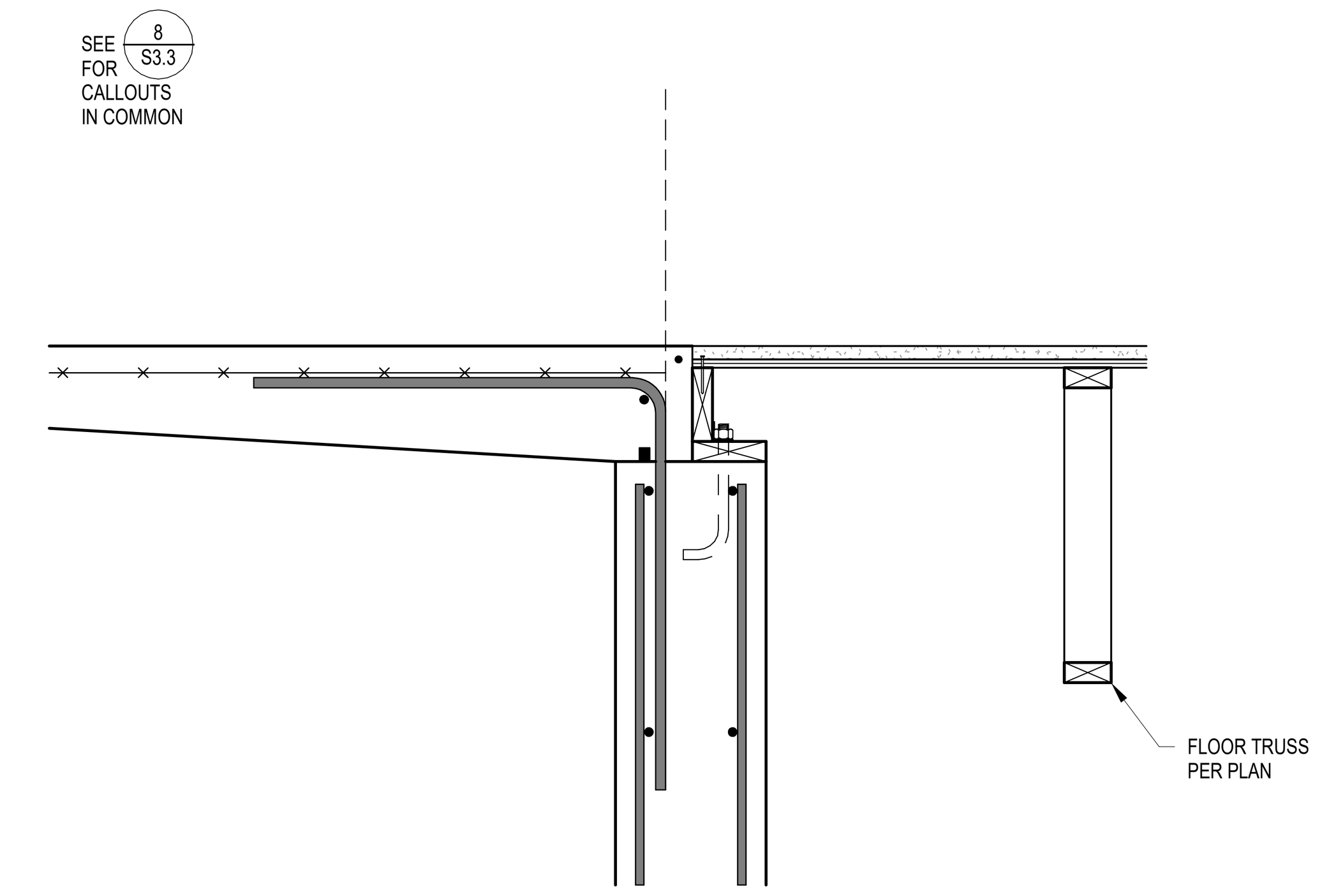

9 **SECTION**
1" = 1'-0"

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BE SUBMITTED AS A D

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1000 university ave. w. suite 130
st. paul, minnesota 55104
612-252-4820

NOTICE:

WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 08/11/17

REVISIONS

No.	Description	Date

2220236.20
PROJECT NUMBER

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PHASE 2 - CARE CENTER

FLOOR FRAMING DETAILS

S3.3

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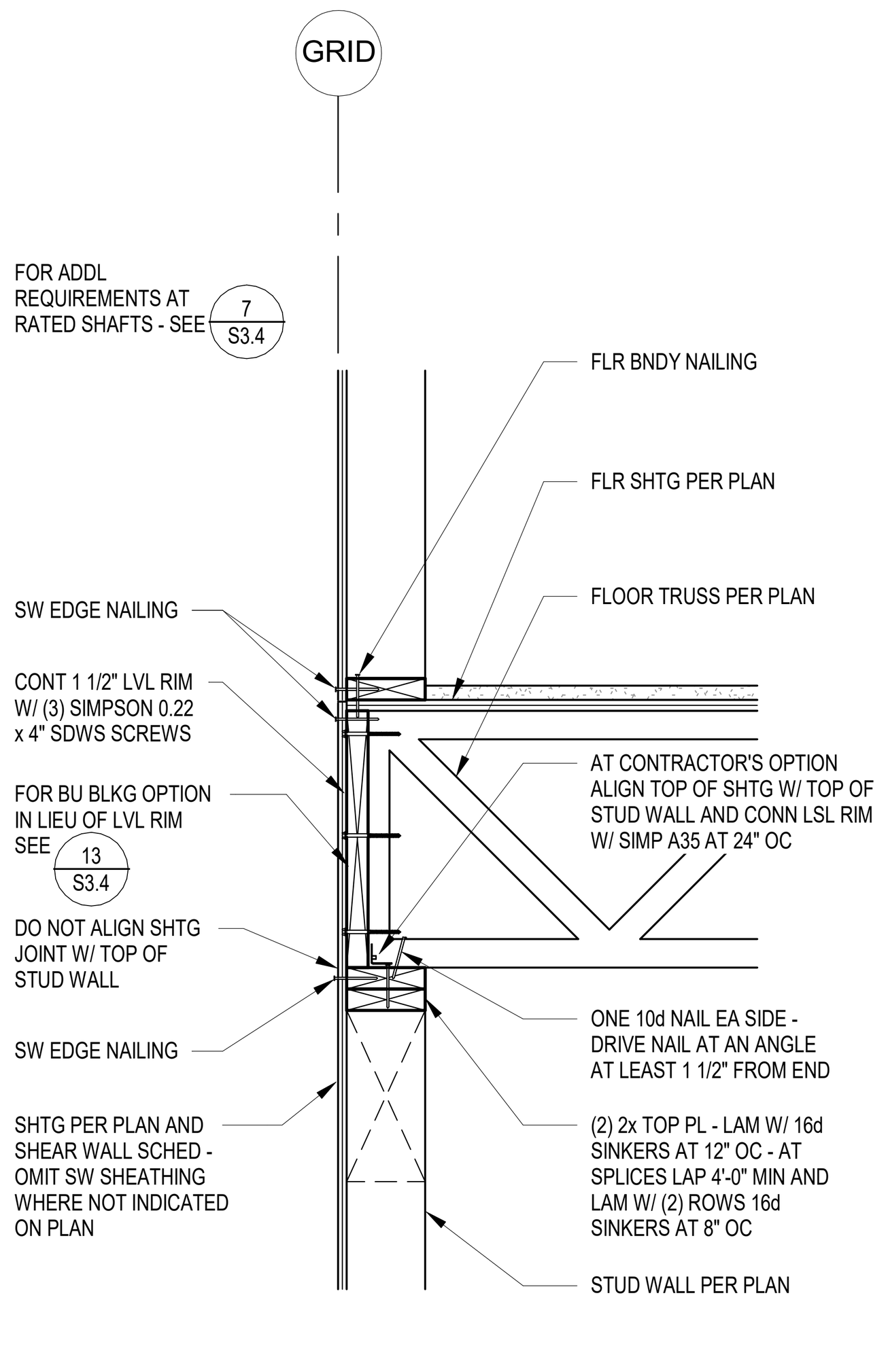

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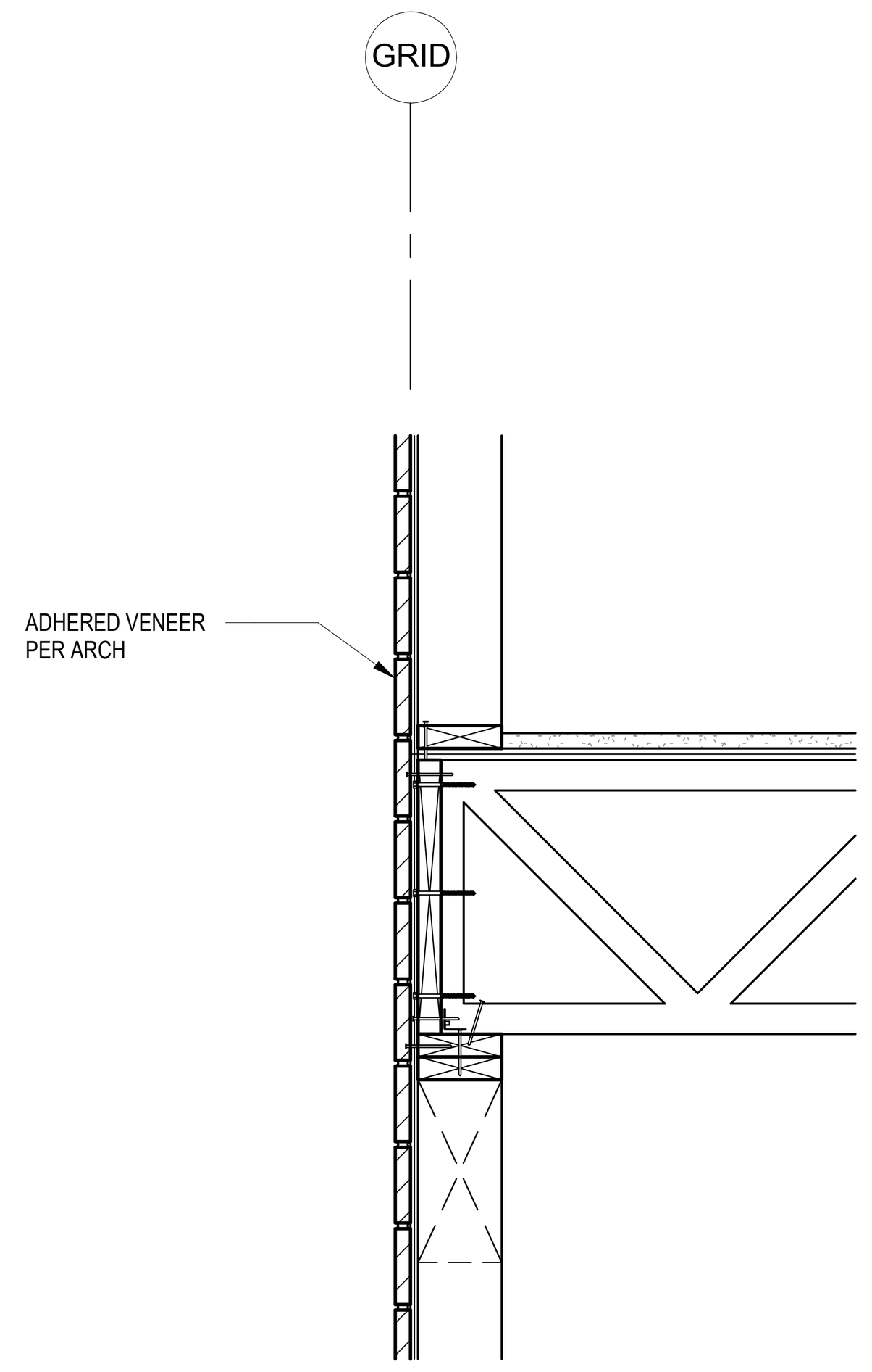

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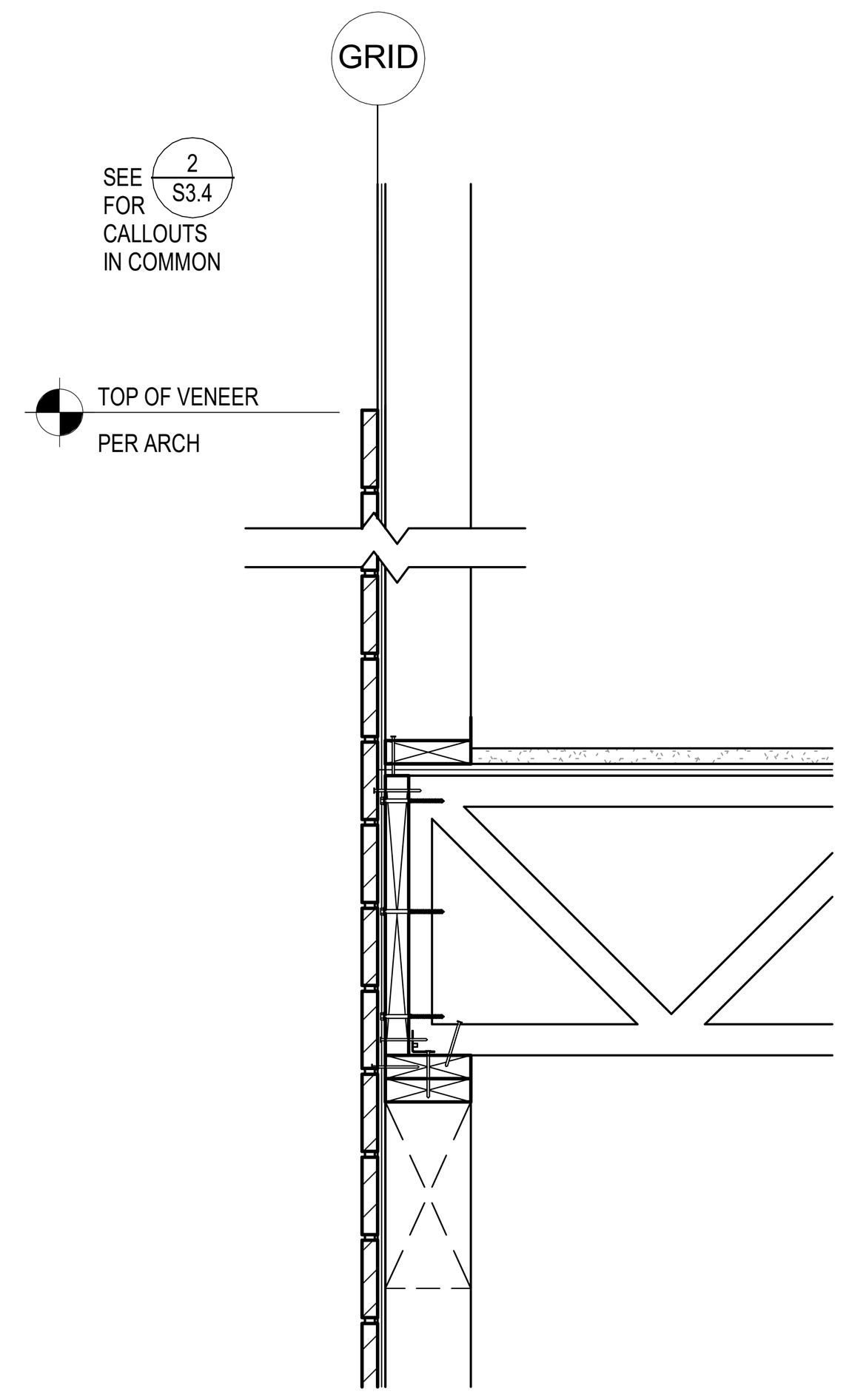

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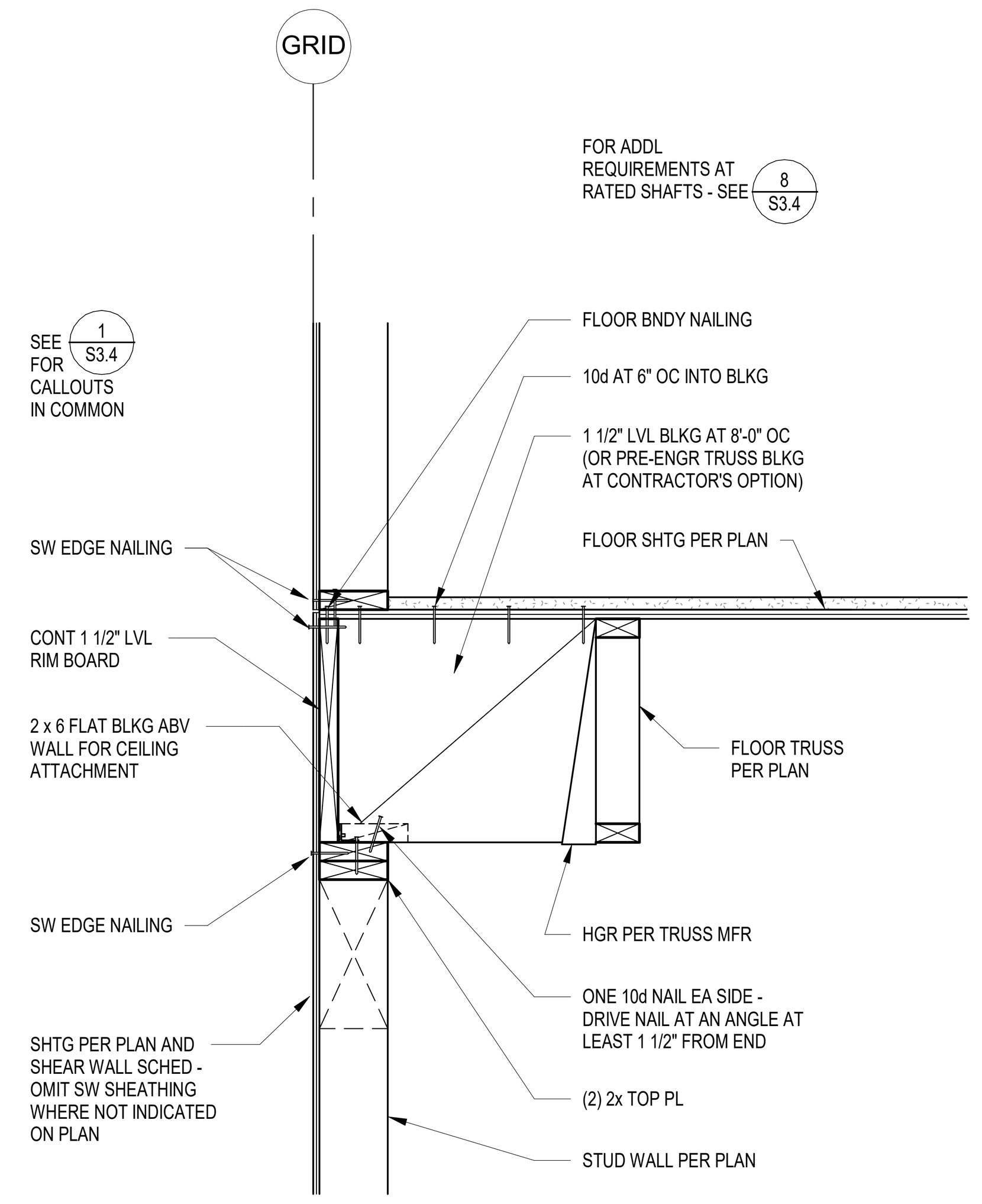
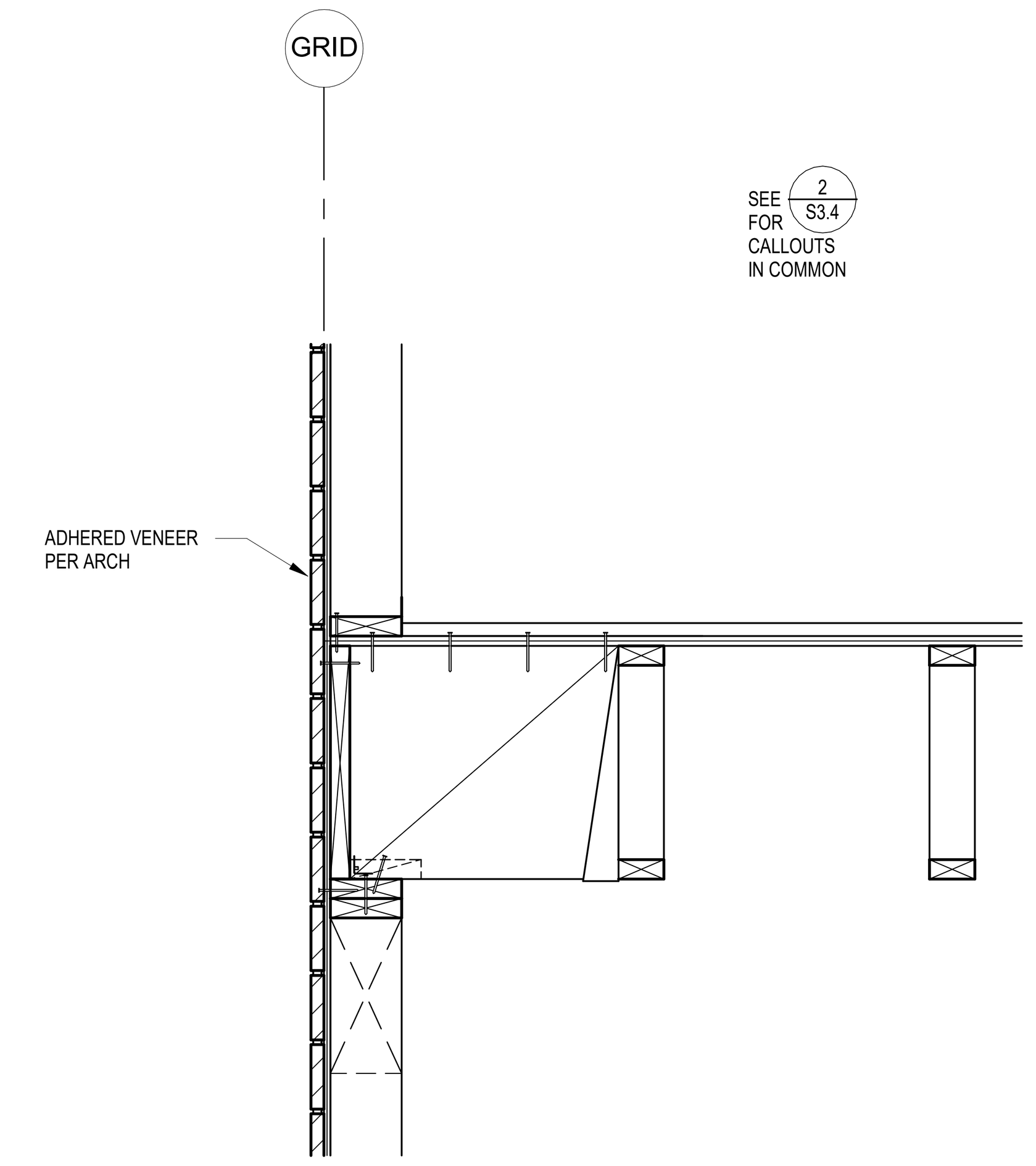
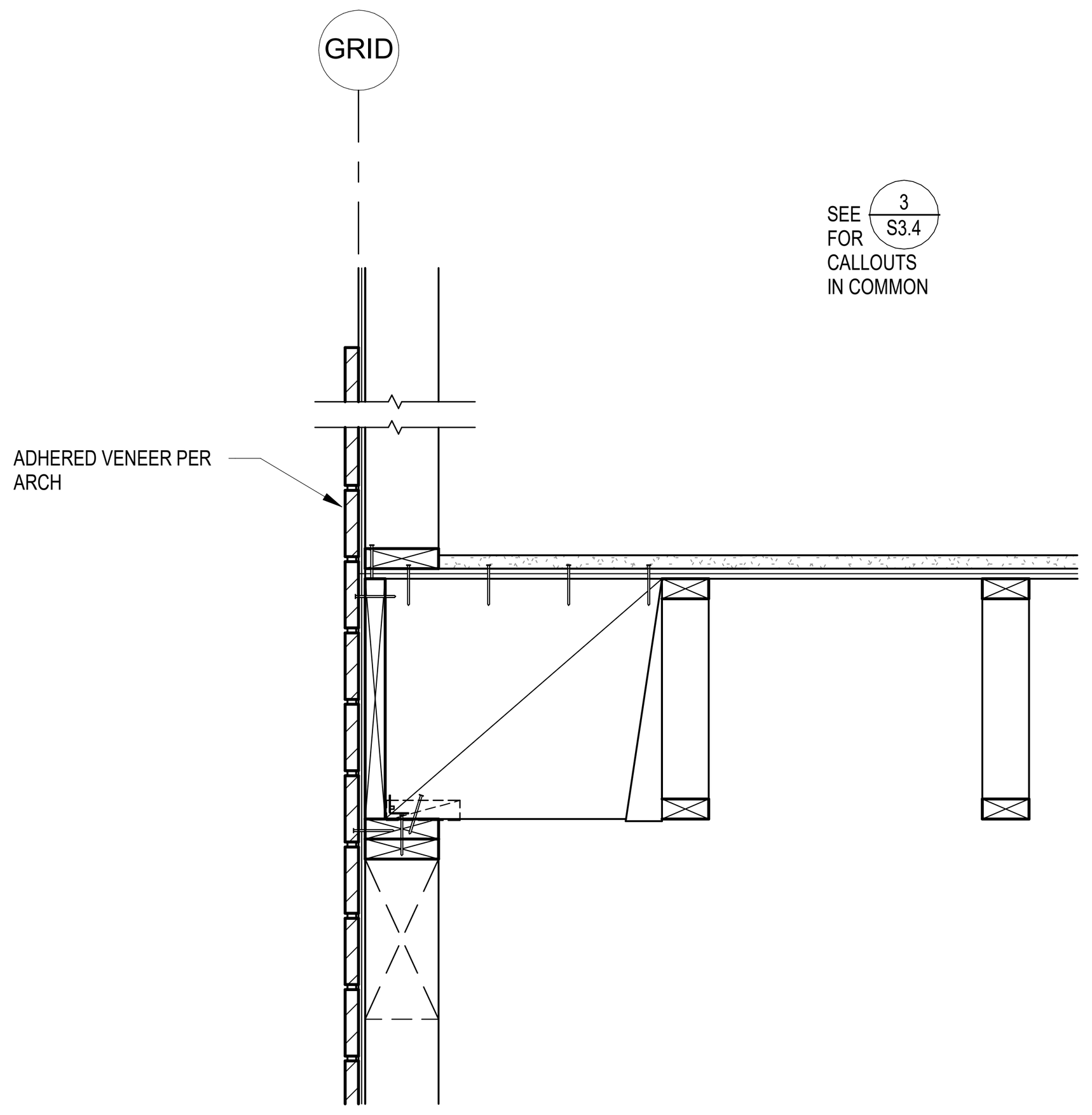


4 **SECTION**
1" = 1'-0" 4 / S3.4



5 **SECTION**
1" = 1'-0" 5 / S3.4



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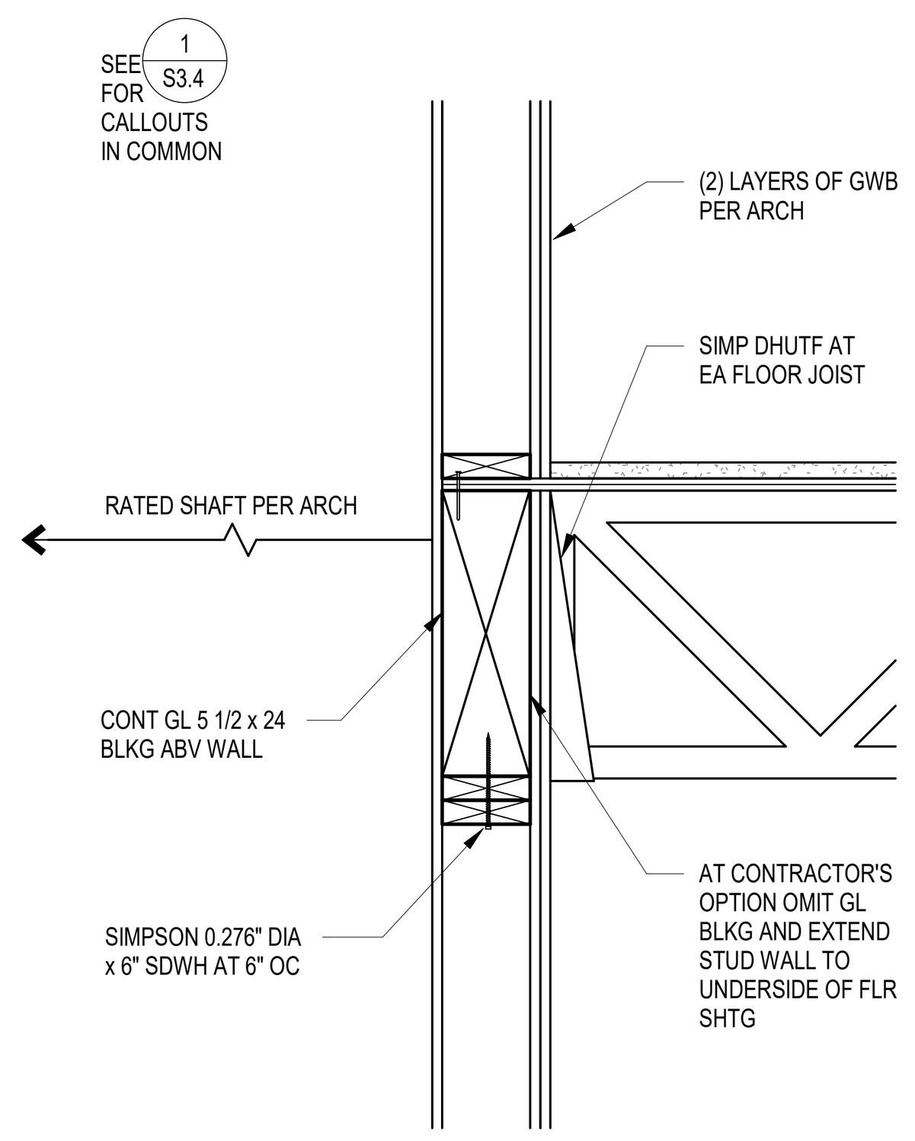
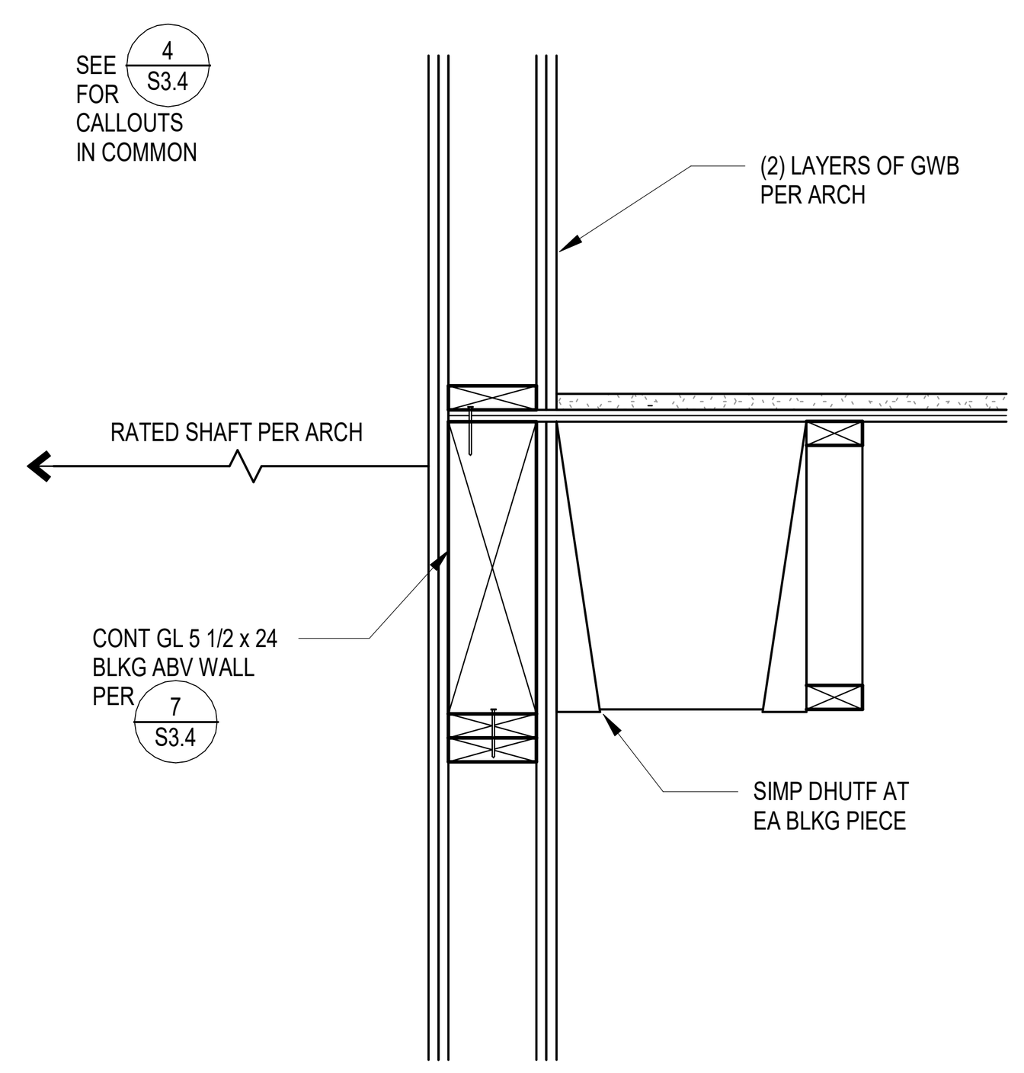
1" = 1'-0" 6 / S3.4



7 **SECTION**
1" = 1'-0" 7 / S3.4



8 **SECTION**
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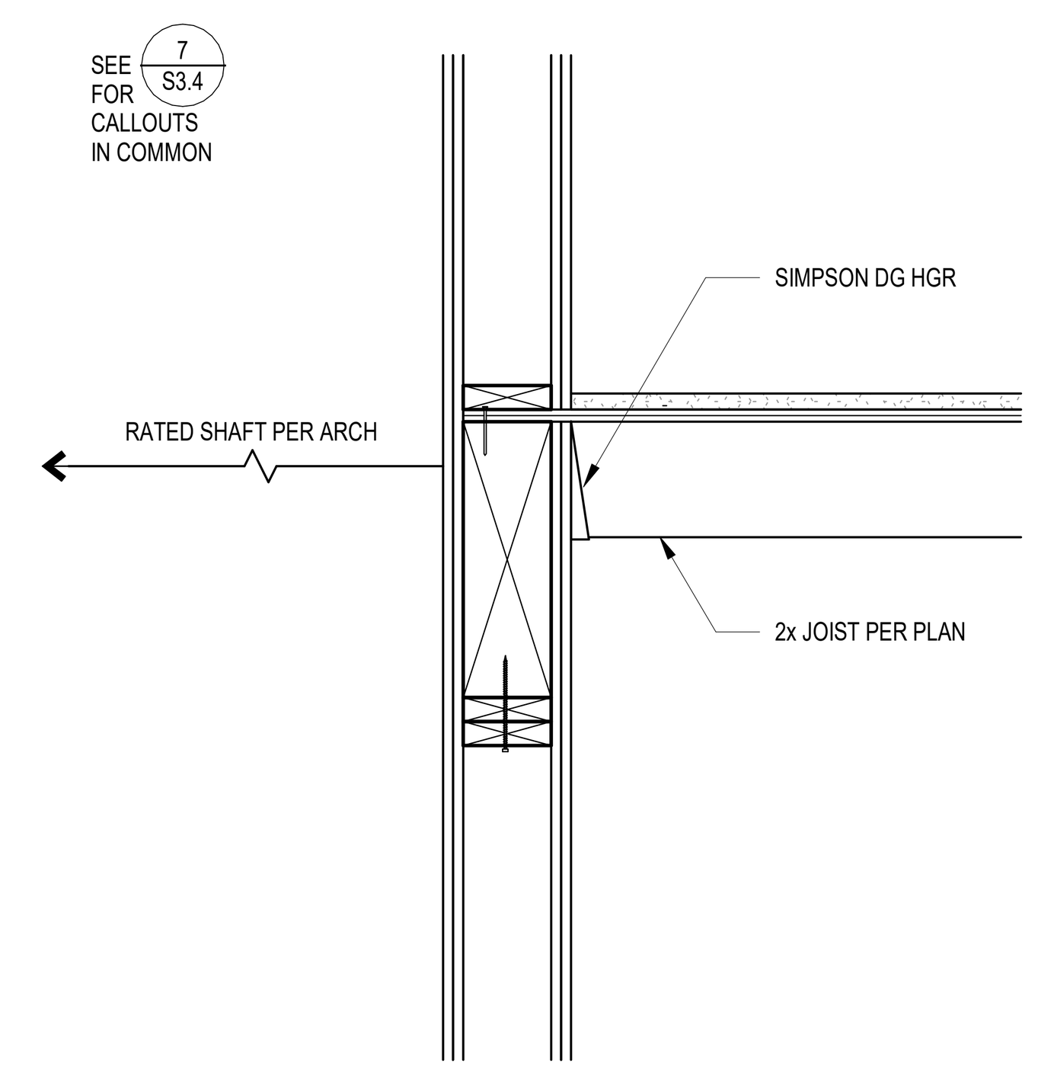
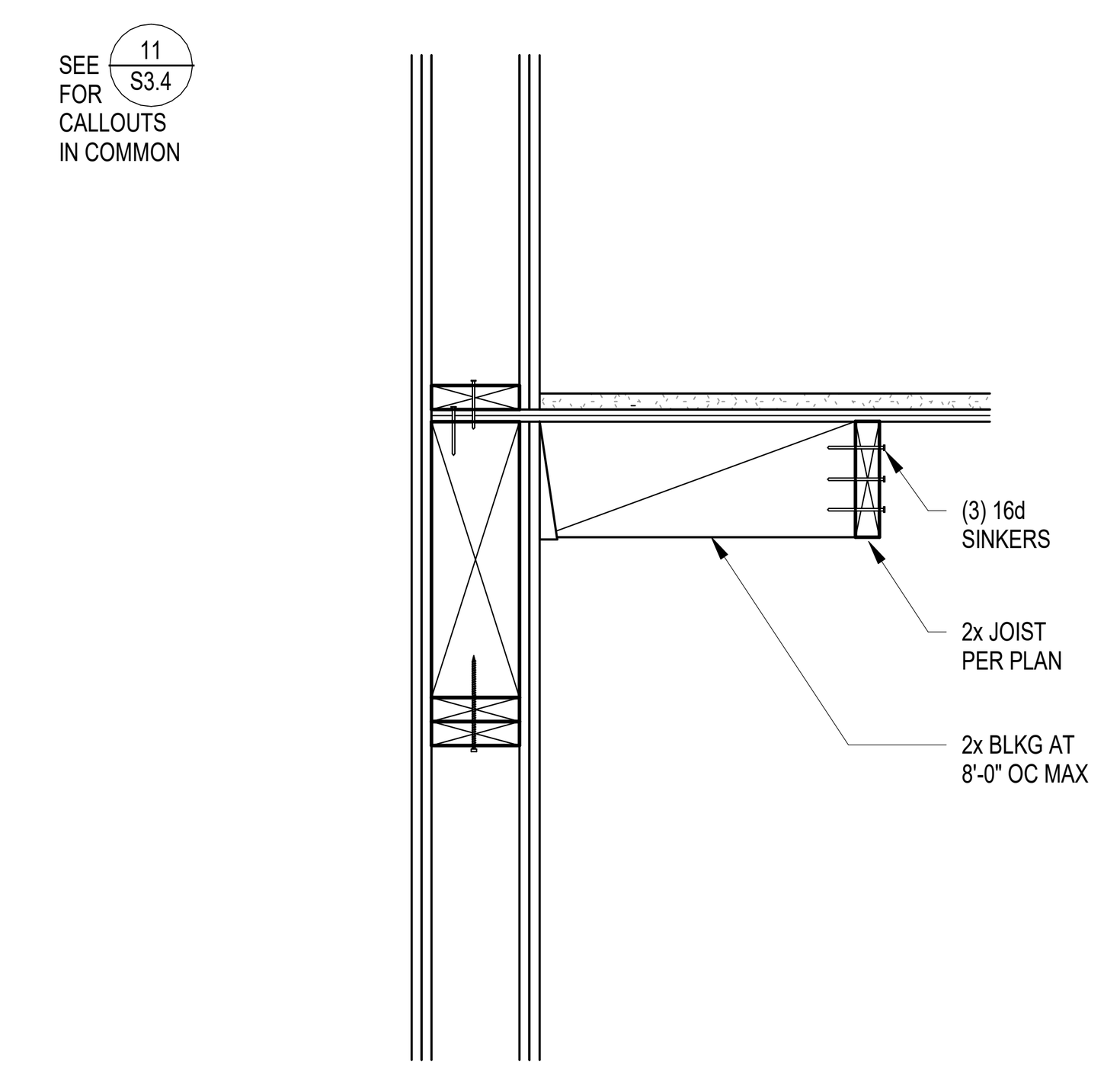


11 **SECTION**
1" = 1'-0" 9/ S3.4



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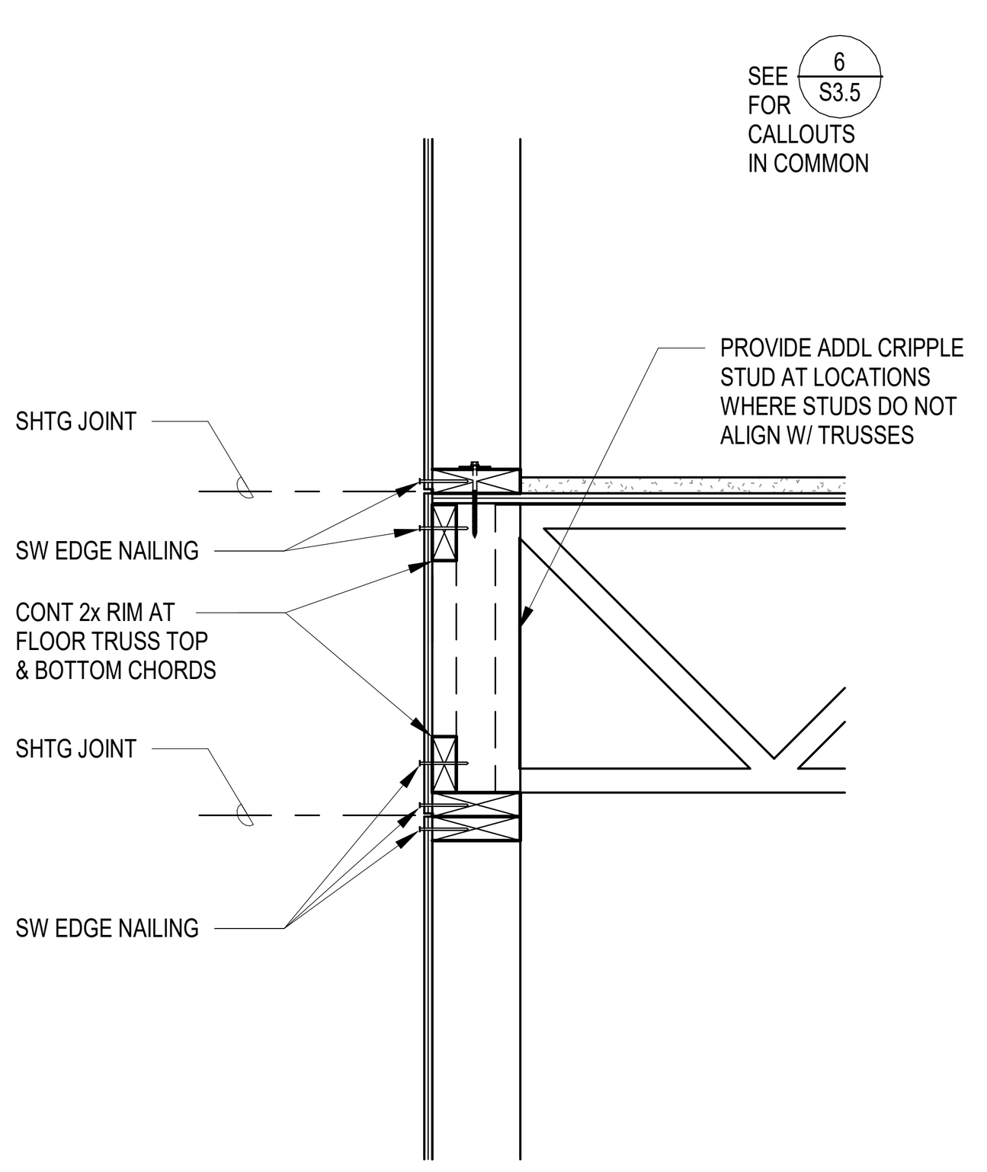

13 **SECTION**
1" = 1'-0" 11 / S3.4

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ANDREW DEAN McEACHRAN — STATE OF WASHINGTON — STRUCTURAL — PROFESSIONAL ENGINEER

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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

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PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 08/11/17

REVISIONS

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KJK DRAWN BY
ADM CHECKED BY

WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER

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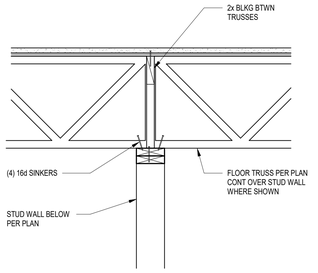
FLOOR FRAMING DETAILS

S3.4



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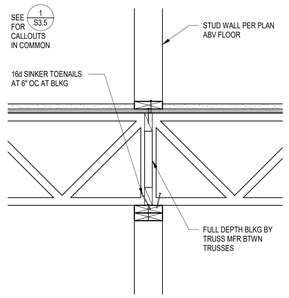

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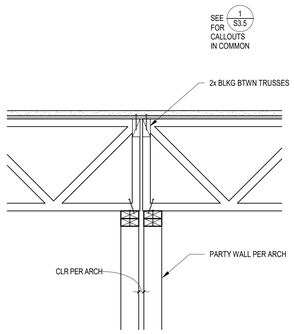

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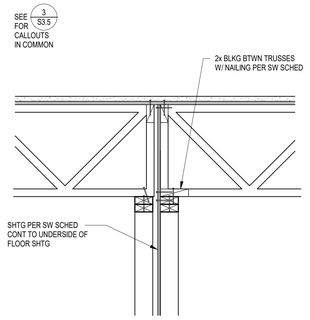
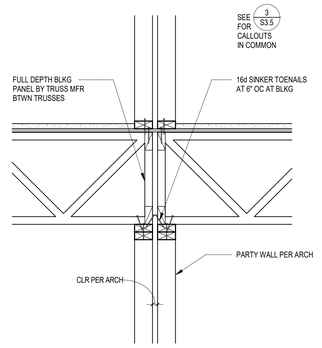


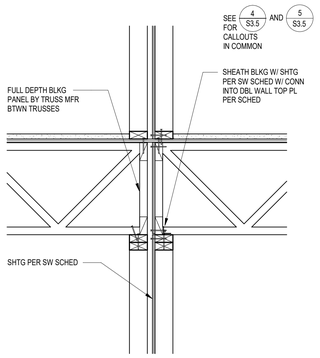
4 SECTION

1" = 1'-0" 4 / S3.5



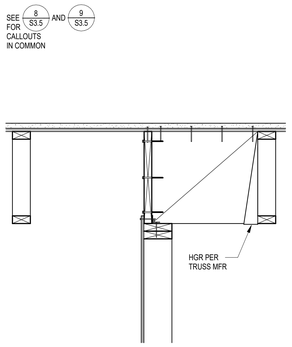
5 SECTION

1" = 1'-0" 5 / S3.5



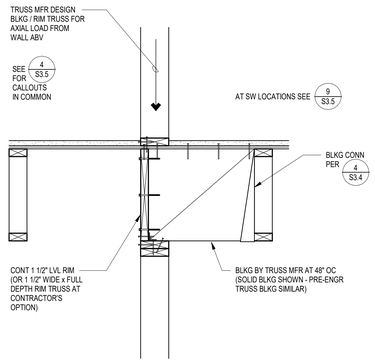
6 SECTION

1" = 1'-0" 6 / S3.5



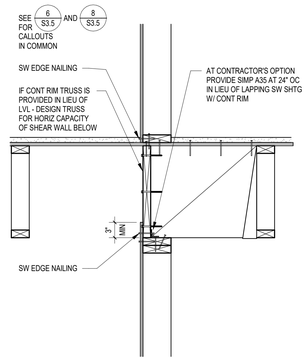
7 SECTION

1" = 1'-0" 7 / S3.5



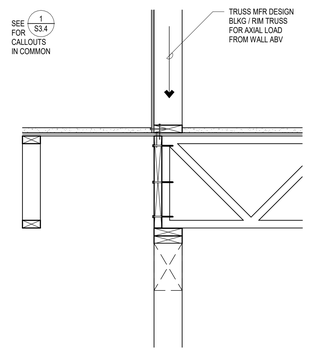
8 SECTION

1" = 1'-0" 8 / S3.5



9 SECTION

1" = 1'-0" 9 / S3.5



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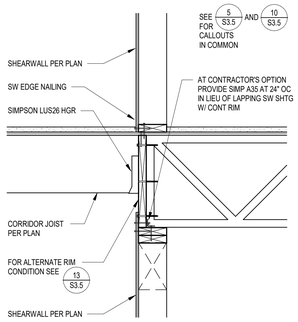

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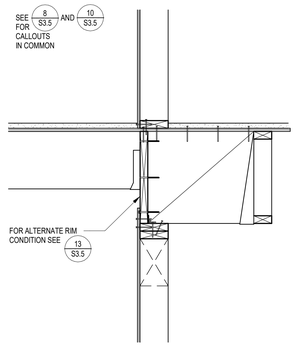

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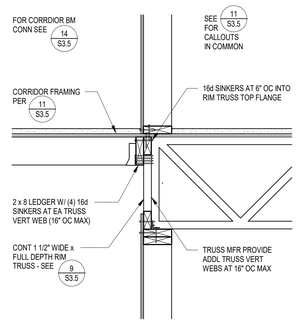

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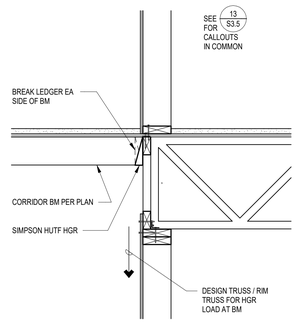

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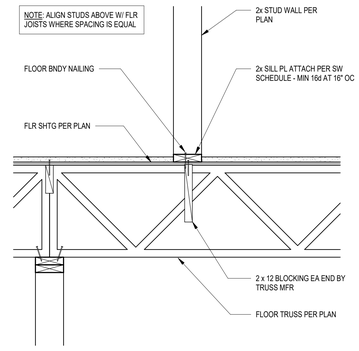

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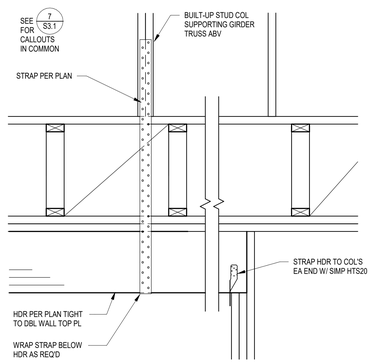

16 SECTION

1" = 1'-0" 16 / S3.5

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WESLEY BRADLEY PARK PHASE 2 - CARE CENTER

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PERMIT SET
DECEMBER 2, 2024

ORIGINAL ISSUE: 08/11/17

REVISIONS

No.	Description	Date

2220236.20
PROJECT NUMBER

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PHASE 2 - CARE CENTER

FLOOR FRAMING DETAILS

S3.5

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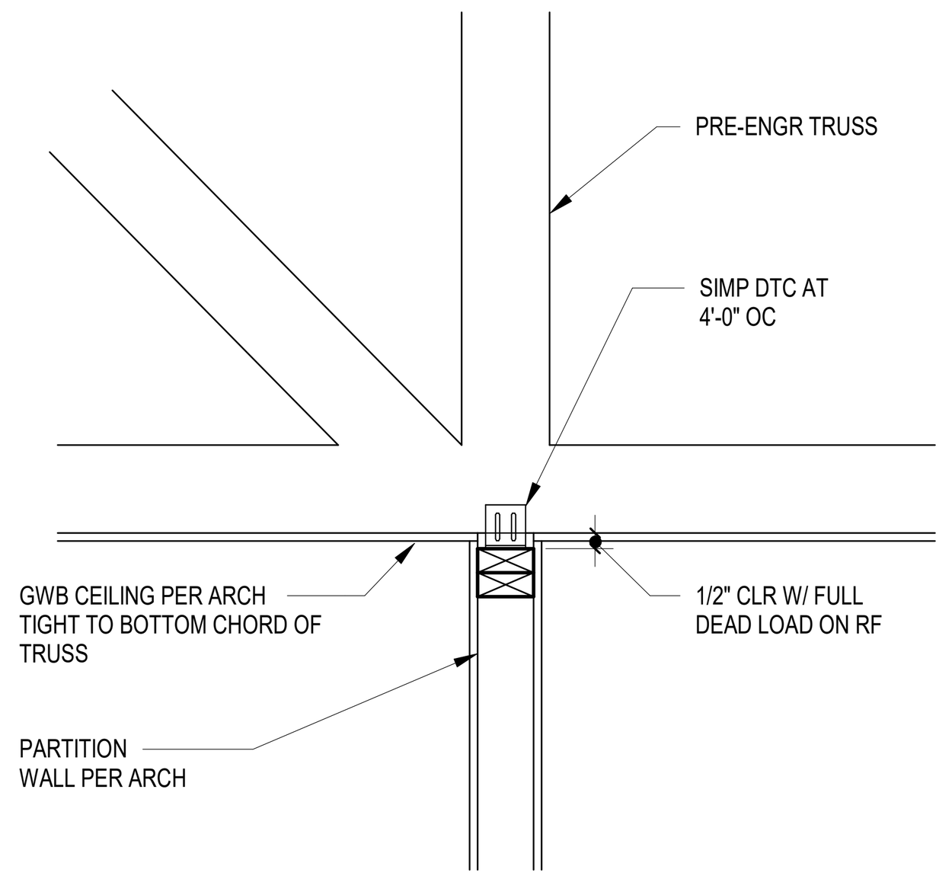

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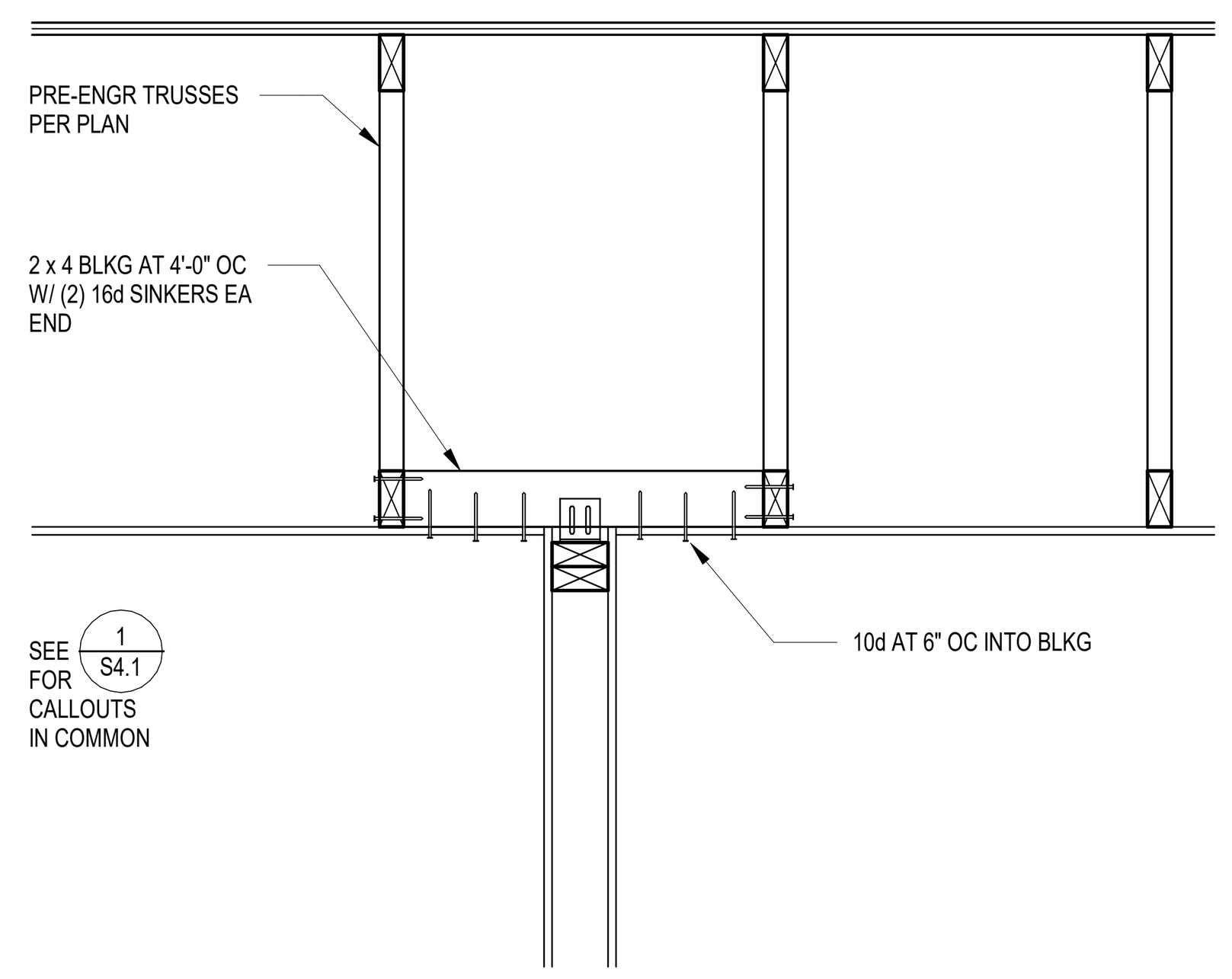

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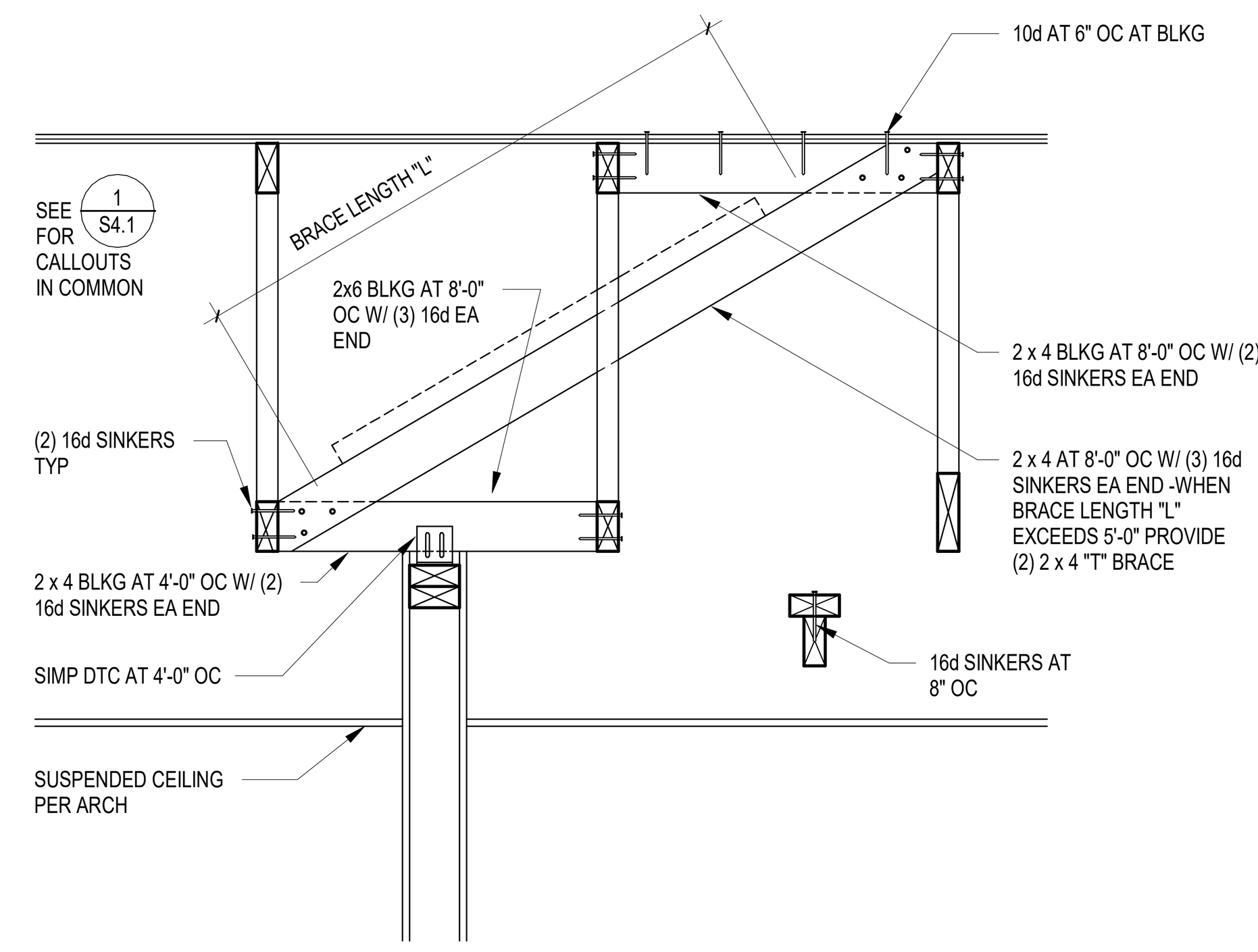

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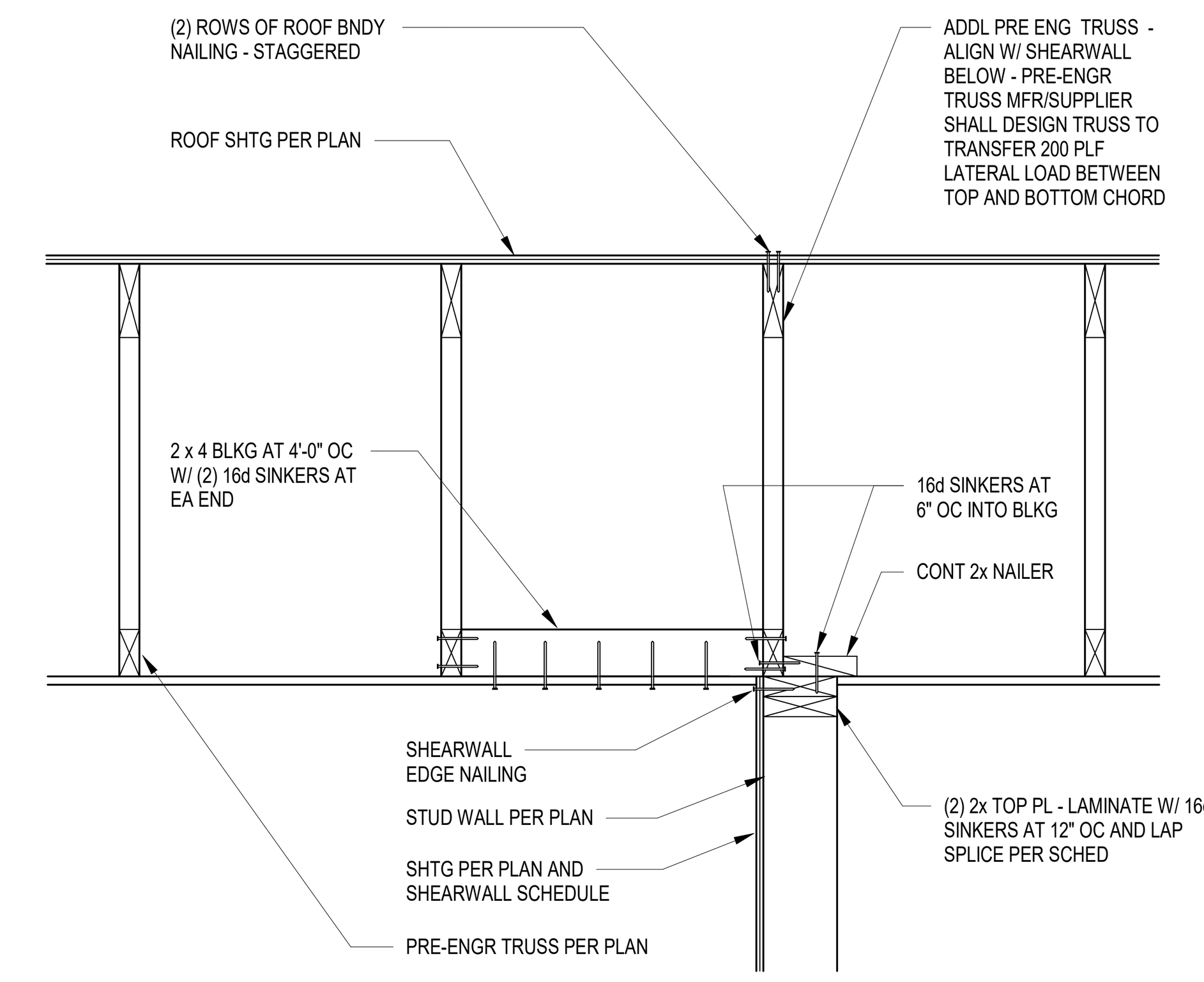

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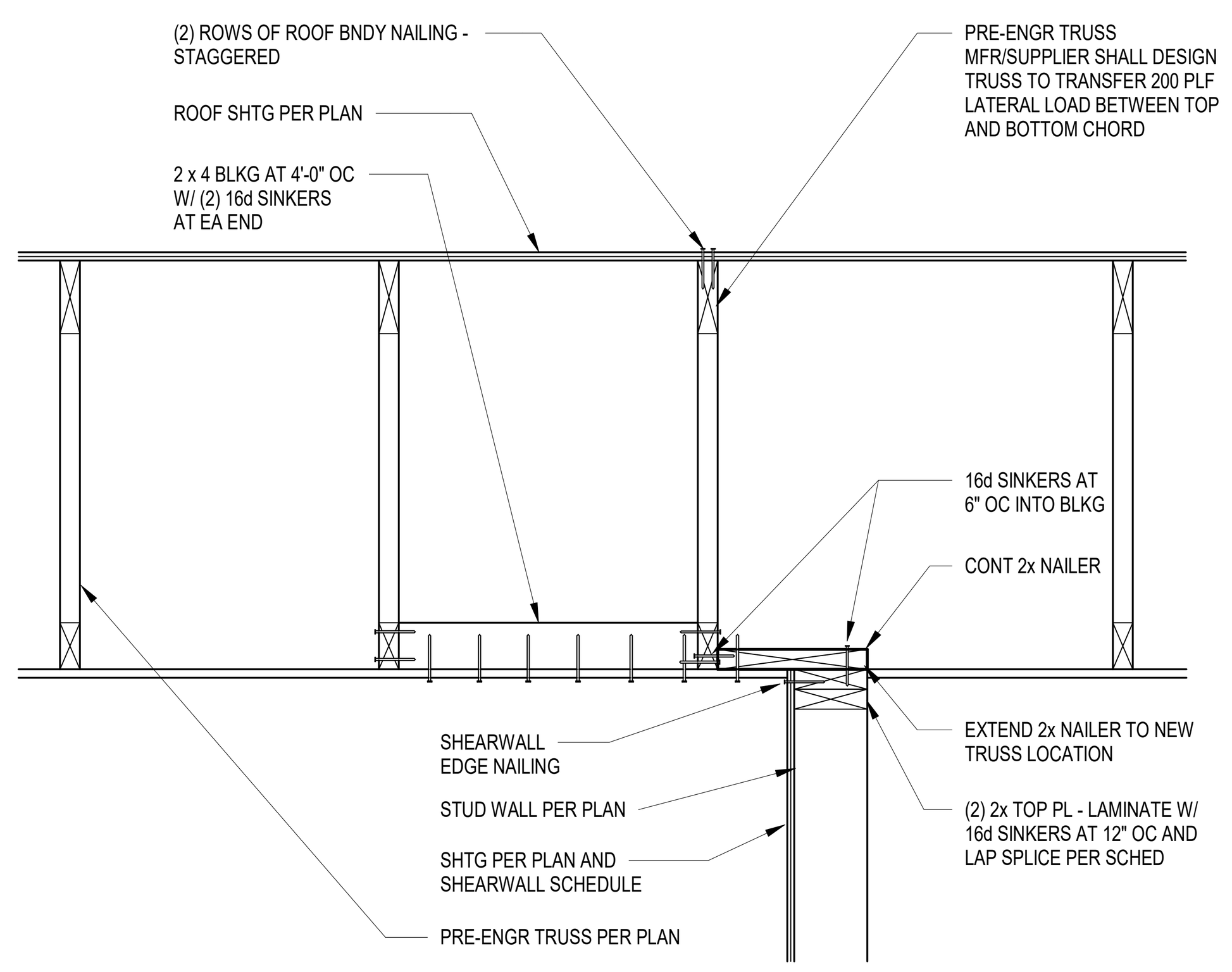

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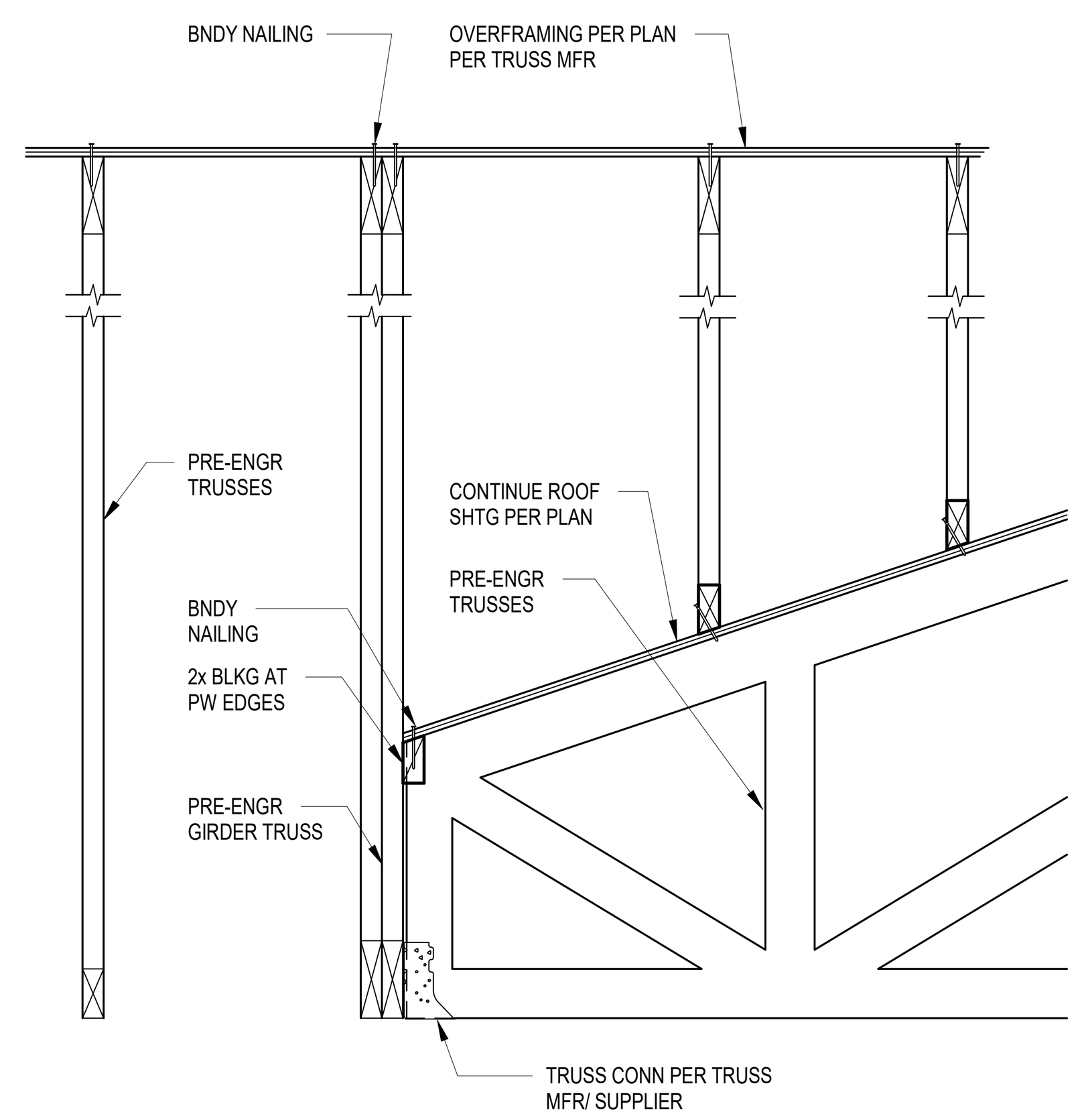

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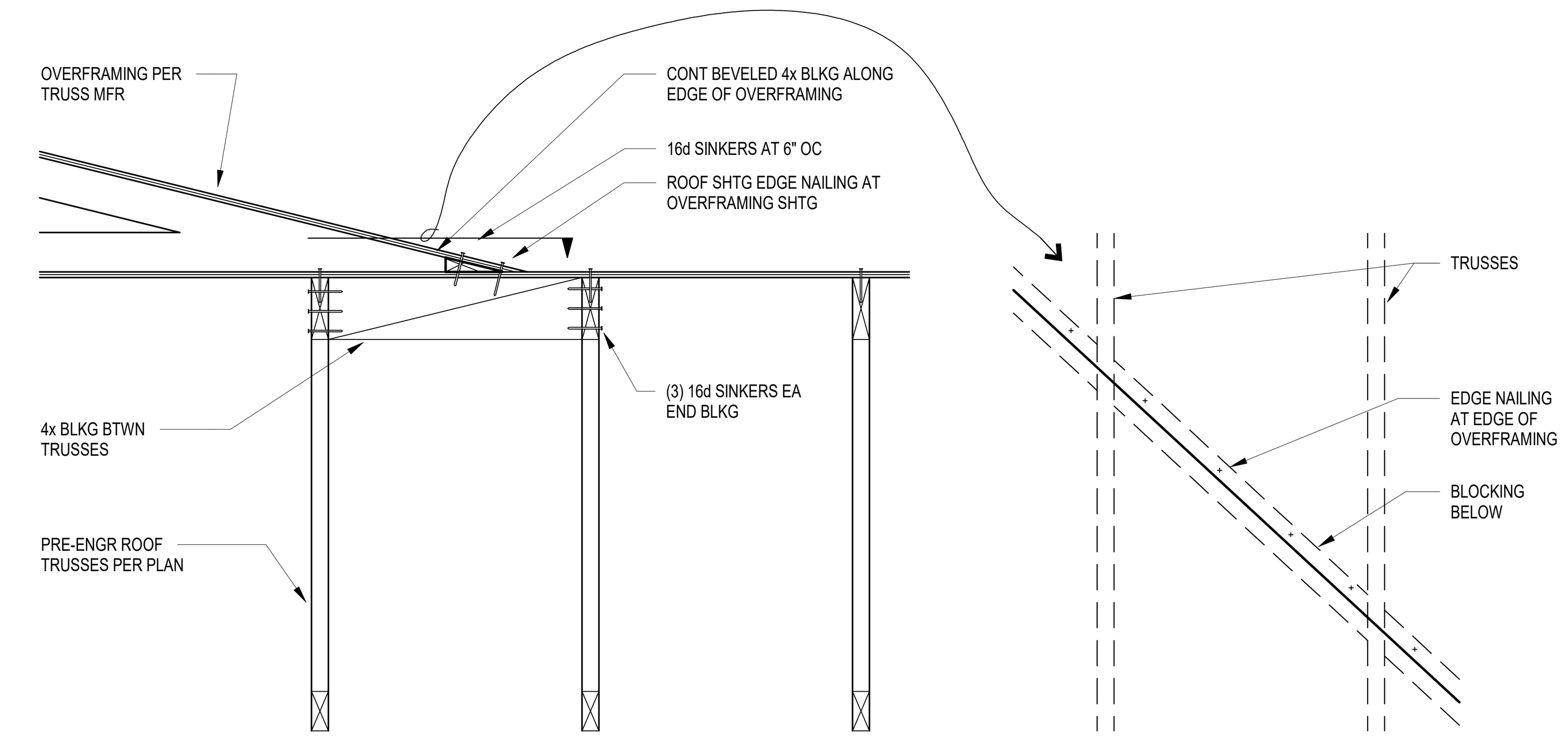

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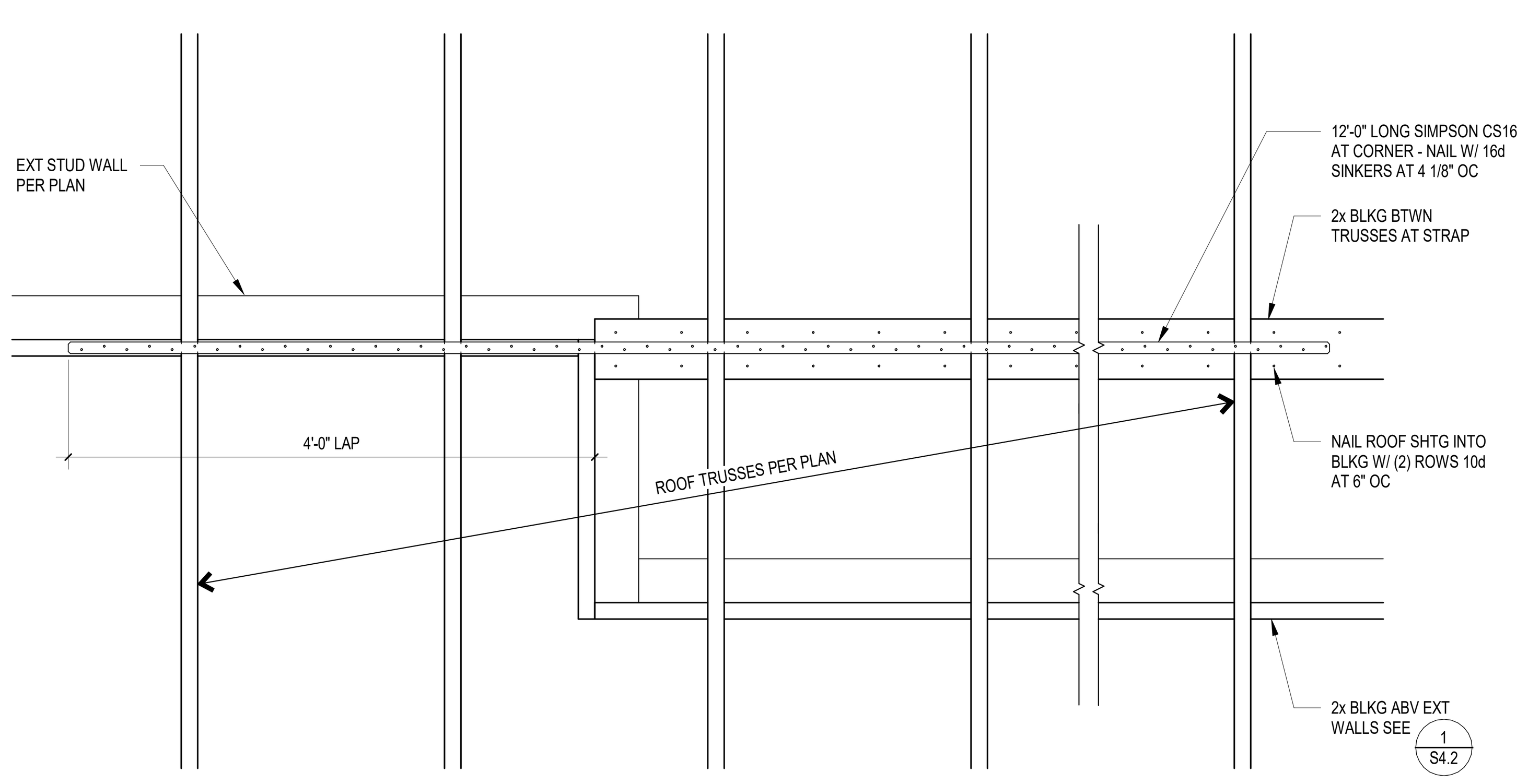
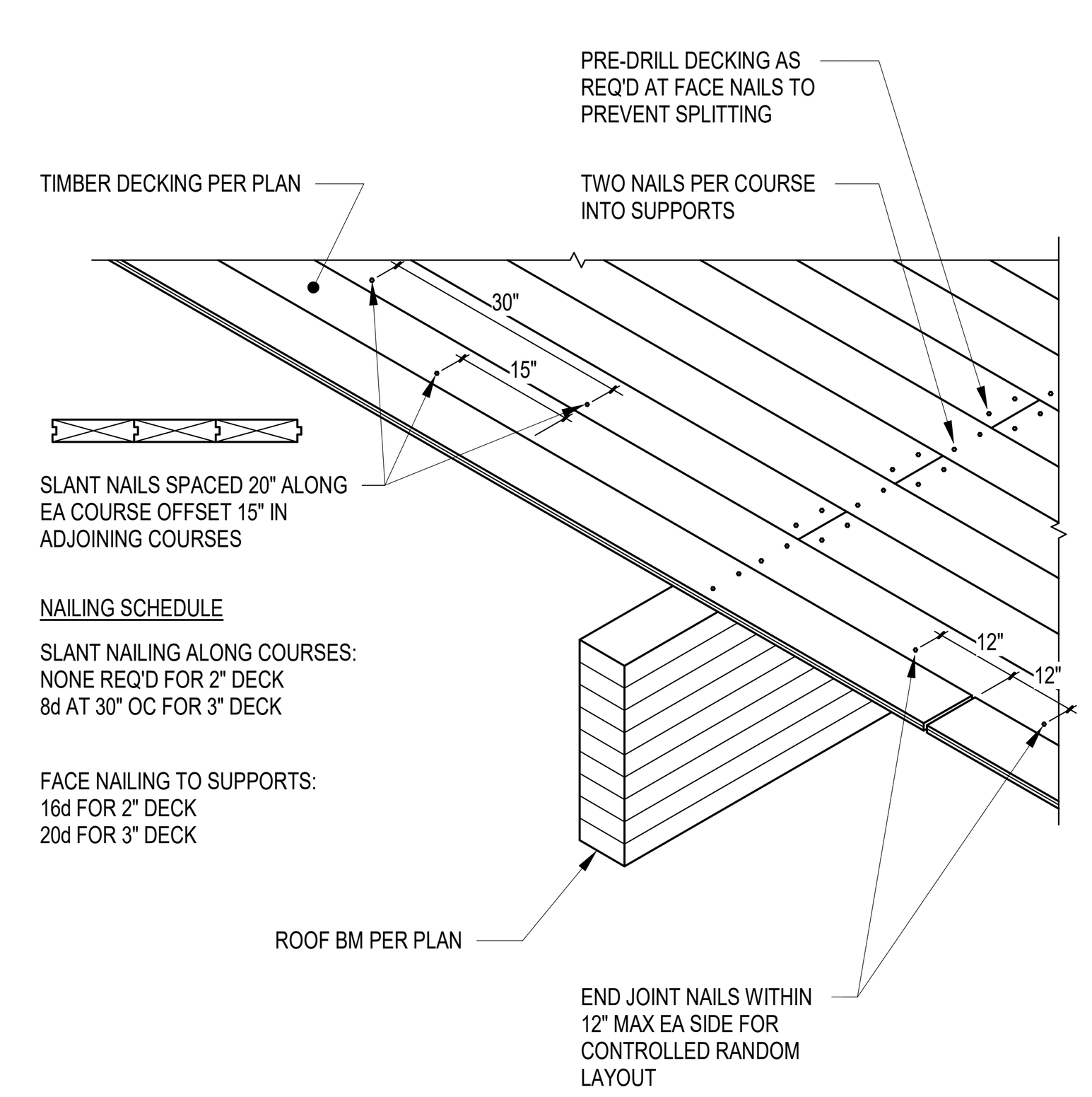
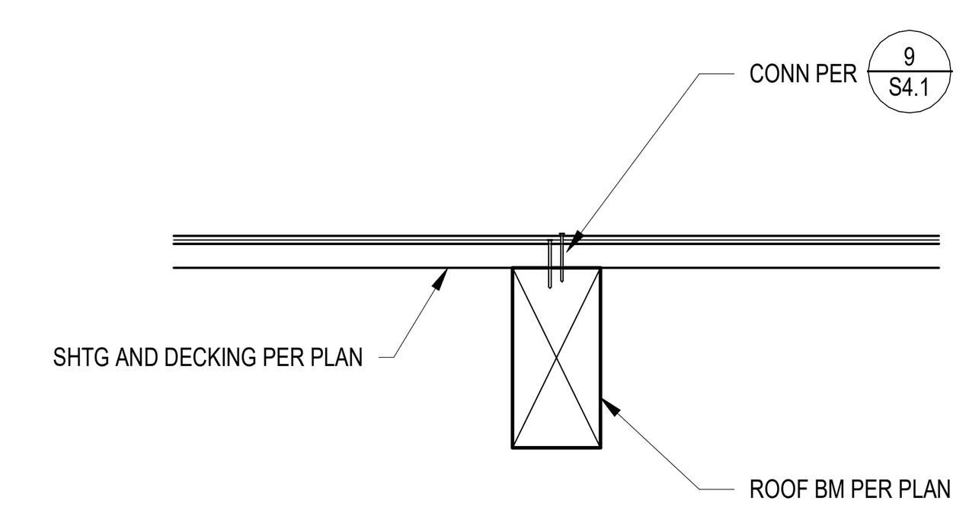


ROOF CHORD SPLICES

8 PLAN
1" = 1'-0" 8 / S4.1



9 SECTION
1" = 1'-0" 9 / S4.1



10 SECTION
1" = 1'-0" 10 / S4.1

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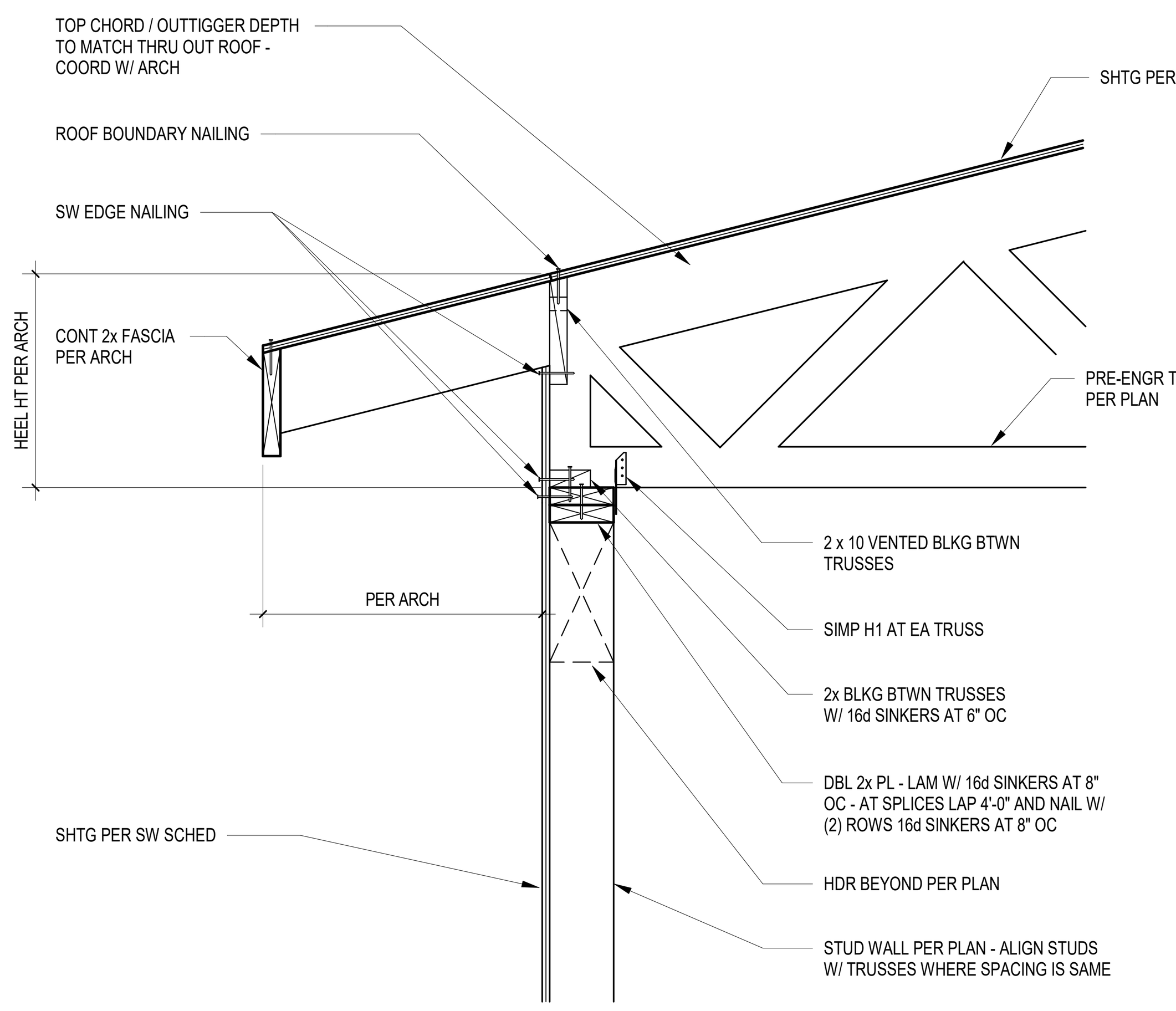
S4.1



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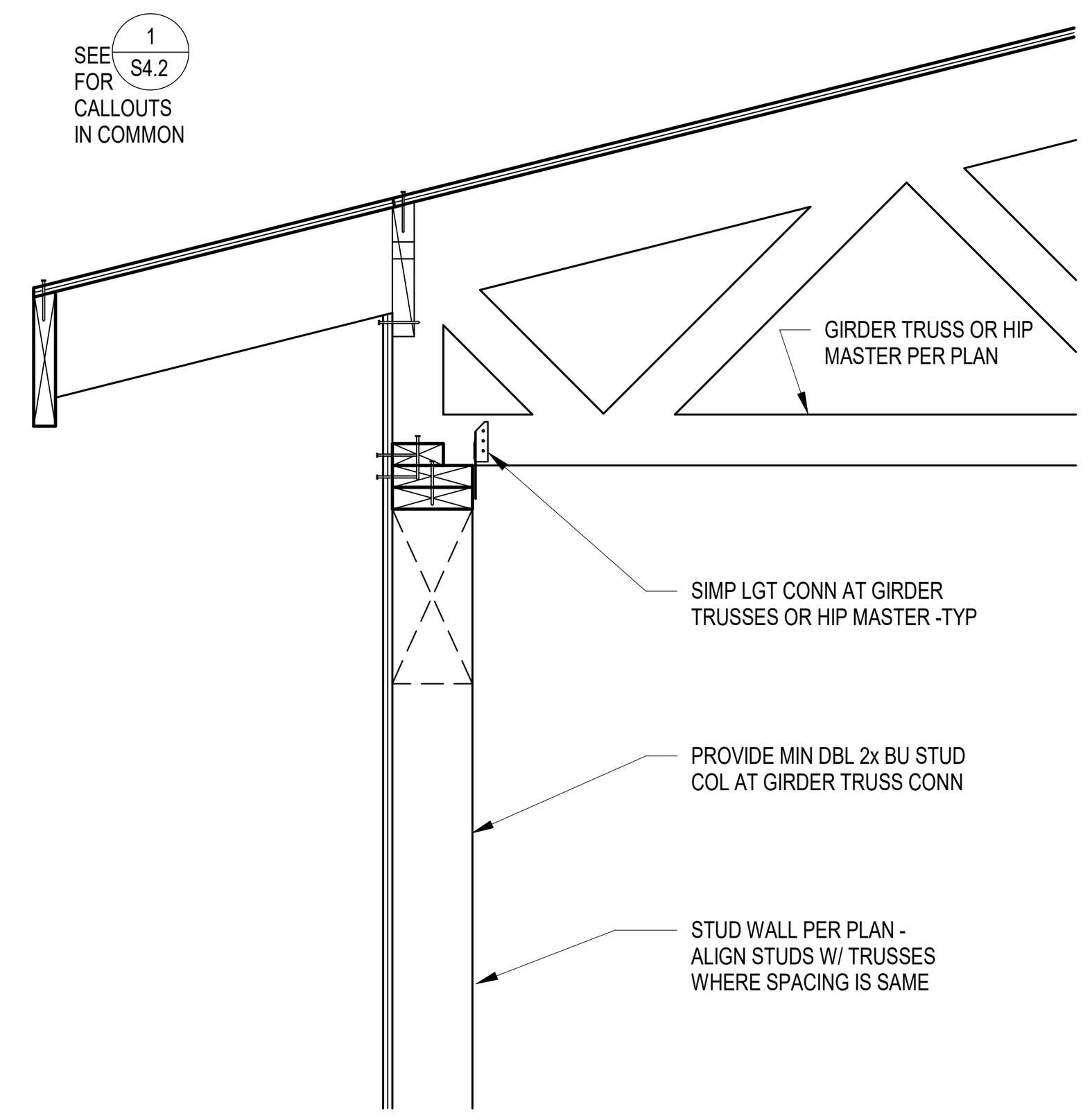

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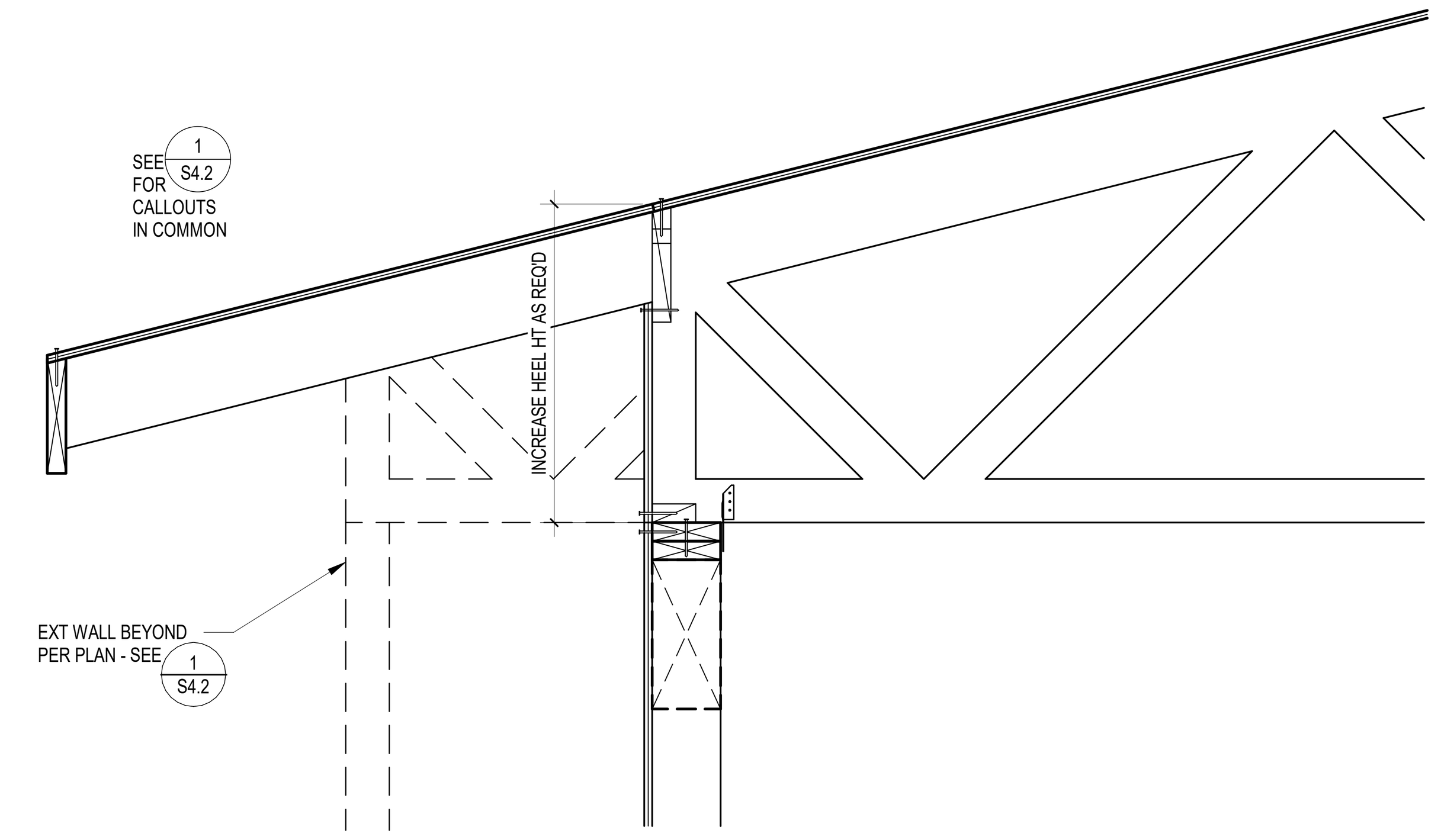

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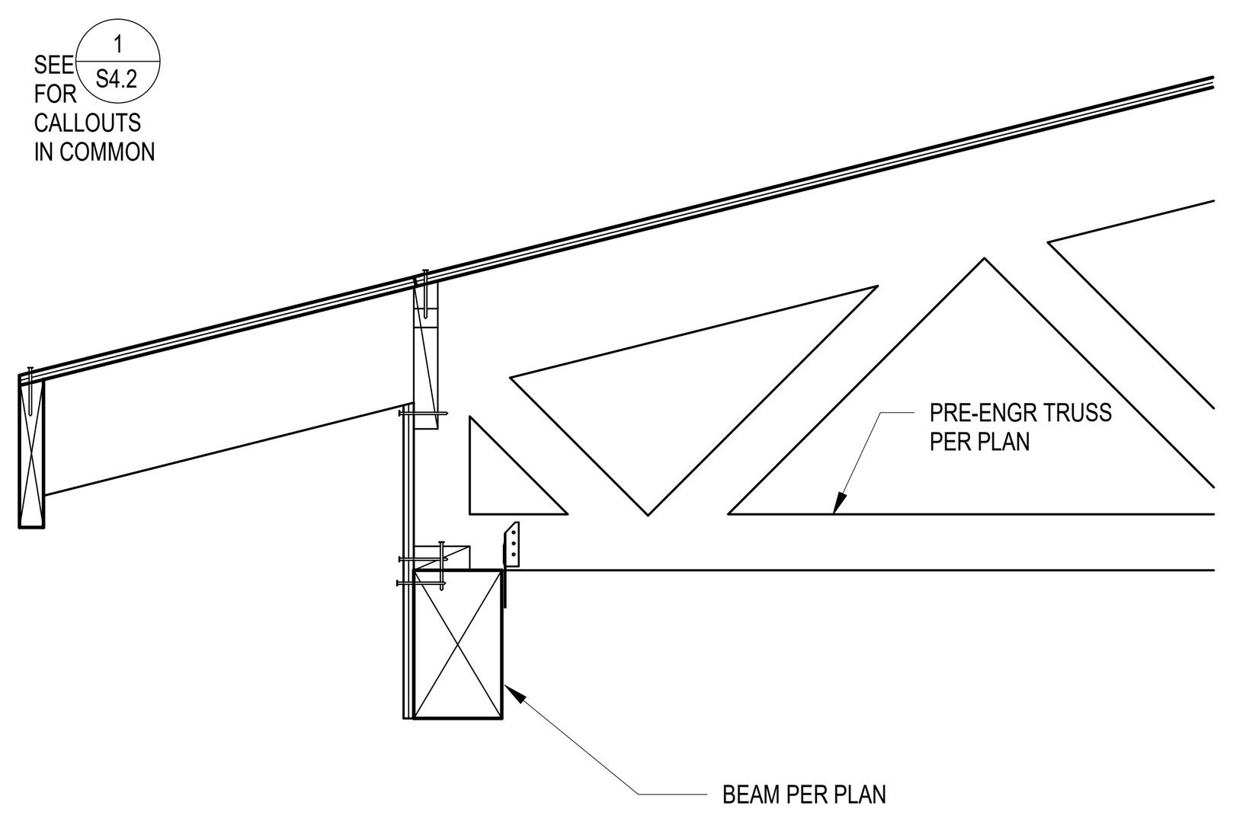

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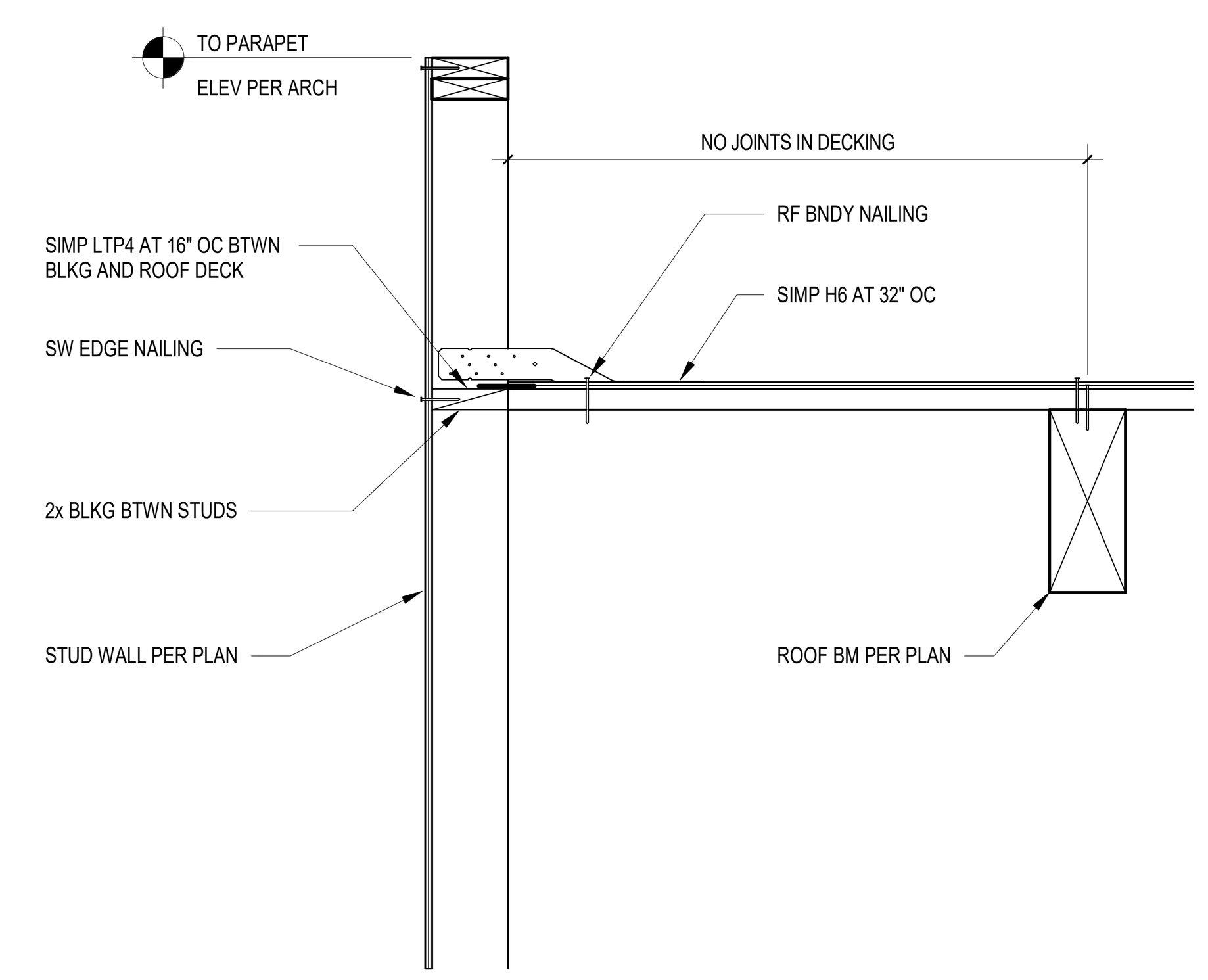

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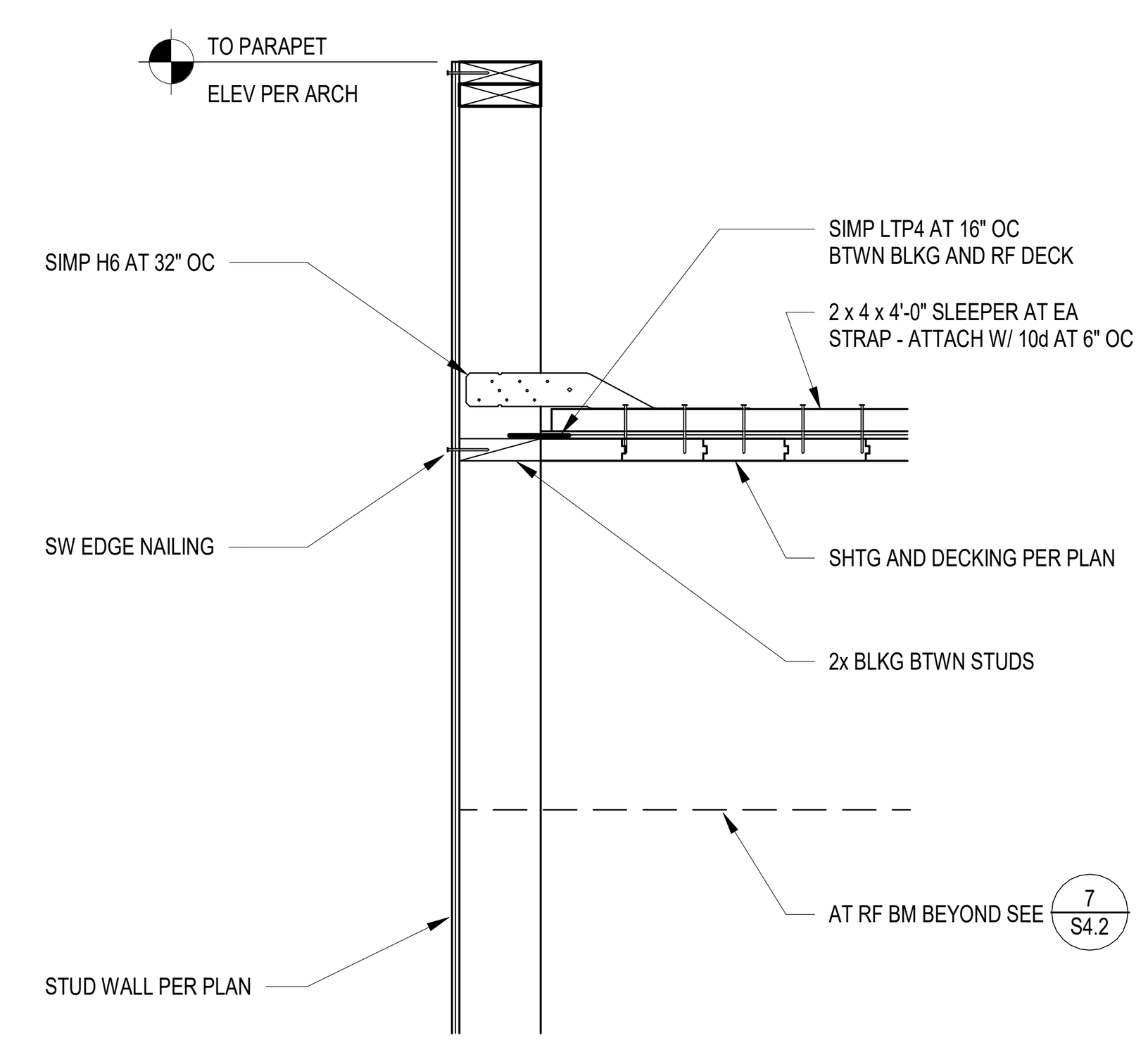

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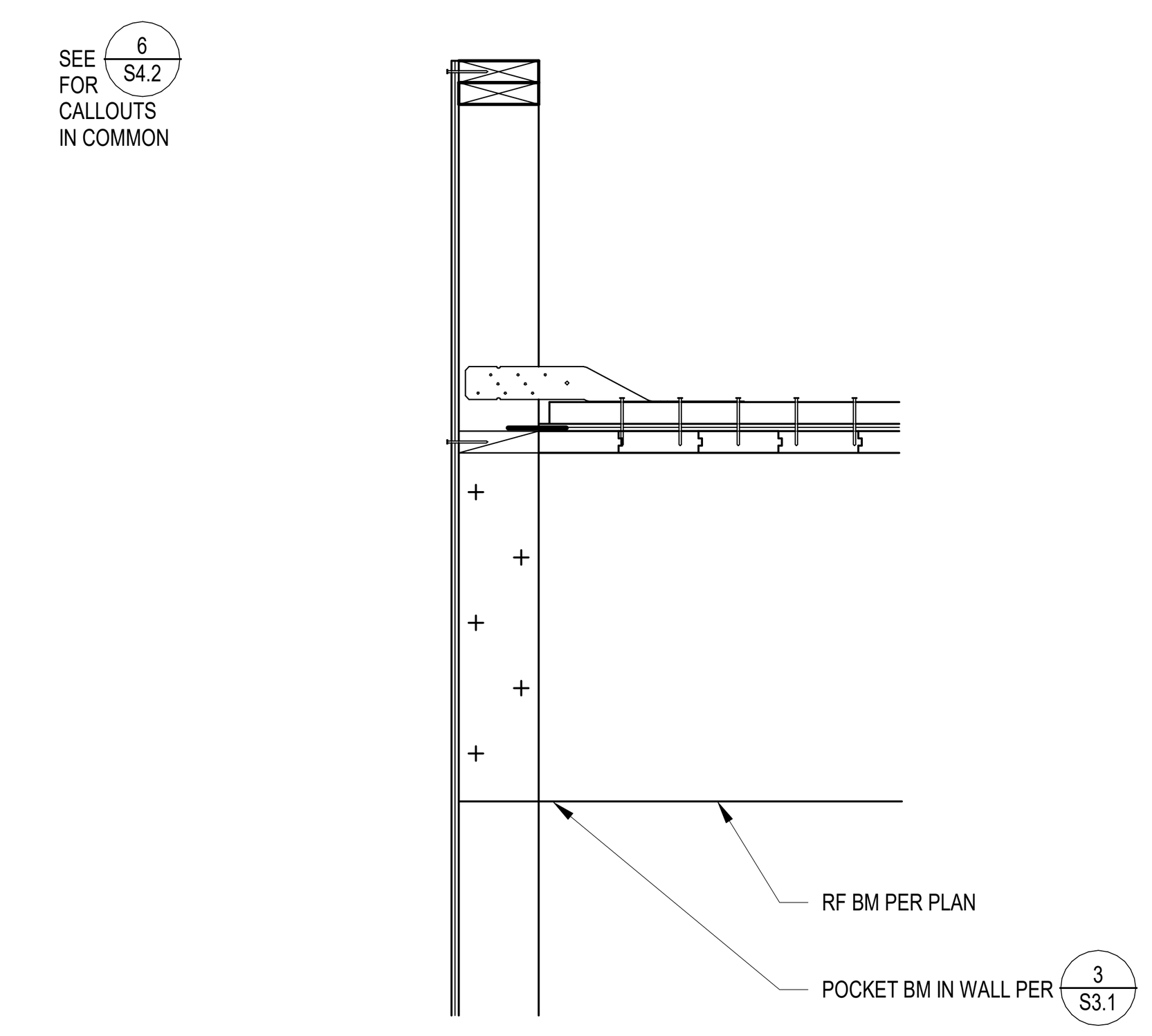

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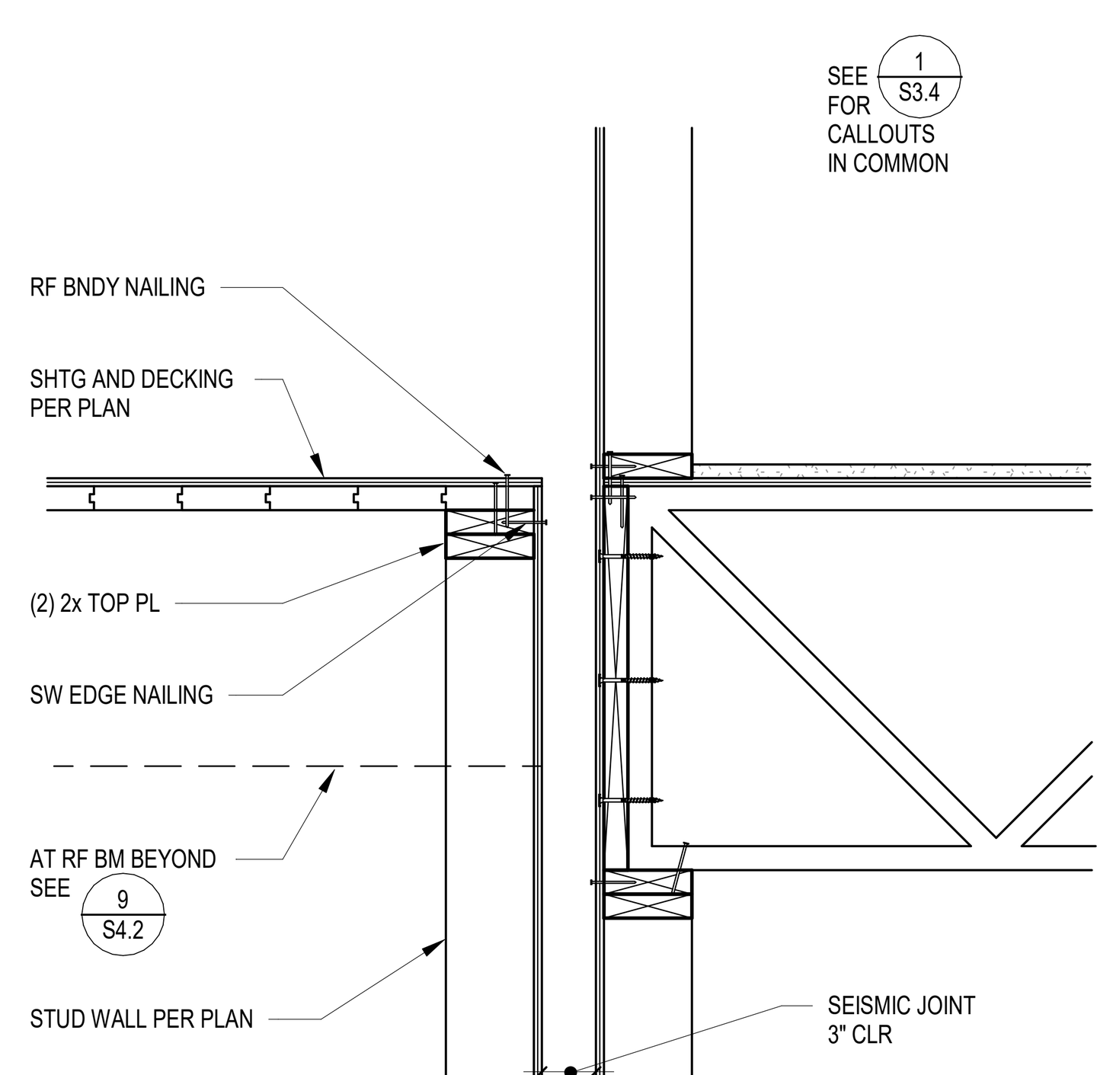
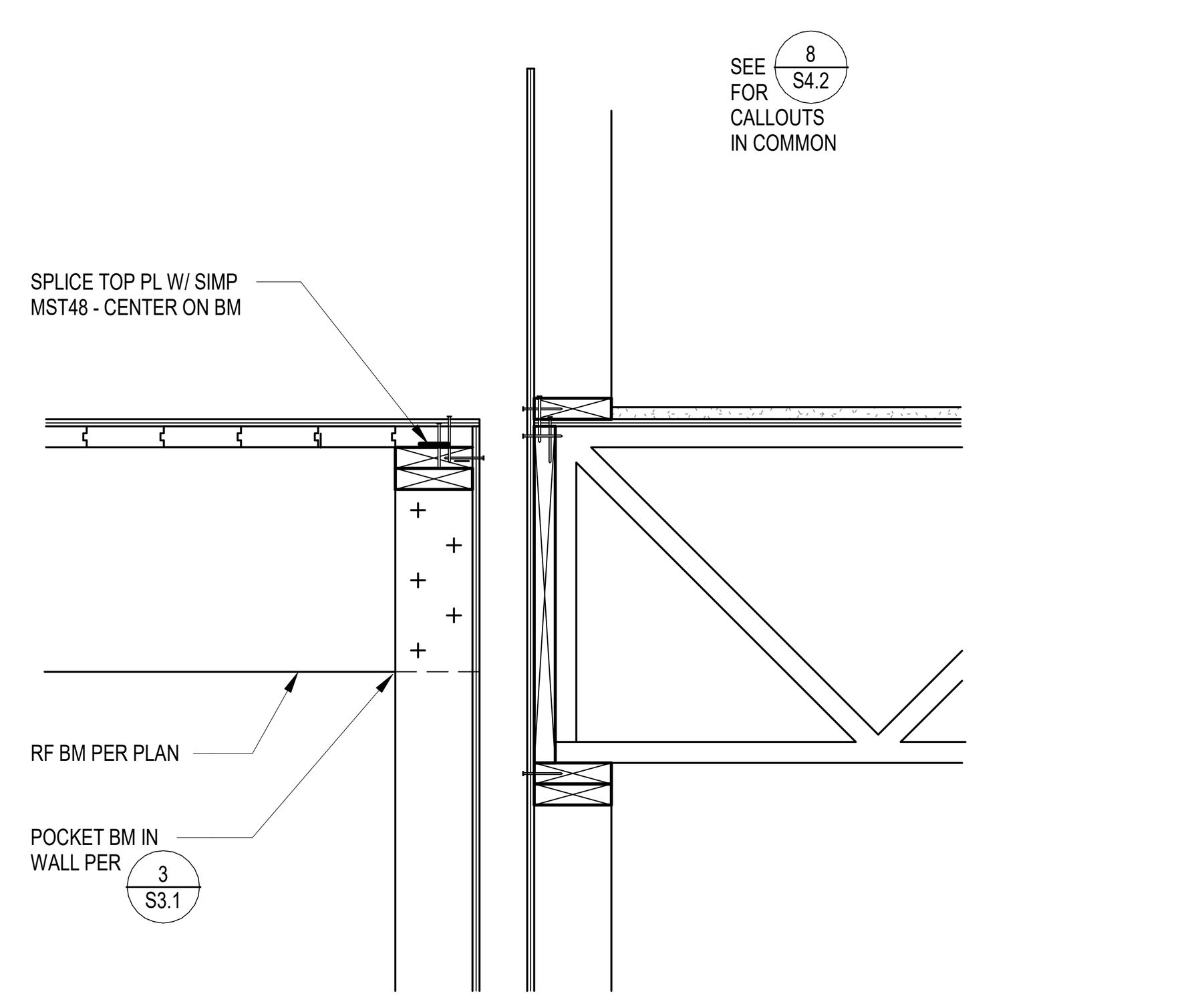


8 **SECTION**
1" = 1'-0" 8 / S4.2



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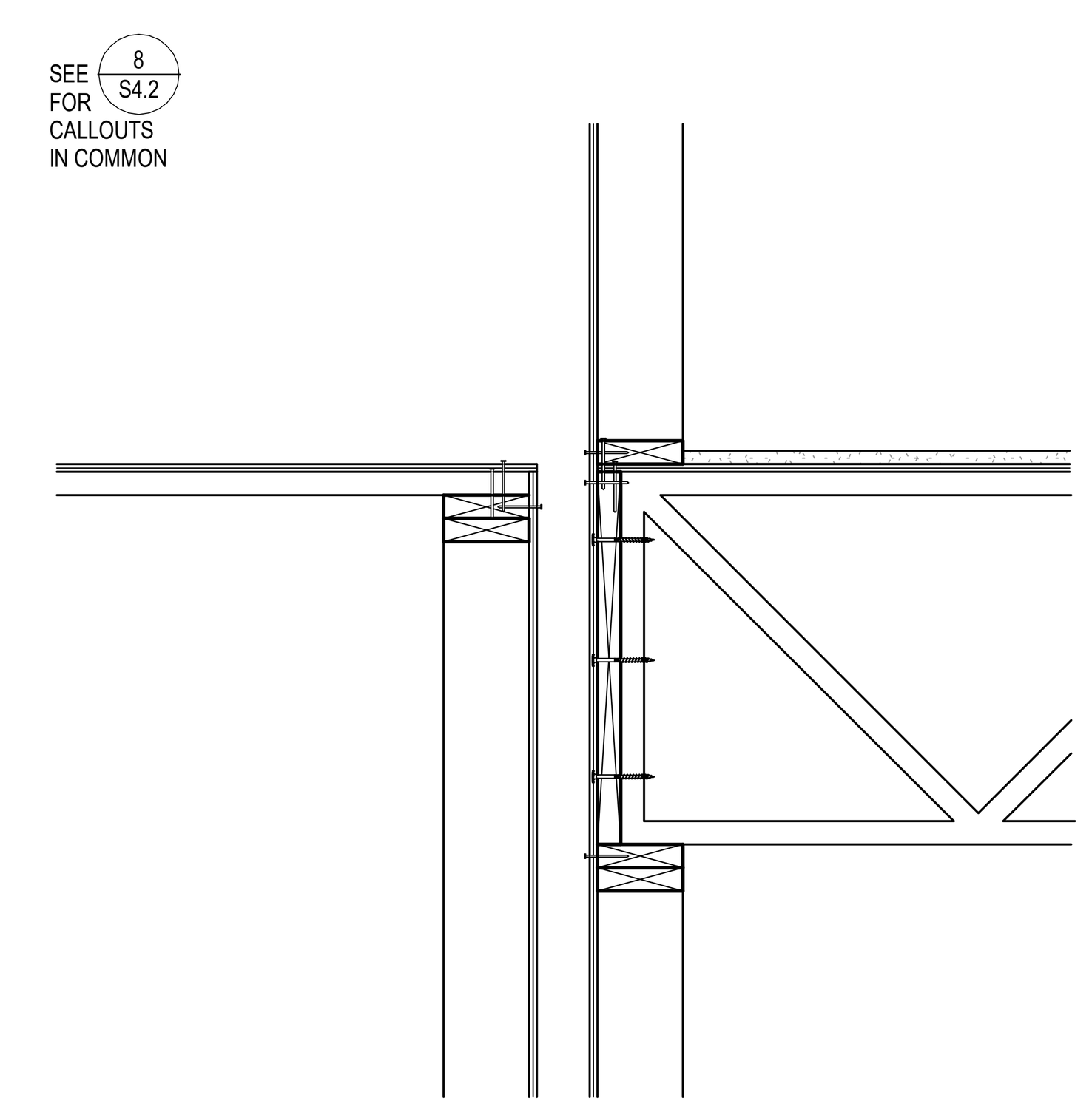

10 **SECTION**
1" = 1'-0" 10 / S4.2

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PERMIT SET
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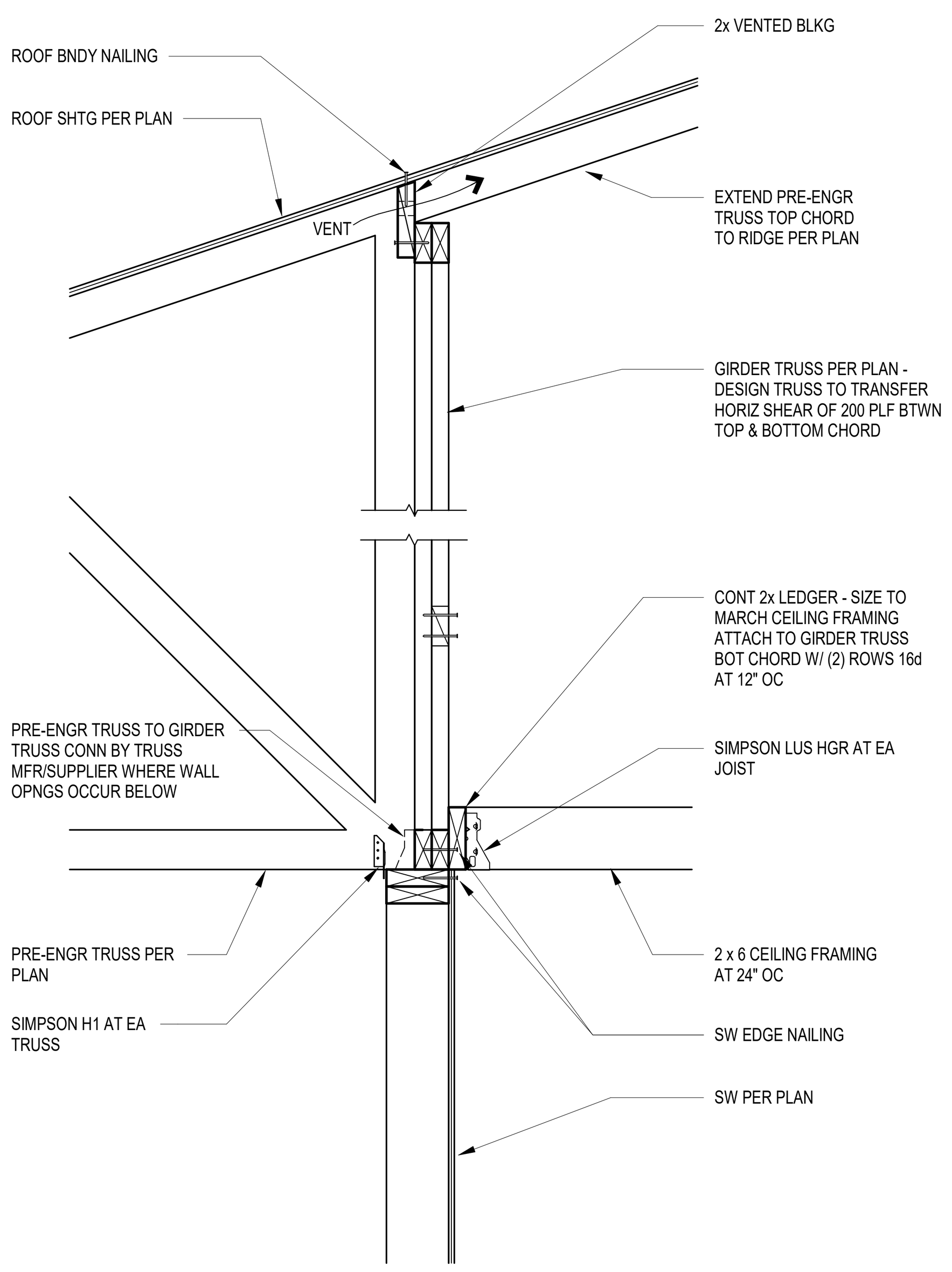
ROOF FRAMING DETAILS
S4.2



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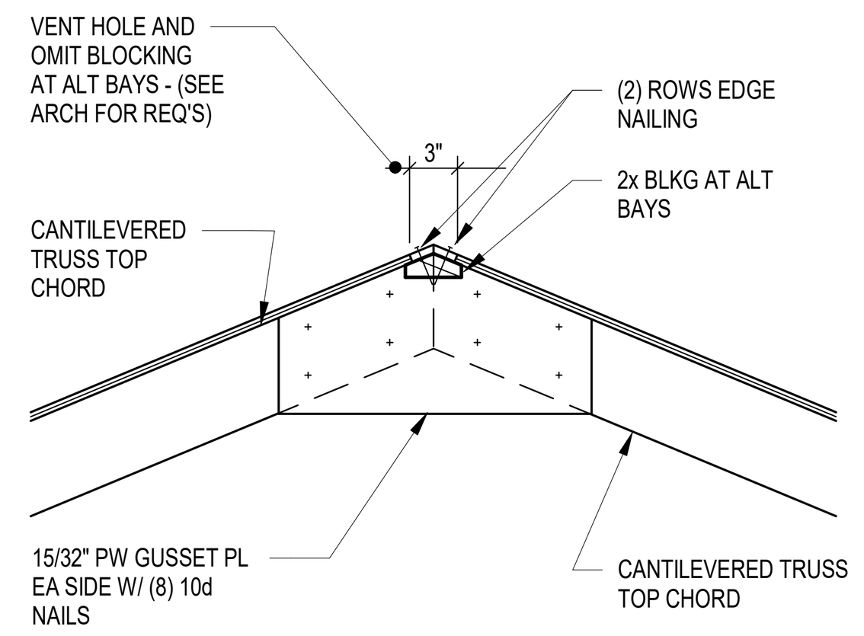

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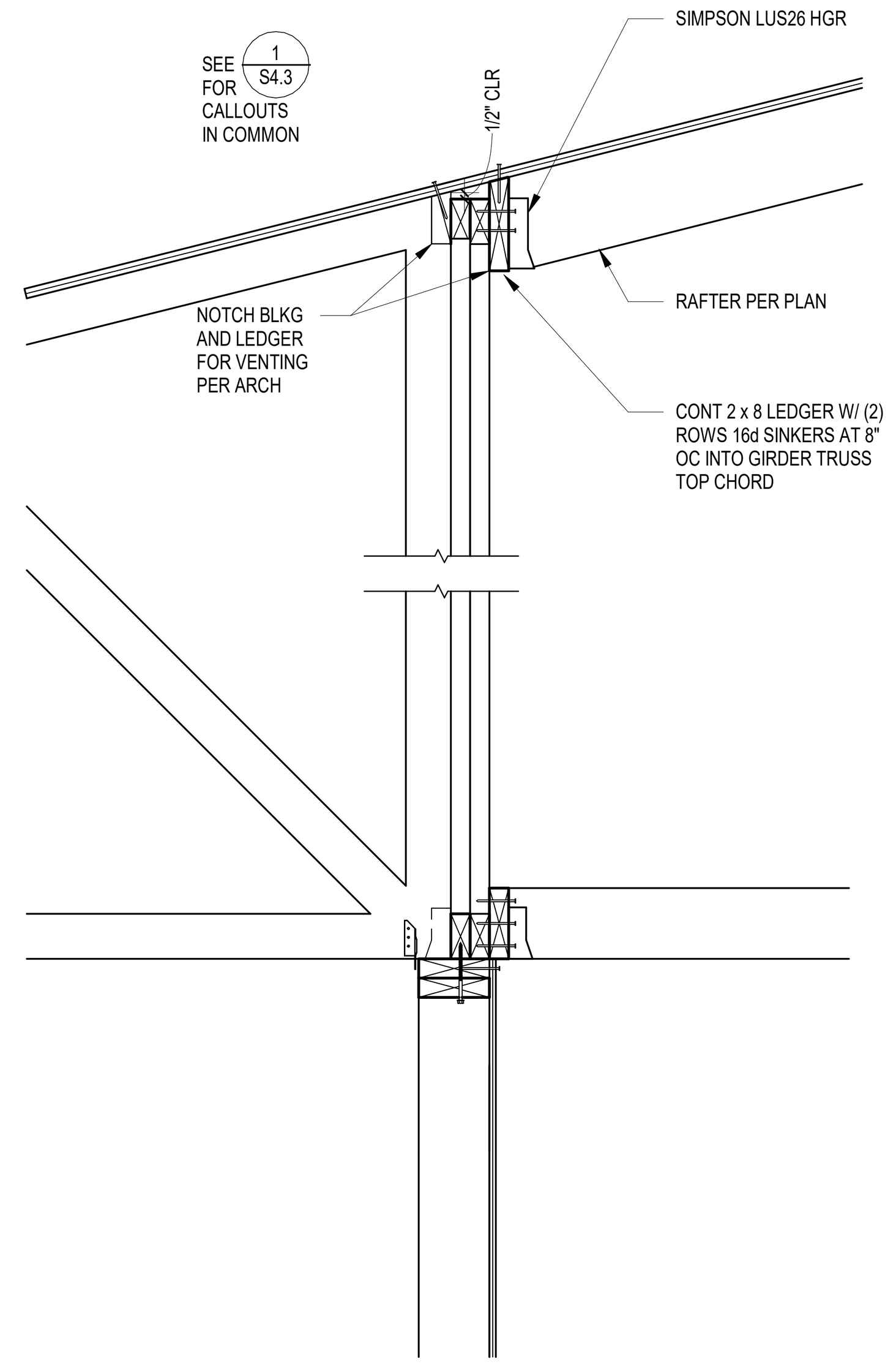

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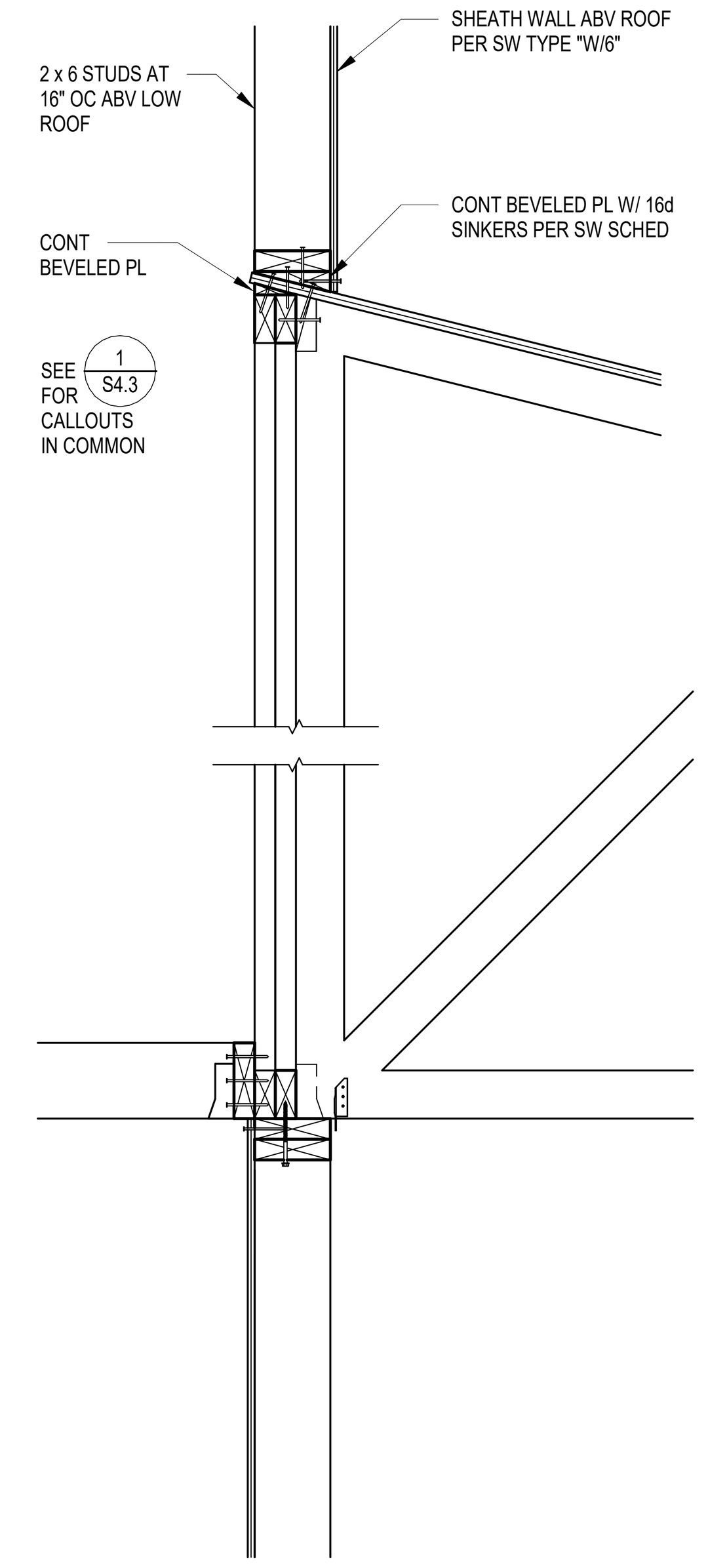

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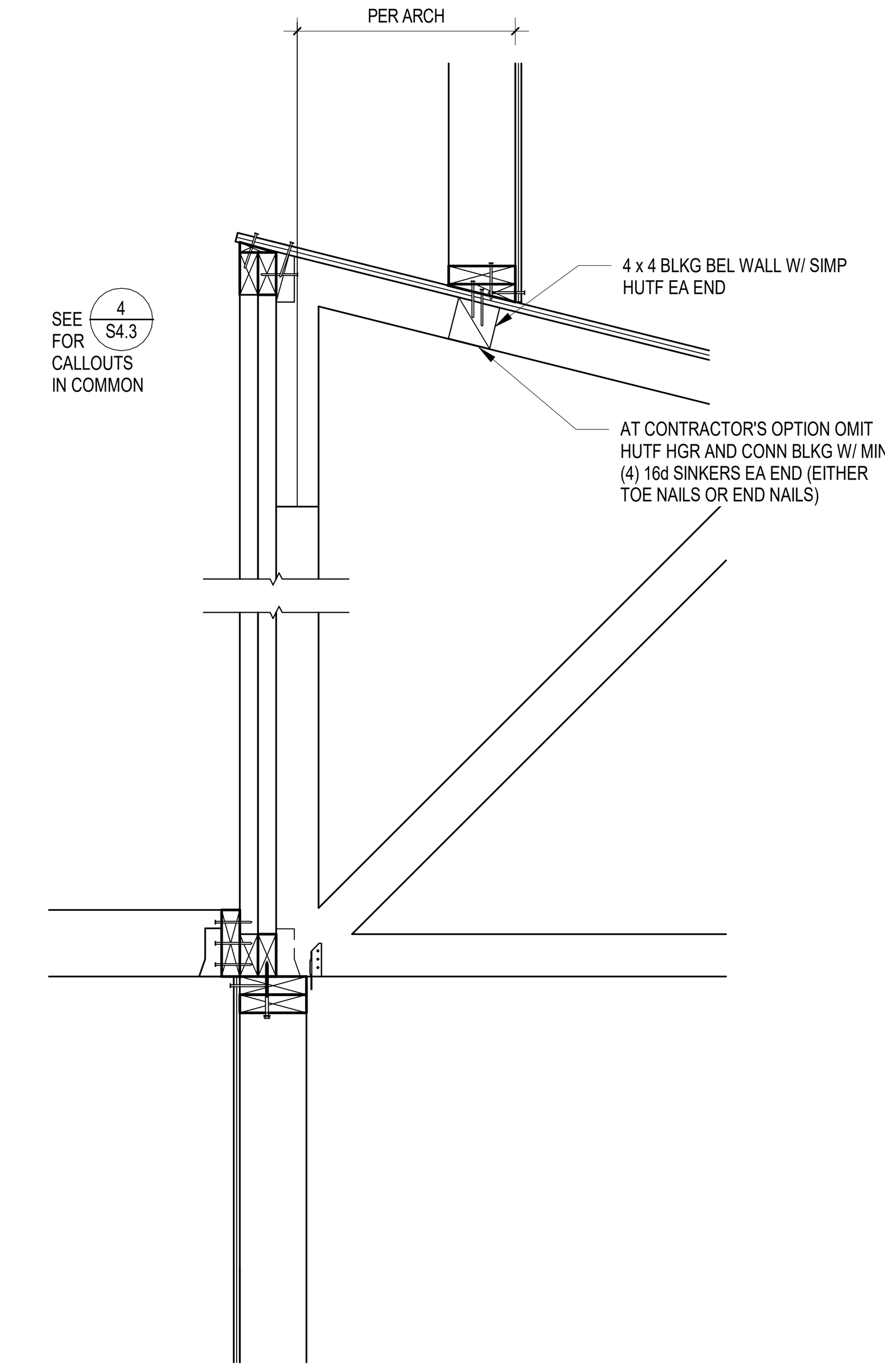

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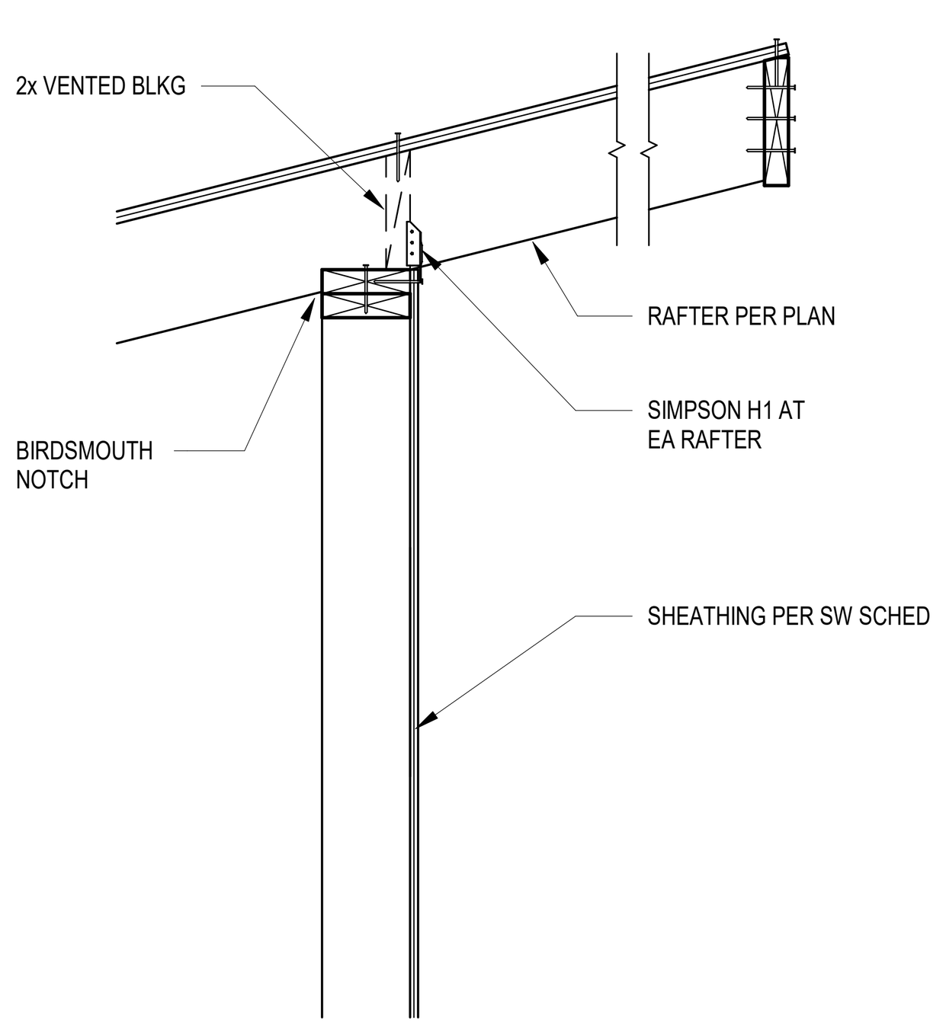

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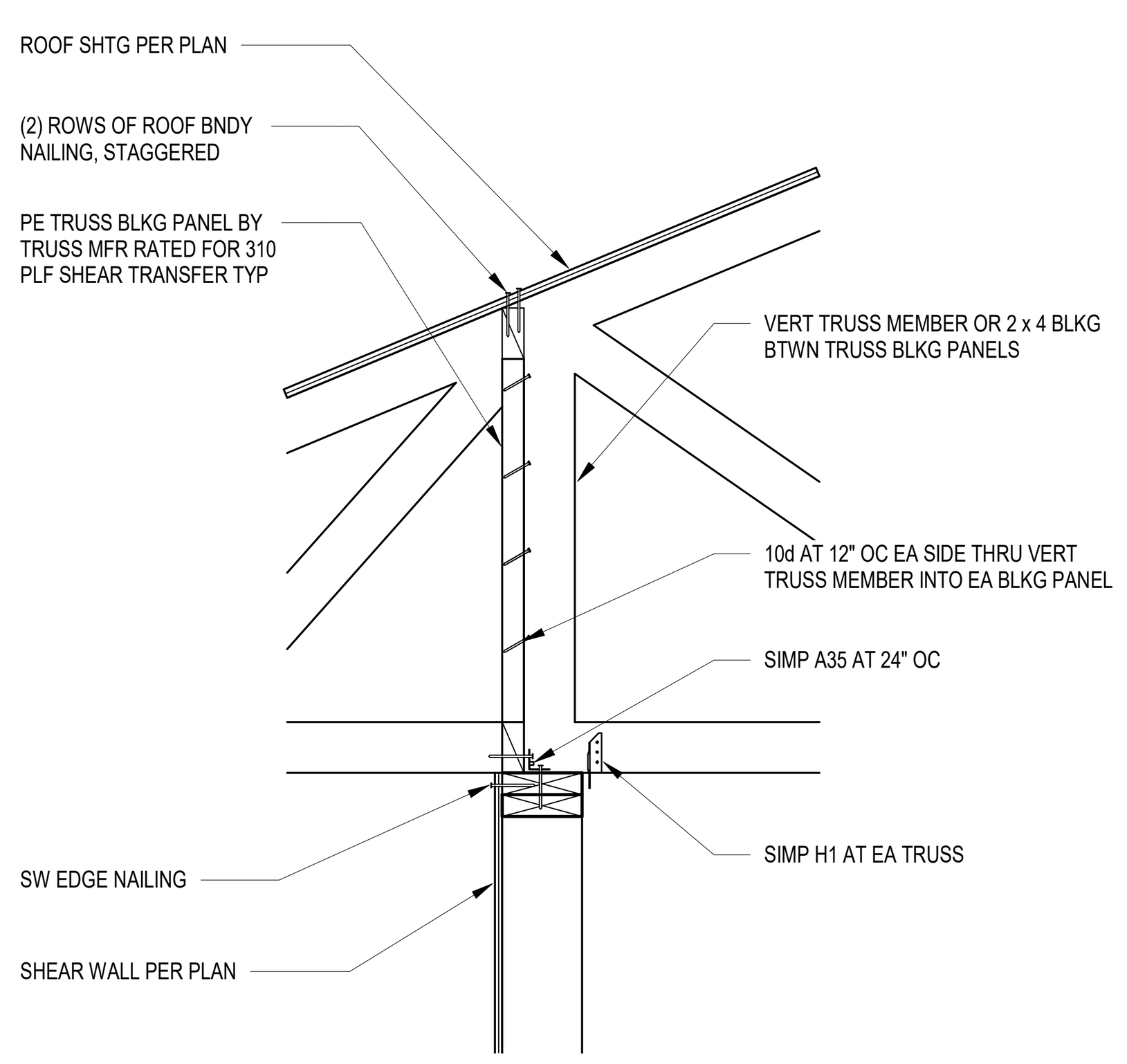

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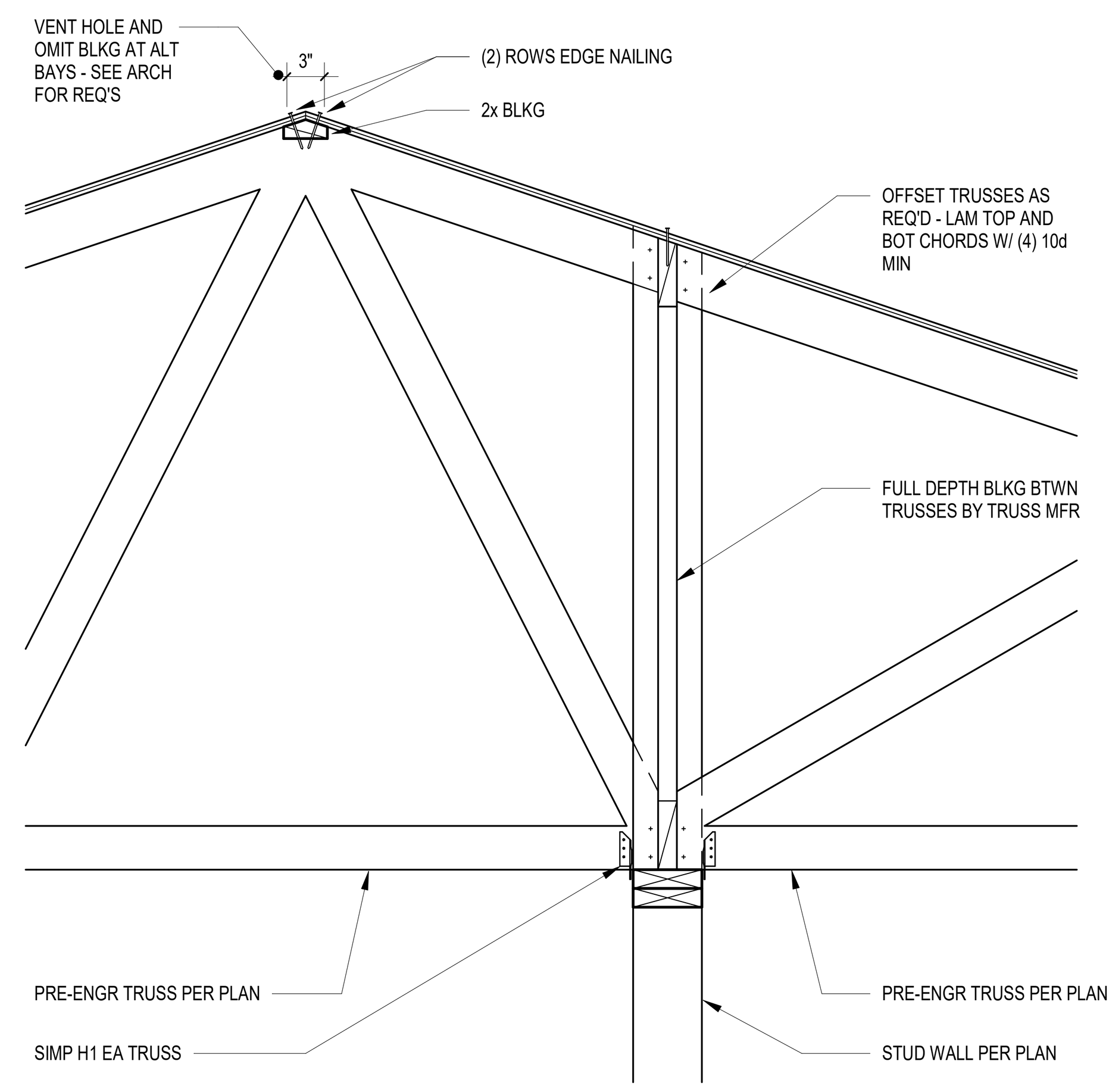

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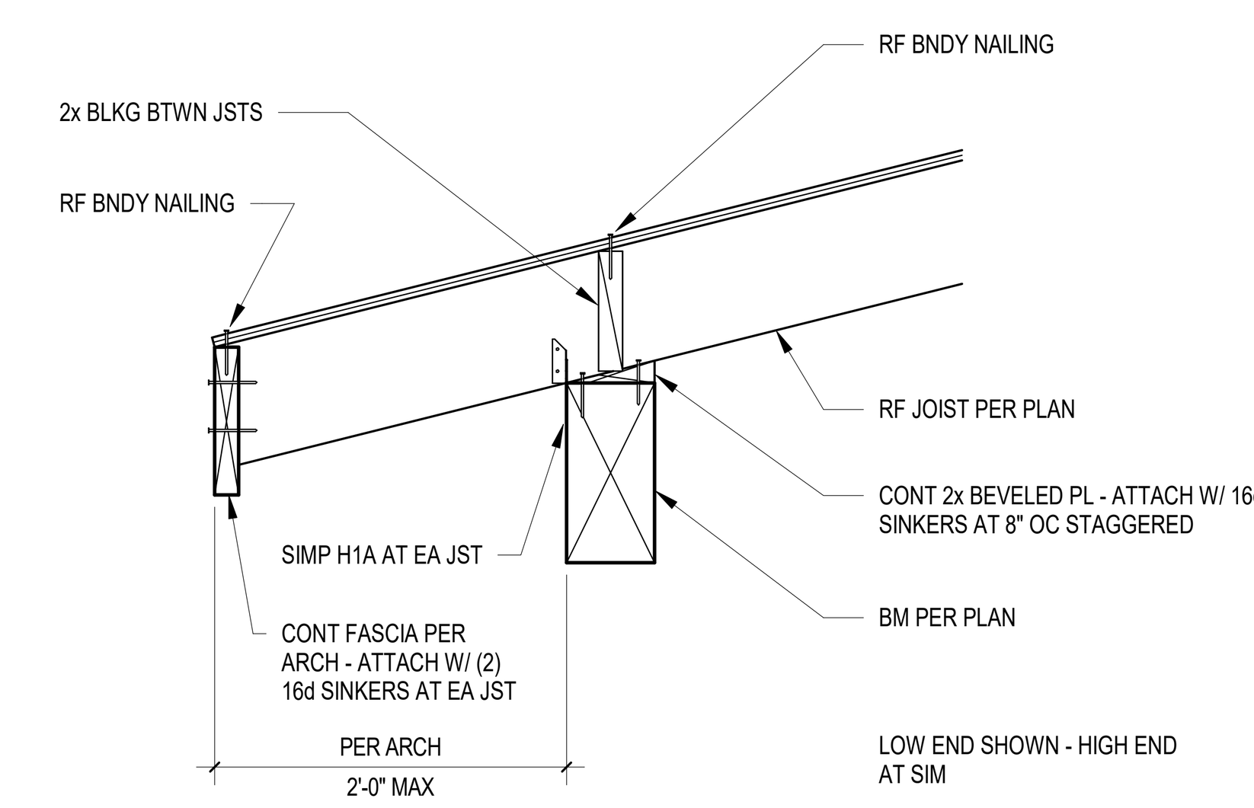
1" = 1'-0" 8 / S4.3



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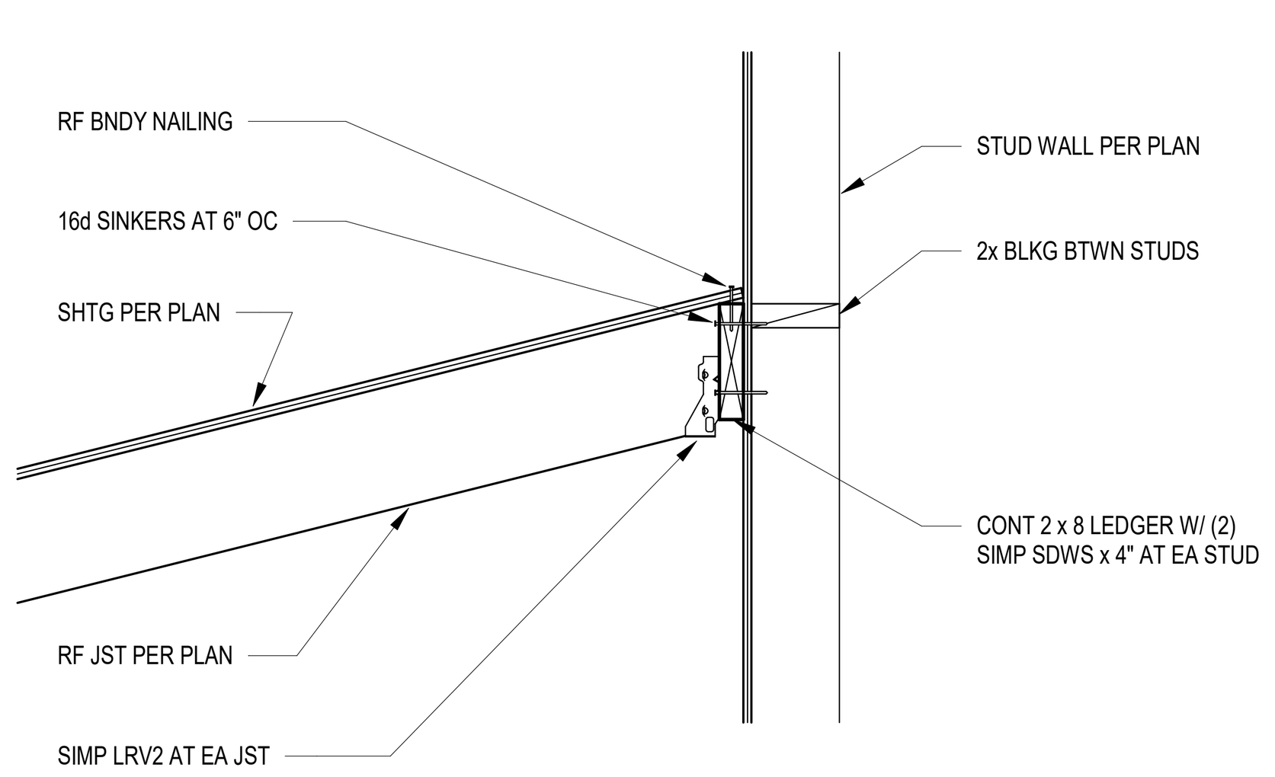

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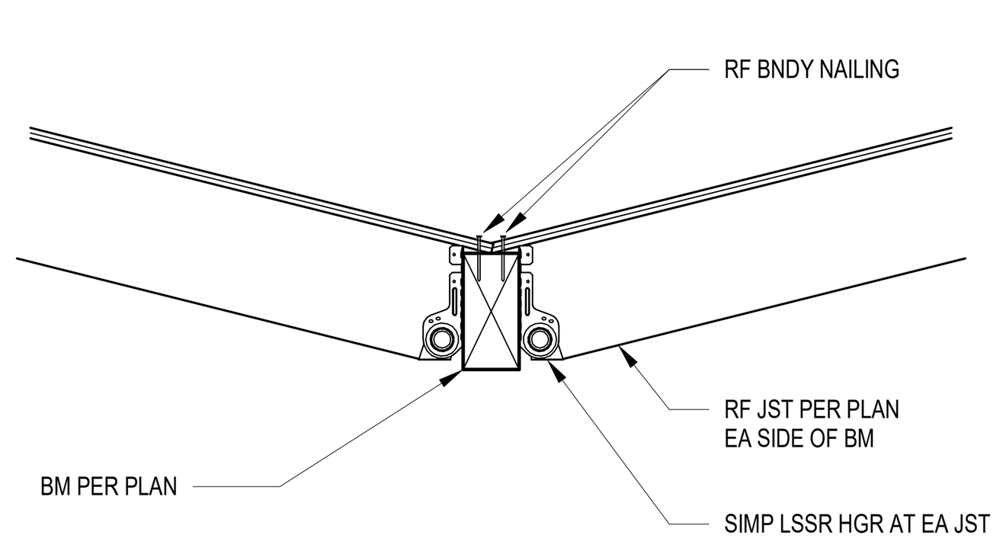

11 SECTION
1" = 1'-0" 11 / S4.3

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STATE OF WASHINGTON
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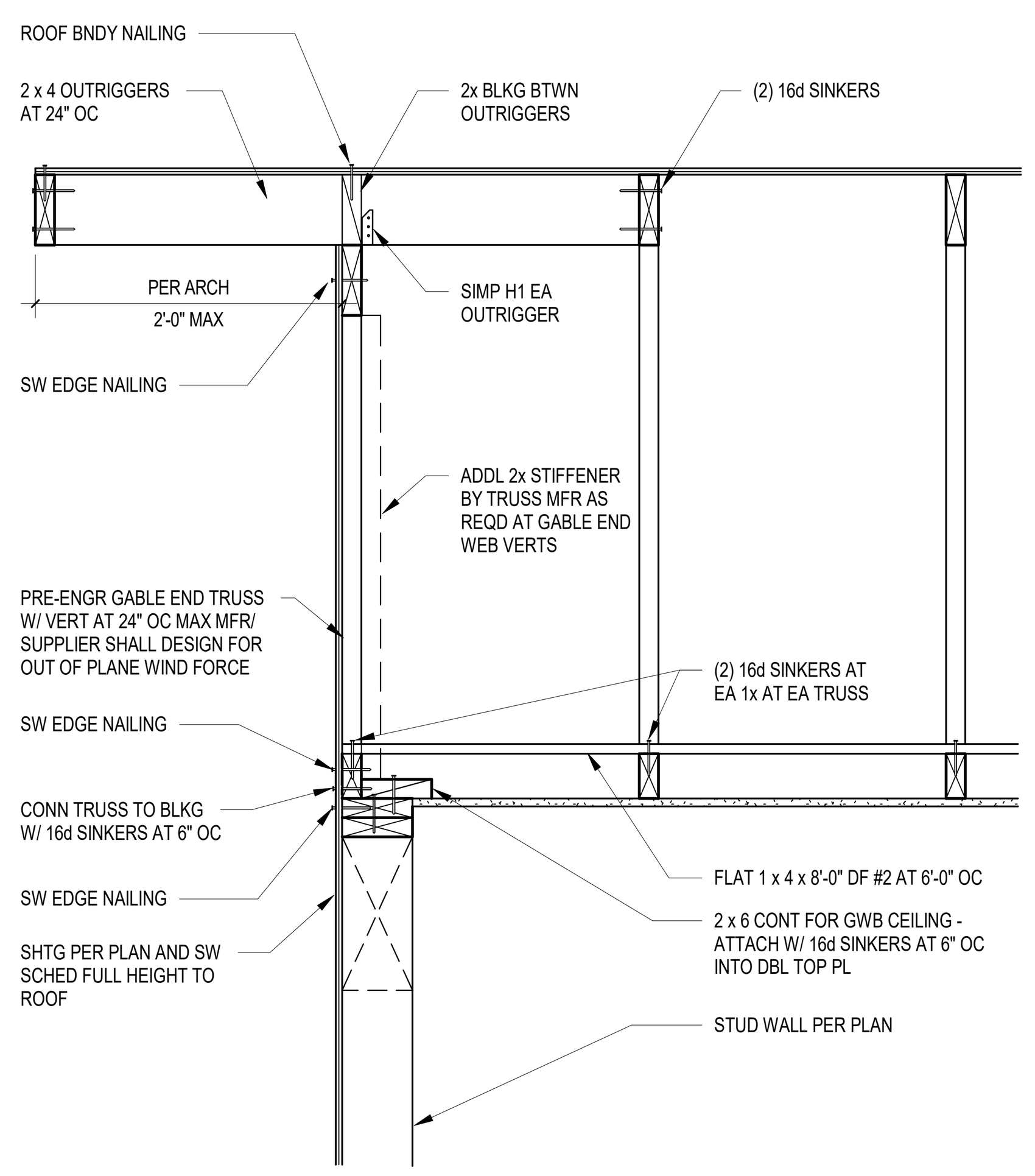
ROOF FRAMING DETAILS

S4.3



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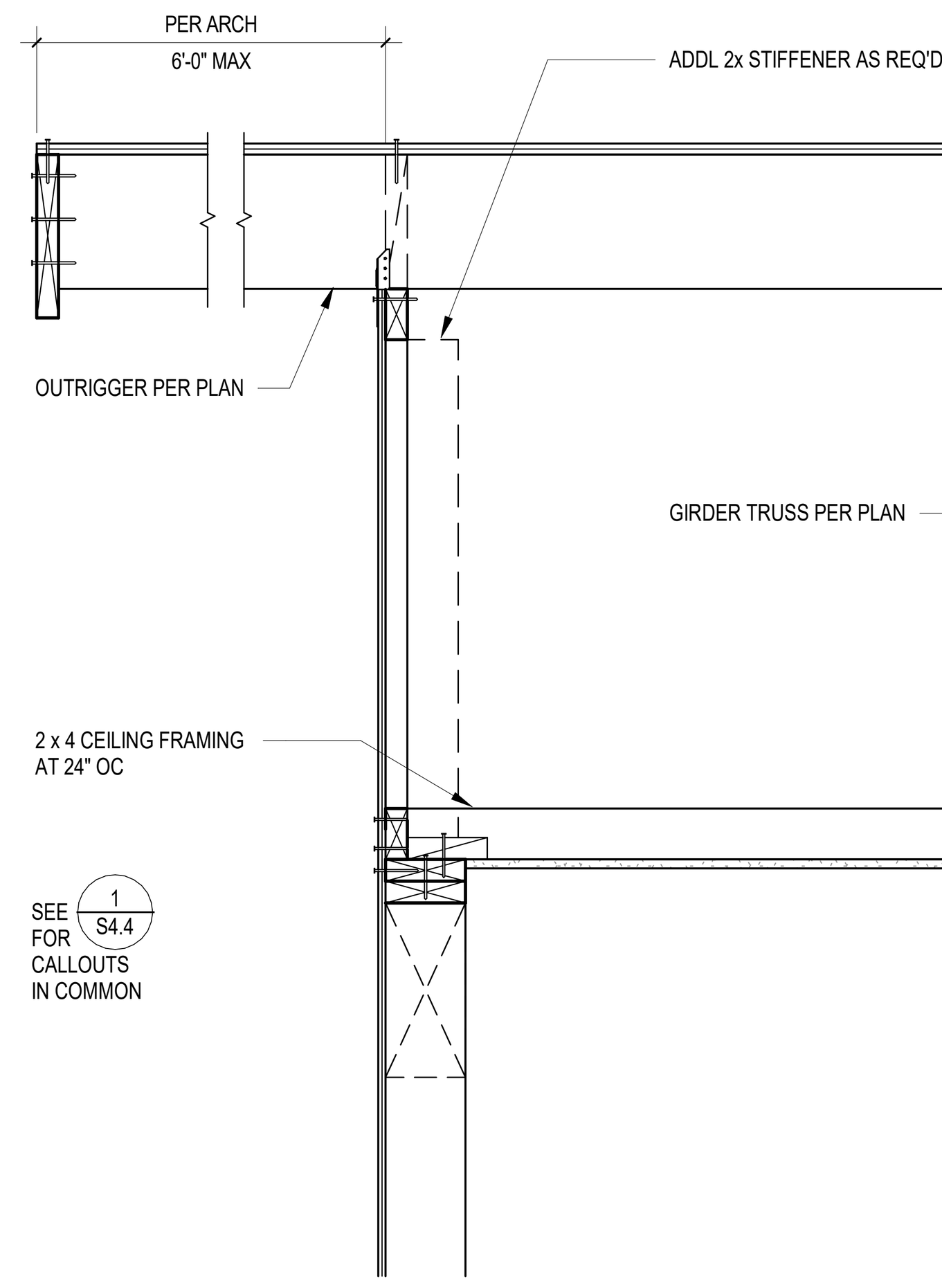

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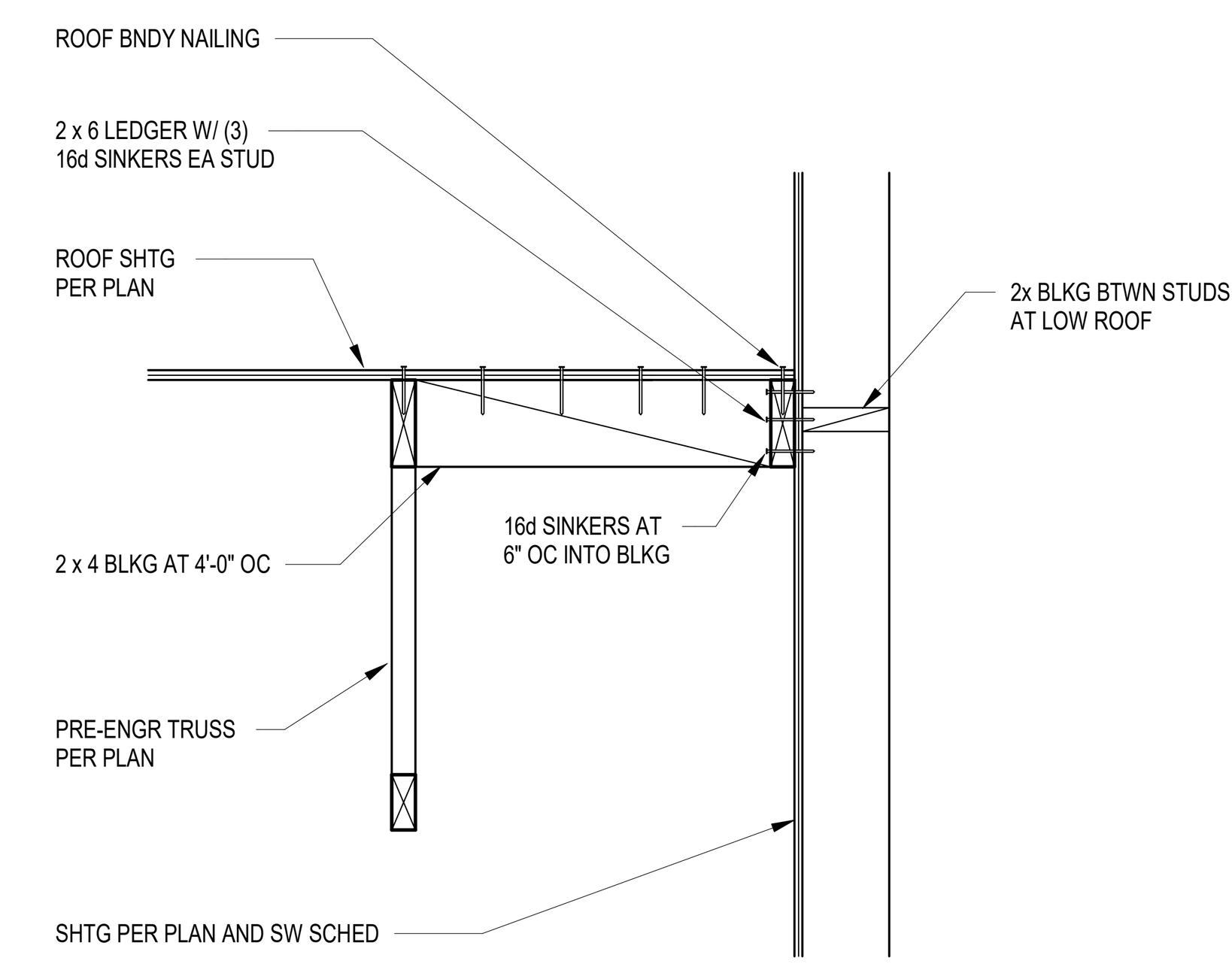

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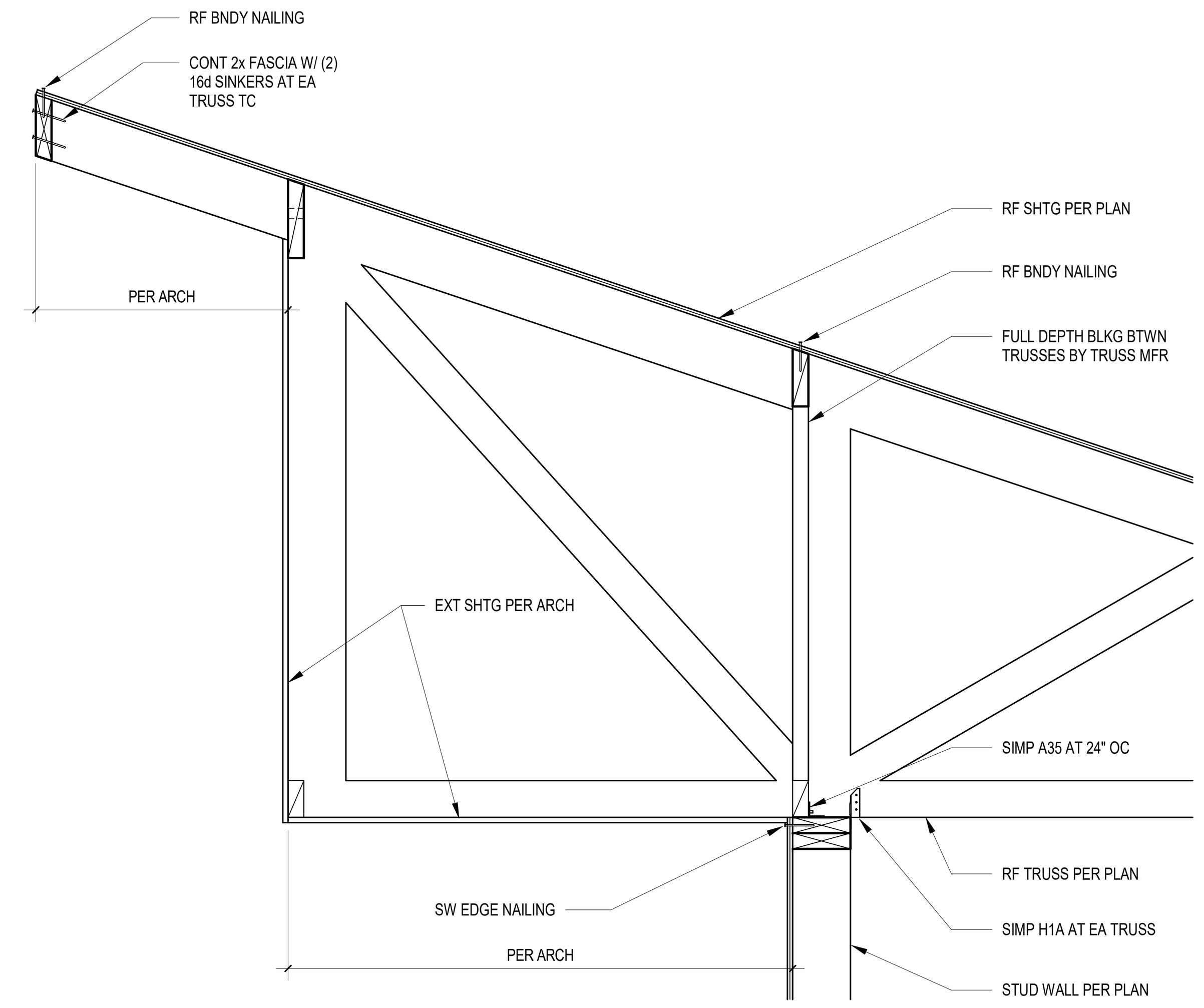

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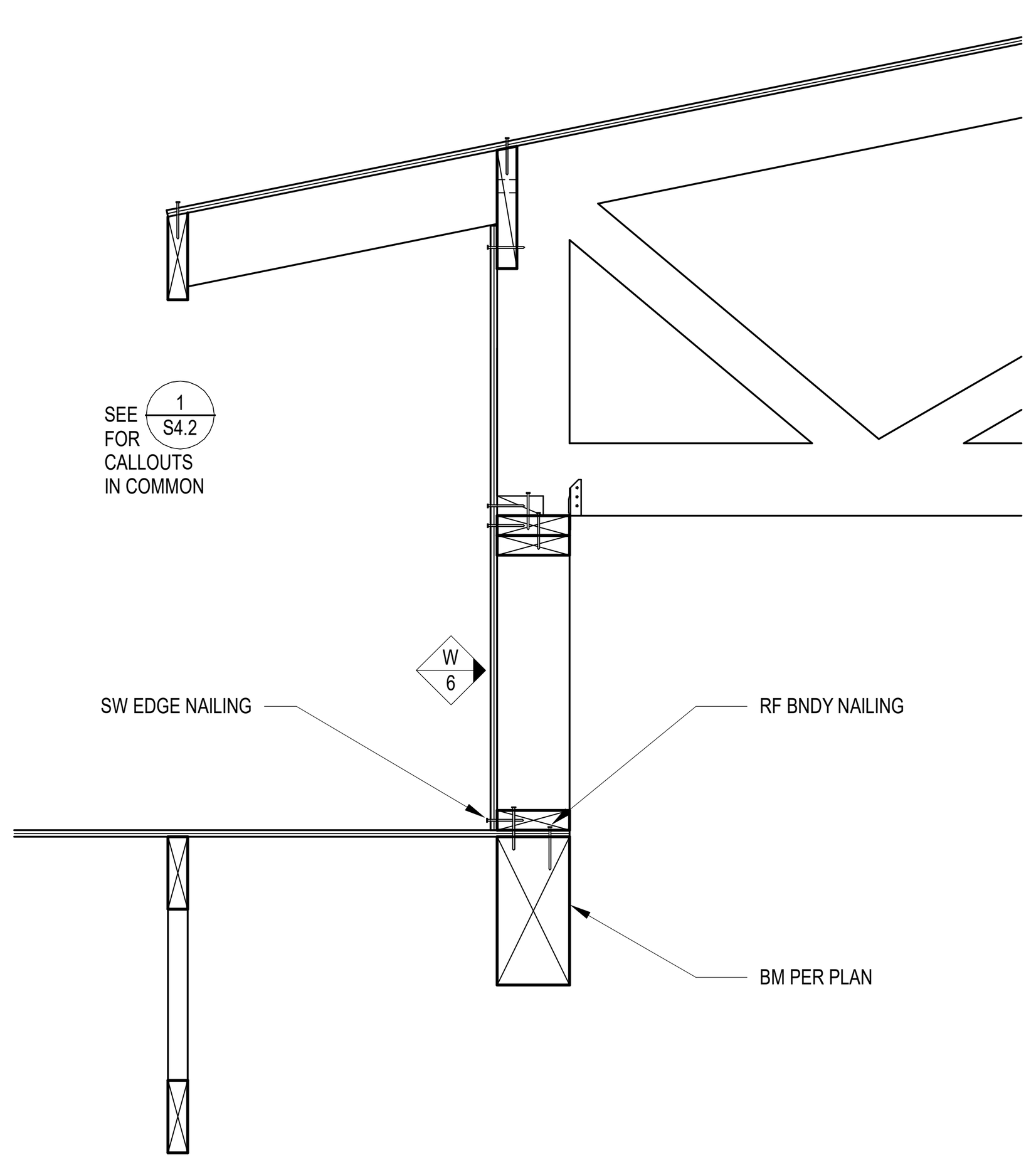

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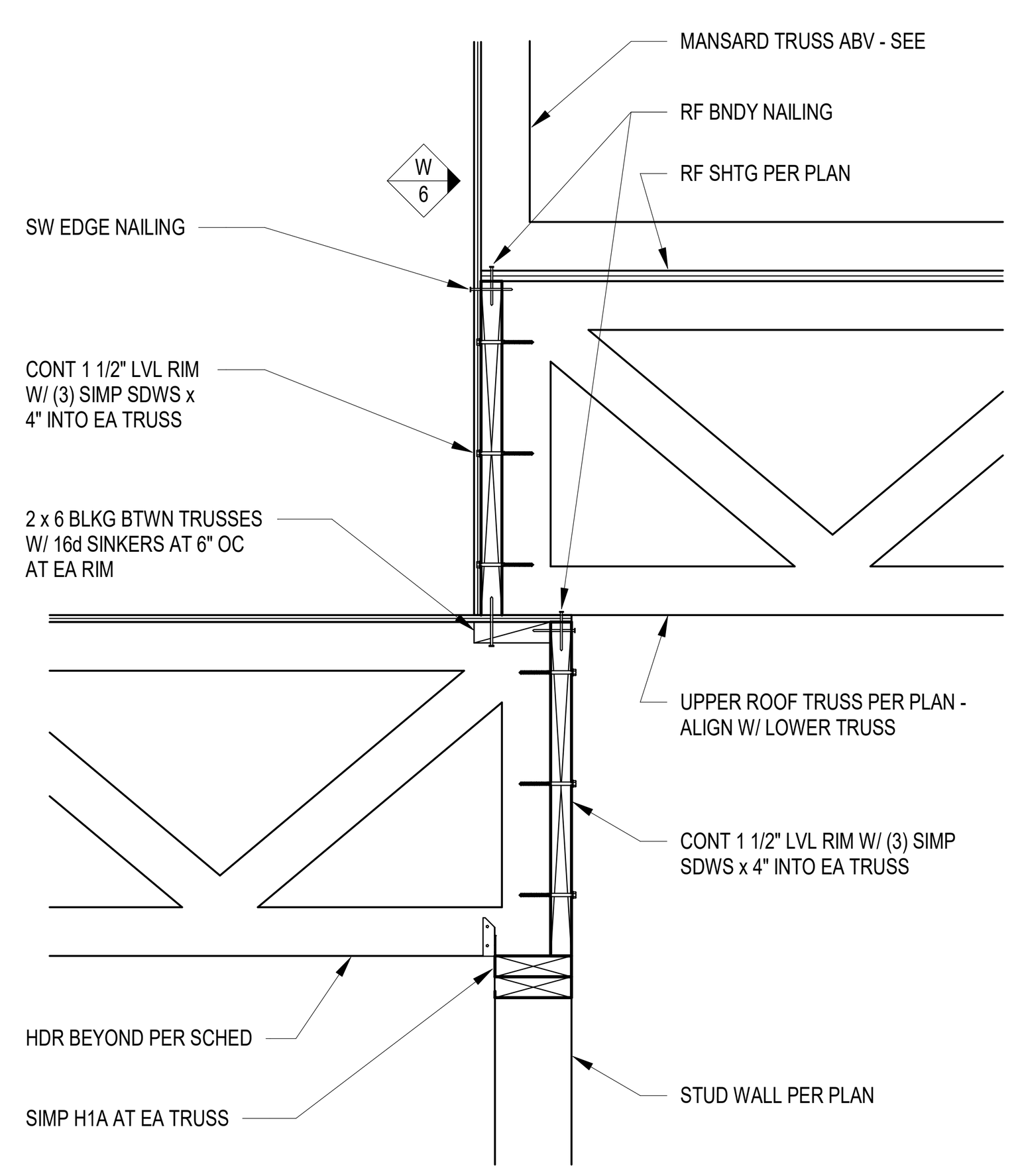

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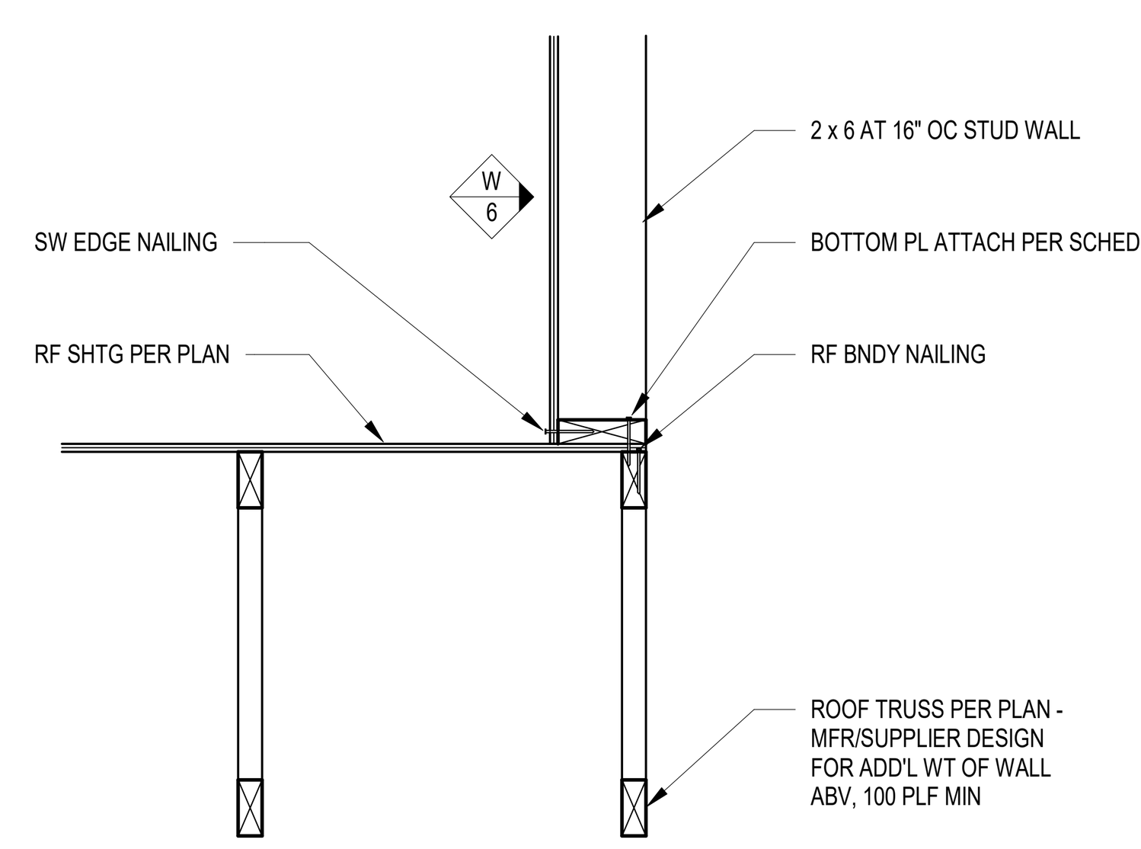

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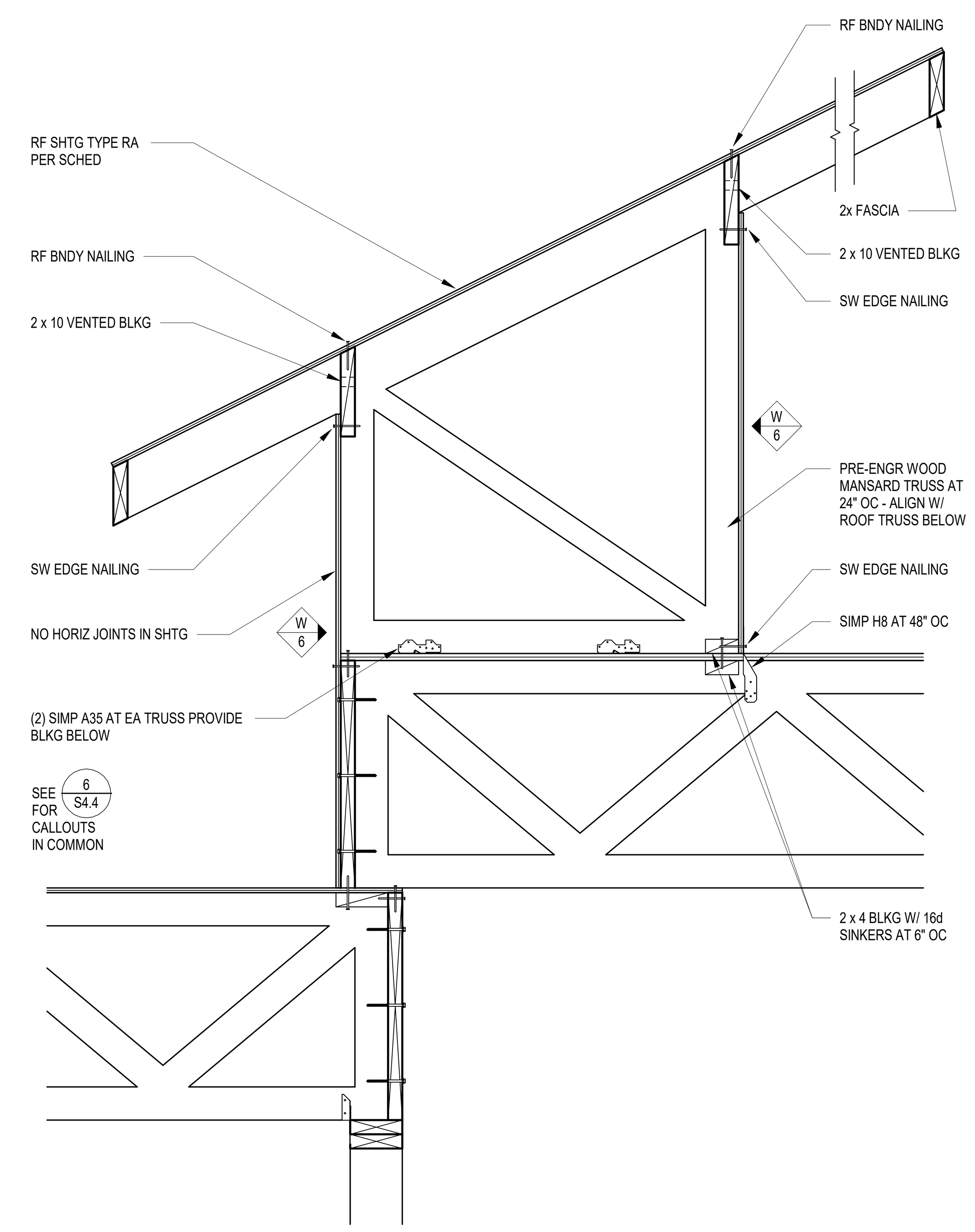

8 **SECTION**
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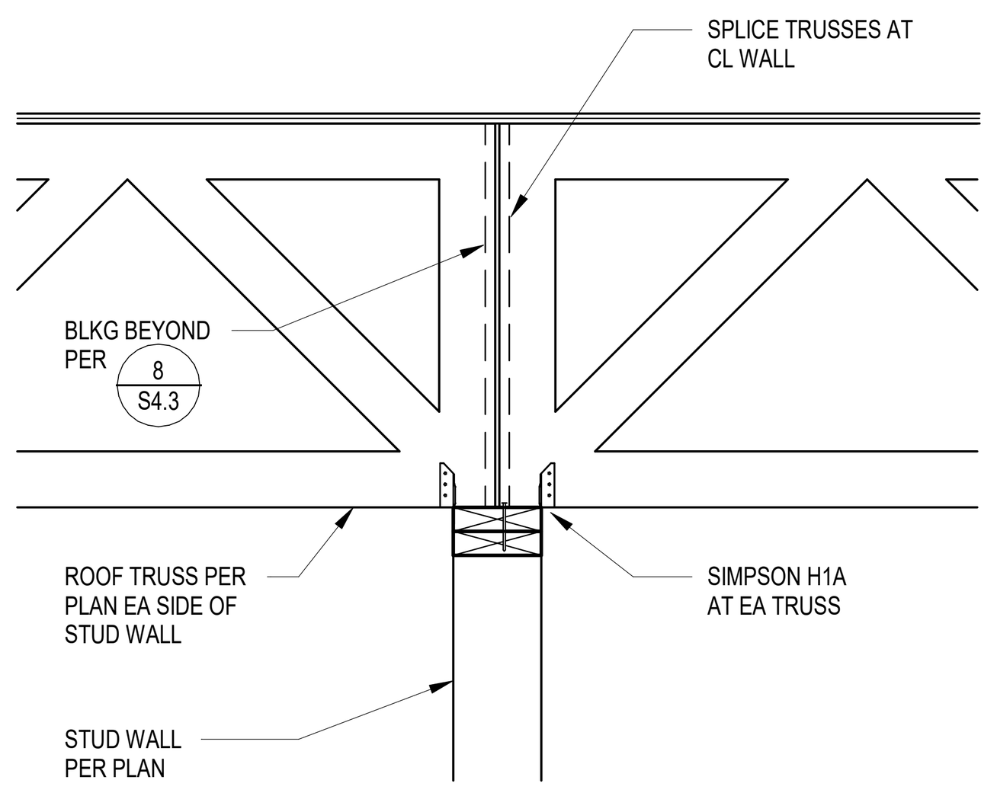

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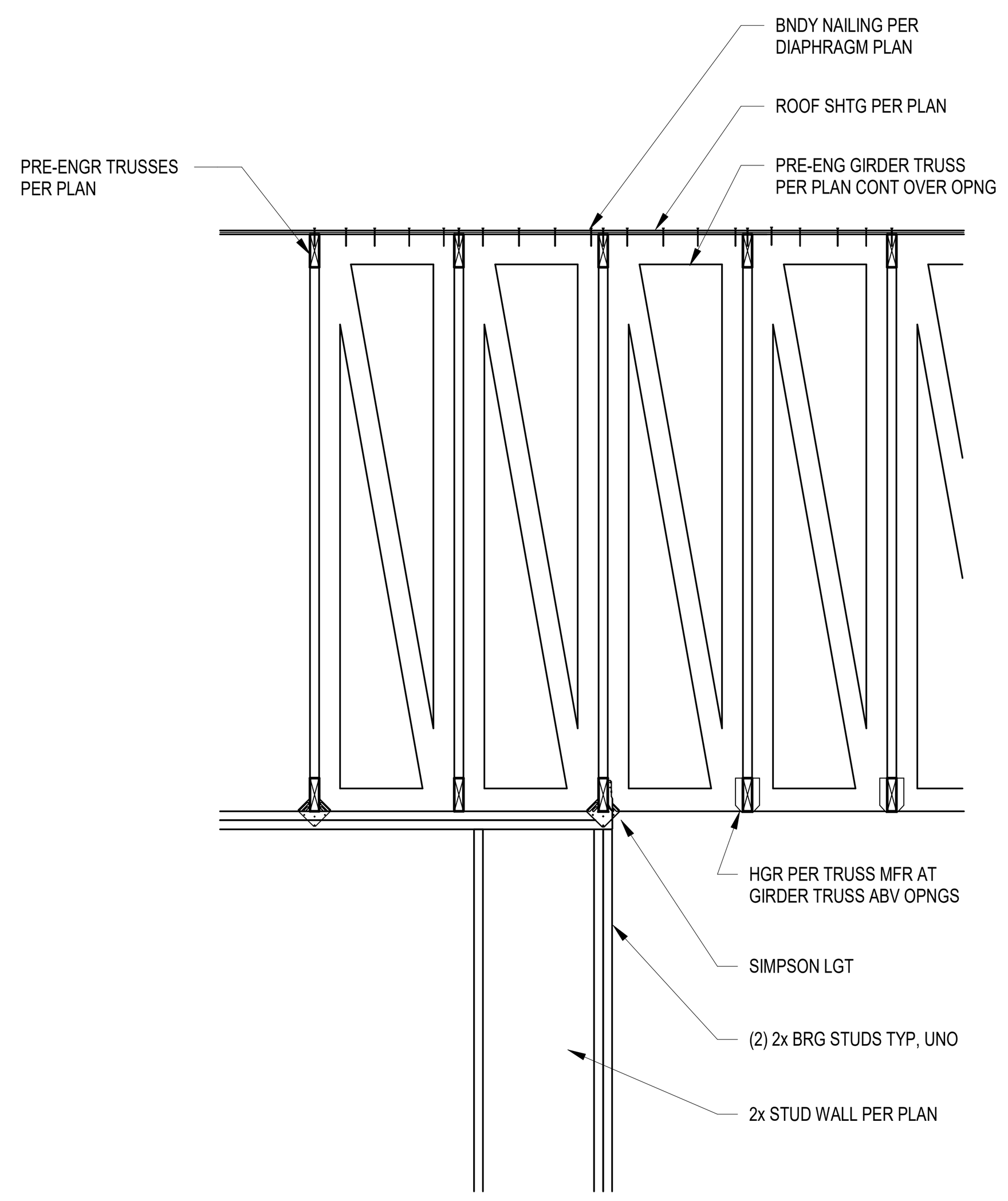

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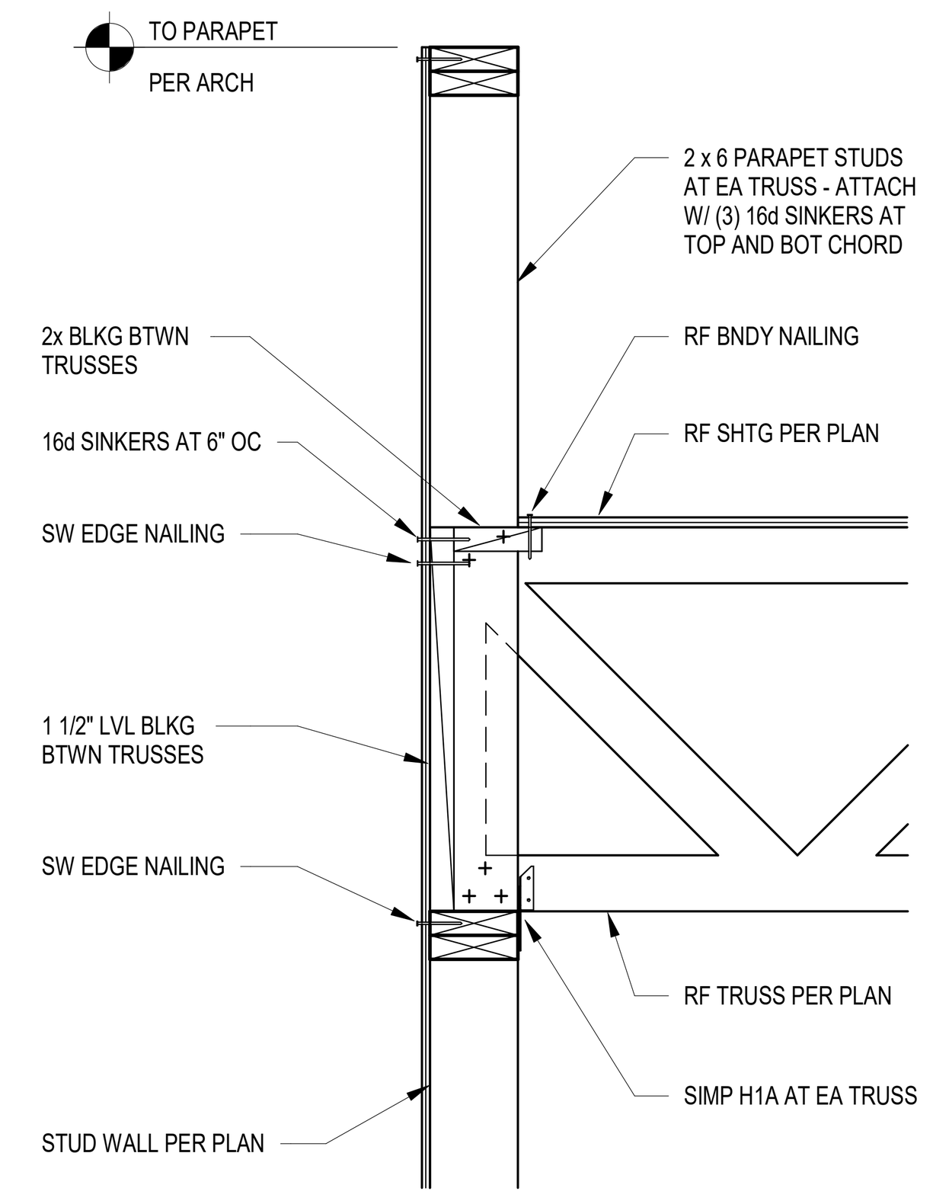

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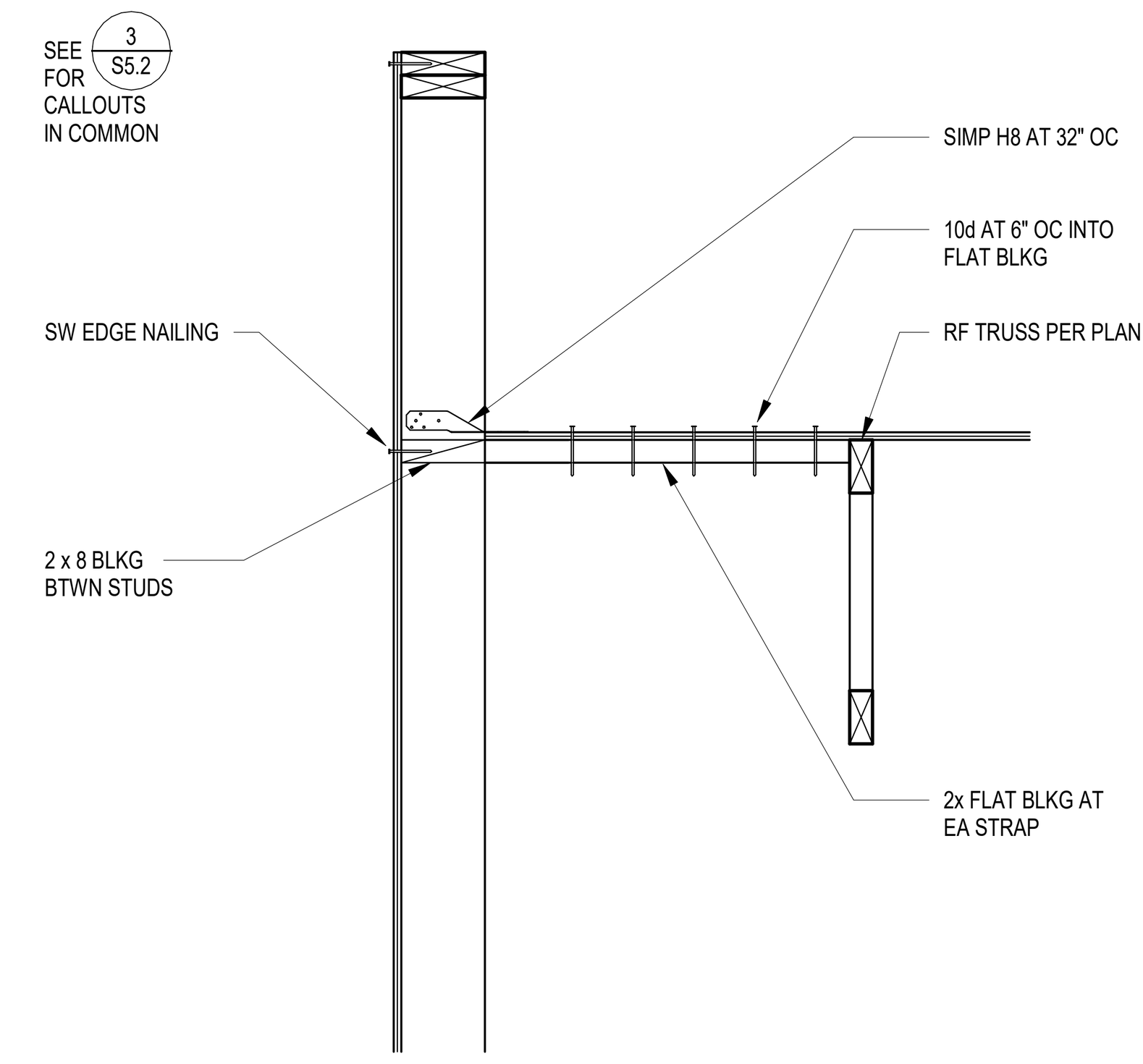

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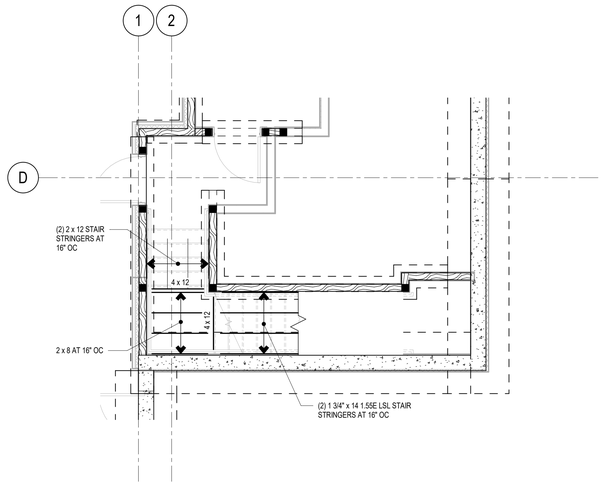
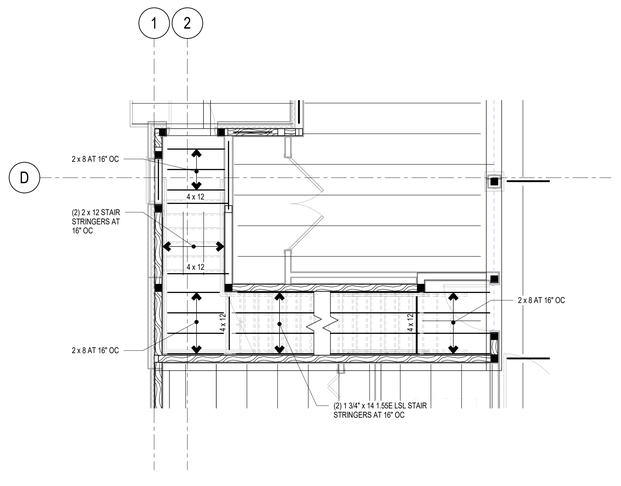
ROOF FRAMING DETAILS

S4.5



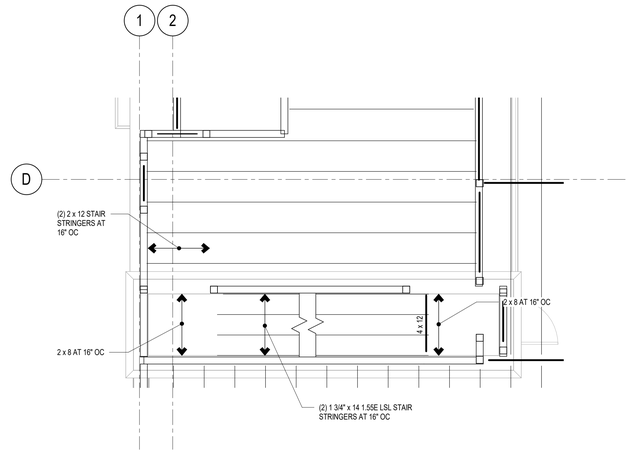
1 BASEMENT STAIR PLAN

1/4" = 1'-0"



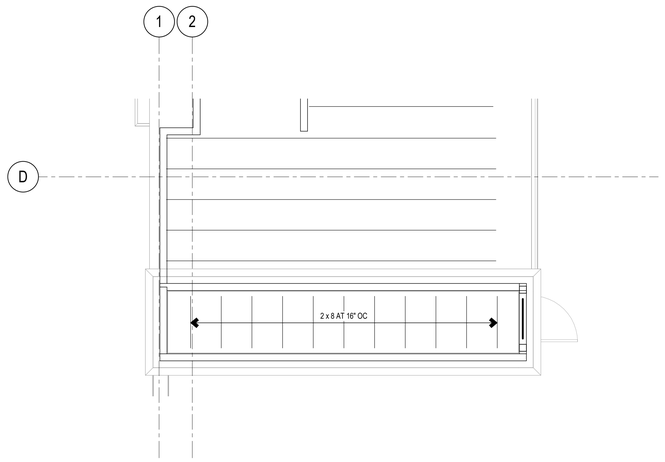
2 LEVEL 1 STAIR PLAN

1/4" = 1'-0"



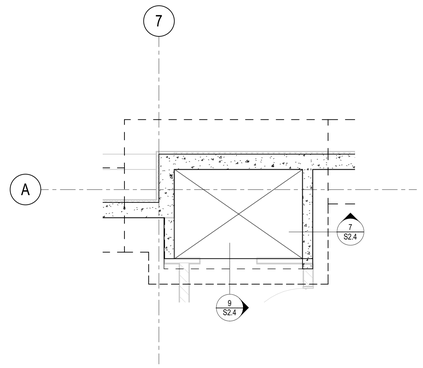
3 ROOF PLAN STAIR PLAN

1/4" = 1'-0"



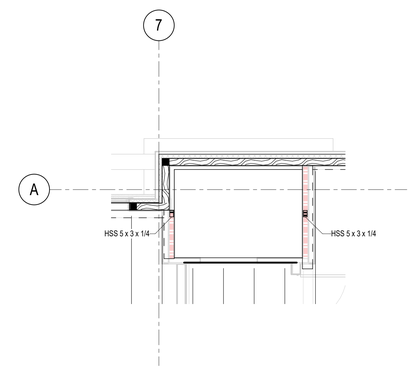
4 HIGH ROOF STAIR PLAN

1/4" = 1'-0"



5 ELEVATOR FOUNDATION PLAN

1/4" = 1'-0"



6 ELEVATOR FLOOR PLAN

1/4" = 1'-0"

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WESLEY BRADLEY PARK
PHASE 2 - CARE CENTER

707 39TH AVENUE SE
PUYALLUP, WA 98374

PERMIT SET
DECEMBER 2, 2024

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2220236.20
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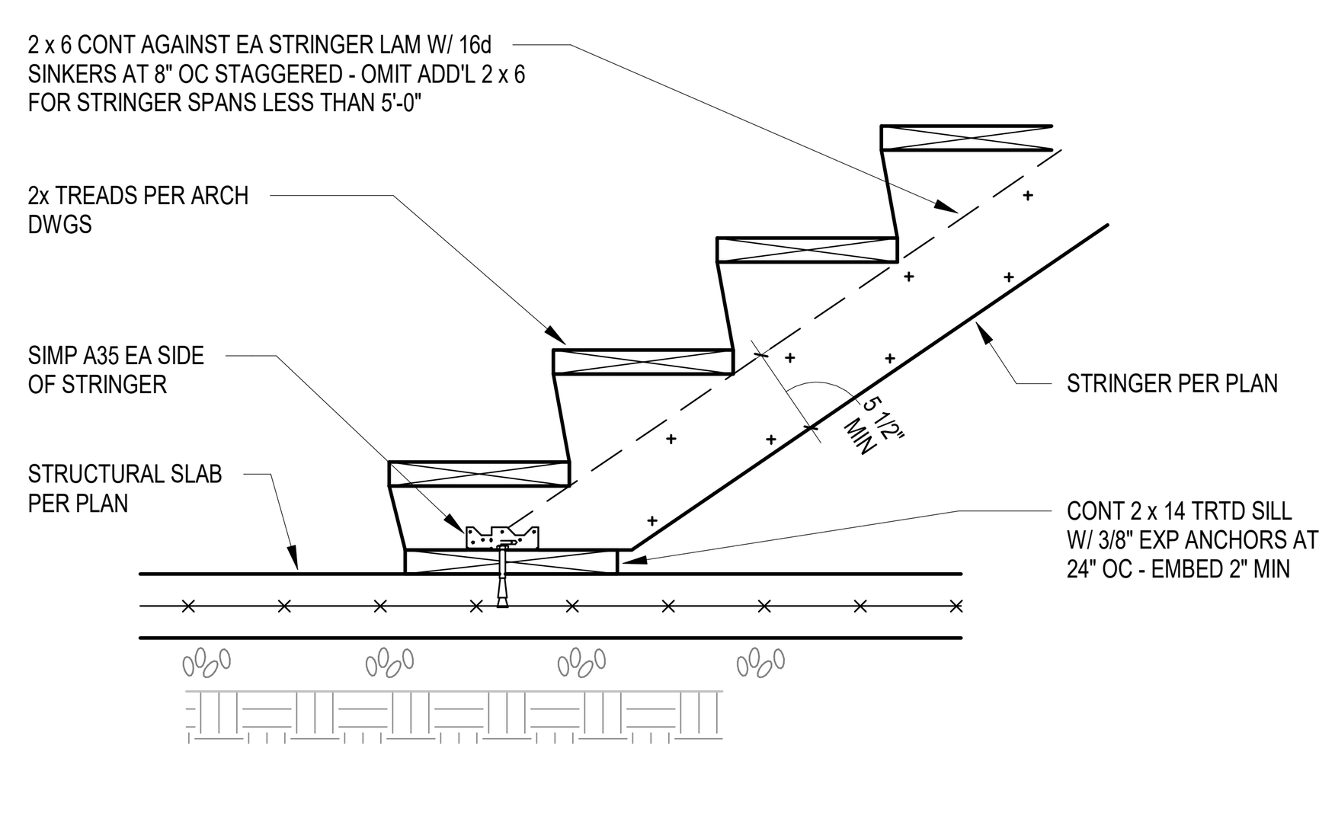
STAIR AND ELEVATOR PLANS

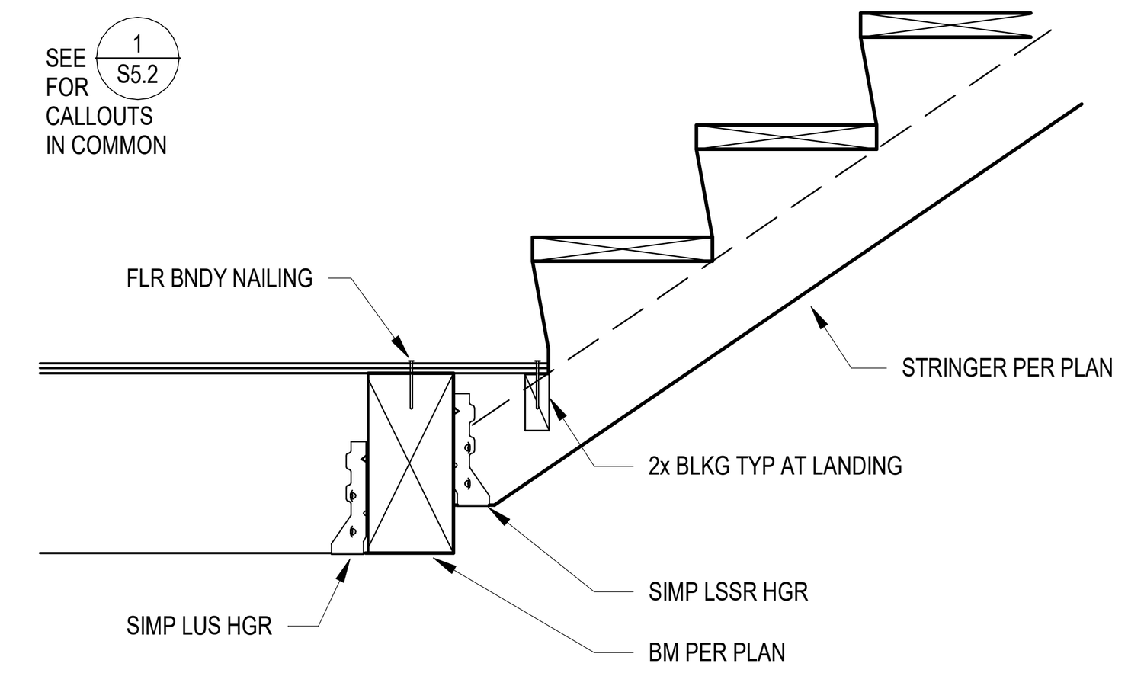
S5.1

12/4/2024 10:56:27 AM



TYP STRINGER TO SLAB CONNECTION

1 SECTION
1" = 1'-0" 1 / S5.2



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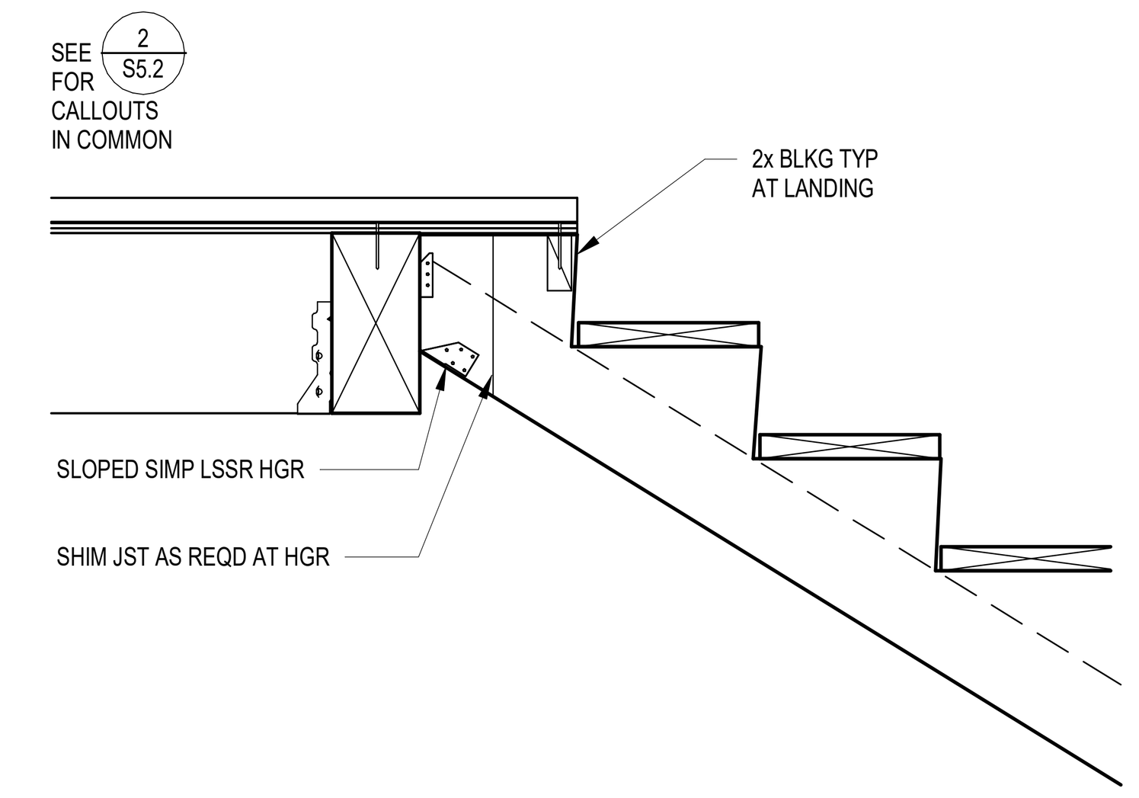

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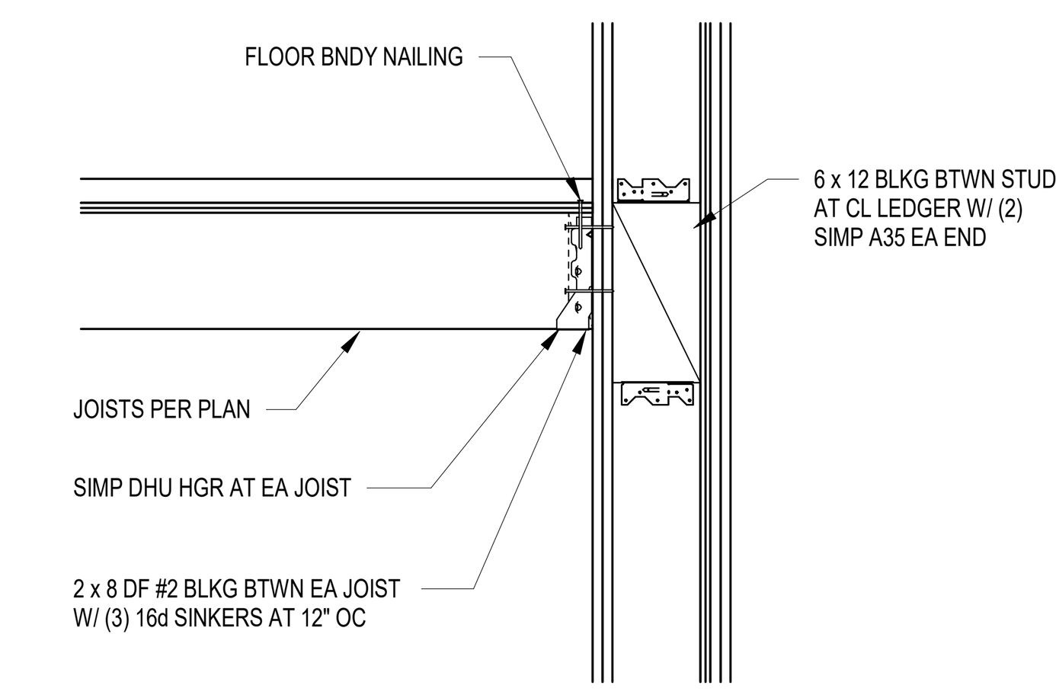

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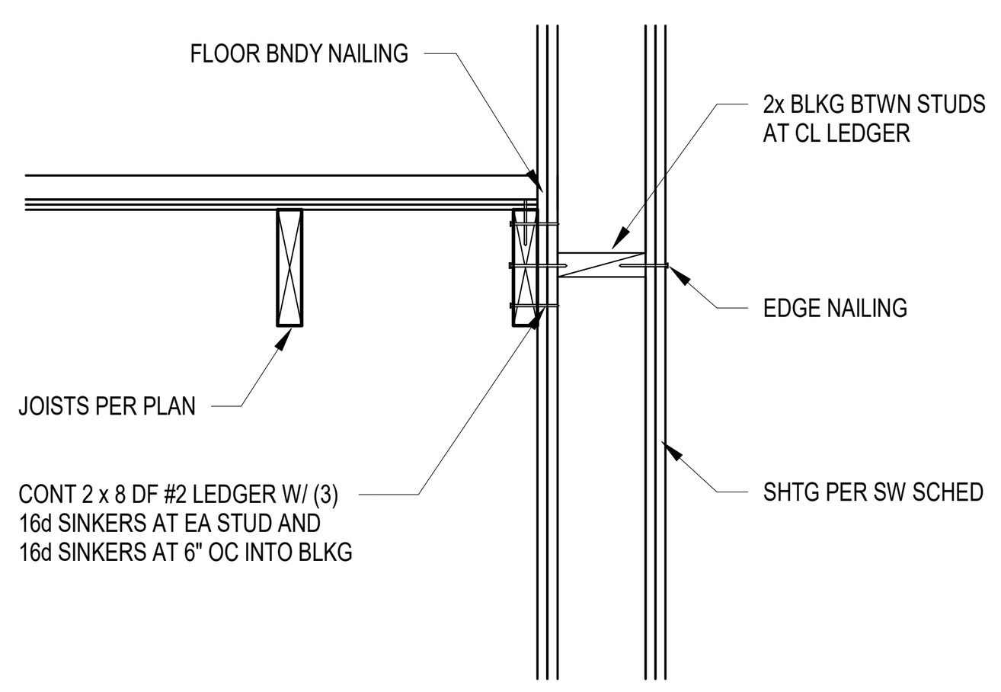
1" = 1'-0" 4 / S5.2



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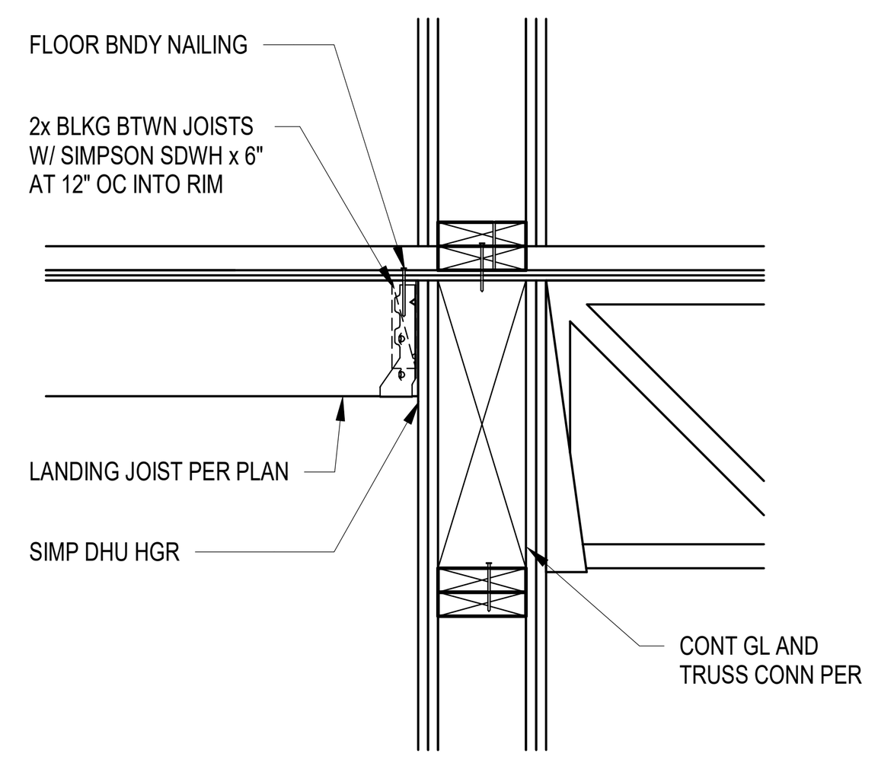

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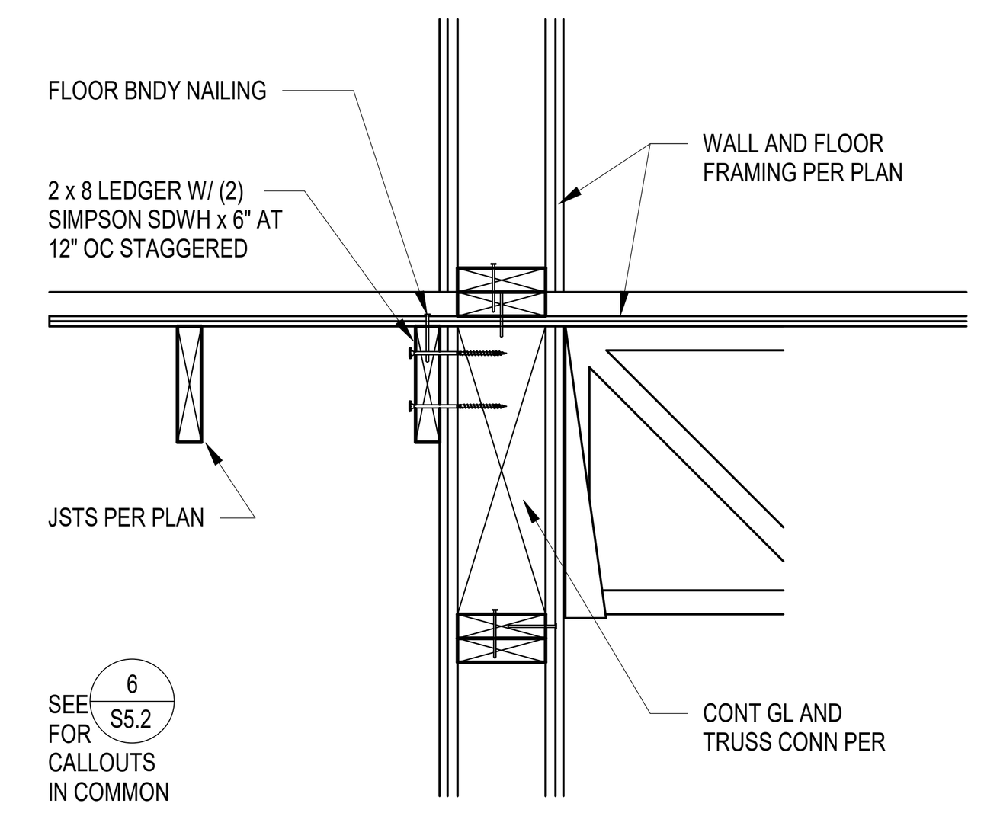
1" = 1'-0" 6 / S5.2



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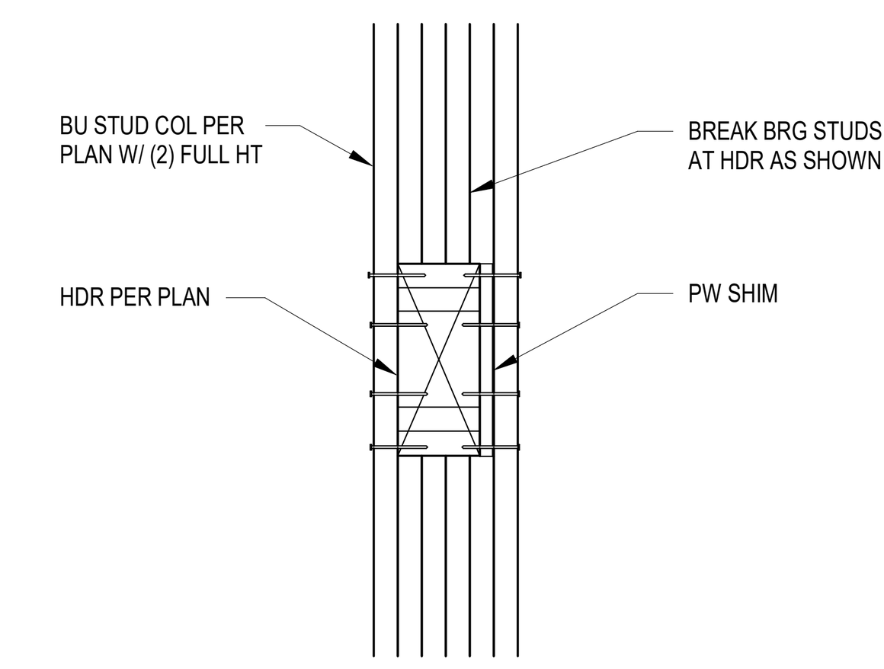
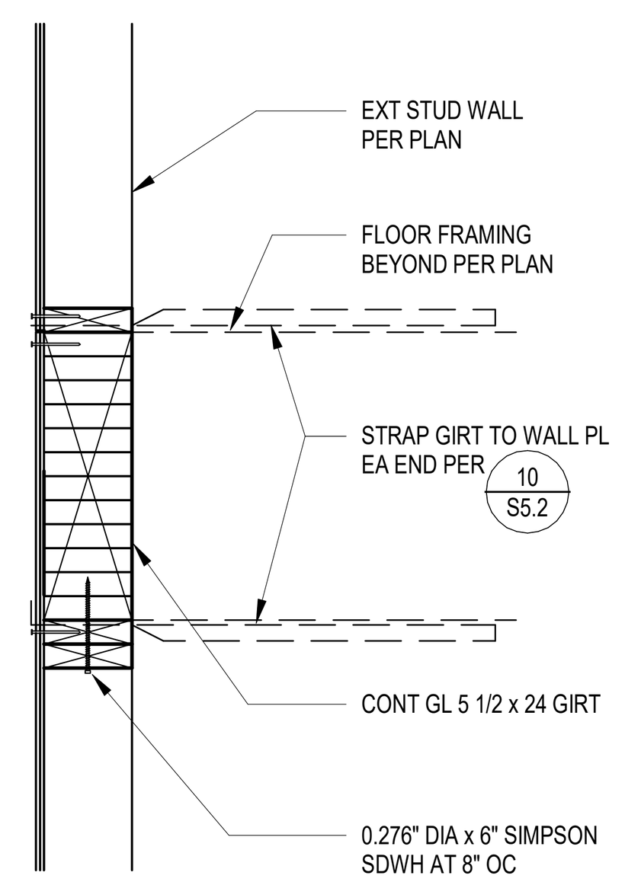
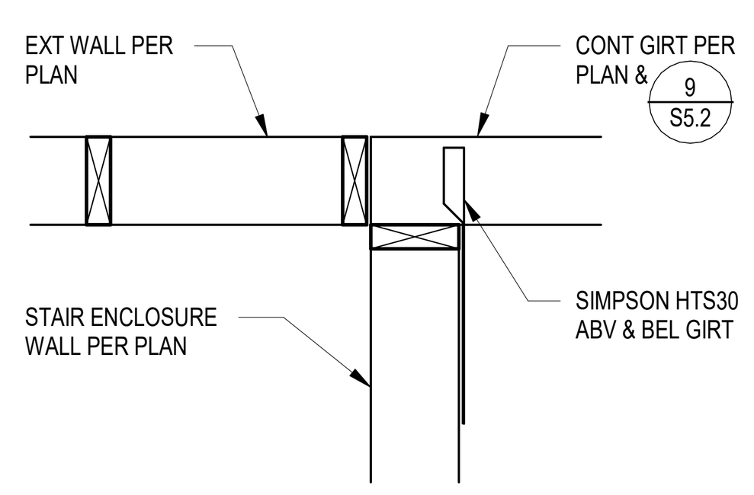
1" = 1'-0" 7 / S5.2



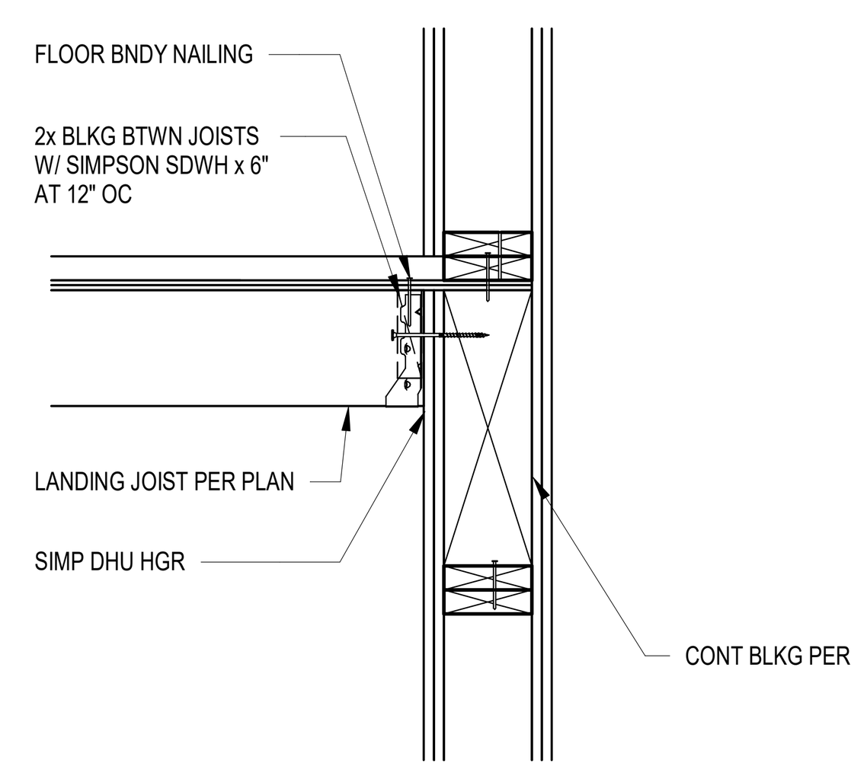
8 SECTION
1" = 1'-0" 8 / S5.2



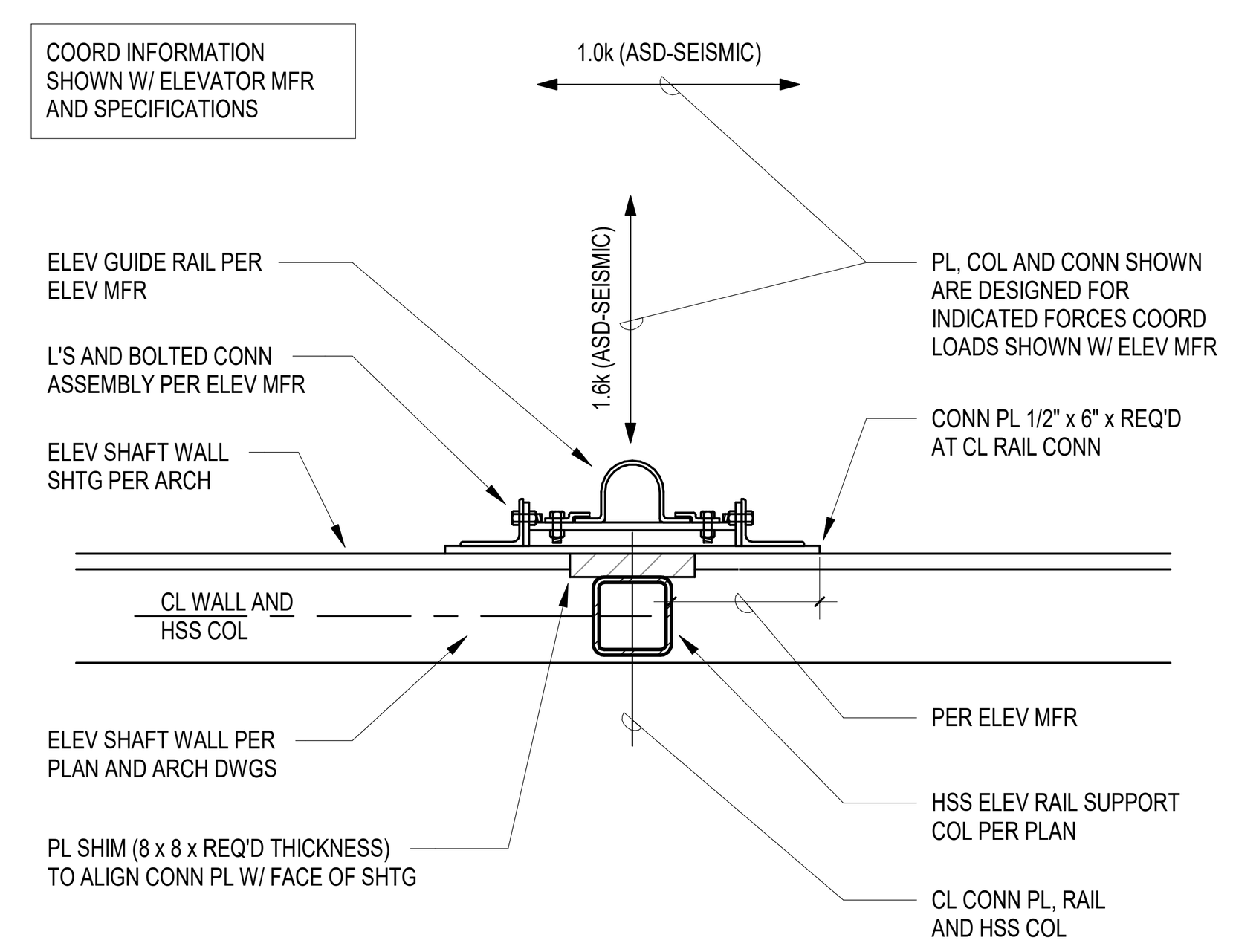
9 SECTION
1" = 1'-0" 9 / S5.2



10 PLAN
1" = 1'-0" 10 / S5.2

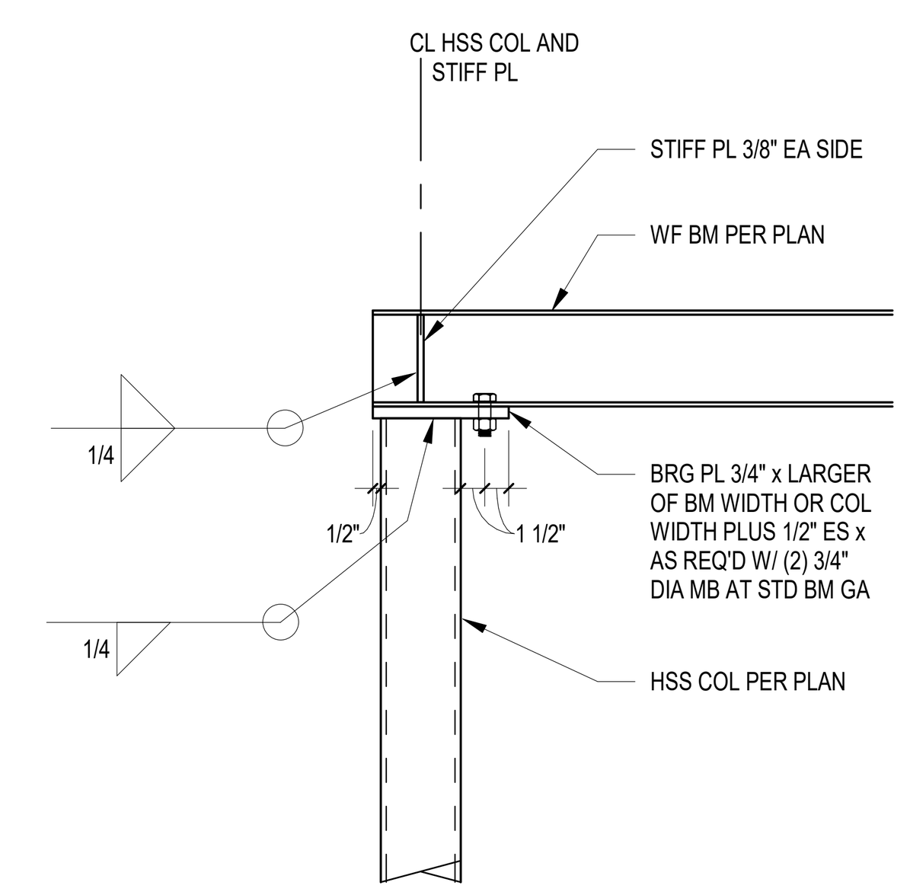


11 SECTION
1" = 1'-0" 11 / S5.2

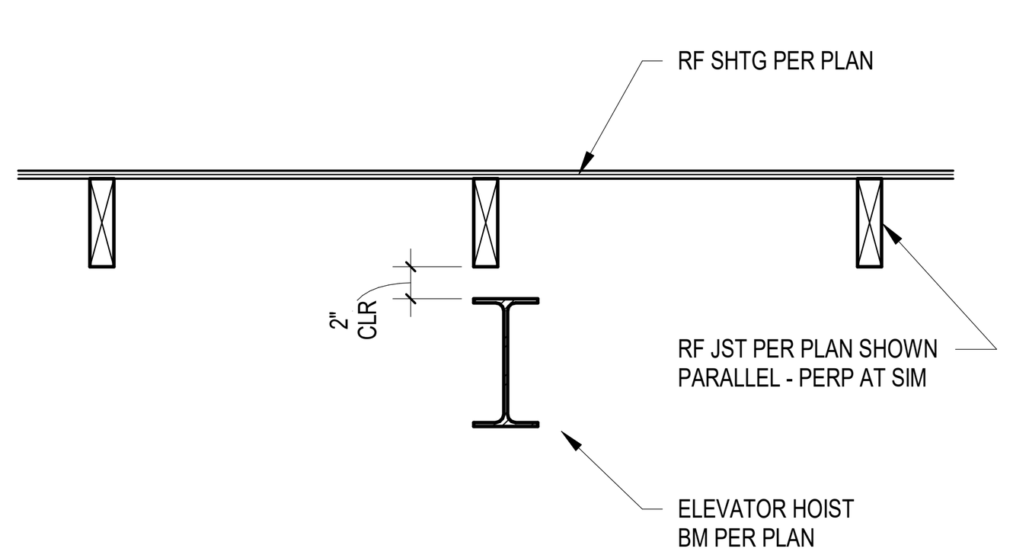


TYP ELEV RAIL CONN

12 PLAN
1" = 1'-0"



13 SECTION
1" = 1'-0" S5.2-13



14 TYPICAL
1" = 1'-0" S5.2-14

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ANDREW DEAN McEACHERN, STATE OF WASHINGTON, STRUCTURAL, PROFESSIONAL ENGINEER

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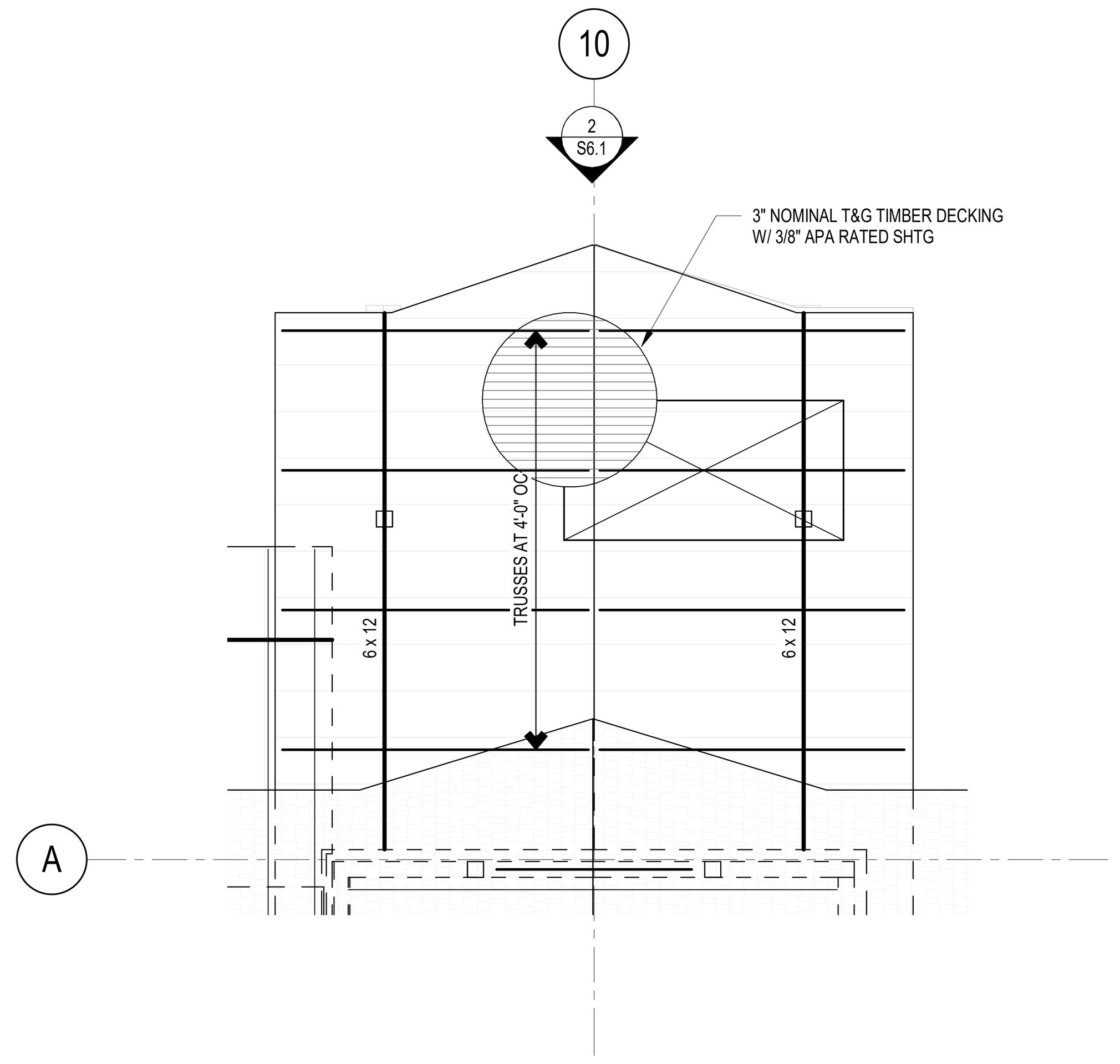
PHASE 2 - CARE CENTER

STAIR AND ELEVATOR DETAILS

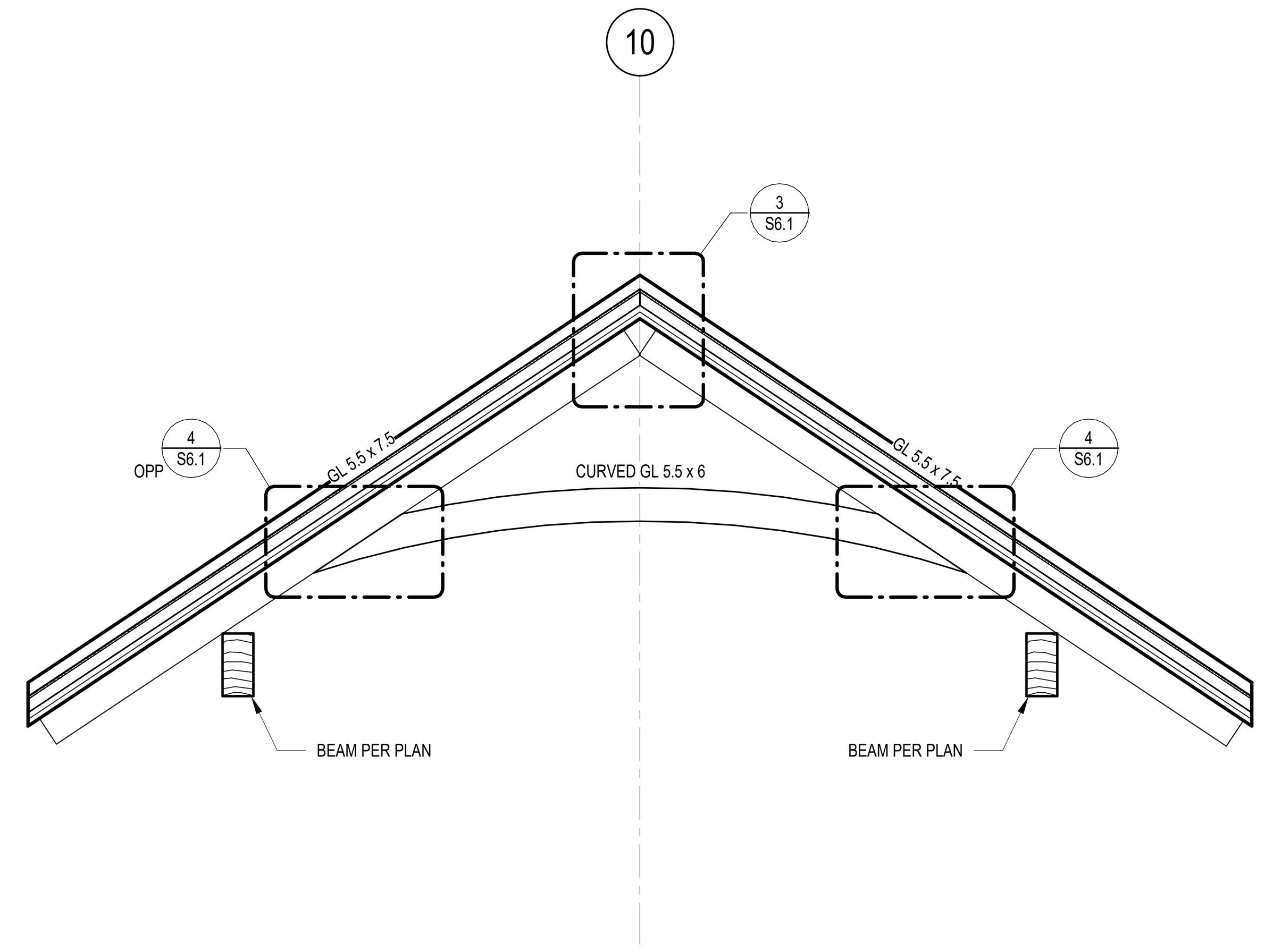
S5.2



1 ENTRY CANOPY FRAMING PLAN

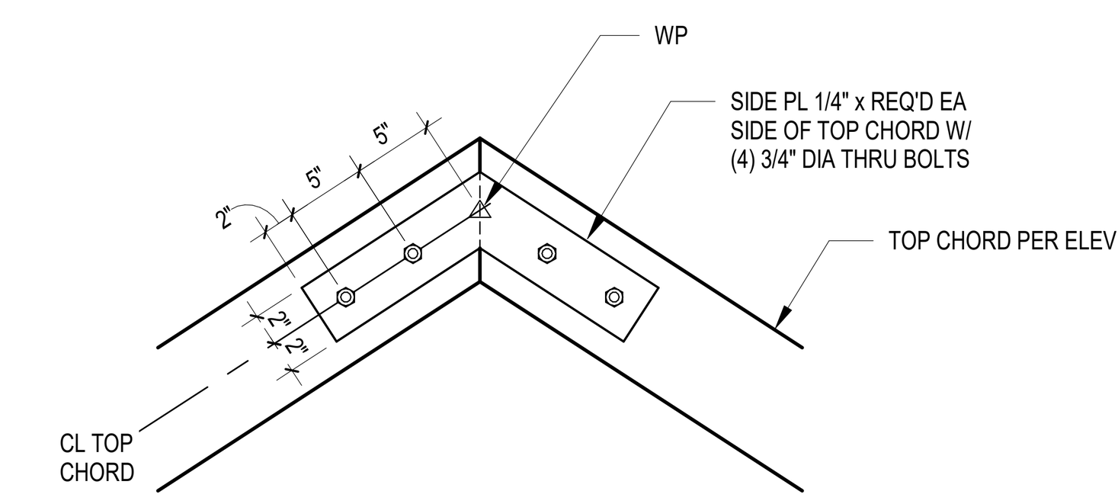
1/4" = 1'-0"

0 2 4 8
SCALE: 1/4" = 1'-0"



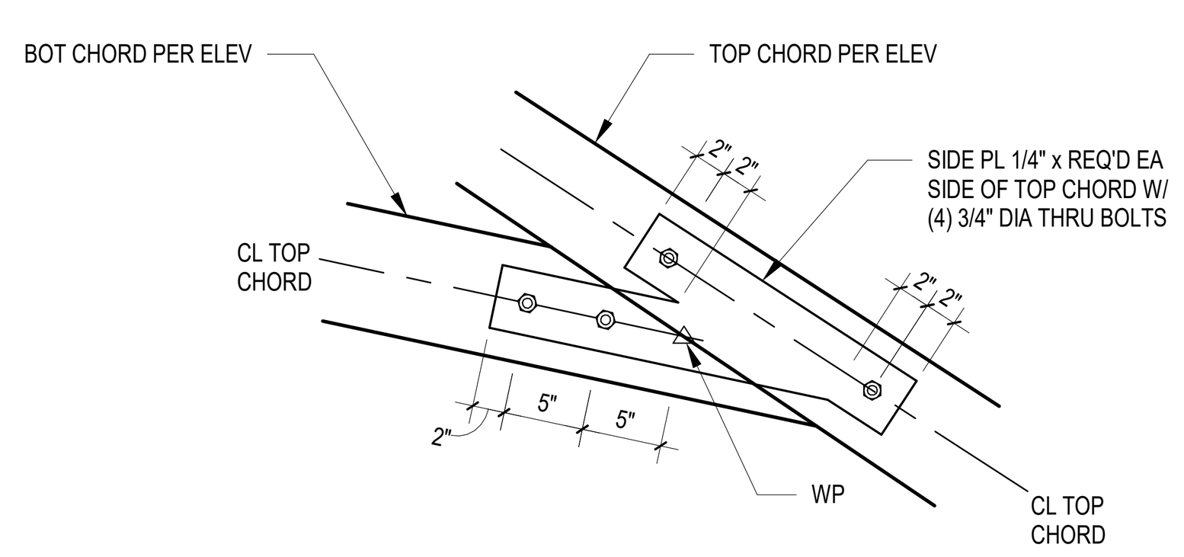
2 SECTION

1/2" = 1'-0" 2 / S6.1



3 SECTION

1" = 1'-0" 3 / S6.1



4 SECTION

1" = 1'-0" 4 / S6.1

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ENTRY CANOPY FRAMING
PLANS AND DETAILS

S6.1

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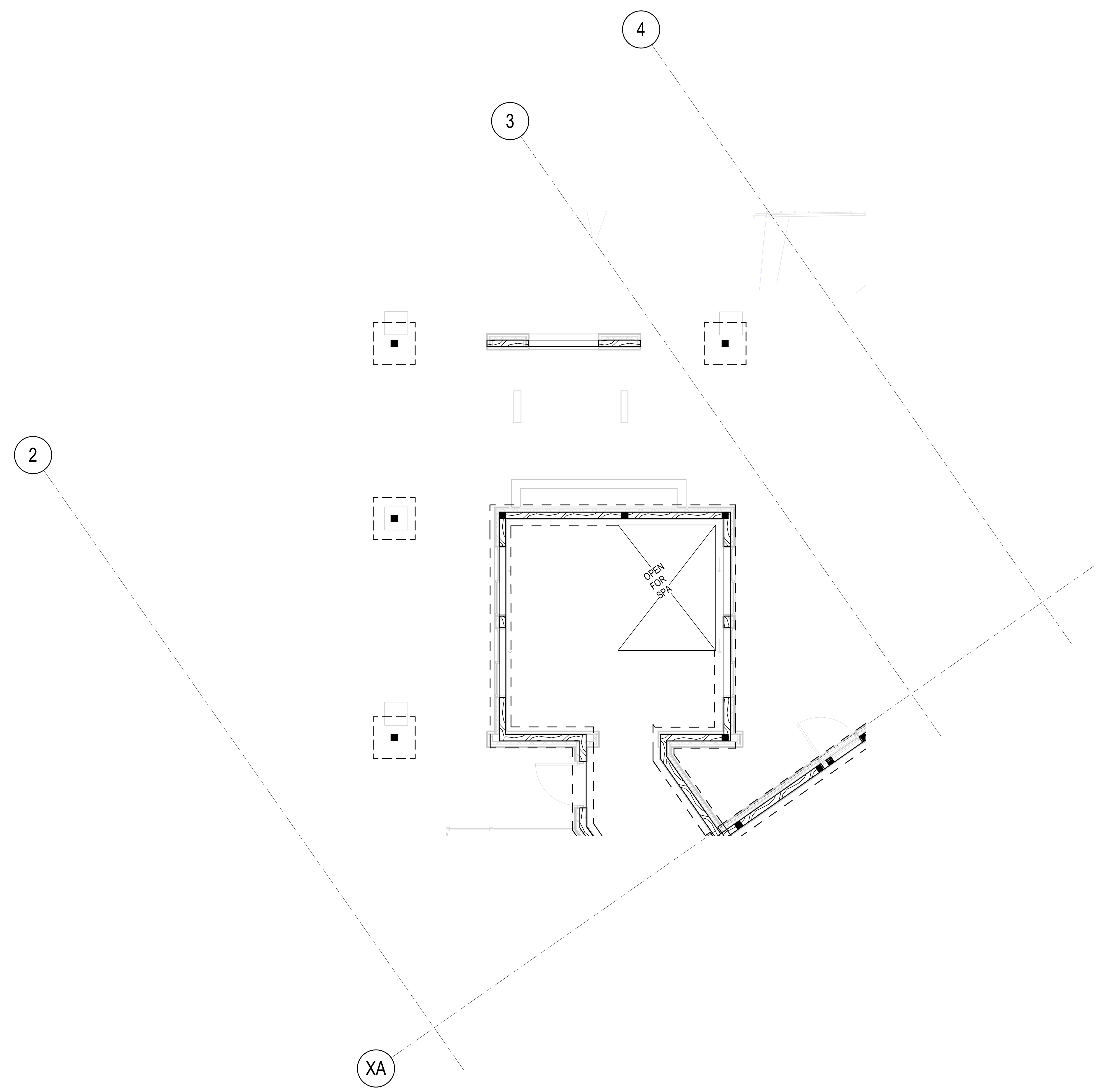
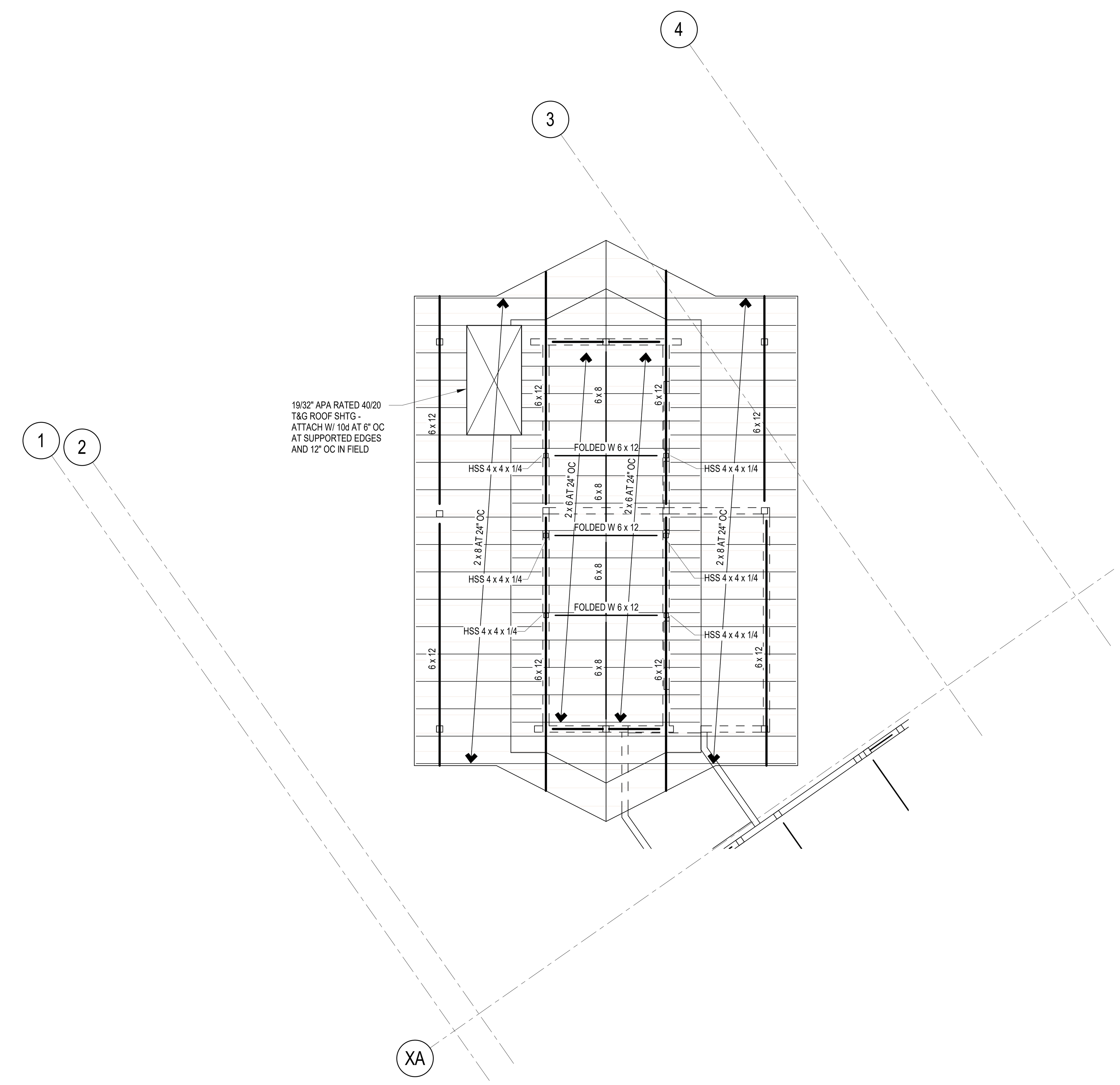


1 PAVILLION FOUNDATION PLAN

3/16" = 1'-0"

0 4' 8' 12'
SCALE IN FEET



2 PAVILLION ROOF PLAN

3/16" = 1'-0"

0 4' 8' 12'
SCALE IN FEET

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PAVILLION FRAMING PLANS AND DETAILS

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