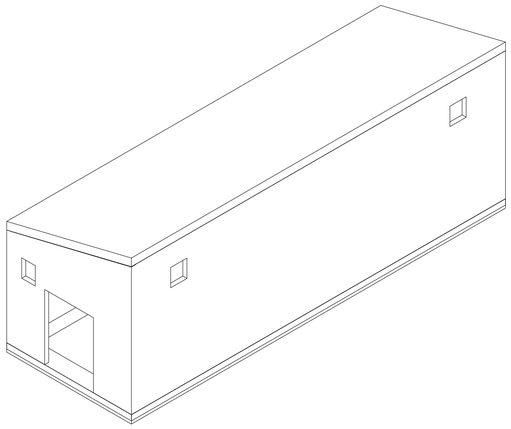
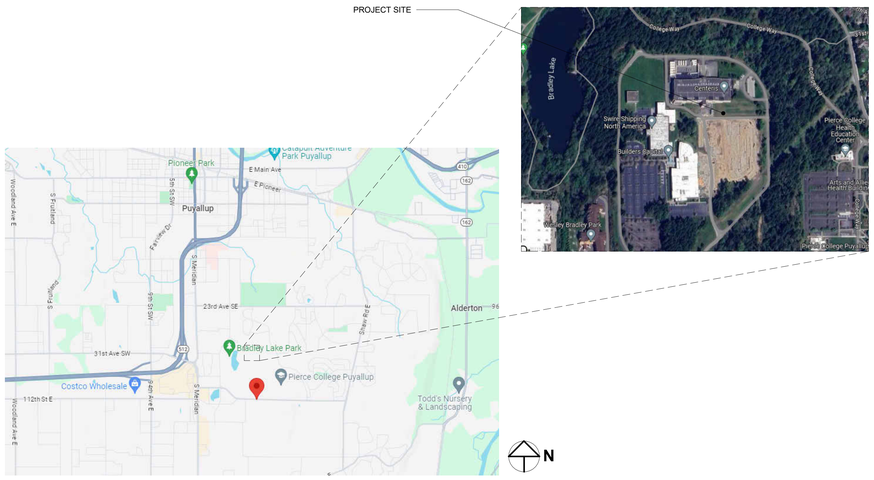
CENTERIS SHED



Vicinity Map



The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection.

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

Separate Electrical Permit is required with the Washington State Department of Labor & Industries.

https://lni.wa.gov/licensing-permits/electrical/electrical-permits-fees-and-inspections

or call for Licensing Information:
1-800-647-0982

City of Puyallup
Building
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City of Puyallup
Development & Permitting Services
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Engineers
1316 Central Ave. S., Suite 200
Kent, WA 98032
(206) 397-0000 ~ www.bse-ps.com

PRCTI20241387

CENTERIS SOUTH UTILITY YARD
SWITCHGEAR BUILDING
1023 39TH AVENUE SOUTH EAST
PUYALLUP, WA 98374

Δ	Issue	Date
	Permit Submittal	11/08/2024

Job #	24201.5
Drawn	RRS
Checked	BJB

COVER SHEET

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GENERAL NOTES

BUILDING CODE
THE 2021 EDITION OF THE 'INTERNATIONAL BUILDING CODE' (IBC), AS ADOPTED OR AMENDED BY THE LOCAL BUILDING OFFICIAL OR JURISDICTION, SHALL GOVERN DESIGN AND CONSTRUCTION.

ENGINEER
THE TERM 'ENGINEER', 'EOR', AND/OR 'SE' AS USED IN THESE STRUCTURAL DOCUMENTS SHALL MEAN BRIENEN STRUCTURAL ENGINEERS, P.S.

REFERENCE STANDARDS
ALL WORK SHALL CONFORM TO ALL REQUIREMENTS OF THE REFERENCE STANDARDS AND CODES INDICATED IN THE DRAWINGS UNLESS NOTED OTHERWISE. REFERENCE TO ASTM AND OTHER STANDARDS SHALL MEAN THE LATEST EDITION AS OF THE BID DATE OR DATE OF OWNER-CONTRACTOR AGREEMENT, WHICHEVER IS LATER, UNLESS NOTED IN THESE DOCUMENTS OR DESIGNATED BY THE BUILDING CODE.

PRIME CONTRACT DRAWINGS
THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. STRUCTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL, AND LANDSCAPING, AMONG OTHERS, ARE SUPPLEMENTARY TO THE ARCHITECTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS TO THE PRIME CONTRACT DRAWINGS, STRUCTURAL DRAWINGS, AND OTHER SUPPLEMENTARY DRAWINGS.

OMISSIONS/CONFLICTS
CONTRACTOR SHALL VERIFY ALL DIMENSIONS, FLOOR ELEVATIONS, DEPRESSIONS, FINISHES, STAIR DETAILS, GUARDRAILS, AND ETC. WITH OTHER DISCIPLINES INCLUDING ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE.

TYPICAL DETAILS
TYPICAL DETAILS SHOWN ON THE DRAWINGS SHALL APPLY UNLESS NOTED OTHERWISE. SOME TYPICAL DETAILS ARE CUT OR OTHERWISE REFERENCED IN THE DRAWINGS HOWEVER MOST OR NOT. WHERE TYPICAL DETAILS ARE NOTED ON THE DRAWINGS THE SPECIFIC DETAIL SHALL BE USED. WHERE NO DETAIL IS NOTED IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO CHOOSE THE APPROPRIATE DETAIL FROM THOSE PROVIDED.

SUBSTITUTIONS
THE CONTRACTOR SHALL SUBMIT ALL SUBSTITUTION REQUESTS (MATERIAL, PROCEDURE, CONFIGURATION, AND/OR DETAIL) TO THE ARCHITECT/ENGINEER PRIOR TO SHOP DRAWING PRODUCTION.

SPECIFICATIONS
REFER TO THE SPECIFICATIONS FOR INFORMATION IN ADDITION TO THESE NOTES AND THE STRUCTURAL DRAWINGS.

CONSTRUCTION MEANS AND METHODS AND SAFETY
CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS/METHODS AND FOR VERIFYING STRUCTURAL CAPACITY PRIOR TO APPLYING CONSTRUCTION LOADING. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY AT THE SITE AND FOR PROVIDING THE STRENGTH AND STABILITY OF ALL PARTIALLY COMPLETED STRUCTURE CONFORMING TO ASCE 37 'DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION'.

CONTRACTOR PROVIDED DESIGN SUBMITTALS
THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE ITEMS NOTED IN THE DEFERRED SUBMITTALS SECTION OF THE GENERAL NOTES.

STRUCTURAL OBSERVATION
THE ENGINEER WILL PERFORM PERIODIC OBSERVATION DURING CONSTRUCTION OF THE FOUNDATION AND STRUCTURAL FRAME AS REQUIRED BY THE BUILDING CODE.

GEOTECHNICAL

ALLOWABLE BEARING PRESSURE (DEAD + LIVE) = 1500 PSF (ASSUMED)
A 1/3 INCREASE IS ALLOWED FOR WIND OR SEISMIC

PASSIVE LATERAL PRESSURE = 250 PSF/FT
ACTIVE LATERAL PRESSURE = 35 PSF/FT
AT-REST LATERAL PRESSURE = 55 PSF/FT

COEFFICIENT OF FRICTION = 0.35 (INCLUDES A 1.5 FACTOR OF SAFETY)

DESIGN CRITERIA

BUILDING CATEGORY
STRUCTURAL RISK CATEGORY II
IMPORTANCE FACTOR SNOW ---------- Is = 1.0
IMPORTANCE FACTOR SEISMIC -------- Ie = 1.0

GRAVITY LOADS
ROOF -
DESIGN DEAD LOAD 20 PSF
LIVE LOAD 20 PSF
SNOW LOAD 25 PSF

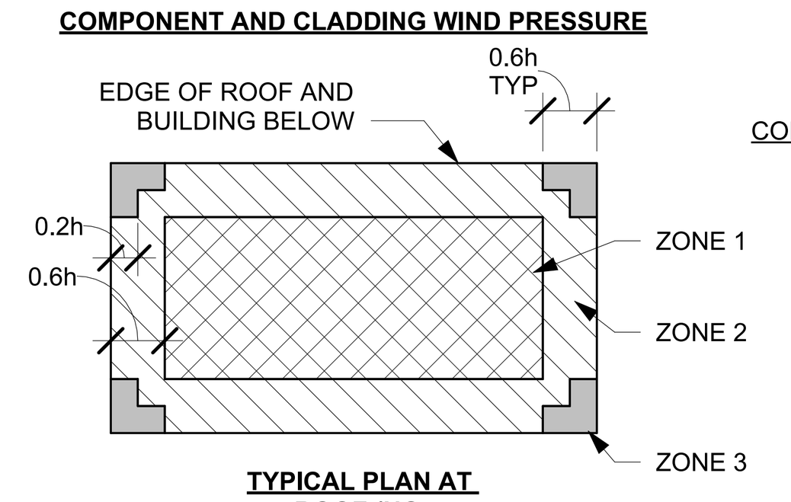
SEISMIC LOADS
SITE CLASS = D - DEFAULT
SEISMIC DESIGN CATEGORY = D
MAPPED SPECTRAL RESPONSE PARAMETERS
Ss = 1.257 g; S1 = 0.434 g
Sds = 1.006 g; Sd1 = ---

ANALYSIS TYPE = EQUIVALENT LATERAL FORCE PROCEDURE (STAGED ANALYSIS PROCEDURE)

R = 6.5 (CFS WALLS WITH WOOD STRUCTURAL PANELS)
WEIGHT, W = 26.3 KIPS, SEISMIC RESPONSE COEFFICIENT, Cs = 0.155
BASE SHEAR, V = Cs*W = 4.1 KIPS
REDUNDANCY FACTOR = 1.0

WIND LOADS
EXPOSURE CATEGORY = B
BASIC WIND SPEED = 98 MPH
Kzt = 1.0

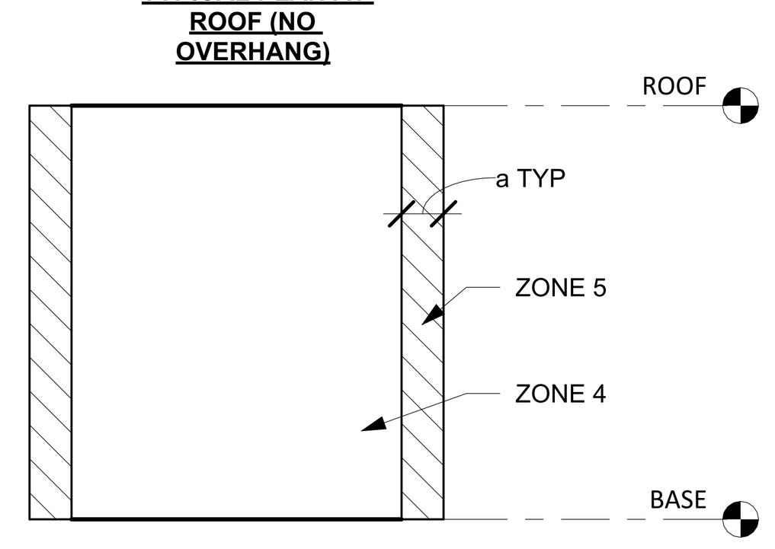
COMPONENT AND CLADDING WIND PRESSURE



TYPICAL PLAN AT ROOF (NO OVERHANG)

COMPONENTS & CLADDING LOADS (PSF)

ZONE 1: -16.0/+16.0
ZONE 2: -17.5/+16.0
ZONE 3: -23.4/+16.0
ZONE 4: -16.0/+16.0
ZONE 5: -17.0/+16.0



TYPICAL BUILDING ELEVATION

NOTES:
1. WIND LOADS FOR COMPONENT AND CLADDING ARE STRENGTH LEVEL AND DETERMINED IN ACCORDANCE WITH ASCE 7-16, CHAPTER 30, PART 1.

2. EXTERIOR COMPONENTS AND CLADDING SHALL BE DESIGNED TO ACCOMMODATE WORST-CASE WIND LOAD SHOWN.

3. POSITIVE PRESSURE ACTS TOWARDS THE SURFACE OF THE STRUCTURE. NEGATIVE PRESSURE ACTS OUTWARD AS SUCTION ON THE BUILDING SURFACE.

4. PRESSURE ARE CALCULATED USING MINIMUM EFFECTIVE AREA OF 10 sf.
FOR ROOF AREAS GREATER THAN 10 sf EXCEPT AT OVERHANGS, NEGATIVE PRESSURE MAY BE REDUCED AS FOLLOWS:
20 sf ≤ AREA < 50 sf — 5% REDUCTION
50 sf ≤ AREA < 80 sf — 12% REDUCTION
80 sf ≤ AREA < 200 sf — 16% REDUCTION
200 sf ≤ AREA — 20% REDUCTION
FOR ALL OVERHANGS, NO WIND LOAD MAY BE REDUCED.
FOR WALL AREAS AND PARAPET AREAS GREATER THAN 10 sf, POSITIVE PRESSURE MAY BE REDUCED AS FOLLOW:
20 sf ≤ AREA < 50 sf — 5% REDUCTION
50 sf ≤ AREA < 80 sf — 12% REDUCTION
80 sf ≤ AREA < 200 sf — 16% REDUCTION
200 sf ≤ AREA — 20% REDUCTION
FOR WALL AREAS AND PARAPET AREAS GREATER THAN 10 sf, NEGATIVE PRESSURE MAY BE REDUCED AS FOLLOW:
20 sf ≤ AREA < 50 sf — 3% REDUCTION
50 sf ≤ AREA < 80 sf — 8% REDUCTION
80 sf ≤ AREA < 200 sf — 10% REDUCTION
200 sf ≤ AREA — 15% REDUCTION

5. EDGE PRESSURE SHALL BE USED FOR A DISTANCE "a" FROM THE BUILDING CORNERS, WHERE "a" IS THE SMALLER OF 10% OF THE LEAST HORIZONTAL DIMENSION OR 0.4*h BUT NOT LESS THAN EITHER 4% OF THE LEAST HORIZONTAL DIMENSION OR 3'-0".

REFERENCE STANDARDS

AISI S100-16 (2020) W/ S2-20 - NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISI S202-20 - CODE OF STANDARD PRACTICE FOR COLD-FORMED STEEL STRUCTURAL FRAMING
AISI S220-20 - NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL NONSTRUCTURAL FRAMING
AISI S230-19 - STANDARD FOR COLD-FORMED STEEL FRAMING - PRESCRIPTIVE METHOD FOR ONE- AND TWO-FAMILY DWELLINGS
AISI S240-20 - NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL STRUCTURAL FRAMING
AISI S400-20 - NORTH AMERICAN STANDARD FOR SEISMIC DESIGN OF COLD-FORMED STEEL STRUCTURAL SYSTEMS

MATERIAL CRITERIA

MATERIAL
COLD-FORMED STEEL MATERIAL SHALL BE MANUFACTURED AND FORMED, PER ASTM A1003/A1003M, FROM GALVANIZED ASTM A653 SS GRADE 50 STEEL FOR 54, 68 AND 97 MIL BASE THICKNESS MATERIAL AND FROM GALVANIZED ASTM A653 SS GRADE 33 MATERIAL FOR 43 AND 33 MIL BASE THICKNESS MATERIAL, UNO. WHERE NOTED, PAINTED COLD-FORMED STEEL MATERIAL SHALL CONFORM TO ASTM A570 SS GRADE 80. MINIMUM COLD-FORMED STEEL ACCEPTANCE CRITERIA SHALL BE PER ICC-ES AC46.

ALL GALVANIZED MEMBERS SHALL CONFORM TO ASTM A924 WITH THE FOLLOWING MINIMUM COATING REQUIREMENTS: NON-STRUCTURAL MEMBERS (ASTM C645 - G40 COATING), STRUCTURAL MEMBERS (ASTM C955 - G60 COATING), EXPOSED EXTERIOR MEMBERS (ASTM C955 - G90 COATING).

EACH MEMBER SHALL BEAR A LEGIBLE STICKER, STAMP, STENCIL, OR EMBOSSMENT, SPACED A MAXIMUM OF 48"OC ON THE WEB OF THE FRAMING MEMBER, INDICATING THE MINIMUM STEEL SHEET THICKNESS, METALLIC-COATING DESIGNATION, MINIMUM YIELD STRENGTH, PRODUCT DESIGNATION, AND NAME OF MANUFACTURER. WHERE MEMBERS ARE NOT LABELED, THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THAT THE STEEL IS IN COMPLIANCE WITH THE PROJECT SPECIFICATIONS.

SUBMITTALS
PRODUCT DATA FOR ALL MEMBERS, ACCESSORIES, AND FASTENERS SHALL BE SUBMITTED TO THE ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING DEPARTMENT FOR REVIEW AND ACCEPTANCE PRIOR TO FABRICATION AND ERECTION. FRAMING SUBSTITUTIONS SHALL BE SUBJECT TO REVIEW AND ACCEPTANCE BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION AND INSTALLATION.

COLD-FORMED STEEL FRAMING
ALL COLD-FORMED STEEL FRAMING SHALL BE IN ACCORDANCE WITH AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", AS AMENDED BY THE INTERNATIONAL BUILDING CODE AND SHALL STRICTLY CONFORM WITH ICC REPORT ER-4943P.

ALL COLD-FORMED STEEL PRODUCTS SHALL BE MANUFACTURED BY CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA). MATERIAL DESIGNATIONS NOTED ON THE DRAWINGS, RELATING TO MEMBER TYPES AND SIZES OR MISCELLANEOUS FRAMING ITEMS, REFER TO PRODUCT IDENTIFICATION STANDARDS ADOPTED BY THE SSMA. SSMA PRODUCTS HAVE A FOUR PART IDENTIFICATION CODE AS INDICATED IN THE FOLLOWING EXAMPLE:

MEMBER DEPTH IN 1/100 INCHES
STYLE:
S= STUD/JOIST SECTION
T= TRACK SECTION
U= CHANNEL SECTION
F= FURRING CHANNEL SECTION
L= ANGLE OR L-HEADER

600 S 162 - 54

FLANGE WIDTH IN 1/100 INCHES
MATERIAL THICKNESS IN MILS (1 MIL = 1/1000 INCH)

INSTALLATION
EACH JOIST, RAFTER, TRUSS AND STRUCTURAL WALL STUDS SHALL BE ALIGNED WITHIN 3/4 FROM CENTERLINE OF HORIZONTAL FRAMING MEMBER TO CENTERLINE OF VERTICAL FRAMING MEMBER, UNO, OR AS SPECIFIED IN FIGURE C1-1 OF THE AISI STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS". STRUCTURAL FRAMING MEMBERS SHALL BE INSTALLED PER ASTM C1007 AND NON-STRUCTURAL FRAMING MEMBERS PER ASTM C754.

CONCRETE BEARING SURFACES AT STRUCTURAL FRAMING SHALL PROVIDE A UNIFORM BEARING SURFACE WITH A MAXIMUM 1/4" GAP BETWEEN THE TRACK AND THE CONCRETE. STEEL BEARING SHIMS OR NON-SHRINK GROUT CAN BE USED TO ACHIEVE THIS REQUIREMENT. THE BOTTOM TRACK OF LOAD BEARING WALLS SHALL NOT EXTEND OVER THE EDGE OF FORMED CONCRETE BEARING SURFACES BELOW.

COLD-FORMED STEEL SHALL NOT BE IN DIRECT CONTACT WITH THE GROUND UNLESS NOTED OTHERWISE.

COLD-FORMED STEEL CONNECTIONS

SCREWS
FOR STEEL-TO-STEEL CONNECTIONS AND FOR STRUCTURAL SHEATHING-TO-STEEL CONNECTIONS SHALL BE SELF TAPPING, SELF DRILLING FASTENERS IN COMPLIANCE WITH ASTM C1513 AND SHALL HAVE A TYPE II COATING IN ACCORDANCE WITH ASTM B633 "ELECTRO-DEPOSITED COATING OF ZINC ON IRON AND STEEL". SELF-PIERCING SCREWS PER ASTM C1002 ARE PERMITTED FOR CONNECTION OF 33 MILS STEEL OR THINNER. THE SCREW MANUFACTURER SHALL PROVIDE VERIFICATION OF THE FASTENERS' RESISTANCE TO HYDROGEN EMBRITTLEMENT. SCREWS SHALL CONFORM TO SAEJ78 "STANDARD SPECIFICATION FOR SELF-DRILL TAPPING SCREWS". SCREW ACCEPTANCE SHALL BE BASED ON ICC-ES AC118 "ACCEPTANCE CRITERIA FOR TAPPING SCREW FASTENERS".

SCREW CONNECTIONS SHALL BE IN COMPLIANCE WITH THE AISI STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS, 2007 EDITION". SCREW CONNECTIONS SHALL BE MADE FROM THE LIGHTER MATERIAL INTO THE THICKER MATERIAL, UNO. SCREWS SHALL EXTEND THROUGH THE STEEL CONNECTION A MINIMUM OF THREE EXPOSED THREADS AND SHALL HAVE MINIMUM CENTER-TO-CENTER SPACING AND EDGE DISTANCES OF THREE TIMES THE NOMINAL SCREW DIAMETER. SCREWS SHALL BE INSTALLED AND TIGHTENED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SHALL NOT CAUSE PERMANENT SEPARATION BETWEEN COMPONENTS. SHEATHING FASTENERS SHALL MAINTAIN A MINIMUM 3/8" EDGE DISTANCE IN SHEATHING AND SHALL HAVE THEIR HEADS FLUSH WITH THE SHEATHING (BUT NO MORE THAN 1/16" BELOW THE SURFACE OF THE SHEATHING).

STRIPPED SCREWS IN DIRECT TENSION SHALL BE CONSIDERED INEFFECTIVE AND SHALL BE REPLACED. STRIPPED SCREWS IN SHEAR THAT CONSTITUTE MORE THAN 25% OF THE TOTAL SCREWS IN THE CONNECTION SHALL BE CONSIDERED INEFFECTIVE AND SHALL BE REPLACED. STRIPPED SCREWS ARE PERMITTED TO BE REMOVED AND REPLACED WITH SCREWS OF THE NEXT LARGER DIAMETER.

MINIMUM SCREW SIZES IN COLD-FORMED STEEL TABLE

CONNECTION	MINIMUM SCREW SIZE
METAL TO METAL (68 MILS)	#10-16 (#3 POINT)
METAL TO METAL (33 MILS - 54 MILS)	#8-18 (#2 POINT)
METAL TO METAL (SHEAR WALLS)	#8-18 (#2) POINT) WAFER HEAD
APA SHEATHING (SHEAR WALLS)	#8-18 (#2 POINT FLAT HEAD w/ 0.292"Ø HEAD MIN
GWB OR GYPSUM SHEATHING	#6 x 1" (#2 POINT) DRYWALL
METAL DECK TO FRAMING	#12-14 (#3 POINT)
SIMPSON HARDWARE	PER SIMPSON CATALOG

ALLOWABLE LOADS FOR SCREW CONNECTIONS (LBS/SCREW) SHALL BE AS INDICATED IN THE TABLE BELOW AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL EITHER PROVIDE SCREW TEST DATA FROM THE SCREW MANUFACTURER'S QUALITY ASSURANCE PROGRAM OR SHALL RETAIN AN INDEPENDENT CERTIFIED TESTING AGENCY TO PROVIDE THE LOAD TEST VERIFICATION. NOMINAL STRENGTH OF SCREWS SHALL BE AT LEAST 3.0 TIMES THE ALLOWABLE LOADS SHOWN IN THE TABLE.

ALLOWABLE LOADS FOR SCREW CONNECTIONS (POUNDS)

SHEET METAL SCREW SIZE	STEEL THICK	33 Mils (0.0346")		43 Mils (0.0451")		54 Mils (0.0566")		68 Mils (0.0713")	
	STEEL PROPTY (KSI)	Fy	Fu	Fy	Fu	Fy	Fu	Fy	Fu
		33	45	33	45	50	65	50	65
NO. 6 (Ø0.138")	SHEAR	141		214		214		214	
	PULLOUT	61		79		140		140	
NO. 8 (Ø0.164")	SHEAR	164		244		426		426	
	PULLOUT	72		94		171		195	
NO. 10 (Ø0.190")	SHEAR	177		236		534		548	
	PULLOUT	84		109		198		249	
No. 12 (Ø0.216")	SHEAR	188		280		569		777	
	PULLOUT	95		124		225		284	

COLD FORMED STEEL CONNECTORS
COLD-FORMED STEEL CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY THE SIMPSON STRONG-TIE COMPANY. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. PROVIDE NUMBER, LENGTH, TYPE, AND SIZE OF FASTENERS AS SPECIFIED BY THE MANUFACTURER. FILL ALL HOLES WITH FASTENERS AS SPECIFIED BY THE MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

WELDING
WELDING OF COLD-FORMED METAL FRAMING SHALL CONFORM TO LATEST EDITION OF THE "STRUCTURAL WELDING CODE - SHEET STEEL" AWS D1.3 AND SHALL BE PERFORMED BY WELDERS CERTIFIED BY W.A.B.O. TO PRODUCE THE SPECIFIED CLASSES OF WELD. ONLY PRE-QUALIFIED WELDS (AS DEFINED BY AWS) SHALL BE USED. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES OR PROCESSES. LOW HYDROGEN ELECTRODES SHALL BE PROVIDED IN HERMETICALLY SEALED CONTAINERS; ELECTRODES THAT HAVE BEEN WET SHALL NOT BE USED. REFER TO AWS REQUIREMENTS REGARDING ALLOWABLE EXPOSURE OF LOW HYDROGEN ELECTRODES TO THE ATMOSPHERE AND FOR RE-DRYING RECOMMENDATIONS AND RESTRICTIONS. MATCHING FILLER METALS PER AWS D1.3 SHALL BE USED AND SHALL BE ADJUSTED TO ELIMINATE BURN-THROUGH IN LIGHT-GAUGE STEEL MATERIALS. WELDED AREAS SHALL BE TREATED WITH ZINC PAINT CONFORMING TO ASTM A780. FOR MATERIAL LESS THAN 0.15" THICK, DRAWINGS SHOW NOMINAL WELD SIZES. FOR SUCH MATERIAL, THE EFFECTIVE THROAT OF WELDS SHALL NOT BE LESS THAN THE THICKNESS OF THE THINNEST CONNECTED PART. PUDDLE WELDS ARE PROHIBITED. WELDING OF COLD-FORMED STEEL SHALL ONLY BE APPLIED TO MATERIAL WITH A BASE STEEL THICKNESS OF 43 MILS OR GREATER, UNO.

BOLTED CONNECTIONS
BOLTED CONNECTIONS SHALL BE IN ACCORDANCE WITH SECTION E3 OF THE AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURES". PRE-DRILLED HOLES FOR BOLTS SHALL NOT BE OVERSIZED MORE THAN 1/16" FOR BOLTS UP TO 1/2" IN DIAMETER AND 1/8" FOR LARGER BOLTS. BURNED HOLES ARE PROHIBITED.

DRILLED ANCHORS
DRILLED ANCHORS USED TO FASTEN COLD-FORMED STEEL MEMBERS TO CONCRETE SHALL BE KWIK BOLT TZ AS MANUFACTURED BY THE HILTI CORPORATION. AND SHALL CONFORM TO ICC REPORT ESR-1917 INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. FASTENERS SHALL NOT BE INSTALLED BEFORE THE SPECIFIED 28 DAY COMPRESSIVE STRENGTH OF THE CONCRETE HAS BEEN OBTAINED. SUBSTITUTIONS PROPOSED BY THE CONTRACTOR SHALL BE SUBMITTED WITH ICC REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. CONNECTED MEMBERS MAY LIMIT ACTUAL DESIGN VALUES.

POWDER-DRIVEN OR PNEUMATIC FASTENERS
POWDER-DRIVEN OR PNEUMATIC FASTENERS USED TO FASTEN COLD-FORMED STEEL MEMBERS TO STRUCTURAL STEEL OR CONCRETE SHALL BE MANUFACTURED BY THE HILTI CORPORATION, AS INDICATED IN THE TABLE BELOW. ALL FASTENERS SHALL CONFORM STRICTLY TO ICC REPORT ESR-2269 INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. FASTENERS THROUGH STRUCTURAL STEEL SHALL FULLY PENETRATE THE STRUCTURAL STEEL WITH A MINIMUM PENETRATION OF 1/4" THROUGH THE LAST MATERIAL JOINED. UNDERDRIVEN PINS SHALL NOT BE RESET BUT SHALL BE REPLACED BY ANOTHER PIN INSTALLED IN ANOTHER LOCATION. FASTENERS IN CONCRETE SHALL NOT BE INSTALLED BEFORE THE SPECIFIED 28 DAY COMPRESSIVE STRENGTH OF THE CONCRETE HAS BEEN ACHIEVED. SUBSTITUTIONS PROPOSED BY THE CONTRACTOR SHALL BE SUBMITTED WITH ICC REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. THE FOLLOWING TABLE INDICATES REQUIRED SHEAR AND PULLOUT VALUES (LBS/FASTENER). CONNECTED MEMBERS MAY LIMIT ACTUAL DESIGN VALUES.

STATEMENT OF SPECIAL INSPECTIONS

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED PER THE REQUIREMENTS OF IBC SECTION 1704 AND AS NOTED HEREIN.

CONCRETE

VERIFICATION AND INSPECTION	C	P	REFERENCED STANDARD	NOTES
ANCHORS CAST IN CONCRETE		X	ACI 318: 17.8.2	SPECIAL INSPECTION SHALL CONFORM TO ACI 26.13 UNO
INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAIN TENSION LOADS.	X		ACI 318: 17.8.2.4	SPECIAL INSPECTIONS NOT REQUIRED FOR CONTINUOUS FOOTINGS SUPPORTING WALLS OF THREE-STORIES AND LESS ABOVE GRADE PLANE WHERE WALLS ARE LIGHT-FRAME CONSTRUCTION AND STRUCTURAL DESIGN IS BASED ON F'c ≤ 2500 PSI
B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN A.		X	ACI 318: 17.8.2	

COLD-FORMED STEEL FRAMING

VERIFICATION AND INSPECTION	C	P	REFERENCED STANDARD	NOTES
SCREW ATTACHMENT, WELDING, BOLTING, ANCHORING AND FASTENING OF SHEAR WALLS, BRACES, DIAPHRAGMS, DRAG STRUTS, AND HOLD-DOWNS THAT ARE PART OF SEISMIC RESISTING SYSTEM		X	AWS D1.3 IBC 1705.12.2, 1705.13.3	EXCEPTIONS PER IBC 1705.12.2
ROOF AND WALL CLADDING		X	IBC 1705.12.3, 1705.13.5	
NON LOAD BEARING WALLS		X	IBC 1705.13.5	EXCEPTIONS PER IBC 1705.13.5

SPECIAL INSPECTION OF PLUMBING, ELECTRICAL AND MECHANICAL COMPONENTS PER IBC 1705.13.6 WHERE APPLICABLE.

"**C**" DENOTES CONTINUOUS INSPECTION
"**P**" DENOTES PERIODIC INSPECTION

TESTING AND SPECIAL INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A DAILY BASIS WHENEVER WORK IS PERFORMED ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO OWNER, CONTRACTOR, BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER. SPECIAL INSPECTOR SHALL UTILIZE DRAWINGS, SPECIFICATIONS, RFI'S, AND OTHER PERTINENT DESIGN DOCUMENTS DURING INSPECTIONS.

SPECIAL INSPECTOR SHALL CLEARLY NOTE ON THE INSPECTION REPORTS WHEN AN ITEM IS NOT IN CONFORMANCE WITH THE PLANS AND SPECIFICATION, AND KEEP A LOG OF EACH ITEM UNTIL THEY ARE CLEARED VIA RFI OR OTHER MEANS.

SPECIAL INSPECTOR SHALL PROVIDE A STRUCTURAL CLOSE OUT LETTER AT THE END OF THE PROJECT. THIS LETTER SHALL CONFIRM THAT ALL STRUCTURAL NON-CONFORMANCES NOTED IN INSPECTION REPORTS HAVE BEEN CLEARED AND THAT TO THE BEST OF THEIR KNOWLEDGE THERE ARE NO OUTSTANDING STRUCTURAL DEFICIENIES TO BE RESOLVED.

STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNATED REPRESENTATIVE IN ACCORDANCE WITH IBC 1704.6.
STRUCTURAL OBSERVATION SHALL BE PERFORMED AS FOLLOWS:

- PERIODIC VISUAL OBSERVATION OF STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES.
- REVIEW OF TESTING AND INSPECTION REPORTS.
- REPORTS SHALL BE PREPARED FOR EACH SITE VISIT AND SHALL BE DISTRIBUTED TO ARCHITECT.

GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL INCLUDE ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

BSE
Brienen Structural Engineers
1316 Central Ave. S., Suite 200
Kent, WA 98032
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sessler INC.
PRCTI20241387

CENTERIS SOUTH UTILITY YARD SWITCHGEAR BUILDING
1023 39TH AVENUE SOUTH EAST
PUYALLUP, WA 98374

Δ	Issue	Date
	Permit Submittal	11/08/2024

Job #	24201.5
Drawn	RRS
Checked	BJB

GENERAL NOTES

City of Puyallup
Development & Permitting Services
ISSUED PERMIT
Building | Planning
Engineering | Public Works
Fire | Traffic

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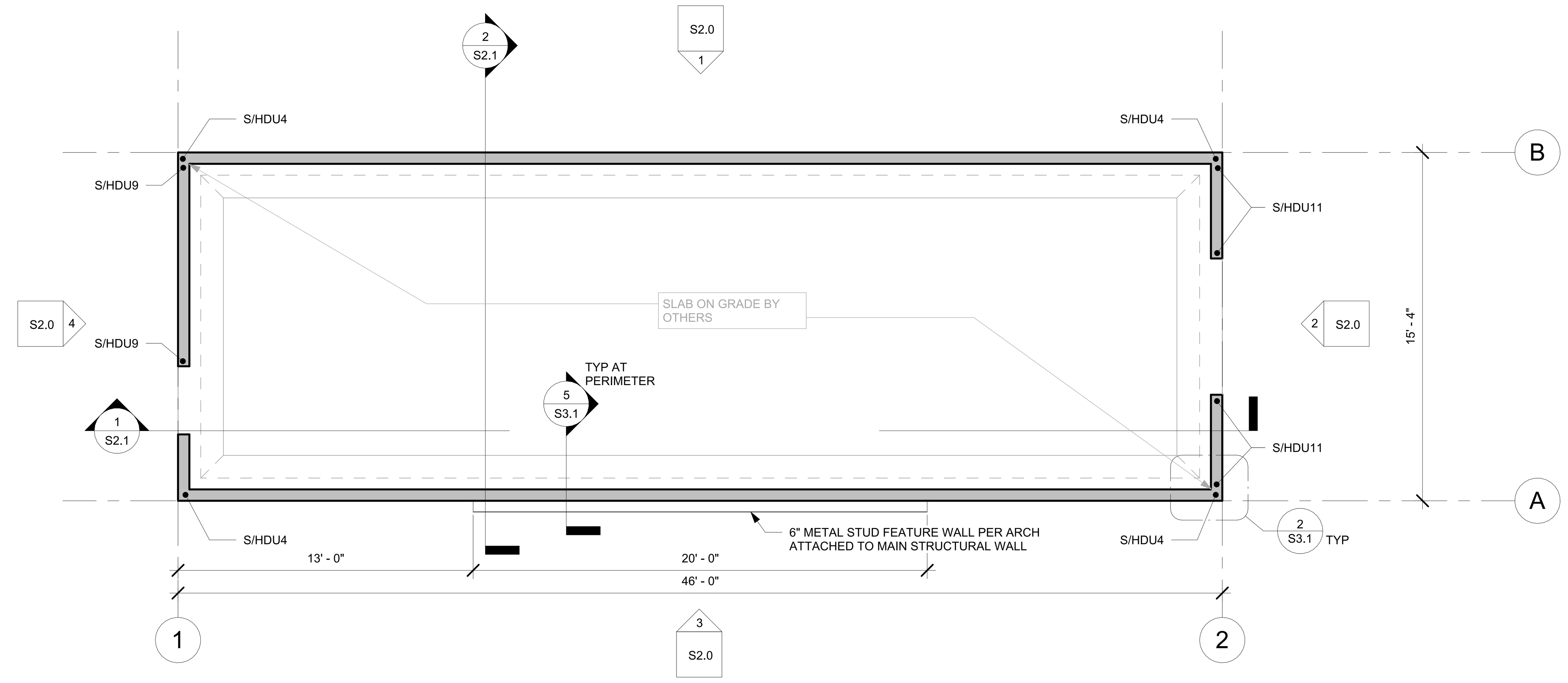
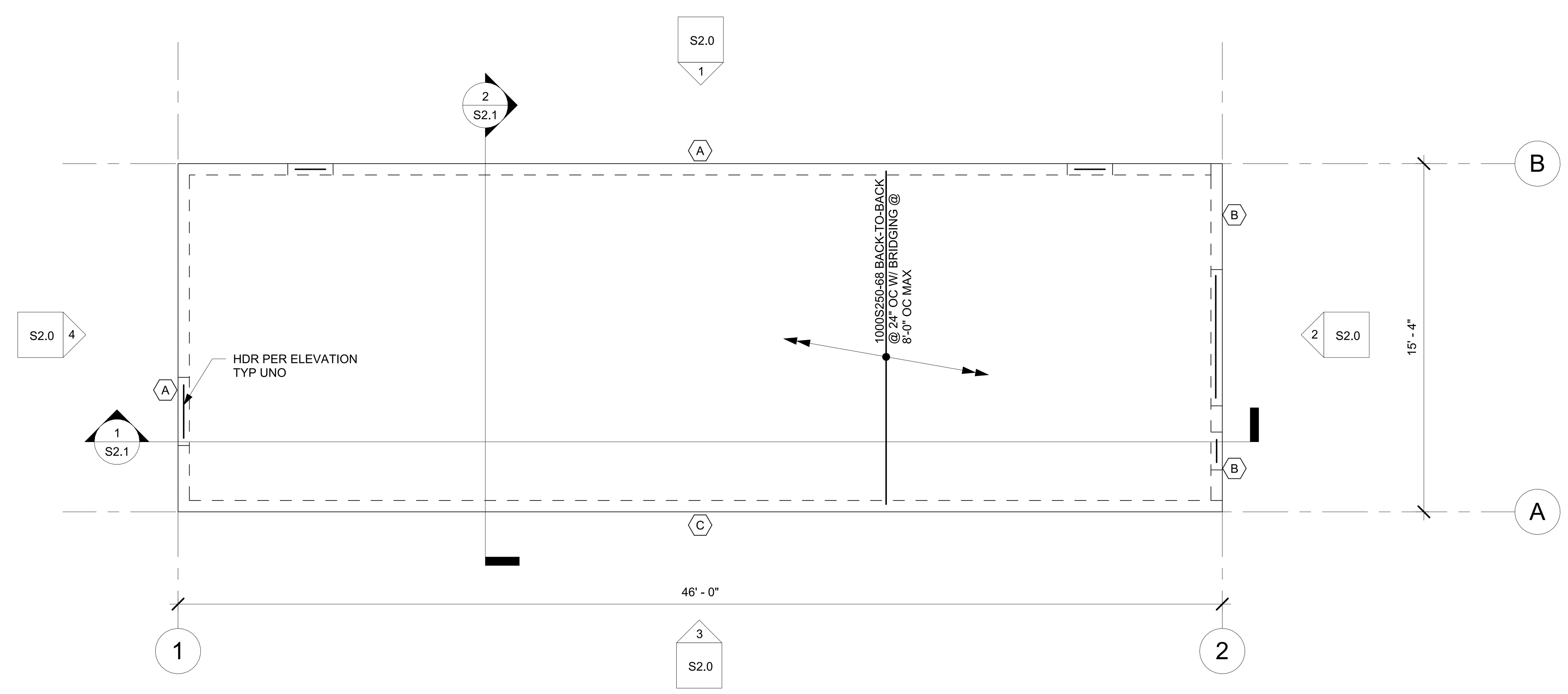


1 FOUNDATION PLAN
1/4" = 1'-0"



2 ROOF PLAN
1/4" = 1'-0"

COLD-FORMED STEEL FRAMING PLAN NOTES

1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
2. COLD-FORMED STEEL STUD WALLS SHALL ALIGN WITH TRUSS/JOIST LAYOUT AND BE SPACED AT 24" OC MAXIMUM, UNLESS NOTED OTHERWISE.
3. (A) INDICATES PLYWOOD SHEAR WALL. SEE SHEAR WALL SCHEDULE ON SHEET S3.0 AT WALLS SHEATHED ON ONE FACE ONLY, PLACE SHEATHING ON THE CHEVRON TAG SIDE. ALL EXTERIOR BEARING AND NON-BEARING WALLS SHALL MEET SHEAR WALL TYPE A REQUIREMENTS UNLESS NOTED OTHERWISE.
4. PLACE ALL HOLDOWNS AND SILL ANCHORS EMBEDDED IN CONCRETE PRIOR TO CASTING OF THE CONCRETE. DRILLING SUCH ITEMS IN AFTER THE DECK IS CAST SHALL BE ASSUMED NOT FESABLE.
5. PROVIDE 3/4" NOMINAL TOUNGE & GROOVE WOOD SHEATHING OVER ALL ROOF STRUCTURE. PROVIDE #8 SMS @ 6" OC EDGE NAILING AND #8 SMS @ 10" OC FIELD NAILING, UNLESS OTHERWISE NOTED.
6. ALL ROOF TRUSSES SHALL ALIGN WITH WALL STUDS.
7. PROVIDE EDGE NAILING OVER ALL RIM JOISTS AND TRUSS BLOCKINGS OVER SHEAR WALLS.
8. REFERENCE DRAWINGS:
   S3.X FOR TYPICAL CFS DETAILS

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FRAMING PLANS

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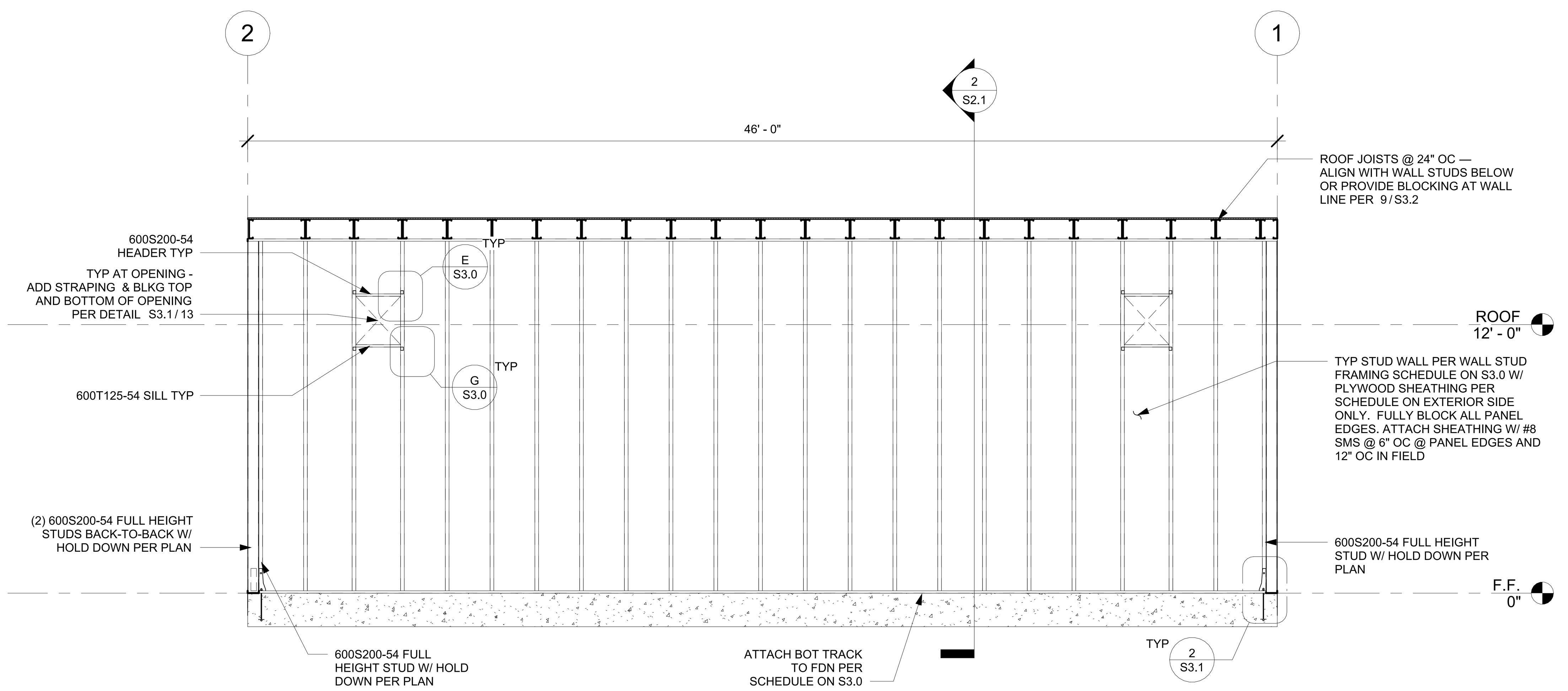
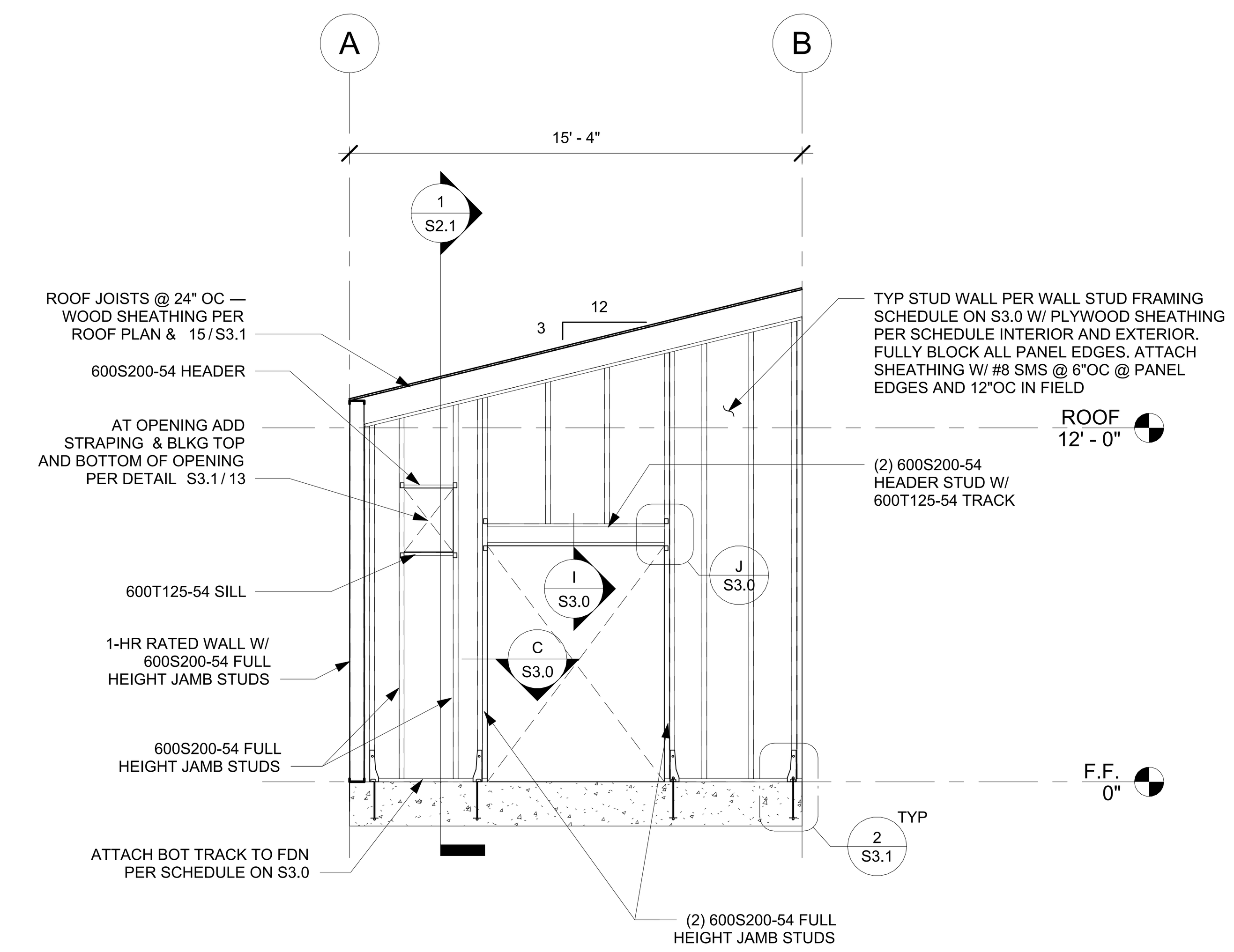
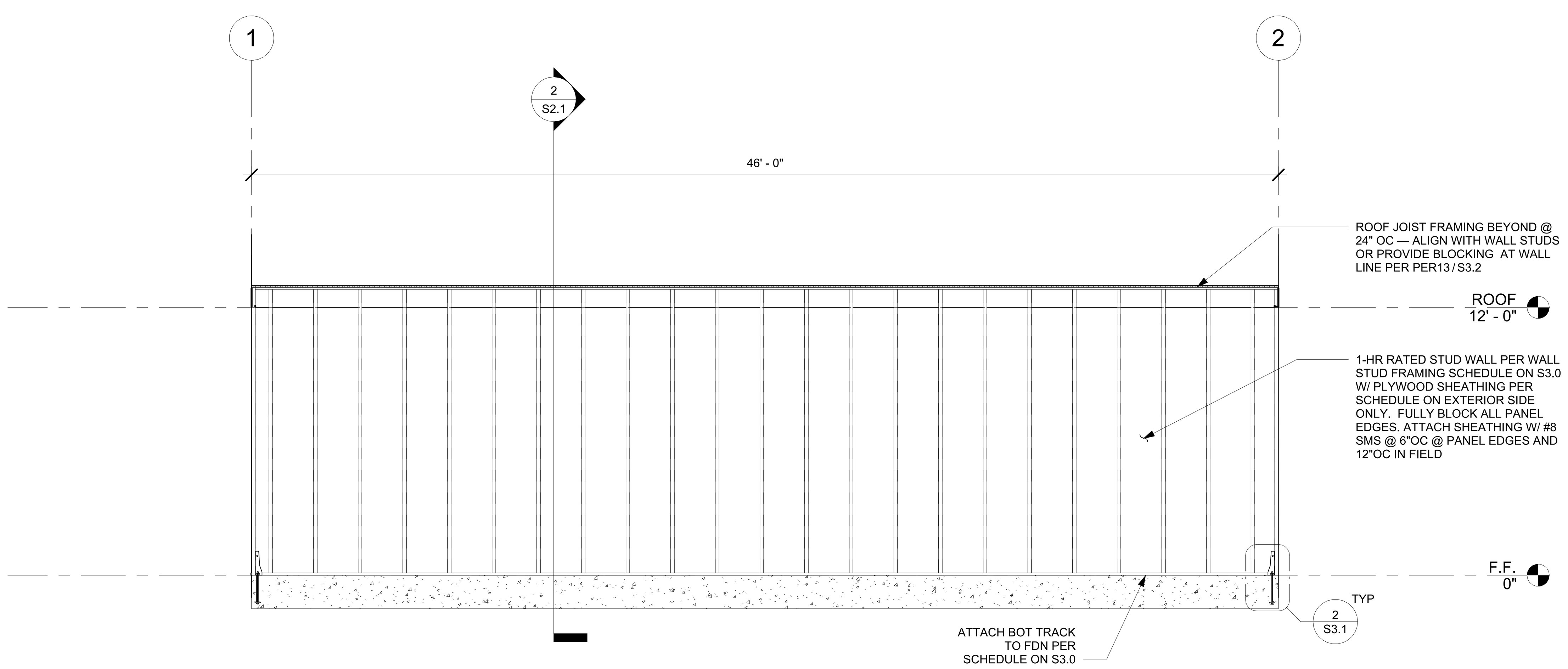


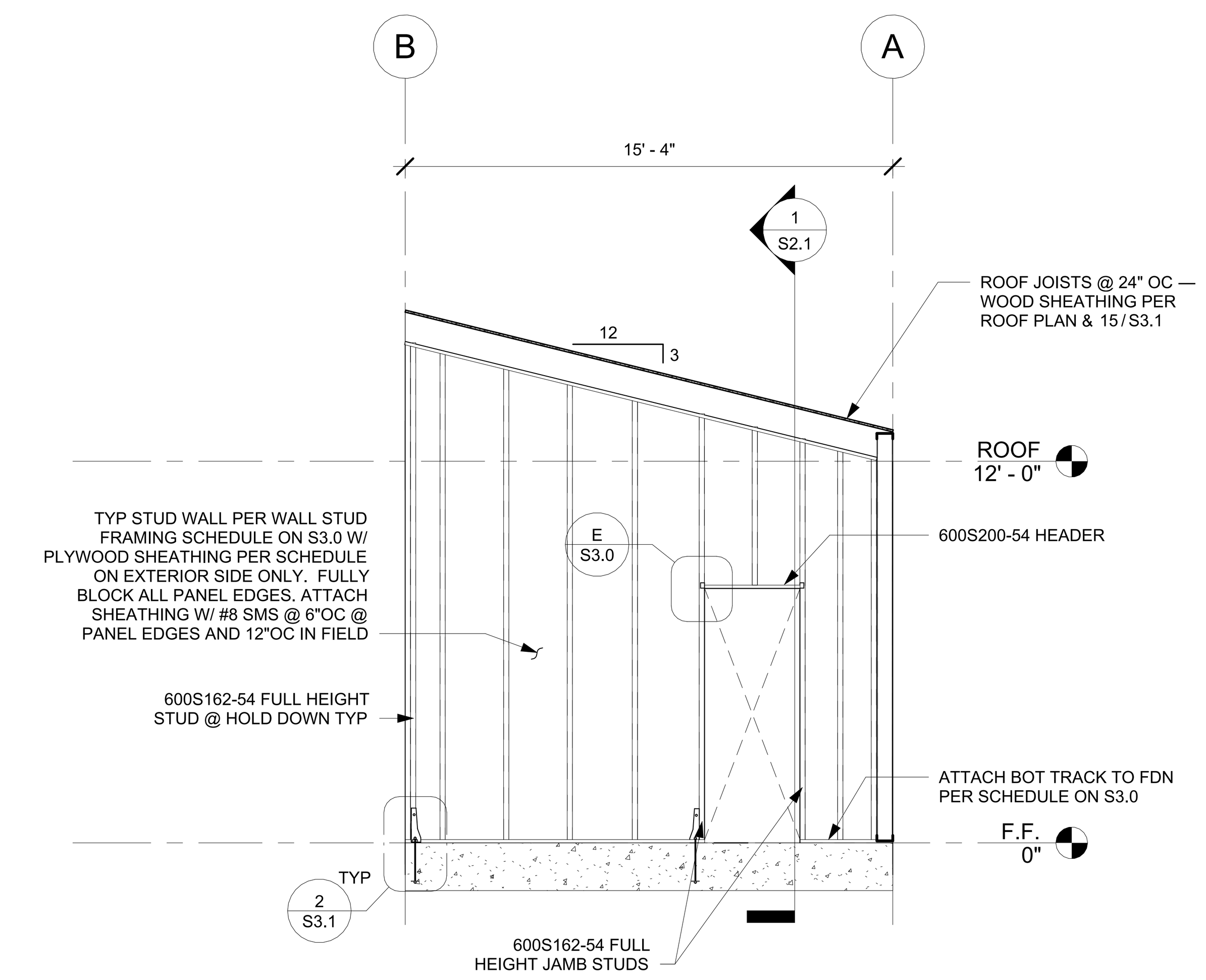
1 NORTH ELEVATION
1/4" = 1'-0"



2 EAST ELEVATION
1/4" = 1'-0"



3 SOUTH ELEVATION
1/4" = 1'-0"



4 WEST ELEVATION
1/4" = 1'-0"

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BUILDING ELEVATIONS

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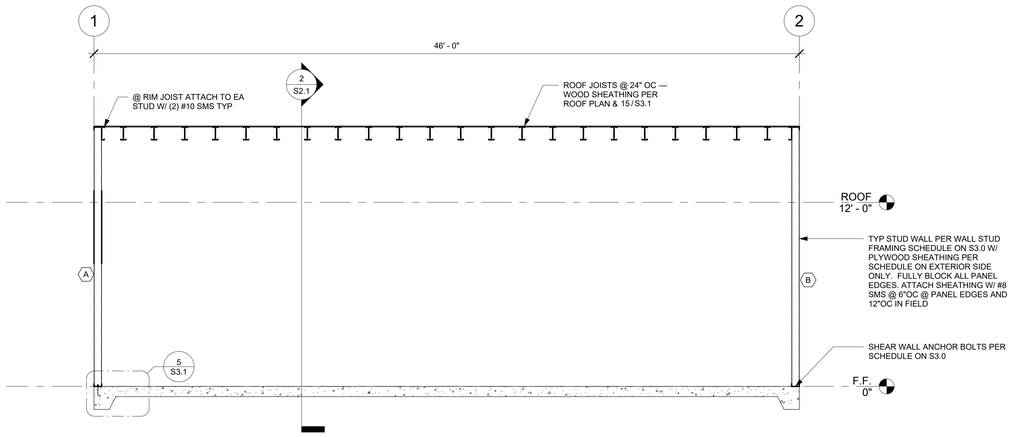
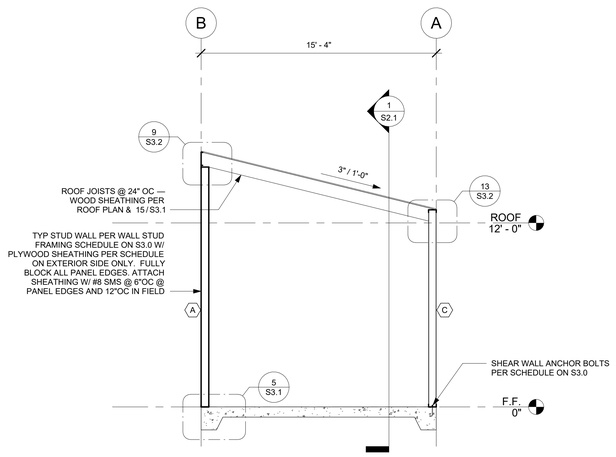


1 EAST-WEST SECTION
1/4" = 1'-0"



2 NORTH-SOUTH SECTION
1/4" = 1'-0"

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BUILDING SECTIONS

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EXTERIOR METAL STUD WALL FRAMING SCHEDULE						
WALL TYPE	STUD TYPE	SPACING	TRACKS		STUD-TO-TRACK CONNECTION	
			BOTTOM	TOP	BOTTOM	TOP
A	600S200-54	24" OC	600T125-54	600T250-54	A	B
B	600S200-54	24" OC	600T125-54	600T250-54	A	B
C	600S200-54	24" OC	600T125-54	600T350-68	A	B

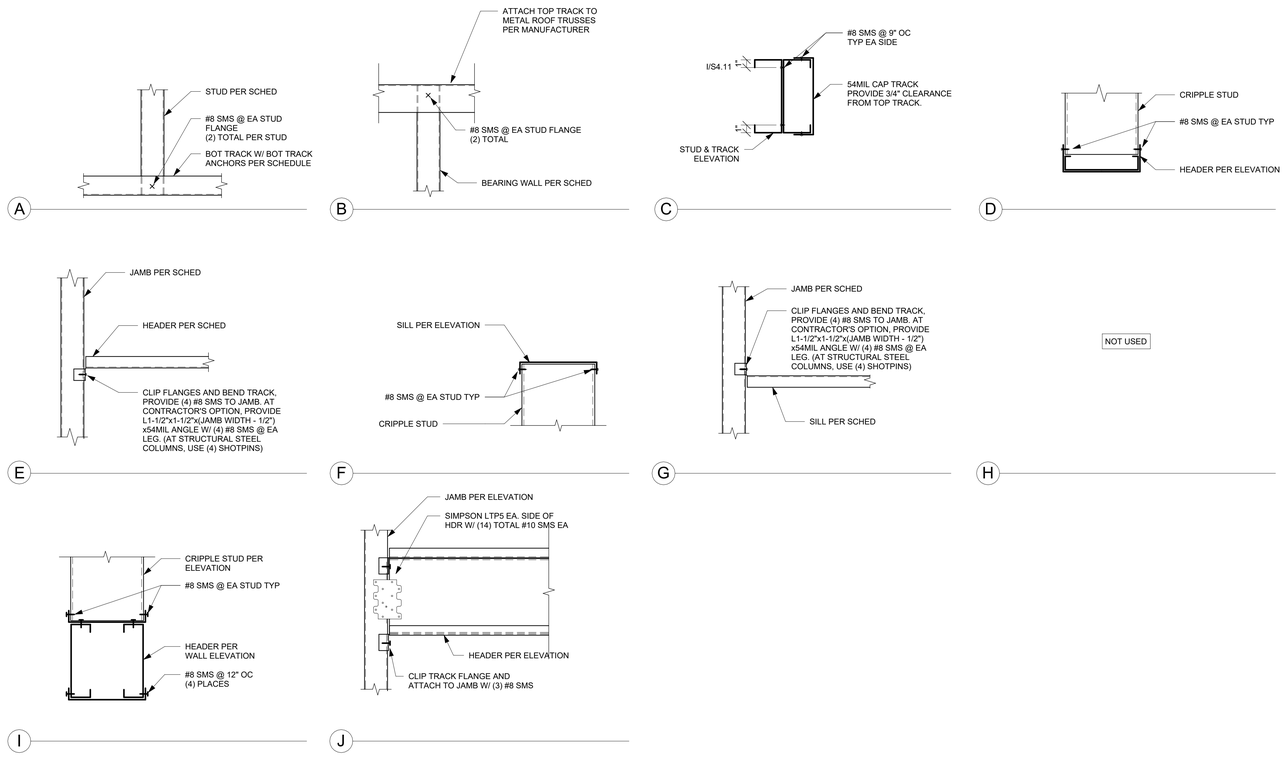
EXTERIOR METAL STUD WALL FRAMING SCHEDULE NOTES

1. FULLY-SHEATHE FACE OF STUDS AS NOTED ON PLANS FULL-HEIGHT OR PROVIDE BRIDGING OR SOLID BLOCKING AT 4'-0" OC MAX UNLESS NOTED OTHERWISE
2. ALL STUDS SHALL FULLY BEAR AT TOP AND BOTTOM TRACK – SHIM WHERE NECESSARY. WEB STIFFENERS ARE NOT REQUIRED UNLESS OTHERWISE SPECIFIED.
3. TOP/BOTTOM TRACK PENETRATIONS OR FLANGE CLIPS UP TO 2/3 (TRACK WIDTH) ARE STRUCTURALLY ACCEPTABLE 16" CLEAR FROM ANY JAMB STUDS - ADD ANCHOR ON EACH SIDE OF OPENING.
4. WALL STUDS, CRIPPLE STUDS, JAMBS, HEADERS AND SILLS SHALL NOT BE SPLICED.
5. ALL COLD-FORMED STEEL STUDS, TRACKS AND LIGHT GAUGE ANGLES SHALL CONFORM TO ASTM A653 SS GRADE 50 (Fy=50KSI) FOR 118, 97, 68 AND 54 MILS MEMBERS AND ASTM 653 SS GRADE 33 (Fy=33KSI) FOR 43 MILS AND LIGHTER MEMBERS.
6. BLOCKING SHALL MATCH GAUGE OF WALL STUD AND 1 1/2" WIDE, MINIMUM.
7. CONCRETE SCREWS SHALL BE HILTI KH-EZ SCREW-TYPE CONCRETE ANCHOR OR SIMPSON TITEN HD. SEE DETAILS FOR REQUIRED DIAMETERS AND EMBEDMENTS. ALL DRILLING IN CONCRETE SHALL CONFORM TO REQUIREMENTS IN GENERAL NOTES. DO NOT DAMAGE REINFORCING IN CONCRETE SLAB OR FOUNDATION.
8. SHEET-METAL SCREWS (SMS) SHALL BE SELF-TAPPING, SELF-DRILLING FASTENERS IN COMPLIANCE WITH ASTM C1513 AND SHALL HAVE A TYPE II COATING IN ACCORDANCE WITH ASTM B633.
9. IT IS STRUCTURALLY ACCEPTABLE TO USE A THICKER FRAMING MEMBER PROVIDED THE WEB SIZE REMAINS UNCHANGED AND FLANGE SIZE REMAINS UNCHANGED OR IS INCREASED.

SPECIAL CFS FRAMING SHEAR WALL W/ PLYWOOD			
SHEAR WALL REQUIREMENTS			
MARK	STRUCTURAL PANEL REQ'MENTS	EDGE SCREWS (NOTES 2, 4)	BOTTOM TRACK ANCHORS (NOTE 6)
A	(1) SIDE 7/16" WOOD SHT'G	#8 @ 6" OC	5/8"Ø ANCHOR @ 32" OC - EMBED 7" UNO
B	(2) SIDE 7/16" WOOD SHT'G	#8 @ 4" OC	5/8"Ø ANCHOR @ 32" OC - EMBED 7" UNO
B	(1) SIDE 7/16" WOOD SHT'G	#8 @ 6" OC	5/8"Ø ANCHOR @ 32" OC - EMBED 7" UNO

SHEAR WALL FRAMING NOTES:

1. (A) INDICATES STRUCTURAL WALL MARK. STUDS SHALL ALIGN WITH JOISTS AND STUDS ABOVE. FOR NUMBER OF STUDS REQUIRED BELOW JOIST/STUD ABOVE SEE SCHEDULE. FOR TYPICAL WALL STUD BRIDGING DETAIL AND ADDITIONAL WALL FRAMING REQUIREMENTS SEE 2/S3.2
2. ALL EXTERIOR AND INTERIOR WALLS DESIGNATED AS SHEAR WALLS SHALL BE BLOCKED AT ALL UNSUPPORTED PANEL EDGES. EDGE SCREW SPACING APPLIES TO TOP AND BOTTOM TRACK, PANEL VERTICAL AND HORIZONTAL JOINTS, WALL CORNERS, HOLDOWN ANCHOR STUDS, AND WALL END STUDS. SHEAR WALL STUD FLANGE WIDTH SHALL BE S162 MINIMUM. BLOCKING SHALL MATCH GAUGE OF WALL STUD x 1 1/2" WIDE MINIMUM.
3. SHEATHING PANELS MAY BE INSTALLED PARALLEL OR PERPENDICULAR TO FRAMING.
4. SCREWS SHALL BE SELF-DRILLING, SELF-TAPPING FLAT HEAD SCREWS WITH MINIMUM 0.292" HEAD DIAMETER.
5. PROVIDE 3"x3"x1/4" PLATE WASHER AT EACH ANCHOR BOLT - WELD PLATE WASHER TO TRACK WITH FILLET WELD ALL AROUND. DISTANCE FROM ANCHOR TO END OF TRACK TO BE 3" MIN AND 6" MAX.
6. ACCEPTABLE TO USE 5/8"Ø HILTI HUS-EZ SCREW ANCHORS WITH 3-1/2" EMBED AS AN ALTERNATE TO THE CAST-IN-PLACE ANCHOR SHOWN IN THE SCHEDULE.



1 EXTERIOR METAL STUD WALL SCHEDULES AND DETAILS
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EXTERIOR WALL SCHEDULES AND DETAILS

S3.0

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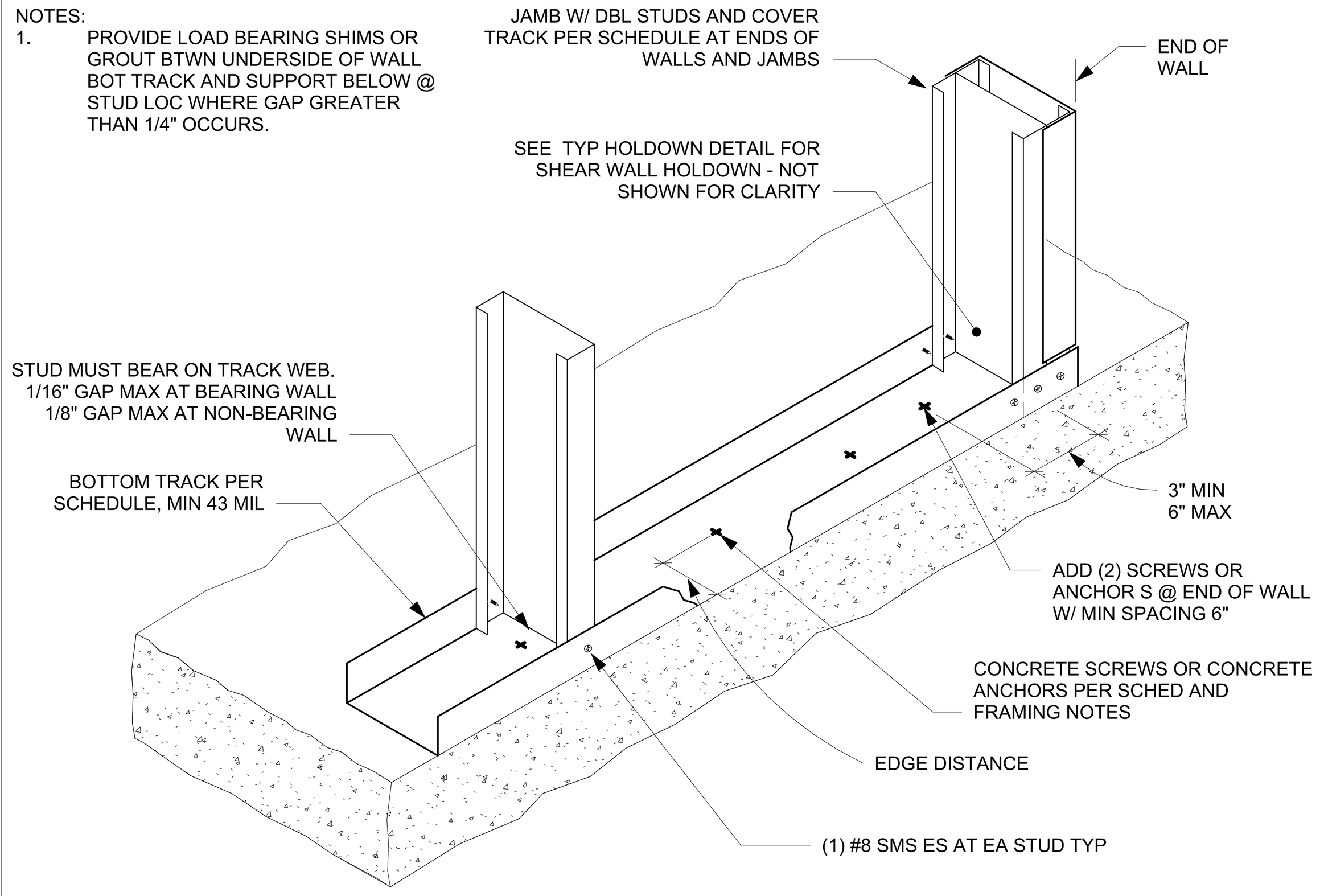
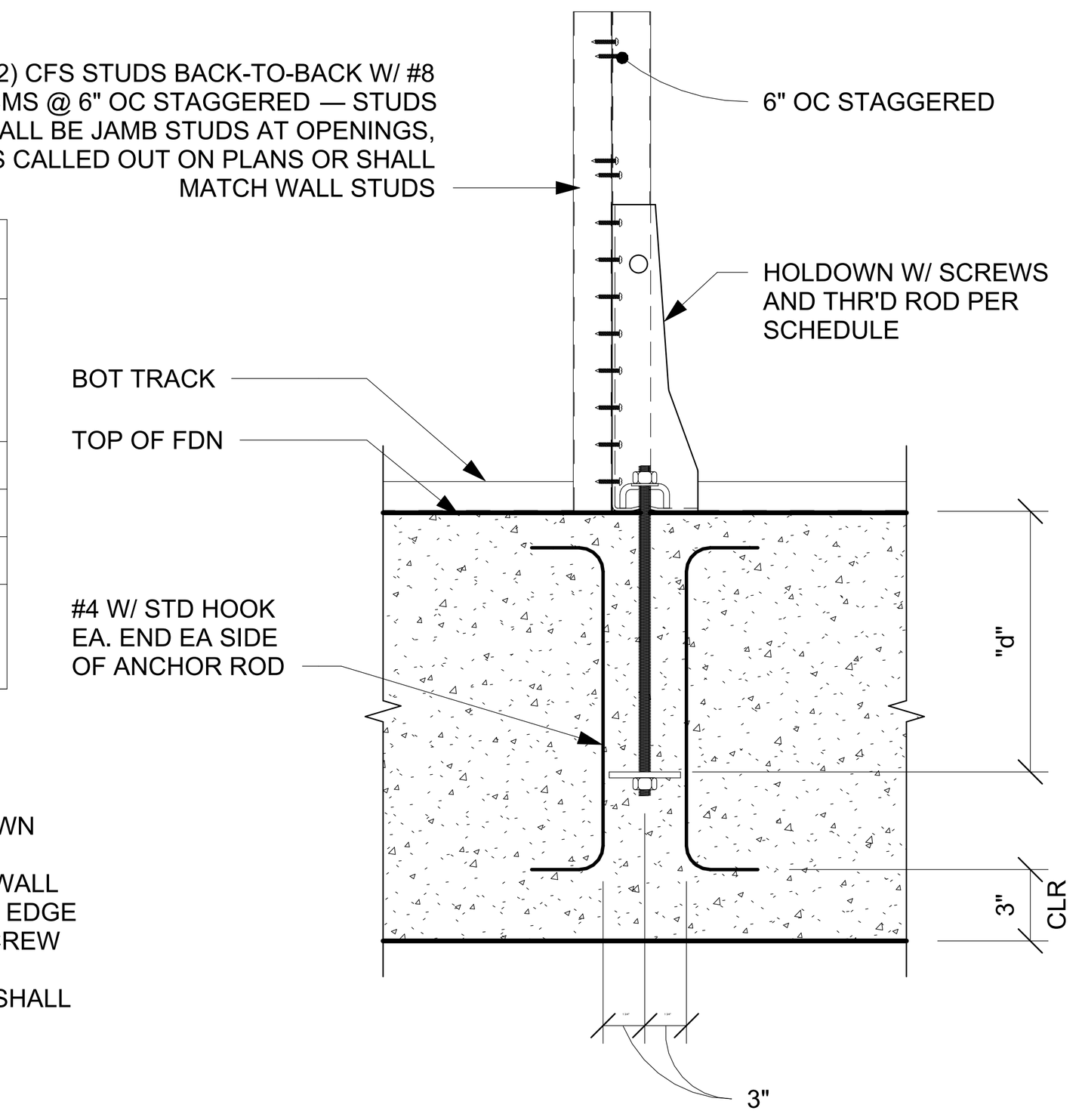
NOTES:
1. PROVIDE LOAD BEARING SHIMS OR GROUT BTWN UNDERSIDE OF WALL BOT TRACK AND SUPPORT BELOW @ STUD LOC WHERE GAP GREATER THAN 1/4" OCCURS.



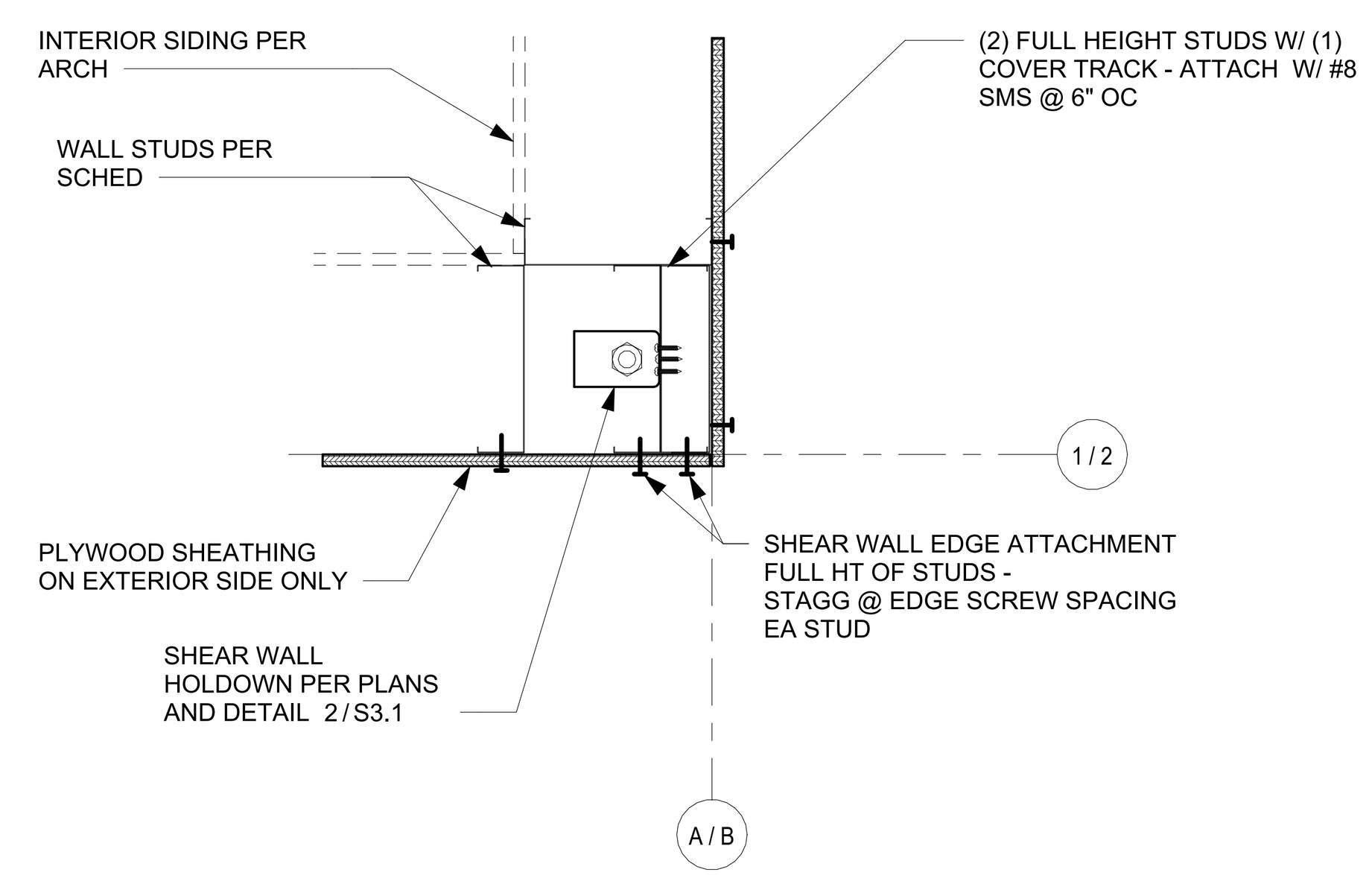
1 TYPICAL BOTTOM TRACK DETAIL UNO
NTS

TYPICAL HOLDOWNS			
SIZE	SCREWS	THREADED ROD	"d"
S/HDU4	(6) #14	5/8"Ø	1'-3"
S/HDU6	(12)#14	5/8"Ø	1'-3"
S/HDU9	(18) #14	7/8"Ø	1'-3"
S/HDU11	(27) #14	7/8"Ø (W/ HEAVY HEX NUT)	1'-3"

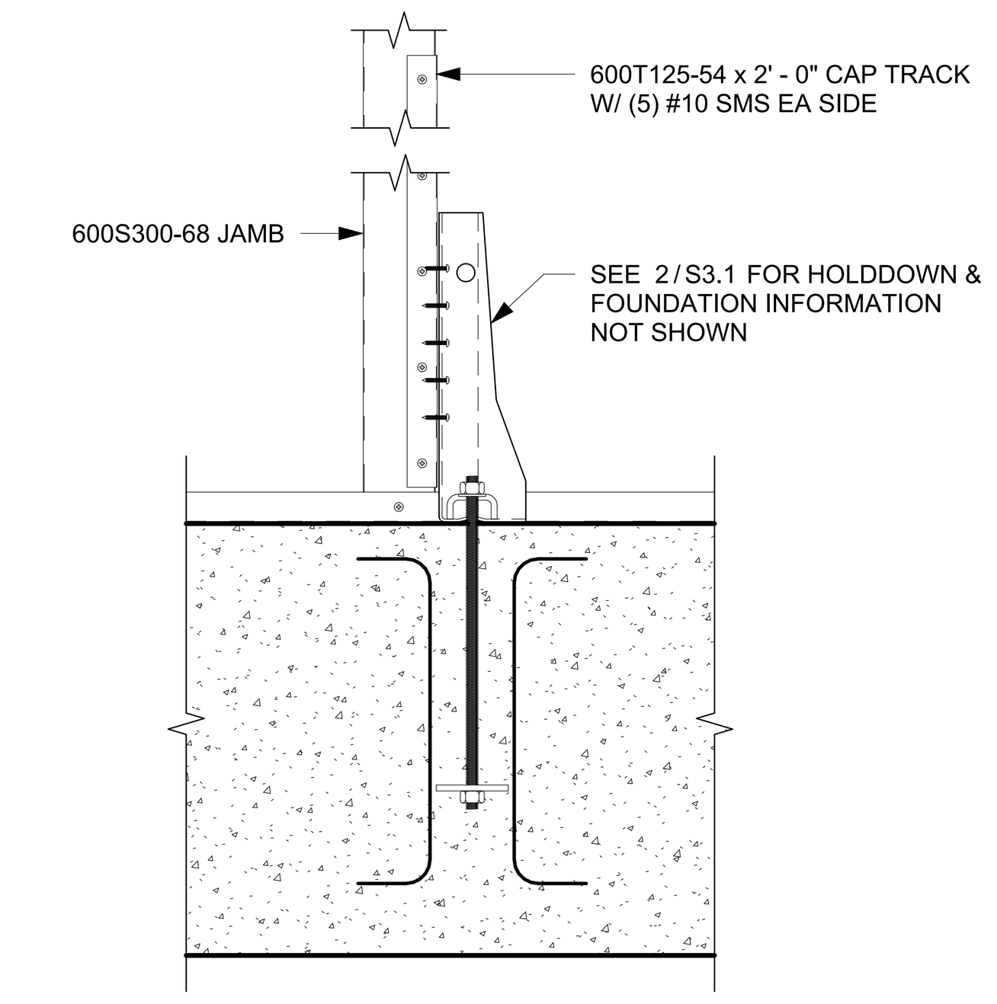
NOTES:
1. SEE FOUNDATION PLAN FOR HOLDOWN LOCATIONS.
2. ALL HOLDOWNS NOT OCCURING AT WALL CORNER SHALL BE LOCATED AT THE EDGE OF A WINDOW OR A DOOR. EDGE SCREW EACH HOLDOWN STUD.
3. ALL HOLDDOWNS AND JAMB STUDS SHALL FULLY BEAR ON BOTTOM TRACK.



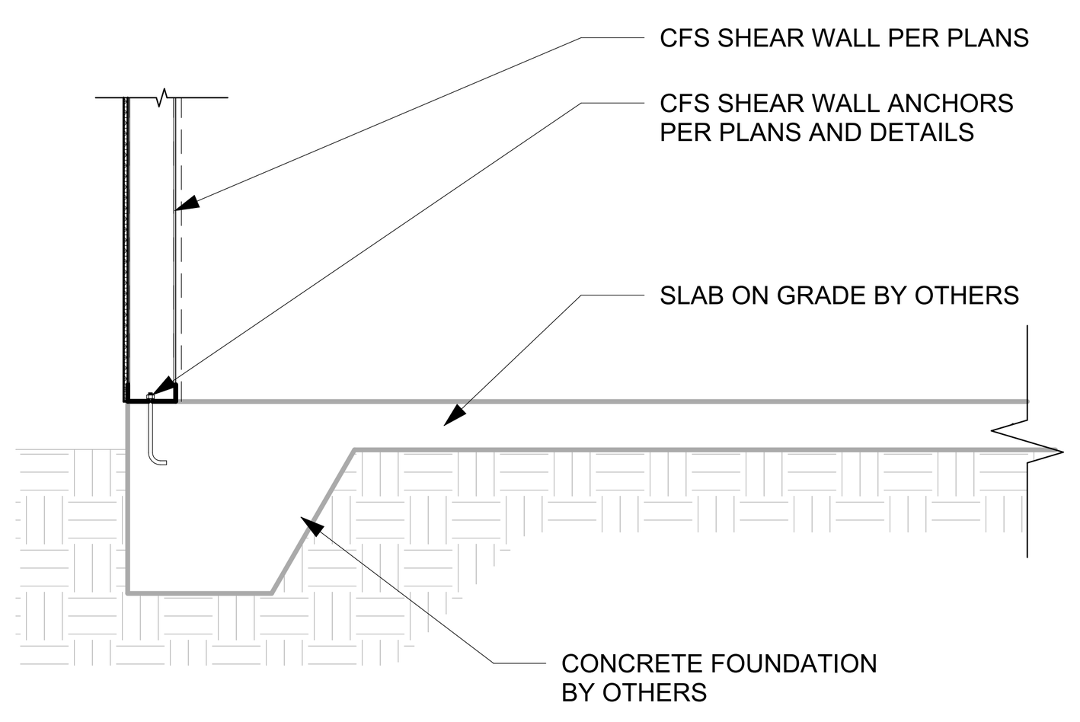
2 TYPICAL HOLDOWN DETAIL
NTS



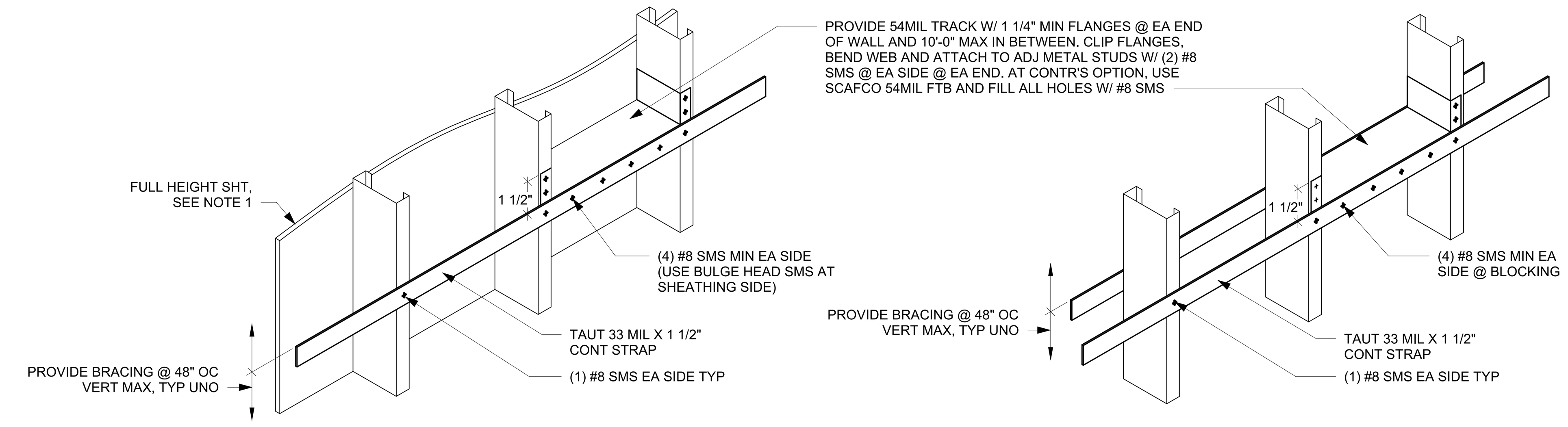
3 TYPICAL HOLDOWN DETAIL @ CORNER
NTS



4 HOLDOWN DETAIL @ GARAGE JAMBS
NTS

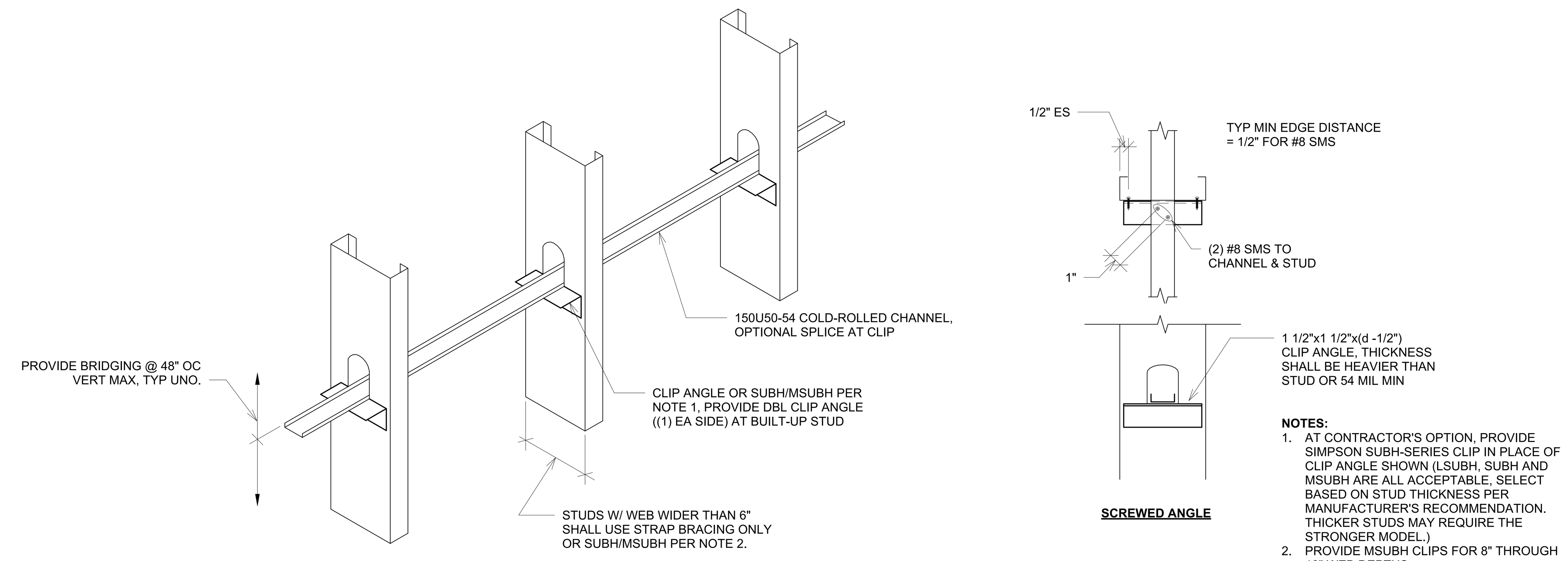


5 THICKENED SLAB EDGE DETAIL
NTS



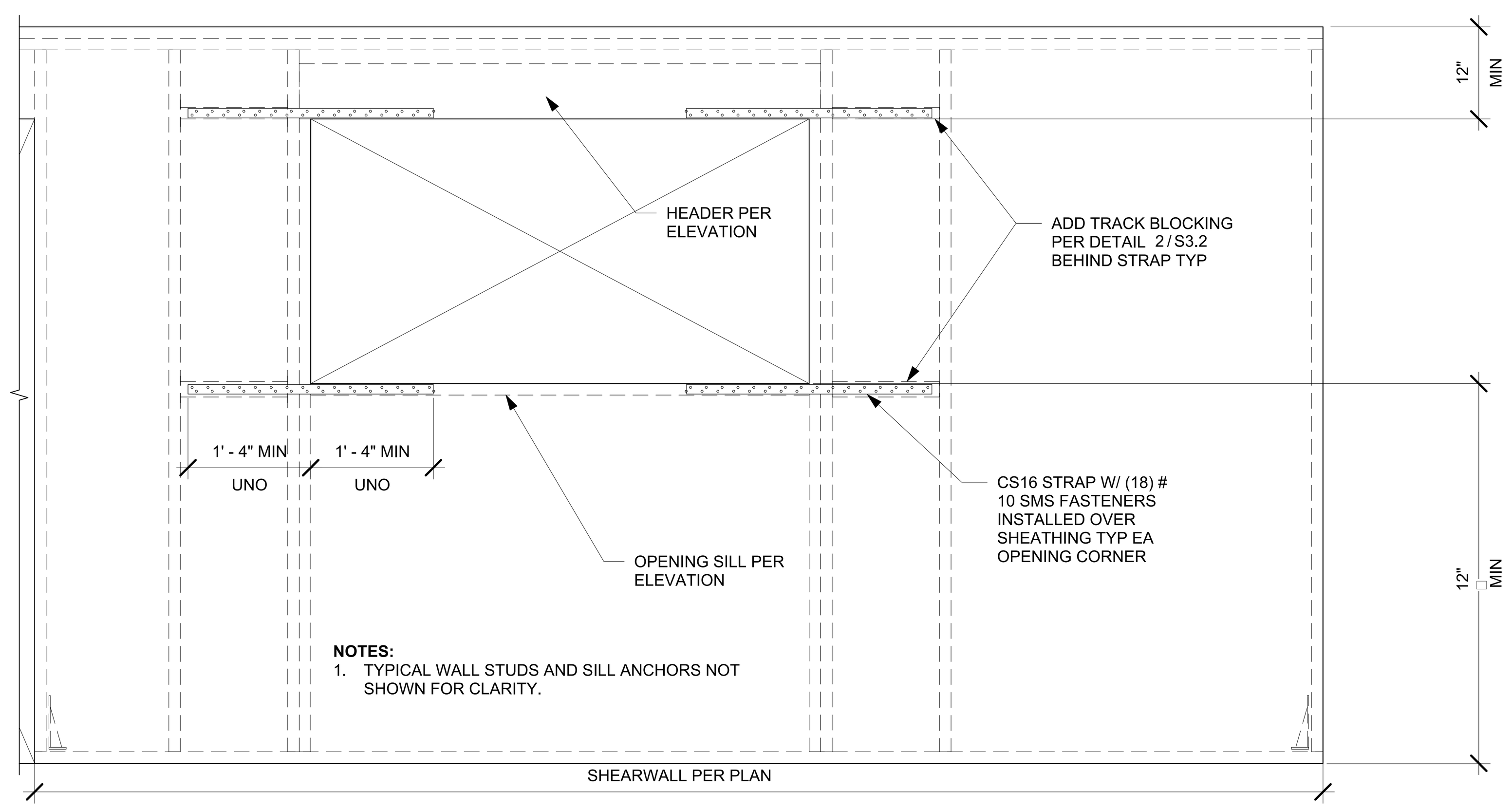
NOTES:
1. QUALIFIED SHEATHING SHALL BE GWB, GYPSHEATHING, PLYWOOD, OR OSB ONLY AND SHALL OCCUR FULL HEIGHT OF WALL. PROVIDE STRAP BRACINGS AT EACH FACE OF WALL IF SHEATHING TERMINATES AT PARTIAL HEIGHT.
2. ALTERNATIVELY, PROVIDE BRIDGING PER TYPICAL DETAILS.

7 TYPICAL STRAP BRACING WHEN PARTITION IS NOT FULLY SHEATHED Copy 1
NTS



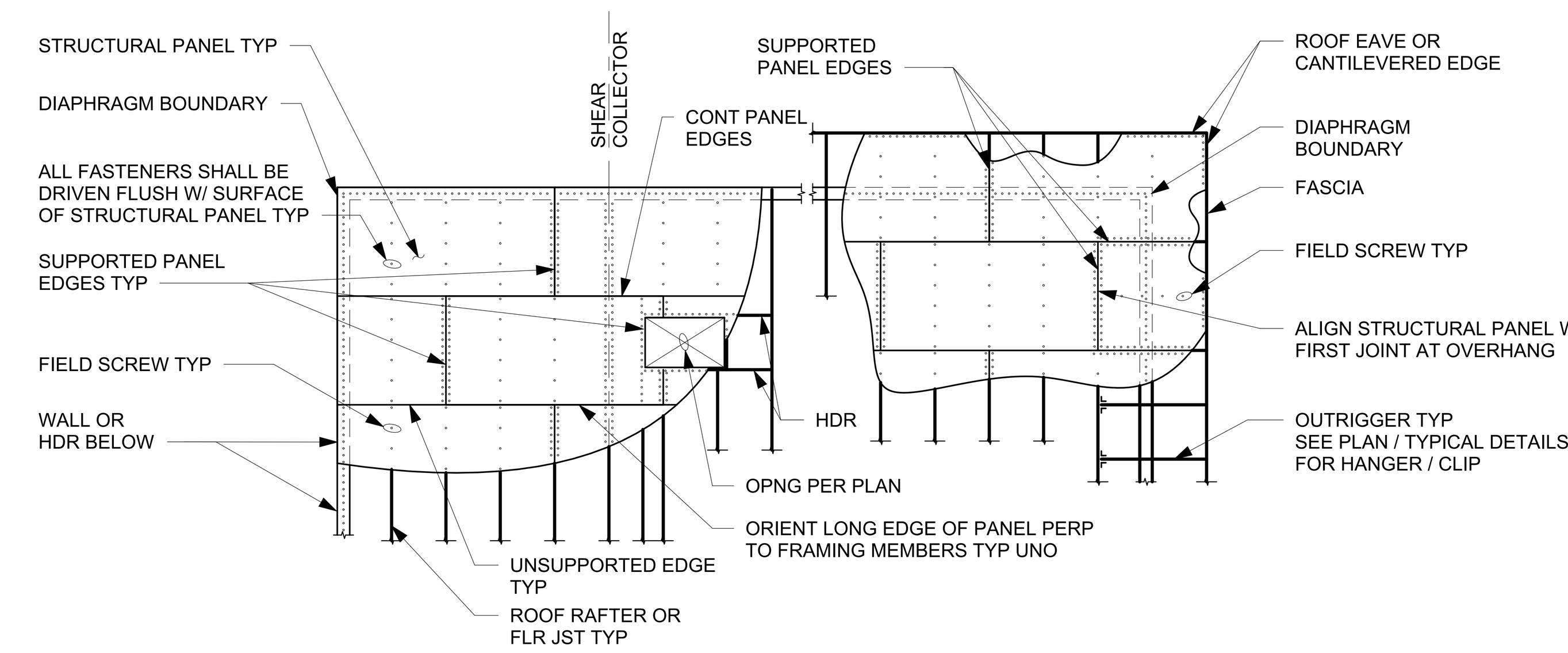
NOTES:
1. AT CONTRACTOR'S OPTION, PROVIDE SIMPSON SUBH-SERIES CLIP IN PLACE OF CLIP ANGLE SHOWN (LSUBH, SUBH AND MSUBH ARE ALL ACCEPTABLE, SELECT BASED ON STUD THICKNESS PER MANUFACTURER'S RECOMMENDATION. THICKER STUDS MAY REQUIRE THE STRONGER MODEL.)
2. PROVIDE MSUBH CLIPS FOR 8" THROUGH 12" WEB DEPTHS.

11 TYPICAL BRIDGING AT PARTITIONS
NTS



NOTES:
1. TYPICAL WALL STUDS AND SILL ANCHORS NOT SHOWN FOR CLARITY.

13 TYPICAL SHEARWALL DETAIL AROUND OPENING
NTS



NOTES:
1. WHERE "SHEAR COLLECTOR" IS CALLED OUT ON PLAN, PROVIDE (2) ROWS OF SPECIFIED DIAPHRAGM BOUNDARY NAILING INDICATED IN THE NAILING SCHEDULE.

15 TYPICAL ROOF DIAPHRAGM NAILING
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FRAMING DETAILS

S3.1

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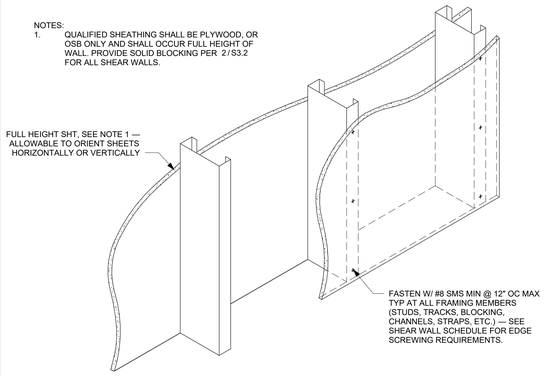
NOTES:
1. QUALIFIED SHEATHING SHALL BE PLYWOOD, OR OSB ONLY AND SHALL OCCUR FULL HEIGHT OF WALL. PROVIDE SOLID BLOCKING PER 2/S3.2 FOR ALL SHEAR WALLS.



1 TYPICAL SHEATHINGS AT SHEAR WALLS
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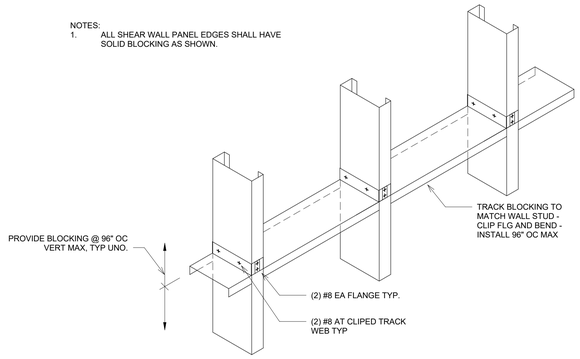
NOTES:
1. ALL SHEAR WALL PANEL EDGES SHALL HAVE SOLID BLOCKING AS SHOWN.



2 TYPICAL BLOCKING AT PARTITIONS
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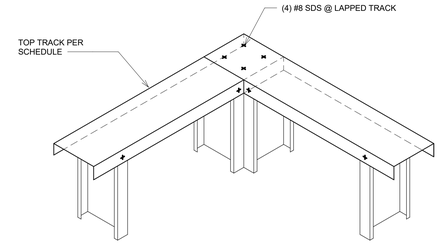
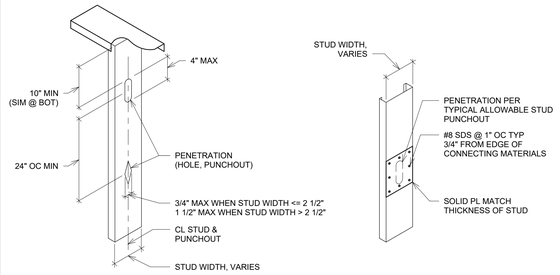
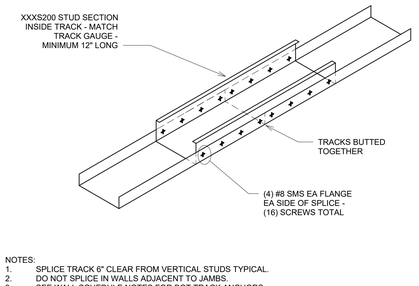




5 TYPICAL METAL STUD PUNCHOUT
NTS



NOTES:
1. SPLICE TRACK 6" CLEAR FROM VERTICAL STUDS TYPICAL.
2. DO NOT SPLICE IN WALLS ADJACENT TO JAMBS.
3. SEE WALL SCHEDULE NOTES FOR BOT TRACK ANCHORS.

6 TYPICAL TRACK SPLICE
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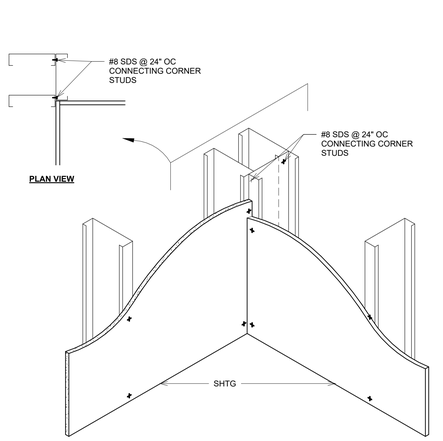

7 TYPICAL STUD WALL AT CORNER
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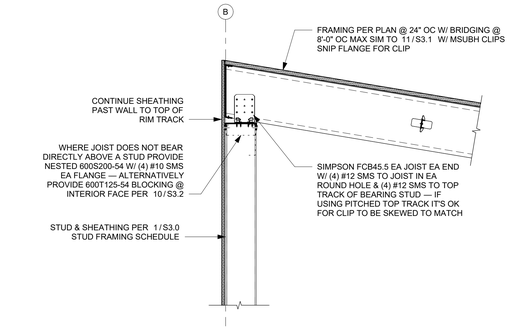

9 TYP ROOF JOIST CONNECTION
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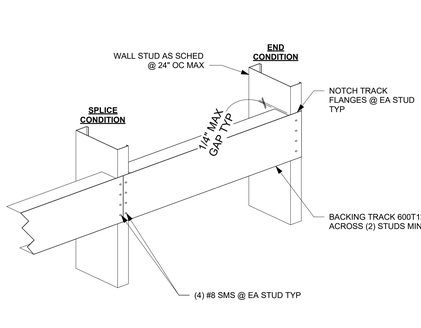
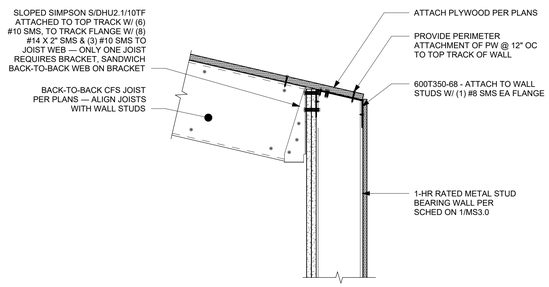


10 54MIL TRACK BACKING
NTS



13 JOIST PERP TO PERIMETER BRG WALL
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