2.	SPECIFICATIONS, THE 2021 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC).	17.	FOUNDATION VALUES LIST THEREFORE N NOTIFY THE S
	ROOF SNOW LOAD25 PSFFLOOR LIVE LOADI25 PSF OR 2,000 LBSSTAIR AND EXITS LIVE LOADI00 PSFGUARDRAILS/BALCONY RAILS (EXIT FACILITY)50 PLF OR 200 LBS.GUARDRAILS/BALCONY RAILS (OTHER THAN EXIT FACILITY)20 PLF OR 200 LBS.MECHANICAL UNITSWEIGHTS FURNISHED BY MANUFACTURERVEHICLE BARRIERS6000 LBS.		FOOTINGS SH OR BOTH) AT SHOWN ON PL OF FOOTINGS FOOTINGS SH BACKFILL BE
	<u>WIND</u> : ANALYSIS PROCEDURE: ASCE 7-16 CHAPTER 27 "PART I - BUILDINGS OF ALL HEIGHTS" RISK CATEGORY II		SUBSURFACE
	98 MPH EXPOSURE "B" TOPOGRAPHIC FACTOR Kzt = 1.0		REPORT: ALLOWABLE
	EARTHQUAKE : ANALYSIS PROCEDURE: IBC "EQUIVALENT LATERAL FORCE PROCEDURE" SEISMIC DESIGN CATEGORY (SDC) = D RISK CATEGORY = II SEISMIC SITE CLASS = D IMPORTANCE FACTOR I = 1.0 MAPPED MCE Ss = 1.26; SI = 0.43 DESIGN ACCELERATION Sds = 1.01; SdI = 0.54		LATERAL EA SEISMIC SURC PASSIVE SOI SOIL COEFFIC SOIL DENSIT
•	LATERAL LOADS ARE TRANSFERRED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR WALLS OR BRACED FRAMES. MOMENTS, SHEARS AND ROTATIONAL FORCES ARE BASED ON THE RIGIDITY OF EACH SHEAR WALL OR BRACED FRAME AND ARE CARRIED BY THE SHEAR WALLS OR BRACED FRAMES TO THE FOUNDATION.	18.	DEMOLITION: SHALL BE INS TO THE WORK PLANS. SAW SAVED. DEM
4.	STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.		STRUCTURE. SYSTEMS TO A. ALL NEW
•	<u>CONTRACTOR</u> SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.		SAW CU ⁻ B. VERIFY C. SMALL F D. WHERE N
<i>.</i>	<u>CONTRACTOR</u> SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.		EXISTING
1.	<u>CONTRACTOR</u> SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THEIR WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.	19.	<u>CONCRETE</u> S CONSTRUCTIC 28-DAY STR SLABS-ON-G
b.	<u>CONTRACTOR-INITIATED</u> CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.		SECTION 190 CEMENT PER (BEFORE THE FOOTINGS A
i.	DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. WHERE INFORMATION ON THE DRAWINGS IS IN CONFLICT WITH THE SPECIFICATIONS, THE MORE STRINGENT SHALL APPLY, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. DO NOT SCALE THE DRAWINGS.		FOR FOOTING THE MINIMUM PERFORMAN APPROVAL THE PERFOR COARSE AG
	ALL STRUCTURAL SYSTEMS WHICH ARE COMPOSED OF FIELD ERECTED COMPONENTS SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.		CONCRETE Y ADMIXTURES PERCENTAGE PERFORMAN THE ATTENTI
Ι.	SHOP DRAWINGS FOR REINFORCING STEEL (FOR BOTH CONCRETE AND MASONRY CONSTRUCTION), STRUCTURAL STEEL, AND METAL DECKING, SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.		ONLY THAT I CONTRACTO
	CONTRACTOR SHALL SUBMIT WALL ELEVATION DRAWINGS OF AT LEAST 1/8" = 1'-0" SCALE INDICATING LOCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS FOR REVIEW PRIOR TO CONSTRUCTION. CONTRACTOR SHALL COORDINATE WITH REINFORCEMENT SHOP DRAWINGS.		AIR-ENTRAIN CONCRETE S CONCRETE T
	SHOP DRAWING REVIEW: DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.		REINFORCINO DETAILED (II CONTINUOUS BARS AT AL DIAMETERS
3.	SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.	21.	318, CLASS I SLABS EXTE CONCRETE F FOOTINGS A FORMED SUF
4.	DEFERRED SUBMITTALS OF DESIGN BUILD COMPONENTS SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE AND SHALL INCLUDE DESIGN CALCULATIONS	22.	(#6 BARS OF (#5 BARS OF COLUMN TIES SLABS AND <u>CAST-IN-PLA</u>
	WITH THE ENGINEER'S STAMP. THE FOLLOWING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT: STAIRS, RAILINGS, AND METAL BUILDINGS		OF DOOR AN AND LOCATH ARCHITECTUM TEXTURE, AN
5.	MECHANICAL UNIT CONNECTIONS TO THE BUILDING SHALL BE DESIGNED BY THE MANUFACTURER FOR THE DESIGN CRITERIA AND CONDITIONS SHOWN ON THE STRUCTURAL DRAWINGS. MANUFACTURER SHALL SUBMIT DETAIL DRAWINGS AND CALCULATIONS, BOTH OF WHICH BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER. MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR DESIGN, CODE CONFORMANCE, AND CONNECTION OF THE UNIT TO THE BASIC STRUCTURE. ALL NECESSARY BRACING, TIES, ANCHORAGE, DISTRIBUTION MEMBERS, AND SIMILAR ELEMENTS SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH SUBMITTED DRAWINGS AND		AND PRECA <u>NON-SHRINK</u> APPROVED MANUFACTUR THE MATERIA <u>POLYSTYRE</u>
5.	ELEMENTS SHALL DE FURNISHED AND INSTALLED IN CONFORMANCE WITH SUDMITTED DRAWINGS AND CALCULATIONS. <u>SPECIAL INSPECTION</u> : CONCRETE CONSTRUCTION, MASONRY CONSTRUCTION, STRUCTURAL STEEL FABRICATION AND ERECTION (INCLUDING FIELD WELDING AND HIGH-STRENGTH FIELD BOLTING), METAL DECK INSTALLATION, EXPANSION BOLTS AND THREADED EXPANSION INSERTS, SCREW ANCHORS, AND EPOXY GROUTED INSTALLATIONS SHALL BE SUPERVISED IN ACCORDANCE WITH IBC SECTIONS 1704 & 1705 AND THE PROJECT SPECIFICATIONS BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE OWNER. THE TESTING AGENCY AND INSPECTOR SHALL BE REGISTERED WITH WABO AND SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE OWNER, ARCHITECT,		SHALL BE R MINIMUM CON RESISTANCE BLOCK JOIN RECOMMEND

GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

<u>GEOTECHNICAL</u>

TES: ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE BASED ON ON PREVIOUSLY PERMITTED BUILDING PLANS DATED DECEMBER 22, 1995 AND ST BE VERIFIED IN THE FIELD. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, RUCTURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

L BEAR ON FIRM, UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL EAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS NS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS IUST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD. UNLESS OTHERWISE NOTED, L BE CENTERED UNDER COLUMNS OR WALLS ABOVE.

ND ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR RAINAGE.

L DESIGN IS BASED ON THE FOLLOWING VALUES FROM THE REFERENCED GEOTECHNICAL

BEARING PRESSURE	2,500 PSF
PRESSURE (RESTRAINED/UNRESTRAINED)	55 PCF/35 PCF
GE PRESSURE (RESTRAINED/UNRESTRAINED)	8H PSF/5H PSF
ESSURE	350 PCF
OF FRICTION	0.35
	120 PCF

RENOVATION

ERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING LLED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE EQUENCES. EXISTING REINFORCING SHALL BE SAVED WHERE AND AS NOTED ON THE JTTING, IF AND WHERE USED, SHALL NOT CUT EXISTING REINFORCING THAT IS TO BE ITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING MIT CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR PSF.

PENINGS THROUGH EXISTING WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY NG WHEREVER POSSIBLE.

L EXISTING CONDITIONS AND LOCATION OF MEMBERS PRIOR TO CUTTING ANY OPENINGS. JND OPENINGS SHALL BE ACCOMPLISHED BY CORE DRILLING, IF POSSIBLE REINFORCING TERMINATES AT EXISTING CONCRETE, REBAR DOWELS EPOXIED INTO THE CONCRETE SHALL BE PROVIDED TO MATCH HORIZONTAL REINFORCING, UNLESS NOTED ON PLANS.

<u>CONCRETE</u>

L BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301. TOLERANCES SHALL NOT EXCEED THOSE LISTED IN ACI 117. CONCRETE SHALL ATTAIN A GTH OF F'C = 3,000 PSI. ALL CONCRETE EXPOSED TO THE WEATHER AND ALL GARAGE DE SHALL ATTAIN A 28-DAY STRENGTH F'C OF 3,500 PSI IN ACCORDANCE WITH IBC AND ACI 318 TABLE 19.3.2.1. MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF IBIC YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS DDITION OF ADMIXTURES). THE WATER/CEMENT RATIO SHALL NOT EXCEED 0.55 FOR 0.45 FOR ALL SLABS AND EXPOSED CONCRETE UNLESS OTHERWISE NOTED. EXCEPT AND SLAB ON GRADE, AGGREGATE SIZE SHALL NOT EXCEED 3/4".

MOUNT OF CEMENT AND THE MAXIMUM SLUMP MAY BE CHANGED IF A CONCRETE MIX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT FOR D WEEKS PRIOR TO PLACING ANY CONCRETE. (THE W/C RATIO LIMITS STILL APPLY). NCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND GATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, _D AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. CHEMICAL ND FLY ASH SHALL CONFORM TO ASTM C494 AND C618 RESPECTIVELY. FLY ASH F TOTAL CEMENTITIOUS MATERIAL SHALL NOT EXCEED 20%. THE USE OF A MIX REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE BROUGHT TO OF THE OWNER. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES DRMATION PRESENTED CONFORMS GENERALLY TO CONTRACT DOCUMENTS. AINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AGENT CONFORMING TO ASTM C260. TOTAL AIR CONTENT FOR FROST-RESISTANT LL BE IN ACCORDANCE WITH ACI 318 TABLE 19.3.2.1 AND IBC SECTION 1904. ALL RECEIVE A STEEL TROWELED FINISH SHALL NOT BE AIR-ENTRAINED.

FEEL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT SI), AND SHALL BE UDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 AND 318. LAP ALL INFORCEMENT #5 AND SMALLER 60 BAR DIAMETERS, 2'-0" MINIMUM. PROVIDE CORNER IALL AND FOOTING INTERSECTIONS. LAP CORNER BARS #5 AND SMALLER 60 BAR 2'-O" MINIMUM. LAPS OF LARGER BARS SHALL BE MADE IN ACCORDANCE WITH ACI PROVIDE (2) #5 MIN. U.N.O. TRIM BARS AROUND ALL OPENINGS IN CONCRETE WALLS OR NG 2'-6" PAST CORNERS, TYPICAL.

TECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

THER UNFORMED SURFACES CAST AGAINST EARTH	3"
ES EXPOSED TO EARTH (i.e. WALLS BELOW GROUND) OR WEATHER RGER) ALLER)	2" - /2"
SPIRALS AND BEAM STIRRUPS	- /2"

LLS (INTERIOR FACE) GREATER OF (BAR DIAMETER PLUS 1/8") OR 3/4"

CONCRETE: SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS NINDOW OPENINGS IN ALL CONCRETE WALLS. SEE MECHANICAL DRAWINGS FOR SIZE OF MISCELLANEOUS MECHANICAL OPENINGS THROUGH CONCRETE WALLS. SEE . DRAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, THER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES, BOTH CAST-IN-PLACE

OUT SHALL BE NON-METALLIC CONFORMING TO ASTM CILOT AND BE FURNISHED BY AN IUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE 'S PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO ON WHICH IT IS PLACED (5000 PSI MINIMUM).

RIGID INSULATION) LIGHTWEIGHT STRUCTURAL FILL PLACED BELOW CONCRETE SLABS CELLULAR POLYSTYRENE CONFORMING TO ASTM D6817 OR ASTM C578, WITH A ESSIVE RESISTANCE OF 5 PSI @ 1% DEFORMATION AND A MINIMUM COMPRESSIVE 15 PSI @ 10 % DEFORMATION, U.O.N. MAXIMUM DENSITY SHALL BE 2.0 PCF. OFFSET BETWEEN ADJACENT LAYERS AND ATTACH BLOCKS PER THE MANUFACTURER'S

25. EXPANSION BOLTS INTO CONCRETE SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-2302 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

<u>ANCHORAGE</u>

- 26. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-1385 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 27. EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3037 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR JAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 28. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH IAPMO UES REPORT NO. ER-240 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 29. SCREW ANCHORS INTO CONCRETE SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3027 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 30. SCREW ANCHORS INTO GROUT FILLED CMU SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3056 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 31. DRIVE PINS, SHOT PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE LOW VELOCITY TYPE FASTENERS AS MANUFACTURED BY HILTI CORPORATION. WHEN CALLED FOR IN THE DRAWINGS, PROVIDE THE APPROPRIATE FASTENER AS NOTED IN THE TABLE BELOW FOR EACH GIVEN APPLICATION. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORTS NO. ESR-2269 FOR THE X-U FASTENERS AND ESR-2379 FOR THE X-CP FASTENERS. MINIMUM EMBEDMENT IN CONCRETE SHALL BE I" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3" TO NEAREST CONCRETE EDGE AND 4" CENTER TO CENTER SPACING. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES.

ALLOWABLE APPLICATION	ALLOWABLE <u>FASTENER TYPE</u>	SHEAR CAF
2X TREATED LUMBER TO CONCRETE (2000 PSI MIN.)	X-CP 72 P8 523 w/ 1.33" EMBED	250
LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN. TO CONCRETE (2000 PSI MIN.)	X-U 27 P8 SI5	190
	X-U 52 MX PLUS R-23 WASHERS	250
LIGHT GAUGE STEEL 43 & 33 MILS (18 & 20 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	445
LIGHT GAUGE STEEL 97, 68 \$ 54 MILS (12, 14 \$ 16 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	720
LIGHT GAUGE STEEL (ALL GA.) TO STRUCTURAL STEEL (3/4" AND GREATER)	X-U 19 P8 TH	350
2X TREATED LUMBER TO GROUTED CMU	X-CP 72 P8 523	105
LIGHT GAUGE STEEL	X-11 32 P8 515	220

LIGHT GAUGE STEEL	X-U 32 P8 515	220
33 MILS (20 GA.) MIN.		
TO GROUTED CMU		

- 32. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO CONCRETE SHALL BE INSTALLED USING "HIT-RE 500 V3" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3814, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.
- 33. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO GROUT FILLED CMU SHALL BE INSTALLED USING "HIT HY 270" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-4143, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR JAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

MASONRY

34. CONCRETE MASONRY UNIT WALLS SHALL BE CONSTRUCTED OF MEDIUM WEIGHT UNITS CONFORMING TO ASTM C90, LAID IN A RUNNING BOND. CONTACT ENGINEER FOR RE-DESIGN OF REINFORCING WHERE STACK BOND LAYOUT IS REQUIRED. LINEAR SHRINKAGE SHALL NOT EXCEED 0.065%. MORTAR SHALL BE TYPE "S" IN ACCORDANCE WITH ASTM C270. GROUT SHALL CONFORM TO IBC REQUIREMENTS AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS, DESIGN F'M = 2,000 PSI. FULL STRESSES ARE REQUIRED. STRENGTH SHALL BE VERIFIED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH TMS 602-16.

PROVIDE (2) #5 ((2)#4 @ 6" AND 4" WALLS) VERT. AT EACH SIDE OF OPENINGS, AT WALL CORNERS AND INTERSECTIONS AND AT FREE ENDS OF WALLS AND (2) #4 HORIZ. AT ELEVATED FLOOR AND ROOF LEVELS, AT TOPS OF WALLS AND ABOVE AND BELOW ALL OPENINGS. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAMS. EXTEND REINFORCEMENT AROUND OPENINGS 2'-O" BEYOND FACE OF OPENING. IF 2'-O" IS UNAVAILABLE, EXTEND AS FAR AS POSSIBLE AND HOOK. PROVIDE CORNER BARS TO LAP HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS. UNLESS NOTED OTHERWISE, LAP ALL REINFORCING STEEL IN CMU 48 BAR DIAMETERS, 2'-O" MINIMUM.

ALL CELLS ARE TO BE SOLID GROUTED UNLESS NOTED AS PARTIAL GROUTING. FOR PARTIAL GROUTING FILL ALL CELLS CONTAINING REINFORCEMENT OR EMBEDDED ITEMS AND ALL CELLS BELOW GRADE WITH GROUT. ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING AND SHALL BE HELD AT TOP, BOTTOM AND 192 BAR DIAMETERS (MAX.) O.C. PER TMS 602 SPECIFICATION 3.5, GROUT POURS SHALL NOT EXCEED 5'-4" IN HEIGHT UNLESS A TEST PANEL IS CONSTRUCTED BY THE MASON AND APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE CLEANOUT HOLES AT BOTTOM OF ALL CELLS CONTAINING REINFORCEMENT FOR ALL GROUT POURS OVER 5'-4" IN HEIGHT. PROVIDE 1 1/2 IN. GROUT KEYS BETWEEN EACH POUR.

APACITY (LBS) TENSION CAPACITY (LBS)

175		
165		
175		
360		
535		
375		
100		
225		

35. STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE BASED ON THE LATEST EDITIONS OF THE A.I.S.C. SPECIFICATIONS AND CODES:

A. AISC - STEEL CONSTRUCTION MANUAL, 15TH EDITION

B. AISC 303-16 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. C. 2014 RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS.

36. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

TYPE OF MEMBER

		-
A. WIDE FLANGE (W AND WT) SHAPES	A992	50 KSI
B. ALL OTHER SHAPES	A36	36 KSI
C. PLATE	A36 OR A572	36 KSI (MIN)
D. PIPE SECTIONS	A53 (TYPE E OR S, GRADE B)	35 KSI
E. STRUCTURAL TUBING (SQUARE OR RECTANGULAR)	A500 (GRADE C)	50 KSI
F. ANCHOR BOLTS AND THREADED RODS	FI554 (GRADE 36) OR	36 KSI
(EMBEDDED IN MASONRY OR CONCRETE)	F1554 (GRADE 55, SUPP. SI)	55 KSI
G. CONNECTION BOLTS	F3125 GRADE A325-N	92 KSI
(7/8" ROUND, UNLESS SHOWN OTHERWISE)		
H. HEADED SHEAR STUDS	A29	49 KSI
I. THREADED RODS	A36	36 KSI
J. STAINLESS STEEL	AISI 316L	30 KSI

ASTM SPECIFICATION

Fy

SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE SHALL NOT BE ALLOWED WITHOUT PRIOR APPROVAL OF THE ENGINEERALL STEEL TO BE FIREPROOFED SHALL BE LEFT UNPAINTED. ALL OTHER STEEL SHALL HAVE ONE COAT OF APPROVED SHOP PAINT.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER OR EARTH SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A123. GALVANIZE BOLTS AND SIMILAR THREADED FASTENERS EXPOSED TO WEATHER OR EARTH IN ACCORDANCE WITH ASTM A153. ALL FIELD WELDS EXPOSED TO WEATHER OR EARTH SHALL BE COATED WITH BRUSH APPLIED ZINC RICH PAINT COMPLYING WITH ASTM A780 (Z.R.C. OR EQUIVALENT).

A MINIMUM OF TWO BOLTS ARE REQUIRED FOR ALL CONNECTIONS. ALTERNATE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ENGINEER.

ALL MEMBERS ARE TO BE ERECTED WITH THE NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM CAMBER ON THE DRAWINGS IS THE UPWARD CAMBER REQUIRED IN THE BEAM AS DELIVERED TO THE JOBSITE. CONTRACTOR TO CONSIDER CAMBER LOSS, IF ANY, DUE TO SHIPPING AND HANDLING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES AND UNEQUAL PARTS

37. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL SHALL CONFORM TO SECTION IO OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES

38. ALL A325 CONNECTION BOLTS SHALL BE INSTALLED TO THE SNUG-TIGHT CONDITION PER RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. ALL NUTS SHALL CONFORM TO ASTM A563. ALL WASHERS SHALL CONFORM TO ASTM F436 OR ASTM F959 TYPE 325. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS OTHERWISE NOTED.

39. ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.S.C. AND A.W.S. STANDARDS AND SHALL BE PERFORMED BY W.A.B.O. CERTIFIED WELDERS USING ETO XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.W.S.) SHALL BE USED. ALL WELDING OF STAINLESS STEEL SHALL USE E309 ELECTRODES WITH A GMAW PROCESS. ALL WELDING SHALL BE PERFORMED BY WELDERS WITH AWS / W.A.B.O. CERTIFICATION WITH THE MATERIAL AND METHOD REQUIRED.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. WELDS SHOWN ON DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON PLATE THICKNESS. MINIMUM WELDING SHALL BE 3/16-INCH. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD ARROWS ARE SHOWN WHERE A FIELD WELD IS REQUIRED BY THE STRUCTURAL DESIGN; THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL DELIVERY AND ERECTION. SEE THE SPECIFICATIONS AND DRAWINGS FOR ADDITIONAL WELDING REQUIREMENTS, ESPECIALLY AT SPECIAL MOMENT RESISTING FRAMES AND OTHER SEISMIC CRITICAL WELDS.

- 40. WELDING OF LATERAL FORCE RESISTING MEMBERS SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS DI.I (INCLUDING AWS DI.8 SEISMIC SUPPLEMENT) AND APPROVED BY THE STRUCTURAL ENGINEER BEFORE WORK BEGINS. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER METAL MANUFACTURER. WELDING ELECTRODES SHALL BE ETOTT-K2 OR ETOT-6 WITH A MINIMUM SPECIFIED CHARPY V-NOTCH (CVN) OF 20 ft-lbs AT -20 DEGREES FAHRENHEIT AND 40 ft-lbs AT 70 DEGREES FAHRENHEIT. REMOVE BOTTOM FLANGE WELD TAB AT MOMENT FRAME CONNECTIONS AND REINFORCE WITH 5/16" FILLET WELD IN CONFORMANCE WITH FEMA-353 RECOMMENDATIONS. WELD ACCESS HOLE DETAILING AT MOMENT FRAME CONNECTIONS SHALL CONFORM WITH FEMA-350 AND FEMA-353 RECOMMENDATIONS.
- 41. METAL FLOOR AND ROOF DECKING PROVIDE SIZE, TYPE, GAUGE, AND ATTACHMENT TO THE SUPPORTING STRUCTURE AS SHOWN ON THE PLANS. ALTERNATES MUST BE CONNECTED ACCORDING TO PUBLISHED I.C.C. OR IAPMO UES CRITERIA FOR DIAPHRAGM SHEARS SHOWN. PROVIDE SHORING WHERE REQUIRED PER MANUFACTURER'S PUBLISHED CRITERIA. ALL DECKING SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.
- 42. HEADED STUDS FOR COMPOSITE CONNECTION OF STRUCTURAL STEEL TO CAST-IN-PLACE CONCRETE SHALL BE MANUFACTURED FROM MATERIAL CONFORMING TO ASTM A29 AND SHALL BE WELDED IN CONFORMANCE WITH A.W.S. REQUIREMENTS.
- 43. DEFORMED BAR ANCHORS (DBA) SHALL BE TYPE D2L ANCHORS BY NELSON STUD WELDING DIVISION, TRW ASSEMBLIES AND FASTENERS GROUP (OR EQUIVALENT). ANCHORS SHALL BE MADE FROM COLD ROLLED, DEFORMED STEEL CONFORMING TO ASTM A1064.

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection.

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

Separate Electrical Permit is required with the Washington State Department of Labor & Industries. https://lni.wa.gov/licensing-permits/electrical/ electrical-permits-fees-and-inspections or call for Licensing Information:

1-800-647-0982

City of Puyallup Building **REVIEWED** FOR COMPLIANCE SKinnear 12/24/2024 11:47:52 AM OFPUYA

City of P Development & Pe ISSUED	
Building	Planning
Engineering	Public Works
Fire	Traffic



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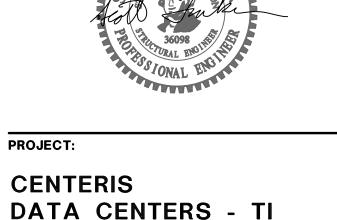
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1023 39TH AVENUE SOUTHEAST **PUYALLUP, WASHINGTON**





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APPROVAL:



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SEATTLE, WA 98101

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SUITE 323

	A COLD-FORMED STEEL FRAMING MEMBERS SHALL BE OF THE SHAPE, SIZE, AND GAUGE SHOWN OF THE PLANS. ALL FRAMING MEMBERS SHALL COMPLY WITH I.C.C. REPORT NO. ESR-3064P. NOTATIONS ON THE DRAWINGS, RELATING TO MEMBER TYPES AND SIZES OR MISCELLANEOUS FRAMING ITEMS, REFER TO CATALOG NUMBERS OF MEMBERS MANUFACTURED BY THE STEEL STI MANUFACTURERS ASSOCIATION (SSMA). PRODUCTS BY OTHER MANUFACTURERS MAY BE SUBSTITUTED FOR FRAMING SHOWN, PROVIDED THEY ARE EQUIVALENT IN SHAPE, SIZE, STIFFNES AND STRENGTH. ALTERNATE FRAMING SHALL BE SUBJECT TO REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION. ALL COLD-FORMED STEEL FRAMING SHALL CONFORM TO THE A.I.S.I. "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURA
F	MEMBERS." 3. MATERIAL:
_	METAL FRAMING SHALL BE GALVANIZED UNLESS OTHERWISE NOTED, CONFORMING AS FOLLOWS
	ASTM A653 SS GRADE 50, CLASS I OR 3 Fy = 50 KSI 118, 97, 68, AND 54 MIL ASTM A653 SS GRADE 33 Fy = 33 KSI 43 AND 33 MIL
	WHERE NOTED, PAINTED STUDS SHALL CONFORM TO: ASTM AIOII SS GRADE 50
C	2. <u>WELDING</u> OF COLD-FORMED METAL FRAMING SHALL CONFORM TO AWS DI.3 AND SHALL BE PERFORMED BY WELDERS QUALIFIED TO PRODUCE THE SPECIFIED CLASSES OF WELD.
Γ	2. <u>WALL FRAMING</u> : ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 4005/62-43 16" O.C. AT INTERIOR WALLS AND 6005/62-43 AT 16" O.C. AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENING TWO 8005/62-54 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS UNLESS OTHERWISE NOT JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS. SOLID BLOCKING FOR MULTI-STU OR STEEL COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW. PROVIDE CONTINUOUS FULL WIDTH BLOCKING AT 1/3 POINTS OF ALL STUD WALLS UNLESS NOTED OTHERWIS MAXIMUM GAP BETWEEN STUD AND TRACK AT ANY POINT SHALL NOT EXCEED 1/16-INCH. NO SPLICES ARE PERMITTED IN STUDS.
	ALL STUD WALLS SHALL HAVE THEIR BOTTOM TRACKS ATTACHED TO FRAMING BELOW WITH #10 SCREWS AT 16" O.C. OR ATTACHED TO CONCRETE WITH 5/32" DIAMETER DRIVE-PINS @ 16" O.C. UNLESS INDICATED OTHERWISE. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE WELDED O SCREWED TO EACH OTHER IN ACCORDANCE WITH THE DETAILS. REFER TO THE PLANS AND SHE WALL SCHEDULE FOR REQUIRED SHEATHING AND STRAP BRACING. WHEN NOT OTHERWISE NOTED PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES AND GYPSUM SHEATHING ON EXTERIOR SURFACES SCREWED TO ALL STUDS, TOP AND BOTTOM TRACKS, AND BLOCKING WITH SCREWS , 12" O.C. ALL SCREWS SHALL BE "GRABBER" TYPE FASTENERS COMPLYING WITH I.C.C. REPORT ESR-1271 ALL SPECIFIED PNEUMATIC FASTENERS SHALL BE ET&F, COMPLYING WITH I.C.C. REPORT NO. ESR-1777 ALL BEARING STUDS SHALL BE LATERALLY SUPPORTED TO PREVENT WEAK AXIS BUCKLING WITH CENTER U-CHANNEL AT 1/3 POINTS AS SHOWN IN THE DETAILS AND CONNECTING EACH FLANGE T
	GYPSUM WALLBOARD PER IBC SECTION 2508.1. TRACK SECTIONS SHALL BE UNPUNCHED AND HAVE AT LEAST I" FLANGES AND MATCH STUD
	THICKNESS. WALLS WHICH HAVE SHEATHING CONNECTED ON ONE SIDE ONLY SHALL HAVE UNSHEATHED
	FLANGES LATERALLY SUPPORTED IN ACCORDANCE WITH THE DETAILS.
	AL BUILDING SYSTEM (MBS)
<i>,</i>	SHOWN ON THESE DRAWINGS.
E	3. ALL COLUMNS SHALL BE DESIGNED ASSUMING THEY ARE FREE TO ROTATE, DO NOT FIX COLUMN BASES.
C	
-	C. DESIGN AND PROVIDE SUPPORTS AROUND OVERHEAD DOORS. A MINIMUM COLLATERAL LOADI OF 10 PSF SHALL BE INCLORED IN THE DESIGN OF THE ROOF TO ACCOUNT FOR MISCELLANEOUS DEAD LOAD. THE LATERAL DIVET OF THE BUILDING SHALL BE LIMITED TO 0.02H, WHERE H IS THE HEIGHT OF THE BUILDING. ALL ROOF MEMBERS SHALL BE LIMITED TO A MAXIMUM TOTAL DEFLECTION OF L/180.
	OF 10 PSF SHALL BE INCLORED IN THE DESIGN OF THE ROOF TO ACCOUNT FOR MISCELLANEOUS DEAD LOAD. THE LATERAL DRIET OF THE BUILDING SHALL BE LIMITED TO 0.02H, WHERE H IS THE HEIGHT OF THE BUILDING. ALL ROOF MEMBERS SHALL BE LIMITED TO A MAXIMUM TOTAL
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GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

ABBREVIA	TIONS	
At	L	Angle
(Nails)	LB.	Pound
ameter egrees	LL LLH	Live Load
Pounds		Long Leg Horizontal Long Leg Vertical
Number	LONGIT.	Longitudinal
Above	LT. WT.	Lightweight
pr Bolt	MAX.	Maximum
ditional	MECH.	Mechanical
ernate	MEZZ.	Mezzanine
ximate chitect	MF MFR.	Moment Frame Manufacturer
Design	MIN.	Minimum
	MISC.	Miscellaneous
Belon tom of	MK.	Mark
Frame	(N)	New
locking	N.	North
Building Beam	N.S. NOM.	Near Side Nominal
Bottom	NTS	Not to Scale
bearing	00	On Cartan
etween	0.C. 0.D.	On Center Outside Diameter
terline	0.F.	Outside Face
amber	O.H. OPNG.	Overhang
Place I Joint	OPP.	Opening Opposite
tration		
Ceiling	PAF	Powder Actuated Fastener
Clear nry Unit	PC PERM.	Precast Permanent
Column	PERP.	Perpendicular
ncrete		Partial Joint Penetration
ections ruction	PL or PL PLF	Plate Pounds per linear Foot
tinuous	PLYND	Plywood
cersink	PREFAB.	Prefabricated
Anchor	PSF PSI	Pounds per Square Foot Pounds per Square Inch
Double	P.T. or PT	Post-Tensioning
Pegree	P/T	Pressure-Treated
-Larch ameter	RAD.	Radius
agonal	REF.	Reference
phragm	REINF.	Reinforce or Reinforcement
nension Down	REQD. REV.	Required Revise
Ditto	R.O.	Rough Opening
Detail	c	Couth
Plate Trawing	S. SCH. or SCHEI	South D. Schedule
	SECT.	Section
ixisting	SHT.	Sheet
East Each	SIM. SOG	Similar Slab On Grade
n Face	SPEC.	Specification
vation	SQ.	Square
evator Lenqth	SQ. FT. SQ. IN.	Square Feet Square Inch(es)
gineer	SPF	JUUI & 1101(85/
Equal	66	
ch May	5.5.	Spruce-Pine-Fir Stainless Steel
ancion	STD.	Spruce-Pine-Fir Stainless Steel Standard
oansion kterior		Spruce-Pine-Fir Stainless Steel
kterior	STD. STIFF. STL. STR.	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural
kterior Indation	STD. STIFF. STL. STR. SUB.	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute
kterior	STD. STIFF. STL. STR.	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural
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kterior Idation Finish Floor olymer	STD. STIFF. STL. STR. SUB. SYM.	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of
kterior Finish Floor olymer ar Side r Feet Tooting	STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G T&G TEMP. THRU	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through
kterior Hation Finish Floor olymer olymer Side Feet Gauge	STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C.	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete
kterior Finish Floor olymer ar Side r Feet Tooting	STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G T&G TEMP. THRU	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through
kterior Hation Finish Floor Olymer Olymer Side Footing Gauge Gauge	STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.M. TRANS.	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall Transverse
kterior hdation Finish Floor olymer ar Side r Feet ooting Gauge anized hinated Board	STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TRANS. TS	Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall Transverse Tube Steel
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@	At	
d	Penny (Nails)	
Ф °	Diameter Degrees	
# #	Pounds	
π	Number	
(A)	Above	
A.B. ADD'L	Anchor Bolt Additional	
ALT.	Alternate	
APPRO)		
ARCH. A.S.D.	Architect Allowable Stress Design	
(B) B/	Below Bottom of	
BF	Braced Frame	
BLKG.	Blocking	
BLDG. BM.	Building Beam	
BOT.	Bottom	
BRG.	Bearing	
BTWN.	Between	
CL or q		
C CIP	Camber Cast In Place	
C.J.	Construction Joint or Control Joint	
CJP	Complete Joint Penetration	
CLG. CLR.	Ceiling Clear	
CMU	Concrete Masonry Unit	
COL.	Column	
CONC. CONN.	Concrete Connections	
CONST.	Construction	
CONT. CSK.	Continuous Countersink	
001.	Oburter Sink	
DBA	Deformed Bar Anchor	
DBL. DEG.	Double Degree	
DF	Doug Fir-Larch	
DIA. DIAG.	Diameter	
DIAO. DIAPH.	Diagonal Diaphragm	
DIM.	Dimension	
DN. DO	Down Ditto	
DTL.	Detail	
DTP	Double Top Plate	
DWG.	Drawing	
(E)	Existing	
E. EA.	East Each	
E.F.	Each Face	
EL. ELEV.	Elevation Elevator	
EMBED.	Embedment Length	
ENGR.	Engineer	
EQ. E.W.	Equal Each Way	
EXP.	Expansion	
EXT.	Exterior	
FDN.	Foundation	
FIN.	Finish	
FLR. FRP	Floor Fiber Reinforced Polymer	
F.S.	Far Šide	
FT. FTG.	Foot or Feet Footing	
	-	
GA. GALV.	Gauge Galvanized	
GL GL	Glue Laminated	
GMB	Gypsum Wall Board	
HDG	Hot Dipped Galvanized	
HDR.	Header	
HF HGR.	Hem Fir Hanger	
HORIZ.	Horizontal	
HSS ∺⊤	Hollow Structural Section	
HT.	Height	
l.D.	Inside Diameter	
I.F. IN.	Inside Face Inch	
INFO.	Information	
INT.	Interior	
JT.	Joint	
К	Kips	
KSF	Kips per Square Foot	
KSI	Kips per Square Inch	

City of Puyallup Development & Permitting Services ISSUED PERMIT				
Building	Planning			
Engineering	Public Works			
Fire	Traffic			



SHEET NO.

GENERAL STRUCTURAL NOTES

	SOUTH	YARD ANCHOR	RAGE	10/23/24	
NO.		DESCRIPTION		DATE	В
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P.M .			SHT		
P.E.			TVM		
DRAWN BY:			SC		
SCALE:			AS SHOWN		
DATE:			10/23/24		
DAT					



1023 39TH AVENUE SOUTHEAST PUYALLUP, WASHINGTON APPROVAL:

DATA CENTERS - TI

PROJECT:

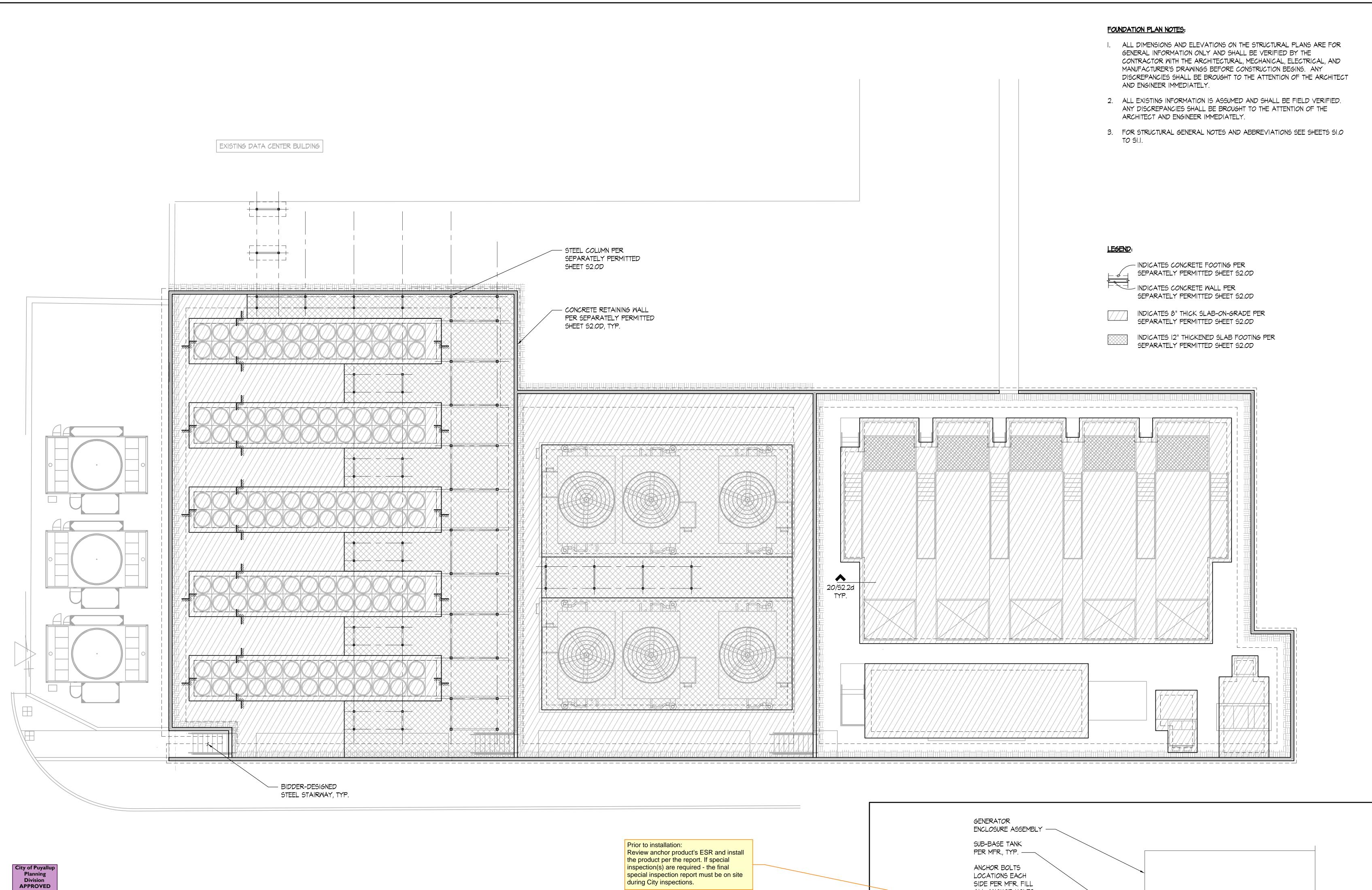
centeris

Centers for Information Systems

SEAL:

CENTERIS





See permit

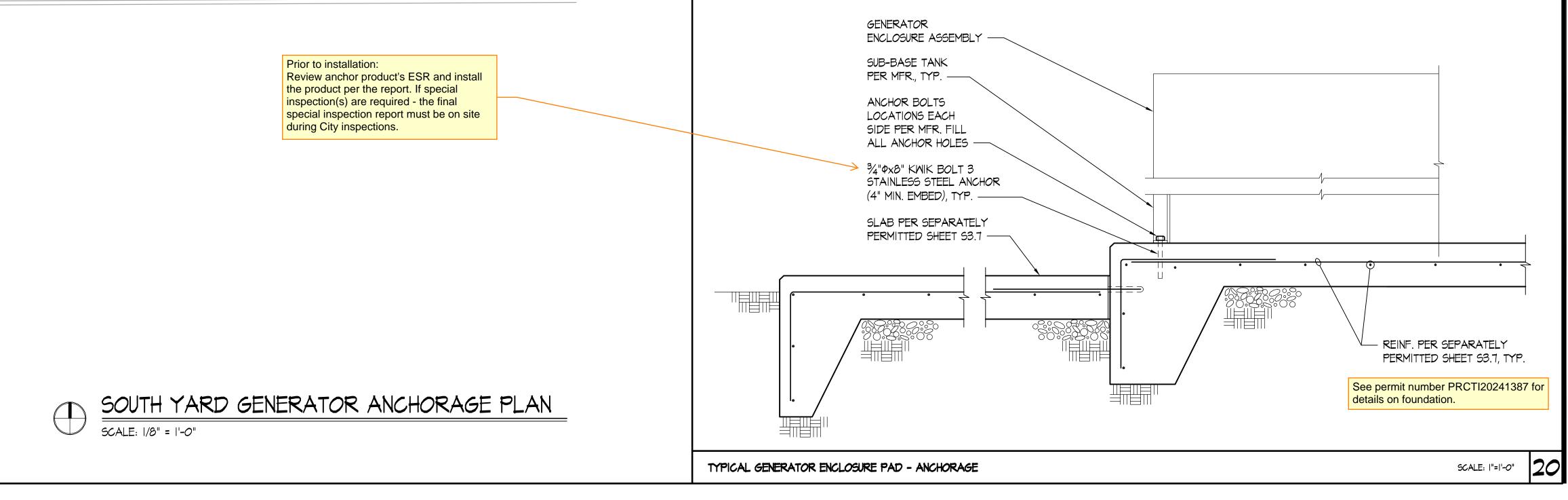
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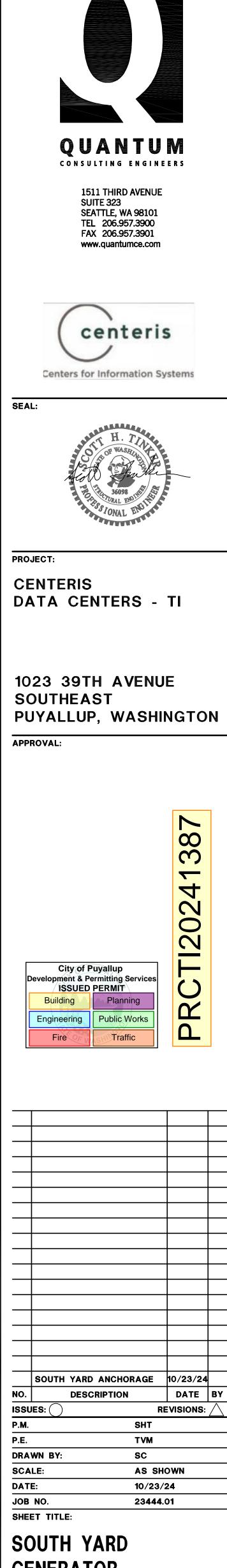
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OF PUYAL

C WASHINGS





GENERATOR ANCHORAGE PLAN

SHEET NO.

S2.2D