	CRITERIA	
١.	ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS,	
2.	SPECIFICATIONS, THE 2021 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC).	17. <u>FOUNDATION NOTES</u> : ALLOWABLE SOIL PRESSU VALUES LISTED ON ON PREVIOUSLY PERMITTED THEREFORE MUST BE VERIFIED IN THE FIELD. IF
	ROOF SNOW LOAD 25 PSF	NOTIFY THE STRUCTURAL ENGINEER FOR POSSIB
	FLOOR LIVE LOAD125 PSF OR 2,000 LBSSTAIR AND EXITS LIVE LOAD100 PSF	OR BOTH) AT LEAST 18" BELOW LOWEST ADJAC
	GUARDRAILS/BALCONY RAILS (EXIT FACILITY)50 PLF OR 200 LBS.GUARDRAILS/BALCONY RAILS (OTHER THAN EXIT FACILITY)20 PLF OR 200 LBS.	SHOWN ON PLANS (OR IN DETAILS) ARE MINIMUM OF FOOTINGS MUST BE ESTABLISHED BY THE CO
	MECHANICAL UNITS WEIGHTS FURNISHED BY MANUFACTURER VEHICLE BARRIERS 6000 LBS.	FOOTINGS SHALL BE CENTERED UNDER COLUMNS BACKFILL BEHIND ALL RETAINING WALLS WITH F
	<u>MIND</u> : ANALYSIS PROCEDURE: ASCE 7-16 CHAPTER 27 "PART I - BUILDINGS OF ALL HEIGHTS" RISK CATEGORY II	SUBSURFACE DRAINAGE. THE STRUCTURAL DESIGN IS BASED ON THE FOL
	98 MPH EXPOSURE "B"	<u>REPORT:</u> ALLOWABLE SOIL BEARING PRESSURE
	TOPOGRAPHIC FACTOR Kzt = 1.0	LATERAL EARTH PRESSURE (RESTRAINED/UNRES
	EARTHQUAKE : ANALYSIS PROCEDURE: IBC "EQUIVALENT LATERAL FORCE PROCEDURE" SEISMIC DESIGN CATEGORY (SDC) = D RISK CATEGORY =    SEISMIC SITE CLASS = D	SEISMIC SURCHARGE PRESSURE (RESTRAINED/UN PASSIVE SOIL PRESSURE SOIL COEFFICIENT OF FRICTION SOIL DENSITY
	IMPORTANCE FACTOR 1e = 1.0 MAPPED MCE Ss = 1.26; S <sub>1</sub> = 0.43 DESIGN ACCELERATION Sds = 1.01; Sd <sub>1</sub> = 0.54	REN
a	LATERAL LOADS ARE TRANSFERRED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR WALLS	18. DEMOLITION: VERIFY ALL EXISTING CONDITIONS
J.	CATERAL LOADS ARE TRANSFERRED BT THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR MALLS OR BRACED FRAMES. MOMENTS, SHEARS AND ROTATIONAL FORCES ARE BASED ON THE RIGIDITY OF EACH SHEAR WALL OR BRACED FRAME AND ARE CARRIED BY THE SHEAR WALLS OR BRACED FRAMES TO THE FOUNDATION.	SHALL BE INSTALLED TO SUPPORT EXISTING CO TO THE WORK SEQUENCES. EXISTING REINFORCI PLANS. SAW CUTTING, IF AND WHERE USED, SHA SAVED. DEMOLITION DEBRIS SHALL NOT BE AL
4.	STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.	STRUCTURE. LIMIT CONSTRUCTION LOADING (INC SYSTEMS TO 40 PSF. A. ALL NEW OPENINGS THROUGH EXISTING WAL
	<u>CONTRACTOR</u> SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.	SAW CUTTING WHEREVER POSSIBLE. B. VERIFY ALL EXISTING CONDITIONS AND LO C. SMALL ROUND OPENINGS SHALL BE ACCOM D. WHERE NEW REINFORCING TERMINATES AT E
	<u>CONTRACTOR</u> SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.	EXISTING CONCRETE SHALL BE PROVIDED OTHERWISE NOTED ON PLANS.
7.	<u>CONTRACTOR</u> SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THEIR WORK. THE STRUCTURAL ENGINEER HAS NO	<u>00</u>
	OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENCINEER WAS NO DUTY TO INSPECT SUPERVISE NOTE	19. <u>CONCRETE</u> SHALL BE MIXED, PROPORTIONED, CO
	TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.	CONSTRUCTION TOLERANCES SHALL NOT EXCEED 28-DAY STRENGTH OF $f'c = 3,000$ PSI. ALL C
8	CONTRACTOR-INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND	SLABS-ON-GRADE SHALL ATTAIN A 28-DAY ST SECTION 1904.1. AND ACI 318 TABLE 19.3.2.1. MIX
	STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT	CEMENT PER CUBIC YARD AND SHALL BE PROP (BEFORE THE ADDITION OF ADMIXTURES). THE W FOOTINGS AND 0.45 FOR ALL SLABS AND EXPO FOR FOOTINGS AND SLAB ON GRADE, AGGREGA
-1.	SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE	THE MINIMUM AMOUNT OF CEMENT AND THE MAXI
	STRUCTURAL ENGINEER. WHERE INFORMATION ON THE DRAWINGS IS IN CONFLICT WITH THE SPECIFICATIONS, THE MORE STRINGENT SHALL APPLY, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. DO NOT SCALE THE DRAWINGS.	PERFORMANCE MIX IS SUBMITTED TO THE STRUC APPROVAL TWO WEEKS PRIOR TO PLACING ANY THE PERFORMANCE MIX SHALL INCLUDE THE AM COARSE AGGREGATE, WATER AND ADMIXTURES
10.	ALL STRUCTURAL SYSTEMS WHICH ARE COMPOSED OF FIELD ERECTED COMPONENTS SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.	CONCRETE YIELD AND SUBSTANTIATING STRENG ADMIXTURES AND FLY ASH SHALL CONFORM TO PERCENTAGE OF TOTAL CEMENTITIOUS MATERIA PERFORMANCE MIX REQUIRES BATCH PLANT INS
11.	SHOP DRAWINGS FOR REINFORCING STEEL (FOR BOTH CONCRETE AND MASONRY CONSTRUCTION), STRUCTURAL STEEL, AND METAL DECKING, SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.	THE ATTENTION OF THE OWNER. REVIEW OF MIX ONLY THAT INFORMATION PRESENTED CONFORM CONTRACTOR MAINTAINS FULL RESPONSIBILITY
	CONTRACTOR SHALL SUBMIT WALL ELEVATION DRAWINGS OF AT LEAST 1/8" = 1'-0" SCALE INDICATING LOCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS FOR REVIEW PRIOR TO CONSTRUCTION. CONTRACTOR SHALL COORDINATE WITH REINFORCEMENT SHOP DRAWINGS.	ALL CONCRETE WITH SURFACES EXPOSED TO ST AIR-ENTRAINING AGENT CONFORMING TO ASTM CONCRETE SHALL BE IN ACCORDANCE WITH AC CONCRETE TO RECEIVE A STEEL TROWELED FIN
2.	SHOP DRAWING REVIEW: DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW	20. <u>REINFORCING STEEL</u> SHALL CONFORM TO ASTM DETAILED (INCLUDING HOOKS AND BENDS) IN AC
	DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.	CONTINUOUS REINFORCEMENT #5 AND SMALLER BARS AT ALL WALL AND FOOTING INTERSECTIO DIAMETERS OR 2'-O" MINIMUM. LAPS OF LARGE
13.	<u>SHOP DRAWING SUBMITTALS</u> PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE	318, CLASS B. PROVIDE (2) #5 MIN. U.N.O. TRIM I SLABS EXTENDING 2'-6" PAST CORNERS, TYPICA
	CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION	21. CONCRETE PROTECTION (COVER) FOR REINFORCE
	METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING	FOOTINGS AND OTHER UNFORMED SURFACES CA
	SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.	FORMED SURFACES EXPOSED TO EARTH (i.e. WA (#6 BARS OR LARGER)
14.	<u>DEFERRED SUBMITTALS OF DESIGN BUILD COMPONENTS</u> SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER AND SHALL BE APPROVED BY THE	(#5 BARS OR SMALLER)
	COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON	COLUMN TIES OR SPIRALS AND BEAM STIRRUPS SLABS AND WALLS (INTERIOR FACE)
	ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE AND SHALL INCLUDE DESIGN CALCULATIONS	22. <u>CAST-IN-PLACE CONCRETE</u> : SEE ARCHITECTUR/ OF DOOR AND WINDOW OPENINGS IN ALL CONCR
	WITH THE ENGINEER'S STAMP.	AND LOCATION OF MISCELLANEOUS MECHANICA ARCHITECTURAL DRAWINGS FOR ALL GROOVES
	THE FOLLOWING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT: STAIRS, RAILINGS, AND METAL BUILDINGS	TEXTURE, AND OTHER FINISH DETAILS AT ALL EX AND PRECAST.
15.	MECHANICAL UNIT CONNECTIONS TO THE BUILDING SHALL BE DESIGNED BY THE MANUFACTURER FOR THE DESIGN CRITERIA AND CONDITIONS SHOWN ON THE STRUCTURAL DRAWINGS. MANUFACTURER SHALL SUBMIT DETAIL DRAWINGS AND CALCULATIONS, BOTH OF WHICH BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER. MANUFACTURER'S ENGINEER SHALL BE	23. <u>NON-SHRINK GROUT</u> SHALL BE NON-METALLIC CO APPROVED MANUFACTURER AND SHALL BE MIXE MANUFACTURER'S PUBLISHED RECOMMENDATION THE MATERIAL ON WHICH IT IS PLACED (5000 P
	RESPONSIBLE FOR DESIGN, CODE CONFORMANCE, AND CONNECTION OF THE UNIT TO THE BASIC STRUCTURE. ALL NECESSARY BRACING, TIES, ANCHORAGE, DISTRIBUTION MEMBERS, AND SIMILAR	24. POLYSTYRENE (RIGID INSULATION) LIGHTWEIGHT
	ELEMENTS SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH SUBMITTED DRAWINGS AND CALCULATIONS.	SHALL BE RIGID CELLULAR POLYSTYRENE CONF MINIMUM COMPRESSIVE RESISTANCE OF 5 PSI @
<b> 6</b> .	<u>SPECIAL INSPECTION:</u> CONCRETE CONSTRUCTION, MASONRY CONSTRUCTION, STRUCTURAL STEEL FABRICATION AND ERECTION (INCLUDING FIELD WELDING AND HIGH-STRENGTH FIELD BOLTING), METAL	RESISTANCE OF 15 PSI @ 10 % DEFORMATION, U. BLOCK JOINTS BETWEEN ADJACENT LAYERS AN RECOMMENDATIONS
	DECK INSTALLATION, EXPANSION BOLTS AND THREADED EXPANSION INSERTS, SCREW ANCHORS, AND EPOXY GROUTED INSTALLATIONS SHALL BE SUPERVISED IN ACCORDANCE WITH IBC SECTIONS 1704 &	RECOMMENDATIONS.
	1705 AND THE PROJECT SPECIFICATIONS BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE OWNER. THE TESTING AGENCY AND INSPECTOR SHALL BE REGISTERED WITH WABO AND SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE OWNER, ARCHITECT,	
	STRUCTURAL ENGINEER, CONTRACTOR AND BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET PROJECT SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.	

## GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

### <u>GEOTECHNICAL</u>

TES: ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE BASED ON ON PREVIOUSLY PERMITTED BUILDING PLANS DATED DECEMBER 22, 1995 AND T BE VERIFIED IN THE FIELD. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, UCTURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

\_ BEAR ON FIRM, UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL AST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS IS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS UST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD. UNLESS OTHERWISE NOTED, BE CENTERED UNDER COLUMNS OR WALLS ABOVE.

ND ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR RAINAGE.

. DESIGN IS BASED ON THE FOLLOWING VALUES FROM THE REFERENCED GEOTECHNICAL

BEARING PRESSURE	2,500 PSF
PRESSURE (RESTRAINED/UNRESTRAINED)	55 PCF/35 PCF
GE PRESSURE (RESTRAINED/UNRESTRAINED)	8H PSF/5H PSF
ESSURE	350 PCF
OF FRICTION	0.35
	120 PCF

#### RENOVATION

ERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING LLED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE EQUENCES. EXISTING REINFORCING SHALL BE SAVED WHERE AND AS NOTED ON THE TTING, IF AND WHERE USED, SHALL NOT CUT EXISTING REINFORCING THAT IS TO BE ITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING 11T CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR PSF.

PENINGS THROUGH EXISTING WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY IG WHEREVER POSSIBLE.

L EXISTING CONDITIONS AND LOCATION OF MEMBERS PRIOR TO CUTTING ANY OPENINGS. IND OPENINGS SHALL BE ACCOMPLISHED BY CORE DRILLING, IF POSSIBLE. REINFORCING TERMINATES AT EXISTING CONCRETE, REBAR DOWELS EPOXIED INTO THE ONCRETE SHALL BE PROVIDED TO MATCH HORIZONTAL REINFORCING, UNLESS NOTED ON PLANS.

#### <u>CONCRETE</u>

L BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301. TOLERANCES SHALL NOT EXCEED THOSE LISTED IN ACI 117. CONCRETE SHALL ATTAIN A 5TH OF F'C = 3,000 PSI. ALL CONCRETE EXPOSED TO THE WEATHER AND ALL GARAGE DE SHALL ATTAIN A 28-DAY STRENGTH 1'C OF 3,500 PSI IN ACCORDANCE WITH IBC AND ACI 318 TABLE 19.3.2.1. MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF BIC YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS DDITION OF ADMIXTURES). THE WATER/CEMENT RATIO SHALL NOT EXCEED 0.55 FOR 0.45 FOR ALL SLABS AND EXPOSED CONCRETE UNLESS OTHERWISE NOTED. EXCEPT AND SLAB ON GRADE, AGGREGATE SIZE SHALL NOT EXCEED 3/4".

OUNT OF CEMENT AND THE MAXIMUM SLUMP MAY BE CHANGED IF A CONCRETE MIX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT FOR D WEEKS PRIOR TO PLACING ANY CONCRETE. (THE W/C RATIO LIMITS STILL APPLY). NCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND GATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, D AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. CHEMICAL D FLY ASH SHALL CONFORM TO ASTM C494 AND C618 RESPECTIVELY. FLY ASH TOTAL CEMENTITIOUS MATERIAL SHALL NOT EXCEED 20%. THE USE OF A MIX REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE BROUGHT TO OF THE OWNER. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES PRMATION PRESENTED CONFORMS GENERALLY TO CONTRACT DOCUMENTS. AINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AGENT CONFORMING TO ASTM C260. TOTAL AIR CONTENT FOR FROST-RESISTANT L BE IN ACCORDANCE WITH ACI 318 TABLE 19.3.2.1 AND IBC SECTION 1904. ALL ECEIVE A STEEL TROWELED FINISH SHALL NOT BE AIR-ENTRAINED.

TEEL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT SI), AND SHALL BE JDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 AND 318. LAP ALL NFORCEMENT #5 AND SMALLER 60 BAR DIAMETERS, 2'-0" MINIMUM. PROVIDE CORNER IALL AND FOOTING INTERSECTIONS. LAP CORNER BARS #5 AND SMALLER 60 BAR 2'-O" MINIMUM. LAPS OF LARGER BARS SHALL BE MADE IN ACCORDANCE WITH ACI PROVIDE (2) #5 MIN. U.N.O. TRIM BARS AROUND ALL OPENINGS IN CONCRETE WALLS OR NG 2'-6" PAST CORNERS, TYPICAL.

TECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

THER UNFORMED SURFACES CAST AGAINST EARTH	3"
ES EXPOSED TO EARTH (I.E. WALLS BELOW GROUND) OR WEATHER RGER) ALLER)	2"  - /2"

LS (INTERIOR FACE) GREATER OF (BAR DIAMETER PLUS 1/8") OR 3/4"

CONCRETE: SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS NINDOW OPENINGS IN ALL CONCRETE WALLS. SEE MECHANICAL DRAWINGS FOR SIZE OF MISCELLANEOUS MECHANICAL OPENINGS THROUGH CONCRETE WALLS. SEE . DRAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, THER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES, BOTH CAST-IN-PLACE

|-|/2"

OUT SHALL BE NON-METALLIC CONFORMING TO ASTM CIIOT AND BE FURNISHED BY AN IUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE S PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO ON WHICH IT IS PLACED (5000 PSI MINIMUM).

RIGID INSULATION) LIGHTWEIGHT STRUCTURAL FILL PLACED BELOW CONCRETE SLABS CELLULAR POLYSTYRENE CONFORMING TO ASTM D6817 OR ASTM C578, WITH A ESSIVE RESISTANCE OF 5 PSI @ 1% DEFORMATION AND A MINIMUM COMPRESSIVE 15 PSI @ 10 % DEFORMATION, U.O.N. MAXIMUM DENSITY SHALL BE 2.0 PCF. OFFSET BETWEEN ADJACENT LAYERS AND ATTACH BLOCKS PER THE MANUFACTURER'S

25. EXPANSION BOLTS INTO CONCRETE SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-2302 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

ANCHORAGE

- 26. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "KWIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-1385 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 27. EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3037 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 28. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH JAPMO VES REPORT NO. ER-240 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
- 29. SCREW ANCHORS INTO CONCRETE SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3027 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 30. SCREW ANCHORS INTO GROUT FILLED CMU SHALL BE "KWIK HUS-EZ", AS MANUFACTURED BY HILTI, INC. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3056 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.
- 31. DRIVE PINS, SHOT PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE LOW VELOCITY TYPE FASTENERS AS MANUFACTURED BY HILTI CORPORATION. WHEN CALLED FOR IN THE DRAWINGS, PROVIDE THE APPROPRIATE FASTENER AS NOTED IN THE TABLE BELOW FOR EACH GIVEN APPLICATION. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORTS NO. ESR-2269 FOR THE X-U FASTENERS AND ESR-2379 FOR THE X-CP FASTENERS. MINIMUM EMBEDMENT IN CONCRETE SHALL BE I" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3" TO NEAREST CONCRETE EDGE AND 4" CENTER TO CENTER SPACING. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES.

ALLOWABLE APPLICATION	ALLOWABLE FASTENER TYPE	SHEAR CAF
2X TREATED LUMBER TO CONCRETE (2000 PSI MIN.)	X-CP 72 P8 523 w/ 1.33" EMBED	250
LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN. TO CONCRETE (2000 PSI MIN.)	X-U 27 P8 515	190
	X-U 52 MX PLUS R-23 WASHERS	250
LIGHT GAUGE STEEL 43 & 33 MILS (18 & 20 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	445
LIGHT GAUGE STEEL 97, 68 \$ 54 MILS (12, 14 \$ 16 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 11/16" MAX)	X-U 19 P8 TH	720
LIGHT GAUGE STEEL (ALL GA.) TO STRUCTURAL STEEL (3/4" AND GREATER)	X-U 19 P8 TH	350
2X TREATED LUMBER TO GROUTED CMU	X-CP 72 P8 523	105
LIGHT GAUGE STEEL	X-U 32 P8 515	220

LIGHT GAUGE STEEL	X-U 32 P8 515	220
33 MILS (20 GA.) MIN.		
TO GROUTED CMU		

- 32. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO CONCRETE SHALL BE INSTALLED USING "HIT-RE 500 V3" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3814, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.
- 33. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO GROUT FILLED CMU SHALL BE INSTALLED USING "HIT HY 270" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-4143, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

#### MASONRY

34. CONCRETE MASONRY UNIT WALLS SHALL BE CONSTRUCTED OF MEDIUM WEIGHT UNITS CONFORMING TO ASTM C90, LAID IN A RUNNING BOND. CONTACT ENGINEER FOR RE-DESIGN OF REINFORCING WHERE STACK BOND LAYOUT IS REQUIRED. LINEAR SHRINKAGE SHALL NOT EXCEED 0.065%. MORTAR SHALL BE TYPE "S" IN ACCORDANCE WITH ASTM C270. GROUT SHALL CONFORM TO IBC REQUIREMENTS AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS, DESIGN F'M = 2,000 PSI, FULL STRESSES ARE REQUIRED. STRENGTH SHALL BE VERIFIED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH TMS 602-16.

PROVIDE (2) #5 ((2)#4 @ 6" AND 4" WALLS) VERT. AT EACH SIDE OF OPENINGS, AT WALL CORNERS AND INTERSECTIONS AND AT FREE ENDS OF WALLS AND (2) #4 HORIZ. AT ELEVATED FLOOR AND ROOF LEVELS, AT TOPS OF WALLS AND ABOVE AND BELOW ALL OPENINGS. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAMS. EXTEND REINFORCEMENT AROUND OPENINGS 2'-O" BEYOND FACE OF OPENING. IF 2'-O" IS UNAVAILABLE, EXTEND AS FAR AS POSSIBLE AND HOOK. PROVIDE CORNER BARS TO LAP HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS. UNLESS NOTED OTHERWISE, LAP ALL REINFORCING STEEL IN CMU 48 BAR DIAMETERS, 2'-O" MINIMUM.

ALL CELLS ARE TO BE SOLID GROUTED UNLESS NOTED AS PARTIAL GROUTING. FOR PARTIAL GROUTING FILL ALL CELLS CONTAINING REINFORCEMENT OR EMBEDDED ITEMS AND ALL CELLS BELOW GRADE WITH GROUT. ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING AND SHALL BE HELD AT TOP, BOTTOM AND 192 BAR DIAMETERS (MAX.) O.C. PER TMS 602 SPECIFICATION 3.5, GROUT POURS SHALL NOT EXCEED 5'-4" IN HEIGHT UNLESS A TEST PANEL IS CONSTRUCTED BY THE MASON AND APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE CLEANOUT HOLES AT BOTTOM OF ALL CELLS CONTAINING REINFORCEMENT FOR ALL GROUT POURS OVER 5'-4" IN HEIGHT. PROVIDE | 1/2 IN. GROUT KEYS BETWEEN EACH POUR.

APACITY (LBS) TENSION CAPACITY (LBS)

175	
165	
175	
360	
535	
375	
100	
225	

35. <u>STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION</u> SHALL BE BASED ON THE LATEST EDITIONS OF THE A.I.S.C. SPECIFICATIONS AND CODES:

A. AISC - STEEL CONSTRUCTION MANUAL, 15TH EDITION

B. AISC 303-16 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. C. 2014 RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS.

36. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

#### TYPE OF MEMBER

		-
A. WIDE FLANGE (W AND WT) SHAPES	A992	50 KSI
B. ALL OTHER SHAPES	A36	36 KSI
C. PLATE	A36 OR A572	36 KSI (MIN)
D. PIPE SECTIONS	A53 (TYPE E OR S, GRADE B)	35 KSI
E. STRUCTURAL TUBING (SQUARE OR RECTANGULAR)	A500 (GRADE C)	50 KSI
F. ANCHOR BOLTS AND THREADED RODS	FI554 (GRADE 36) OR	36 KSI
(EMBEDDED IN MASONRY OR CONCRETE)	FI554 (GRADE 55, SUPP. SI)	55 KSI
G. CONNECTION BOLTS	F3125 GRADE A325-N	92 KSI
(7/8" ROUND, UNLESS SHOWN OTHERWISE)		
H. HEADED SHEAR STUDS	A29	49 KSI
I. THREADED RODS	A36	36 KSI
J. STAINLESS STEEL	AISI 316L	30 KSI

ASTM SPECIFICATION

Fy

SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE SHALL NOT BE ALLOWED WITHOUT PRIOR APPROVAL OF THE ENGINEERALL STEEL TO BE FIREPROOFED SHALL BE LEFT UNPAINTED. ALL OTHER STEEL SHALL HAVE ONE COAT OF APPROVED SHOP PAINT.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER OR EARTH SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A123. GALVANIZE BOLTS AND SIMILAR THREADED FASTENERS EXPOSED TO WEATHER OR EARTH IN ACCORDANCE WITH ASTM A153. ALL FIELD WELDS EXPOSED TO WEATHER OR EARTH SHALL BE COATED WITH BRUSH APPLIED ZINC RICH PAINT COMPLYING WITH ASTM A780 (Z.R.C. OR EQUIVALENT).

A MINIMUM OF TWO BOLTS ARE REQUIRED FOR ALL CONNECTIONS. ALTERNATE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ENGINEER.

ALL MEMBERS ARE TO BE ERECTED WITH THE NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM CAMBER ON THE DRAWINGS IS THE UPWARD CAMBER REQUIRED IN THE BEAM AS DELIVERED TO THE JOBSITE. CONTRACTOR TO CONSIDER CAMBER LOSS, IF ANY, DUE TO SHIPPING AND HANDLING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES AND UNEQUAL PARTS

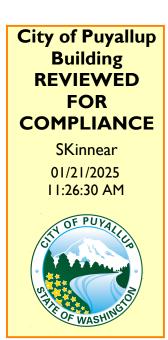
37. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL SHALL CONFORM TO SECTION IO OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES

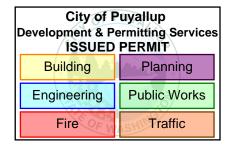
38. ALL A325 CONNECTION BOLTS SHALL BE INSTALLED TO THE SNUG-TIGHT CONDITION PER RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. ALL NUTS SHALL CONFORM TO ASTM A563. ALL WASHERS SHALL CONFORM TO ASTM F436 OR ASTM F959 TYPE 325. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS OTHERWISE NOTED.

39. ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.S.C. AND A.W.S. STANDARDS AND SHALL BE PERFORMED BY W.A.B.O. CERTIFIED WELDERS USING ETO XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.W.S.) SHALL BE USED. ALL WELDING OF STAINLESS STEEL SHALL USE E309 ELECTRODES WITH A GMAW PROCESS. ALL WELDING SHALL BE PERFORMED BY WELDERS WITH AWS / WABO. CERTIFICATION WITH THE MATERIAL AND METHOD REQUIRED.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. WELDS SHOWN ON DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON PLATE THICKNESS. MINIMUM WELDING SHALL BE 3/16-INCH. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD ARROWS ARE SHOWN WHERE A FIELD WELD IS REQUIRED BY THE STRUCTURAL DESIGN; THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL DELIVERY AND ERECTION. SEE THE SPECIFICATIONS AND DRAWINGS FOR ADDITIONAL WELDING REQUIREMENTS, ESPECIALLY AT SPECIAL MOMENT RESISTING FRAMES AND OTHER SEISMIC CRITICAL WELDS.

- 40. WELDING OF LATERAL FORCE RESISTING MEMBERS SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS DI.I (INCLUDING AWS DI.8 SEISMIC SUPPLEMENT) AND APPROVED BY THE STRUCTURAL ENGINEER BEFORE WORK BEGINS. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER METAL MANUFACTURER. WELDING ELECTRODES SHALL BE ETOTT-K2 OR ETOT-6 WITH A MINIMUM SPECIFIED CHARPY V-NOTCH (CVN) OF 20 ft-165 AT -20 DEGREES FAHRENHEIT AND 40 ft-165 AT 70 DEGREES FAHRENHEIT. REMOVE BOTTOM FLANGE WELD TAB AT MOMENT FRAME CONNECTIONS AND REINFORCE WITH 5/16" FILLET WELD IN CONFORMANCE WITH FEMA-353 RECOMMENDATIONS. WELD ACCESS HOLE DETAILING AT MOMENT FRAME CONNECTIONS SHALL CONFORM WITH FEMA-350 AND FEMA-353 RECOMMENDATIONS.
- 41. METAL FLOOR AND ROOF DECKING PROVIDE SIZE, TYPE, GAUGE, AND ATTACHMENT TO THE SUPPORTING STRUCTURE AS SHOWN ON THE PLANS. ALTERNATES MUST BE CONNECTED ACCORDING TO PUBLISHED I.C.C. OR IAPMO UES CRITERIA FOR DIAPHRAGM SHEARS SHOWN. PROVIDE SHORING WHERE REQUIRED PER MANUFACTURER'S PUBLISHED CRITERIA. ALL DECKING SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.
- 42. HEADED STUDS FOR COMPOSITE CONNECTION OF STRUCTURAL STEEL TO CAST-IN-PLACE CONCRETE SHALL BE MANUFACTURED FROM MATERIAL CONFORMING TO ASTM A29 AND SHALL BE WELDED IN CONFORMANCE WITH A.W.S. REQUIREMENTS.
- 43. DEFORMED BAR ANCHORS (DBA) SHALL BE TYPE D2L ANCHORS BY NELSON STUD WELDING DIVISION, TRW ASSEMBLIES AND FASTENERS GROUP (OR EQUIVALENT). ANCHORS SHALL BE MADE FROM COLD ROLLED, DEFORMED STEEL CONFORMING TO ASTM A1064.





Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection.

Separate Electrical Permit is required with the Washington State Department of Labor & Industries. https://lni.wa.gov/licensing-permits/electrical/

electrical-permits-fees-and-inspections or call for Licensing Information: 1-800-647-0982



**NOTES** SHEET NO.

STRUCTURAL

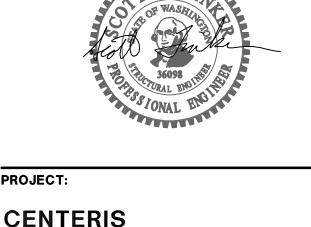
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**1023 39TH AVENUE** SOUTHEAST **PUYALLUP, WASHINGTON** 

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CONSULTING ENGINEERS **1511 THIRD AVENUE** SUITE 323 **SEATTLE, WA 98101** TEL 206.957.3900 FAX 206.957.3901 www.quantumce.com

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SEAL:

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SCREWS AT 16 UNLESS INDICA SCREWED TO E WALL SCHEDUL PROVIDE GYPS SURFACES SCR 12" O.C. ALL S ESR-1271 ALL S ESR-1271 ALL S NO. ESR-1777 ALL BEARING S CENTER U-CHA GYPSUM WALLE TRACK SECTIO THICKNESS. WALLS WHICH H FLANGES LATE METAL BUILDING MANU SHOWN ON THE B. ALL COLUMNS	O.C. OR ATTACHED TO C TED OTHERWISE. INDIVID ACH OTHER IN ACCORDA E FOR REQUIRED SHEATH SUM WALLBOARD ON INTER REWED TO ALL STUDS, TOR CREWS SHALL BE "GRABE SPECIFIED PNEUMATIC FAS STUDS SHALL BE LATERAN NNEL AT 1/3 POINTS AS SH BOARD PER IBC SECTION INS SHALL BE UNPUNCHED HAVE SHEATHING CONNECT RALLY SUPPORTED IN AC	DNCRETE WITH 5/32" DIAMETI AL MEMBERS OF BUILT-UP PO CE WITH THE DETAILS. REFE IG AND STRAP BRACING. WH IOR SURFACES AND GYPSUM AND BOTTOM TRACKS, AND ER" TYPE FASTENERS COMPL TENERS SHALL BE ET&F, CON LY SUPPORTED TO PREVENT DWN IN THE DETAILS AND CO 508.1. AND HAVE AT LEAST I" FLAN	ER DRIVE-PINS @ 16" O.C. DSTS SHALL BE WELDED O ER TO THE PLANS AND SH HEN NOT OTHERWISE NOTE SHEATHING ON EXTERIOR BLOCKING WITH SCREWS LYING WITH I.C.C. REPORT (PLYING WITH I.C.C. REPORT WEAK AXIS BUCKLING WITH I.C.C. REPORT WEAK AXIS BUCKLING WITH DNNECTING EACH FLANGE GES AND MATCH STUD
CENTER U-CHA GYPSUM WALLE TRACK SECTIO THICKNESS. WALLS WHICH H FLANGES LATE METAL BUILDING SY A. BUILDING MANU SHOWN ON THE B. ALL COLUMNS	NNEL AT 1/3 POINTS AS SH BOARD PER IBC SECTION INS SHALL BE UNPUNCHED HAVE SHEATHING CONNECT RALLY SUPPORTED IN AC	DWN IN THE DETAILS AND CO 508.1. AND HAVE AT LEAST I" FLAN ED ON ONE SIDE ONLY SHALI	DNNECTING EACH FLANGE GES AND MATCH STUD
THICKNESS. WALLS WHICH H FLANGES LATE METAL BUILDING SY A. BUILDING MANU SHOWN ON THE B. ALL COLUMNS	AVE SHEATHING CONNECT RALLY SUPPORTED IN ACT	ED ON ONE SIDE ONLY SHALI	L HAVE UNSHEATHED
FLANGES LATE METAL BUILDING SY A. BUILDING MANU SHOWN ON THE B. ALL COLUMNS	RALLY SUPPORTED IN AC		
A. BUILDING MANU SHOWN ON THE B. ALL COLUMNS			
dajej.		BUILDINGS FOR THE LOADS, 41NG THEY ARE FREE TO RO	
OF IO PSF SHA DEAD LOAD. T	LL BE INCLORED IN THE D HE LATERAL DRIFT OF TH BUILDING. ALL ROOF ME	OVERHEAD DOORS. A MIN SIGN OF THE ROOF TO ACCO BUILDING SHALL BE LIMITED BRACESHALL BE LIMITED TO	OUNT FOR MISCELLANEOUS D TO 0.02H, WHERE H IS T
D.COORDINATE A BUILDING DESI		IOWN ON THESE DRAWINGS M	NTH THE PREFABRICATED
STRUCTURAL E		DING AND BASE PLATE CON IGN SHOWN IN THESE PLANS I COLUMN LOCATIONS.	
ENGINEER FOR AND SIGNATUR	REVIEW PRIOR TO FABRI E OF A STATE OF WASHIN	CULATIONS TO THE ARCHITE ATION. DESIGN SUBMITTALS TON REGISTERED PROFESSI FOR CODE CONFORMANCE	SHALL BEAR THE STAMP ONAL ENGINEER. THE
DRAWINGS. SH		OOUT ON THE ARCHITECTURA CATE MAGNITUDE AND DIREC	
	STRUCTU	AL OBSERVATION	
STRUCTURAL OBSER BUT NOT LIMITED TO THE COMPLETED ST SPECIFICATIONS. S	VATION MEANS THE VISU, D, THE ELEMENTS AND CON RUCTURE FOR GENERAL C	OBSERVATION IS REQUIRED OBSERVATION OF THE STR ECTIONS AT SIGNIFICANT CO NFORMANCE TO THE APPRO DOES NOT INCLUDE OR WAIN 110 AND 1704.	UCTURAL SYSTEM, INCLUD DNSTRUCTION STAGES ANI DVED PLANS AND
AS OBSERVE THE S SPECIFICATIONS. S ACCEPTED STANDA INDICATE ACTUAL C	TRUCTURAL SYSTEM FOR UCH REVIEW PROCEDURES RDS OF PRACTICE. THE	ELECT PORTIONS OF WORK T ENERAL CONFORMANCE TO WILL BE CONDUCTED IN ACC UILDING OFFICIAL UNDERSTA HE REVIEW IS PERFORMED A REVIEWED.	THE APPROVED PLANS A ORDANCE WITH COMMONL NDS THAT SUCH PROCEDU
MINIMIZE THE RISK ( DESIGN PROFESSIO CASES, THE CONTRA	OF PROBLEMS ARISING DI NAL DOES NOT CONSTITUT	AT STRUCTURAL REVIEW IS A RING CONSTRUCTION. STRUC WARRANTY OR GUARANTEE PONSIBILITY FOR THE QUALIT PECIFICATIONS.	TURAL OBSERVATION BY OF ANY TYPE. IN ALL

GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

ABBREVIA	TIONS	
At	L	Angle
Nails)	LB.	Pound
neter grees		Live Load
ounds		Long Leg Horizontal Long Leg Vertical
umber	LONGIT.	Longitudinal
I	LT. WT.	Lightweight
bove Bolt	MAX.	Maximum
tional	MECH.	Mechanical
rnate	MEZZ.	Mezzanine
imate	MF	Moment Frame
nitect Pesign	MFR. MIN.	Manufacturer Minimum
	MISC.	Miscellaneous
Below	MK.	Mark
om of Frame	(N)	New
ocking	N.	North
vilding	N.S.	Near Side
Beam	NOM.	Nominal
ottom aring	NTS	Not to Scale
ween	O.C.	On Center
	0.D.	Outside Diameter
erline Imber	0.F. 0.H.	Outside Face Overhang
Place	OPNG.	Opening
Joint	OPP.	Opposite
ration	DAE	Roudon Actuated Eastonen
eiling Clear	PAF PC	Powder Actuated Fastener Precast
ry Unit	PERM.	Permanent
ōlumn	PERP.	Perpendicular
crete ctions	PJP PL or PL	Partial Joint Penetration Plate
uction	PLF	Pounds per linear Foot
inuous	PLYWD	Plywood
ersink	PREFAB.	Prefabricated Rounds non Sougna East
nchor	PSF PSI	Pounds per Square Foot Pounds per Square Inch
ouble	P.T. or PT	Post-Tensioning
egree	P/T	Pressure-Treated
Larch neter	RAD.	Radius
gonal	REF.	Reference
nragm	REINF.	Reinforce or Reinforcement
ension Down	REQD. RE√.	Required Revise
Ditto	R.O.	Rough Opening
Detail		
Detail Plate	S. SCH or SCHEI	South
Detail	S. SCH. or SCHEI SECT.	South
Detail Plate awing Kisting	SCH. or SCHEI SECT. SHT.	South Schedule Section Sheet
Detail Plate awing Kisting East	SCH. or SCHEI SECT. SHT. SIM.	South Schedule Section Sheet Similar
Detail Plate awing Kisting	SCH. or SCHEI SECT. SHT.	South Schedule Section Sheet Similar Slab On Grade
Detail Plate awing Kisting East Each	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ.	South Schedule Section Sheet Similar
Detail Plate awing tisting East Each Face vation vator	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet
Detail Plate awing East Each Face vation vator ength	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es)
Detail Plate awing tisting East Each Face vation vator	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet
Detail Plate awing East Each Face vation vator ength gineer Equal h Way	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard
Detail Plate awing Kisting East Each Face vator ength yineer Equal h Way ansion	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener
Detail Plate awing East Each Face vation vator ength gineer Equal h Way	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard
Detail Plate awing East Each Face vator ength jineer Equal h Way ansion cerior	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute
Detail Plate awing East Each Face vator ength yineer Equal h Way ansion cerior dation Finish	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural
Detail Plate awing Kisting East Each Face vator ength vator ength sineer Equal h Way ansion cerior dation Finish Floor	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical
Detail Plate awing Kisting East Each Face vator ength vator ength vator ength sineer Equal h Way ansion cerior dation Finish Floor lymer	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute
Detail Plate awing isting East Each Face vator ength vator ength sineer Equal h Way ansion cerior dation Floor lymer Side Feet	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove
Detail Plate awing Kisting East Each Face vator ength vator ength vator ength sineer Equal h Way ansion cerior dation Finish Floor lymer	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary
Detail Plate awing Kisting East Each Face vator ength jineer Equal h Way ansion cerior dation Floor Floor Side Feet poting	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove
Detail Plate awing Kisting East Each Face vator ength sineer Equal h Way ansion cerior dation Finish Floor Side Feet poting Eauge anized	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S.	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel
Detail Plate awing Listing East Each Face vator ength jineer Equal h Way ansion cerior dation Finish Floor Side Feet poting Eauge mized	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Steel Top of Steel Top of Wall
Detail Plate awing Kisting East Each Face vator ength sineer Equal h Way ansion cerior dation Finish Floor Side Feet poting Eauge anized	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Steel Top of Steel Top of Steel Top of Wall Transverse
Detail Plate awing Kisting East Each Face vator ength sineer Equal h Way ansion cerior dation Floor Side Feet poting Sauge mated Board	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.M. TRANS.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Steel Top of Steel Top of Wall
Detail Plate awing Kisting East Each Face vator ength fineer Equal h Way ansion cerior dation Floor Floor Side Feet poting cauge mized bard Board	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TS TYP.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
Detail Plate awing Kisting East Each Face vator ength fineer Equal h Way ansion cerior dation Floor Side Feet poting Sauge mated Board mized cader cm Fir	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TRANS. TS	South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Vall Transverse Tube Steel
Detail Plate awing Listing East Each Face vator ength Equal h Way ansion Equal h Way ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Educ h May anside H	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. STM. T/ T&B T&G STM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT.	5. South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical Top of Top and Bottom Tongue & Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical
Detail Plate awing Kisting East Each Face vator ength fineer Equal hasion cerior dation Floor Side Footing Fauge hased board mized cader contal	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B SYM. T/ T&B SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N.	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square</li> <li>Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fin Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue \$ Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical</li> </ul>
Detail Plate awing Listing East Each Face vator ength Equal h Way ansion Equal h Way ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Equal h May ansion Educ h May anside H	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G STM. T/ T&B T&G SYM. T/ T&B T&G SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TS TYP. U.O.N. VERT. VIF	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square</li> <li>Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue &amp; Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> </ul>
Detail Plate awing Kisting East Each Face vator ength fineer Equal hasion cerior dation Floor Side Footing Fauge hased board mized cader contal	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. STM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT.	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fin Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue &amp; Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Wall Transverse Tube Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical</li> </ul>
Detail Plate awing Listing Each Face vator engen Equal hasion cerior dation Floor Foting cauge hated Board mized contail conta	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S.	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square</li> <li>Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fir Stainless Steel Standard</li> <li>Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue &amp; Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> <li>West With</li> </ul>
Detail Plate awing East East Face vator enger ansion dation Finor Side face has bouged h	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square</li> <li>Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fir Stainless Steel Standard</li> <li>Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue \$ Groove Temporary Through Top of Steel Top of Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> <li>West With</li> <li>Welded Headed Stud Without</li> </ul>
Detail Plate awing Listing Each Face vator engen Equal hasion cerior dation Floor Foting cauge hated Board mized contail conta	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S.	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square</li> <li>Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fir Stainless Steel Standard</li> <li>Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue &amp; Groove Temporary Through Top of Concrete Top of Steel Top of Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> <li>West With</li> </ul>
Detail Plate awing sisting East Face vator faco faco faco face faco faco faco faco faco faco faco faco	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.W. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.P.	<ul> <li>South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue \$ Groove Temporary Through Top of Concrete Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Top of Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> <li>West With</li> <li>Welded Headed Stud Without Work Point</li> </ul>
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Detail Plate awing sisting East Face vator faco faco faco face faco faco faco faco faco faco faco faco	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. W/O W.P. W.T.S.	<ul> <li>South Schedule Section Sheet Similar Slab On Grade Specification Square Square Feet Square Inch(es) Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue &amp; Groove Temporary Through Top of Concrete Top of Steel Top of Steel Tube Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> <li>West With</li> <li>Melded Headed Stud Without</li> <li>Work Point</li> <li>Welded Threaded Stud</li> </ul>
Detail Plate awing East Each Face vator fineral pass Each vator fineral pass Face vator fineral pass for fineral finer	SCH. or SCHEI SECT. SHT. SIM. SOG SPEC. SQ. FT. SQ. IN. SPF S.S. STD. STIFF. STL. STR. SUB. SYM. T/ T&B T&G TEMP. THRU T.O.C. T.O.S. T.O.N. TRANS. TS TYP. U.O.N. VERT. VIF W. W/ or w/ W.H.S. WWF X SECT.	<ul> <li>South Schedule Section Sheet Similar</li> <li>Slab On Grade Specification Square Square Feet Square Inch(es)</li> <li>Spruce-Pine-Fir Stainless Steel Standard Stiffener Steel Standard Stiffener Steel Structural Substitute Symmetrical</li> <li>Top of Top and Bottom Tongue &amp; Groove Temporary Through Top of Concrete Top of Steel Top of Steel Tube Steel Typical</li> <li>Unless Otherwise Noted</li> <li>Vertical Verify in Field</li> <li>West With</li> <li>Welded Headed Stud Without</li> <li>Work Point</li> <li>Welded Threaded Stud Welded Wire Fabric</li> <li>Cross Section</li> </ul>

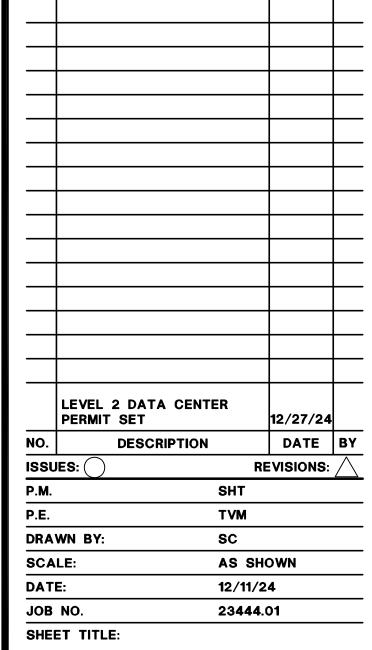
	ABBR	EVIA
@ d	At Penny (Nails)	
Ф °	Diameter Degrees	
# #	Pounds Number	
(A)	Above	
A.B.	Anchor Bolt	
ADD'L ALT.	Additional Alternate	
APPROX		
ARCH.	Architect	
A.S.D.	Allomable Stress Design	
(B) B/	Below Bottom of	
BF	Braced Frame	
BLKG. BLDG.	Blocking Building	
BLDO. BM.	Beam	
BOT. BRG.	Bottom	
BTWN.	Bearing Between	
CL or Q	Centerline	
C	<u>Camber</u>	
CIP C.J.	Cast In Place	
C.J. CJP	Construction Joint or Control Joint Complete Joint Penetration	
CLG.	Ceiling	
CLR. CMU	Clear Concrete Masonry Unit	
COL.	Column	
CONC. CONN.	Concrete Connections	
CONST.	Construction	
CONT. CSK.	Continuous Countersink	
DBA DBL.	Deformed Bar Anchor Double	
DEG.	Degree	
DF DIA.	Doug Fir-Larch Diameter	
DIAG.	Diagonal	
DIAPH. DIM.	Diaphragm Dimension	
DN.	Dimension Down	
DO DTL.	Ditto	
DTP.	Detail Double Top Plate	
DWG.	Drawing	
(E)	Existing	
E. EA.	East Each	
E.F.	Each Face	
EL. ELEV.	Elevation Elevator	
EMBED.	Embedment Length	
ENGR. EQ.	Engineer Equal	
E.M.	Each Ńay	
EXP. EXT.	Expansion Exterior	
	Foundation	
FDN. FIN.	Foundation Finish	
FLR. FRP	Floor Eiloon Reinforced Relumon	
F.S.	Fiber Reinforced Polymer Far Side	
FT. FTG.	Foot or Feet	
	Footing	
GA. GALV.	Gauge Galvanized	
GL	Glue Laminated	
GMB	Gypsum Wall Board	
HDG	Hot Dipped Galvanized	
HDR. HF	Header Hem Fir	
HGR.	Hanger	
HORIZ. HSS	Horizontal Hollow Structural Section	
HT.	Height	
I.D.	Inside Diameter	
I.F. IN.	Inside Face Inch	
INFO.	Information	
INT.	Interior	
JT.	Joint	
K	Kips	
KSF KSI	Kips per Square Foot Kips per Square Inch	

City of Puyallup Development & Permitting Service ISSUED PERMIT	
Building	Planning
Engineering	Public Works
Fire	Traffic

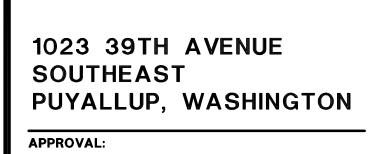


SHEET NO.

# GENERAL STRUCTURAL NOTES



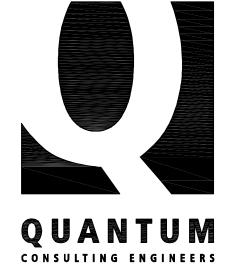




DATA CENTERS - TI

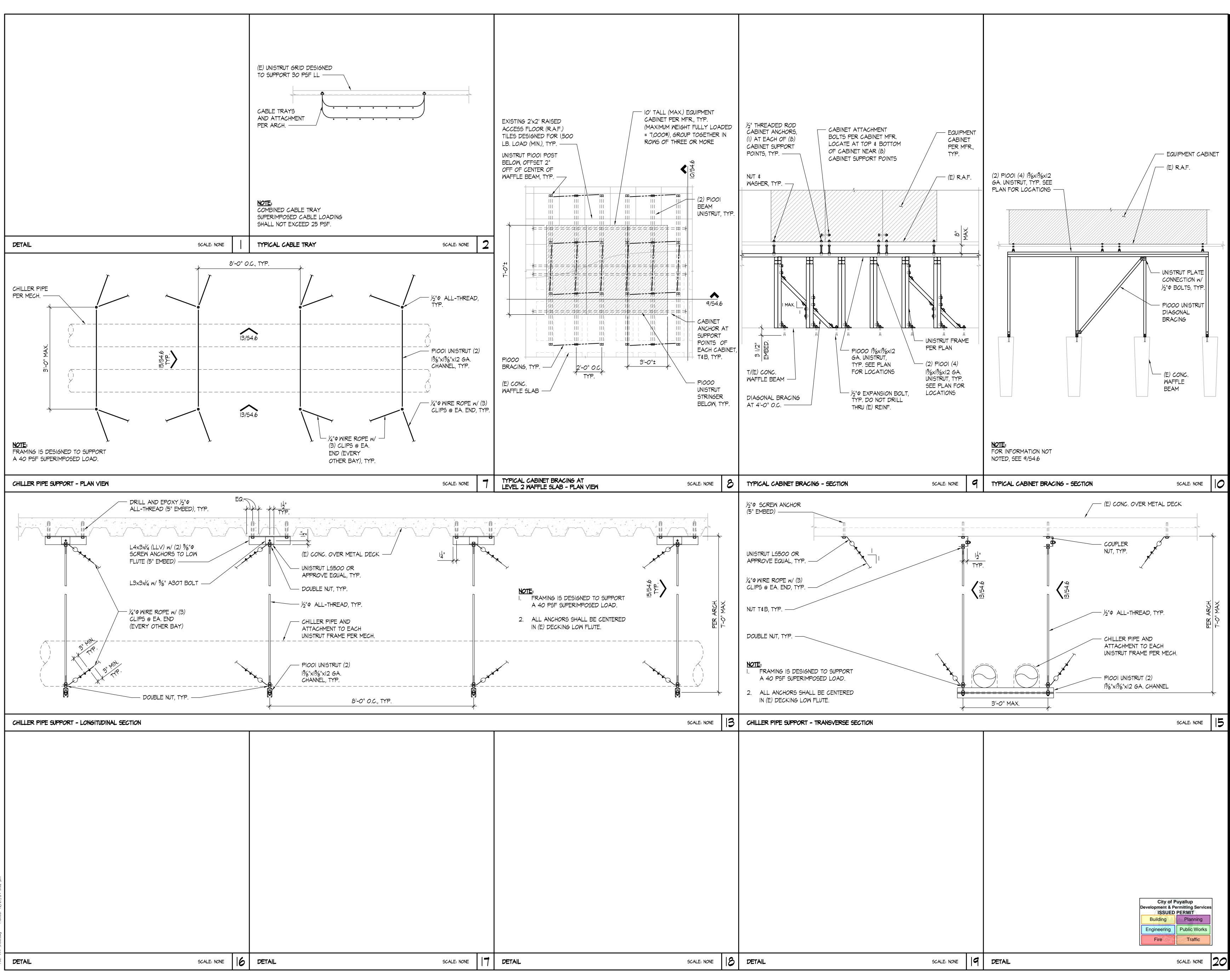
CENTERIS

Centers for Information Systems SEAL: PROJECT:



1511 THIRD AVENUE SUITE 323 SEATTLE, WA 98101 TEL 206.957.3900 FAX 206.957.3901 www.quantumce.com

centeris



SCALE: NONE	9	DETAIL	SCALE: NONE



SHEET NO.

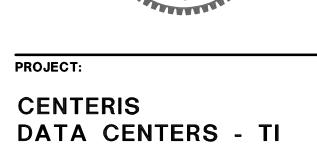
# DETAILS

	LEVEL 2 DATA CENT Permit set	ER	12/27/24		
NO.	DESCRIPTION		DATE	BY	
ISSUES:					
P.M.		SHT			
<b>P.E</b> .		ТVМ			
DRA	WN BY:	SC			
SCALE:		AS SHOWN			
DATE:		12/11/24			
JOB NO.		23444.01			
SHE	ET TITLE:				



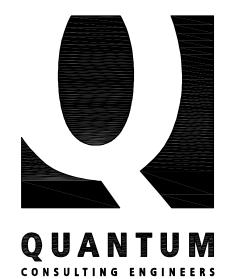
PUYALLUP, WASHINGTON APPROVAL:

SOUTHEAST



**1023 39TH AVENUE** 





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