AND ENGINEER.

1.263 = S5 - MAPPED SPECTRAL RESPONSE COEFFICIENT (SHORT PERIOD) 0.436 = S1 - MAPPED SPECTRAL RESPONSE COEFFICIENT (I-SECOND PERIOD) 1.010 = Sps- DESIGN SPECTRAL RESPONSE COEFFICIENT (SHORT PERIOD)

0.813 = S_{DI} - MAPPED SPECTRAL RESPONSE COEFFICIENT (1-SECOND PERIOD) D (DEFAULT) = SEISMIC SOIL SITE CLASS) = SEISMIC DESIGN CATEGORY II = RISK CATEGORY

COMPONENTS AND CLADDING:

PARTITION WALLS: Rp = 2.5, ap = 1.0, Ip = 1.0CEILING: Rp = 2.5, ap = 1.0, Ip = 1.0

5 PSF - MIN. HORIZONTAL LOAD ON PARTITIONS (LIVE LOAD) (REF. IBC SECTION 1607.15)

THE INTERNATIONAL BUILDING CODE (IBC) AND STANDARDS SHALL GOVERN ALL MATERIALS AND WORKMANSHIP.

DRAWINGS ARE FOR REFERENCE ONLY AND SHALL BE VERIFIED BY THE ARCHITECTURAL DRAWINGS.

ALL TEMPORARY SHORING OR BRACING IS THE RESPONSIBILITY OF THE CONTRACTOR. THE DRAWINGS REFLECT THE FINAL FINISHED CONDITION

OF THE STRUCTURE. THESE DRAWINGS ARE NOT INTENDED TO SHOW EACH AND EVERY CONDITION BUT TO INDICATE THE GENERAL CONSTRUCTION. WHERE

CONDITIONS ARE NOT SPECIFICALLY DETAILED, SIMILAR CONDITIONS SHALL BE USED AT THE DISCRETION AND APPROVAL OF THE ARCHITECT

THE CONTRACTOR IS RESPONSIBLE FOR ALL JOB SITE SAFETY AS WELL AS ALL MEANS, METHODS, AND SEQUENCES OF CONSTRUCTION TO SAFELY PERFORM THE WORK. AUE HAS NO EXPERTISE IN NOR HAS BEEN RETAINED TO PROVIDE REVIEW OF THE CONTRACTORS' SAFETY

PRECAUTIONS AS THEY RELATE TO THE CONSTRUCTION OF THIS PROJECT. IF ANY ERROR OR OMISSION APPEARS IN THESE DRAWINGS, SPECIFICATIONS, OR OTHER DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE OWNER AND STRUCTURAL ENGINEER OF RECORD IN WRITING OF SUCH OMISSION OR ERROR BEFORE PROCEEDING WITH THE WORK OR ACCEPT

FULL RESPONSIBILITY FOR THE COST TO RECTIFY SAME. VERIFY AND COORDINATE OPENINGS IN FLOORS, WALLS, AND ROOF WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. THE ARCHITECTURAL DRAWINGS SHALL BE REFERENCED FOR WALLS, FINISHES AND DIMENSIONS. DIMENSIONS PROVIDED ON THE STRUCTURAL

WHERE "ENGINEER APPROVED ALTERNATE" IS SPECIFIED HEREIN, AN ALTERNATE PRODUCT OF EQUAL OR GREATER STRENGTH AND SIMILAR IN NATURE, DURABILITY, AND FORM MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE STRUCTURAL ENGINEER OF RECORD (EOR) FOR REVIEW. NO ALTERNATE PRODUCTS MAY BE USED PRIOR TO THE APPROVAL OF THE EOR. ALTERNATE PRODUCTS THAT ARE SUBMITTED WITHOUT ADEQUATE DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF THE ORIGINALLY SPECIFIED PRODUCT WILL BE REJECTED.

DRAWINGS ARE NOT TO BE SCALED.

GENERAL CONTRACTOR REQUESTED CHANGES OR SUBSTITUTIONS SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. THE GENERAL CONTRACTOR SHALL ALLOW 10 WORKING DAYS MINIMUM FOR THE STRUCTURAL ENGINEER OF RECORD TO COMPLETE THEIR REVIEW. A RESPONSE FROM THE ARCHITECT AND/OR THE STRUCTURAL ENGINEER OF RECORD IS REQUIRED PRIOR TO THE FABRICATION AND/OR CONSTRUCTION OF THE REQUESTED CHANGES OR SUBSTITUTIONS. CHANGES SHOWN IN SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY ADDITIONAL ENGINEERING EFFORT AND ASSOCIATED FEES REQUIRED FOR REVIEW AND APPROVAL OF THE REQUESTED CHANGES AND SUBSTITUTIONS.

POST INSTALLED ANCHORS:

REFERENCE STANDARDS: ACI 318-19 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" TMS 402-16 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"

SEE THE BELOW LIST OF ICC/ IAPMO EVALUATION REPORTS FOR THE ANCHORS SPECIFIED PER PLAN. NO ANCHOR SUBSTITUTIONS FROM THE ANCHORS SPECIFIED ON PLAN ARE PERMITTED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.

ADHESIVE ANCHORS:

- TO CONCRETE: ◆ DEWALT PUREIIO+ (ICC ESR-3298, ESR-3298 LABC) ♦ HILTI HIT HY-200-R V3 (ICC ESR-4868, ESR-4868 LABC)
- GROUTED MASONRY:
- ◆ DEWALT ACIOO+ GOLD (ICC ESR-3200, ESR-3200 LABC) ♦ HILTI HIT HY-270 (ICC ESR-4143, ESR-4143 LABC)
- UNGROUTED MASONRY:
- ◆ DEWALT ACIOO+ GOLD W/ SCREEN TUBE (ICC ESR-3200, ESR-3200 LABC) ♦ HILTI HIT HY-270 W/ SCREEN TUBE (ICC ESR-4143, ESR-4143 LABC)
- UNREINFORCED MASONRY: ◆ DEWALT ACIOO+ GOLD W/ SCREEN TUBE (ICC ESR-4105, ESR-4105 LABC)
- ♦ HILTI HIT HY-270 W/ SCREEN TUBE (ICC ESR-4144, ESR-4144 LABC)

MECHANICAL ANCHORS TO CONCRETE:

- ◆ DEWALT POWER-STUD+ SD2 (ICC ESR-2502, ESR-2502 LABC)
- ◆ DEWALT SCREW-BOLT+ (ICC ESR-3889, ESR-3889 LABC)) ◆ HILTI KWIK BOLT TZ2 (KB-TZ2) (ICC ESR-4266, ESR-4266 LABC)
- ◆ HILTI KWIK HUS-EZ (KH-EZ) (ICC ESR-3027, ESR-3027 LABC) GROUTED MASONRY:
- ◆ DEWALT POWER-STUD+ SDI (ICC ESR-2966, ESR-2966 LABC) ◆ DEWALT SCREW-BOLT+ (ICC ESR-4042, ESR-4042 LABC)
- ◆ HILTI KWIK BOLT TZ2 (KB-TZ2) (ICC ESR-4561, ESR-4561 LABC) ♦ HILTI KWIK HUS-EZ (KH-EZ) (ICC ESR-3056, ESR-3056 LABC)

MANUFACTURER'S RECOMMENDATIONS AND ICC/IAPMO EVALUATION REPORT, NOTED ABOVE, SHALL BE FOLLOWED DURING THE PREPARATION AND INSTALLATION OF ALL GROUTED BOLTS, RODS, REINFORCING BARS, AND MECHANICAL ANCHORS.

MASONRY INSTALLATION, UNLESS NOTED OTHERWISE: INSTALLATION SHALL BE IN GROUTED CELLS ONLY. IF EMBEDMENT MUST BE PROVIDED IN AN UN-GROUTED CELL, NEW GROUT SHALL BE ADDED AT THAT CELL EXTENDING TO THE HORIZONTAL BOND BEAM BELOW. ONE EMBEDDED TITEM ONLY PER GROUTED CELL IS ALLOWED AND NO EMBEDDED ITEMS ARE ALLOWED WITHIN 8" OF A FREE EDGE.

INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARD INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM

NO REINFORCING STEEL SHALL BE CUT TO INSTALL ANCHORS. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT THAT AS A MINIMUM COMPRESSIVE STRENGTH THAT MATCHES THE ADJACENT CONCRETE/MASONRY GROUT COMPRESSIVE STRENGTH.

WHERE SPECIAL INSPECTIONS ARE REQUIRED PER THE ANCHOR ICC/IAPMO EVALUATION REPORT, INSPECTIONS SHOULD BE AS FOLLOWS:

1. FOR ALL ANCHORS PRIOR TO CONCEALMENT, VERIFY: ANCHOR TYPE, ANCHOR DIMENSIONS, ANCHOR SPACING AND EDGE DISTANCE

2. FOR EACH ANCHOR TYPE AND SIZE, THE INSPECTOR SHALL BE ONSITE TO CONTINUOUSLY INSPECT A MINIMUM OF (10) ANCHORS INSTALLED BY EACH INSTALLER FOR CONFORMANCE WITH THE APPLICABLE ICC/IAPMO EVALUATION REPORT.

IF THE TOTAL NUMBER OF ANCHORS INSTALLED BY EACH INSTALLER IS LESS THAN (10) THEN ALL ANCHORS INSTALLED BY EACH INSTALLER SHALL BE INSPECTED FOR CONFORMANCE WITH THE APPLICABLE ICC/IAPMO EVALUATION REPORT

PROVIDED ALL ANCHORS ARE INSTALLED CORRECTLY PER MANUFACTURER'S INSTRUCTIONS, PROVIDE PERIODIC INSPECTION ON A MINIMUM OF 10 PERCENT OF THE NEXT 1000 ANCHORS BY EACH INSTALLER AND A MINIMUM OF 5 PERCENT OF THE REMAINING ANCHORS BY EACH INSTALLER. INSPECTIONS SHOULD OCCUR ONCE A WEEK AT A RANDOM TIME WHILE ANCHOR INSTALLATION IS ONGOING. ANY NON-COMPLIANCE ISSUES SHALL RESET THE INSPECTION REQUIREMENTS TO THE BEGINNING OF THIS SECTION. ALL NON-COMPLIANT ANCHORS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD FOR REVIEW AND SHALL BE BROUGHT INTO COMPLIANCE EITHER THRU TESTING OR REINSTALLATION

3. FREQUENCY OF TESTING EXCEEDING THE REQUIREMENTS LISTED IN ITEM 2 SHALL BE PER THE ICC/IAPMO EVALUATION REPORT REQUIREMENTS. & CBC SECTION 1901.3.4 OR 1910A.5 OF THE CBC.

4. THE SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THE MINIMUM NUMBER OF ANCHORS WERE INSPECTED.

<u>POWDER ACTUATED FASTENERS:</u>

MANUFACTURERS RECOMMENDATIONS AND ICC REPORTS SHALL BE FOLLOWED DURING THE PREPARATIONS AND INSTALLATION OF ALL PAF'S. ALL REINFORCEMENT IN CONCRETE MUST BE LOCATED PRIOR TO INSTALLATION TO PREVENT DAMAGE DURING INSTALLATION. WASHERS ARE REQUIRED WHEN PAF'S ARE USED WITH WOOD MEMBERS.

HILTI X-U (POWDER ACTUATED FASTENER):

WHERE X-U PAF'S ARE SPECIFIED PER PLAN 0.157" DIAM. HILTI X-U ## PAF'S (ESR-2269, LARR 25675) SHALL BE USED OR ENGINEER APPROVED ALTERNATE.

AT CONCRETE SUBSTRATES, X-U PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO. I-1/4" MINIMUM PAF EMBEDMENT. 4" MINIMUM SPACING BETWEEN PAF'S. 3" MINIMUM CONCRETE EDGE DISTANCE. MINIMUM CONCRETE SUBSTRATE THICKNESS EQUAL TO 3X THE EMBEDMENT DEPTH OF THE FASTENER.

AT CONCRETE METAL DECK SUBSTRATES, X-U PAF'S SHALL BE INSTALLED AS SPECIFIED PER PLAN.

AT STRUCTURAL STEEL SUBSTRATES, X-U PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO. AT STEEL SUBSTRATES WITH A THICKNESS LESS THAN 1/2" PROVIDE FULL EMBED OF PAF TO STEEL (POINT OF FASTENER TO PENETRATE THROUGH STEEL BASE MATERIAL). AT STEEL SUBSTRATES WITH A THICKNESS GREATER THAN OR EQUAL TO 1/2" PROVIDE MIN 1/2" POINT PENETRATION OF PAF TO STEEL. I" MINIMUM SPACING BETWEEN PAF'S. 1/2" MINIMUM STEEL EDGE DISTANCE.

HILTI X-P (POWER ACTUATED FASTENER):

WHERE X-P PAF'S ARE SPECIFIED PER PLAN 0.118" DIAM. HILTI X-P ## G3 (GAS DRIVEN) OR 0.118" DIAM. HILTI X-P ## B3 (ELECTRI-MECHANICALLY DRIVEN) PAF'S (ESR-1752, LARR 25662) SHALL BE USED OR ENGINEER APPROVED ALTERNATE.

AT CONCRETE SUBSTRATES, X-P PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO. 5/8" PAF EMBEDMENT. 4" MINIMUM SPACING BETWEEN PAF'S. 3" MINIMUM CONCRETE EDGE DISTANCE. MINIMUM CONCRETE SUBSTRATE THICKNESS EQUAL TO 3X THE EMBEDMENT DEPTH OF THE FASTENER.

AT CONCRETE METAL DECK SUBSTRATES, X-P PAF'S SHALL BE INSTALLED AS SPECIFIED PER PLAN.

AT STRUCTURAL STEEL SUBSTRATES, X-P PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO. AT STEEL SUBSTRATES WITH A THICKNESS LESS THAN OR EQUAL TO 3/8" PROVIDE FULL EMBED. OF PAF TO STEEL (POINT OF FASTENER TO PENETRATE THROUGH STEEL BASE MATERIAL). AT STEEL SUBSTRATES WITH A THICKNESS GREATER THAN 3/8" X-P PAF'S ARE NOT PERMITTED FOR INSTALLATION. I MINIMUM SPACING BETWEEN PAF'S. 1/2" MINIMUM STEEL EDGE DISTANCE.

<u>STRUCTURAL STEEL</u>

REFERENCE STANDARDS: ANSI / AISC 303-16 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"

 ANSI / AISC 360-16 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" • 2014 RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS

 AWS DI.I-I5 "STRUCTURAL WELDING CODE - STEEL" ANSI / AISC 341-16 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS"

 W # WT SHAPES: ASTM A992 (Fy=50 ksi). • SQUARE OR RECTANGULAR STRUCTURAL TUBE (HSS): ASTM A500 GRADE C (Fy = 50 ksi) ROUND STRUCTURAL TUBE (HSS): ASTM A500 GRADE C (Fy = 46 ksi)

 PIPE MEMBERS: ASTM A53 GRADE B (Fy = 35 ksi) PLATES, CHANNELS, ANGLES, \$ RODS: ASTM A36 (Fy = 36 ksi) • PLATES (NOTED AS GRADE 50 ON DRAWINGS): ASTM A572 (Fy = 50 ksi)

AWS DI.8-16 "STRUCTURAL WELDING CODE - SEISMIC SUPPLEMENT"

 ALL STAINLESS STEEL: TYPE 316L (Fy = 30 ksi). • MILD STEEL WELDING ELECTRODES: E70XX (70 kgi MIN.), UNLESS NOTED OTHERWISE

 STAINLESS STEEL WELDING ELECTRODES: E316 (70 ksi MIN.) W/ GMAC PROCESS, UNLESS NOTED OTHERWISE TYPICAL HIGH STRENGTH BOLTS: ASTM F3I25 GRADE A325, UNLESS NOTED OTHERWISE

• AT CONNECTIONS SPECIFIED AS "SLIP CRITICAL" OR AT STRUCTURAL MEMBERS LABELED W/ "LFRS" ON THE PLANS PROVIDE ASTM F3125

GRADE A325-SC BOLTS INSTALLED PER CRITERIA FOR SLIP CRITICAL CONNECTIONS W/ CLASS A FAYING SURFACES. ANCHOR BOLTS OR ANCHORS RODS: ASTM F1554 GRADE 36 (Fy = 36 ksi) FURNISH ANCHOR RODS WITH MATCHING DOUBLE HEAVY HEX NUTS AT THE END EMBEDDED IN CONCRETE

◆ WELDED HEADED STUDS (WHS) 3/4" OR 7/8": ASTM AI08 - NELSON S3L OR ENGINEER APPROVED ALTERNATE ◆ WELDED HEADED STUDS (WHS) 1/2" OR 5/8": ASTM AI08 - NELSON H4L OR ENGINEER APPROVED ALTERNATE

DOWEL BAR ANCHORS: ASTM A496 NELSON D2L OR ENGINEER APPROVED ALTERNATE

DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE SPECIFICATIONS AND STANDARDS OF ANSI / AISC 303 AND ANSI/ AISC 360.

PROVIDE ONE COAT OF APPROVED SHOP PAINT MINIMUM TO ALL STEEL EXCEPT STEEL THAT IS HOT DIPPED GALVANIZED, SPRAY FIREPROOFED, OR TO BE EMBEDDED IN CONCRETE.

ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED, UNLESS NOTED OTHERWISE.

THE LATERAL FORCE RESISTING SYSTEM (LFRS) INCLUDES ALL STRUCTURAL STEEL FRAMING MEMBERS CALLED OUT IN ELEVATIONS AND PLANS WITH A "LFRS" DESIGNATION.

OPENINGS THRU BEAMS AND COLUMNS SHALL NOT BE PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.

ALL STRUCTURAL TUBES AND PIPES SHALL BE CAPPED W/ MIN. 1/4" PLATES AT ENDS.

FOR MEMBERS AND CONNECTIONS THAT ARE PART OF THE LATERAL FORCE RESISTING SYSTEM (LFRS) DISCONTINUITIES CREATED BY ERRORS, FABRICATION OR ERECTION OPERATIONS, SUCH AS TACK WELDS, ERECTION AIDS, AIR ARC GOUGING AND THERMAL CUTTING SHALL BE REPAIRED AS REQUIRED BY THE STRUCTURAL ENGINEER OF RECORD.

CONTRACTOR SHALL DESIGN AND SUPPLY ALL ADDITIONAL MISCELLANEOUS METALS THAT ARE INDICATED IN THE ARCHITECTURAL DRAWINGS OR THOSE METALS WHICH ARE FOUND TO BE NECESSARY TO SUPPORT THE ARCHITECTURAL FINISHES OR OTHER BUILDING SYSTEMS. ALL FRAMING AND CONNECTIONS DESIGNED BY THE CONTRACTOR SHALL NOT RESULT IN ECCENTRIC LOADS BEING APPLIED TO THE PRIMARY STRUCTURE OR LATERAL LOADS BEING APPLIED TO THE BOTTOM FLANGE OF STEEL BEAMS. SUBMIT CALCULATIONS STAMPED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF THE PROJECT ALONG WITH SHOP DRAWINGS NOTING THE LOADING IMPOSED ON THE

TEMPORARY BRACING OF STRUCTURAL STEEL ELEMENTS IS THE RESPONSIBILITY OF THE CONTRACTOR. STRUCTURAL STABILITY SHALL BE MAINTAINED AT ALL TIMES DURING THE ERECTION PROCESS.

THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE STRUCTURAL ENGINEER OF RECORD FOR ANY FIELD MODIFICATIONS OF ANCHOR BOLTS / RODS, COLUMN BASE PLATES, AND STRUCTURAL FRAMING MEMBERS PRIOR TO FIELD MODIFICATIONS TAKING PLACE.

GENERAL BOLTING REQUIREMENTS:

PRIMARY STRUCTURE.

ALL HIGH STRENGTH BOLT ASSEMBLIES SHALL BE IN ACCORDANCE WITH RCSC.

ALL BOLTED CONNECTIONS NOT SPECIFIED AS SLIP CRITICAL SHALL BE INSTALLED AS BEARING TYPE CONNECTIONS WITH THREADS EXCLUDED FROM THE SHEAR PLANE (TYPE "X") AND IN A "SNUG-TIGHTENED" CONDITION AS DEFINED PER RCSC UNLESS NOTED OTHERWISE IN THE

ALL BOLTED CONNECTIONS SPECIFIED AS SLIP CRITICAL OR AT MEMBERS WITH A "LFRS" DESIGNATION WHICH INCLUDES BUT IS NOT LIMITED TO - MOMENT FRAMES, BRACED FRAMES, DRAGS, ETC. ARE REQUIRED TO BE PRE-TENSIONED HIGH-STRENGTH BOLTS INSTALLED AS SLIP CRITICAL WITH A CLASS A FAYING SURFACE AS DEFINED PER RCSC UNLESS NOTED OTHERWISE IN THE PLANS. (EXCEPTION: FAYING SURFACES OF WELDED JOINTS WHOSE PARTS ARE JOINED INITIALLY BY ERECTION BOLTS NEED ONLY BE PREPARED SUCH THAT THEY ARE FREE FROM DEBRIS AND OTHER DEFECTS THAT WOULD PREVENT SOLID FIT-UP OF THE PARTS TO BE JOINED.)

TYPICAL HOLE SIZES FOR STEEL-TO-STEEL CONNECTIONS MAY HAVE A MAXIMUM HOLE DIAMETER OVER THE NOMINAL BOLT DIAMETER OF 1/16". TYPICAL HOLE SIZES FOR STEEL TO CONCRETE / CMU CONNECTIONS MAY HAVE A MAXIMUM HOLE DIAMETER PER TABLE 14-2 OF THE AISC CONSTRUCTION MANUAL AT COLUMN BASE PLATES. AT CONDITIONS OTHER THAN COLUMN BASEPLATES, THE MAXIMUM HOLE DIAMETER OVER THE NOMINAL BOLT DIAMETER SHALL BE 1/16".

PROVIDE HOT-DIPPED GALVANIZED "TYPICAL HIGH STRENGTH BOLTS" PER THE MATERIALS SECTION W/ GALVANIZED HARDENED WASHERS AND GALVANIZED HEX NUTS AT CONNECTIONS FOR ALL GALVANIZED MEMBERS.

PROVIDE WASHERS FOR ALL BOLTS AS REQUIRED PER RCSC. AS A MINIMUM PROVIDE STANDARD CUT WASHERS UNDER ALL NUTS. GENERAL WELDING REQUIREMENTS

ALL WELDING PROCEDURES, ELECTRODES AND WELDER QUALIFICATIONS SHALL CONFORM TO THE REQUIREMENTS IN AWS DI.I OR SET BY THE GOVERNING LOCAL JURISDICTION, WHICHEVER IS MORE STRINGENT, FOR ARC WELDING IN BUILDING CONSTRUCTION.

ALL FIELD AND SHOP WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS IN ACCORDANCE WITH AWS DI.I OR THE GOVERNING

JURISDICTION REQUIREMENTS, WHICHEVER IS MORE STRINGENT. ALL WELD ELECTRODES SHALL CONFORM WITH THE SPECIFIED TENSILE STRENGTHS IN THE "MATERIALS" SECTION ABOVE. ELECTRODES MUST BE KEPT DRY AT ALL TIMES.

WELDING OR TACKING OF HIGH-STRENGTH BOLTS IS NOT PERMITTED. BUTT WELDS ARE TO BE A COMPLETE JOINT PENETRATION (CJP) WELD, UNLESS NOTED OTHERWISE. ALL EXPOSED BUTT WELDS SHALL BE

GROUND SMOOTH. ALL FULL PENETRATION WELDS SHALL BE ULTRASONIC TESTED IN ACCORDANCE WITH AWS DI.I.

UNLESS A LARGER SIZE IS SPECIFIED, MINIMUM WELD SIZE SHALL BE 3/16" OR AS NOTED IN SECTION J.26 OF AISC 360, WHICHEVER IS LARGER. FIELD WELDS HAVE BEEN INDICATED WHERE THEY ARE EXPECTED TO OCCUR AND ARE INTENDED ONLY TO AID THE CONTRACTOR IN DETERMINATION OF FIELD VS SHOP WELDING. THE CONTRACTOR SHALL DETERMINE THE ACTUAL FIELD WELDING AS NEEDED FOR THE

CONSTRUCTION SEQUENCE AND INCLUDE ALL ASSOCIATED COSTS WITHIN THEIR BASE BID. SEE "SPECIAL INSPECTIONS" SECTION FOR OTHER REQUIREMENTS REGARDING SPECIAL INSPECTIONS FOR WELDING OF STRUCTURAL STEEL

LIGHT GAUGE STEEL FRAMING:

AWS DI.3-18 "STRUCTURAL WELDING CODE - SHEET STEEL"

REFERENCE STANDARDS:

 AISI SI00-I6(20) "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL - STRUCTURAL MEMBERS" AISI S240-20 "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL STRUCTURAL FRAMING"

 AISI S400-20 "NORTH AMERICAN STANDARD FOR SEISMIC DESIGN OF COLD-FORMED STEEL STRUCTURAL SYSTEMS" SSMA "PRODUCT TECHNICAL GUIDE"

 STRUCTURAL SECTIONS ◆ 54, 68, 97, II8 MIL THICKNESS (GAUGE 16, 14, 12, 10): ASTM A653, SS GRADE 50 CLASS I (Fy = 50 ksi, Fu = 65 ksi) ◆ 18, 27, 33, 43 MIL THICKNESS (GAUGE 25, 22, 20, 20, 18): ASTM A653, SS GRADE 33 (Fy = 33 ksi, Fu = 45 ksi)

• SELF-DRILLING SHEET METAL SCREWS: ASTM C1513 & SAE J78 W/ CORROSION RESISTANT COATING ◆ APPROVED SHEET METAL SCREWS > HILTI KWIK-FLEX (ICC ESR-2196, LARR 25095) OR ENGINEER APPROVED ALTERNATE

> ITW BUILDEX TEK (ICC ESR-1976, LARR 23768) OR ENGINEER APPROVED ALTERNATE

• TYP. FASTENERS TO STEEL: SEE "POWDER ACTUATED FASTENERS" SECTION OF NOTES TYP. FASTENERS TO CONCRETE: SEE "POWDER ACTUATED FASTENERS" SECTION OF NOTES WELDING ELECTRODES: E60XX (60 ksi MIN.)

GENERAL FRAMING REQUIREMENTS: DEPTH, GAUGE, AND SECTION PROPERTIES OF STUDS, JOISTS, TRACKS, ETC. SHALL MEET OR EXCEED THE THICKNESS AND SECTION

PROPERTIES AS LISTED IN THE SSMA. SEE ICC ESR-3064P FOR ADDITIONAL MATERIAL INFORMATION. ALL STUDS AND TRACKS SHALL BE GALVANIZED PER ASTM A653, G60 COATING UNLESS NOTED OTHERWISE.

ALL TRACKS, BRIDGING, STRAPS ETC. ARE TO BE FORMED FROM STEEL OF THE SAME THICKNESS AS THE STUDS OR JOISTS TO WHICH THEY ARE ATTACHED, UNLESS NOTED OTHERWISE

ALL FRAMING COMPONENTS SHALL BE SQUARE CUT FOR ATTACHMENT TO PERPENDICULAR MEMBERS, OR AS REQUIRED FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS. ALL FIELD CUTTING SHALL BE DONE BY SAWING OR SHEARING. STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB

PRIOR TO STUD AND TRACK ATTACHMENT. STUDS SHALL SECURELY BE FASTENED TO BOTH FLANGES OF THE TOP AND BOTTOM TRACK AS

NOTED IN THE STRUCTURAL DETAILS. ALL BEARING / SHEAR WALL SILL TRACKS ARE TO BEAR FULLY ON THE TOP OF FOUNDATION WALLS AND/OR SLABS. THE TOPS OF ALL FOUNDATION WALLS / SLABS SHALL BE SMOOTH AND LEVEL. SMOOTH AND LEVEL IS APPLICABLE WHEN THE MAXIMUM DEVIATION FROM GRADE IS +/- I/8" AND THE DEPRESSION BETWEEN THE HIGH SPOTS IS NOT GREATER THAN I/8" ALONG A 10 FOOT STRAIGHT EDGE.

ALL BEARING / SHEAR WALL SILL TRACKS SHALL HAVE LOAD BEARING SHIMS OR GROUT PROVIDED BETWEEN THE UNDERSIDE OF THE SILL TRACK AND THE TOP OF THE CONCRETE WALL / SLAB AT LOCATIONS WHERE A GAP LARGER THAN 1/4" OCCURS. SHIMS. SHALL BE LOCATED DIRECTLY BENEATH THE STUDS W/ FULL BEARING ON THE WALL / SLAB. SHIM PRODUCT DATA IS TO BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW PRIOR TO INSTALLATION.

ABUTTING LENGTHS OF TRACK SHALL BE SPLICED TOGETHER USING A PIECE OF STUD OF THE SAME WIDTH AND THICKNESS AND (4) #10 SCREWS PER FLANGE ON EACH SIDE OF THE SPLICE, UNLESS NOTED OTHERWISE.

PROVIDE DOUBLE STUDS AT ALL JAMBS, CORNERS, INTERSECTIONS AND BEAM BEARING, UNLESS NOTED OTHERWISE.

BUILT-UP MEMBER CONNECTIONS, UNLESS NOTED OTHERWISE:

• BACK-TO-BACK STUDS / JOISTS: (2) #10 SCREWS AT 12" O.C. BOXED HEADERS W/ STUD AND TRACK MEMBERS: #10 SCREW AT 6" O.C. THRU EACH FLANGE

PROVIDE WALL BRIDGING PER "TYP. WALL BLOCKING/BRACING" DETAIL, "ALT. WALL BLOCKING/BRACING" DETAIL, OR MANUFACTURER'S SPECIFICATIONS WHERE WALL IS NOT SHEATHED CONTINUOUSLY ON BOTH SIDES AND AS SHOWN IN DRAWINGS. PROVIDE WALL BRIDGING PER MANUFACTURER'S SPECIFICATIONS FOR WALLS DURING THE CONSTRUCTION PROCESS WHERE THEY ARE NOT SHEATHED PRIOR TO RESISTING

SEE "TYP. STUD/JOIST PUNCHOUTS" DETAIL FOR PUNCHOUT REQUIREMENTS.

JOISTS SHALL BE LOCATED DIRECTLY OVER STUDS, TYPICAL.

NO SPLICES ARE PERMITTED IN STUDS / JOISTS, UNLESS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.

CONNECTION REQUIREMENTS: SCREWS SHALL BE SELF-TAPPING / SELF-DRILLING PAN HEAD, HEX HEAD, OR WAFER HEAD SHEET METAL SCREWS, UNLESS NOTED

ALL SCREW CONNECTIONS SHALL BE MADE FROM THE LIGHTER MATERIAL INTO THE HEAVIER MATERIAL UNLESS NOTED OTHERWISE.

SCREWS THAT ARE REMOVED SHALL BE REPLACED BY A SCREW WITH A LARGER DIAMETER WHERE THE REPLACEMENT IS MADE INTO AN

REPLACE ALL SCREWS THAT STRIP OUT MATERIAL.

SCREWS SHALL HAVE A MINIMUM CENTER TO CENTER SPACING DIMENSION OF 3d AND A MINIMUM DISTANCE FROM CENTER OF SCREW TO THE EDGE OF ANY CONNECTING ELEMENT OF 1.5d, WHERE 'd' IS THE SCREW DIAMETER.

#8 SCREW (d = 0.164") --- MIN. SPACING: 1/2" \$ MIN EDGE: 1/4" #10 SCREW (d = 0.190") --- MIN. SPACING: 5/8" \$ MIN EDGE: 5/16" #12 SCREW (d = 0.216") --- MIN. SPACING: 3/4" \$ MIN EDGE: 3/8"

ALL SCREWS #8 AND LARGER SHALL HAVE A MINIMUM HEAD SIZE OF 0.292".

MINIMUM SCREW SIZE TO BE #8-18 (#2 POINT) OR #10-16 (#2 POINT) FOR 54 MIL (16 GA) OR LESS, U.N.O.

MINIMUM SCREW SIZE TO BE #10-16 (#3 POINT) OR #12-14 (#2 OR #3 POINT) FOR MATERIAL HEAVIER THAN 54 MIL (16 GA), U.N.O. PENETRATION OF SCREW SHALL NOT BE LESS THAN (3) EXPOSED THREADS PROJECTION THROUGH THE LAST MATERIAL JOINED.

MAXIMUM ATTACHMENT SPACING OF 12" SHALL BE USED FOR SHEATHING TO METAL CONNECTION, UNLESS NOTED OTHERWISE.

THE SCREW MANUFACTURER SHALL PROVIDE VERIFICATION OF THE FASTENERS RESISTANCE TO HYDROGEN EMBRITTLEMENT. LIGHT GAUGE FRAMING SHALL NOT BE WELDED UNLESS SPECIFIED BY THE STRUCTURAL PLANS OR DETAILS. ALL SPECIFIED WELDING, SHALL BE PERFORMED BY WELDERS EXPERIENCED IN LIGHT GAUGE STRUCTURAL STEEL FRAMING WORK. ALL WELDS OF GALVANIZED STEEL SHALL BE

SCREWS FOR SHEATHING CONNECTIONS SHALL BE OF THE PROPER SIZE AND TYPE FOR A POSITIVE SHEATHING TO METAL CONNECTION. A

TOUCHED UP WITH ZINC-RICH PAINT. ALL WELDS OF CARBON SHEET STEEL SHALL BE TOUCHED UP WITH PAINT. ALL CLIP ANGLES OR FLAT PLATES USED FOR ATTACHMENTS SHALL BE 33 MIL (20 GA.) MIN., UNLESS NOTED OTHERWISE

SAMPLE LIGHT GAUGE FRAMING STRUCTURAL SECTION DESIGNATION: 600SI62-54

• '600' - MEMBER DEPTH IN 1/100 INCH (6") 'S' - STYLE OF MEMBER (S = STUD / JOIST, T = TRACK, U = CHANNEL, F = FURRING CHANNEL)

 '162' - MEMBER FLANGE WIDTH IN 1/100 INCH (1.625" = 1-5/8") • '54' - MINIMUM STEEL THICKNESS IN MILS (54 MILS. = 0.054", OLD DESIGNATION OF 16 GA.)

SPECIAL INSPECTIONS: GENERAL REQUIREMENTS:

REQUIREMENTS OF THE SPECIAL INSPECTOR:

ALL SPECIAL INSPECTIONS SHALL MEET THE REQUIREMENTS OF THE IBC, CHAPTER 17

ALL INSPECTIONS AS REQUIRED BY SECTION 110 OF THE IBC ARE REQUIRED. INSPECTIONS SPECIFIED IN THESE NOTES ARE IN ADDITION TO THESE INSPECTIONS.

CITY INSPECTION IS NOT A SUBSTITUTE FOR SPECIAL INSPECTION.

ANY WORK WHICH HAS BEEN COVERED BUT NOT PROPERLY INSPECTED BY THE SPECIAL INSPECTOR AND/OR THE CITY INSPECTOR IS SUBJECT TO REMOVAL OR EXPOSURE.

WHERE SPECIFICALLY REQUIRED, CONTINUOUS INSPECTION IS REQUIRED DURING THE PERFORMANCE OF THE WORK. THIS MAY BE A REQUIREMENT OF THE BUILDING CODE / LOCAL JURISDICTION OR THE MANUFACTURER.

THE SPECIAL INSPECTOR MUST BE CERTIFIED TO PERFORM THE TYPES OF INSPECTION SPECIFIED AND SHALL DEMONSTRATE COMPETENCE TO THE SATISFACTION OF THE BUILDING OFFICIAL.

THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING AND INFORMING THE SPECIAL INSPECTOR OR CITY INSPECTOR AT LEAST ONE WORKING DAY BEFORE THE WORK IS TO BE PERFORMED UNLESS OTHER CONDITIONS ARE AGREED UPON.

THE SPECIAL INSPECTOR MUST WORK UNDER THE SUPERVISION OF A LICENSED CIVIL ENGINEER OF THE GOVERNING LOCAL JURISDICTION. THE SPECIAL INSPECTOR MUST PERSONALLY BE FAMILIAR WITH THE DRAWINGS AND MUST PERSONALLY OBSERVE ALL OF THE WORK REPORTED ON.

THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING DEPARTMENT AND ENGINEER. ANY DISCREPANCIES SHALL BE

IMMEDIATELY BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION; THEN, IF NOT CORRECTED, TO THE BUILDING DEPARTMENT

THE FINAL REPORT SHALL BE SIGNED BY A LICENSED CIVIL ENGINEER OF THE GOVERNING LOCAL JURISDICTION AND SHALL STATE THAT THE WORK WAS IN CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF IBC.

SPECIFIC SPECIAL INSPECTIONS REQUIRED: PERIODIC INSPECTIONS:

• ALL MECHANICAL POST-INSTALLED ANCHORS (SEE POST-INSTALLED ANCHORS SECTION FOR TEST FREQUENCY)

ALL WELDING OF STEEL EXCEPT WELDING PERFORMED IN AN AISC APPROVED SHOP.

AND ENGINEER.

THESE DOCUMENTS AND THE DESIGN ARE SPECIFIC TO THIS PROJECT ONLY AND MAY NOT BE REUSED IN ANY WAY WITHOUT WRITTEN APPROVAL OF AUE. IT IS OUR INTENT THAT THIS DESIGN MEETS THE NORMAL STANDARD OF CARE WITHIN THIS INDUSTRY. NO OTHER WARRANTY IS PROVIDED OR IMPLIED.

> City of Puyallup **Building REVIEWED FOR** COMPLIANCE SKinnear 03/03/2025 8:38:07 AM

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

to be provided by the permitee on site for inspection. Separate Electrical Permit is required with

Full sized legible color plans are required

the Washington State Department of Labor

https://lni.wa.gov/licensing-permits/electrical

electrical-permits-fees-and-inspections

or call for Licensing Information:

Engineering

Fire

& Industries.

1-800-647-0982 City of Puyallup **Development & Permitting Services ISSUED PERMIT** Planning Building

Public Works

Traffic

STRONG

GENERAL ABBREVIATIONS

CAST IN PLACE EF, E.F. EACH FACE

MICROLLAM BEAM PRESSURE TREATED T&G ANCHOR BOLT CONTRACTUAL JOINT EJ, E.J. EXPANSION JOINT GRD TONGUE AND GROOVE ABOVE GWB GYPSUM WALL MAS. MASONRY POST TENSIONED THR'D CENTER LINE EMBED. THREAD(ED) **ADDITIONAL** CLEAR EL, ELEV. ELEVATION BOARD MATERIAL QTY MAT'L TRUSS JOIST HDR HEADER **ADJACENT** CMU CONC. MASONRY INIT EN, E.N. EDGE NAIL MACHINE BOLT REF. REFERENCE MACMILLAN HEM-FIR ALTERNATE COL. COLUMN ENGINEER HF, H.F. REINFORCEMENT TMPRY TEMPORARY CONC. HGR HANGER APPROXIMATE CONCRETE EQUAL REQ'D REQUIRED TN, T.N. TOE NAIL MID-DEPTH ARCH HORZ. HORIZONTAL ARCHITECTURAL CONN. EACH SIDE T.O. CONNECT(ION) ES, E.S. MOMENT RESISTING ROUGH OPENING TOP OF HIGH STRENGTH BOLT HSB EXISTING FXIST TRANS. TRANSVERSE CONST. CONSTRUCTION SCHEDULE BOTTOM TYP. CONT. **EXTERIOR** (A325 UNO) TYPICAL CONTINUOUS SHEET MANUFACTURER BRACED FRAME FINISH FLOOR ELEV UNO CTSK COUNTERSINK HEIGHT UNLESS OTHERWISE METAL SHEATHING BELOW FABRICATION INSIDE FACE PENNY (NAILS) SIMILAR NEW MEMBER NOTED BUILDING VERT. DETAIL FOUNDATION INTERIOR SKEW(ED) VERTICAL DET. SKW NEAR SIDE BLOCKING DOUBLE FIN. FINISH(ED) JOIST VFY VERIFY SLAB ON GRADE NOT TO SCALE BOUNDARY NAIL MITH DF, D.F. DOUGLAS FIR FLG FLANGE JOINT SPACING *O*N CENTER BOUNDARY FLR FL00R KIPS (1,000 POUNDS) OF, O.F. WELDED HEADED φ, DIA. DIAMETER OUTSIDE FACE SQUARE WHS BFAM DIAGONAL FN, F.N. FACE NAIL SELECT STRUCTURAL STUD LAT. LATERAL OVER HANG BOTTOM OF WORK POINT LDGR DIAPH. DIAPHRAGM FO, F.O. FACE OF LEDGER **OPENING** STANDARD *o*pn'g BRIDGE (ING) WESTERN SERIES DITTO (DO OVER) FRM'G LINEAL FEET STAGGER ORIENTATE(ION) BEARING DEPTH FS, F.S. FAR SIDE LONG LEG HORIZ. STIFF. STIFFENER WTS WELDED THREADED PARALLEL BETWEEN FOOTING LONG LEG VERTICAL P/C PRECAST CONCRETE STIR. DRAWING FTG LLV STIRRUP WELDED WIRE FABRIC CAMBER(ED) GAGE WWF DOWEL STEEL DWL LS LAG SCREW PERPENDICULAR X-STG EXTRA STRONG GALV. GALVANIZED TIMBER STRAND STRUC. STRUCTURE(AL) CANTILEVER(ED) LSL EXISTING MEMBER XX-STG DOUBLE EXTRA GB, G.B. GRADE BEAM BFAM SYM. SYMETRICAL CENTER OF GRAVITY EA. EACH PLYWOOD

LIGHT WEIGHT

PSL

PARALLAM BEAM

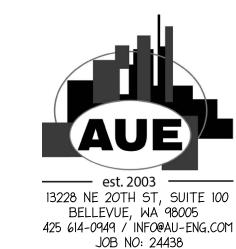
LT WT

GLB GLU-LAM. BEAM

BARNES&NOBLE

PROJECT DESIGNER:

ARCHITECT: JOEL TORIELLI 422 BOGERT AVE RIDGEWOOD, NJ 07450



3500 S. MERIDIAN UNIT #800

PUYALLUP, WA 98373

Project Number 33247 Store Number 3507

> Revision Loa: Description

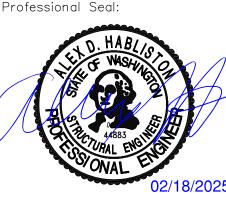
12-19-24 ISSUED FOR PERMIT, BID, LANDLORD & CLIENT REVIEW 02-20-25 BUILDING DEPARTMENT _____ COMMENTS

These plans are an instrument of service and the property of the Architect infringements will be prosecuted. General Contractor to verify all conditions

and dimensions at the premises.

Discrepancies shall be reported to the

Architect prior to the commencement of any



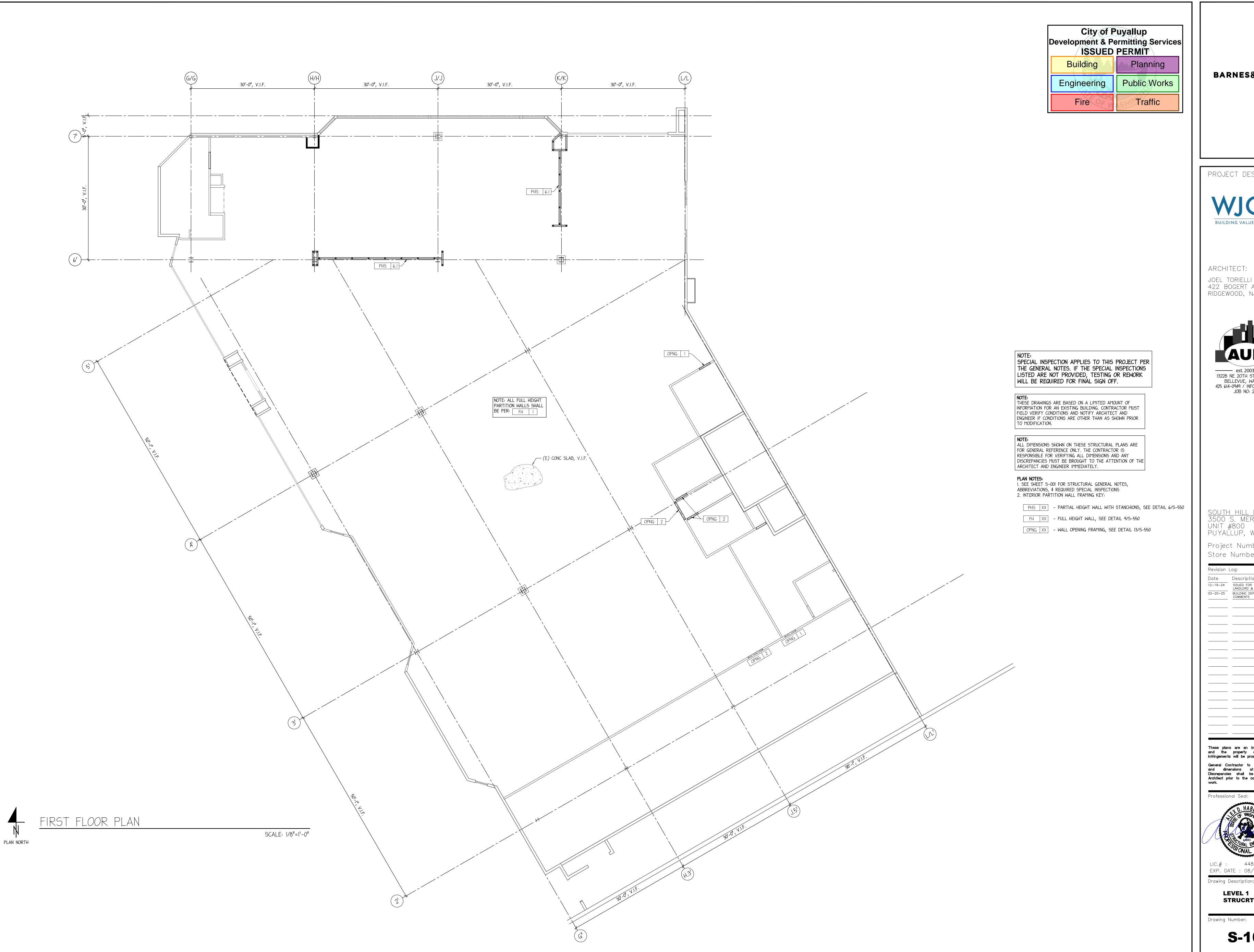
44883 EXP. DATE : 08/07/25 Drawing Description:

ABBREVIATIONS

GENERAL NOTES &

S-001

Drawing Number:



BARNES&NOBLE

PROJECT DESIGNER:



ARCHITECT: JOEL TORIELLI 422 BOGERT AVE RIDGEWOOD, NJ 07450



SOUTH HILL MALL 3500 S. MERIDIAN

PUYAĽLUP, WA 98373 Project Number 33247

Store Number 3507

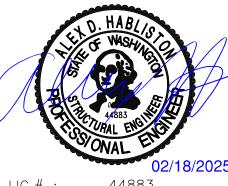
Date Description

These plans are an instrument of service and the property of the Architect, infringements will be prosecuted.

General Contractor to verify all conditions and dimensions at the premises.

Discrepancies shall be reported to the Architect prior to the commencement of any

Professional Seal:



LIC.# : 44883 EXP. DATE : 08/07/25

Drawing Description:

STRUCRTURAL PLAN

Drawing Number:



BARNES&NOBLE

PROJECT DESIGNER:



ARCHITECT:

JOEL TORIELLI 422 BOGERT AVE RIDGEWOOD, NJ 07450



est. 2003

13228 NE 20TH ST, SUITE 100
BELLEVUE, WA 98005
425 614-0949 / INFO@AU-ENG.COM
JOB NO: 24438

PUYAĽLUP, WA 98373 Project Number 33247

Store Number 3507 Revision Log:

Date Description 12-19-24
ISSUED FOR PERMIT, BID,
LANDLORD & CLIENT REVIEW

02-20-25
BUILDING DEPARTMENT
COMMENTS

1

These plans are an instrument of service and the property of the Architect, infringements will be prosecuted. General Contractor to verify all conditions and dimensions at the premises.

Discrepancies shall be reported to the Architect prior to the commencement of any

Professional Seal:

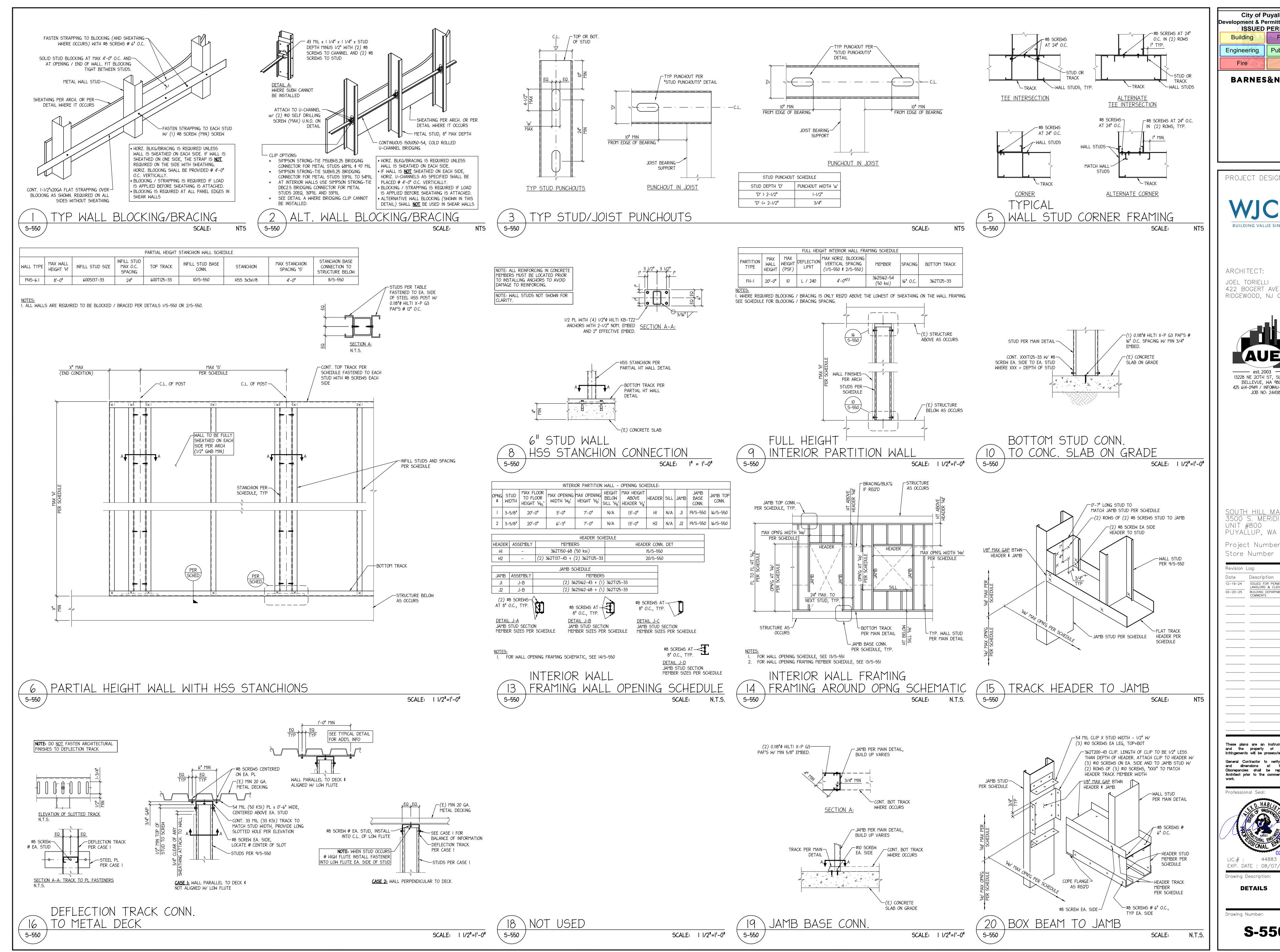


LIC.# : 44883 EXP. DATE : 08/07/25

Drawing Description:

RELFECTED CEILING

Drawing Number:



City of Puyallup Development & Permitting Services **ISSUED PERMIT** Building Planning Engineering Public Works Traffic

BARNES&NOBLE

PROJECT DESIGNER:

BUILDING VALUE SINCE 1994

ARCHITECT: JOEL TORIELLI



——— est. 2003 ———— 13228 NE 20TH ST, SUITE 100 BELLEVUE, WA 98005 425 614-0949 / INFO@AU-ENG.COM JOB NO: 24438

PUYAĽLUP, WA 98373 Project Number 33247 Store Number 3507

Revision Log: Date Description

12-19-24 ISSUED FOR PERMIT, BID, LANDLORD & CLIENT REVIEW 02-20-25 BUILDING DEPARTMENT

and the property of the Architect

infringements will be prosecuted. and dimensions at the premises. Discrepancies shall be reported to the Architect prior to the commencement of any

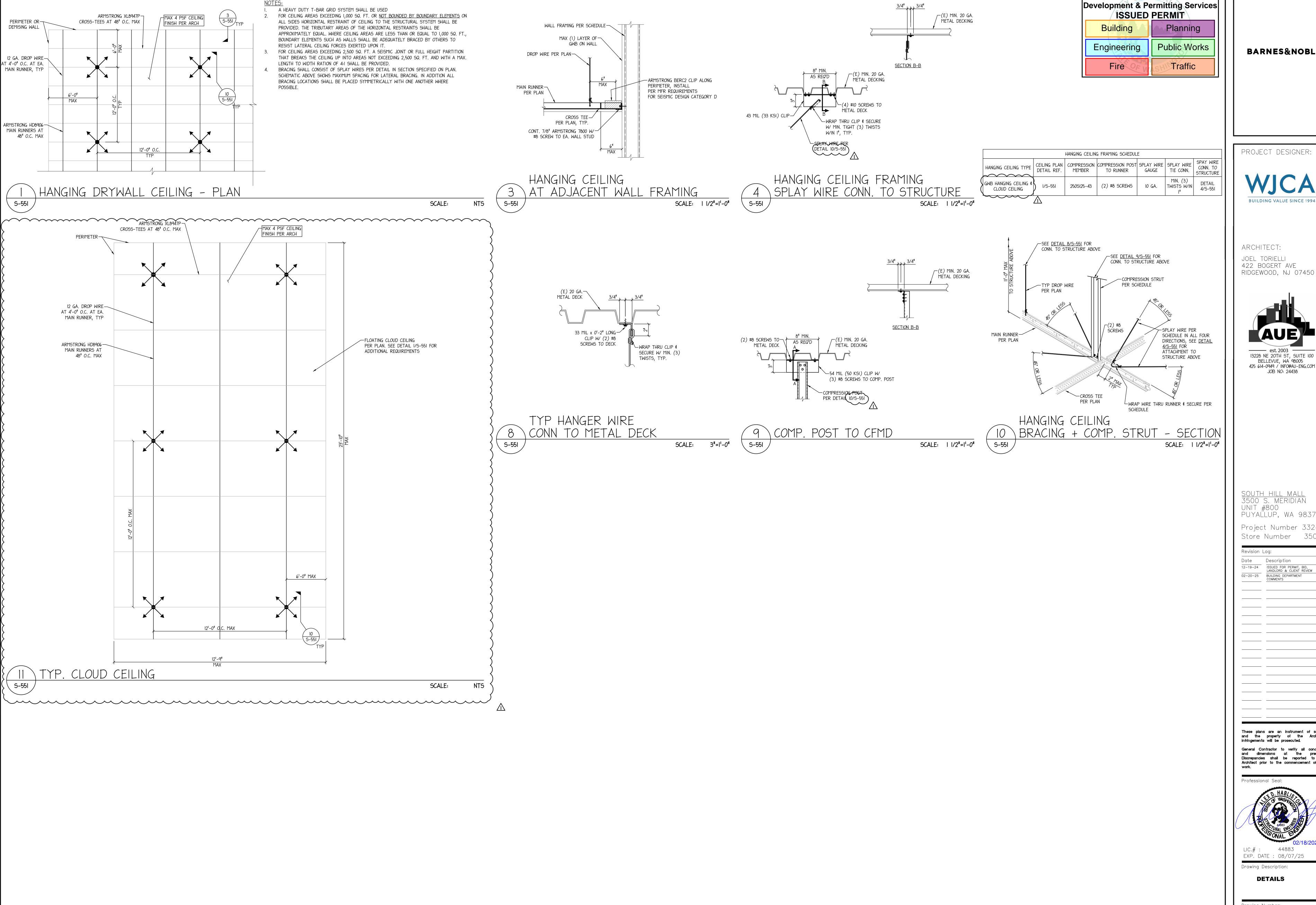
Professional Seal:



EXP. DATE : 08/07/25 Drawing Description:

DETAILS

Drawing Number:



BARNES&NOBLE

City of Puyallup

PROJECT DESIGNER:



ARCHITECT:

JOEL TORIELLI 422 BOGERT AVE RIDGEWOOD, NJ 07450



JOB NO: 24438

PUYAĽLUP, WA 98373 Project Number 33247

Store Number 3507

Revision Log: Date Description

12-19-24 ISSUED FOR PERMIT, BID, LANDLORD & CLIENT REVIEW DEPARTMENT COMMENTS

and the property of the Architect, infringements will be prosecuted.

General Contractor to verify all conditions and dimensions at the premises.

Discrepancies shall be reported to the Architect prior to the commencement of any

Professional Seal:



EXP. DATE : 08/07/25

DETAILS

Drawing Number: