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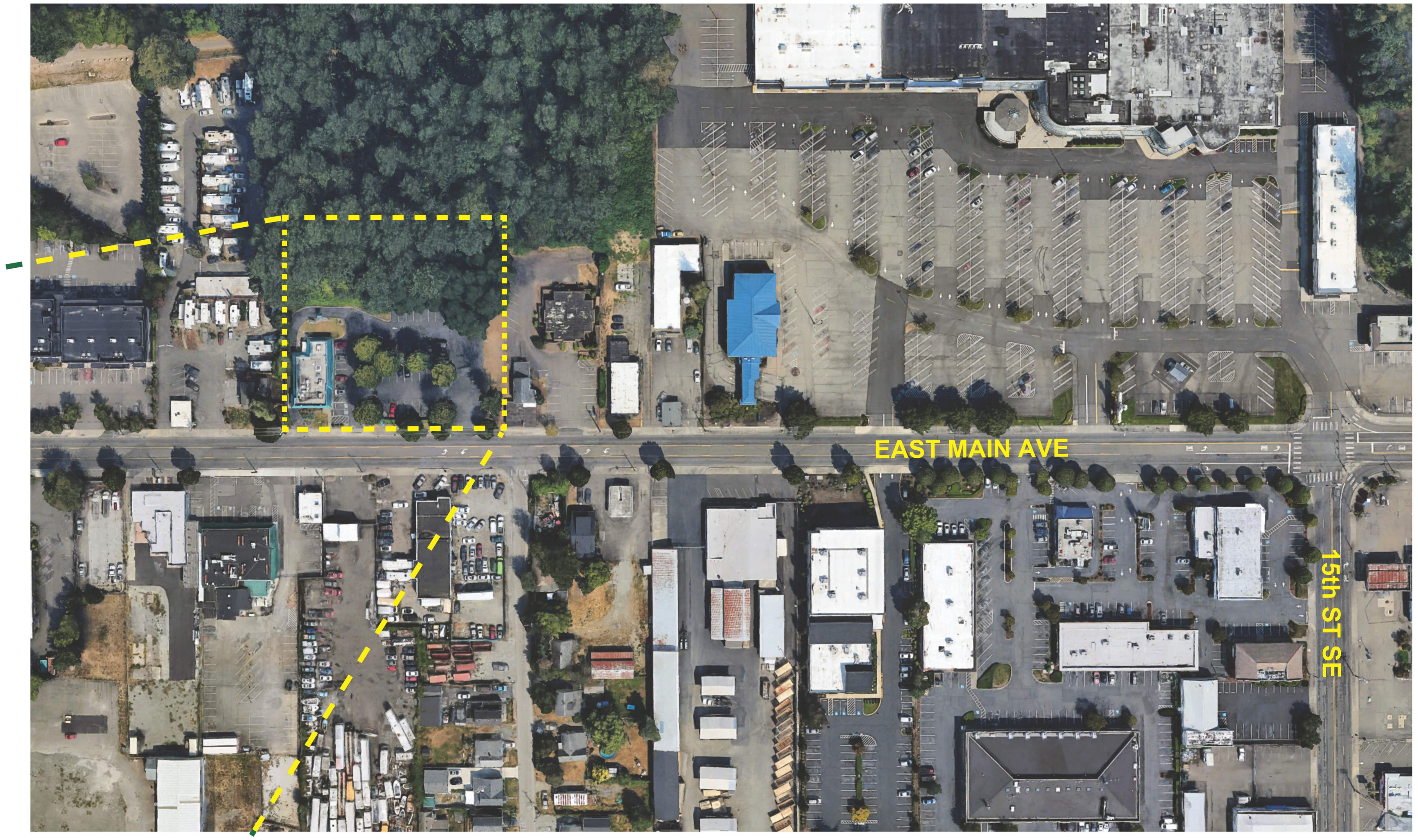

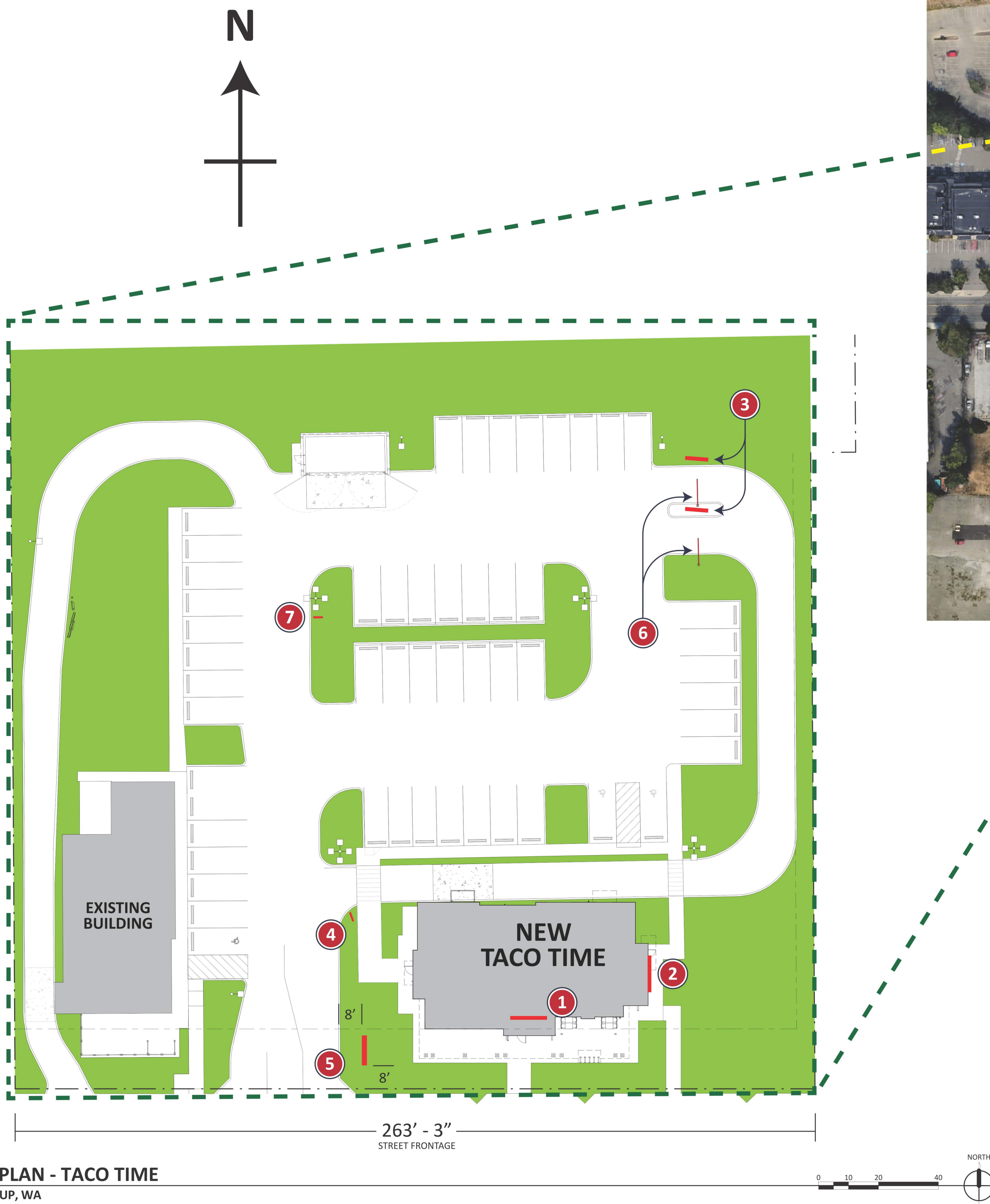

SITE PLAN - TACO TIME

PUYALLUP, WA

City of Puyallup	
Development & Permitting Services	
ISSUED PERMIT	
Building	Planning
Engineering	Public Works
Fire	Traffic

City of Puyallup Building REVIEWED FOR COMPLIANCE

BSnowden

04/25/2025

9:50:34 AM

CITY OF PUYALLUP STATE OF WASHINGTON

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection.

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

DESIGN	PROJECT	INSTALL LOCATION	APPROVED BY/DATE	SALES	SCALE	SHEET
17469	TACO TIME | EAST MAIN PUYALLUP	1115 E MAIN, PUYALLUP 98372	X	CARIN TAYLOR	NA	1 OF 7
				DRAWN BY: CARTER / PETERSEN	START DATE: 09.23.24	UPDATED: R2 02.04.25

plumb
PLUMB SIGNS, INC
909 SOUTH 28TH ST
TACOMA, WA 98409
TEL# 253-473-3323
FAX#253-472-3107
PLUMBSIGNS.COM

PRSG20250425

5 ILLUMINATED D/F MONUMENT SIGN (FLAT LEXAN FACE)

35.78 SQ. FT. (EACH SIDE) + 20.22 SQ. FT. BASE — QTY: 1 (SIGN)

FABRICATE AND INSTALL ONE (1) NEW MONUMENT SIGN
- DOUBLE FACED AND ILLUMINATED

- .090 ALUMINUM CONTRUCTION, PAINTED MATTHEWS HARTFORD GREEN
- 3/16" WHITE LEXAN FACE FOR ILLUMINATION
- 2" RETAINER
- 2" REVEAL
- APPLY TRANSLUCENT VINYL: HOLLY GREEN (3M 230-76)
  & DIGITAL PRINT MATCH TO PANTONE 5767c
  + BLACK (3M 3630-22) AND HOLLY GREEN (3M 3630-76)
- INTERNALLY ILLUMINATED w/ LEDs

MAXIMUM SQ FT = 150'
MAXIMUM HEIGHT = 10'
MINIMUM SETBACK = 6'

City of Puyallup
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DESIGN	PROJECT	INSTALL LOCATION	APPROVED BY/DATE
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SALES	SCALE	SHEET
CARIN TAYLOR	3/4"=1'	6 OF 7
DRAWN BY	START DATE	UPDATED
CARTER / PETERSEN	09.23.24	R2 02.04.25

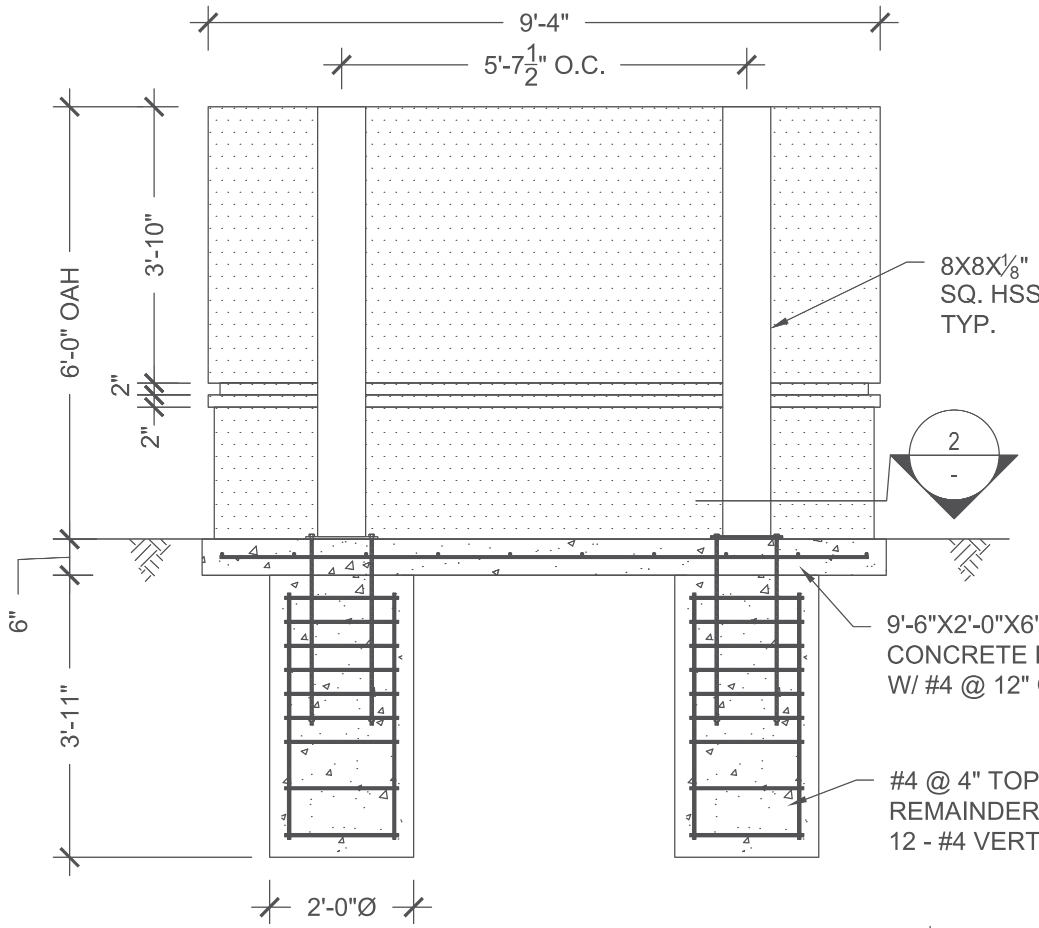
PRSG20250425

SULLAWAY ENGINEERING

12396 WORLD TRADE DRIVE, SUITE 312
SAN DIEGO, CA 92128
PROJECTMANAGER@SULLAWAYENG.COM
PHONE: 1-858-312-5150 FAX: 1-858-777-3534

PROJECT: TACO TIME, SIGN TYPE: PYLON SIGN, 1115 E. MAIN, PUYALLUP, WA
PROJECT #: 50191A
CLIENT: PLUMB SIGNS

DATE: 02/28/2025
ENGINEER: JC
LAST REVISED:

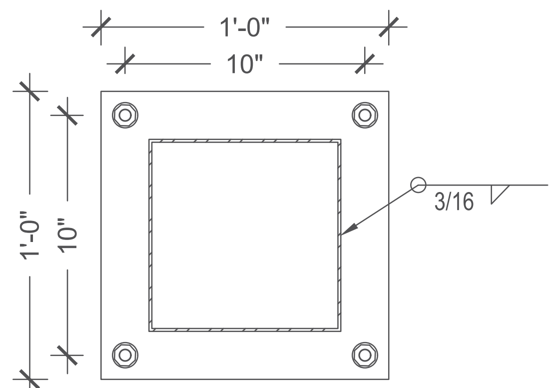


1 - ELEVATION

MICHAEL FRANKLIN SULLAWAY
STATE OF WASHINGTON
46800
REGISTERED PROFESSIONAL ENGINEER

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic



1'-0"X1'-0"X3/8" STEEL PLATE
W/ (4) 1/2"Ø THREADED ANCHOR RODS
MIN EMBED. = 24" INTO CONCRETE FOOTING
W/NUT/WASHER/NUT @ EMBED. END

2 - BASE PLATE, TYP.

GENERAL NOTES

1. DESIGN CODE: IBC 2021, WASHINGTON STATE BC 2021
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY 110 MPH EXPOSURE C
4. CONCRETE 2500 PSI MINIMUM
5. SQ. HSS STEEL ASTM A500 GR. B, F_y = 46 KSI MIN.
6. PLATE STEEL ASTM A36
7. THREADED ANCHOR ROD STEEL ASTM F1554 GR. 36, Fu=58 KSI MIN.
8. WELDING STRENGTH, FEXX = 70 KSI MIN.
9. STEEL REINFORCEMENT IN CONCRETE ASTM A615 GR 60
10. PROVIDE MIN. 3" CLEAR COVER ON ALL STEEL EMBEDDED IN CONCRETE WHEN CAST AGAINST SOIL
11. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
12. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
13. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION

PRSG20250425

SULLAWAY ENGINEERING

PROJECT: TACO TIME DATE: 02/28/2025

PROJ. NO.: 50191A ENGINEER: JC

CLIENT: PLUMB SIGNS

V5.5 units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16 (LRFD):

$F=q_z*G*C_f*A_f$ with $q_z = 0.00256K_zK_{zt}K_dV^2$ (29.3.2 & 29.4)

C_f= 1.433 (Fig. 29.3-1) 2 pole Cf factor= 0.96 5.625 max. height= 6.50

K_{zt}= 1.0 (26.8.2) (=1.0 unless unusual landscape)

K_z= from table 28.3-1 Exposure= c

K_d= 0.85 for signs (table 26.6-1)

V= 110 mph

G= 0.85 (26.9) weight= 0.556 kips

s/h= 0.923 M_{DL}= 0.000

B/s= 1.56

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure q_z*G*C_f	A_f	shear	Wind Moment M_W
	1	0.25	0.85	22.38	27.25	0.00	0	0
	2	1.42	0.85	22.38	27.25	16.81	458	649
	3	2.42	0.85	22.38	27.25	1.56	42	102
	4	2.58	0.85	22.38	27.25	1.50	41	106
	5	4.58	0.85	22.38	27.25	35.78	975	4469

sums: 55.64 1516 5.33 (M_w) k-ft arm= 3.5

two pole distribution factor *b*s (asce fig. 29.4-1): x 0.74 1129 3.97

for s/h=1, add 10% (asce fig. 29.4-1): x 1.10 4.36

P_u= 0.67 kip M= 4.36 k-ft $M=\text{sqrt}(M_{DL}^2+M_w^2)$

$M_u=\text{sqrt}(1.2M_{DL}^2+1.0M_W^2)$ = 4.36 k-ft

Pole Design section; tube

f_y= 46 ksi ϕ= 0.9

H	M_u(k-ft)	Z req'd. (in)	Size(in)	t (in)	Z	USE
6" below grade	4.36	1.26	2.5	0.188	1.3	8x8x1/8" Sq. HSS, ϕMn = 24.8 k-ft

Footing Design footprint: round

ω= 1.33 IBC 1605.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2 S=(2x1.33x150 psf/ft)

P= 0.90 kip S1 = S x d / 3 A = 2.34 x P / (S1 x b) S= 399

S1= 525 d =0.5xA (1+ (1+4.36x h/A) ^.5) IBC 1807.3.2.1

A= 2.01

footing: 2' - 0" dia. 3' - 11" deep

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SULLAWAY ENGINEERING

PROJECT: TACO TIME	DATE: 02/28/2025
PROJ. NO.: 50191A	ENGINEER: JC
CLIENT: PLUMB SIGNS	

V5.5

units; pounds, feet unless noted otherwise

Check 12x12x0.25" Steel Base Plate, A36 (LRFD):

arm = 1.200 in b = 12.00 in t = 0.375 in n= 2

Mplate =	T per bolt * arm * n=	6.091	k-in	(T= 2.538 k from Simpson)
Z=	bt^2/4=	0.422	in^3	
ϕMn =	ϕ*Fy*Z = 0.9*36ksi*Z =	13.669	k-in	**OK**

PRSG20250425

SIMPSON Strong-Tie®

Anchor Designer™
Software
Version 3.1.2301.3

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Address:			
Phone:			
E-mail:			

1.Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General
Design method:ACI 318-14
Units: Imperial units

Anchor Information:
Anchor type: Cast-in-place
Material: AB
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 24.000
Anchor category: -
Anchor ductility: Yes
h_{min} (inch): 25.88
C_{min} (inch): 3.00
S_{min} (inch): 3.00

Base Material
Concrete: Normal-weight
Concrete thickness, h (inch): 47.00
State: Cracked
Compressive strength, f'_c (psi): 2500
$\Psi_{c,V}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental edge reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Ignore 6do requirement: No
Build-up grout pad: No

Base Plate
Length x Width x Thickness (inch): 12.00 x 12.00 x 0.37
Yield stress: 36000 psi

Profile type/size: HSS8X8X1/8

Recommended Anchor
Anchor Name: PAB Pre-Assembled Anchor Bolt - PAB4 (1/2"Ø)

Prior to installation:
Review anchor product's ICC-ES Report and install the product per the report. If the report states special inspection(s) are required - the final special inspection report must be on site during City inspections.

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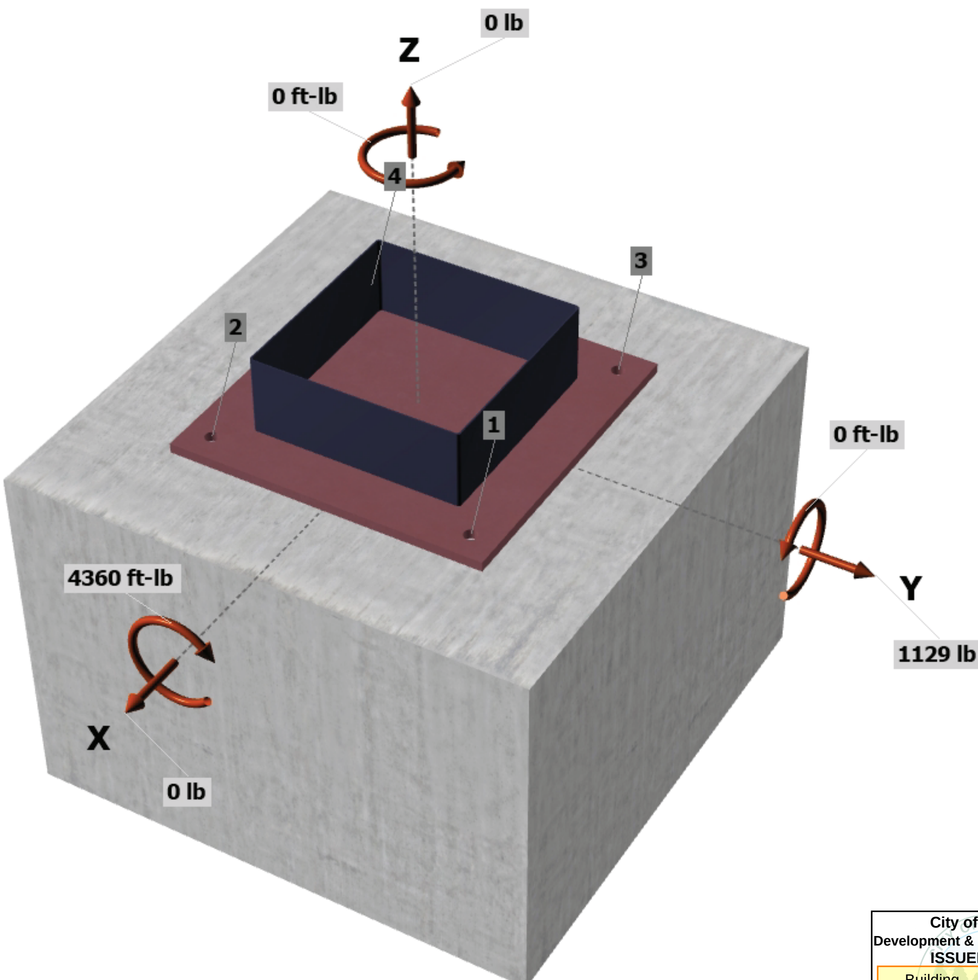
Load and Geometry
Load factor source: ACI 318 Section 5.3
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: Not applicable
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]: 0
V_{uax} [lb]: 0
V_{uay} [lb]: 1129
M_{ux} [ft-lb]: -4360
M_{uy} [ft-lb]: 0
M_{uz} [ft-lb]: 0

<Figure 1>



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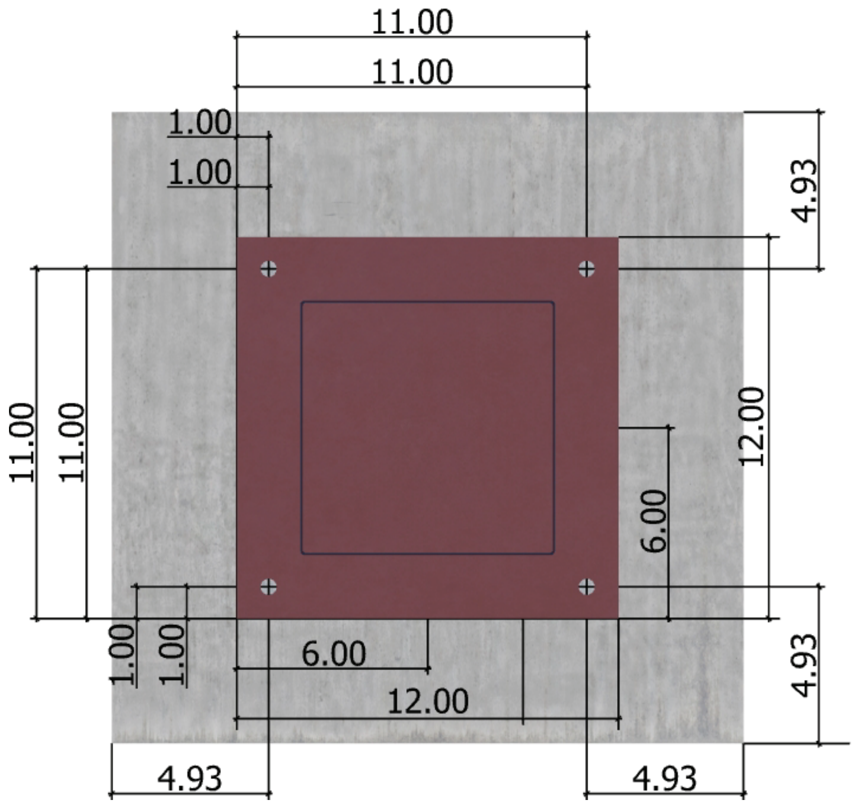
<Figure 2>



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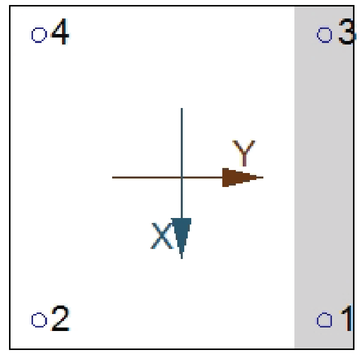
3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2+(V_{uay})^2}$ (lb)
1	0.0	0.0	282.3	282.3
2	2537.1	0.0	282.3	282.3
3	0.0	0.0	282.3	282.3
4	2537.1	0.0	282.3	282.3
Sum	5074.3	0.0	1129.0	1129.0

Maximum concrete compression strain (‰): 0.09
Maximum concrete compression stress (psi): 408
Resultant tension force (lb): 5074
Resultant compression force (lb): 5074
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8235	0.75	6176

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$N_b = 16\lambda_a\sqrt{f'_c}h_{ef}^{5/3}$ (Eq. 17.4.2.2b)

λ_a	f'_c (psi)	h_{ef} (in)	N_b (lb)
1.00	2500	9.953	36844

$\phi N_{cbg} = \phi\,(A_{Nc} / A_{Nco})\,\Psi_{ec,N}\,\Psi_{ed,N}\,\Psi_{c,N}\,\Psi_{cp,N}N_b$ (Sec. 17.3.1 & Eq. 17.4.2.1b)

A_{Nc} (in^2)	A_{Nco} (in^2)	$c_{a,min}$ (in)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
394.42	891.62	4.93	1.000	0.799	1.00	1.000	36844	0.70	9117

6. Pullout Strength of Anchor in Tension (Sec. 17.4.3)

$\phi N_{pn} = \phi\Psi_{c,P}N_p = \phi\Psi_{c,P}8A_{brg}f'_c$ (Sec. 17.3.1, Eq. 17.4.3.1 & 17.4.3.4)

$\Psi_{c,P}$	A_{brg} (in^2)	f'_c (psi)	ϕ	ϕN_{pn} (lb)
1.0	1.57	2500	0.70	21994

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7. Side-Face Blowout Strength of Anchor in Tension (Sec. 17.4.4)

$\phi N_{sbg} = \phi\{(1+c_{a2}/c_{a1})/4\}(1+s/6c_{a1})N_{sb} = \phi\{(1+c_{a2}/c_{a1})/4\}(1+s/6c_{a1})(160c_{a1}\sqrt{A_{brg}})\lambda\sqrt{f'_c}$ (Sec. 17.3.1, Eq. 17.4.4.1 & 17.4.4.2)

s (in)	c_{a1} (in)	c_{a2} (in)	A_{brg} (in²)	λ_a	f'_c (psi)	ϕ	ϕN_{sbg} (lb)
10.00	4.93	4.93	1.57	1.00	2500	0.70	23151

8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4940	1.0	0.65	3211

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear perpendicular to edge in y-direction:

$V_{by} = \min|7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5}; 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.500	1.00	2500	14.93	21640

$\phi V_{cbgy} = \phi\,(A_{Vc}/A_{Vco})\Psi_{ec,V}\Psi_{ed,V}\Psi_{c,V}\Psi_{h,V}V_{by}$ (Sec. 17.3.1 & Eq. 17.5.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgy} (lb)
444.76	1003.07	1.000	0.766	1.000	1.000	21640	0.70	5145

Shear parallel to edge in x-direction:

$V_{by} = \min|7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5}; 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.500	1.00	2500	4.93	4106

$\phi V_{cbgx} = \phi\,(2)(A_{Vc}/A_{Vco})\Psi_{ec,V}\Psi_{ed,V}\Psi_{c,V}\Psi_{h,V}V_{by}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
146.86	109.37	1.000	1.000	1.000	1.000	4106	0.70	7719

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

$\phi V_{cpg} = \phi k_{cp}N_{cbg} = \phi k_{cp}(A_{Nc}/A_{Nco})\Psi_{ec,N}\Psi_{ed,N}\Psi_{c,N}\Psi_{cp,N}N_b$ (Sec. 17.3.1 & Eq. 17.5.3.1b)

k_{cp}	A_{Nc} (in²)	A_{Nco} (in²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕV_{cpg} (lb)
2.0	394.42	100.00	1.000	0.996	1.000	1.000	5951	0.70	32720

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.6)

Tension	Factored Load, N_{ua} (lb)	Design Strength, $øN_n$ (lb)	Ratio	Status
Steel	2537	6176	0.41	Pass
Concrete breakout	**5074**	**9117**	**0.56**	**Pass (Governs)**
Pullout	2537	21994	0.12	Pass
Side-face blowout	5074	23151	0.22	Pass

Shear	Factored Load, V_{ua} (lb)	Design Strength, $øV_n$ (lb)	Ratio	Status
Steel	282	3211	0.09	Pass
T Concrete breakout y+	**1129**	**5145**	**0.22**	**Pass (Governs)**
|| Concrete breakout x-	**565**	**7719**	**0.07**	**Pass (Governs)**

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Pryout	1129	32720	0.03	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. 17.6.1	0.56	0.00	55.7%	1.0	Pass

PAB4 (1/2"Ø) with hef = 24.000 inch meets the selected design criteria.

12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.

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