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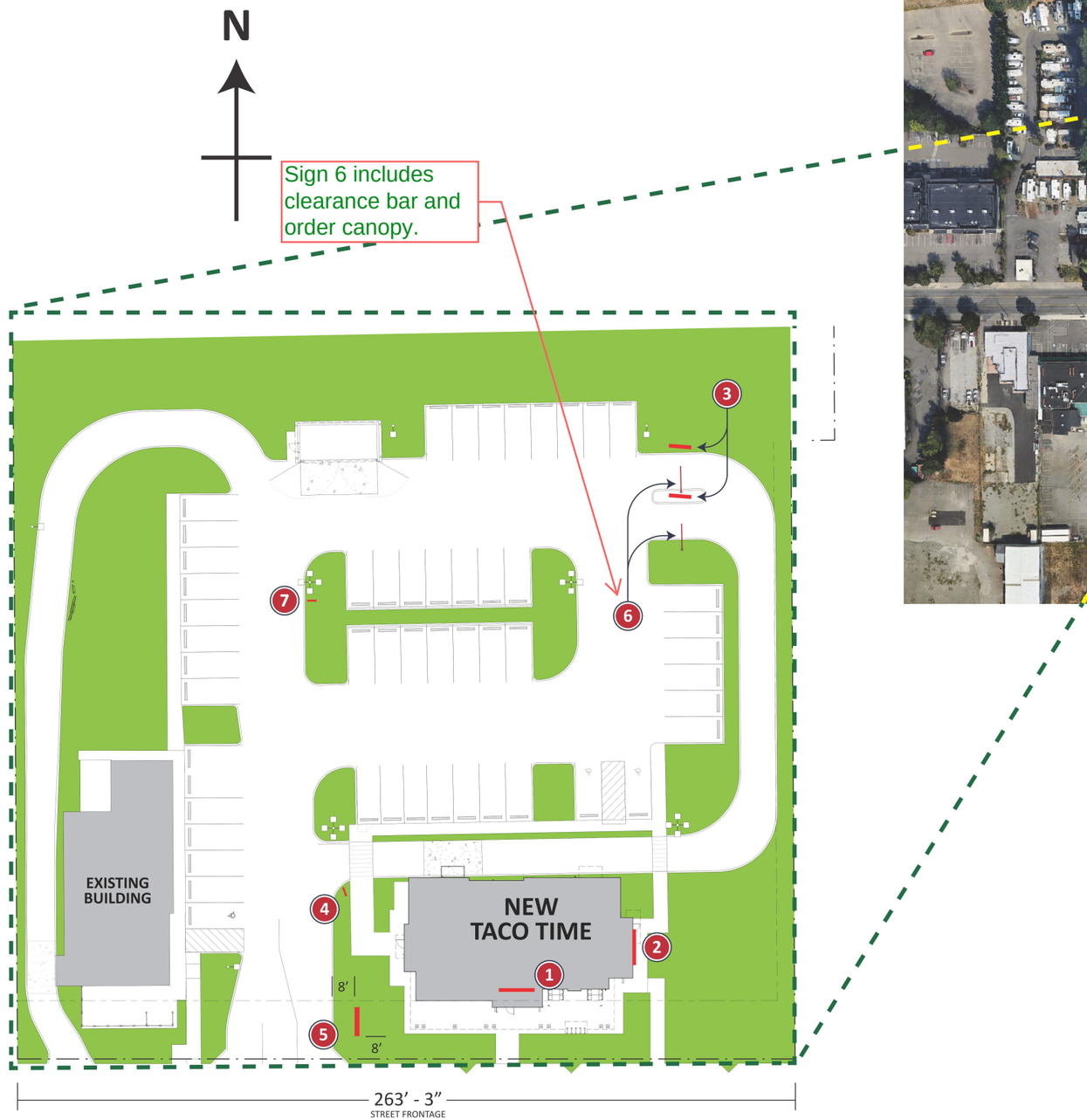

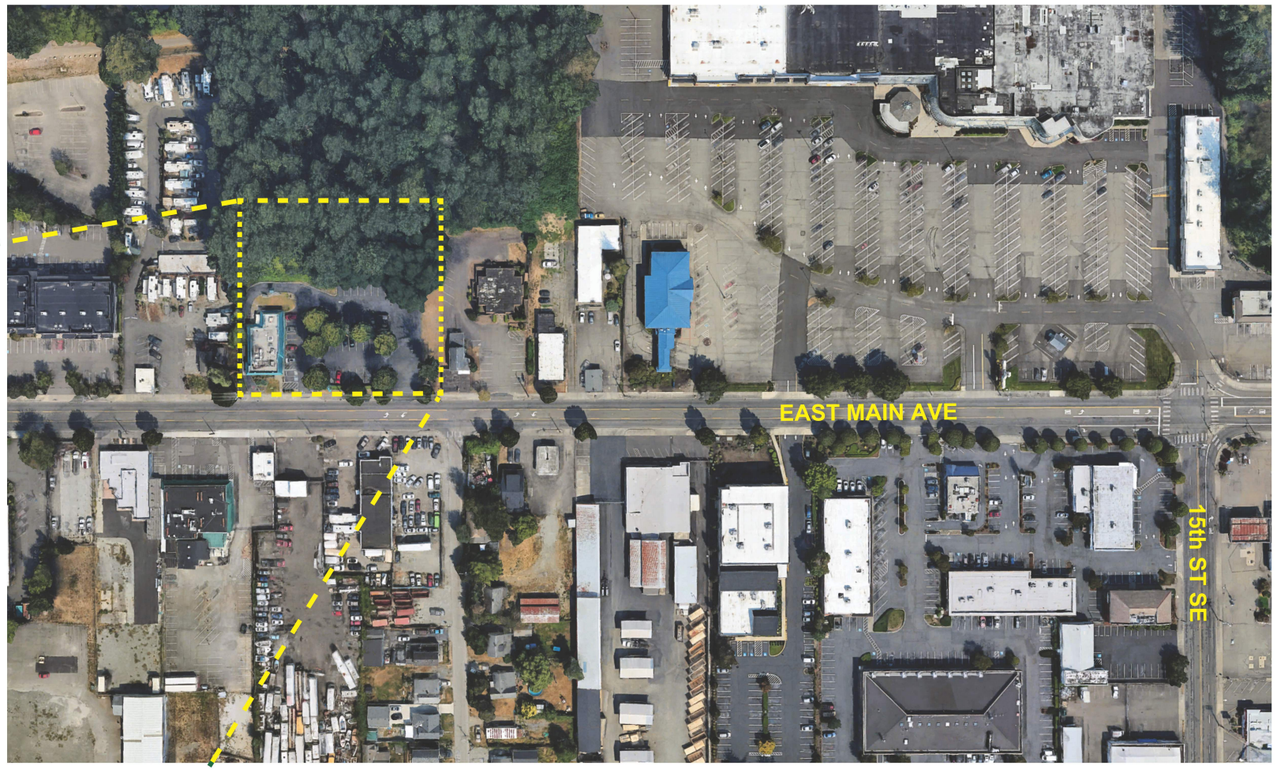

SITE PLAN - TACO TIME
PUYALLUP, WA

NORTH
0 10 20 40

City of Puyallup
Development & Permitting Services
ISSUED PERMIT
Building | Planning
Engineering | Public Works
Fire | Traffic

City of Puyallup
Building
REVIEWED FOR COMPLIANCE
BSnowden
04/25/2025
9:50:02 AM

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection.

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

DESIGN	PROJECT	INSTALL LOCATION	APPROVED BY/DATE
17469	TACO TIME | EAST MAIN PUYALLUP	1115 E MAIN, PUYALLUP 98372	X

PLUMB SIGNS, INC
909 SOUTH 28TH ST
TACOMA, WA 98409
TEL# 253-473-3323
FAX#253-472-3107
PLUMBSIGNS.COM

SALES	SCALE	SHEET
CARIN TAYLOR	NA	1 OF 7
DRAWN BY	START DATE	UPDATED
CARTER / PETERSEN	09.23.24	R2 02.04.25

PRSG20250425

City of Puyallup
Development & Permitting Services
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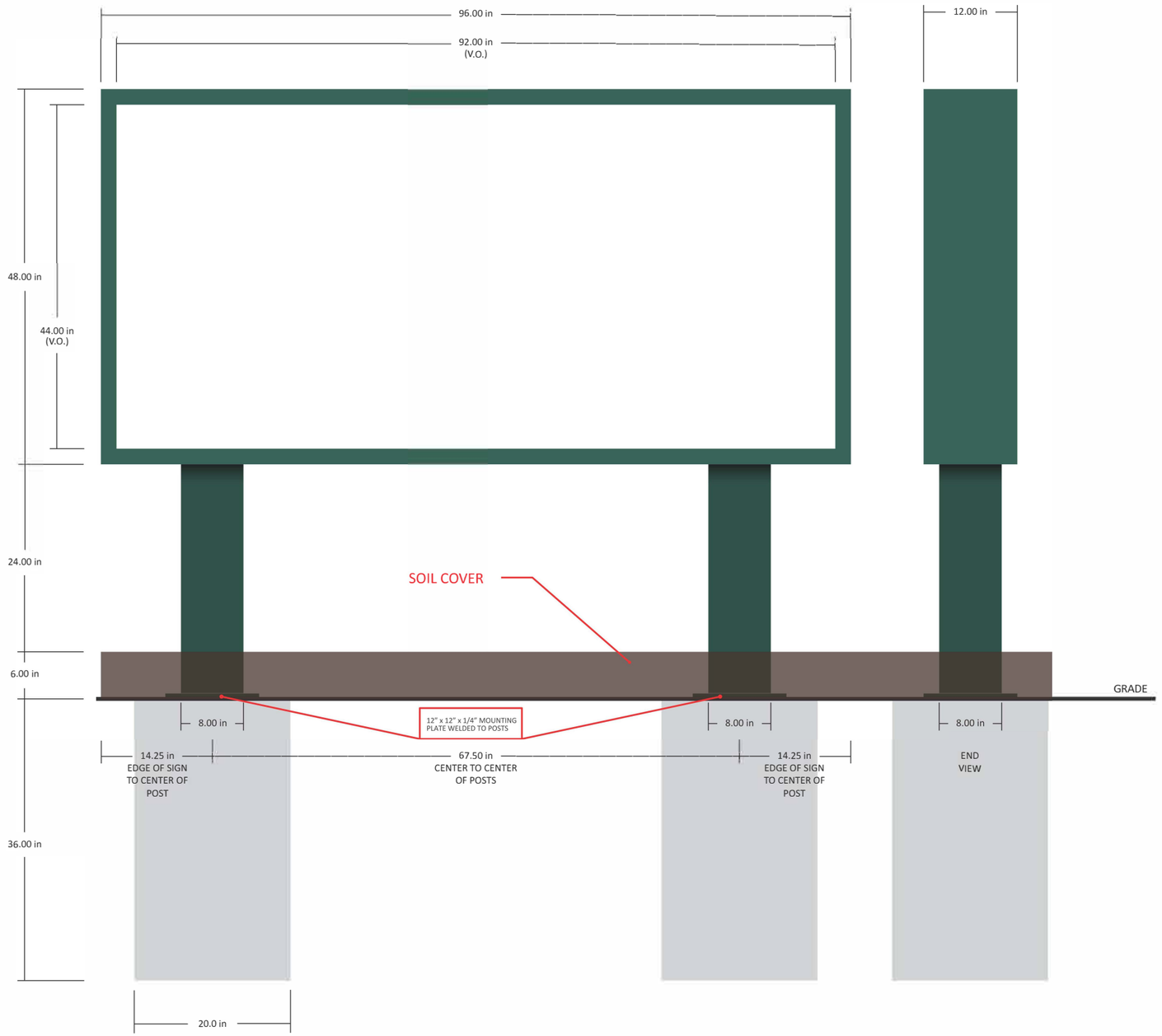
Engineering	Public Works
Fire	Traffic



3 MONUMENT MENU PYLON SIGN

32.0 SQ. FT. **QTY: 2**

FABRICATE AND INSTALL TWO (2) NEW MONUMENT MENU SIGN

- 2" SQUARE TUBE CONSTRUCTED FRAME w/ .090 ALUMINUM EXTERIOR
- PAINT CABINET AND ALL EXPOSED EXTERIOR MATTHEWS HARTFORD GREEN
- 3/16" LEXAN FACE
- GRAPHICS FOR FACE PROVIDED BY TACO TIME MARKETING DEPT
- 2" RETAINER SYSTEM
- SIDE PANEL REMOVEABLE FOR FACE REPLACEMENT/UPDATE IN FUTURE
- HANG BAR
- ILLUMINATED w/ WHITE LEDs

FABRICATE 12" x 12" x ¼" SQUARE MOUNTING PLATE
- MOUNTING PLATE BOLT INTO 20" x 36" FOOTINGS SUPPLIED BY GC w/ ½" ANCHOR BOLTS
- CONCRETE WILL BE DRILLED ONSITE TO MATCH PLATE HOLE PATTERN
- DO NOT DOUBLE NUT. WILL NOT NEED LEVELING NUTS.

Prior to installation:
Review anchor product's ICC-ES Report and install the product per the report. If the report states special inspection(s) are required - the final special inspection report must be on site during City inspections.

DESIGN	PROJECT	INSTALL LOCATION	APPROVED BY/DATE
17469	TACO TIME | EAST MAIN PUYALLUP	1115 E MAIN, PUYALLUP 98372	X_____

plumb
PLUMB SIGNS, INC
909 SOUTH 28TH ST
TACOMA, WA 98409
TEL# 253-473-3323
FAX#253-472-3107
PLUMBSIGNS.COM

SALES	SCALE	SHEET
CARIN TAYLOR	3/4"=1'	4 OF 7
DRAWN BY	START DATE	UPDATED
CARTER / PETERSEN	09.23.24	R2 02.04.25

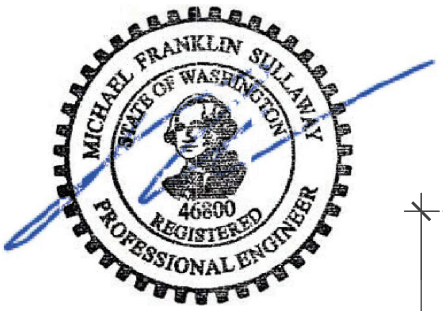
PRSG20250425

SULLAWAY ENGINEERING

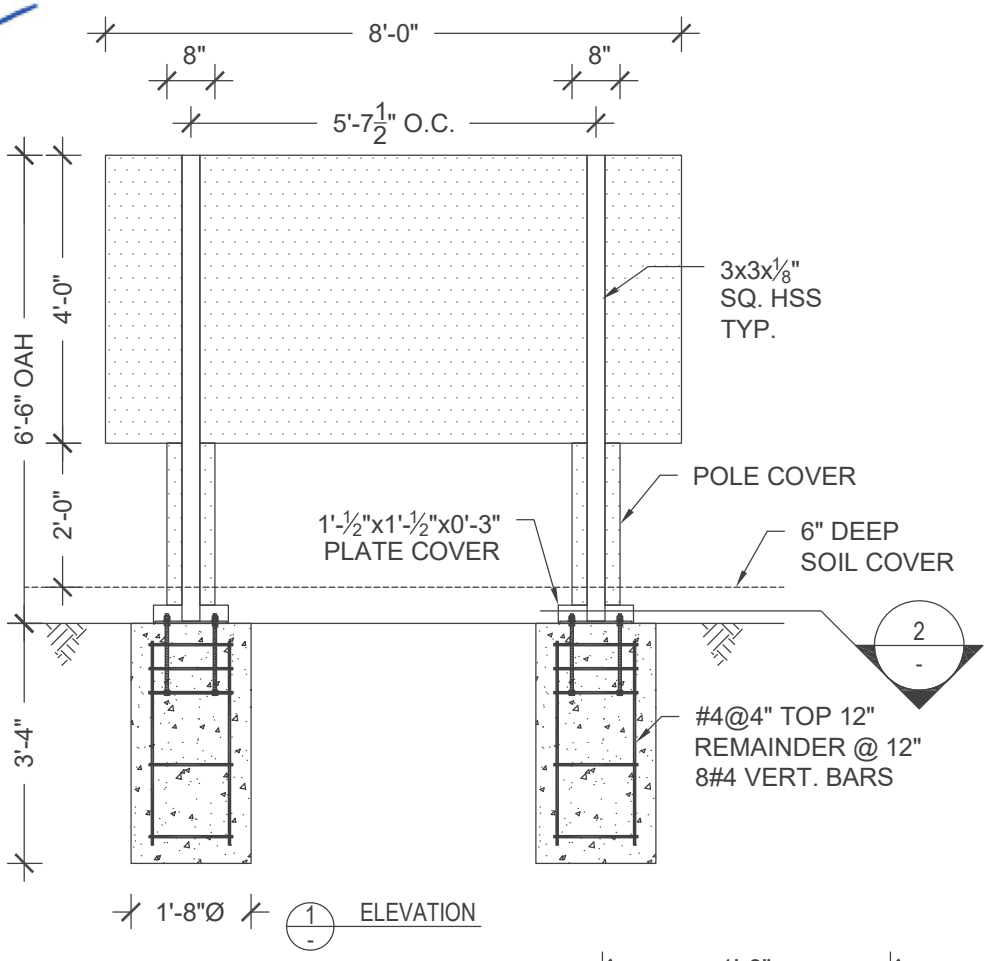
12396 WORLD TRADE DRIVE, SUITE 312,
SAN DIEGO, CA 92128
PROJECTMANAGER@SULLAWAYENG.COM
PHONE: 1-858-312-5150 FAX: 1-858-777-3534

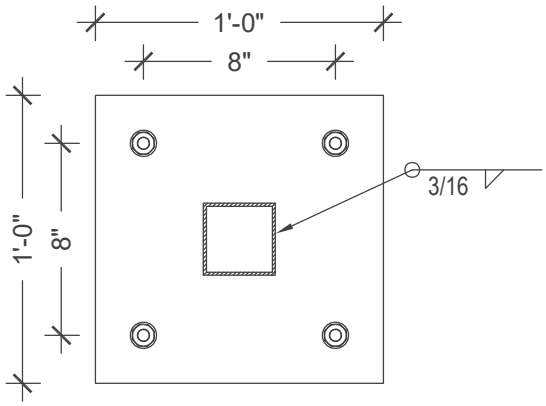
PROJECT: TACO TIME, SIGN TYPE: MONUMENT SIGN, 1115 E. MAIN, PUYALLUP, WA
PROJECT #: 50191D
CLIENT: PLUMB SIGNS

DATE: 03/18/2025
ENGINEER: JD
LAST REVISED:



3-19-25





1'-0"x1'-0"x⅜" STEEL PLATE
W/ (4) ½"Ø THREADED ANCHOR ROD
MIN. EMBED.= 12" INTO CONCRETE
W/NUT/WASHER/NUT @ EMBEDDED END

2
-
BASE PLATE, TYP.

GENERAL NOTES

1. DESIGN CODE: IBC 2021 & WASHINGTON SBCC 2021
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY 100 MPH EXPOSURE C
4. CONCRETE 2500 PSI MINIMUM
5. SQ. HSS STEEL ASTM A500 GR. B, F_y = 46 KSI MIN.
6. PLATE STEEL ASTM A36
8. WELDING STRENGTH, Fexx = 70 KSI MIN.
9. THREADED ANCHOR ROD STEEL ASTM F1554 GR. 36, Fu=58 KSI MIN.
10. STEEL REINFORCEMENT IN CONCRETE ASTM A615 GR. 60
11. PROVIDE MIN. 3" CLEAR COVER ON ALL STEEL EMBEDDED IN CONCRETE WHEN CAST AGAINST SOIL
12. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
13. PROVIDE PROTECTION AGAINST DISSIMILAR METALS
14. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION

PRSG20250425

SULLAWAY ENGINEERING

PROJECT: TACO TIME
PROJ. NO.: 50191D
CLIENT: PLUMB SIGNS

DATE: 3/18/2025
ENGINEER: JD

V5.5

units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16 (LRFD):

$F=q_z*G*C_f*A_f$ with $q_z = 0.00256K_zK_{zt}K_dV^2$ (29.3.2 & 29.4)

C_f= 1.533 (Fig. 29.3-1) 2 pole Cf factor= 0.93 5.625 max. height= 6.50

K_{zt}= 1.0 (26.8.2) (=1.0 unless unusual landscape)

K_z= from table 28.3-1 Exposure= c

K_d= 0.85 for signs (table 26.6-1)

V= 100 mph

G= 0.85 (26.9) weight= 0.355 kips

s/h= 0.615 M_{DL}= 0.000 k-ft

B/s= 2.00

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure q_z*G*C_f	A_f	shear	Wind Moment M_W
	1	0.250	0.850	18.50	24.10	0.85	21	5
	2	1.500	0.850	18.50	24.10	2.67	64	96
	3	4.500	0.850	18.50	24.10	32.00	771	3471
					sums:	35.52	856	3.57 (M_w) k-ft arm= 4.2
	two pole distribution factor *b*s (asce fig. 29.4-1):				x 0.68		579	2.42

P_u= 0.43 kip M= 2.42 k-ft $M=sqrt(M_{DL}^2+M_w^2)$

$M_u=sqrt(1.2M_{DL}^2+1.0M_W^2)$ = 2.42 k-ft

Pole Design section; tube

$M_u \le \phi M_n$ with $M_n = f_yZ$ f_y= 46 ksi ϕ= 0.9

H	M_u(k-ft)	Z req'd. (in)	Size(in)	t (in)	Z	USE
at grade	2.42	0.70	2	0.25	1.0	3x3x1/8" Sq. HSS, ϕMn= 4.82 k-ft

Footing Design footprint: round

ω= 1.3 IBC 1605.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2 S=(1.3x2x150 psf/ft)

P= 0.45 kip S1 = S x d / 3 A = 2.34 x P / (S1 x b) S= 400

S1= 446 d =0.5xA (1+ (1+4.36x h/A) ^.5) IBC 1807.3.2.1

A= 1.42

footing: **1' - 8" dia.** **3' - 4" deep**

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CLIENT: PLUMB SIGNS

DATE: 3/18/2025
ENGINEER: JD

V5.5

units; pounds, feet unless noted otherwise

Check 12x12x0.375" Steel Base Plate, A36 (LRFD):

arm =	2.575	in	b =	12.00	in	t =	0.375	in	n= 2
Mplate =			T per bolt * n * arm =				7.998	k-in	(Tu=From Simpson)
Z=			bt^2/4=				0.422	in^3	(Tu= 1.553k)
ϕMn =			ϕ*Fy*Z = 0.9*36ksi*Z =				13.669	k-in	
Ratio check=			Mplate/ϕMn=				0.585	<1	**OK**

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SIMPSON Strong-Tie

Anchor Designer™ for Concrete Software
Version 3.3.2501.2

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Address:			
Phone:			
E-mail:			

1.Project information

Project description:
Location:
Design name: Design

Comment:

2. Input Data & Anchor Parameters

General
Design method:ACI 318-14
Units: Imperial units

Anchor Information:
Anchor type: Cast-in-place
Material: AB
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 12.000
Anchor category: -
Anchor ductility: Yes
h_{min} (inch): 13.88
C_{min} (inch): 3.00
S_{min} (inch): 3.00

Base Material
Concrete: Normal-weight
Concrete thickness, h (inch): 40.00
State: Cracked
Compressive strength, f'_c (psi): 2500
$\Psi_{c,V}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental edge reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Ignore 6do requirement: No
Build-up grout pad: No

Base Plate
Length x Width x Thickness (inch): 12.00 x 12.00 x 0.38
Yield stress: 36000 psi

Profile type/size: 3X3X1/8

Recommended Anchor
Anchor Name: PAB Pre-Assembled Anchor Bolt - PAB4 (1/2"Ø)

Prior to installation:
Review anchor product's ICC-ES Report and install the product per the report. If the report states special inspection(s) are required - the final special inspection report must be on site during City inspections.

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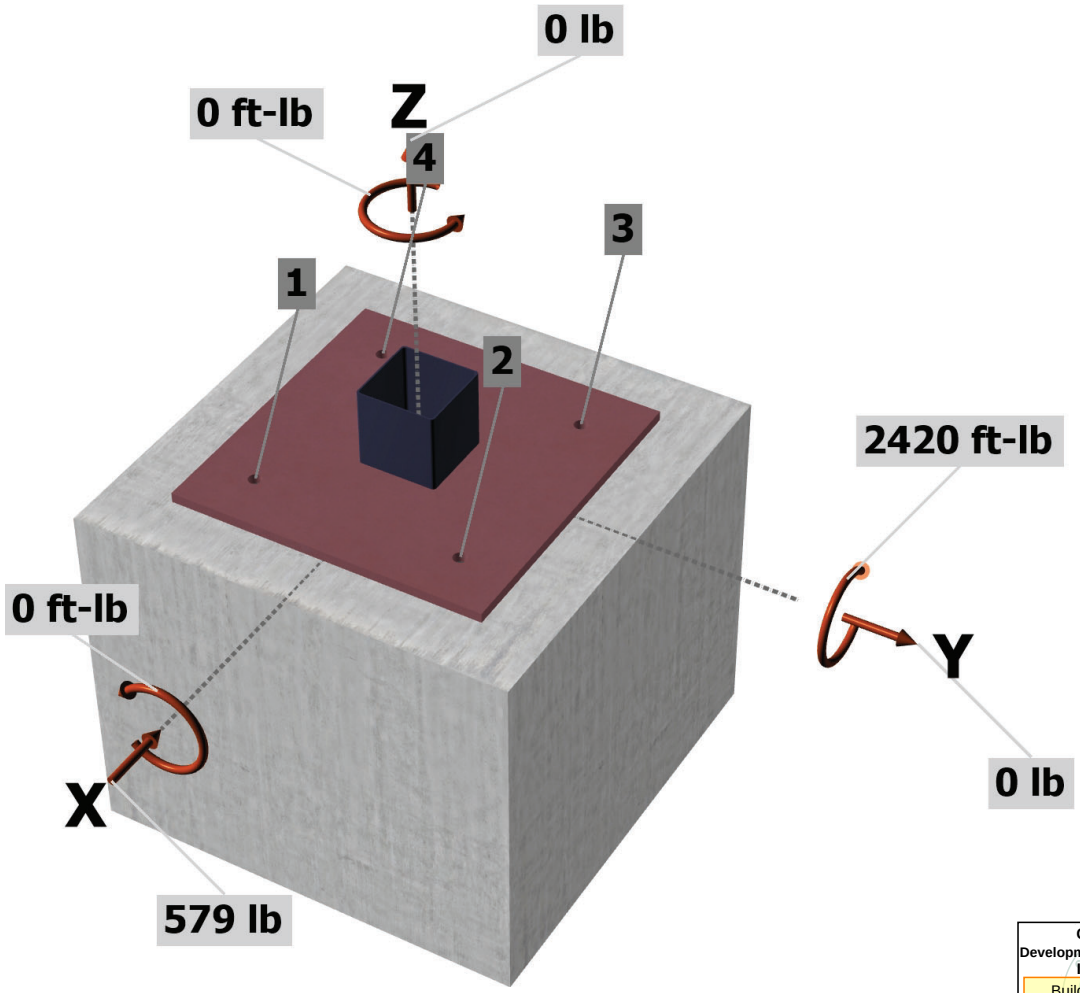
Load and Geometry
Load factor source: ACI 318 Section 5.3
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: Not applicable
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]:	0
V_{uax} [lb]:	-579
V_{uay} [lb]:	0
M_{ux} [ft-lb]:	0
M_{uy} [ft-lb]:	-2420
M_{uz} [ft-lb]:	0

<Figure 1>



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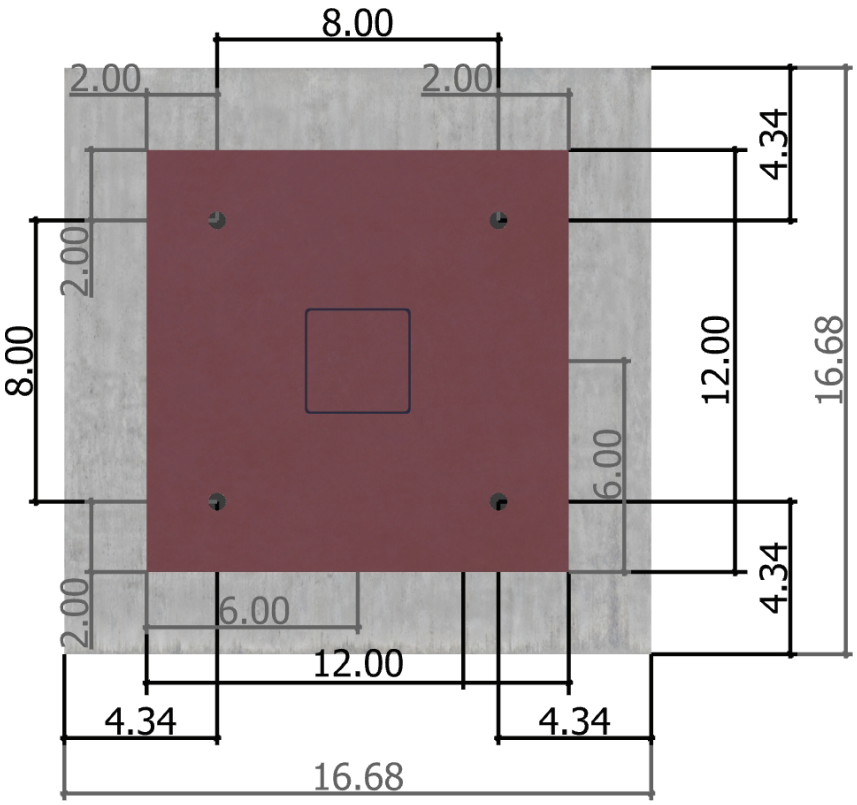
<Figure 2>

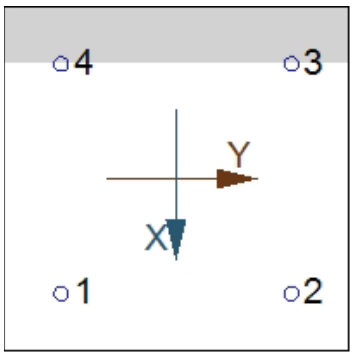


3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2+(V_{uay})^2}$ (lb)
1	1552.8	-144.8	0.0	144.8
2	1552.8	-144.8	0.0	144.8
3	5.8	-144.8	0.0	144.8
4	5.8	-144.8	0.0	144.8
Sum	3117.0	-579.0	0.0	579.0

Maximum concrete compression strain (‰): 0.06
Maximum concrete compression stress (psi): 264
Resultant tension force (lb): 3117
Resultant compression force (lb): 3117
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 3.97
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



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4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8235	0.75	6176

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$N_b = 16\lambda_a\sqrt{f'_c}h_{ef}^{5/3}$ (Eq. 17.4.2.2b)

λ_a	f'_c (psi)	h_{ef} (in)	N_b (lb)
1.00	2500	2.893	4700

$\phi N_{cbg} = \phi\,(A_{Nc} / A_{Nco})\,\Psi_{ec,N}\,\Psi_{ed,N}\,\Psi_{c,N}\,\Psi_{cp,N}N_b$ (Sec. 17.3.1 & Eq. 17.4.2.1b)

A_{Nc} (in^2)	A_{Nco} (in^2)	$c_{a,min}$ (in)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
278.22	75.34	4.34	0.522	1.000	1.00	1.000	4700	0.70	6345

6. Pullout Strength of Anchor in Tension (Sec. 17.4.3)

$\phi N_{pn} = \phi\Psi_{c,P}N_p = \phi\Psi_{c,P}8A_{brg}f'_c$ (Sec. 17.3.1, Eq. 17.4.3.1 & 17.4.3.4)

$\Psi_{c,P}$	A_{brg} (in^2)	f'_c (psi)	ϕ	ϕN_{pn} (lb)
1.0	1.57	2500	0.70	21994

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7. Side-Face Blowout Strength of Anchor in Tension (Sec. 17.4.4)

$\phi N_{sbg} = \phi\{(1+c_{a2}/c_{a1})/4\}(1+s/6c_{a1})N_{sb} = \phi\{(1+c_{a2}/c_{a1})/4\}(1+s/6c_{a1})(160c_{a1}\sqrt{A_{brg}})\lambda\sqrt{f'_c}$ (Sec. 17.3.1, Eq. 17.4.4.1 & 17.4.4.2)

s (in)	c_{a1} (in)	c_{a2} (in)	A_{brg} (in²)	λ_a	f'_c (psi)	ϕ	ϕN_{sbg} (lb)
8.00	4.34	4.34	1.57	1.00	2500	0.70	19911

8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4940	1.0	0.65	3211

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9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear perpendicular to edge in x-direction:

$V_{bx} = \min|7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.500	1.00	2500	12.34	16261

$\phi V_{cbgx} = \phi\,(A_{Vc}/A_{Vco})\,\Psi_{ec,V}\,\Psi_{ed,V}\,\Psi_{c,V}\,\Psi_{h,V}V_{bx}$ (Sec. 17.3.1 & Eq. 17.5.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
308.75	685.24	1.000	0.770	1.000	1.000	16261	0.70	3951

Shear parallel to edge in y-direction:

$V_{bx} = \min|7(l_e/d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.500	1.00	2500	4.34	3392

$\phi V_{cbgy} = \phi\,(2)(A_{Vc}/A_{Vco})\,\Psi_{ec,V}\,\Psi_{ed,V}\,\Psi_{c,V}\,\Psi_{h,V}V_{bx}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in²)	A_{Vco} (in²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgy} (lb)
108.59	84.76	1.000	1.000	1.000	1.000	3392	0.70	6083

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

$\phi V_{cpg} = \phi k_{cp}N_{cbg} = \phi k_{cp}(A_{Nc}/A_{Nco})\,\Psi_{ec,N}\,\Psi_{ed,N}\,\Psi_{c,N}\,\Psi_{cp,N}N_b$ (Sec. 17.3.1 & Eq. 17.5.3.1b)

k_{cp}	A_{Nc} (in²)	A_{Nco} (in²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕV_{cpg} (lb)
2.0	278.22	75.34	1.000	1.000	1.000	1.000	4700	0.70	24298

11. Results

Interaction of Tensile and Shear Forces (Sec. R17.6)

Tension	Factored Load, N_{ua} (lb)	Design Strength, øN_n (lb)	Ratio	Status
Steel	1553	6176	0.25	Pass
Concrete breakout	**3117**	**6345**	**0.49**	**Pass (Governs)**
Pullout	1553	21994	0.07	Pass
Side-face blowout	3106	19911	0.16	Pass

Shear	Factored Load, V_{ua} (lb)	Design Strength, øV_n (lb)	Ratio	Status
Steel	145	3211	0.05	Pass
T Concrete breakout x-	**579**	**3951**	**0.15**	**Pass (Governs)**
|| Concrete breakout y+	**290**	**6083**	**0.05**	**Pass (Governs)**
Pryout	579	24298	0.02	Pass

Interaction check	$(N_{ua}/\phi N_{ua})^{5/3}$	$(V_{ua}/\phi V_{ua})^{5/3}$	Utilization Ratio	Permissible	Status
Sec. R17.6	0.31	0.04	34.7%	1.0	Pass

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PAB4 (1/2"Ø) with hef = 12.000 inch meets the selected design criteria.

12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.

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