

1. STRUCTURAL NOTES
- 1.1. ANY DISCREPANCY FOUND AMONG THE DRAWINGS, SPECIFICATIONS, THESE NOTES, AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT AND THE STRUCTURAL ENGINEER, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S RISK. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION BRACING, FORMWORK AND TEMPORARY CONSTRUCTION SHORING.

1.2. BY THE ACT OF SUBMITTING A BID FOR THE PROPOSED CONTRACT, THE CONTRACTOR WARRANTS THAT:

1.2.1. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE (INCLUDING AGENTS AND SUPPLIERS) HAVE CAREFULLY AND THOROUGHLY REVIEWED THE DRAWINGS AND STRUCTURAL NOTES AND HAVE FOUND THEM COMPLETE AND FREE FROM AMBIGUITIES AND SUFFICIENT FOR THE PURPOSE INTENDED.

1.2.2. THE CONTRACTOR HAS CAREFULLY EXAMINED THE SITE OF THE WORK AND FROM THEIR OWN INVESTIGATIONS, THEY HAVE SATISFIED THEMSELVES AS TO THE NATURE AND LOCATION OF THE WORK, AS TO THE CHARACTER, QUALITY, AND QUANTITIES OF MATERIAL AND DIFFICULTIES TO BE ENCOUNTERED, AS TO THE EXTENT OF EQUIPMENT AND OTHER FACILITIES NEEDED FOR THE PERFORMANCE OF THE WORK AND AS TO THE GENERAL AND LOCAL CONDITIONS, AND OTHER ITEMS WHICH MAY IN ANY WAY AFFECT THE WORK OR ITS PERFORMANCE.

1.2.3. THE CONTRACTOR AND ALL WORKERS THEY INTEND TO USE ARE SKILLED AND EXPERIENCED IN THE TYPE OF CONSTRUCTION REPRESENTED BY THE DRAWINGS AND DOCUMENTS BID UPON.

1.2.4. NEITHER THE CONTRACTOR NOR ANY OF THEIR EMPLOYEES, AGENTS, INTENDED SUPPLIERS, OR SUBCONTRACTORS HAVE RELIED UPON ANY VERBAL REPRESENTATIONS ALLEGEDLY AUTHORIZED OR UNAUTHORIZED FROM THE OWNER OR THEIR EMPLOYEES OR AGENTS, INCLUDING THE ARCHITECT OR ENGINEERS, IN ASSEMBLING THE BID FIGURES.

1.2.5. THE REQUIREMENTS CONTAINED WITHIN THIS SECTION SUPERSEDE REQUIREMENTS AND/OR RECOMMENDATIONS CONTAINED IN THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES", AS WELL AS CASE DOCUMENT 962-D "A GUIDELINE ADDRESSING COORDINATION AND COMPLETENESS OF STRUCTURAL CONSTRUCTION DOCUMENTS".

1.2.6. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE ARE AWARE OF AND ACKNOWLEDGE THAT CLOSE COORDINATION AMONG ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS IS REQUIRED.

1.2.7. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE SHALL RECOGNIZE THAT THE PROJECT CONTRACT DOCUMENTS INCLUDE THE ARCHITECTURAL, STRUCTURAL, MECHANICAL AND ELECTRICAL AND OTHER TRADE DRAWINGS AND SPECIFICATIONS.

1.2.8. CONTRACTOR AND ALL SUBCONTRACTORS ACKNOWLEDGE THAT CLOSE COORDINATION BETWEEN DISCIPLINES INCLUDED WITHIN THE CONTRACT DOCUMENTS IS NECESSARY. ELEMENTS THAT WILL REQUIRE CLOSE COORDINATION BY THE CONTRACTOR INCLUDE (BUT ARE NOT LIMITED TO):

A. VERIFICATION OF ALL DIMENSIONS INDICATED ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS

B. DETERMINATION OF ALL COLUMN LOCATIONS

C. DETERMINATION OF TOP OF FLOOR, TOP OF STEEL WALL PLATE AND/OR TOP OF BEAM ELEVATIONS

D. DETERMINATION OF TOP OF FOOTING ELEVATIONS AND FOOTING STEP LOCATIONS

E. MECHANICAL/ELECTRICAL EQUIPMENT LOCATIONS AND WEIGHTS

F. LOCATION AND SIZE OF ALL MECHANICAL/ELECTRICAL PENETRATIONS THROUGH WALLS AND FLOORS/ROOFS

G. COORDINATION WITH DESIGNERS' SUPPLIERS OF PRE-ENGINEERED COMPONENTS (JOISTS, TRUSSES, STAIRS, ETC.)

1.2.9. THE CONTRACTOR ACKNOWLEDGES THAT TEMPORARY SHORING AND/OR BRACING MAY BE REQUIRED TO COMPLETE THE PROJECT, DESIGN AND IMPLEMENTATION OF TEMPORARY SHORING AND/OR BRACING DURING CONSTRUCTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

1.2.10. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE SHALL MAKE CONSIDERATION FOR, AND INCLUDE MONIES FOR THE ABOVE IN THE PREPARATION OF THEIR BIDS.

1.2.11. THE CONTRACTOR SHALL NOT SCALE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR LOCATIONS OF ELEMENTS NOTED ABOVE.

1.2.12. ELECTRONIC COPIES OF THE STRUCTURAL DRAWINGS (PDF'S, CAD DRAWINGS OR BIM MODELS) MAY BE PROVIDED TO THE CONTRACTOR FOR THEIR USE. THESE FILES MAY BE PROVIDED AT THE REQUEST OF THE CONTRACTOR FOR THEIR CONVENIENCE ONLY. THE CONTRACTOR AGREES THAT THESE FILES SHALL NOT SUPERSEDE INFORMATION SHOWN ON THE ORIGINAL BID CONSTRUCTION DOCUMENTS. THE CONTRACTOR AGREES TO HOLD THE STRUCTURAL ENGINEER HARMLESS FOR ANY ERRORS OR DISCREPANCIES CONTAINED WITHIN THESE ELECTRONIC FILES.

1.2.13. THE BID FIGURE IS BASED SOLELY UPON THE CONSTRUCTION CONTRACT DOCUMENTS AND PROPERLY ISSUED WRITTEN OR VERBAL REPRESENTATIONS.

1.3. CODES

1.3.1. ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2021 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING AUTHORITY.

1.3.2. ALL REFERENCES TO OTHER CODES, STANDARDS AND SPECIFICATIONS (ACI, ASTM, ETC.) SHALL BE FOR THE EDITION CURRENTLY REFERENCED BY IBC AS AMENDED AND ADOPTED BY THE LOCAL BUILDING AUTHORITY.

1.4. DESIGN CRITERIA

1.4.1. UNIFORM LOADS:

LOCATION	LIVE LOAD	DEAD LOAD
STAIRS AND EXITS	100 PSF	ACTUAL
HANDRAILS AND GUARDS	50 PLF OR 200# CONCENTRATED LOAD	

1.5. STATEMENT OF SPECIAL INSPECTIONS

SEE STATEMENT OF SPECIAL INSPECTION AND TESTING SHEET SWO.2.
- 1.6. SHOP DRAWINGS

1.6.1. SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR THE FOLLOWING:

CONCRETE MIX DESIGN SUBMITTALS

REINFORCING STEEL

1.6.2. SHOP DRAWING REVIEW NOTES

ENGINEER OF RECORD SHALL REVIEW SHOP DRAWINGS FOR GENERAL CONFORMANCE WITH THE PROJECT CONSTRUCTION DOCUMENTS (PLANS AND SPECIFICATIONS).

ENGINEER OF RECORD REVIEW OF SHOP DRAWINGS SHALL NOT RELIEVE THE GENERAL CONTRACTOR OF THEIR RESPONSIBILITY FOR REVIEW OF THE SHOP DRAWINGS FOR COMPLIANCE WITH THE PROJECT REQUIREMENTS.

APPROVAL OF THE SHOP DRAWINGS BY THE ENGINEER OF RECORD SHALL NOT BE CONSIDERED AS A GUARANTEE BY THE ENGINEER THAT THE SHOP DRAWINGS COMPLY WITH ALL PROJECT REQUIREMENTS.

CONCURRENT SHOP DRAWING REVIEW SHALL ONLY BE PERMITTED IF APPROVED BY THE ARCHITECT/ENGINEER OF RECORD PRIOR TO THE START OF SHOP DRAWING REVIEW.

1.7. MISCELLANEOUS

1.7.1. VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD.

1.7.2. CONSTRUCTION DETAILS NOT SPECIFICALLY SHOWN ON THE DRAWINGS SHALL FOLLOW SIMILAR DETAILS OF SECTIONS OF THIS PROJECT AS APPROVED BY THE ARCHITECT/ENGINEER.

1.7.3. THE STRUCTURE HAS BEEN DESIGNED TO RESIST CODE REQUIRED VERTICAL AND LATERAL FORCES AFTER THE CONSTRUCTION OF ALL STRUCTURAL ELEMENTS HAS BEEN COMPLETED. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO JOB SITE SAFETY, ERECTION MEANS, METHODS, AND SEQUENCES, TEMPORARY SHORING, FORMWORK, AND BRACING, USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES.

2. SITE PREPARATION/SOIL REMEDIATION

2.1. SOIL DATA

ALLOWABLE SOIL PRESSURE 3,000 PSF. ALLOW 33-1/3% INCREASE FOR LOADS FROM WIND OR SEISMIC ORIGIN. SEE GEOTECHNICAL ENGINEERING REPORT BY TERRA ASSOCIATES, INC. DATED NOVEMBER 14, 2019. SEE GEOTECH REPORT FOR ALL SUBGRADE PREPARATION REQUIREMENTS AS WELL AS CAPILLARY BREAK AND VAPOR BARRIER RECOMMENDATIONS.

2.1.1. RETAINING WALL DESIGN CRITERIA:

A. ACTIVE EARTH PRESSURE: 35 PCF

B. AT-REST EARTH PRESSURE: 100 PSF (UNIFORM)

C. SEISMIC EARTH PRESSURE: 8 x "H" PSF

D. PASSIVE EARTH PRESSURE: 350 PCF \*

E. FRICTION COEFFICIENT: 0.35 \*

F. \* INCLUDES FACTOR OF SAFETY OF 1.5

2.2. EXCAVATION

EXCAVATE TO DEPTH SHOWN AND TO FIRM UNDISTURBED MATERIAL. OVER-EXCAVATIONS SHALL BE BACKFILLED WITH LEAN CONCRETE (f<sub>c</sub>=500-1200 PSI) OR STRUCTURAL FILL AT THE CONTRACTORS EXPENSE. EXERCISE EXTREME CARE DURING EXCAVATION TO AVOID DAMAGE TO BURIED LINES, TANKS, AND OTHER CONCEALED ITEMS. UPON DISCOVERY, DO NOT PROCEED WITH WORK UNTIL RECEIVING WRITTEN INSTRUCTIONS FROM THE ARCHITECT. A COMPETENT REPRESENTATIVE OF THE OWNER SHALL INSPECT ALL FOOTING EXCAVATIONS FOR SUITABILITY OF BEARING SURFACES PRIOR TO PLACEMENT OF REINFORCING STEEL. PROVIDE DRAINAGE AS NECESSARY TO AVOID WATER-SOFTENED SUBGRADE.

2.3. FILL, BACKFILL AND COMPACTION

BACKFILL AGAINST WALLS SHALL NOT BE PLACED UNTIL AFTER THE REMOVAL OF ALL MATERIAL SUBJECT TO ROT OR CORROSION. ALL FILL PLACED AGAINST RETAINING WALLS OR BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL. STRUCTURAL FILL OTHER THAN PEA GRAVEL SHALL BE GRANULAR PLACED IN 6-INCH LIFTS AND COMPACTED TO AT LEAST 95% OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557 (MOD PROCTOR). PEA GRAVEL FILL SHALL HAVE A MAXIMUM PARTICLE SIZE OF 3/8" DIAMETER.

3. STRUCTURAL CONCRETE

3.1. GENERAL

ALL CONCRETE SHALL BE HARD ROCK CONCRETE MEETING THE REQUIREMENTS OF ACI-301. "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS." PROPORTIONING OF INGREDIENTS FOR EACH CONCRETE MIX SHALL BE BY METHOD 2 OR THE ALTERNATE PROCEDURE GIVEN IN ACI-301. PLACE CONCRETE PER ACI-304 AND CONFORM TO ACI-604 (208) FOR WINTER CONCRETING AND ACI-408 (09) FOR HOT WEATHER CONCRETING. USE INTERIOR MECHANICAL VIBRATORS WITH 7,000 RPM MINIMUM FREQUENCY. DO NOT OVER-VIBRATE. CONCRETE SHALL BE PLACED MONOLITHICALLY BETWEEN CONSTRUCTION OR CONTROL JOINTS. PROTECT ALL CONCRETE FROM PREMATURE DRYING, EXCESSIVE HOT OR COLD TEMPERATURE FOR SEVEN DAYS AFTER PLACING.

3.2. STRENGTH

TWENTY-EIGHT DAY COMPRESSIVE STRENGTHS (f<sub>c</sub>) SHALL BE AS FOLLOWS WITH EXPOSURE CATEGORY AND CLASS PER ACI TABLE 19.3.1.1 GIVEN IN PARENTHESIS:

FOOTINGS (F0/S0/W0/C1) 3000 PSI

VERTICALLY FORMED WALLS (F1/S0/W0/C0) 4000 PSI \*

\* MAXIMUM W/C RATIO SHALL BE 0.55

CONCRETE SUPPLIER TO PROVIDE TEST RECORDS PER SECTION 26.4 OF ACI 318. WHEN NO PRIOR EXPERIENCE OR TRIAL MIXTURE DATA ARE AVAILABLE, THE WATER/CEMENT RATIO FROM THE TABLE BELOW MAY BE USED, BUT ONLY WHEN SPECIAL PERMISSION IS GIVEN BY ENGINEER.

MAXIMUM ABSOLUTE WATER/CEMENT RATIO BY WEIGHT FOR CONCRETE MIXES WITHOUT TEST RECORDS SHALL BE AS FOLLOWS:

SPECIFIED COMPRESSIVE STRENGTH	NON-AIR ENTRAINED CONCRETE	AIR- ENTRAINED CONCRETE
3000 PSI	0.58	0.46
4000 PSI	0.44	0.35

3.3. MATERIALS

3.3.1. CEMENT: ASTM C150, TYPE I OR TYPE II OR ASTM C595 TYPE II, ENGINEER'S APPROVAL, IS NEEDED FOR USE OF TYPE III CEMENT.

3.3.2. COARSE AND FINE AGGREGATE: ASTM C33.

3.3.3. WATER SHALL BE CLEAN AND POTABLE.

3.3.4. FLYASH: ASTM C618 CLASS C (CLASS F MAY BE ALLOWED IF APPROVED BY THE STRUCTURAL ENGINEER)

3.3.5. GROUND GRANULATED BLAST FURNACE SLAG (GGBS): ASTM C899 GRADE 100 OR 120. GGBFS SHALL NOT BE PERMITTED UNLESS REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER. MIX DESIGNS SUBMITTED INCLUDING GGBFS SHALL INCLUDE SHRINKAGE TEST RESULTS AT 28 DAYS.

3.4. ADMIXTURES

3.4.1. WATER REDUCING ADMIXTURE: ASTM C494. ADMIXTURES SHALL BE USED IN EXACT ACCORDANCE WITH MANUFACTURERS' INSTRUCTIONS.

3.4.2. WATER REDUCING ADMIXTURES SHALL BE USED AT ALL HEAVILY CONGESTED AREAS (I.E. CONCRETE BEAMS, COLUMNS AND WALLS WITH REINFORCING SPACING OF 4" OR LESS)

3.4.3. CONCRETE USING ADMIXTURES TO PRODUCE FLOWABLE CONCRETE MAY BE USED SUBJECT TO ENGINEER'S APPROVAL.

3.4.4. AIR ENTRAINMENT: ASTM C260 AND ASTM C494 ENTRAIN 5% PLUS/MINUS 1.5% BY VOLUME IN ALL CONCRETE EXPOSED TO WEATHER.

3.4.5. NO OTHER ADMIXTURES PERMITTED UNLESS APPROVED BY THE ENGINEER.

3.5. FORMWORK AND SHORING

3.5.1. FOLLOW RECOMMENDED PRACTICE FOR CONCRETE FORMWORK (ACI-347).

3.5.2. ALL SHORING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. FORMWORK SUPPORTS AND SHORING SHALL BE COVERED PRIOR TO PROVIDING FINISHED CONCRETE SURFACES AT ALL FACES LEVEL, PLUMB AND TRUE TO THE DIMENSIONS AND ELEVATIONS SHOWN. TOLERANCES AND VARIATIONS SHALL BE AS SPECIFIED.

3.6. REINFORCING STEEL:

3.6.1. DETAIL, FABRICATE, AND PLACE PER ACI-315 AND ACI-318. SUPPORT REINFORCEMENT WITH APPROVED CHAIRS, SPACERS, OR TIES.

3.6.2. DEFORMED BAR REINFORCEMENT: ASTM A615 GR 60

3.6.3. WELDABLE DEFORMED BAR REINFORCEMENT: ASTM A706 GR 60 WHERE NOTED ON STRUCTURAL DRAWINGS

3.6.4. EXCEPT AS NOTED SPECIFICALLY ON THE DRAWINGS, ALL CONCRETE REINFORCEMENT SHALL BE LAP-SPLICED AS INDICATED ON THE REINFORCING BAR DEVELOPMENT AND SPLICE LENGTH SCHEDULE PROVIDED ON THE STRUCTURAL DRAWINGS; NO MORE THAN 50% OF HORIZONTAL OR VERTICAL REINFORCING BARS SHALL BE SPLICED AT ANY ONE LOCATION.

3.6.5. EXCEPT AS NOTED SPECIFICALLY ON THE DRAWINGS, PROVIDE CORNER BARS TO MATCH QUANTITY AND DIAMETER OF HORIZONTAL REINFORCEMENT AND LAP WITH SPECIFIED HORIZONTAL REINFORCEMENT FOR 1'-1" PER REINFORCING BAR DEVELOPMENT AND SPLICE LENGTH TABLES PROVIDED ON THE STRUCTURAL DRAWINGS. THESE CORNER BARS SHALL BE PLACED AT ALL CORNERS AND INTERSECTIONS IN CONCRETE FOOTINGS AND WALLS.

3.7. CONCRETE COVER ON REINFORCING SHALL BE AS FOLLOWS (UNLESS SHOWN OTHERWISE):

BOTTOM OF FOOTINGS 3"

FORMED EARTH FACE AND SLAB ON GRADE 2"

WALLS, WEATHER FACE 1-1/2"

3.8. CONSTRUCTION OR CONTROL JOINTS

3.8.1. CONSTRUCTION OR CONTROL JOINT SPACING IN WALLS SHALL NOT EXCEED 50' ON CENTER EXCEPT AS DIRECTED BY THE ARCHITECT/ENGINEER.

3.9. SHOTCRETE

3.9.1. SHOTCRETE SHALL BE DEFINED AS MORTAR OR CONCRETE PNEUMATICALLY PROJECTED AT HIGH VELOCITY ONTO A SURFACE, EXCEPT AS SPECIFIED IN THIS SECTION. SHOTCRETE SHALL CONFORM TO THE REQUIREMENTS FOR PLAIN CONCRETE OR REINFORCED CONCRETE.

3.9.2. PROPORTIONS AND MATERIALS: SHOTCRETE PROPORTIONS SHALL BE SELECTED THAT ALLOW SUITABLE PLACEMENT PROCEDURES USING THE DELIVERY EQUIPMENT SELECTED AND SHALL RESULT IN FINISHED IN-PLACE HARDENED SHOTCRETE MEETING THE SPECIFIED STRENGTH REQUIREMENTS.

3.9.3. AGGREGATE: COARSE AGGREGATE, IF USED, SHALL NOT EXCEED 3/4 INCH.

3.9.4. REINFORCEMENT: LAP SPLICES IN REINFORCING BARS SHALL BE BY THE NON-CONTACT LAP SPLICE METHOD WITH AT LEAST 2 INCHES CLEARANCE BETWEEN BARS. THE BUILDING OFFICIAL MAY PERMIT THE USE OF CONTACT LAP SPLICES WHEN NECESSARY FOR THE SUPPORT OF THE REINFORCING PROVIDED IT CAN BE DEMONSTRATED BY MEANS OF PRE-CONSTRUCTION TESTING THAT ADEQUATE ENCASEMENT OF THE BARS AT THE SPLICE CAN BE ACHIEVED, AND PROVIDED THAT THE SPLICES ARE PLACED SO THAT A LINE THROUGH THE CENTER OF THE TWO SPLICED BARS IS PERPENDICULAR TO THE SURFACE OF THE SHOTCRETE WORK.

3.9.5. PRE-CONSTRUCTION TESTS: WHEN REQUIRED BY THE SPECIFICATIONS OR BUILDING OFFICIAL, A TEST PANEL SHALL BE SHOT, CURED, CORED OR SAWN, EXAMINED AND TESTED PRIOR TO COMMENCEMENT OF THE PROJECT. THE SAMPLE PANEL SHALL BE REPRESENTATIVE OF THE PROJECT AND SIMULATE JOINT CONDITIONS AS CLOSELY AS POSSIBLE. THE PANEL THICKNESS AND REINFORCING SHALL REPRODUCE THE THICKEST AND MOST CONGESTED AREA SPECIFIED IN THE STRUCTURAL DESIGN. IT SHALL BE SHOT AT THE SAME ANGLE, USING THE SAME NOZZLE/MAN AND WITH THE SAME CONCRETE MIX DESIGN THAT WILL BE USED ON THE PROJECT.

3.9.6. REBOUND: ANY REBOUND OR ACCUMULATED LOOSE AGGREGATE SHALL BE REMOVED FROM THE SURFACES TO BE COVERED PRIOR TO PLACING THE INITIAL OR ANY SUCCEEDING LAYERS OF SHOTCRETE. REBOUND SHALL NOT BE REUSED AS AGGREGATE.

3.9.7. JOINTS: EXCEPT WHERE PERMITTED HEREIN, UNFINISHED WORK SHALL NOT BE ALLOWED TO STAND FOR MORE THAN 30 MINUTES UNLESS ALL EDGES ARE SLOPED TO A THIN EDGE. BEFORE PLACING ADDITIONAL MATERIAL ADJACENT TO PREVIOUSLY APPLIED WORK, SLOPING AND SQUARE EDGES SHALL BE CLEANED AND WETTED WITH A CONCRETE BONDING AGENT AS APPROPRIATE.

3.9.8. DAMAGE: IN-PLACE SHOTCRETE WHICH EXHIBITS SAGS OR SLOUGHS, SEGREGATION, HONEYCOMING, SAND POCKETS OR OTHER OBVIOUS DEFECTS SHALL BE REMOVED AND REPLACED. SHOTCRETE ABOVE SAGS AND SLOUGHS SHALL BE REMOVED AND REPLACED WHILE STILL PLASTIC.

3.9.9. CURING: DURING THE CURING PERIODS SPECIFIED HEREIN, SHOTCRETE SHALL BE MAINTAINED ABOVE 40° F, AND IN MOIST CONDITION. IN INITIAL CURING, SHOTCRETE SHALL BE KEPT CONTINUOUSLY MOIST FOR 24 HOURS AFTER PLACEMENT IS COMPLETE. FINAL CURING SHALL CONTINUE FOR SEVEN DAYS AFTER SHOTCRETING. FOR THREE DAYS IF HIGH-EARLY-STRENGTH CEMENT IS USED, OR UNTIL THE SPECIFIED STRENGTH IS OBTAINED. FINAL CURING SHALL CONSIST OF A FOG SPRAY OR AN APPROVED MOISTURE-RETAINING COVER OR MEMBRANE. IN SECTIONS WITH A DEPTH IN EXCESS OF 12", FINAL CURING SHALL BE THE SAME AS THAT FOR INITIAL CURING.

3.9.10. STRENGTH TEST: STRENGTH TEST FOR SHOTCRETE SHALL BE MADE BY AN APPROVED AGENCY ON SPECIMENS WHICH ARE REPRESENTATIVE OF WORK AND WHICH HAVE BEEN WATER SOAKED FOR AT LEAST 24 HOURS PRIOR TO TESTING. WHEN THE MAXIMUM SIZE AGGREGATE IS LARGER THAN 3/8" SPECIMENS SHALL CONSIST OF NOT LESS THAN (3) 3" DIAMETER CORES OR 3-INCH CUBES. WHEN THE MAXIMUM SIZE AGGREGATE IS 3/8" OR SMALLER, SPECIMENS SHALL CONSIST OF NOT LESS THAN (3) 2" DIAMETER CORES OR 2" CUBES. SPECIMENS SHALL BE TAKEN IN ACCORDANCE WITH ONE OF THE FOLLOWING:

A. FROM THE IN-PLACE WORK: TAKEN AT LEAST ONCE EACH SHIFT BUT NOT LESS THAN ONE FOR EACH 50 CUBIC YARDS OF SHOTCRETE, OR

B. FROM TEST PANELS: MADE NOT LESS THAN ONCE EACH SHIFT OR NOT LESS THAN ONE FOR EACH 50 CUBIC YARDS OF SHOTCRETE PLACED. WHEN THE MAXIMUM SIZE AGGREGATE IS LARGER THAN 3/8", THE TEST PANELS SHALL HAVE A MINIMUM DIMENSION OF 18" X 18". WHEN THE MAXIMUM SIZE AGGREGATE IS 3/8" OR SMALLER, THE TEST PANELS SHALL HAVE A MINIMUM DIMENSION OF 12" X 12". PANELS SHALL BE GUNNED IN THE SAME POSITION AS THE WORK, DURING THE COURSE OF THE WORK AND BY NOZZLE/MEN DOING THE WORK. THE CONDITION UNDER WHICH THE PANELS ARE CURED SHALL BE THE SAME AS THE WORK.

C. THE AVERAGE OF THREE CORES FROM A SINGLE PANEL SHALL BE EQUAL TO OR EXCEED 0.85 f<sub>c</sub> WITH NO SINGLE CORE LESS THAN 0.75 f<sub>c</sub>. THE AVERAGE OF THREE CUBES TAKEN FROM A SINGLE PANEL MUST EQUAL OR EXCEED f<sub>c</sub> WITH NO INDIVIDUAL CUBE LESS THAN 0.85 f<sub>c</sub> TO CHECK TESTING ACCURACY. LOCATIONS REPRESENTED BY ERRATIC CORE STRENGTHS MAY BE RETESTED.

3.9.11. INSPECTIONS

DURING PLACEMENT: WHEN SHOTCRETE IS USED FOR STRUCTURAL MEMBERS, A SPECIAL INSPECTOR IS REQUIRED BY IBC TABLE 1706.3. THE SPECIAL INSPECTOR SHALL PROVIDE CONTINUOUS INSPECTION OF THE PLACEMENT OF THE REINFORCEMENT AND SHOTCRETING AND SHALL SUBMIT A STATEMENT INDICATING COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.

VISUAL EXAMINATION FOR STRUCTURAL SOUNDNESS OF IN-PLACE SHOTCRETE: COMPLETED SHOTCRETE WORK SHALL BE CHECKED VISUALLY FOR REINFORCING BAR EMBEDMENT, VOIDS, ROCK POCKETS, SAND STREAKS AND SIMILAR DEFICIENCIES BY EXAMINING A MINIMUM OF (3) 3" CORES TAKEN FROM (3) AREAS CHOSEN BY THE DESIGN ENGINEER WHICH REPRESENT THE WORST CONGESTION OF REINFORCING BARS OCCURRING IN THE PROJECT. EXTRA REINFORCING BARS MAY BE ADDED TO NON-CONGESTED AREAS TO REPRESENT THE MOST HEAVILY CONGESTED AREAS ELSEWHERE ON THE PROJECT. THE CORES SHALL BE EXAMINED BY THE SPECIAL INSPECTOR AND A REPORT SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO FINAL APPROVAL OF THE SHOTCRETE.

TESTING EQUIPMENT: THE EQUIPMENT USED IN PRE-CONSTRUCTION TESTING SHALL BE THE SAME EQUIPMENT USED IN THE WORK REQUIRING SUCH TESTING, UNLESS SUBSTITUTE EQUIPMENT IS APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING OFFICIAL.

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.


The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permittee on site for inspection.

Separate Electrical Permit is required with the Washington State Department of Labor & Industries.  
<https://lni.wa.gov/licensing-permits/electrical/electrical-permits-fees-and-inspections> or call for Licensing Information: 1-800-647-0982

City of Puyallup  
Building  
REVIEWED  
FOR  
COMPLIANCE

SKinnear  
04/15/2025  
8:30:40 AM



PRRWF20250002

PERMIT SET  
DECEMBER 2,  
2024

ORIGINAL ISSUE: 08/17/16

REVISIONS

No.	Description	Date
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2220236.20  
PROJECT NUMBER

Author	Checker
DRAWN BY	CHECKED BY

WESLEY BRADLEY PARK  
PHASE 2 - CARE CENTER

STRUCTURAL NOTES

SWO.1

NOTICE:  
AS DIRECTOR OF THIS DOCUMENT SHALL VALIDATE THE PROFESSIONAL AND ENGINEERING REGISTRATION OF THE SIGNATURES OF THE DESIGN PROFESSIONAL PERSONNEL ISSUED BY THE LOCAL AGENCY OF USE ON THE PROJECT AND THAT THE DESIGN PROFESSIONAL PERSONNEL ARE CURRENTLY LICENSED IN THE STATE OF WASHINGTON.

WESLEY BRADLEY PARK  
PHASE 2 - CARE CENTER  
707 39TH AVENUE SE  
PUYALLUP, WA 98374



11. STATEMENT OF SPECIAL INSPECTIONS			
IBC	SI	SO	TITLE
1705.3	✓	✓	CONCRETE CONSTRUCTION (SEE TABLE 13)
1705.6	✓	N/R	SOILS (SEE TABLE 12A)

SI = SPECIAL INSPECTION  
SO = STRUCTURAL OBSERVATION  
✓ = ITEM IS REQUIRED  
N/R = ITEM IS NOT REQUIRED  
SPECIAL INSPECTIONS INDICATED ARE FOR STRUCTURAL ELEMENTS ONLY. SEE ARCH, MECH AND ELEC DRAWINGS FOR ADDITIONAL SPECIAL INSPECTIONS.

- 11.
- 11.1. INSPECTION/TESTING REQUIREMENTS:  
SEE DRAWINGS, SPECIFICATIONS, AND IBC SECTIONS 110, AND CHAPTER 17.
- 11.2. INSPECTIONS BY THE BUILDING OFFICIAL (IBC SECTION 110):
- 11.2.1. FOOTING AND FOUNDATION INSPECTIONS SHALL BE MADE AFTER EXCAVATIONS ARE COMPLETE AND ANY REQUIRED REINFORCING IS IN PLACE. ANY REQUIRED FORMS SHALL BE IN PLACE PRIOR TO INSPECTION.
- 11.2.2. IN ADDITION TO THE INSPECTIONS SPECIFIED ABOVE, THE BUILDING OFFICIAL IS AUTHORIZED TO MAKE OR REQUIRE OTHER INSPECTIONS OF ANY CONSTRUCTION WORK TO ASCERTAIN COMPLIANCE WITH THE PROVISIONS OF THE IBC OR OTHER LAWS ENFORCED BY THE BUILDING OFFICIAL.
- 11.3. STRUCTURAL TESTS AND SPECIAL INSPECTIONS (IBC CHAPTER 17):
- 11.3.1. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- 11.3.2. STRUCTURAL TESTS AND SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE IBC AS WELL AS ANY ADDITIONAL REQUIREMENTS OF THE BUILDING OFFICIAL. OMISSION FROM THE LIST BELOW OF TESTING AND INSPECTION REQUIREMENTS SHALL NOT RELIEVE THE CONTRACTOR FROM PROVIDING TESTING AND INSPECTION REQUIRED BY THE SPECIFICATIONS, THE IBC AND THE BUILDING OFFICIAL.
- 11.3.3. TESTING AND SPECIAL INSPECTIONS SHALL BE COMPLETED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE IBC FOR THE ITEMS LISTED IN THIS SECTION.
- 11.4. STRUCTURAL OBSERVATION
- 11.4.1. STRUCTURAL OBSERVATION MAY BE PERFORMED DURING CONSTRUCTION IN A MANNER AS REQUIRED TO BECOME GENERALLY FAMILIAR WITH THE IN-PLACE CONSTRUCTION.
- 11.4.2. STRUCTURAL OBSERVATION EXTENT SHALL BE AS INDICATED ABOVE. TIMING AND DURATION OF OBSERVATIONS SHALL BE COORDINATED WITH THE GENERAL CONTRACTOR DURING CONSTRUCTION.
- 11.4.3. CONSTRUCTION OBSERVATION REPORTS AND FINDINGS SHALL NOT BE VIEWED AS A WARRANTY OR GUARANTEE BY THE STRUCTURAL ENGINEER.
- 11.5. SPECIAL INSPECTOR: SHALL BE CURRENTLY WABO CERTIFIED.
- 11.5.1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.
- 11.5.2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, ENGINEER OF RECORD, ARCHITECT OF RECORD, AND OTHER DESIGNATED PERSONS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE GENERAL CONTRACTOR FOR CORRECTION, THEN, IF NOT IN CONFORMANCE, TO THE PROPER DESIGN AUTHORITY AND BUILDING OFFICIAL.
- 11.5.3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.

12A. REQUIRED SPECIAL INSPECTIONS AND TEST OF SOILS		
IBC TABLE 1705.6		
SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	N/R	✓
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	N/R	✓
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIAL	N/R	✓
4. DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	✓	N/R
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	N/R	✓

- 12.
- 12.1. SPECIAL INSPECTIONS AND TESTS FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT, AND LOAD-BEARING REQUIREMENTS PER IBC 1705.6., AS NOTED IN TABLE 12A.
- 12.1.1. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONALS SHALL BE USED TO DETERMINE COMPLIANCE.

13. REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION				
IBC TABLE 1705.3				
SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	N/R	✓	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	
2. REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706 B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" C. INSPECT ALL OTHER WELDS	N/R N/R ✓	✓ ✓ N/R	AWS D1.4 ACI 318:26.6.4	
3. INSPECT ANCHORS CAST IN CONCRETE	N/R	✓	ACI 318: 17.8.2	
4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN A.	✓ N/R	N/R ✓	ACI 318: 17.8.2.4 ACI 318: 17.8.2	
5. VERIFY USE OF REQUIRED DESIGN MIX	N/R	✓	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	✓	N/R	ASTM C 172 ASTM C 31 ACI318:26.4, 26.12	
7. INSPECT CONCRETE AND SHOTORETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	✓	N/R	ACI 318: 26.5	
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	N/R	✓	ACI 318: 26.5.3-26.5.5	
14. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	N/R	✓	ACI 318: 26.11.1.2(b)	

- 13.
- 13.1. CONCRETE: SPECIAL INSPECTION AND TESTING PER IBC TABLE 1705.3 AS NOTED IN TABLE 13, INCLUDING:
- 13.1.1. SHOTORETE: SEE STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
- 13.1.2. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IPMO, ETC.).

CONTRACTOR NOTE

Special inspection reports shall be available and presented at time of inspection by the City of Puyallup.



in site architects

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PRRWF20250002

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DECEMBER 2,  
2024

ORIGINAL ISSUE: 08/17/16

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No. Description Date

2220236.20  
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TESTING AND INSPECTION  
NOTES

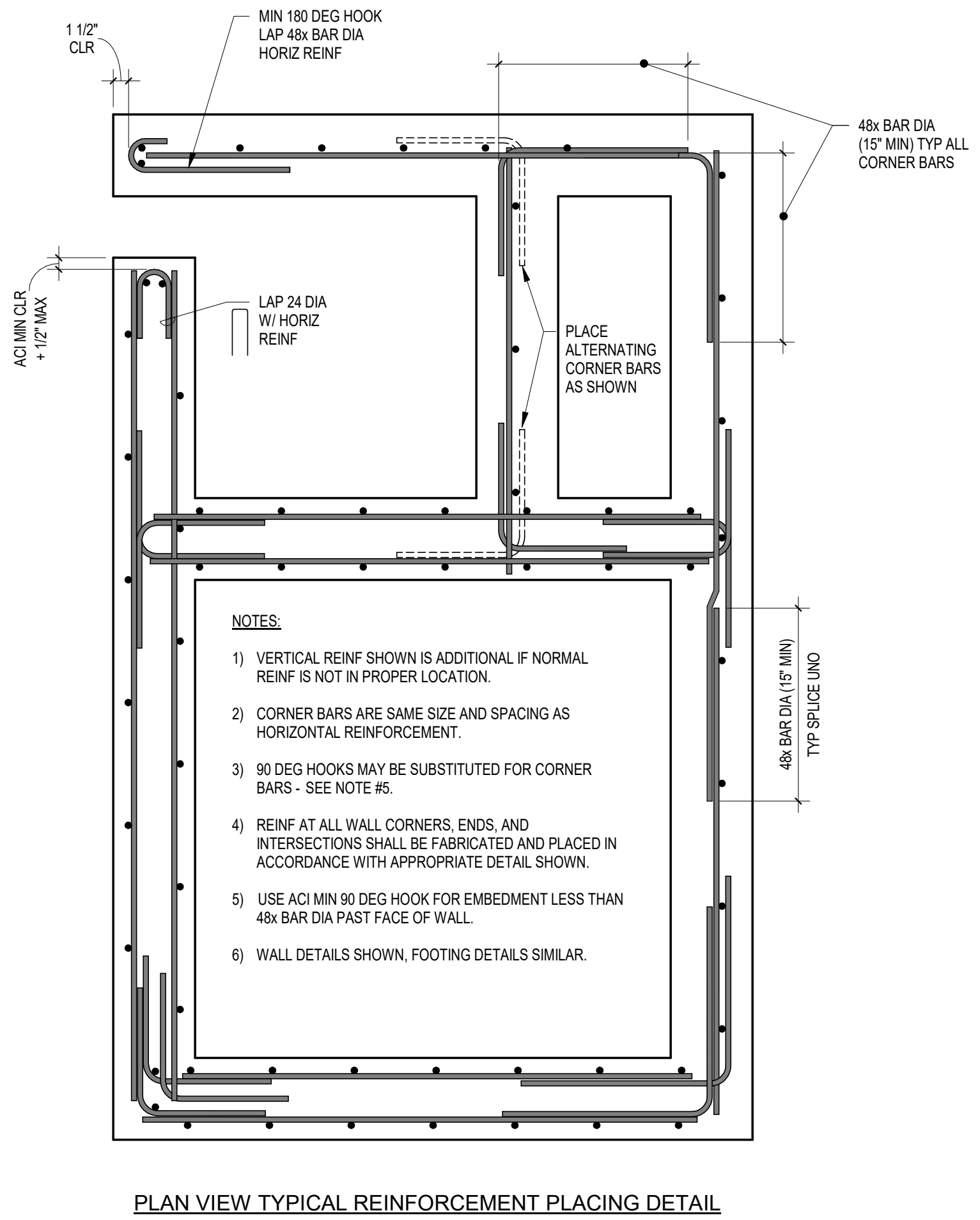
SW0.2



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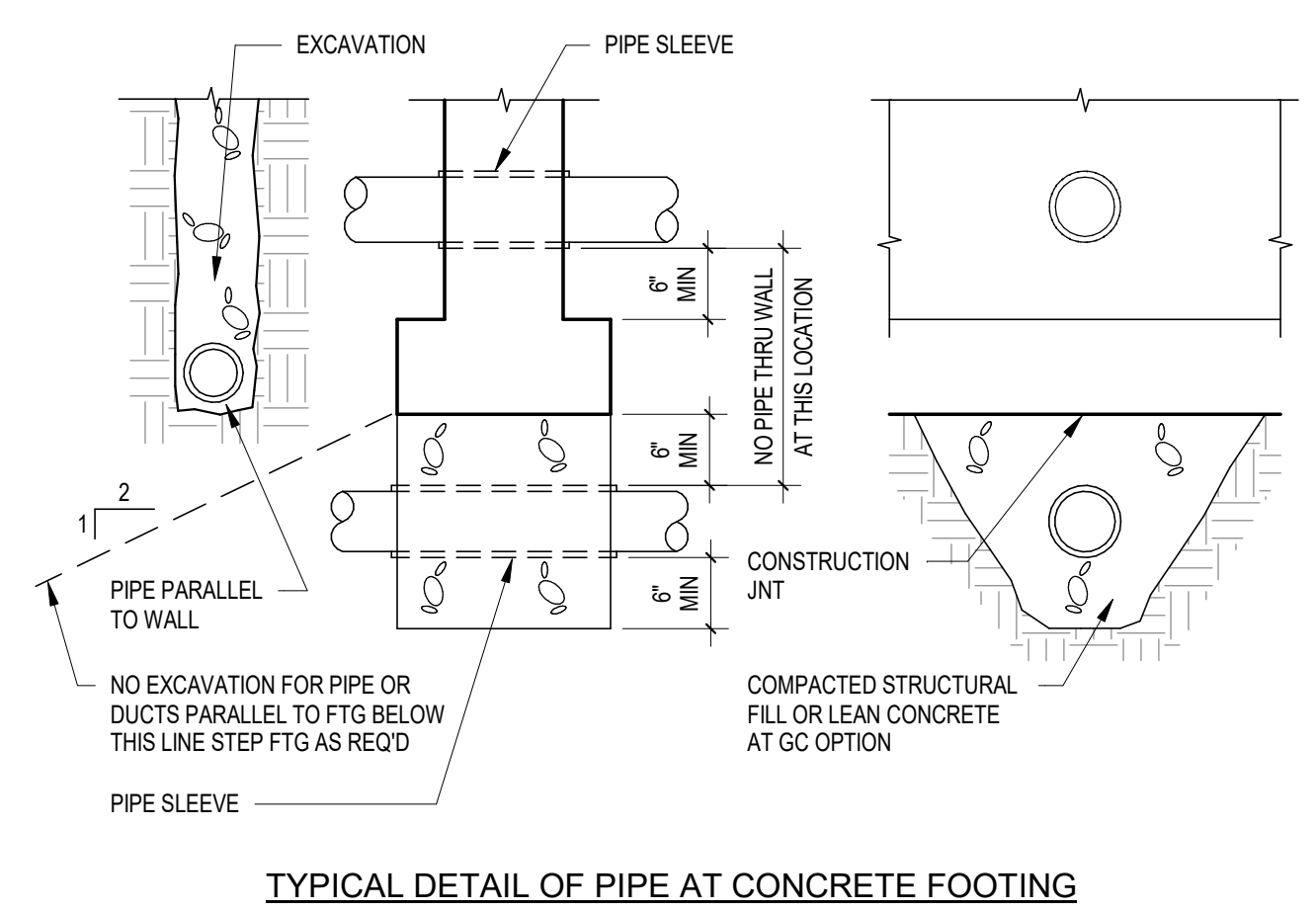


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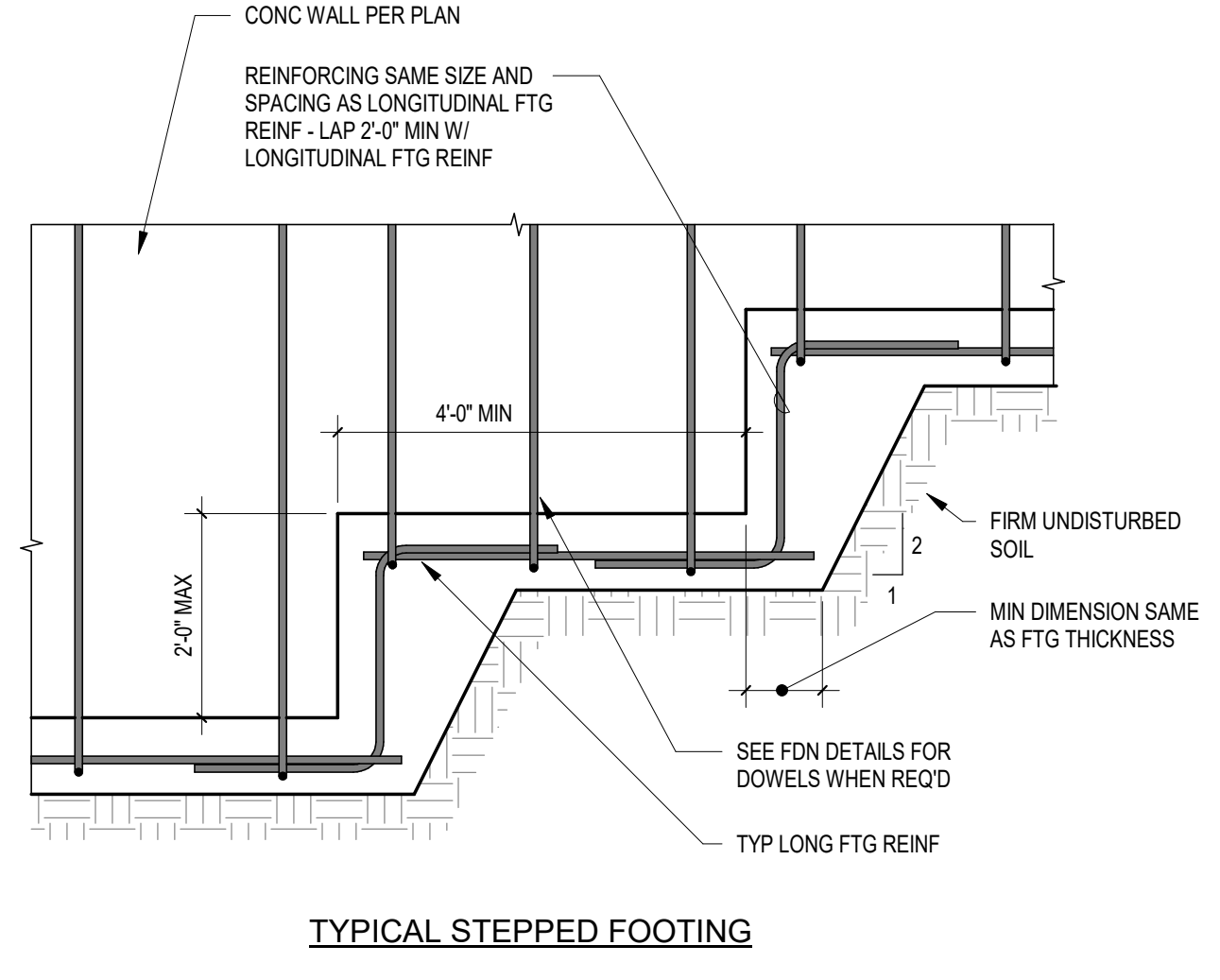


**1 TYPICAL**  
1" = 1'-0" 1/ SW0.3

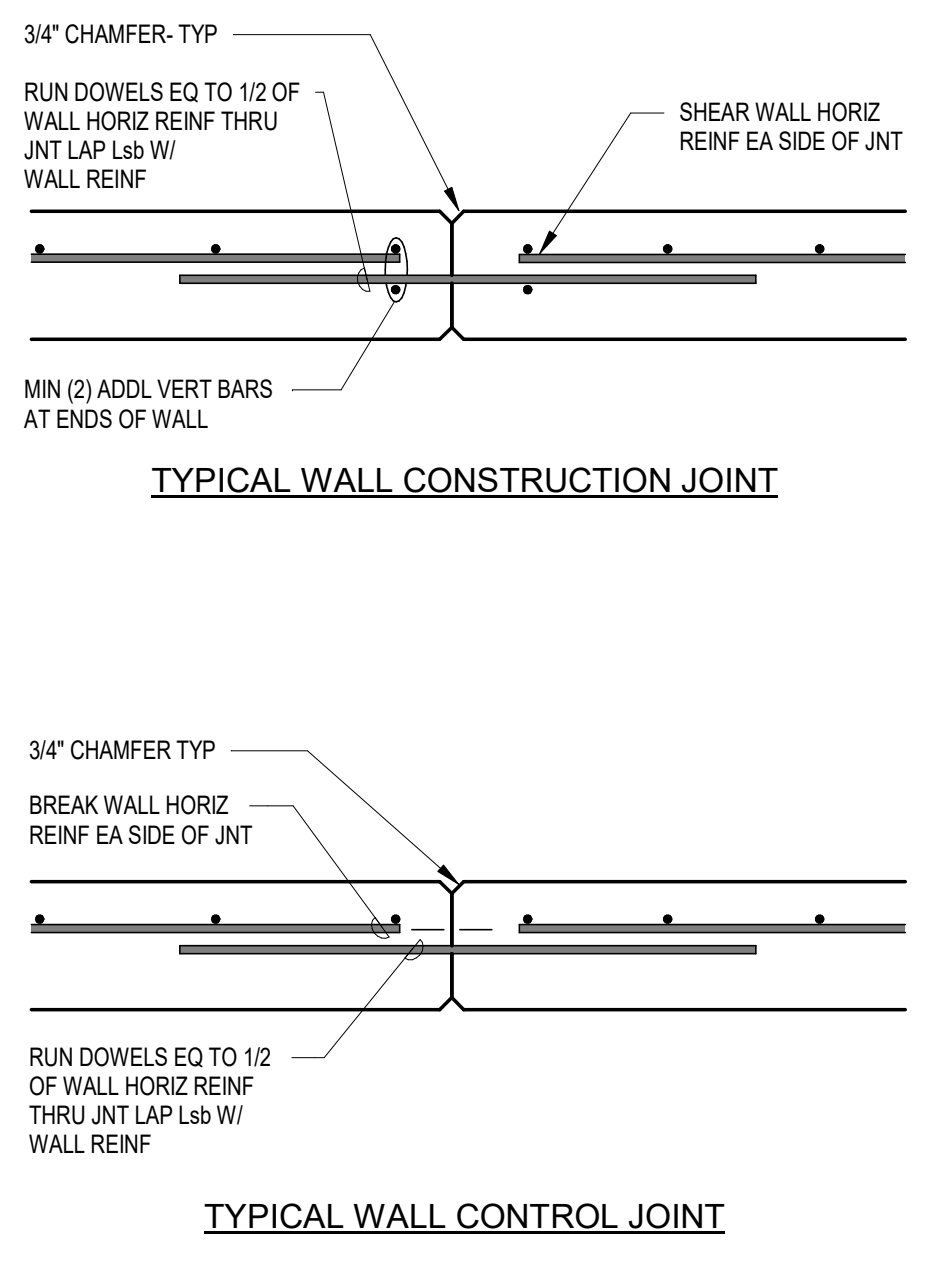
- SITE RETAINING WALL NOTES**
1. SEE SHEET SW0.1 FOR STRUCTURAL NOTES, SHEET SW0.3 FOR TYPICAL DETAILS AND SHEET SW0.2 FOR TESTING AND INSPECTION NOTES.
  2. SEE GEOTECHNICAL ENGINEERING REPORT FOR ALL FOUNDATION SUPPORT REQUIREMENTS, THIS INCLUDES ALL EXCAVATION FILL AND FILL PLACEMENT REQUIREMENTS.
  3. SEE CIVIL DRAWINGS FOR DRAINS, GRADES, SLOPES AND OTHER SITE FEATURES NOT SHOWN.
  4. PROVIDE CONSTRUCTION / CONTROL JOINTS IN CAST IN PLACE CONCRETE WALLS AT 50'-0" OC MAX AND WITHIN 4'-0" EACH SIDE OF CORNERS.
  5. TOP OF FOOTING ELEVATIONS SHOWN ARE RELATIVE BASED ON THE BUILDING LEVEL 1 ELEVATION OF 100'-0". COORDINATE ACTUAL ELEVATIONS WITH CIVIL DRAWINGS.



**2 TYPICAL**  
1" = 1'-0" TYPICAL



**3 TYPICAL**  
1" = 1'-0" TYPICAL



**4 TYPICAL**  
1" = 1'-0" TYPICAL

REINFORCING DEVELOPMENT AND SPLICE LENGTH SCHEDULE															
F'c = 3000 PSI					F'c = 4000 PSI					ALL CONCRETE STRENGTHS					
BAR SIZE	Ld	Lt	Lsb	Lsbt	BAR SIZE	Ld	Lt	Lsb	Lsbt	BAR SIZE	Ld	Lc	Lcs	.	.
#3	17	22	22	28	#3	15	19	19	25	#3	9	12	12		
#4	22	29	29	38	#4	19	25	25	33	#4	11	15	12		
#5	28	36	36	47	#5	24	31	31	41	#5	14	19	15		
#6	33	43	43	56	#6	29	37	37	49	#6	17	23	17		
#7	46	63	63	81	#7	42	54	54	71	#7	20	27	20		
#8	55	72	72	93	#8	48	62	62	81	#8	22	30	23		
#9	62	81	81	105	#9	54	70	70	91	#9	25	34	26		
#10	70	91	91	118	#10	61	79	79	102	#10	28	39	29		
#11	78	101	101	131	#11	67	87	87	114	#11	31	43	32		
#14	93	121	-	-	#14	81	105	-	-	#14	38	-	-		
#18	124	161	-	-	#18	108	140	-	-	#18	50	-	-		

- REINFORCING DEVELOPMENT AND SPLICE LENGTH SCHEDULE NOTES:**
1. REINFORCEMENT DEVELOPMENT AND SPLICE LENGTHS ARE IN ACCORDANCE WITH ACI 318.
  2. NOTATIONS:  
db: NOMINAL BAR DIAMETER (IN)  
Ld: TENSION DEVELOPMENT LENGTH (IN) FOR REINFORCEMENT SATISFYING THE FOLLOWING REQUIREMENTS: SLABS AND WALLS: CLEAR SPACING GREATER THAN 2db; AND CONCRETE CLEAR COVER GREATER THAN db BEAMS AND COLUMNS: CLEAR SPACING GREATER THAN db; AND CONCRETE CLEAR COVER GREATER THAN db.  
Lt: DEVELOPMENT LENGTH OF TOP BARS IN TENSION = 1.3 x Ld (IN)  
Lb: DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = 22 x db (IN)  
Lc: TIED COLUMN LAP SPLICE IN COMPRESSION = 30 x db (IN)  
Lcs: SPIRAL COLUMN LAP SPLICE IN COMPRESSION = 22.5 x db (IN)  
Lsb: TENSION LAP SPLICE LENGTH FOR OTHER THAN TOP BARS = 1.3 x Ld (IN)  
Lsbt: TENSION LAP SPLICE LENGTH OF TOP BARS = 1.69 x Ld (IN)  
3. MULTIPLY VALUES IN THE TABLE BY 1.5 IF CLEAR SPACING OR CONCRETE COVER DO NOT MEET THE REQUIREMENTS FOR Ld IN NOTE 2.  
4. TOP BARS: HORIZONTAL BEAM REINFORCING WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW.  
5. THE DEVELOPMENT AND SPLICE LENGTHS ARE BASED ON REINFORCEMENT STRENGTH Fy = 60 KSI.  
6. #14 AND #18 BARS SHALL NOT BE LAP SPLICED. SEE GENERAL NOTES.

**5 TYPICAL**  
1" = 1'-0" 5/ SW0.3

KEY TO ABBREVIATIONS			
AB	ANCHOR BOLT	L	ANGLE
ABV	ABOVE	LH	LONG LEG HORIZONTAL
ADDL	ADDITIONAL	LLV	LONG LEG VERTICAL
ADJ	ADJACENT	LOC	LOCATION
AF	ABOVE FINISH FLOOR	LONGIT	LONGITUDINAL
ALT	ALTERNATE	MAX	MAXIMUM
ARCH	ARCHITECTURAL ARCHITECT	MB	MACHINE BOLT
ASD	ALLOWABLE STRESS DESIGN	MECH	MECHANICAL
BEL	BELOW	MFR	MANUFACTURER
BLKG	BLOCKING	MIN	MINIMUM
BM	BEAM	MIV	MALLEABLE IRON WASHER
BNDY	BOUNDARY	NS	NEAR SIDE
BOT	BOTTOM	NTS	NOT TO SCALE
BRG	BEARING	NWT	NORMAL WEIGHT
BS	BOTH SIDES	O	OVER
BTWN	BETWEEN	OC	ON CENTER
BU	BUILT UP	O.F.	OUTSIDE FACE
CIP	CAST IN PLACE	OP	OPPOSITE HAND
CJ	CONSTRUCTION CONTROL JOINT	OPNG	OPENING
CL	CENTERLINE	OSB	ORIENTED STRAND BOARD
CLG	CEILING	PC	PRE-CAST
CLR	CLEAR	PDF	POWER DRIVEN FASTENERS, PAF
CMU	CONCRETE MASONRY UNIT	PAF	POWER ACTUATED FASTENERS, PDF
COL	COLUMN	PERP	PERPENDICULAR
CONC	CONCRETE	PL	PLATE
CONN	CONNECT, CONNECTION	PLF	POUNDS PER LINEAR FOOT
CONT	CONTINUOUS	PNL	PANEL
COORD	COORDINATE	PRE-ENGR	PRE-ENGINEERED
CSK	COUNTERSINK	PROV	PROVIDE
CTR	CENTER	PT	POST TENSIONED
CVR	COVER	PW	PLYWOOD
DEG	DEGREE	REF	REFERENCE
DIA	DIAMETER	REINF	REINFORCE, REINFORCEMENT
DBL	DOUBLE	REQD	REQUIRED
EA	EACH	RF	ROOF
EF	EACH FACE	SCHED	SCHEDULE
ELEV	ELEVATION, ELEVATOR	SFRS	SEISMIC FORCE RESISTING SYSTEM
EMB	EMBEDMENT	SHGT	SHEATHING
ENGR	ENGINEER	SM	SIMILAR
EQ	EQUAL/EQUIVALENT	SIMP	SIMPSON STRONG-TIE
EQUIV	EQUIVALENT	SG	SUB ON GRADE
ES	EACH SIDE	SPCG	SPACING
EW	EACH WAY	SQ	SQUARE
EX	EXISTING	STD	STANDARD
EXP	EXPANSION	STIFF	STIFFENER
EXT	EXTERIOR	SW	SHEARWALL
FON	FOUNDATION	T&G	TONGUE AND GROOVE
FF	FINISH FLOOR	THK	THICK
FFE	FINISH FLOOR ELEVATION	THRD	THREADED
FOC	FACE OF CONCRETE	T.O.	TOP OF
FOM	FACE OF MASONRY	TOC	TOP OF CONCRETE
FOS	FACE OF STUD	TOF	TOP OF FOOTING
FS	FAR SIDE	TOP	TOP OF STUD
FTG	FOOTING	TOPL	TOP OF PLATE
GA	GAGE	TOS	TOP OF STEEL
GALV	GALVANIZED	T.O.W.	TOP OF WALL
GC	GENERAL CONTRACTOR	TRANSV	TRANSVERSE
GL	GLUE LAMINATED	TRTD	TREATED
GWB	GYPSUM WALL BOARD	TYP	TYPICAL
HGR	HANGER	UNO	UNLESS NOTED OTHERWISE
HORIZ	HORIZONTAL	VFT	VERTICAL
HSS	HOLLOW STEEL SECTION	VERT	VERTICAL
HT	HEIGHT	W	WITH
I.F.	INSIDE FACE	W/O	WITHOUT
INT	INTERIOR	WF	WIDE FLANGE
JNT	JOINT	WHS	WELDED HEADED STUD
JST	JOIST	WP	WORK POINT
K, KIPS	KIPS=1000 LBS	WTS	WELDED THREADED STUD
		WWF	WELDED WIRE FABRIC

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TYPICAL DETAILS  
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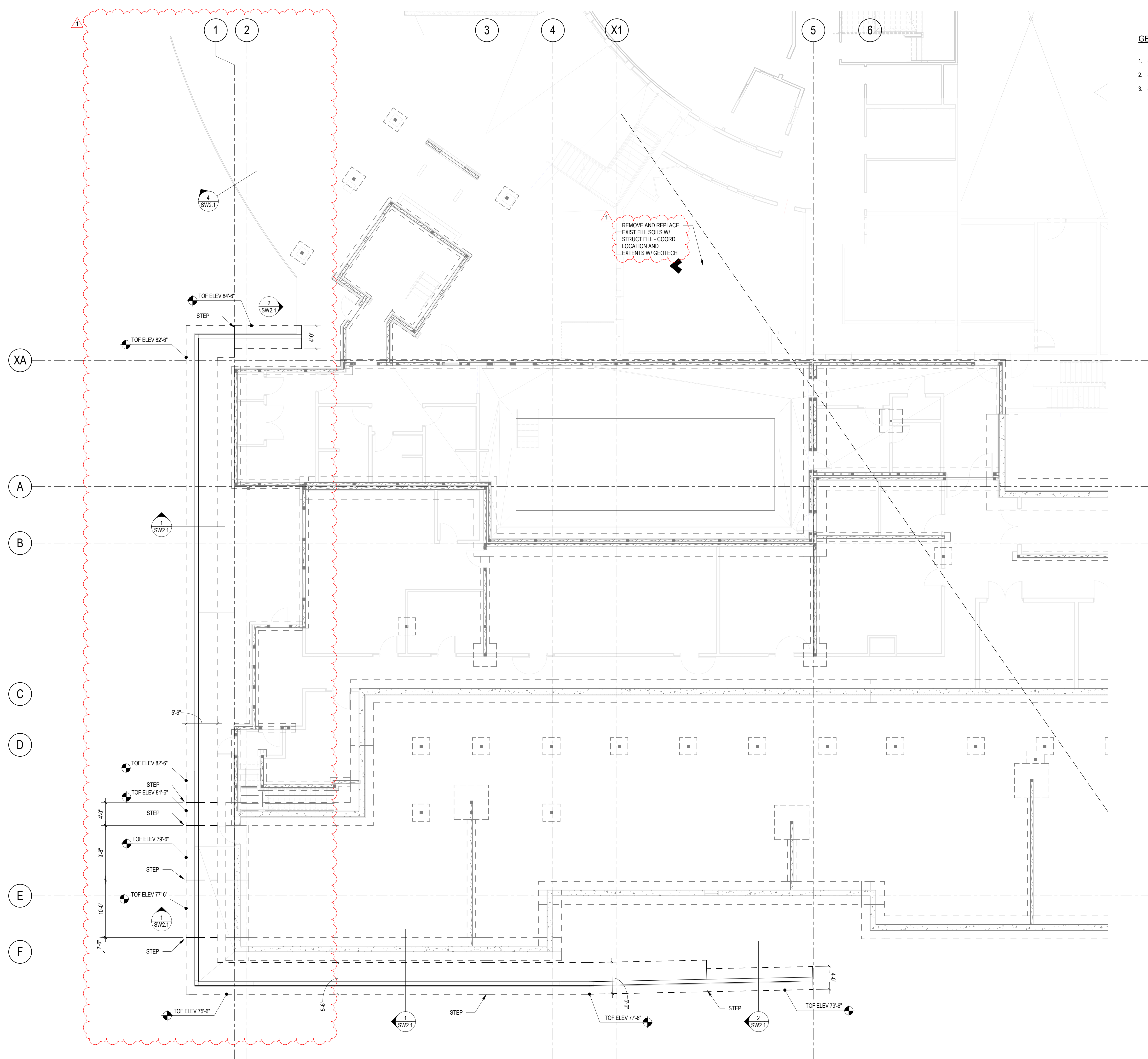
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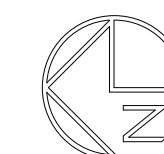
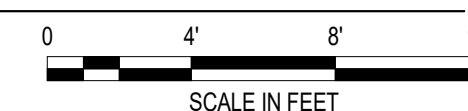
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WALL PLAN

# W1.0



## 4 SITE WALL PLAN

 $1/8^{\circ} = 1'-0"$ 

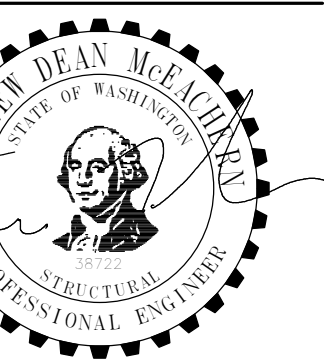
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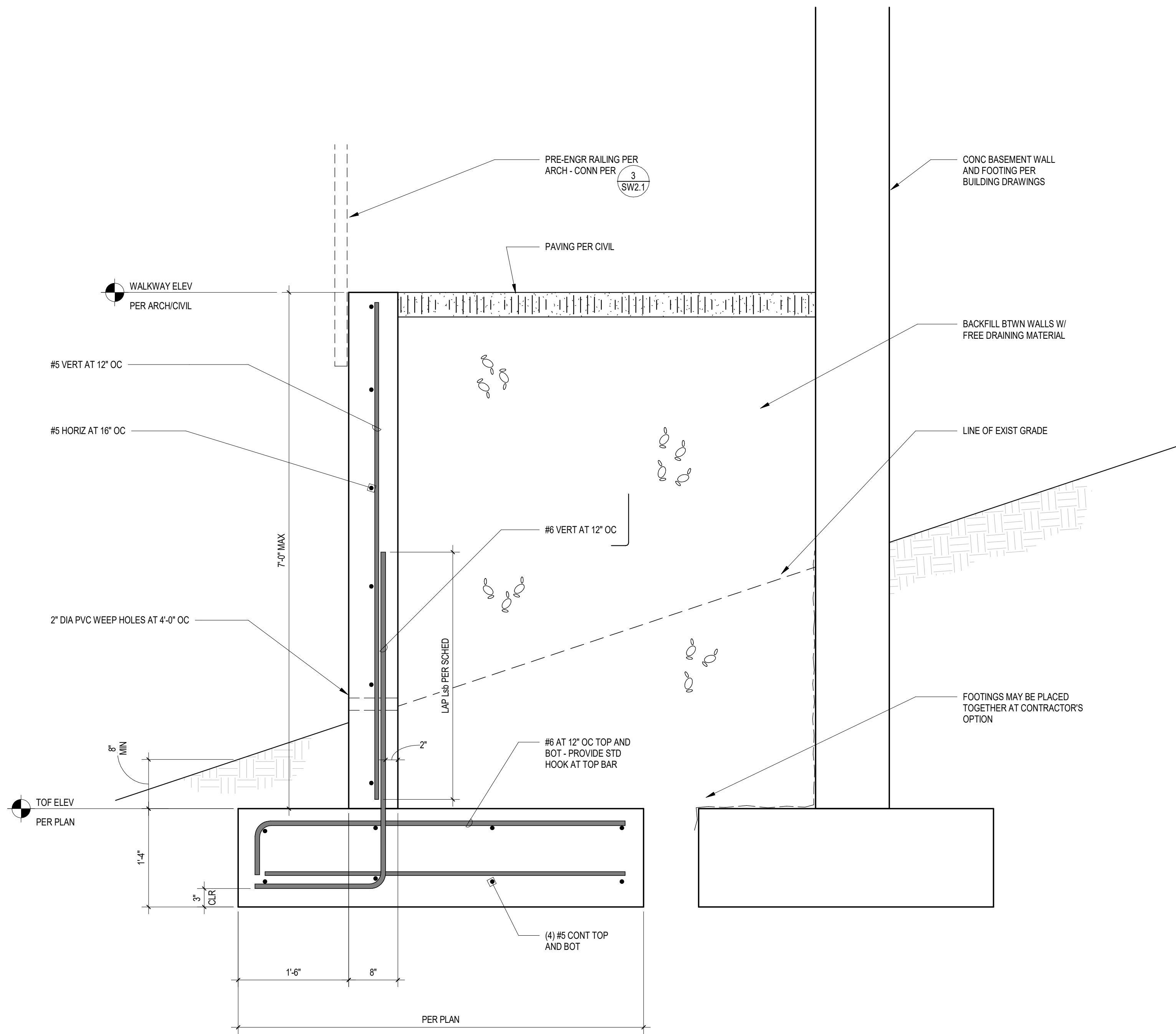
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ALL DETAILS

## W2.1

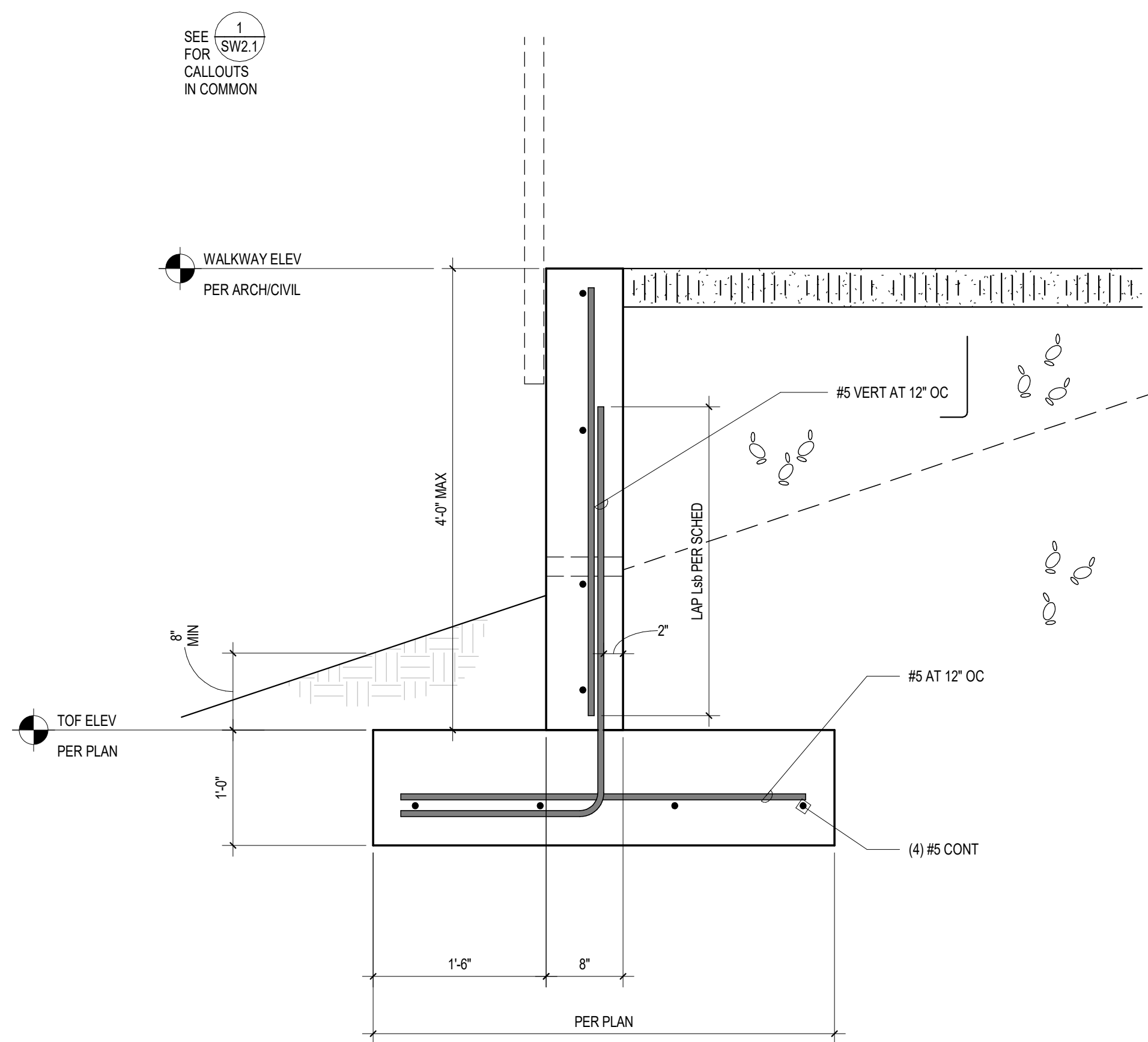


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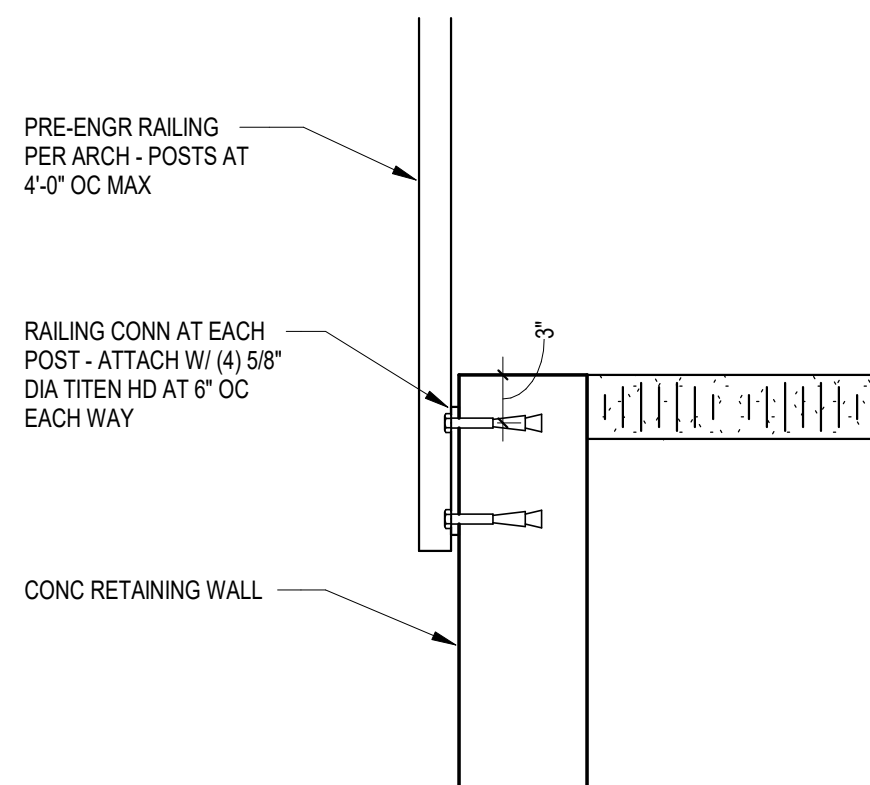
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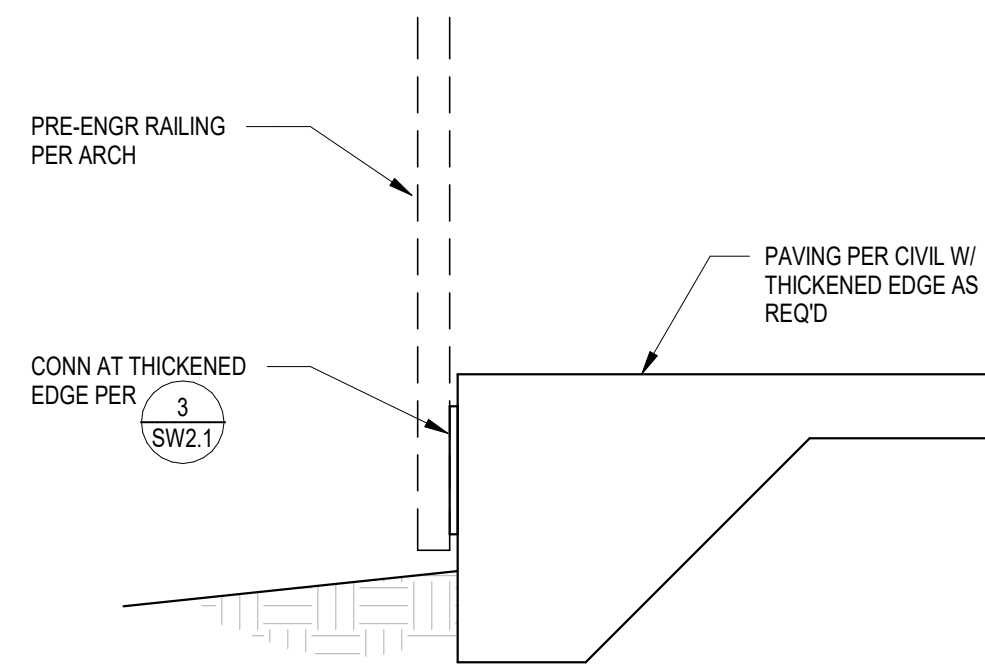
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1" = 1'-0" 2 / SW2.1

1" = 1'-0" 2 / SW2.1



**3 SECTION**  
1" = 1'-0" 3 / SW2.1

1" = 1'-0" 3 / SW2.1



**4** SECTION  
1" = 1'-0" 4 / SW2.1

1" = 1'-0" 4 / SW2:

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