



STRUCTURAL DETAILS WITH FOUNDATION INSTALLATION

FOR A:
**35' TALL, CONCEALMENT
ANTENNA/LIGHT POLE**

LOCATED AT:
**VARIOUS NODES
IN AND AROUND
PUYALLUP, WA
KING COUNTY**

Pierce [Plans; Sht T-1]

CONSULTANT



5032 SALEM DALLAS HWY
SALEM, OR 97304
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CLIENT



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ANTENNA/LIGHT POLE

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**VARIOUS NODES
IN AND AROUND**
PUYALLUP, WA
KING COUNTY

ISSUED FOR
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DWN BY CK'D BY DATE
JLH AWM 31AUG21

REVISIONS

REV.	CHANGES	DATE	BY

ENGINEER SEAL

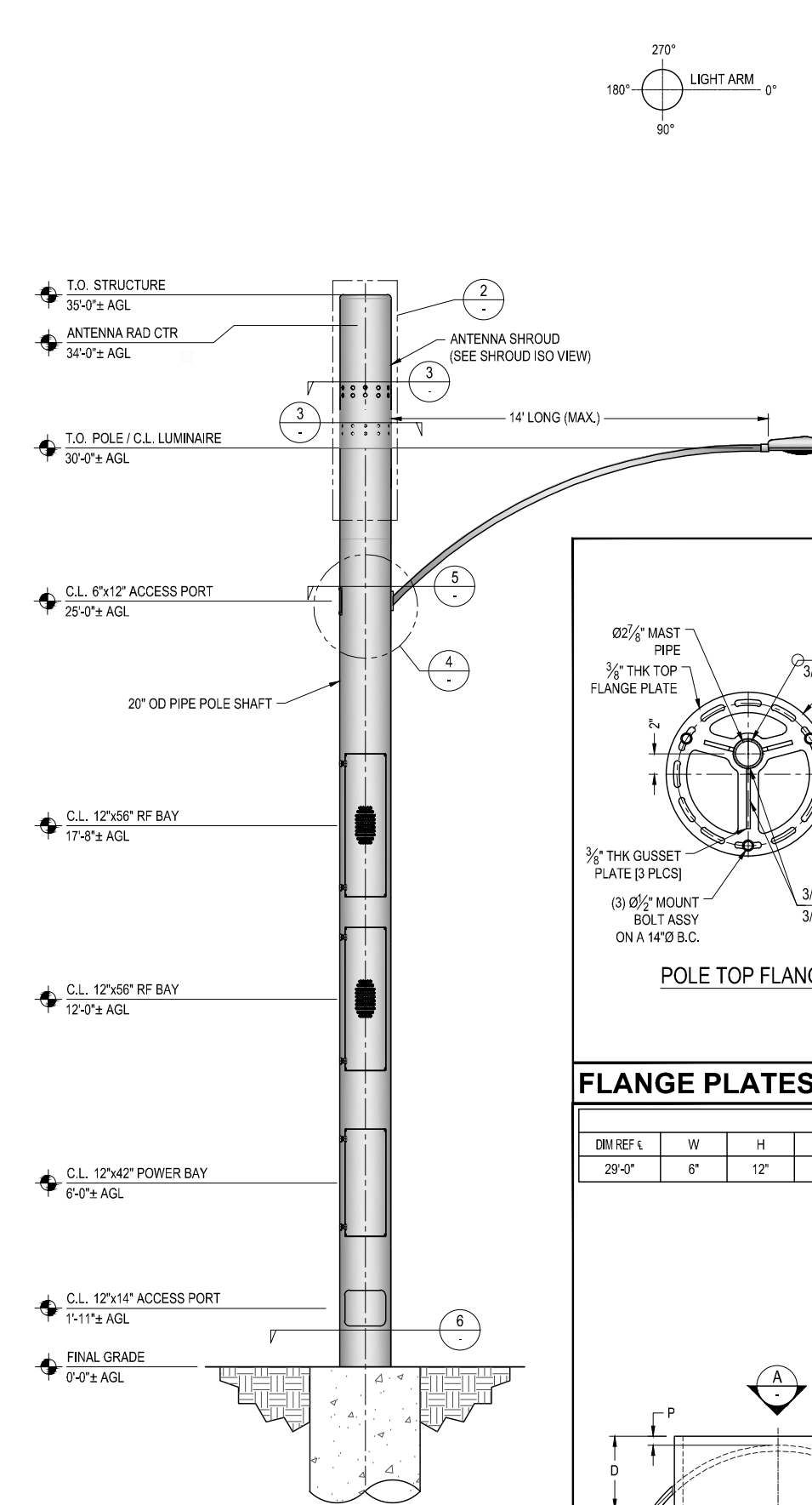


SHEET TITLE
TITLE SHEET

WUT PROJECT NUMBER
21-0267

DRAWING NUMBER
PD-002173

PAGE NUMBER
T-1



POLE ELEVATION

SCALE: 3/16" = 1'-0"

1

PORT DETAIL

SCALE: NONE

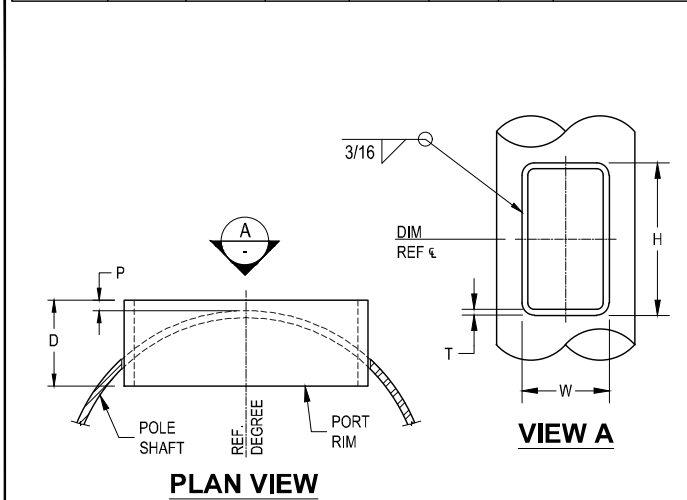
5

FLANGE PLATES

SCALE: NONE

3

PORT SCHEDULE							
DIM REF ϵ	W	H	D	T	P	QTY.	DEGREE
29'-0"	6"	12"	2"	1/4"	3/8"	1	180°



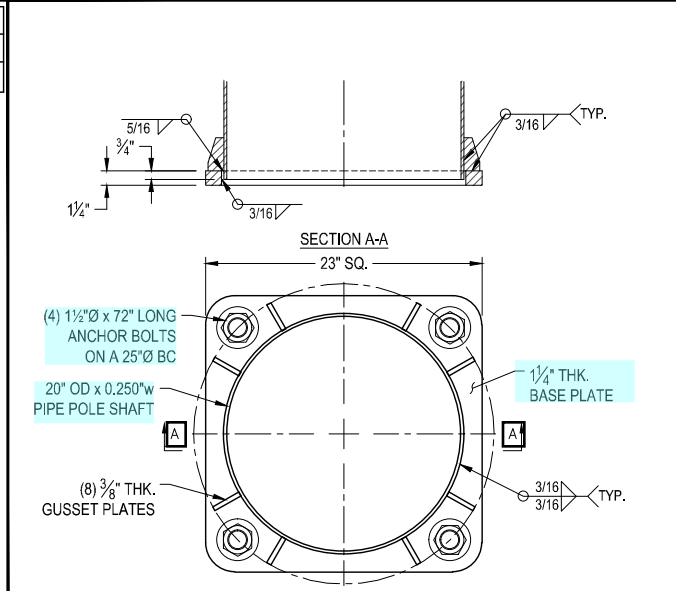
PLAN VIEW

VIEW A

LIGHT ARM CONNECTION

SCALE: NONE

4



SECTION A-A

BASE PLATE DETAIL

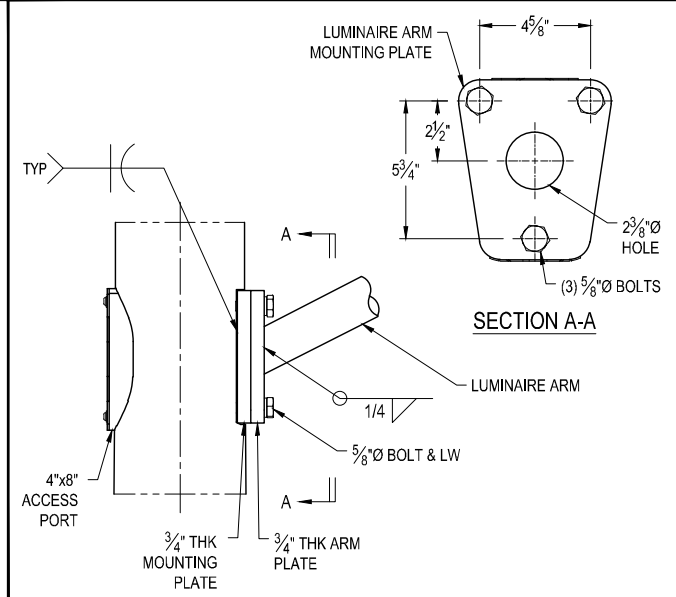
SCALE: 3/4" = 1'-0"

6

ANTENNA MAST DETAILS

SCALE: NONE

2



FRONT VIEW

SIDE VIEW

SECTION A-A

GENERAL DESIGN NOTES:

DESIGN NOTES AND MATERIAL REQUIREMENTS:

1) THE DESIGN CRITERIA FOR THIS STRUCTURE IS AS FOLLOWS:

A) STANDARDS AND DESIGN CODES:

INDUSTRY STANDARD: ANSI/TIA-222-H, 2018 IBC, & 2015 AASHTO LRFD-LTS-1 W/ 2017 INTERIM REVISIONS
 STEEL MANUAL: AISC 15TH EDITION
 CONCRETE CODE: ACI 318-14
 WELDING CODE: ANS/AWS D1.1.15

B) DESIGN:

BWS (V): 97 MPH (3-SECOND GUST) PER ANSI/TIA-222-H & 2018 IBC, SECTION 1609.1.1, EXC. 5
 RISK CATEGORY: II
 EXPOSURE CATEGORY: C
 TOPOGRAPHIC CATEGORY: 1
 V_{AASHTO}: 97 MPH (3-SECOND GUST) PER 2015 AASHTO LRFD LTS-1 W/ 2017 INTERIMS
 ICE: NOT REQ'D FOR THIS STRUCTURE PER ANNEX B
 SOIL: PER 2018 IBC, TABLE 1806.2, CLASS 5 MAT'L
 SEISMIC: PER 2018 IBC SECTION 1613, ASCE 7-16, SECTIONS 15.1.3 & 12.8

C) LOADING:

(1) - 12" \times 60" TALL ANTENNA/RADIO SHROUD, WT. = 165 LBS., C.L. @ 32.5' \pm AGL
 (1) - NOKIA 5G ANTENNAS/RRHS IN TOP SHROUD, WT. = 35 LBS., C.L. @ 31.7' \pm AGL
 (1) - 9" TALL X 30" LONG COBRA HEAD LUMINAIRE, WT. = 45 LBS., C.L. @ 30' \pm AGL
 (1) - 2 7/8" OD X 14" (MAX.) LONG LUMINAIRE ARM, WT. = 85 LBS., C.L. @ 27.5' \pm AGL
 (1) - NOKIA B12/B14 MICRO RRH, WT. = 77 LBS., C.L. @ 17.7' \pm AGL
 (1) - NOKIA B25/B66 MICRO RRH, WT. = 66 LBS., C.L. @ 12' \pm AGL
 (1) - CCI TWIN RRU PSU, WT. = 34 LBS., C.L. @ 6' \pm AGL
 (1) - BASE COVER, WT. = 66 LBS., C.L. @ 0.33' \pm AGL

D) FACTORED BASE REACTIONS:

OVERTURNING MOMENT = 30.0 FT-KIPS
 BASE SHEAR = 1.63 KIPS
 BASE WEIGHT = 3.37 KIPS

2) GENERAL STRUCTURAL NOTES:

A) ALL MATERIALS SHALL CONFORM TO THE FOLLOWING STANDARDS:
 POLE SHAFT: ASTM A53/A500 Gr.B/API 5L Gr.B (MIN. Fy = 35 ksi)
 BASE PLATE: ASTM A36
 PORTS: ASTM A500 GR.B (RECT. TUBE)
 PLATES: ASTM A36
 WELDS: E70XX ELECTRODES
 STRUCTURAL BOLTS: ASTM F3125 GR. A325
 ANCHOR BOLTS: ASTM F1554 GR. 55
 REBAR: ASTM A615 GR. 60
 B) ALL WELDING TO BE PERFORMED BY WELDERS CERTIFIED IN ACCORDANCE WITH AWS D1.1. FIELD WELDING IS PROHIBITED.
 C) ALL STRUCTURAL STEEL SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.
 D) ALL STRUCTURAL BOLTS SHALL BE TIGHTENED TO SNUG-TIGHT AS DEFINED BY AISC 15TH EDITION, PG. 16.2-51 PARAGRAPH 8.1

GENERAL NOTES:

- CONTRACTOR SHALL FIELD VERIFY SITE OR LAYOUT RESTRICTIONS, SITE CONDITIONS, DIMENSIONS, AND ELEVATIONS BEFORE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF WESTERN UT, INC. PRIOR TO BEGINNING PROJECT. ALL WORK SHALL BE PERFORMED USING ACCEPTED CONSTRUCTION PRACTICES.
- NO FIELD MODIFICATIONS MAY BE MADE TO MONOPOLE WITHOUT THE EXPRESS WRITTEN CONSENT FROM THE ENGINEER OF RECORD, WESTERN UT, INC. AND ENGINEER OF RECORD ASSUME NO RESPONSIBILITY FOR THE STRUCTURE IF ALTERATIONS AND/OR ADDITIONS ARE MADE TO THE DESIGN AS SHOWN IN THESE DRAWINGS.
- THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL COMPLY WITH ALL LOCAL CODES, REGULATIONS, AND ORDINANCES AS WELL AS STATE DEPARTMENT OF INDUSTRIAL REGULATIONS AND DIVISION OF INDUSTRIAL SAFETY (OSHA) REQUIREMENTS.
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- IT IS THE INTENT OF THESE DRAWINGS TO SHOW THE COMPLETED INSTALLATION OF THE STRUCTURE SHOWN.
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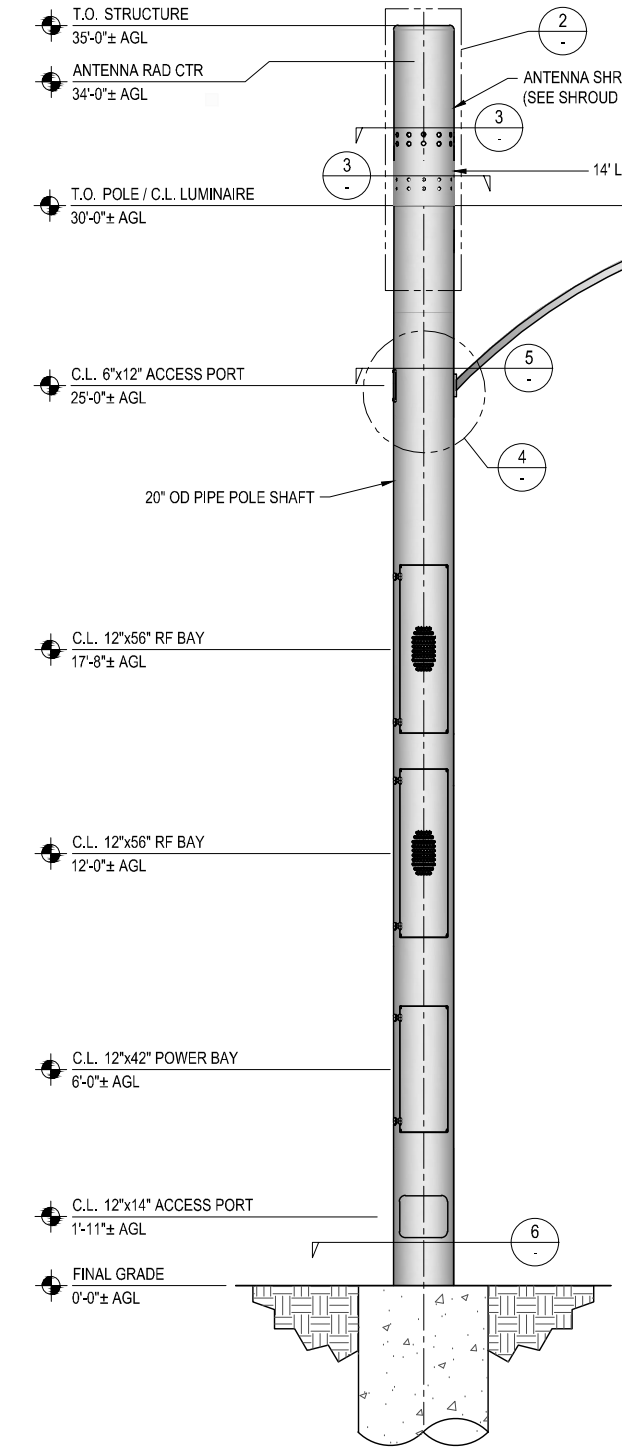
ADRIAN W. McJUNKIN
 STATE OF WASHINGTON
 36767
 REGISTERED
 PROFESSIONAL ENGINEER

SHEET TITLE
**ELEVATION VIEW &
 STRUCTURAL DETAILS**

WUT PROJECT NUMBER
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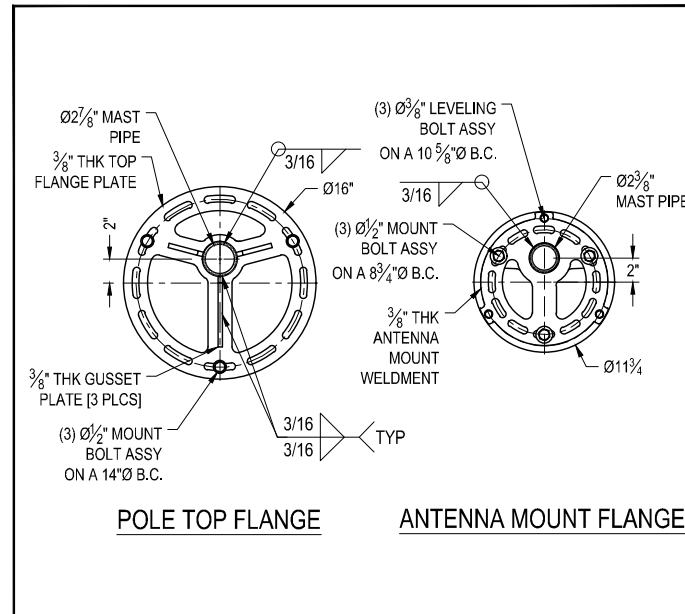
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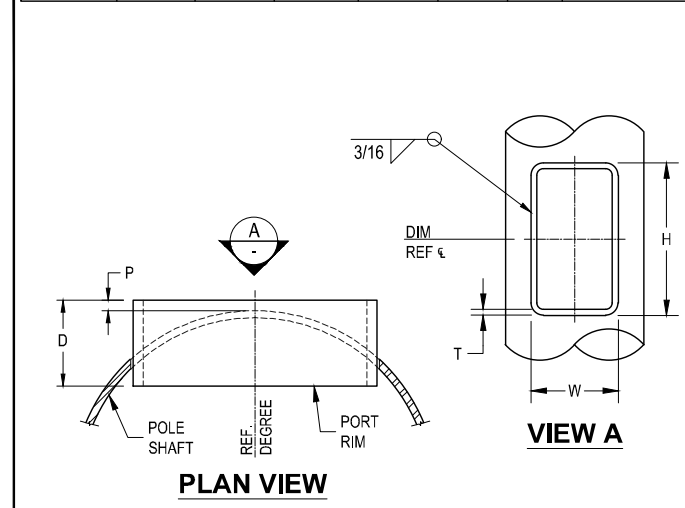
POLE ELEVATION

SCALE: 3/16" = 1'-0" **1**



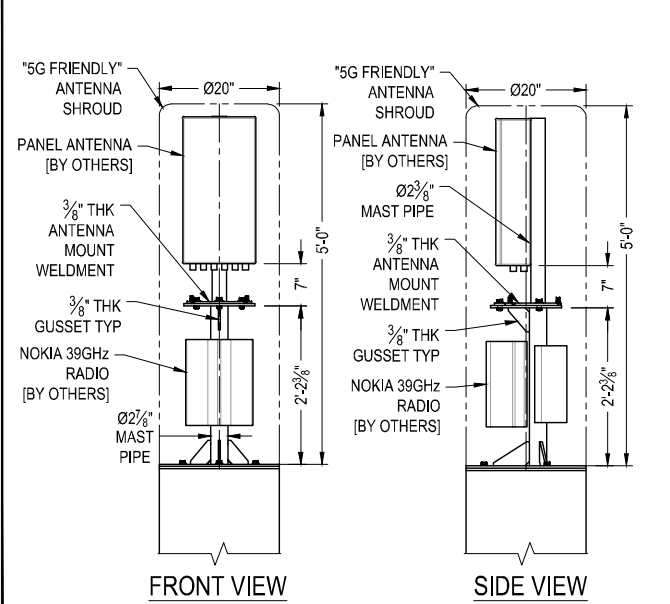
FLANGE PLATES

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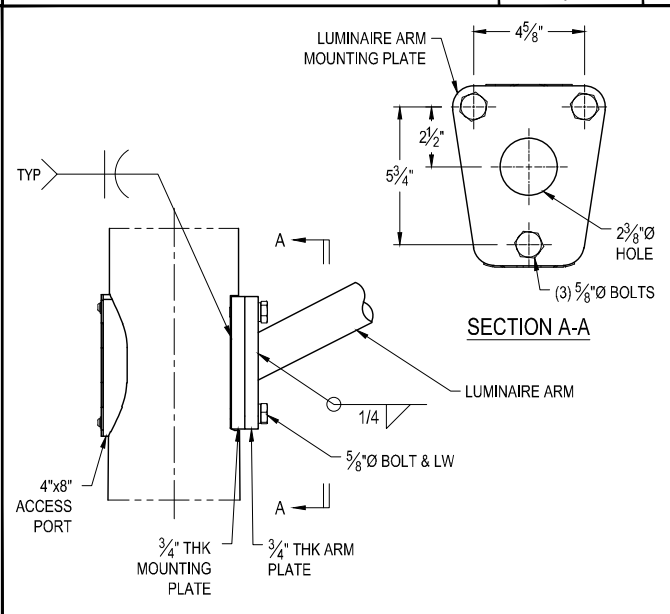
PORT DETAIL

SCALE: NONE **5**



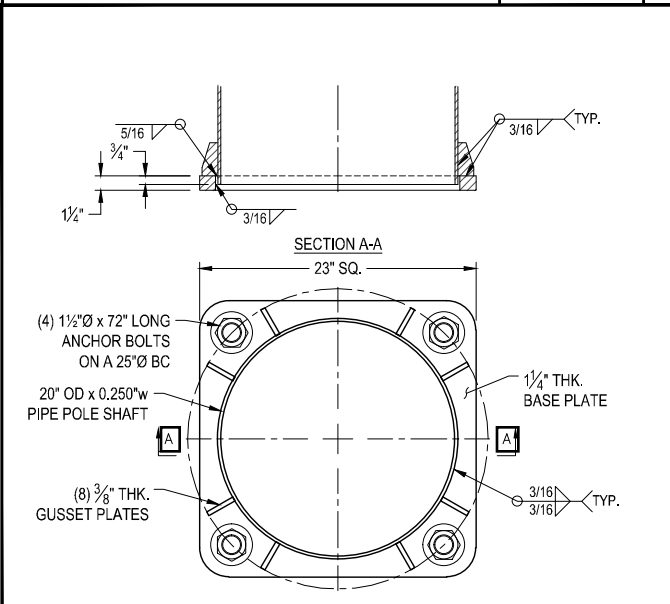
ANTENNA MAST DETAILS

SCALE: NONE **2**



LIGHT ARM CONNECTION

SCALE: NONE **4**



BASE PLATE DETAIL

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ADRIAN W. McJUNKIN
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SHEET TITLE
**ELEVATION VIEW &
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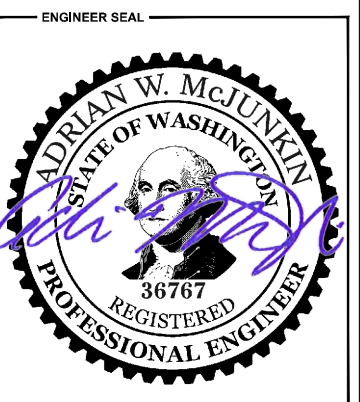
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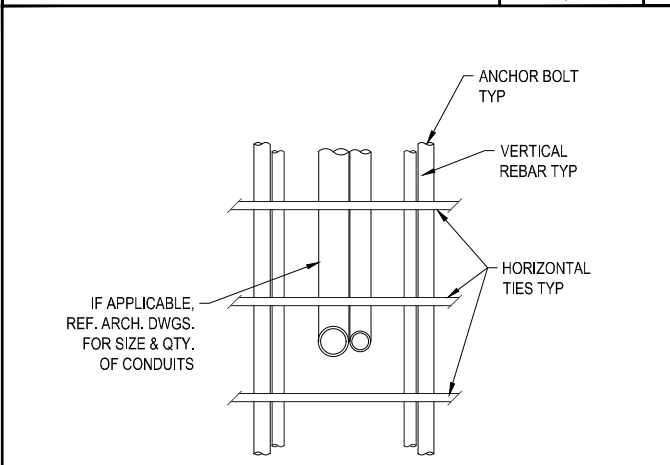
REV.	CHANGES	DATE	BY



GENERAL NOTES

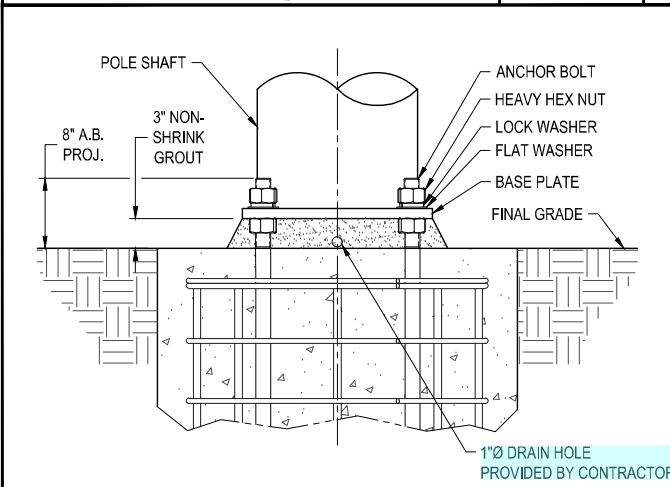
- THE CONTRACTOR IS RESPONSIBLE FOR CHECKING AREA FOR UNDERGROUND FACILITIES PRIOR TO EXCAVATING ANY MATERIALS.
- CONTRACTOR SHALL REFER TO SOILS REPORT (IF AVAILABLE) FOR SITE CONDITIONS AND FURTHER CONSTRUCTION INFORMATION.
- CONTRACTOR SHALL INSPECT AND REMOVE ALL DEBRIS FROM BOTTOM OF EXCAVATION.
- CONTRACTOR SHALL VERIFY ANCHOR BOLT LAYOUT PRIOR TO, AND IMMEDIATELY AFTER PLACING CONCRETE. ANCHOR BOLT LAYOUT IS CRITICAL FOR MONOPOLE INSTALLATION.
- CONTRACTOR SHALL USE AND PROVIDE DEFORMED REINFORCING BARS CONFORMING TO A615 GR.60 (60,000 PSI MIN. YIELD). CONTRACTOR SHALL USE STEEL WIRE TO HOLD REINFORCING BARS TOGETHER. IF WELDING REBAR IS PREFERRED, SUBSTITUTE USING A706 GR. 60 DEFORMED BARS.
- CONTRACTOR SHALL USE AND PROVIDE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI. CONCRETE SHALL USE 1" MAXIMUM STONE AGGREGATE. MIX DESIGN: 6½ SACKS OF CEMENT MINIMUM PER CUBIC YARD. 5" MINIMUM AND 7" MAXIMUM CONCRETE SLUMP.
- CONCRETE SHALL BE CONSOLIDATED USING VIBRATORY METHODS THROUGHOUT DEPTH OF FOUNDATION. VIBRATING LOWER DEPTHS MAY BE ACCOMPLISHED BY TOUCHING REBAR CAGE WITH VIBRATOR.
- CONTRACTOR SHOULD ANTICIPATE THE USE OF A FULL-LENGTH TEMPORARY CASING TO STABILIZE THE EXCAVATION. THE CASING SHALL BE WITHDRAWN DURING THE PLACEMENT OF CONCRETE IN THE EXCAVATED HOLE. CONCRETE SHALL BE PLACED USING CONVENTIONAL METHODS TO MINIMIZE SEGREGATION OF CONCRETE AND AGGREGATE. CONCRETE SHALL NOT FREE FALL MORE THAN 5 FT. CONCRETE MAY BE PLACED BELOW WATER USING TREMIE METHODS.
- CONCRETE SHALL BE PLACED TO THE DEPTH INDICATED, AND THE ABOVE GRADE PORTION SHALL BE FORMED. THE REBAR CAGE, ANCHOR BOLTS, AND CONCRETE SHALL BE PLACED WITHIN 24 HOURS OF COMPLETING THE EXCAVATION. COLD JOINTS ARE NOT ALLOWED.
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ADEQUATE CONCRETE COVERAGE OVER REINFORCING BARS TO MINIMIZE CORROSION POTENTIAL. UNLESS OTHERWISE NOTED, CONTRACTOR SHALL USE 3" CONCRETE COVER OVER REBAR. TOP OF FOOTING SHALL BE TOWELED LEVEL AND SMOOTH.
- DRILLED PIER FOUNDATION DESIGN PER 2018 IBC, TABLE 1806.2, CLASS 5 MATERIAL.
- TOTAL VOLUME OF CONCRETE REQUIRED FOR THIS FOUNDATION IS APPROXIMATELY 2.5 CU. YDS.

GROUNDING KIT DETAIL

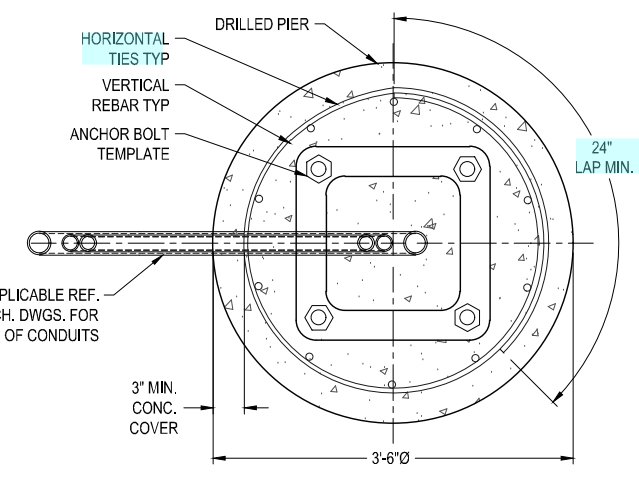


* ADJUST REBAR AS NEEDED TO ACCOMMODATE CONDUIT. SEE REINFORCEMENT SUMMARY FOR SIZE, QUANTITY AND LOCATION OF VERTICAL BARS AND HORIZONTAL TIES.

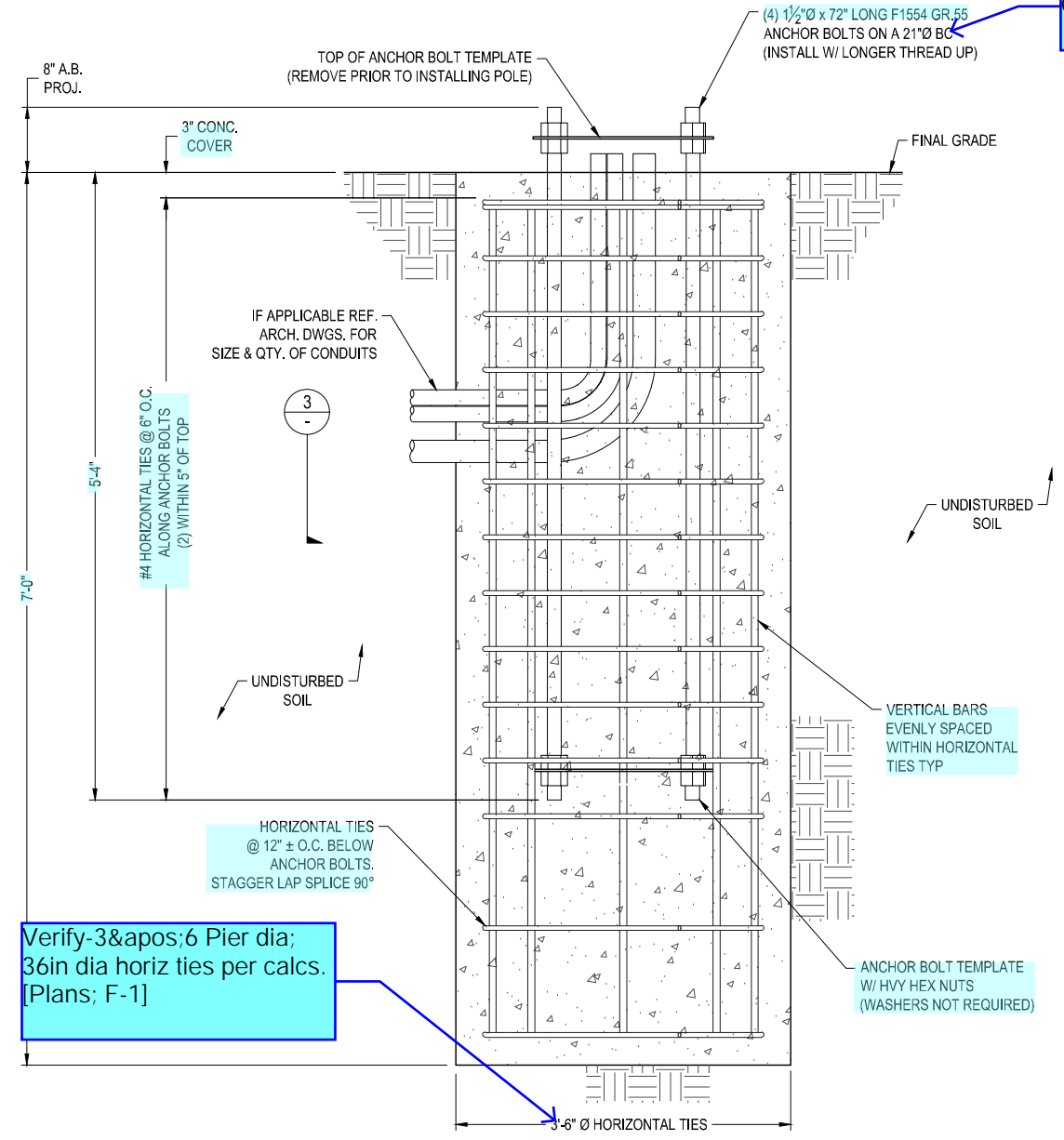
CONDUIT DETAIL @ PIER FACE



* NON-SHRINK GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 6,000 PSI.
 BASE PLATE SHALL NOT BE GROUTED UNTIL AFTER THE STRUCTURE HAS BEEN INSTALLED AND PLUMBED.



Verify-25" dia BC used in Calcs [Plans; Sht F-1]



Verify-3'±6 Pier dia; 36in dia horz ties per calcs. [Plans; F-1]

REBAR SCHEDULE

DESCRIPTION	SIZE	QTY.	LENGTH	WEIGHT	OVERLAP
VERTICAL BARS	#8	10	6'-6"	174 LBS	N/A
HORIZONTAL TIES	#4	14	11'-6"	108 LBS	2'-0"

PIER ELEVATION

SCALE: 5/8" = 1'-0" 1

BASE GROUTING DETAIL

SCALE: 5/8" = 1'-0" 4

FACTORED BASE REACTIONS

MOMENT = 30.0 FT-KIPS
 SHEAR = 1.63 KIPS
 VERTICAL = 3.37 KIPS

SPECIAL INSPECTIONS

SPECIAL INSPECTION: THE FOLLOWING ELEMENTS OF CONSTRUCTION SHALL REQUIRE SPECIAL INSPECTION PER 2018 IBC, SECTION 17.

ITEM	DESCRIPTION	INSPECTION BY	MATERIAL
1	ALLOWABLE PIER EXCAVATION LATERAL BEARING	SOILS ENGINEER	100 PSF/FT LATERAL
2	PIER CONSTRUCTION REINFORCING STEEL BAR SIZES AND INSTALLATION	SPECIAL INSPECTOR	ASTM A615 GR.60
3	ANCHOR BOLTS BOLT SIZE AND LENGTHS INSTALLATION	SPECIAL INSPECTOR	ASTM F1554 GR.55
4	CONCRETE TEST SPECIMENS	SPECIAL INSPECTOR	f _c = 4,000 PSI TYPE II CEMENT