GENERAL NOTES

THESE GENERAL NOTES ARE TO BE USED AS A SUPPLEMENT TO THE SPECIFICATIONS. ANY DISCREPANCIES FOUND AMONG THE DRAWINGS, THE SPECIFICATIONS, THESE GENERAL NOTES AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE GENERAL CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE GENERAL CONTRACTOR'S RISK. THE GENERAL CONTRACTOR SHALL VERIFY AND COORDINATE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE STRUCTURE HAS BEEN DESIGNED TO RESIST CODE SPECIFIED VERTICAL AND LATERAL FORCES AFTER THE CONSTRUCTION OF ALL STRUCTURAL ELEMENTS HAS BEEN COMPLETED. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO JOB SITE SAFETY; ERECTION MEANS, METHODS, AND SEQUENCES; TEMPORARY SHORING, FORMWORK, BRACING; USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES. PROVIDE ADEQUATE RESISTANCE TO LOADS ON THE STRUCTURES DURING CONSTRUCTION PER SEI/ASCE STANDARD NO. 37-14 "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION."

CONSTRUCTION OBSERVATION BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH DESIGN ASPECTS ONLY AND IS NOT INTENDED IN ANY WAY TO REVIEW THE CONTRACTOR'S CONSTRUCTION PROCEDURES.

STANDARDS
ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2015 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION.

CONTRACT DRAWINGS / DIMENSIONS

ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. CONSULTANT DRAWINGS BY OTHER DISCIPLINES ARE SUPPLEMENTARY TO ARCHITECTURAL DRAWINGS. REPORT DIMENSIONAL OMISSIONS OR DISCREPANCIES BETWEEN ARCHITECTURAL DRAWINGS AND STRUCTURAL, MECHANICAL, ELECTRICAL OR CIVIL DRAWINGS TO ARCHITECT PRIOR TO PROCEEDING WITH WORK.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS. PRIMARY STRUCTURAL ELEMENTS ARE DIMENSIONED ON STRUCTURAL PLANS AND DETAILS AND OVERALL LAYOUT OF STRUCTURAL PORTION OF WORK. SOME SECONDARY ELEMENTS ARE NOT DIMENSIONED, SUCH AS WALL CONFIGURATIONS, INCLUDING EXACT DOOR AND WINDOW LOCATIONS, ALCOVES, SLAB SLOPES AND DEPRESSIONS, CURBS, ETC. VERTICAL DIMENSIONAL CONTROL IS DEFINED BY ARCHITECTURAL WALL SECTIONS AND BUILDING SECTIONS. STRUCTURAL DETAILS SHOW DIMENSIONAL RELATIONSHIPS TO CONTROL DIMENSIONS DEFINED BY ARCHITECTURAL DRAWINGS. DETAILING AND SHOP DRAWING PRODUCTION FOR STRUCTURAL ELEMENTS WILL REQUIRE DIMENSIONAL INFORMATION CONTAINED IN **BOTH** ARCHITECTURAL AND STRUCTURAL DRAWINGS.

DESIGN CRITERIA

VERTICAL LOADS

AREA	DESIGN DEAD LOAD	LIVE LOAD (2)	PARTITION LOAD	CONCENTRATED LOADS
ROOF	14 PSF	25 PSF (1)		300#
HOTEL ROOM/RESIDENTIAL CORRIDOR	28 PSF AT WOOD FRM'G	40 PSF		1,000#
PASSENGER VEHICLE GARAGES	ACTUAL	40 PSF (3)		3,000#
STAIRS	ACTUAL	100 PSF		300#

(1) DRIFT AND UNBALANCED SNOW LOAD PER ASCE 7-10, CHAPTER 7.
(2) LIVE LOADS EXCEPT SNOW LOADS ARE REDUCED PER IBC SECTION 1607.10.
(3) LIVE LOAD REDUCTION NOT PERMITTED EXCEPT AS NOTED IN IBC SECTION 1607.10.

SNOW: (MINIMUM ROOF SNOW LOAD = 25 PSF)

LATERAL FORCES

LATERAL FORCES ARE TRANSMITTED BY DIAPHRAGM ACTION OF ROOF AND FLOORS TO BRACED FRAME/SHEAR WALLS. LOADS ARE THEN TRANSFERRED TO FOUNDATION BY BRACED FRAME/SHEAR WALL ACTION WHERE ULTIMATE DISPLACEMENT IS RESISTED BY PASSIVE PRESSURE OF EARTH AND/OR SLIDING FRICTION. OVERTURNING IS RESISTED BY DEAD LOAD OF THE STRUCTURE.

WIND:

THE BUILDING MEETS THE CRITERIA TO USE THE "METHOD 2 - SIMPLIFIED ENVELOPE PROCEDURE" PER ASCE 7-10.

- EXPOSURE CATEGORY = B
- BASIC WIND SPEED, (3 SEC. GUST), V_{ULT} = 110 MPH
- RISK CATEGORY PER TABLE 1.5-1 = II
- TOPOGRAPHIC FACTOR K_{ZT} = 1.0
- INTERNAL PRESSURE COEFFICIENT (ENCLOSED) = ± 0.18
- COMPONENTS AND CLADDING LOADS, SEE THE FOLLOWING TABLES:

ROOF SURFACES [1]						
EFFECTIVE WIND AREA	POSITIVE PRESSURES (PSF)			NEGATIVE PRESSURES (PSF)		
	ZONE [2]					
	1	2	3	1	2	3
10 SF	16.0	16.0	16.0	-21.8	-36.5	-55.0
20 SF	16.0	16.0	16.0	-21.2	-32.5	-45.0
50 SF	16.0	16.0	16.0	-20.5	-27.5	-33.1
100 SF	16.0	16.0	16.0	-19.9	-23.6	-23.6

WALL SURFACES AND ROOF OVERHANGS [1]						
EFFECTIVE WIND AREA	POSITIVE PRESSURE (PSF)		NEGATIVE PRESSURE (PSF)		ROOF OVERHANGS (PSF)	
	ZONE [2]					
	4	5	4	5	2	3
10 SF	21.8	21.8	-23.6	-29.1	-40.6	-68.3
20 SF	20.8	20.8	-22.6	-27.2	-40.6	-61.6
50 SF	19.5	19.5	-21.3	-24.6	-40.6	-52.8
100 SF	18.5	18.5	-20.4	-22.6	-40.6	-46.1
500 SF	16.2	16.2	-18.1	-18.1	-	-

1. VALUES SHOWN IN TABLE ARE GROSS ULTIMATE WIND PRESSURES.
2. ZONES ARE AS DEFINED BY FIGURE 30.5-1 IN ASCE 7-10.

SEISMIC: (ASCE 7-10) V = CsW

WHERE $Cs = \frac{S_{DS}}{\left(\frac{R}{I_e}\right)}$; WITH

$Cs\ MINIMUM = 0.044\ S_{DS} I_E \geq 0.01$
OR
$Cs\ MINIMUM = \frac{0.5 S_1}{\frac{R}{I_e}}$ FOR $S_1 > 0.6g$

$Cs\ MAXIMUM = \frac{S_{D1}}{T\left(\frac{R}{I_e}\right)}$ FOR $T \leq T_L$
OR
$Cs\ MAXIMUM = \frac{S_{D1} T_L}{T^2 \left(\frac{R}{I_e}\right)}$ FOR $T > T_L$

SEISMIC IMPORTANCE FACTOR, I_e = 1
RISK CATEGORY OF BUILDING PER TABLE 1.5-1 = II
SPECTRAL RESPONSE ACCELERATIONS Ss = 1.25 & S_1 = 0.48
SITE CLASS PER TABLE 20.3-1 = D
DESIGN SPECTRAL RESPONSE ACCELERATIONS S_{DS} = 0.833 & S_{D1} = 0.486
SEISMIC DESIGN CATEGORY = D
W = EFFECTIVE SEISMIC WEIGHT OF BUILDING = 1702 K
ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE
RESPONSE MODIFICATION FACTOR PER TABLE 12.2-1, R = 6.5 (WOOD SHEAR WALL), R = 5 (CONCRETE SHEAR WALL)
Cs = 0.128 (WOOD), Cs = 0.169 (CONCRETE)
DESIGN BASE SHEAR V = 289 K

PIPES, DUCTS AND MECHANICAL EQUIPMENT SUPPORTED OR BRACED FROM STRUCTURE. CONFORM TO SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION, INC. PUBLICATION "SEISMIC RESTRAINT MANUAL: GUIDELINES FOR MECHANICAL SYSTEMS". SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA PAMPHLET 13.

FOUNDATION DESIGN CRITERIA (REFER TO GEOTECHNICAL REPORT BY SOUTH SOUND HEOTECHNICAL CONSULTING DATED DEC 19TH 2018. MATCH BACKFILL DESCRIPTION BELOW WITH INFORMATION IN GEOTECH REPORT).

SOIL BEARING PRESSURE: 4000

ACTIVE PRESSURE - RESTRAINED: 55 PCF +7H SEISMIC SURCHARGE
ACTIVE PRESSURE - UNRESTRAINED: 40 PCF +4H SEISMIC SURCHARGE
PASSIVE RESISTANCE: 300 PCF (INCLUDES F.O.S. ≥ 1.5)
COEFFICIENT OF FRICTION: .50 (INCLUDES F.O.S. ≥ 1.5)
*1/3 INCREASE ALLOWED FOR SEISMIC OR WIND LOADING

CONCRETE

CAST-IN-PLACE CONCRETE

MIX DESIGNS: THE CONTRACTOR SHALL DESIGN CONCRETE MIXES THAT MEET OR EXCEED THE REQUIREMENTS OF THE CONCRETE MIX TABLE. THE MIX DESIGNS SHALL FACILITATE ANTICIPATED PLACEMENT METHODS, WEATHER, REBAR CONGESTION, ARCHITECTURAL FINISHES, CONSTRUCTION SEQUENCING, STRUCTURAL DETAILS, AND ALL OTHER FACTORS REQUIRED TO PROVIDE A STRUCTURALLY SOUND, AESTHETICALLY ACCEPTABLE FINISHED PRODUCT. WATER REDUCING ADMIXTURES WILL LIKELY BE REQUIRED TO MEET THESE REQUIREMENTS. CONCRETE MIX DESIGNS SHALL CLEARLY INDICATE THE TARGET SLUMP. SLUMP TOLERANCE SHALL BE ± 1-1/2 INCHES.

AGGREGATE: COARSE AND FINE AGGREGATE SHALL CONFORM TO ASTM C33

CEMENT: CEMENT SHALL CONFORM TO ASTM C150, TYPE II PORTLAND CEMENT, UNLESS NOTED OTHERWISE.

FLYASH: SHALL CONFORM TO ASTM C618 CLASS C OR F, MAXIMUM LOSS OF IGNITION SHALL BE 1.0%.

SLAG: GROUND GRANULATED BLAST-FURNACE (GGBF) SLAG SHALL CONFORM TO ASTM C989 GRADE 100 OR 120.

ALTERNATE MIX DESIGNS: VARIATIONS TO THE MIX DESIGN PROPORTIONS MAY BE ACCEPTED IF SUBSTANTIATED IN ACCORDANCE WITH ACI 318, CHAPTER 19. PROVIDE SUBMITTALS A MINIMUM OF TWO WEEKS PRIOR TO BID FOR DETERMINATION OF ACCEPTABILITY.

ADMIXTURES: ADMIXTURES SHALL BE BY MASTER BUILDERS, W.R. GRACE, OR PRE-APPROVED EQUAL. ALL MANUFACTURER'S RECOMMENDATIONS SHALL BE FOLLOWED.

WATER: SHALL BE CLEAN AND POTABLE.

MAXIMUM CHLORIDE CONTENT: THE MAXIMUM WATER SOLUBLE CHLORIDE CONTENT SHALL NOT EXCEED 0.15% BY WEIGHT OF CEMENTITIOUS MATERIAL UNLESS NOTED OTHERWISE.

CONCRETE EXPOSED TO WEATHER (COLUMNS, RETAINING WALLS, SLAB ON GRADE): PROVIDE 5.0% TOTAL AIR CONTENT FOR ALL CONCRETE EXPOSED TO WEATHER. TOTAL AIR CONTENT IS THE SUM OF ENTRAINED AIR PROVIDED BY ADMIXTURES AND NATURALLY OCCURRING ENTRAPPED AIR. AIR CONTENT SHALL BE TESTED PRIOR TO BEING PLACED IN THE PUMP HOPPER OR BUCKET; IT IS NOT REQUIRED TO BE TESTED AT THE DISCHARGE END OF THE PUMP HOSE. THE TOLERANCE ON ENTRAPPED AIR SHALL BE +2.0% AND -1.5% WITH THE AVERAGE OF ALL TESTS NOT LESS THAN THE SPECIFIED AMOUNT.

TOTAL CEMENTITIOUS MATERIAL: THE SUM OF ALL CEMENT PLUS FLYASH AND SLAG. AT THE CONTRACTORS OPTION FLYASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT BUT SHALL NOT EXCEED 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIAL. IN NO CASE SHALL THE AMOUNT OF FLYASH OR SLAG BE LESS THAN REQUIRED BY THE CONCRETE MIX DESIGN TABLE. FOOTING MIXES SHALL CONTAIN NOT LESS THAN 5 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD, ALL OTHER MIXES SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD, UNLESS NOTED OTHERWISE.

ITEM	DESIGN f'c (PSI) (AT 28 DAYS U.N.O.)	MAX. W/C RATIO	MIN. FLYASH OR SLAG (PCY)	AGGREGATE GRADING ASTM AASHTO	NOTES
SLABS ON GRADE - UNO	4000	0.45	100	57 OR 67	1
FOUNDATIONS - UNO	4000	0.50	--	57 OR 67	
MAT FOUNDATIONS	5000 @ 56 DAYS	0.50	100	57 OR 67	
ELEVATED DECKS, SHEAR WALLS, COLUMN	5000	0.40	--	57 OR 67	
ALL OTHER CONCRETE	4000	0.50	--	57 OR 67	

Reviewed for Building Code Compliance
Building Division
Building Permit No. B-20-0078
Date of Approval 7/10/2020

CONCRETE MIX NOTES:

1. FIBROUS CONCRETE REINFORCEMENT SHALL BE "FIBERMESH" MANUFACTURED BY PROPEX CONCRETE SYSTEMS OR PRE-APPROVED EQUAL AND SHALL CONFORM TO ASTM C1116 TYPE III 4.1.3, PERFORMANCE LEVEL 1, AND SHALL BE 100 PERCENT VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS CONTAINING NO REPROCESSED OLEFIN MATERIALS AND SPECIFICALLY MANUFACTURED FOR USE AS CONCRETE SECONDARY REINFORCEMENT. DOSAGE SHALL FOLLOW MANUFACTURER'S RECOMMENDATION BUT NOT LESS THAN 1.5 LB/CU. YD.

2. MAXIMUM WATER CONTENT 240 PCY.

3. THIS MIX SHALL CONTAIN 1 GALLON PER CY OF 'ECLIPSE' SHRINKAGE REDUCING ADD MIXTURE BY W.R. GRACE OR APPROVED ALTERNATE. FOR CONCRETE REQUIRING AN AIR ENTRAINMENT ADMIXTURE, 'ECLIPSE PLUS' SHALL BE USED.

CONCRETE PLACEMENT

PLACE CONCRETE FOLLOWING ALL APPLICABLE ACI RECOMMENDATIONS. CONCRETE SHALL BE PROPERLY CONSOLIDATED PER ACI 309 USING INTERIOR MECHANICAL VIBRATORS, DO NOT OVER-VIBRATE. CONCRETE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION OR EXPANSION JOINTS. IF CONCRETE IS PLACED BY THE PUMP METHOD, HORSES SHALL BE PROVIDED TO SUPPORT THE HOSE, THE HOSE SHALL NOT BE ALLOWED TO RIDE ON THE REINFORCING. WEATHER FORECASTS SHALL BE MONITORED AND ACI RECOMMENDATIONS FOR HOT AND COLD WEATHER CONCRETING SHALL BE FOLLOWED AS REQUIRED. CONCRETE SHALL NOT FREE FALL MORE THAN 5 FEET DURING PLACEMENT WITHOUT WRITTEN APPROVAL OF STRUCTURAL ENGINEER.

FLOATING & FINISHING OPERATIONS

WATER SHALL NOT BE ADDED TO THE CONCRETE SURFACE DURING FLOATING & FINISHING OPERATIONS. PRE-APPROVED EVAPORATION RETARDER SPECIFICALLY DESIGNED FOR FLOATING & FINISHING OPERATIONS ARE ACCEPTABLE.

FORMED SURFACES:

FORMWORK CLASS OF SURFACE PER ACI 347 TABLE 3.1	
ITEM	CLASS OF FINISH
ALL SURFACES EXPOSED TO PUBLIC VIEW, U.N.O.	A
ALL SURFACES RECEIVING A COURSE TEXTURED COATING SUCH AS PLASTER OR STUCCO, UNLESS NOTED OTHERWISE	B
ALL OTHER SURFACES, UNLESS NOTED OTHERWISE	C

FORMWORK STRIPPING:

COLUMNS & WALLS: COLUMNS AND WALLS NOT SUPPORTING FRAMING WEIGHT MAY BE STRIPPED AS SOON AS FORMS CAN BE REMOVED WITHOUT DAMAGING THE CONCRETE AND THE CONCRETE HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 500 PSI.

BEAMS & SLABS: BEAMS AND SLABS MAY BE STRIPPED AND BECOME SELF SUPPORTING AS SOON AS THEIR COMPRESSIVE STRENGTH REACHES 75% OF THE SPECIFIED DESIGN STRENGTH. RESHORING SHALL BE PROVIDED FOR ALL CONSTRUCTION LOADS THEREAFTER PER THE GENERAL CONTRACTOR.

COLD WEATHER PLACEMENT:

1. COLD WEATHER IS DEFINED BY ACI 306 AS "A PERIOD WHEN FOR MORE THAN 3 SUCCESSIVE DAYS THE MEAN DAILY TEMPERATURE DROPS BELOW 40° F."

2. NO CONCRETE SHALL BE PLACED ON FROZEN OR PARTIALLY FROZEN GROUND. THAWING THE GROUND WITH HEATERS IS PERMISSIBLE.

3. CONCRETE MIX TEMPERATURES SHALL BE AS SHOWN BELOW. HEATING OF WATER AND/OR AGGREGATES MAY BE REQUIRED TO ATTAIN THESE TEMPERATURES.

4. THE CONCRETE MAY REQUIRE PROTECTION FOR 4-7 DAYS AFTER POURING. IF TEMPERATURES REMAIN BELOW FREEZING, INSULATING BLANKET COVERAGE IS REQUIRED. IF TEMPERATURES ARE SLIGHTLY BELOW FREEZING (30° F MIN.) AT NIGHT AND ABOVE FREEZING DURING THE DAY, KRAFT PAPER WITH COMPLETE COVERAGE MAY BE USED IN LIEU OF INSULATED BLANKETS.

5. NO ADDITIVES CONTAINING CHLORIDES SHALL BE USED. USE "POZZUTEC 20+" BY MASTER BUILDERS OR "POLARSET" BY W.R. GRACE OR PRE-APPROVED EQUAL.

CONDITION OF PLACEMENT AND CURING		WALLS & SLABS	FOOTINGS
MIN. TEMP. FRESH CONCRETE AS MIXED FOR WEATHER INDICATED, DEGREES F.	ABOVE 30° F. 0° TO 30° F. BELOW 0° F.	60° 65° 70°	55° 60° 65°
MIN. TEMP. FRESH CONCRETE AS PLACED AND MAINTAINED, DEGREES F.		55°	50°
MAX. ALLOWABLE GRADUAL DROP IN TEMP. THROUGHOUT FIRST 24 HOURS AFTER END OF PROTECTION, DEGREES F.		50°	40°

STRUCTURAL DRAWING INDEX

THE APPROVED CONSTRUCTION PLANS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

Approval of submitted plans is not an approval of omissions or oversight by this office or noncompliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable building codes and regulations of the local government.

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

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JEFFREY S. KLEIN
STATE OF WASHINGTON
38554
PROFESSIONAL ENGINEER

GENERAL NOTES
Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S1.0
PERMIT SET

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HOT OR WINDY WEATHER PLACEMENT

HOT WEATHER IS DEFINED BY ACI 305 AS "ANY COMBINATION OF HIGH AIR TEMPERATURE, LOW RELATIVE HUMIDITY, AND WIND VELOCITY, TENDING TO IMPAIR THE QUALITY OF FRESH HARDENED CONCRETE. ACI 305 FIGURE 2.1.5 SHALL BE USED BY THE CONTRACTOR TO ESTIMATE THE RATE OF EVAPORATION. WHEN THE ESTIMATED RATE OF EVAPORATION IS GREATER THAN O.2 PSF/HOUR THE PLACEMENT SHALL BE CONSIDERED A HOT WEATHER PLACEMENT. PRECAUTIONS AGAINST PLASTIC SHRINKAGE CRACKING ARE NECESSARY. PRECAUTIONS TAKEN BY THE CONTRACTOR VARY DEPENDING UPON THE FACTORS ASSOCIATED WITH WATER EVAPORATION AND INCLUDE BUT ARE NOT LIMITED TO:

1. LIMITING CONCRETE TEMPERATURE TO 100°F AT TIME OF PLACEMENT.

2. APPLICATION OF AN EVAPORATION RETARDER.

3. USE OF FOG SPRAY.

4. REDUCTION OF POUR SIZE.

5. PLACING CONCRETE AT NIGHT.

CONDITION OF PLACEMENT AND CURING		WALLS & SLABS	FOOTINGS
MIN. TEMP. FRESH CONCRETE AS MIXED FOR WEATHER INDICATED, DEGREES F.	ABOVE 30° F. 0° TO 30° F. BELOW 0° F.	60° 65° 70°	55° 60° 65°
MIN. TEMP. FRESH CONCRETE AS PLACED AND MAINTAINED, DEGREES F.		55°	50°
MAX. ALLOWABLE GRADUAL DROP IN TEMP. THROUGHOUT FIRST 24 HOURS AFTER END OF PROTECTION, DEGREES F.		50°	40°

HOT OR WINDY WEATHER PLACEMENT

HOT WEATHER IS DEFINED BY ACI 305 AS "ANY COMBINATION OF HIGH AIR TEMPERATURE, LOW RELATIVE HUMIDITY, AND WIND VELOCITY, TENDING TO IMPAIR THE QUALITY OF FRESH HARDENED CONCRETE. ACI 305 FIGURE 2.1.5 SHALL BE USED BY THE CONTRACTOR TO ESTIMATE THE RATE OF EVAPORATION. WHEN THE ESTIMATED RATE OF EVAPORATION IS GREATER THAN O.2 PSF/HOUR THE PLACEMENT SHALL BE CONSIDERED A HOT WEATHER PLACEMENT. PRECAUTIONS AGAINST PLASTIC SHRINKAGE CRACKING ARE NECESSARY. PRECAUTIONS TAKEN BY THE CONTRACTOR VARY DEPENDING UPON THE FACTORS ASSOCIATED WITH WATER EVAPORATION AND INCLUDE BUT ARE NOT LIMITED TO:

1. LIMITING CONCRETE TEMPERATURE TO 100°F AT TIME OF PLACEMENT.

2. APPLICATION OF AN EVAPORATION RETARDER.

3. USE OF FOG SPRAY.

4. REDUCTION OF POUR SIZE.

5. PLACING CONCRETE AT NIGHT.

CONTROL AND CONSTRUCTION JOINTS

CONSTRUCTION JOINTS SHALL MEET THE REQUIREMENTS OF ACI 301 SECTIONS 2.2.2.5 AND 5.3.2.6. SPECIAL BONDING METHODS PER SECTION 5.3.2.6 SHALL BE SATISFIED BY ITEM 5 BELOW UNLESS OTHERWISE DETAILED ON THE STRUCTURAL DRAWINGS. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN ON PLAN OR ADDITIONAL CONSTRUCTION JOINTS ARE REQUIRED SUBMIT PROPOSED JOINTING FOR STRUCTURAL ENGINEERS APPROVAL. PROVIDE CONSTRUCTION JOINTS AS INDICATED BELOW UNLESS NOTED OTHERWISE ON THE PLANS:

1. SLABS ON GRADE: PROVIDE CONSTRUCTION AND/OR CONTROL JOINTS AT 16 FEET O.C. MAXIMUM FOR UNEXPOSED SLABS ON GRADE AND 12 FEET O.C. FOR EXPOSED SLABS ON GRADE. COORDINATE JOINTS WITH ARCHITECTURAL DRAWINGS.

2. WALLS AND COLUMNS: COORDINATE CONSTRUCTION JOINTS WITH ARCHITECTURAL REVEALS.

3. TOPPING OVER WOOD FRAMING: PROVIDE JOINTS AT 12' O.C. MAXIMUM.

4. BONDING AGENT: WHERE BONDING AGENT IS SPECIFICALLY CALLED OUT ON THE STRUCTURAL DRAWINGS USE "WELD CRETE" BY LARSON PRODUCTS CORPORATION OR PRE-APPROVED EQUAL. FOLLOW ALL MANUFACTURERS RECOMMENDATIONS.

5. ATTACHMENT OF NEW CONCRETE TO EXISTING: WHERE SHOWN, ROUGHEN CONCRETE TO A MINIMUM AMPLITUDE OF 1/4" USING IMPACT HAMMER. REMOVE ALL LOOSE OR DAMAGED CONCRETE, THOROUGHLY FLUSH ALL SURFACES WITH POTABLE WATER, AIR BLAST WITH OIL FREE COMPRESSED AIR TO REMOVE ALL WATER.

EMBEDDED ITEMS

1. NO ALUMINUM ITEMS SHALL BE EMBEDDED IN ANY CONCRETE.

2. ALL EMBED PLATES SHALL BE SECURELY FASTENED IN PLACE.

3. ALL EMBEDDED STEEL ITEMS EXPOSED TO EARTH SHALL BE GALVANIZED.

4. ALL EMBEDDED STEEL ITEMS EXPOSED TO WEATHER SHALL BE PAINTED UNLESS NOTED AS GALVANIZED. SEE DRAWINGS AND SPECIFICATIONS FOR PAINT, PRIMER, AND GALVANIZING REQUIREMENTS.

5. EMBEDDED CONDUIT IS NOT PERMITTED IN CONCRETE SLABS ON METAL DECK UNLESS SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS.

6. EMBEDDED FLEXIBLE CONDUIT IS PERMITTED IN OTHER CAST IN PLACE CONCRETE SLABS WITH A THICKNESS GREATER OR EQUAL TO 5-1/2 INCHES. WHERE PERMITTED IT MAY BE PLACED ON TOP OF THE BOTTOM MAT OF REINFORCING. THE OUTSIDE DIAMETER OF THE CONDUIT SHALL NOT BE GREATER THAN 1-INCH. A MINIMUM OF 2-INCHES CLEAR SHALL BE PROVIDED BETWEEN CONDUIT AND PARALLEL REINFORCING. SPACE CONDUITS A MINIMUM OF 12-INCHES APART, WHERE THIS IS NOT POSSIBLE NOTIFY ENGINEER FOR ADDITIONAL REINFORCING REQUIREMENTS.

CONCRETE CURING AND SEALING

CURING PROCEDURES SHALL COMMENCE IMMEDIATELY AFTER FINISHING CONCRETE TO MAINTAIN CONCRETE IN A MOIST CONDITION. VERIFY CURING AND/OR SEALING PRODUCTS ARE COMPATIBLE WITH FLOOR COVERINGS SHOWN ON THE ARCHITECTURAL DRAWINGS. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. SLABS ARE DEFINED AS SLABS ON GRADE, CONCRETE ON METAL DECK, ELEVATED POST-TENSIONED OR MILD REINFORCED DECKS, AND TOPPING SLABS.

ITEM	CONCRETE CURING NOTES
SLABS EXPOSED TO EARTH OR WEATHER OR VEHICLE OR FORKLIFT TRAFFIC INCLUDING LOADING DOCKS	1, (3 OR 4 OR 5), 6
ALL OTHER SLABS	1, (3 OR 4 OR 5)
FORMED SURFACES EXCLUDING FOUNDATIONS	2
SHOTCRETE WALLS	4
ALL OTHER CONCRETE	NONE

CONCRETE CURING NOTES:

1. WHEN THE ESTIMATED EVAPORATION RATE IS GREATER THAN 0.2 PSF/HOUR PROVIDE A SPRAY APPLIED EVAPORATION RETARDER IMMEDIATELY AFTER CONCRETE PLACEMENT. THE EVAPORATION RATE MAY BE CALCULATED PER ACI 305 FIGURE 2.1.5.

2. APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS TO ALL FORMED SURFACES IMMEDIATELY AFTER FINAL FORM REMOVAL. NOT REQUIRED IF FORMWORK REMAINS IN PLACE FOR MORE THAN 7 DAYS.

3. PROVIDE PRE-APPROVED CONTINUOUS WET CURE METHOD FOR A MINIMUM OF 14 DAYS.

4. APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS OR ASTM C1315 TYPE 1 CLASS A SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS IMMEDIATELY AFTER FINAL FINISHING. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR COVERINGS AND SEALERS.

5. PROVIDE 'ULTRACURE MAX' MOISTURE RETAINING COVER BY MCTECH GROUP, OR APPROVED EQUAL, FOR A MINIMUM OF 14 DAYS.

6. APPLY A SILANE SEALER WITH MINIMUM SOLIDS CONTENT OF 40% PER MANUFACTURER'S RECOMMENDATIONS.

GROUT

NON-SHRINK GROUT: MASTER BUILDERS "MASTERFLOW 928" OR PRE-APPROVED EQUAL. GROUT SHALL CONFORM TO CRD-C621 AND ASTM C1107 WHEN TESTED AT A FLUID CONSISTENCY PER CRD-C611-85 FOR 30 MINUTES. GROUT MAY BE PLACED FROM A 25 SECOND FLOW TO A STIFF PACKING CONSISTENCY. FILL OR PACK ENTIRE SPACE UNDER PLATES OR SHAPES. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR PREPARATION, INSTALLATION, AND CURING.

REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO:

ASTM A615, GRADE 60 TYPICAL UNLESS NOTED OTHERWISE.

DETAIL FABRICATE AND PLACE PER ACI 315 AND ACI 318.

WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185. LAP ONE FULL MESH ON SIDES AND ENDS BUT NOT LESS THAN 8 INCHES. WELDED WIRE REINFORCING SHALL BE SUPPORTED TO WITHSTAND CONCRETE PLACEMENT. PULLING OF MESH INTO PLACE AFTER PLACEMENT IS NOT ALLOWED.

REINFORCING SPLICE AND DEVELOPMENT LENGTH SCHEDULE, Fy=60 KSI (UNLESS NOTED OTHERWISE)

BAR SIZE	MINIMUM LAP SPLICE LENGTHS ("Ls")		MINIMUM DEVELOPMENT LENGTHS ("Ld")		MINIMUM EMBEDMENT LENGTH FOR STANDARD END HOOKS ("Ldh")
	TOP BARS (1)	OTHER BARS	TOP BARS (1)	OTHER BARS	
#3	2'-0"	1'-6"	1'-6"	1'-3"	0'-7"
#4	2'-8"	2'-0"	2'-0"	1'-7"	0'-9"
#5	3'-4"	2'-7"	2'-7"	2'-0"	1'-0"
#6	4'-0"	3'-1"	3'-1"	2'-4"	1'-2"
#7	5'-10"	4'-6"	4'-6"	3'-6"	1'-5"
#8	6'-8"	5'-2"	5'-2"	3'-11"	1'-7"

SPLICE TABLE NOTES:

1. "TOP BARS" ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM.

MECHANICAL COUPLERS: "LENTON" BY ERICO, "CADWELD" BY ERICO, "BAR-LOCK" BY DAYTON SUPERIOR L-SERIES, OR PRE-APPROVED EQUAL. COUPLERS SHALL BE TYPE 2 PER ACI 318 SECTION 18.2.7.1.

FORM SAVERS: "LENTON" BY ERICO THREADED FORM SAVERS TYPE FS OR APPROVED EQUAL.

STUD RAILS: "STUD RAILS" BY DECON INC. FLAT SLAB SHEAR REINFORCING RAILS OR APPROVED EQUAL.

REINFORCING STEEL COVER

PROVIDE CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH ---------- 3"
EXPOSED TO WEATHER OR EARTH ---------- 2"
TIES ON BEAMS AND COLUMNS ---------- 1-1/2"
WALLS AND SLABS NOT EXPOSED TO WEATHER---- 3/4"

CONCRETE INSERTS: THREADED DOWEL BAR SUBSTITUTIONS SHALL BE MANUFACTURED BY RICHMOND SCREW ANCHOR CO., INC., OR PRE-APPROVED EQUAL AND SHALL BE CAPABLE OF DEVELOPING THE FULL TENSILE CAPACITY OF THE BAR.

POST-INSTALLED ANCHORS

POST-INSTALLED ANCHORS: SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH REBAR. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. INSTALLER SHALL BE QUALIFIED AND TRAINED BY THE MANUFACTURER. HOLES SHALL BE HAMMER DRILLED ONLY (ROTARY DRILLED ONLY AT UNREINFORCED MASONRY - NO HAMMER TOOLS).

SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED FOR APPROVAL A MINIMUM OF 2 WEEKS PRIOR TO BID, ALONG WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER (LICENSED IN THE STATE IN WHICH THE PROJECT OCCURS) DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

CONCRETE ANCHORS:
- ADHESIVE ANCHORS: HILTI HIT-HY 200 (ICC-ESR-3187)
  - *CONCRETE SHALL BE A MINIMUM OF 21 DAYS OLD AT TIME OF INSTALLATION.
  - *CONCRETE SHALL BE IN THE TEMPERATURE RANGE AS REQUIRED BY THE CONCRETE MANUFACTURER.
  - *HOLE SHALL BY HAMMER-DRILLED ONLY.
  - *HOLE SHALL BE DRY AT TIME OF INSTALLATION.
  - *INSTALLER OF HORIZONTAL OR UPWARDLY INCLINED (ANY POSITION EXCEPT DIRECTLY DOWNWARD) ANCHORS SHALL ALSO BE CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.
- EXPANSION ANCHORS: KWIKBOLT TZ (ICC ESR-1917) BY HILTI, INC. OR STRONG-BOLT 2 (ICC ESR-3037) BY SIMPSON STRONG TIE, INC.
- SCREW ANCHORS: KWIK HUS-EZ (ICC ESR-3027) BY HILTI, INC. OR TITEN HD (ICC ESR-2713) BY SIMPSON STRONG TIE, INC.

CARPENTRY:

NAILS: CONNECTION DESIGNS ARE BASED ON "COMMON WIRE" NAILS WITH THE FOLLOWING PROPERTIES:

PENNYWEIGHT	DIAMETER (INCHES)	LENGTH (INCHES)	TRACKER** EMBOSSED HEAD / COLOR
8d	0.131	2-1/2	3 / BLUE
10d	0.148	3	4 / WHITE
16d	0.162	3-1/2	6 / ORANGE
20d	0.192	4	-

FOR DIAPHRAGM OR SHEAR WALL NAILING THE FOLLOWING FASTENER TYPES MAY BE USED AT EQUIVALENT SPACING TO THAT SPECIFIED ON PLANS:

FASTENER TYPE	DIAMETER (INCHES)	LENGTH (INCHES)	EQUIVALENT SPACING (INCHES)			TRACKER** EMBOSSED HEAD / COLOR
8d COMMON WIRE	0.131	2-1/2	6	4	3	3/ BLUE
8d "DIPPED GALV. BOX"	0.131	2-1/2	6	4	3	E3 / NONE
8d "SHINY BOX"	0.113	2-1/2	4-1/2	3	2-1/2	1 / BLUE
12 GA. STAPLES	0.1055	1-7/8*	6	5-1/2	4	-
14 GA. STAPLES	0.080	1-1/2*	6	4	3	-
15 GA STAPLES	0.072	1-1/2*	5	3	2-1/2	-
10d COMMON WIRE	0.148	3	6	4	3	4 / WHITE
10d "HOT DIPPED GALV. BOX"	0.148	3	6	4	3	F4 / NONE
10d "SHINY BOX"	0.128	3	4-1/2	3	2-1/4	3 / WHITE

*BASED ON 15/32" PLYWOOD OR OSB.
**REFERENCE TO EMBOSSED HEAD / COLOR CODED NAILS PER TRACKERS SYSTEM.

WOOD SHEATHING (STRUCTURAL): **SHEATHING ON ROOF SURFACES SHALL BE PLYWOOD ONLY**. SHEATHING ON FLOOR AND WALLS SHALL BE PLYWOOD OR ORIENTED STRAND BOARD (OSB). PLYWOOD SHEATHING SHALL BE 5-PLY MINIMUM WHERE INDICATED AS PERFORMANCE CATEGORY 3/4" OR THICKER. WOOD SHEATHING SHALL BE "STRUCTURAL I" CONFORMING TO PS1-09 AND/OR PS2-10. ALL PANELS SHALL BEAR THE STAMP OF AN APPROVED GRADING AGENCY. SPAN RATING SHALL BE PROVIDED AS FOLLOWS: ROOF FRAMING AT 32"O.C. (48/24); ROOF FRAMING AT 24"O.C. (32/16); WALLS (32/16); FLOORS (48/24) ALL WOOD SHEATHED WALLS SHALL BE BLOCKED AT ALL PANEL EDGES UNLESS NOTED OTHERWISE.

GLUE-LAMINATED MEMBERS: CONFORM TO ANSI/AITC A190.1. MEMBERS SHALL BE COMBINATION 24F-V4 DOUGLAS FIR (DF) FOR SIMPLE SPANS AND 24F-V8 DF FOR CANTILEVERED SPANS (Fb=2400 PSI, Fv=265 PSI, E= 1.8X10^6 PSI) AND DF COMBINATION 2 FOR COLUMNS.

TRUSSES SHALL HAVE A BALANCED LAY-UP FOR CHORDS AND COMBINATION 2 FOR WEBS.

MEMBERS INDICATED IN STRUCTURAL DRAWINGS AS "POC" SHALL BE PORT ORFORD CEDAR COMBINATION 22F-V/POC1 (Fb=2200 PSI, Fv=265 PSI, E=1.8X10^6 PSI) AND POC COMBINATION 2 FOR COLUMNS.

MEMBERS INDICATED IN STRUCTURAL DRAWINGS AS "AYC" SHALL BE ALASKAN YELLOW CEDAR COMBINATION 20F-V13 (Fb=2000 PSI, Fv=265 PSI, E=1.5X10^6 PSI) AND AYC COMBINATION 2 FOR COLUMNS.

MEMBERS INDICATED IN STRUCTURAL DRAWINGS AS "PPT" SHALL BE PRESERVATIVE PRESSURE TREATED COMBINATION 24F-V5 SOUTHERN PINE (SP) (Fb=2400 PSI, Fv=300 PSI, E=1.7X10^6 PSI) AND SP COMBINATION 2 FOR COLUMNS.

APPEARANCE GRADE WHERE EXPOSED TO VIEW; INDUSTRIAL APPEARANCE WHERE NOT EXPOSED TO VIEW. ALL MEMBERS TO HAVE EXTERIOR GLUE AND HAVE AN APPROVED GRADE STAMP. CAMBER AS SHOWN ON STRUCTURAL DRAWINGS.

FRAMING LUMBER: STANDARDS. EACH PIECE SHALL BEAR THE GRADE TRADEMARK OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB), WESTERN WOOD PRODUCTS ASSOCIATION (WWPA), OR OTHER AGENCY ACCREDITED BY THE AMERICAN LUMBER STANDARD COMMITTEE (ALSC) TO GRADE UNDER ALSC CERTIFIED GRADING RULES.

SPECIES AND GRADE (BASE DESIGN VALUE)
1. 6x BEAMS AND HEADERS. "DOUG FIR-LARCH" NO. 1 (Fb=1350 PSI, Fv=170 PSI)
2. 2x TO 4x JOISTS, PURLINS AND HEADERS. "DOUG FIR-LARCH" NO. 2 (Fb=900 PSI, Fv=180 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fv=150 PSI)
3. 6x POSTS AND COLUMNS. "DOUG FIR-LARCH" NO. 1 (Fc=1000 PSI)
4. EXTERIOR STUDS, INTERIOR BEARING WALLS AND 4x COLUMNS. "DOUG FIR-LARCH" NO. 2 (Fb= 900 PSI, Fc=1350 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI).
5. INTERIOR NON-BEARING STUD WALLS. "DOUG FIR-LARCH" NO. 2 (Fb=900 PSI. Fc=1350 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI)
6. 2x & 3x T&G DECKING: "DOUG FIR-LARCH" COMMERCIAL (Fb=1450 PSI, E=1700 KSI)
7. THE MINIMUM GRADE OF ALL OTHER STRUCTURAL FRAMING. "DOUG FIR-LARCH" NO. 2 (Fb= 900 PSI, Fc=1350 PSI), OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI).
8. UTILITY & STANDARD GRADES NOT PERMITTED.

STRUCTURAL COMPOSITE LUMBER (SCL): SHALL BE MANUFACTURED BY REDBUILT LLC., OR PRE-APPROVED EQUAL IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS CONFORMING TO A CURRENT EVALUATION REPORT.

MIINIMUM DESIGN VALUES:
1. 2x SCL: Fb = 1700 PSI, Fv = 285 PSI, E = 1300 KSI
2. 1-3/4" SCL: Fb = 2600 PSI, Fv = 285 PSI, E = 1800 KSI
3. 3-1/2" SCL: Fb = 2900 PSI, Fv = 285 PSI, E = 2000 KSI
4. 5-1/4" SCL: Fb = 2900 PSI, Fv = 285 PSI, E = 2000 KSI
5. RIMBOARD:APA/EWS PERFORMANCE RATED RIM (PRR-401) 1-1/4" MINIMUM THICKNESS

MEMBERS HAVE BEEN DESIGNED TO SERVICEABILITY AND OTHER PERFORMANCE BASED REQUIREMENTS, WHICH MAY EXCEED MINIMUM DESIGN LOADS AND CODE REQUIREMENTS. SUBSTITUTIONS MUST MEET OR EXCEED MOMENT, SHEAR, AND STIFFNESS OF THOSE MEMBERS SPECIFIED AT THE SAME DEPTH AND SPACING.

PRESERVATIVE TREATED WOOD REQUIREMENTS:

TREATMENTS OTHER THAN THOSE LISTED BELOW ARE NOT PERMITTED:

		APPLICATION	SPECIFIED MATERIAL	PRESERVATIVE TREATMENT (1)	CONNECTORS & FASTENERS (2)(3)
EXPOSURE	DRY	FOUNDATION SILL PLATES, TOP PLATES & LEDGERS ON CONCRETE OR MASONRY WALLS (4)	2x, 4x, 6x (FIR), OR GLULAM (SP)	SBX	GALV (G60)
				ACQ, CBA, CA	GALV (G185)
	WET	FRAMING, DECKING, POSTS & LEDGERS	2x, & 4x (FIR)	ACQ, CBA, CA	GALV (G185)
			2x, & 4x (CEDAR)	NONE	GALV (G90)
		BEAMS & COLUMNS	6x (FIR), OR GLULAM (SP)	ACQ, CBA, CA	GALV (G185)
			6x OR GLULAM (CEDAR)	NONE	GALV (G90)

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JEFFREY S. FRANK — STATE OF WASHINGTON — 35554 — PROFESSIONAL ENGINEER

GENERAL NOTES

Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S1.1

PERMIT SET

1. CCA: CHROMATED COPPER ARSENATE NOT PERMITTED
   SBX: DOT SODIUM BORATE
   ACQ: ALKALINE COPPER QUAT
   CBA & CA: COPPER AZOLE
   FIR: DOUG-FIR OR HEM-FIR
   SP: SOUTHERN PINE

2. CONNECTORS: JOIST HANGERS, STRAPS, FRAMING CONNECTORS, COLUMN CAPS AND BASES, ETC. FASTENERS: MACHINE BOLTS, ANCHOR BOLTS AND LAG SCREWS WITH ASSOCIATED PLATE WASHERS AND NUTS. NAILS, SPIKES, WOOD SCREWS, ETC.

3. G60, G90 & G185 PER ASTM A653 FOR COLD-FORMED STEEL CONNECTORS. BATCH/POST HOT-DIP GALVANIZED PER ASTM A123 FOR CONNECTORS AND ASTM A153 STRUCTURAL STEEL CONNECTORS. HOT-DIP GALVANIZED PER ASTM A153 FOR FASTENERS OR MECHANICALLY GALVANIZED FASTENERS PER ASTM B695, CLASS 55 OR GREATER.

4. AT CONTRACTORS OPTION, LEDGERS AND TOP PLATES A MINIMUM OF 8 FEET ABOVE GRADE ON CONCRETE OR MASONRY WALLS MAY BE UN-TREATED IF COMPLETELY SEPARATED FROM THE WALL BY A SELF ADHERING ICE & WATER SHIELD BARRIER (40 MIL MINIMUM).

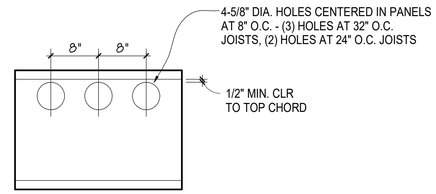
GENERAL REQUIREMENTS: PROVIDE MINIMUM NAILING PER IBC TABLE 2304.10.1 OR MORE, AS OTHERWISE SHOWN. STAGGER ALL NAILING TO PREVENT SPLITTING OF WOOD MEMBERS. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED WITH THE EXCEPTION OF INTERIOR CONCRETE TOPPINGS ON WOOD FLOOR SYSTEMS. HOLES AND CUTS IN 3x OR 4x PLATES SHOULD BE TREATED WITH A 9% SOLUTION OF COPPER NAPHTHENATE. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. PROVIDE CUT WASHERS WHERE BOLT HEADS, NUTS AND LAG SCREW HEADS BEAR ON WOOD. PROVIDE A MINIMUM 3"x3"x0.229" PLATE WASHER ON ALL ANCHOR BOLTS WHICH CONNECT MUD SILLS TO FOUNDATION. DO NOT NOTCH OR DRILL STRUCTURAL MEMBERS, EXCEPT AS ALLOWED BY IBC SECTIONS 2308.4.2.4, 2308.5.9, 2308.5.10 AND 2308.7.4 OR AS RESTRICTED BY PLANS OR DETAILS, OR AS APPROVED PRIOR TO INSTALLATION. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

WOOD SHRINKAGE AND CONSOLIDATION: SHRINKAGE OF WOOD MEMBERS AND CONSOLIDATION OF BEARING WALLS IS EXPECTED FROM TIME OF FRAMING UNTIL AFTER BUILDING IS PUT IN SERVICE. MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS SHALL BE CONSTRUCTED TO ACCOMODATE 1/4" OF TOTAL SETTLEMENT PER STORY.

FRAMING CONNECTORS: SHALL CONFORM TO CURRENT EVALUATION REPORT AND BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, SAN LEANDRO, CA., OR PRE-APPROVED EQUAL. PROVIDE MAXIMUM SIZE AND QUANTITY OF NAILS OR BOLTS PER MANUFACTURER, EXCEPT AS NOTED OTHERWISE. PROVIDE LEAD HOLES AS REQUIRED TO PREVENT SPLITTING OF WOOD MEMBERS. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

LAG SCREWS: SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. LAG SCREWS SHALL BE OF A DIAMETER INDICATED ON DRAWINGS WITH A MINIMUM OF 8x DIA. EMBEDMENT IN SUPPORTING MEMBER UNLESS NOTED OTHERWISE. CLEARANCE HOLE FOR THE SHANK SHALL BE THE SAME DIAMETER AS THE SHANK AND THE SAME DEPTH OF PENETRATION AS THE UNTHREADED PORTION OF THE SHANK. THE LEAD HOLE FOR THE THREADED PORTION SHALL HAVE A DIAMETER EQUAL TO 60 TO 75 PERCENT OF THE SHANK DIAMETER AND A LENGTH EQUAL TO AT LEAST THE LENGTH OF THE THREADED PORTION. THE THREADED PORTION OF THE SCREW SHALL BE INSERTED IN ITS LEAD HOLE BY TURNING WITH A WRENCH. SOAP OR OTHER LUBRICANT SHALL BE USED ON THE SCREWS OR IN THE LEAD HOLE TO FACILITATE INSERTION AND PREVENT DAMAGE TO THE SCREW. LAG SCREWS SHALL NOT BE DRIVEN WITH A HAMMER. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

I-JOISTS: SHALL BE MANUFACTURED BY REDBUILT LLC, OR PRE-APPROVED EQUAL IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS. MEMBERS SHALL BE DESIGNED UNDER THE DIRECT SUPERVISION OF A STRUCTURAL ENGINEER LICENSED IN THE STATE OF PROJECT. THE ENTIRE OPEN-WEB TRUSS/I-JOIST ASSEMBLY SHALL BE AS APPROVED BY CURRENT EVALUATION REPORT. MEMBERS SHALL BE DESIGNED TO CARRY THE LOADS LISTED IN THE DESIGN CRITERION AND ANY ADDITIONAL LOADS INDICATED ON THE FRAMING PLANS AND DETAILS. THE TRUSS ENGINEER SHALL ASSUME ALL RESPONSIBILITY FOR THE WORK OF ALL SUBORDINATES INVOLVED IN THE PREPARATION OF THE TRUSS PLACEMENT PLANS AND TRUSS DESIGN DRAWINGS. TRUSSES/I-JOISTS SHALL BE PROVIDED TO COMPLETE THE ROOF AND/OR FLOOR FRAMING FROM THE SHEATHING TO THE SUPPORTING MEMBERS BELOW. MEMBER DESIGNATIONS ON PLANS ARE FOR TYPICAL UNIFORMLY LOADED CONDITIONS. MANUFACTURER SHALL PROVIDE ADDITIONAL MEMBERS AS REQUIRED TO SUPPORT SPECIAL LOADING CONDITIONS INDICATED ON DRAWINGS. TOP CHORD AT STRAP CONNECTIONS TO CONCRETE OR MASONRY WALLS SHALL BE COMPOSED OF A STRUCTURAL COMPOSITE LUMBER MEMBER APPROVED BY A CURRENT EVALUATION REPORT FOR SUCH A USE OR AT CONTRACTORS OPTION, STRAP NAIL HOLES SHALL BE PRE-DRILLED IN CHORD. PROVIDE SHOP AND INSTALLATION DRAWINGS AND CALCULATIONS PRODUCED UNDER THE SUPERVISION OF AND BEARING THE STAMP OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF PROJECT. DETAIL DRAWINGS TO INDICATE MEMBER TYPES, SIZE, SPACING, BRIDGING, BLOCKING, CONNECTIONS, ANCHORING, BEARING PLATE AND OTHER PERTINENT DETAILS. PROVIDE 1 1/2" DIA. OPEN KNOCKOUTS AT 12" O.C. ON ALL ROOF I-JOISTS.



TYPICAL I-JOIST VENTED BLOCKING
NO SCALE

MEMBER DESIGN CALCULATIONS SHALL BE PROVIDED FOR STANDARD LOADING ALONG WITH DESIGN CHECKS FOR SPECIAL LOADING CONDITIONS WHICH INCLUDE FREE BODY DIAGRAMS, LOADING BREAK DOWN, DESCRIPTION OF LOADS (I.E. MECH UNIT, SUSPENDED WALL, ETC.) AND THE RATIONALE FOR LOADING DISTRIBUTION ON MULTIPLE MEMBERS. SUBMITTAL SHALL ALSO PROVIDE ANY DOCUMENTATION NECESSARY TO INTERPRET DATA INDICATED ON CALCULATIONS.

MEMBERS HAVE BEEN DESIGNED TO MEET SERVICEABILITY AND OTHER PERFORMANCE BASED REQUIREMENTS, WHICH MAY EXCEED MINIMUM DESIGN LOADS AND CODE REQUIREMENTS. SUBSTITUTIONS MUST MEET OR EXCEED MOMENT, SHEAR, AND STIFFNESS OF THOSE MEMBERS SPECIFIED AT THE SAME DEPTH AND SPACING.

REFER TO THE FRAMING CONNECTORS SECTION OF THESE GENERAL NOTES FOR REQUIREMENTS PLACED UPON CONNECTOR HARDWARE SPECIFIED BY TRUSS ENGINEER AND/OR PROVIDED BY TRUSS MANUFACTURER.

SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA 13 AND COMMERCIAL PUBLICATION "SPRINKLER SYSTEM INSTALLATION WITH GUIDELINES FOR REDBUILT OPEN-WEB TRUSSES AND I-JOISTS". LOADS HUNG FROM JOIST NOT SPECIFICALLY IDENTIFIED ON STRUCTURAL DRAWINGS SHALL NOT EXCEED 30 POUNDS AT ANY ONE POINT, NOR SHALL TOTAL LOADS IN POUNDS ON ANY ONE JOIST EXCEED 8 TIMES THE JOIST SPAN IN FEET, UNLESS DETAILED OTHERWISE ON THE DRAWINGS. ATTACHMENT OF LOADS EXCEEDING 90 POUNDS SHALL BE APPROVED PRIOR TO INSTALLATION. DO NOT NOTCH OR DRILL THRU TRUSS MEMBERS.

MISCELLANEOUS:

PRE-APPROVED SUBSTITUTIONS: SUBSTITUTIONS MAY BE ALLOWED ONLY IF THEY MEET THE REQUIREMENTS OF THESE GENERAL NOTES AND THE SPECIFICATIONS, AND IF COMPLETE WRITTEN ENGINEERING DATA FOR EACH CONDITION REQUIRED FOR THIS PROJECT IS PROVIDED TO THE STRUCTURAL ENGINEER TWO WEEKS PRIOR TO BID DATE AND APPROVED IN WRITTEN ADDENDA BY THE ARCHITECT. DATA IS TO INDICATE CODE BASIS BY YEAR, AUTHORITY FOR STRESSES AND STRESS INCREASES, IF ANY, AND AMOUNT OF EXPECTED DEFLECTION FOR FLEXURAL MEMBERS UNDER (1) TOTAL LOAD AND (2) LIVE LOAD ONLY. ALL INCREASED COSTS IN MECHANICAL, SPRINKLER, ELECTRICAL OR GENERAL INSTALLATION AND ANY ARCHITECTURAL OR STRUCTURAL REDESIGN RESULTING FROM SUBSTITUTION SHALL BE BORNE BY THE GENERAL CONTRACTOR.

SHOP DRAWINGS/SUBMITTALS

THE FOLLOWING SHOP DRAWINGS/SUBMITTALS SHALL BE PROVIDED FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION OR DELIVERY.

		STRUCTURAL ENGR.	BLDG. DEPT.
1.	CONCRETE MIX DESIGNS	X	X
2.	REINFORCING STEEL SHOP DRAWINGS	X	
3.	VENEER ANCHORAGE SYSTEMS	X	X
4.	GLU-LAMINATED MEMBERS	X	X
5.	STRUCTURAL COMPOSITE LUMBER	X	X
6.	CONDUIT EMBEDDED IN CONCRETE	X	X
7.	CONTRACTOR'S STATEMENT OF RESPONSIBILITY	X	X
8.	SLAB COORDINATION DRAWINGS		

DEFERRED SUBMITTALS

THE FOLLOWING ARE NOT INCLUDED WITH THE BUILDING PERMIT DRAWINGS AND SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL AS A DEFERRED SUBMITTAL. SUBMITTALS SHALL BEAR THE SEAL OF AN ENGINEER LICENSED IN THE STATE OF THE PROJECT AS NOTED.

		ENGINEER STAMP REQUIRED
1.	I-JOISTS	PE
2.	SOLDIER PILE WALL	PE
3.	METAL STAIR	PE

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED BY AN INDEPENDENT TESTING LABORATORY PER THE REQUIREMENTS OF IBC CHAPTER 17 AND THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION AND THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS AND A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL FOR THE ITEMS LISTED IN THE QUALITY ASSURANCE/SPECIAL INSPECTION SECTION:

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GENERAL NOTES

Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S1.2

PERMIT SET

STATEMENT OF SPECIAL INSPECTIONS:

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED PER THE REQUIREMENTS OF IBC SECTION 1704 AND 1705 AND AS NOTED HEREIN.

STRUCTURAL SYSTEM	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	COMMENTS	REFERENCES
SOILS	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		X		IBC 1705.6
	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		X		
	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		X		
	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X			
	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		X		
CONCRETE	REINFORCING STEEL AND PLACEMENT		X	SPECIAL INSPECTIONS NOT REQUIRED FOR THE FOLLOWING CONDITIONS: NON-STRUCTURAL SLAB ON GRADE	ACI 318: CH 20, 25.2, 25.3, 26.6-1 TO 26.6-3, IBC 1908.4
	ANCHORS CAST IN CONCRETE-PRIOR TO AND DURING PLACEMENT OF CONCRETE		X		ACI 318: 17.8.2 AISC 360 SECTION N7
	ANCHORS POST-INSTALLED IN HARDENED CONCRETE		X		ACI 318: 3.8.6, 8.1.3, 21.2.8 IBC 1909.1
	VERIFY USE OF REQUIRED DESIGN MIX		X		ACI 318, CH 19
	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X			ASTM C172, C31 ACI 318: 26.4, 26.12 IBC 1908.10
	CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION	X			ACI 318: 26.5 IBC 1908.6, 1908.7, 1908.8
	MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X		ACI 318: 26.5.3 TO 26.5.5 IBC 1908.9
	ERECTION OF PRECAST CONCRETE		X		ACI 318: 26.8
	VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		X		ACI 318: 26.11.2
	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X		ACI 318: 26.11.1.2(b)
	MATERIAL VERIFICATION OF REINFORCEMENT STEEL FOR ASTM A615 REINFORCING		X	MANUFACTURER SHALL PROVIDE MILL TEST REPORTS. CONTINUOUS INSPECTION FOR ALL WELDS GREATER THAN 5/16" FILLET. PERIODIC INSPECTION FOR FILLET WELD 5/16" AND SMALLER	ACI 318: 26.6.4 AWS D1.4 IBC 1705.3.1
	TESTING OF MATERIALS		X		IBC 1705.3.2
WOOD FRAMING	SHEAR WALL NAILING		X	SPECIAL INSPECTION NOT REQUIRED FOR FASTENER SPACING > 4" O.C.	IBC 1705.11.1, 1705.12.2, 1705.5
	DIAPHRAGM NAILING		X	SPECIAL INSPECTION NOT REQUIRED FOR FASTENER SPACING > 4" O.C.	IBC 1705.11.1, 1705.12.2, 1705.5
	NAILING, BOLTING, AND ANCHORAGE OF COMPONENTS THAT ARE PART OF DRAG STRUTS, BRACES AND HOLD-DOWNS THAT ARE PART OF THE SEISMIC RESISTING SYSTEM		X		IBC 1705.11.1, 1705.12.2
	METAL-PLATE-CONNECTED WOOD TRUSSES SPANNING 60 FEET OF GREATER		X	TEMPORARY AND PERMANENT BRACING	IBC 1705.5.2

TESTING AND SPECIAL INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A DAILY BASIS WHENEVER WORK IS PERFORMED ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO OWNER, CONTRACTOR, BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.

STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNATED REPRESENTATIVE IN ACCORDANCE WITH IBC 1704.6. STRUCTURAL OBSERVATION SHALL BE PERFORMED AS FOLLOWS:

- PERIODIC VISUAL OBSERVATION OF STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES.
- REVIEW OF TESTING AND INSPECTION REPORTS.
- REPORTS SHALL BE PREPARED FOR EACH SITE VISIT AND SHALL BE DISTRIBUTED TO ARCHITECT.

GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL INCLUDE ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

ABBREVIATION LIST					
@	AT	ELEV.	ELEVATOR	N.F.	NEAR FACE
A.B.	ANCHOR BOLT	ENGR.	ENGINEER	N.S.	NEAR SIDE
ADD'L	ADDITIONAL	EQ.	EQUAL	NTS	NOT TO SCALE
A.F.F.	ABOVE FINISH FLOOR	E.W.	EACH WAY	O.C.	ON CENTER
ALT.	ALTERNATE	EXP.	EXPANSION	OPN'G	OPENING
ARCH.	ARCHITECTURAL	EXT.	EXTERIOR	OPP.	OPPOSITE
BLD'G	BUILDING	FDN	FOUNDATION	P.A.F.	POWDER ACTUATED FASTENER
BLK'G	BLOCKING	F.F.	FAR FACE	PERP.	PERPENDICULAR
BM	BEAM	FLR	FLOOR	PL	PLATE
B.O.F.	BOTTOM OF FOOTING	F.O.M.	FACE OF MASONRY	P.P.	PARTIAL PENETRATION
BOT.	BOTTOM	F.O.S.	FACE OF STUD	P.P.T.	PRESERVATIVE PRESSURE TREATED
BRG	BEARING	FRM'G	FRAMING	P.S.F.	POUNDS PER SQUARE FOOT
BTWN	BETWEEN	F.R.T.	FIRE RETARDANT TREATED	PSL	PARALLAM
B.U.	BUILT UP	F.S.	FAR SIDE	P.T.	POST TENSION
(C=)	CAMBER	FTG	FOOTING	PW.	PLYWOOD
CANT.	CANTILEVER	GA.	GAGE/GAUGE	REINF.	REINFORCING
C.F.S.	COLD-FORMED STEEL	GALV.	GALVANIZED	REQ'D	REQUIRED
C.J.	CONTROL/CONSTRUCTION JOINT	GL.	GLULAM	SCHED.	SCHEDULE
℄	CENTERLINE	GR.	GRADE	S.C.L.SHT'G	STRUCTURAL COMPOSITE LUMBER
CLR.	CLEARANCE	GWB	GYPSUM WALL BOARD	SHT'G	SHEATHING
CMU	CONCRETE MASONRY UNIT	HDR	HEADER	SIM.	SIMILAR
COL.	COLUMN	HGR	HANGER	S.O.G.	SLAB ON GRADE
CONC.	CONCRETE	HORIZ.	HORIZONTAL	SQ.	SQUARE
CONN.	CONNECTION	HSS	HOLLOW STRUCTURAL SECTION	STD	STANDARD
CONST.	CONSTRUCTION	HT	HEIGHT	STIFF.	STIFFENER
CONT.	CONTINUOUS	INT.	INTERIOR	STL	STEEL
CONTR.	CONTRACTOR	JST	JOIST	STRUCT.	STRUCTURAL
COORD.	COORDINATE	JT	JOINT	T&B	TOP & BOTTOM
C.P.	COMPLETE PENETRATION	L	ANGLE	T&G	TONGUE AND GROOVE
CTR'D	CENTERED	L.L.	LIVE LOAD	THR'D	THREADED
C.Y.	CUBIC YARD	LLH	LONG LEG HORIZONTAL	T.O.F.	TOP OF FOOTING
DBL.	DOUBLE	LLV	LONG LEG VERTICAL	T.O.S.	TOP OF STEEL
D.F.	DOUGLAS FIR	LOC.	LOCATION	TRT'D	TREATED
DIA. OR ϕ	DIAMETER	LSL	LAMINATED STRAND LUMBER	TYP.	TYPICAL
DIAG.	DIAGONAL	LVL	LAMINATED VENEER LUMBER	U.N.O.	UNLESS NOTED OTHERWISE
DIM.	DIMENSION	MAX.	MAXIMUM	U.T.	ULTRASONIC TESTED
D.L.	DEAD LOAD	M.B.	MACHINE BOLT	VERT.	VERTICAL
DWG	DRAWING	MECH.	MECHANICAL	W/	WITH
DWL	DOWEL	MEZZ.	MEZZANINE	W.P.	WORK POINT
(E)	EXISTING	MFR	MANUFACTURER	WT	WEIGHT
EA.	EACH	MIN.	MINIMUM	W.W.R.	WELDED WIRE REINFORCING
E.F.	EACH FACE	MISC.	MISCELLANEOUS		
EL.	ELEVATION	MTL	METAL		

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JEFFREY S. KLEIN — STATE OF WASHINGTON — 35554 — PROFESSIONAL ENGINEER

GENERAL NOTES

Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S1.3

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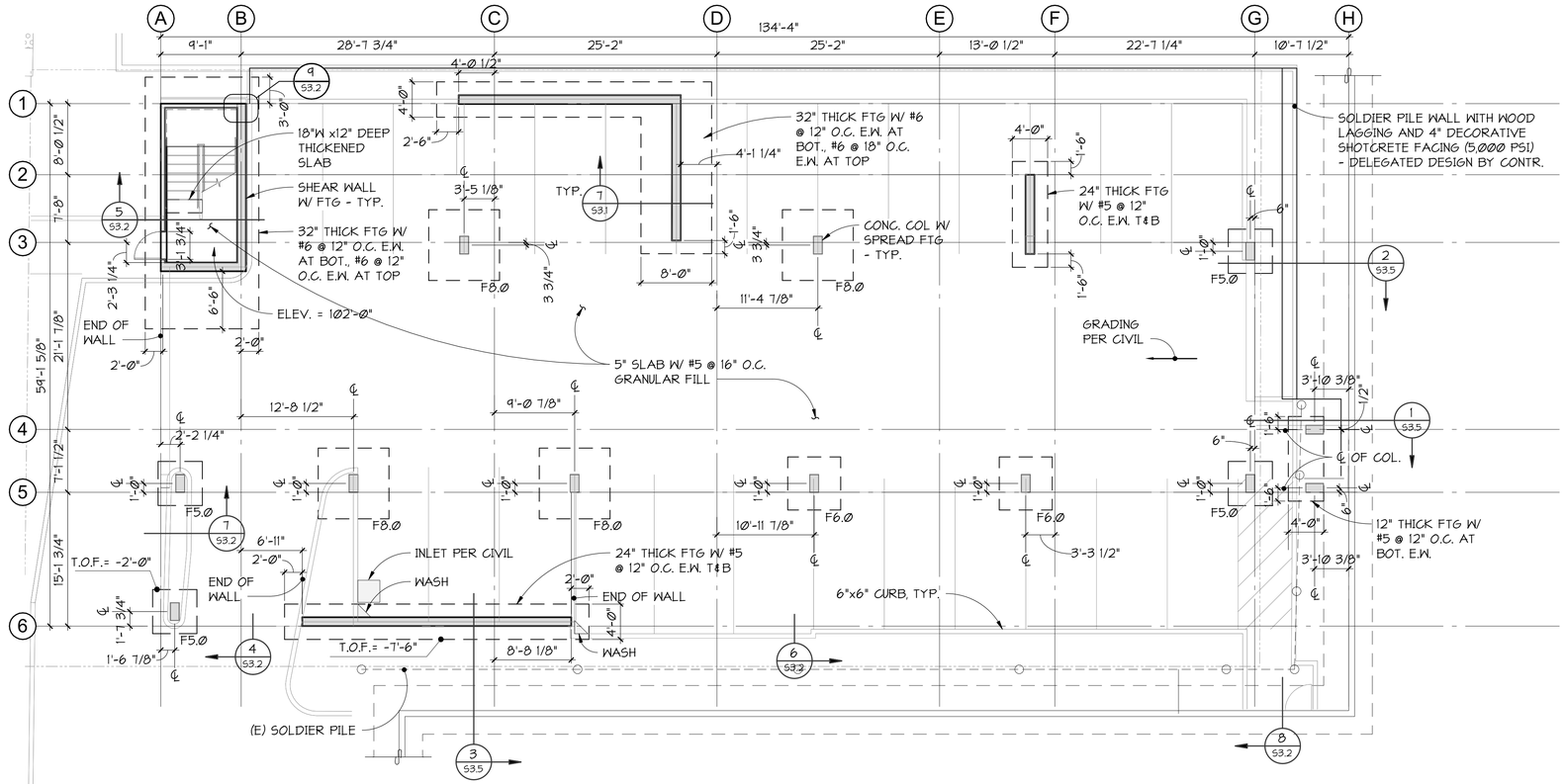

FOUNDATION PLAN

NORTH 1 S2.0 — 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
2. INDICATES 12" CONCRETE WALL WITH #5 @ 12" O.C. EACH FACE EACH WAY, UNLESS NOTED OTHERWISE ON PLAN. FOR TYPICAL DETAILS SEE 7/S3.1, 4/S3.3, 5/S3.3.
3. INDICATES CONCRETE COLUMN, SEE 3/S3.3 FOR COLUMN SCHEDULE.
4. TOP OF SLAB = 0'-0" (USGS 99'-0") UNLESS NOTED OTHERWISE.
5. FOR TYPICAL CONCRETE SLAB-ON-GRADE DETAILS SEE SHEET 1/S3.2.
6. FOR TYPICAL GRADE BEAM ELEVATION AND DETAILS SEE 6/S3.1.
7. "F_" INDICATES CONCRETE SPREAD FOOTING. FOR SCHEDULE SEE 1/S3.3.
8. TOP OF FOOTING ELEVATIONS = -1'-0" UNLESS NOTED OTHERWISE ON PLANS AND DETAILS.

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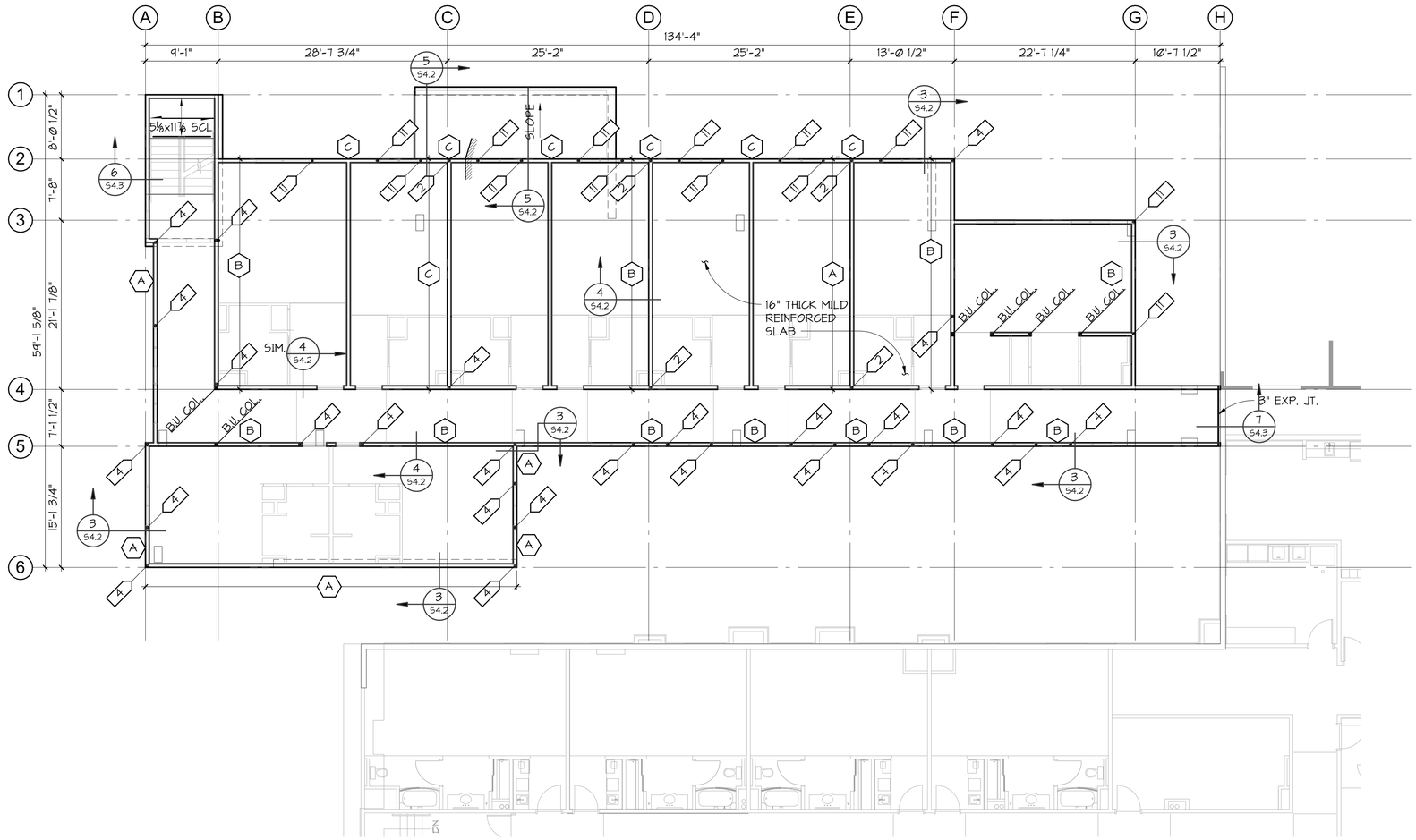

NORTH 1 S2.1

LEVEL 2 FRAMING PLAN

1/8" = 1'-0"

WOOD FRAMING PLAN NOTES:

1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
2. INDICATES WOOD STUD WALL. WOOD STUDS SHOULD BE 2x6 @ 16" ON CENTER MAXIMUM UNLESS NOTED OTHERWISE. SEE NOTE #5 FOR ADDITIONAL SHEAR WALL NAILING. EXTERIOR STUD WALLS PER SCHEDULE. UNLESS NOTED OTHERWISE.
3. B.U. COL. INDICATES WOOD STUD BUILT-UP COLUMN. SEE 3/S4.0 FOR TYPICAL DETAIL.
4. FOR TYPICAL FRAMING REQUIREMENTS AT OPENING IN STRUCTURAL WALLS SEE 1/S4.0 AND 2/S4.0 FOR TYPICAL DETAIL.
5. X OR XY INDICATES SPECIAL WOOD STUD WALL TYPE. SEE 6/S4.0 FOR SCHEDULE.
6. INDICATES HOLDOWN. SEE 5/S4.0 AND 1/S4.2 FOR SCHEDULE.
7. INDICATES WALL EXTENDING TO ROOF STRUCTURE.
8. INDICATES TYPICAL HEADER IN WALL BELOW. SEE _/S_. _. RIM SERVES AS HEADER AT EXTEIOR HEADER UNLESS NOTED OTHERWISE.
9. INDICATES CONCRETE COLUMN BELOW.
10. PROVIDE 23/32" T&G WOOD SHEATHING OVER ENTIRE FLOOR STRUCTURE WITH 1¼" GYPCRETE & 1/4" SOUNDMAT TOPPING. NAIL WOOD FLOOR SHEATHING WITH 10d @ 6" O.C. AT ALL SUPPORTED PANEL EDGES AND 10d @ 12" O.C. AT INTERMEDIATE FRAMING. TYPICAL UNLESS NOTED OTHERWISE. SEE 4/S4.0 FOR NAILING.
11. NON-STRUCTURAL STUD WALLS ARE NOT SHOWN OR SHOWN SCREENED. FOR LOCATION SEE ARCHITECTURAL DRAWINGS. FOR BRACING AT TOP OF WALL SEE 6/S4.2 AND 7/S4.2.

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LEVEL 2 FRAMING PLAN
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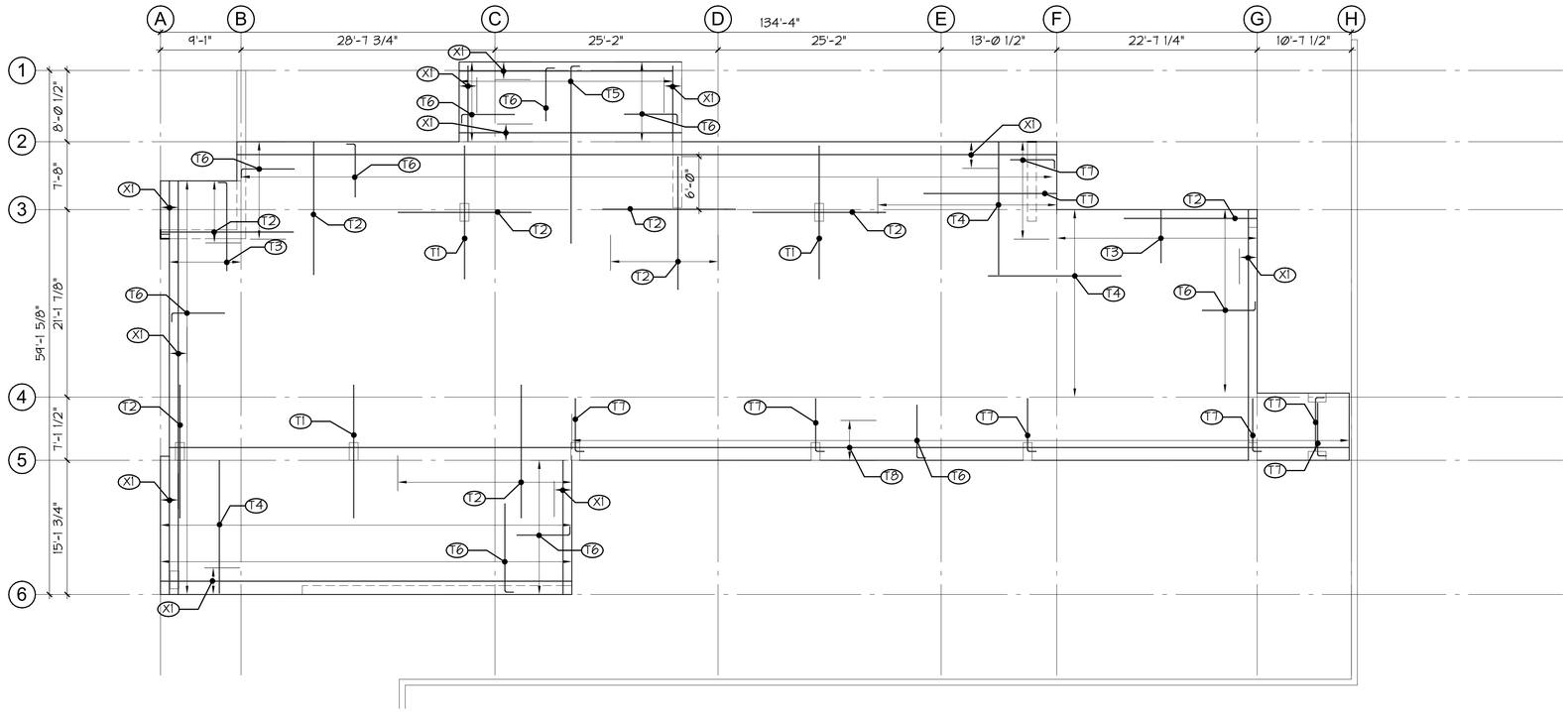


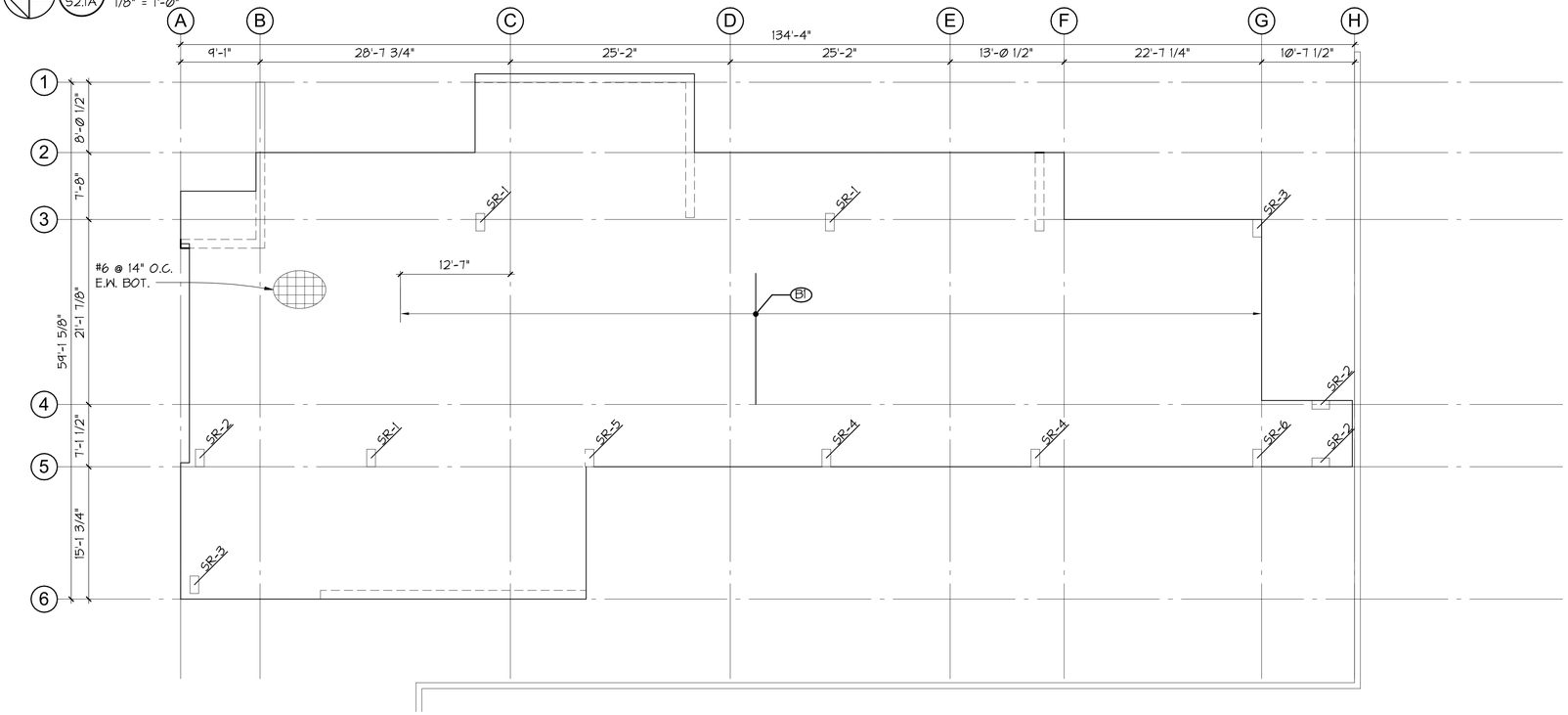
NORTH 1 S2.1A
LEVEL 2 TOP REINFORCING PLAN
1/8" = 1'-0"

MILD STEEL REINFORCEMENT SCHEDULE		
BOTTOM BARS		
MARK	REINFORCEMENT	REMARKS
B1	#4x15'-0" @ 24" O.C.	
DIAPHRAGM BARS		
MARK	REINFORCEMENT	REMARKS
X1	(3) #8 @ 8" O.C. CONT.	PLACE TO SUPPORT HOOKED BAR
TOP BARS		
MARK	REINFORCEMENT	REMARKS
T1	(15) #8x15'-0" @ 12" O.C.	
T2	(10) #8x15'-0" @ 12" O.C.	
T3	#8x10'-0" @ 24" O.C.	STD HOOK AT END
T4	#8x15'-0" @ 24" O.C.	
T5	(12) #8x20'-0"	STD HOOK AT END
T6	#5x6'-0" @ 12" O.C.	EXCULDE EDGE REINF. AT COL. TOP REINF., STD HOOK AT END
T7	(12) #8x6'-0" @ 12" O.C.	STD HOOK AT END
T8	(8) #8 @ 8" O.C. CONT.	STD HOOK AT END
T9	#8 @ 12" O.C. CONT.	

3 S2.1A SCHEDULE
NO SCALE

CONCRETE FRAMING NOTES:

1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
2. INDICATES NON-STRUCTURAL STUD WALLS. ALL WALLS ARE NOT SHOWN. FOR LOCATION SEE ARCHITECTURAL.
3. INDICATES STRUCTURAL WALL BELOW EXTENDING TO FLOOR STRUCTURE.
4. B1 INDICATES REINFORCEMENT IN SLAB, SEE 3/S2.1A FOR REINFORCEMENT.
5. "SR-1" INDICATES STUD RAIL, SEE 1/S3.4 FOR SCHEDULE & DETAIL.
6. TOP OF SLAB = 11'-7⅛" UNLESS NOTED OTHERWISE.



NORTH 2 S2.1A
LEVEL 2 BOTTOM REINFORCING PLAN
1/8" = 1'-0"

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LEVEL 2 BOTTOM & TOP REINFORCING PLANS

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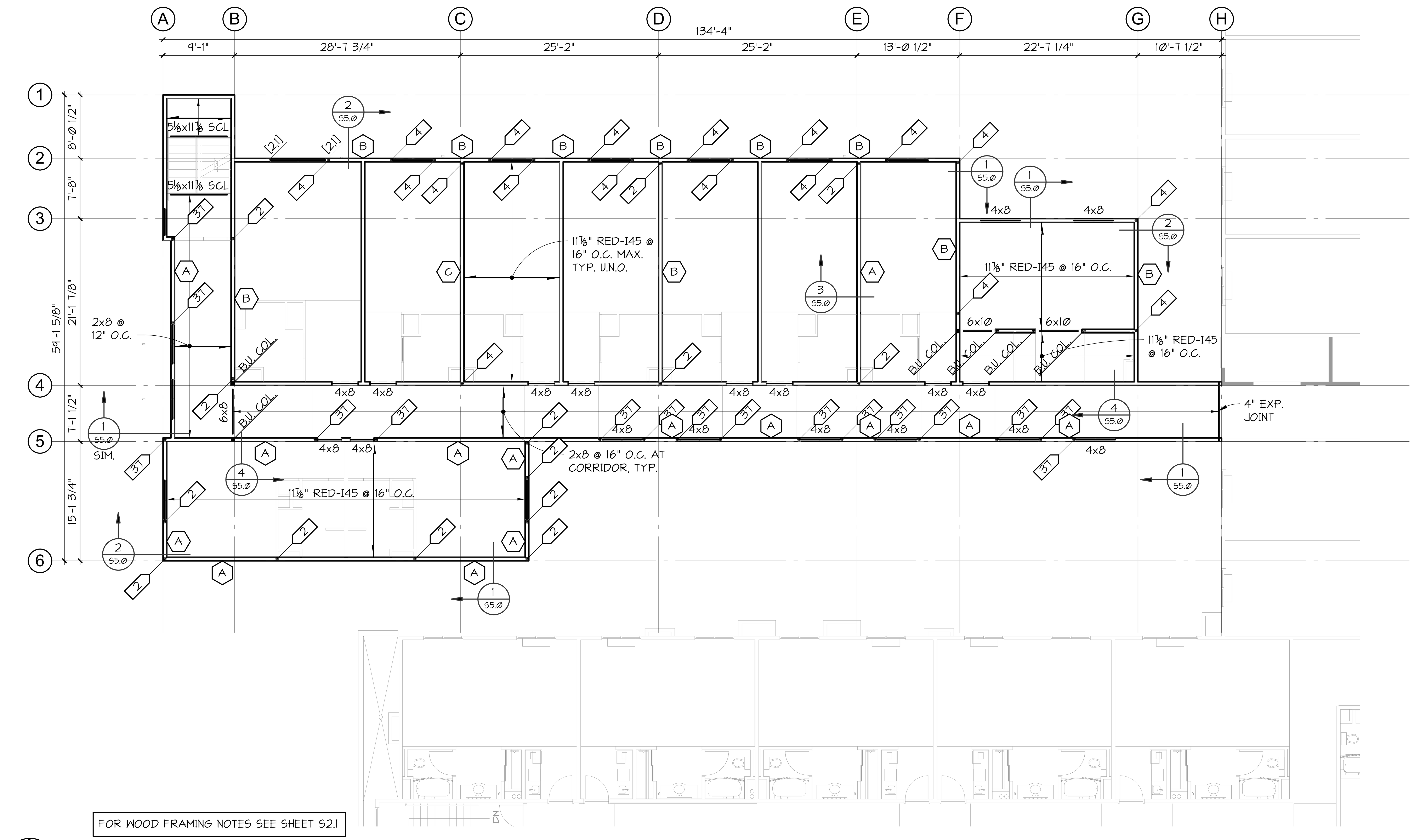

FOR WOOD FRAMING NOTES SEE SHEET S2.1

1 LEVEL 3 FRAMING PLAN

S2.2 1/8" = 1'-0"

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LEVEL 3 FRAMING PLAN

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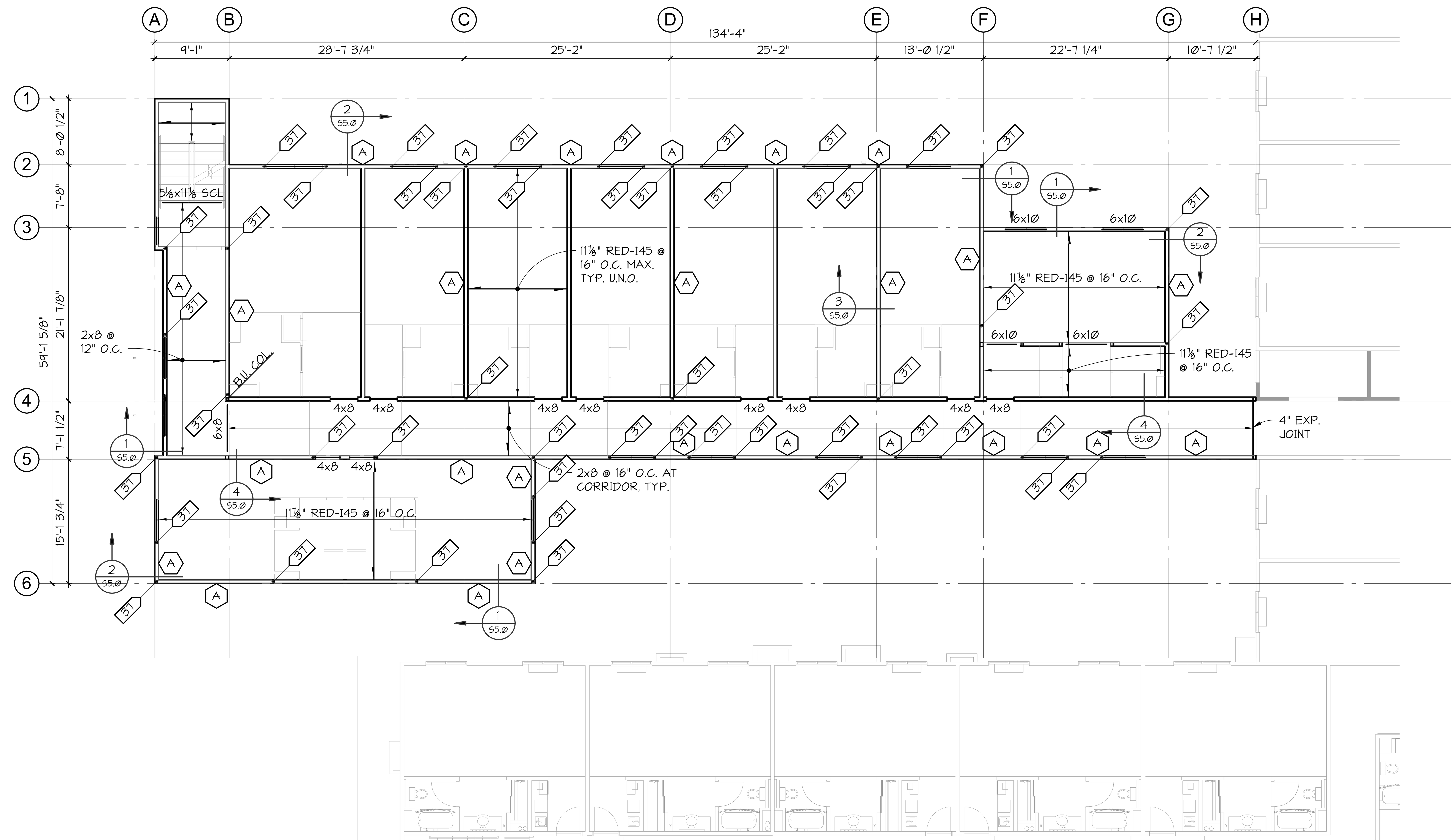

FOR WOOD FRAMING NOTES SEE SHEET S2.1

LEVEL 4 FRAMING PLAN

1 / S2.3 — NORTH

1/8" = 1'-0"

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LEVEL 4 FRAMING PLAN

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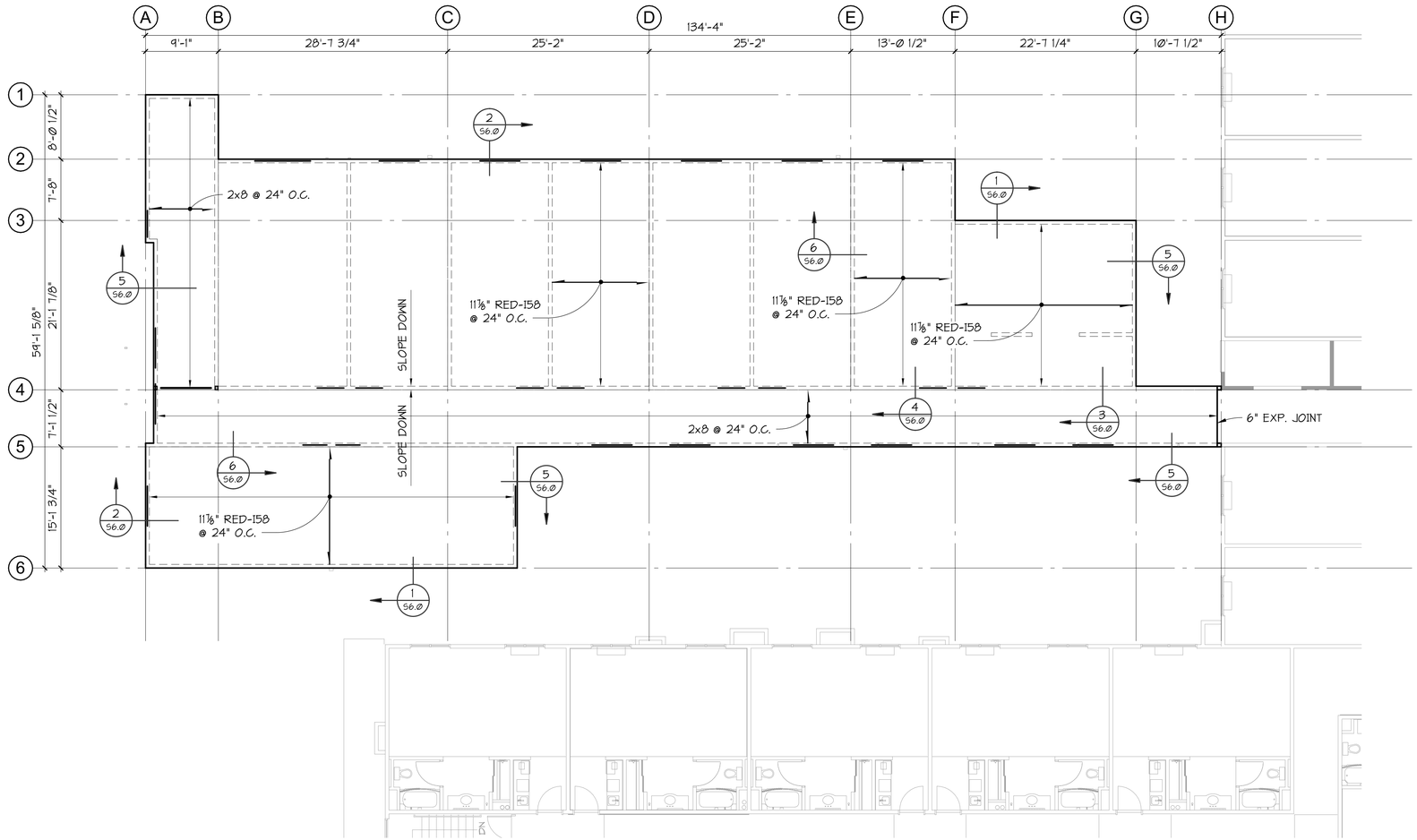

NORTH 1 S2.4 1/8" = 1'-0"

ROOF FRAMING PLAN

ROOF FRAMING NOTES

1. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
2. INDICATES WALL EXTENDING TO ROOF STRUCTURE.
3. INDICATES TYPICAL HEADER IN WALL BELOW. SEE 1/S4.0. RIM SERVES AS HEADER AT EXTERIOR EHADER UNLESS NOTED OTHERWISE.
4. PROVIDE 23/32" TONGUE & GROOVE WOOD SHEATHING OVER ENTIRE ROOF STRUCTURE. NAIL SHEATHING WITH 10d @ 6" ON CENTER AT ALL SUPPORTED PANEL EDGES AND 10d @ 12" ON CENTER AT INTERMEDIATE FRAMING. TYPICAL UNLESS NOTED OTHERWISE.
5. PROVIDE 2x4 FLAT PLATE OR BLOCKING BENEATH SHEATING WITH STRAP ON TOP OF SHEATING.
6. NON-STRUCTURAL STUD WALLS ARE NOT SHOWN OR SHOWN SCREENED. FOR LOCATION SEE ARCHITECTURAL DRAWINGS. FOR BRACING AT TOP OF WALL SEE 7/S4.2 AND 10/S4.2.

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ROOF FRAMING PLAN
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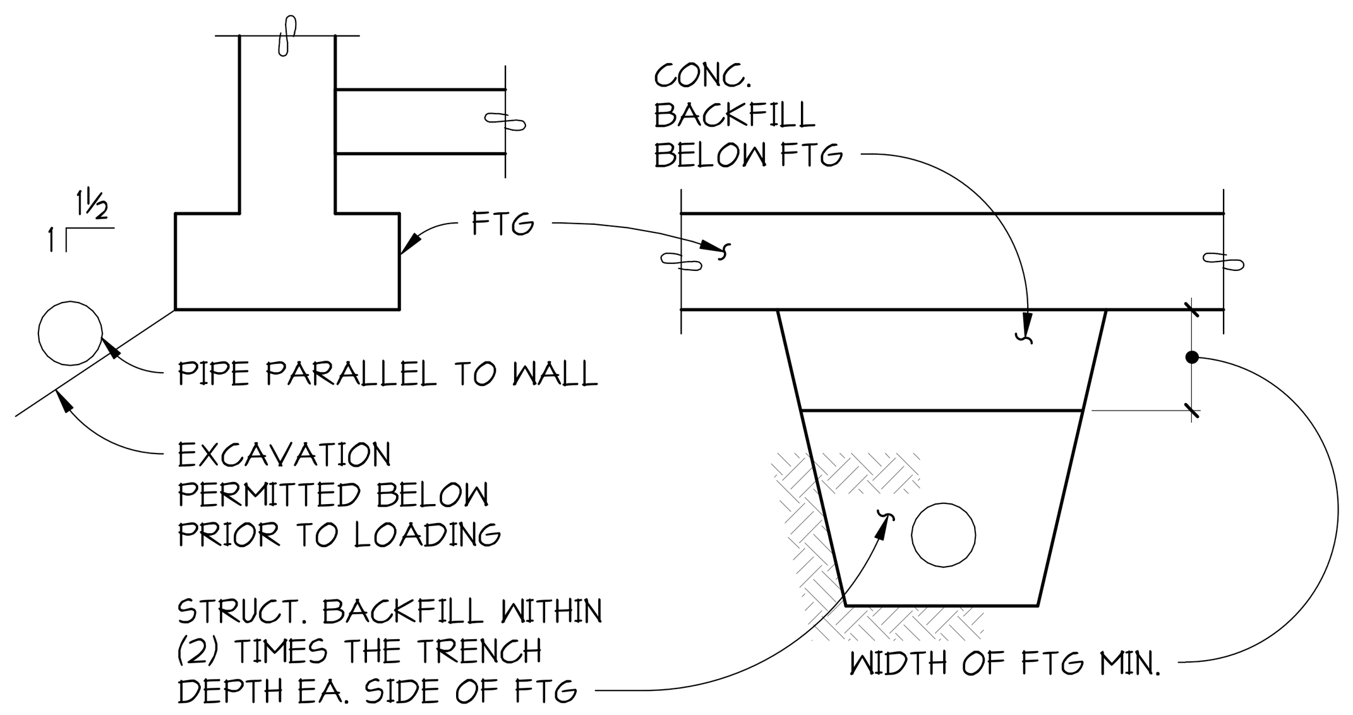


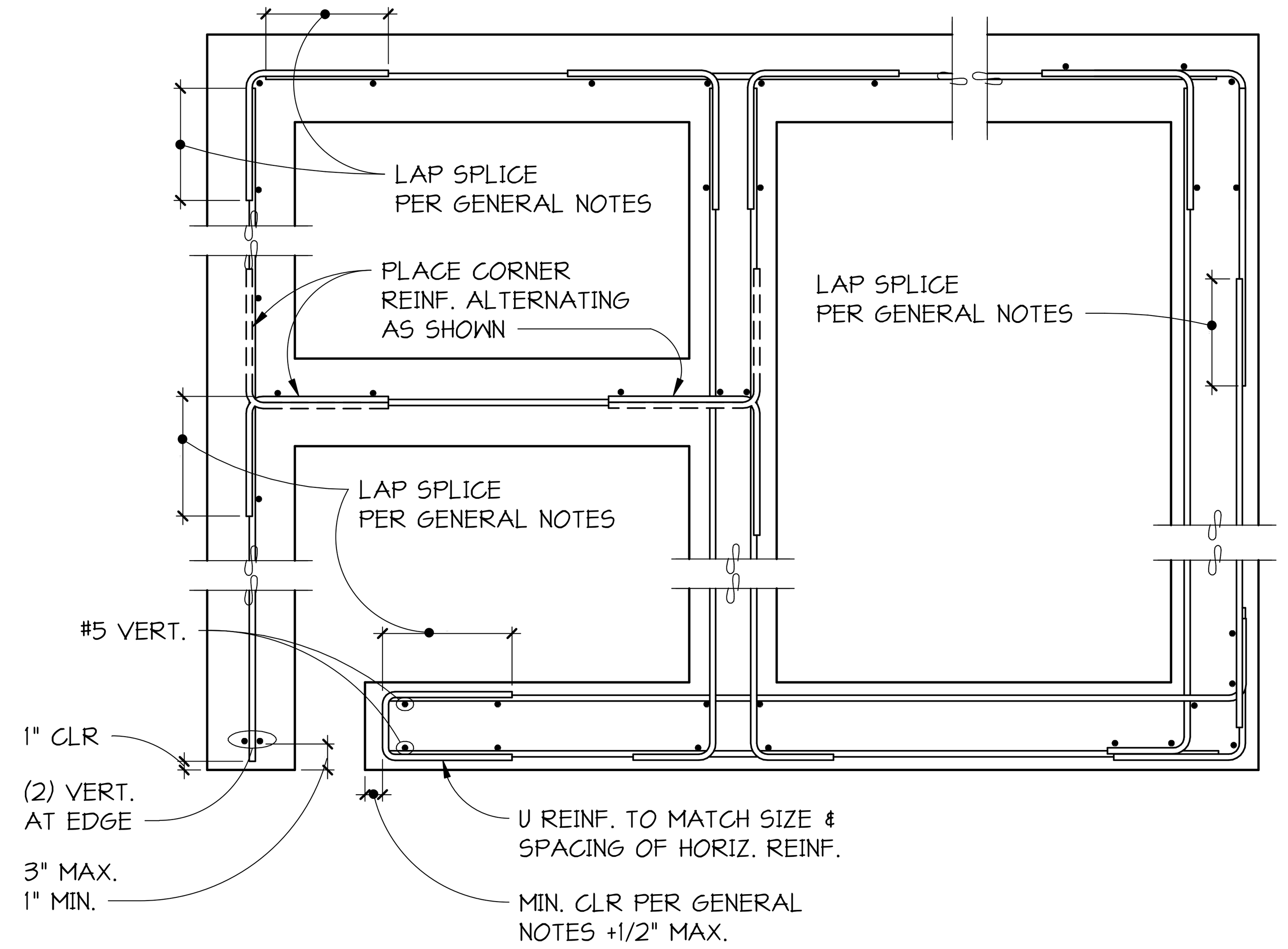
TYPICAL FOR PARALLEL PIPES ADJACENT TO FOOTINGS

TYPICAL FOR PERPENDICULAR PIPES UNDER FOOTINGS

NOTES:

1. FOUNDATIONS SHALL NOT BE LOADED PRIOR TO COMPLETING STRUCTURAL BACKFILL UNDER AND NEAR FOOTINGS.
2. CONCRETE BACKFILL SHALL BE USED UNDER FOOTINGS WHERE 95% COMPACTION CANNOT BE ACCOMPLISHED.
3. ALL STRUCTURAL BACKFILL NOTED SHALL BE COMPACTED TO 95% OF MAXIMUM DENSITY PER ASTM.
4. A PIPE SLEEVE SHALL BE PROVIDED FOR SHALLOW PIPES CAST IN CONCRETE.
5. PIPES SHALL NOT BE PLACED IN THE FOOTING WITHOUT SPECIFIC APPROVAL FROM THE ENGINEER.
6. FOR VARIATIONS CONTACT ENGINEER.

1 SECTION
S3.1 NO SCALE

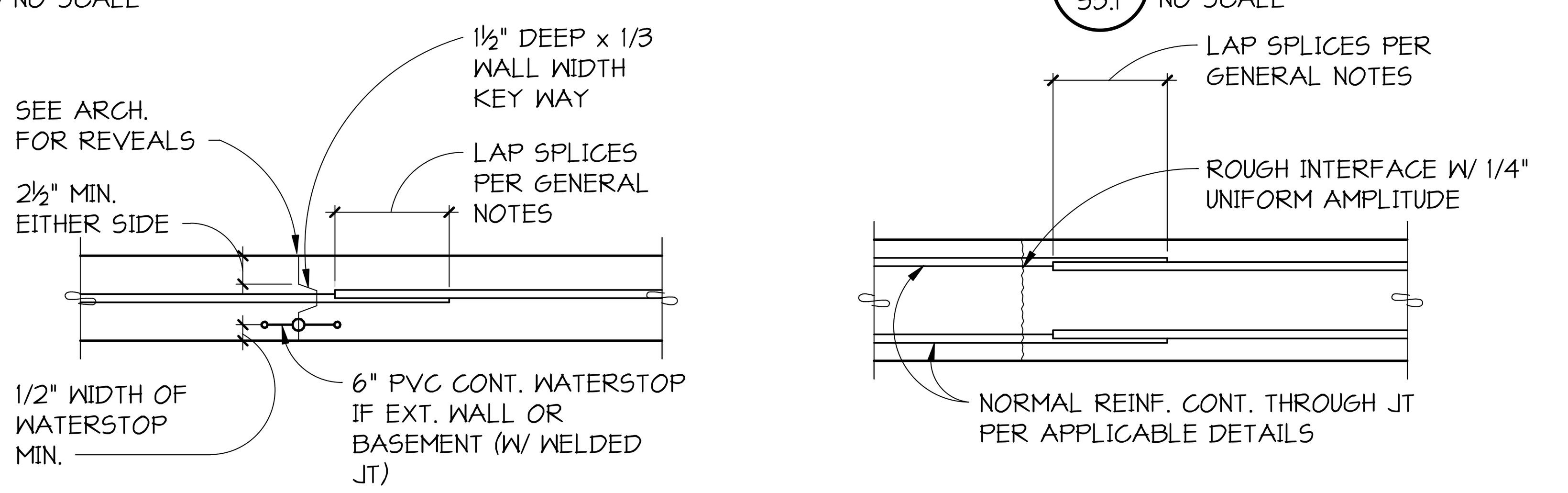


NOTES:

1. VERTICAL REINFORCEMENT SHOWN IS ADDITIONAL IF NORMAL STEM WALL REINFORCEMENT IS NOT IN PROPER LOCATION.
2. CORNER REINFORCEMENT IS SAME SIZE AND SPACING AS HORIZONTAL REINFORCEMENT.
3. STANDARD HOOK MAY BE SUBSTITUTED FOR CORNER REINFORCEMENT. SEE NOTE #5.
4. REINFORCEMENT AT ALL CORNERS, ENDS, AND INTERSECTIONS OF WALLS SHALL BE PLACED IN ACCORDANCE WITH APPROPRIATE DETAIL SHOWN.
5. USE STANDARD HOOK FOR EMBEDMENT LESS THAN 24" PAST FACE OF WALL.

TYPICAL REINFORCEMENT PLACEMENT FOR CAST IN PLACE CONCRETE WALLS AND STEM WALLS

2 DETAIL
S3.1 NO SCALE



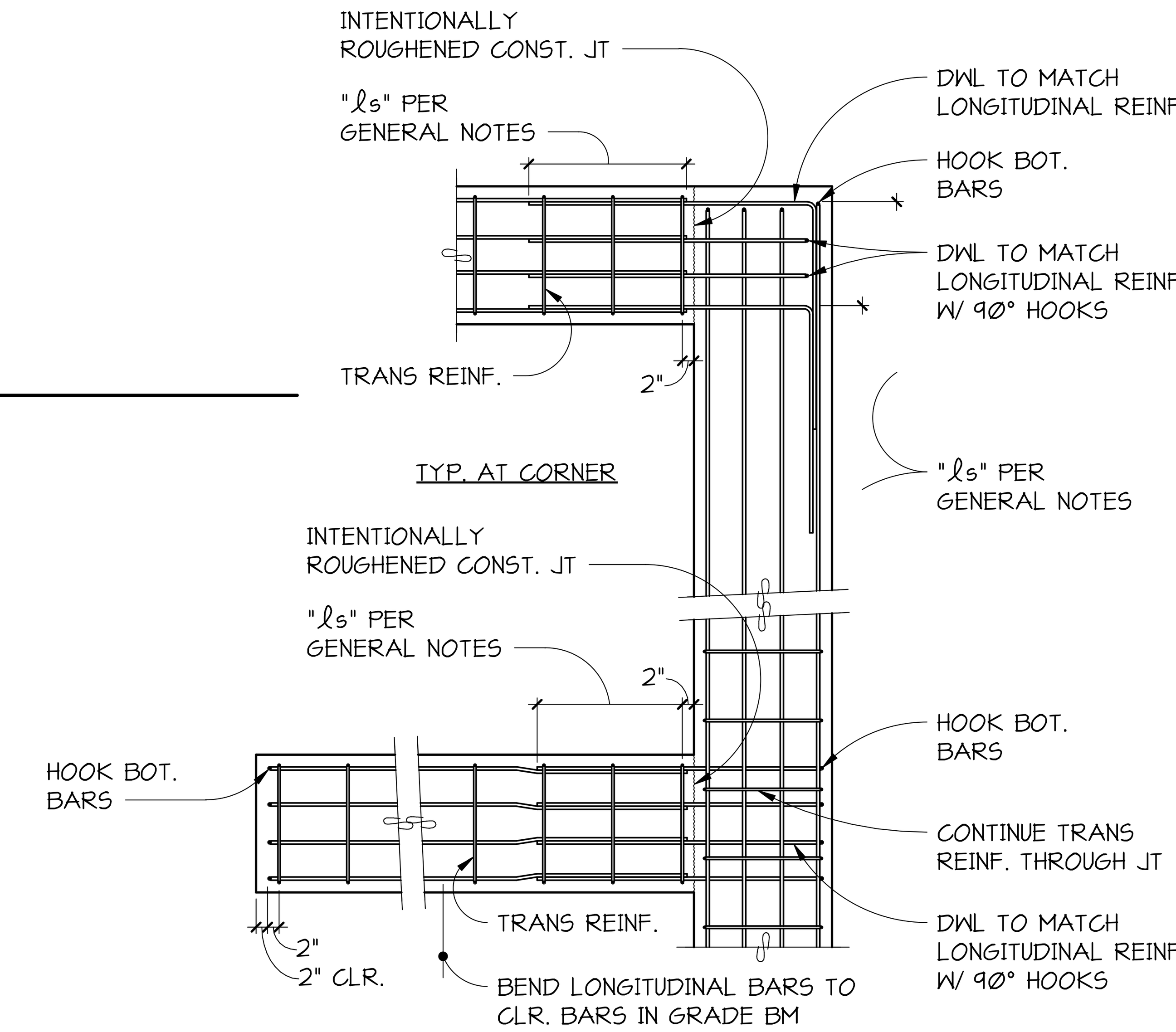
TYPICAL AT VERTICAL WALL JOINTS

TYPICAL AT CONCRETE GRADE BEAMS & FOOTINGS

NOTE: OBTAIN APPROVAL OF ENGINEER FOR LOCATION OF ANY CONSTRUCTION JOINT

TYPICAL FOR CAST IN PLACE CONCRETE WALLS, GRADE BEAMS AND FOOTINGS

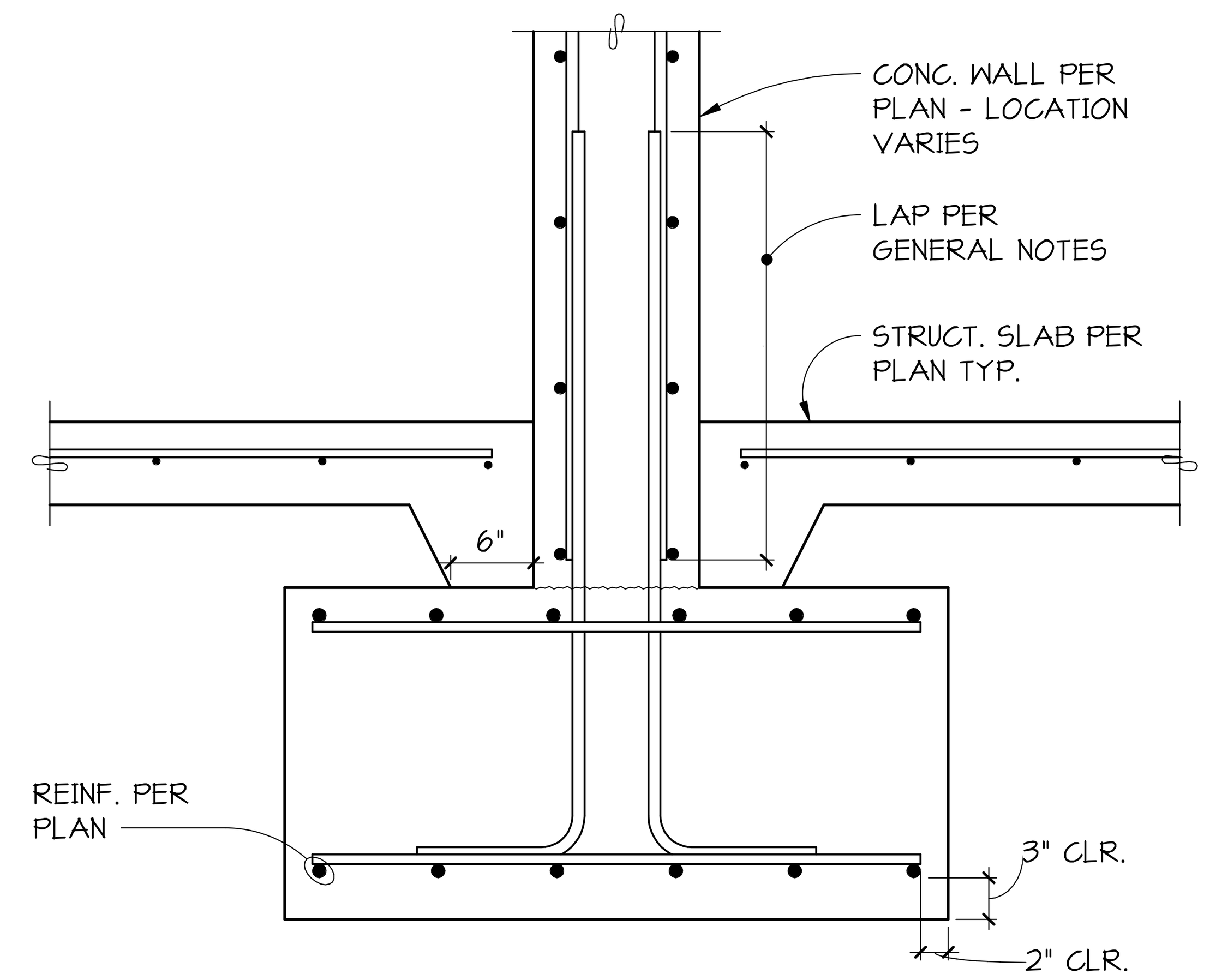
3 CONSTRUCTION JOINTS
S3.1 NO SCALE



END OF GRADE BEAM

TYPICAL AT GRADE BEAM TO GRADE BEAM

5 PLAN DETAIL
S3.1 NO SCALE



7 SECTION
S3.1 NO SCALE

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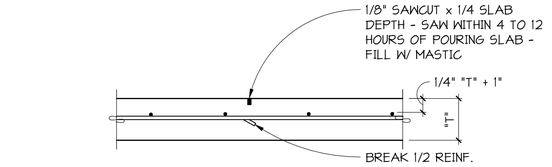
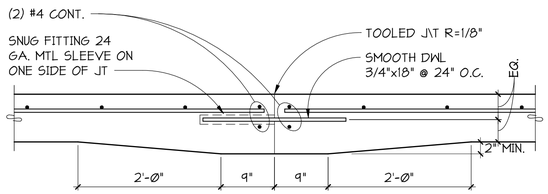
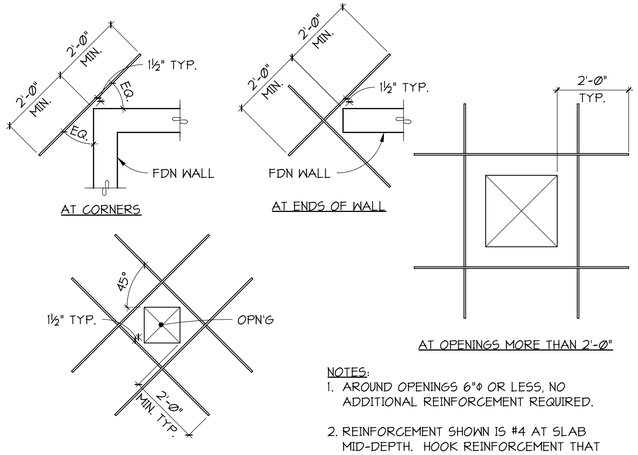


TYPICAL CONTROL JOINT FOR SLAB ON GRADE



TYPICAL CONSTRUCTION JOINT FOR SLAB ON GRADE

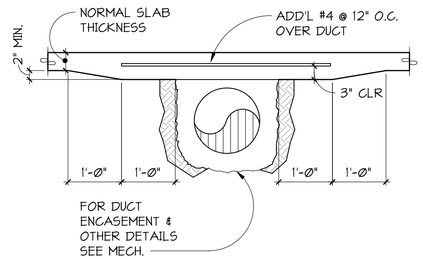
1 S3.2 **SECTION**
NO SCALE



NOTES:
1. AROUND OPENINGS 6"Ø OR LESS, NO ADDITIONAL REINFORCEMENT REQUIRED.
2. REINFORCEMENT SHOWN IS #4 AT SLAB MID-DEPTH. HOOK REINFORCEMENT THAT CANNOT BE EXTENDED.

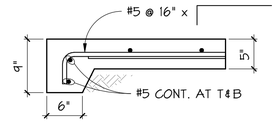
TYPICAL SLAB ON GRADE DISCONTINUITY REINFORCEMENT

2 S3.2 **PLAN DETAILS**
NO SCALE

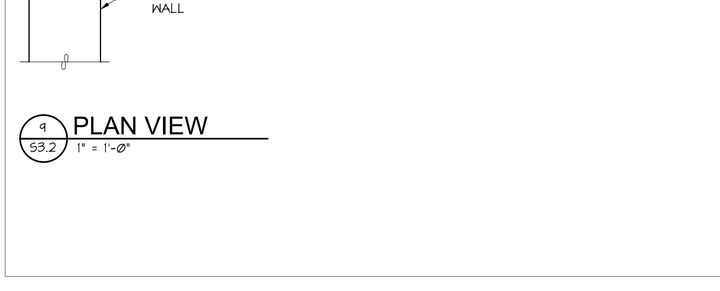


TYPICAL UNDER SLAB DUCT OVER 12"Ø

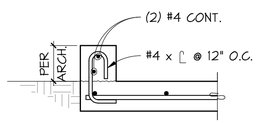
3 S3.2 **SECTION**
NO SCALE



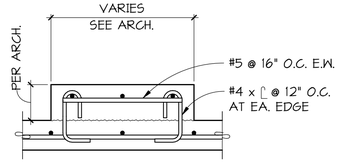
4 S3.2 **SECTION**
NO SCALE



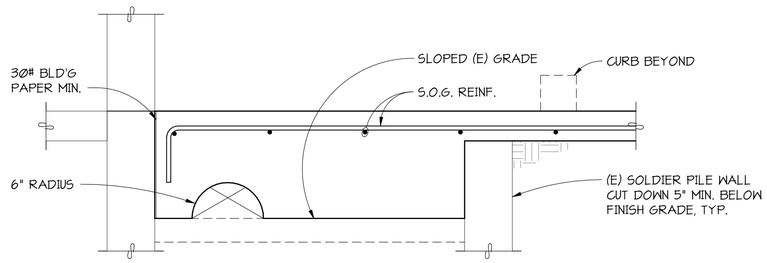
5 S3.2 **SECTION**
1" = 1'-Ø"



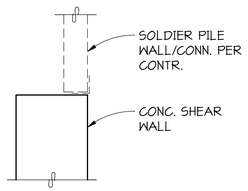
6 S3.2 **SECTION**
1" = 1'-Ø"



7 S3.2 **SECTION**
1" = 1'-Ø"



8 S3.2 **SECTION**
1" = 1'-Ø"



9 S3.2 **PLAN VIEW**
1" = 1'-Ø"

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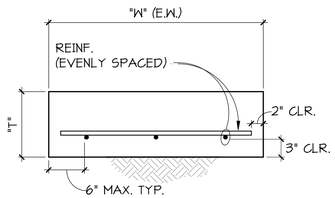
FOUNDATION DETAILS

Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S3.2
PERMIT SET



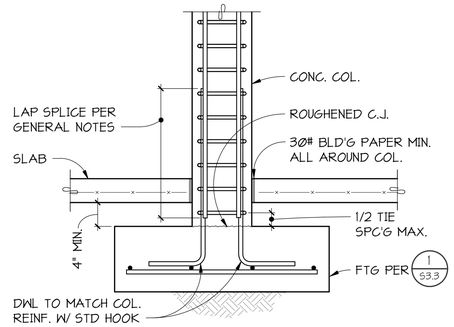
SECTION

MARK	DIMENSIONS		REINFORCEMENT EACH WAY
	"W"	"T"	
F5.0	5'-0"	12"	(5) #5
F6.0	6'-0"	14"	(6) #6
F8.0	8'-0"	16"	(8) #7

NOTES:
1. CENTER ALL FOOTINGS ON COLUMN ABOVE EXCEPT AS SHOWN OTHERWISE.
2. FOOTINGS SHALL BEAR ON UNDISTURBED OR COMPACTED MATERIAL PER GENERAL NOTES. DESIGN BEARING PRESSURE IS 4000 POUNDS PER SQUARE FOOT.

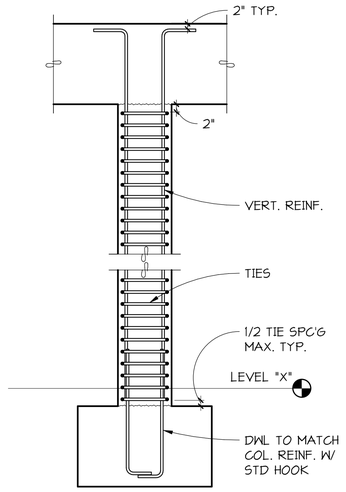
TYPICAL CONCRETE SPREAD FOOTING

1 / S3.3 SECTION
NO SCALE



TYPICAL CONCRETE COLUMN AT SPREAD FOOTING

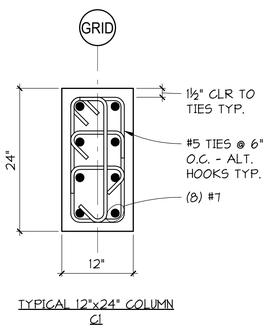
2 / S3.3 SECTION
NO SCALE



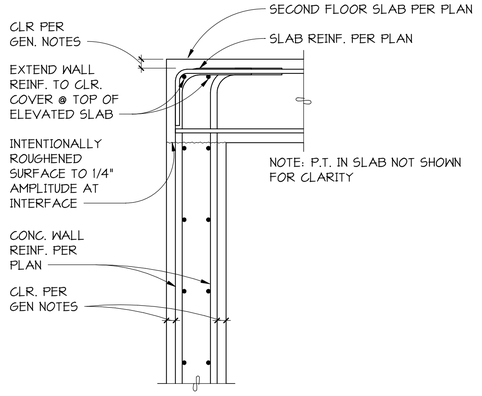
GRAVITY COLUMN DETAIL

3 / S3.3 DETAIL
NO SCALE

SCHEDULE	
CONCRETE COLUMN LAP SPLICES FOR f'c=4,000 psi	
BAR SIZE	SPLICE LENGTH
#7	2'-8"

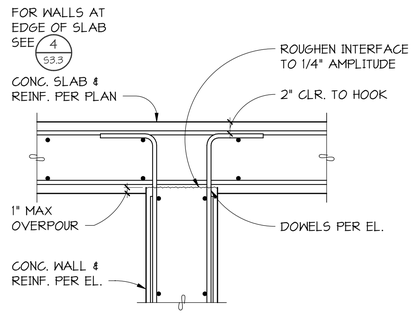


TYPICAL 12"x24" COLUMN
C1



TYPICAL TOP OF WALL CONNECTION TO CONCRETE SLAB

4 / S3.3 SECTION
NO SCALE



TYPICAL TOP OF CONCRETE WALL

5 / S3.3 SECTION
NO SCALE

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Job #: 18741
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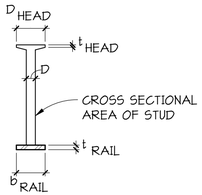
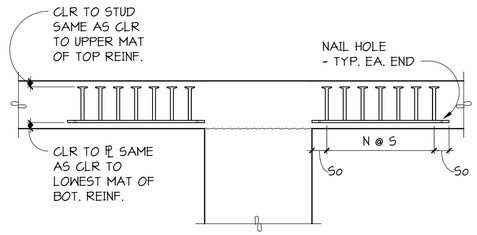
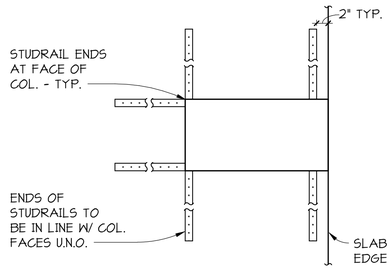


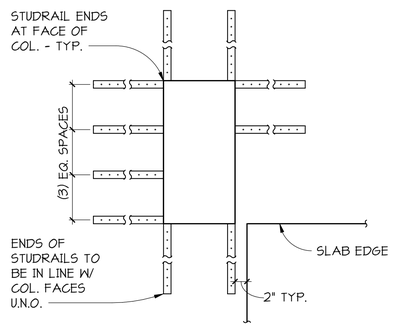
STUD TABLE					
D in.	CROSS SECTIONAL AREA in.^{2}	^{D}HEAD in.	^{t}HEAD in.	^{b}RAIL in.	^{t}RAIL in.
1/2	0.196	1.58	0.28	1-1/4	1/4
3/4	0.442	2.37	0.42	2	3/8



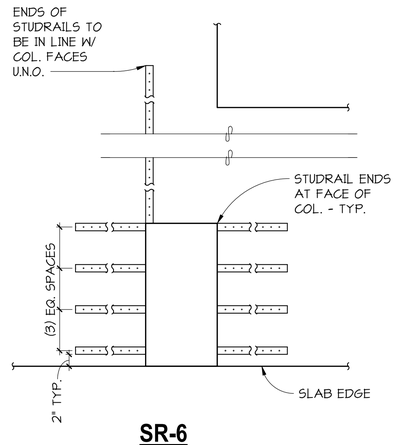
STUD RAIL SCHEDULE					
MARK	SHEAR STUDS				DETAIL
	DIAMETER	So	N	S	
SR-1	3/4	4⅛	11	5.5	SR-1
SR-2	3/4	4⅛	17	3.5	SR-2
SR-3	3/4	4⅛	11	5.5	SR-3
SR-4	3/4	4⅛	13	5.5	SR-4
SR-5	3/4	4⅛	17	5.0	SR-5
SR-6	3/4	4⅛	11	5.0	SR-6



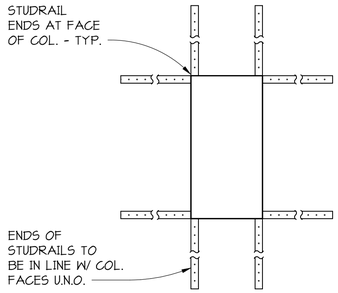
SR-4



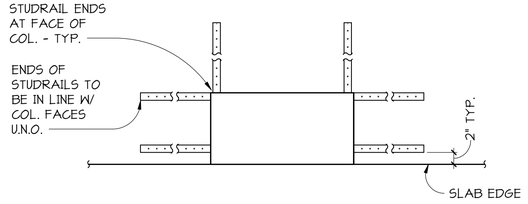
SR-5



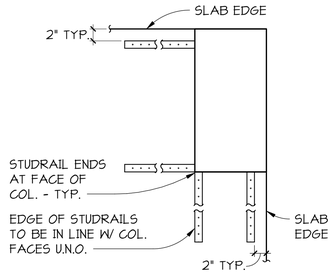
SR-6



SR-1



SR-2



SR-3

1 / S3.4 STUD RAIL SCHEDULE AND DETAILS

NO SCALE

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CONCRETE STUD RAIL DETAILS
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Job #: 18741
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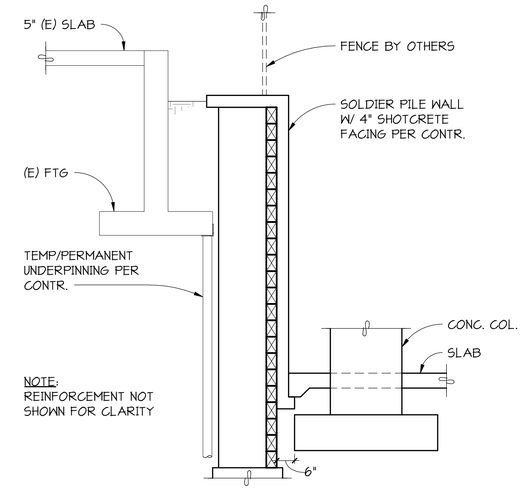
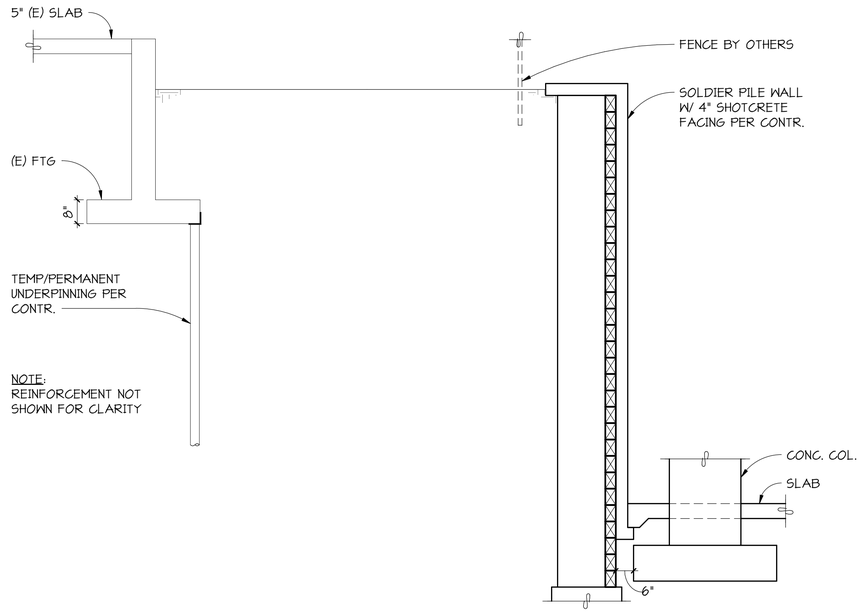
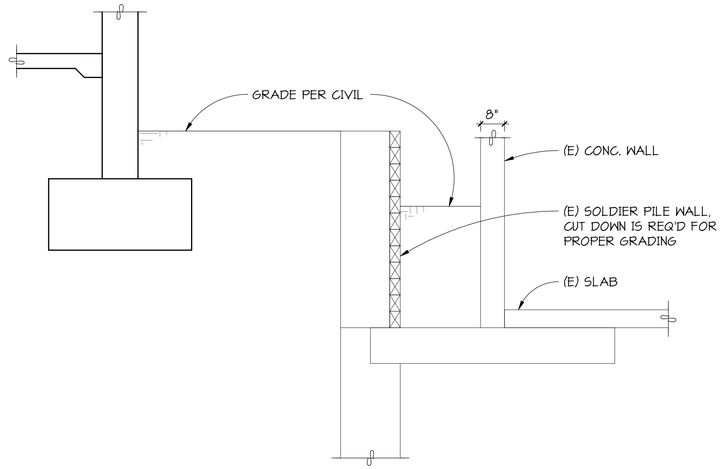
PERMIT SET



1 S3.5 SECTION
1/2" = 1'-0"



2 S3.5 SECTION
1/2" = 1'-0"



3 S3.5 SECTION
1/2" = 1'-0"

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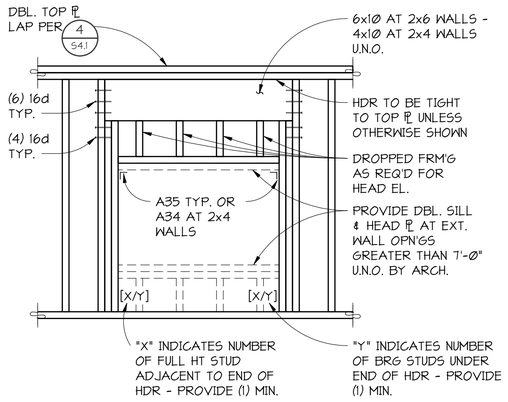
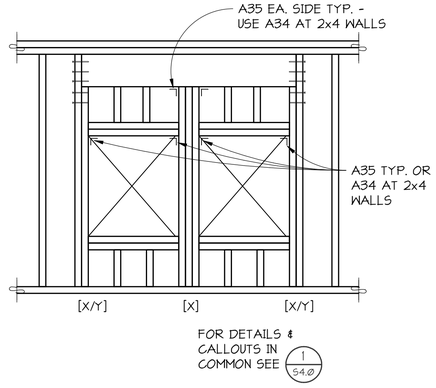
PERMIT SET



TYPICAL STUD WALL CONSTRUCTION AT HEADER

1/54.Ø SECTION

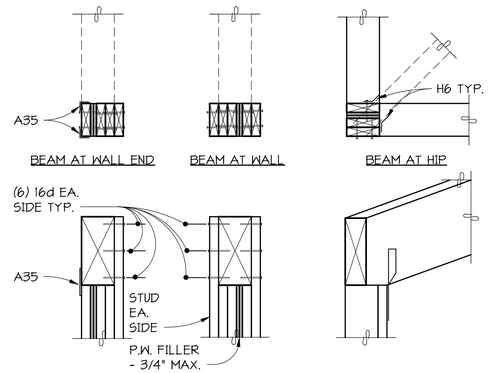
NO SCALE



TYPICAL STUD WALL CONSTRUCTION AT HEADER WITH INTERMEDIATE MULLION

2/54.Ø SECTION

NO SCALE



TYPICAL BUILT-UP COLUMN AT BEAM PERPENDICULAR TO WALL

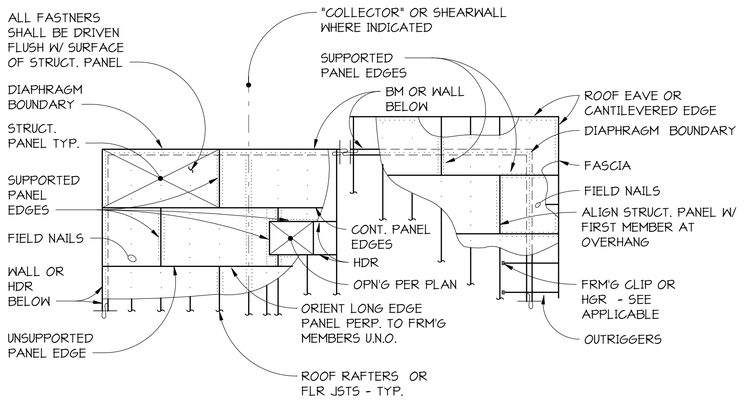
3/54.Ø DETAIL

NO SCALE

DIAPHRAGM NAILING SCHEDULE			
DIAPHRAGM TYPE	LOCATION	NAILS	SPACING
FLOOR DIAPHRAGM 23/32" TONGUE AND GROOVE SHEATHING UNBLOCKED UNLESS NOTED OTHERWISE	DIAPHRAGM BOUNDARY	10d	6" O.C.
	FIELD NAILS	10d	12" O.C.
	SUPPORTED PANEL EDGES	10d	6" O.C.
ROOF DIAPHRAGM 19/32" SHEATHING UNBLOCKED UNLESS NOTED OTHERWISE	DIAPHRAGM BOUNDARY	10d	6" O.C.
	FIELD NAILS	10d	10" O.C.
	SUPPORTED PANEL EDGES	10d	6" O.C.

NOTES:

1. PROVIDE (2) ROWS OF SPECIFIED DIAPHRAGM BOUNDARY NAILING OVER INTERIOR SHEAR WALLS AND THE FULL LENGTH OF "COLLECTORS" WHERE INDICATED.
2. AT BLOCKED DIAPHRAGMS PROVIDE 2x4 FLATWISE BLOCKING WITH "Z2" CLIPS AT EACH END AT ALL UNSUPPORTED PANEL EDGES. USE 2x4 STRUCTURAL COMPOSITE LUMBER FLATWISE BLOCKING IN LIEU OF SOLID SAWN WHERE NAILING SIZE OR SPACING EXCEEDS 10d @ 4" O.C.



TYPICAL DIAPHRAGM NAILING

4/54.0 SCHEDULE

1" = 1'-Ø"

STUD WALL CONSTRUCTION SCHEDULE										
TABLE 1 - SHEAR WALL REQUIREMENTS										
MARK	WALL SHEATHING	SIDES WITH SHEATHING	SHEATHING NAILS NOTE 2	EDGE NAILING ON CENTER	EDGE FRAMING NOTE 5	FIELD NAILING ON CENTER	BOTTOM PLATE NOTE 6	BOTTOM PLATE NAILING	5/8" ANCHOR BOLT SPACING (EMBED 7" MINIMUM)	RIM/BLOCKING CONNECTOR TO TOP PLATE BELOW
A	15/32"	(1)	10d	6"	2x	12"	2x	16d @ 8" O.C.	48"	A35 @ 24" O.C.
B	15/32"	(1)	10d	4"	2x	12"	2x	16d @ 4" O.C.	32"	A35 @ 16" O.C.
C	15/32"	(1)	10d	3"	3x	12"	2x	SDS @ 6" O.C.	32"	A35 @ 12" O.C.

CHARACTER INDICATES SPECIAL SHEAR WALL REQUIREMENTS PER TABLE 1

NOTES:

1. (X) INDICATES SPECIAL STRUCTURAL WALL MARK. ALL WALLS SHOWN ON STRUCTURAL DRAWINGS ARE 2x6 AT 16" ON CENTER UNLESS DESIGNATED SPECIAL. STUD LAYOUT SHALL MATCH FRAMING MEMBER LAYOUT ABOVE WHERE APPLICABLE. ALL EXTERIOR WALLS SHALL HAVE 15/32" WOOD SHEATHING AND BE NAILED WITH 10d AT 6" ON CENTER AT EDGES AND 12" ON CENTER IN FIELD UNLESS DESIGNATED SPECIAL.
2. ALL EXTERIOR WALLS AND ALL DESIGNATED SHEAR WALLS SHALL BE BLOCKED AT ALL SHEATHING EDGES. EDGE NAILING APPLIES TO ALL TOP AND BOTTOM PLATES, VERTICAL JOINTS, HORIZONTAL BLOCKED JOINTS, WALL CORNERS, AND HOLDOWN ANCHORED STUDS.
3. WHERE BEAMS OR HEADERS FRAME INTO WALLS AND A COLUMN IS NOT CALLED OUT, PROVIDE BUILT-UP COLUMNS PER 3/S4.Ø FOR BEAM PERPENDICULAR TO WALL.
4. [XY]INDICATES BUILT-UP STUD COLUMNS AT HEADERS IN WALLS - SEE 1/WSTD2 FOR BEAM PARALLEL TO WALL.
5. PROVIDE 3x OR DOUBLE 2x MEMBERS FACE NAILED PER 1/S4.1 AT ALL ABUTTING PANEL EDGES WHERE INDICATED.
6. 3x BOTTOM PLATE WHERE INDICATED.
7. WHERE SOLID SAWN STUD LENGTH CANNOT BE OBTAINED, STRUCTURAL COMPOSITE LUMBER STUDS MAY BE SUBSTITUTED. SOLID SAWN FRAMING MAY NOT BE SUBSTITUTED FOR SPECIFIED STRUCTURAL COMPOSITE LUMBER FRAMING.

5/54.Ø SCHEDULE

NO SCALE

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WOOD FRAMING DETAILS
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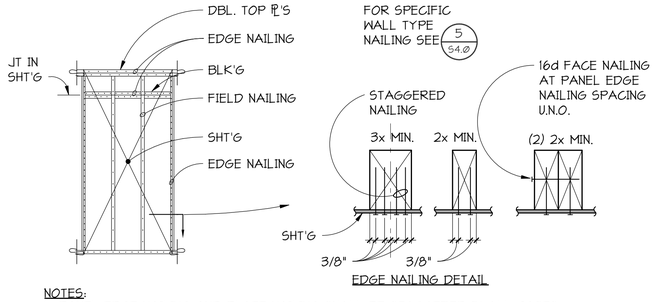
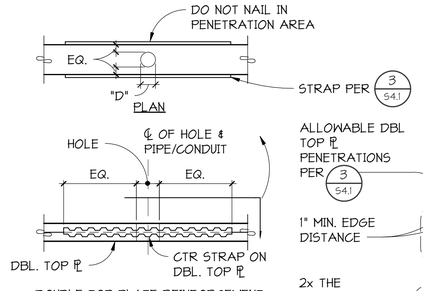


EDGE NAILING DETAIL

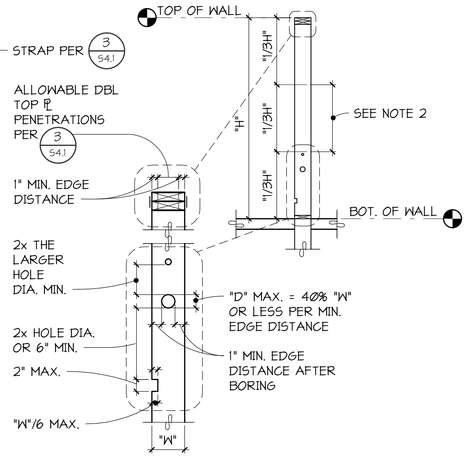
NOTES:
1. PANEL EDGE NAILING AND PLATE NAILING SHALL BE STAGGERED IN ALL CASES.
2. SHEATHING JOINT SHALL OCCUR AT COMMON MEMBER UNLESS IT OCCURS AT A SPECIFIED DOUBLE MEMBER.
3. EDGE NAILING APPLIES TO AREAS INDICATED AND AT HOLDOWN ANCHORED STUDS.

TYPICAL SHEARWALL NAILING

1/54.1 SCHEDULE
NO SCALE



DOUBLE TOP PLATE REINFORCEMENT



NOTES:
1. BORING, CUTTING, AND NOTCHING WALL FRAMING OTHER THAN INDICATED HERE REQUIRES PRIOR APPROVAL FROM ENGINEER OF RECORD.
2. BORING, CUTTING, AND NOTCHING WITHIN THE MIDDLE 1/3 OF THE STUD HEIGHT IS NOT ALLOWED WITHOUT PRIOR APPROVAL FROM ENGINEER OF RECORD.
3. NOTCHING AS INDICATED IS PERMITTED ON ONE FACE OF THE WALL ALONE.
4. MAXIMUM BORED HOLE DIAMETER IN STUD "D" MAXIMUM. LIMITED TO 25%. "W" AT BUILT-UP STUDS, AND HOLDOWN ANCHORED STUDS.

ALLOWABLE LOAD BEARING/SHEARWALL STUD BORING, CUTTING, AND NOTCHING

2/54.1 DETAIL
NO SCALE

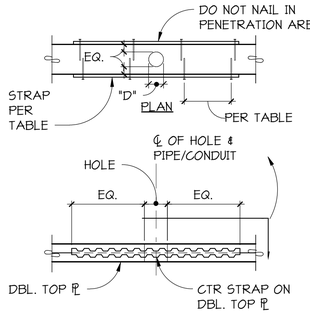
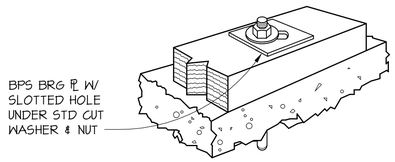


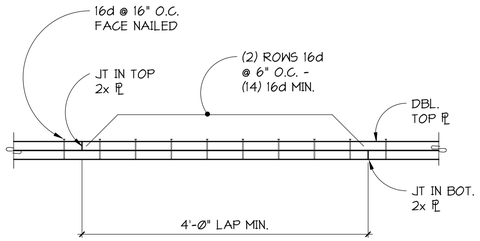
PLATE SIZE	HOLE DIAMETER "D" INCHES	STRAP
2x4	Ø" < "D" < 1" 1" ≤ "D" ≤ 2¼"	NO STRAP REQUIRED (2) ST2215 WITH (8) 16d EACH END
2x6	Ø" < "D" < 2½" 2½" ≤ "D" ≤ 3⅜"	NO STRAP REQUIRED (2) ST2215 WITH (8) 16d EACH END
2x8	Ø" < "D" < 2½" 2½" ≤ "D" ≤ 4"	NO STRAP REQUIRED (2) ST6224 WITH (12) 16d EACH END

TYPICAL REINFORCING AT WALL DOUBLE TOP PLATE PENETRATIONS

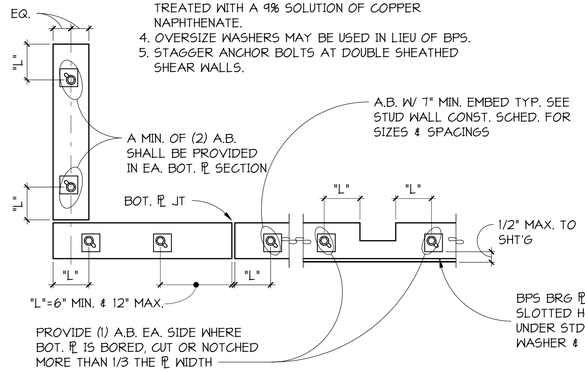
3/54.1 DETAIL
NO SCALE

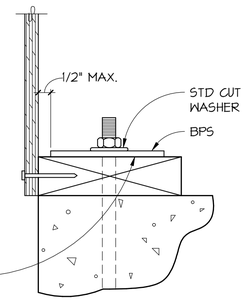
NOTES:
1. BOTTOM SILL PLATE SHALL BE PRESERVATIVE PRESSURVATIVE PRESSURE TREATED. SEE GENERAL NOTES FOR GALVANIZED REQUIREMENTS FOR CONNECTORS AND FASTENERS.
2. HOLES IN BOTTOM PLATE SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER.
3. HOLES, CUTS, AND NOTCHES IN 3x OR 4x PRESERVATIVE PRESSURE PLATES SHALL BE TREATED WITH A 9% SOLUTION OF COPPER NAPHTHENATE.
4. OVERSIZE WASHERS MAY BE USED IN LIEU OF BPS.
5. STAGGER ANCHOR BOLTS AT DOUBLE SHEATHED SHEAR WALLS.





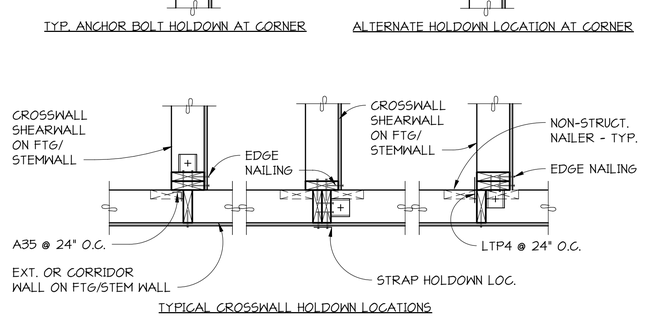
4/54.1 SECTION
NO SCALE





TYPICAL BOTTOM PLATE ANCHORAGE

5/54.1 DETAIL
NO SCALE



TYP. ANCHOR BOLT HOLDOWN AT CORNER

ALTERNATE HOLDOWN LOCATION AT CORNER

TYPICAL CROSSWALL HOLDOWN LOCATIONS

6/54.1 PLAN DETAIL
NO SCALE

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WOOD FRAMING DETAILS

Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S4.1
PERMIT SET

City of Puyallup
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HOLDOWN SCHEDULE

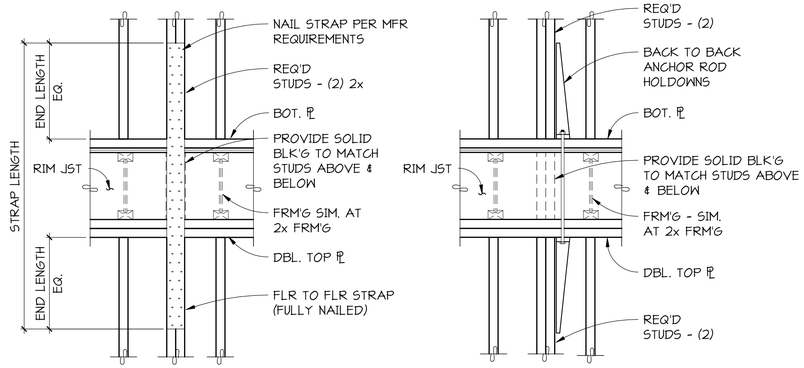
MARK	HOLDOWN [2]	ANCHOR ROD [3]		REQUIRED STUDS	REFERENCED DETAILS
		DIAMETER	EPOXY [4]		
2	HDU2	5/8"	HIT-HY200	(2) 2x	2/S4.2 & 6/S4.1
4	HDU4	5/8"	HIT-HY200	(2) 2x	2/S4.2 & 6/S4.1
11	HDU11	1"	HIT-HY200	6x	2/S4.2 & 6/S4.1

FLOOR TO FLOOR STRAP [1]					
MARK	COILED STRAP CUT TO LENGTH	NAILING	OPTIONAL STRAP	REQUIRED STUDS	REFERENCED DETAILS
37	CMST12x37'	16d @ 1¾"	MST37	(2) 2x	2/S4.2

NOTES:

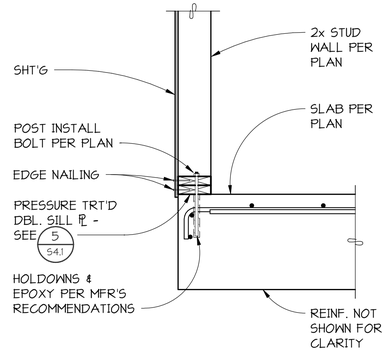
1. ALL HOLDOWNS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
2. PROVIDE BACK TO BACK ANCHOR ROD HOLDOWNS ACROSS FLOOR LINE PER 2/S4.2.
3. ALL-THREAD ROD ASTM A36 WITH 3"x3"x3/8" PLATE WITH HIT-Z OR EQUAL.

1 / S4.2 SCHEDULE
NO SCALE



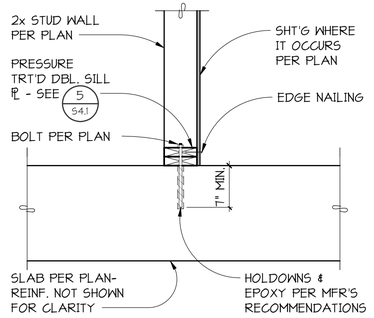
TYPICAL FLOOR TO FLOOR HOLDOWN STRAP

2 / S4.2 DETAIL
NO SCALE



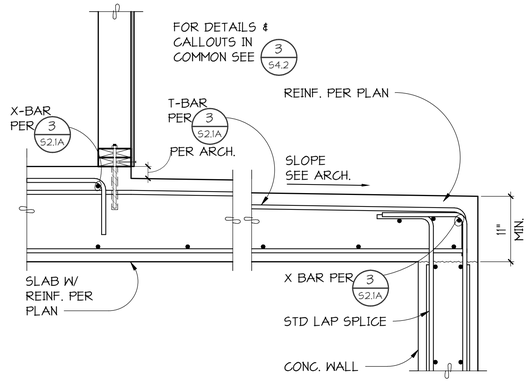
TYPICAL WALL AT EXTERIOR

3 / S4.2 SECTION
NO SCALE

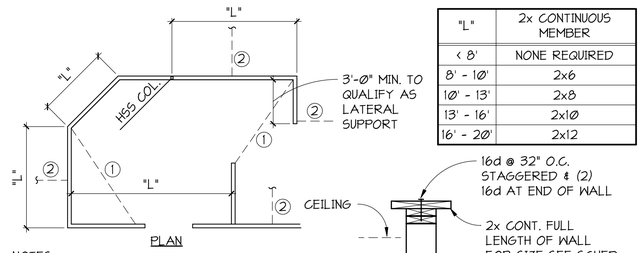


TYPICAL SINGLE WALL

4 / S4.2 DETAIL
NO SCALE



5 / S4.2 DETAIL
NO SCALE



"L"	2x CONTINUOUS MEMBER
< 8'	NONE REQUIRED
8' - 10'	2x6
10' - 13'	2x8
13' - 16'	2x10
16' - 20'	2x12

16d @ 32" O.C. STAGGERED & (2) 16d AT END OF WALL

2x CONT. FULL LENGTH OF WALL FOR SIZE SEE SCHED.

NOTES:

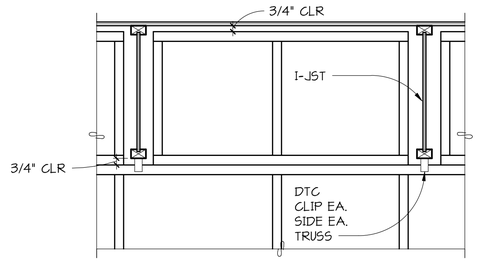
1. THIS PLAN IS AN EXAMPLE ONLY IT DOES NOT REPRESENT A SPECIFIC WALL.
2. "L" INDICATES UNBRACED LENGTH OF WALLS.
3. AT CONTRACTORS OPTION IN LIEU OF EXTRA TOP PLATE.

① INDICATES HORIZONTAL BRACE EXTENDING TO ADJACENT CORNER SEE 10/S4.2.

② INDICATES BRACE UP TO ROOF STRUCTURE SEE 7/S4.2 & 8/S4.2.

TYPICAL LATERAL SUPPORT FOR INTERIOR NON-BEARING WALLS NOT EXTENDNG TO STRUCTURE

6 / S4.2 DETAIL
NO SCALE



TYPICAL CONNECTION AT TOP OF NON-BEARING WALL EXTENDING TO ROOF STRUCTURE PERPENDICULAR TO JOISTS

9 / S4.2 SECTION
NO SCALE

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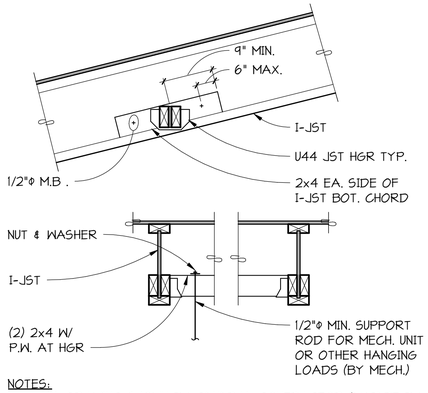
WOOD FRAMING DETAILS

Addition to Hampton Inn & Suites
Hampton Inn & Suites
1515 S. Meridian, Puyallup, WA

Job #: 18741
Date: January 6, 2020
Revs:

S4.2

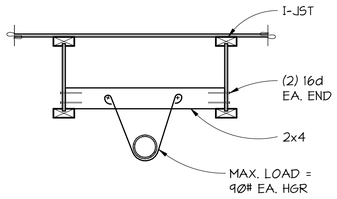
PERMIT SET



NOTES:
1. ALL FRAMING SHOWN BY GENERAL CONTRACTOR (EXCEPT BY SPRINKLER CONTRACTOR IF USED FOR SPRINKLER LINES.)
2. DO NOT CUT OR DRILL THRU JOIST.
3. MAXIMUM SUPPORT LOAD 500#.

TYPICAL DETAIL FOR HANGING LOADS FROM I-JOIST
(ALL HEAT PUMPS OR FANS OVER 90#)

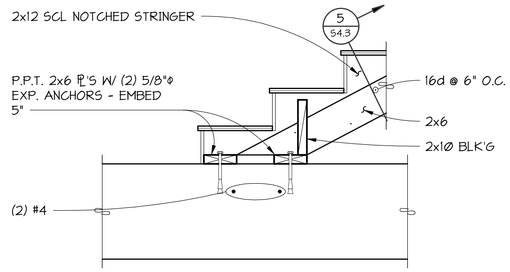
1 / S4.3 SECTION
NO SCALE



NOTE: FOR LINES GREATER THAN 3"ø HANG FROM GLULAM BEAMS OR STUD WALL OR HANG PER OR SUPPORT OFF WALLS WHEN SPECIFICALLY APPROVED BY STRUCTURAL ENGINEER.

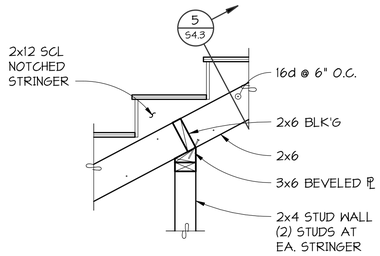
TYPICAL SPRINKLER LINE ATTACHMENT FOR LINES 3"ø OR SMALLER

2 / S4.3 SECTION
NO SCALE



TYPICAL STRINGER TO LOWER LANDING

3 / S4.3 SECTION
NO SCALE

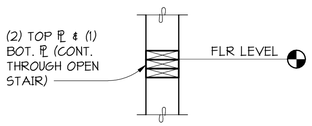


TYPICAL STRINGER INTERMEDIATE SUPPORT

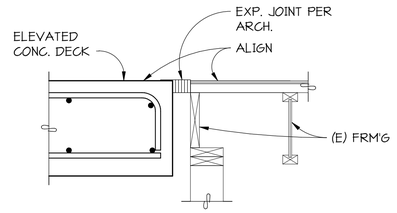
4 / S4.3 SECTION
NO SCALE

TYPICAL STAIR STRINGER

5 / S4.3 SECTION
NO SCALE



6 / S4.3 SECTION
1" = 1'-0"



7 / S4.3 SECTION
1" = 1'-0"

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WOOD FRAMING DETAILS

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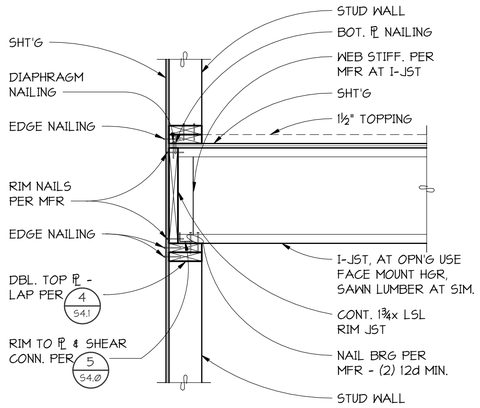
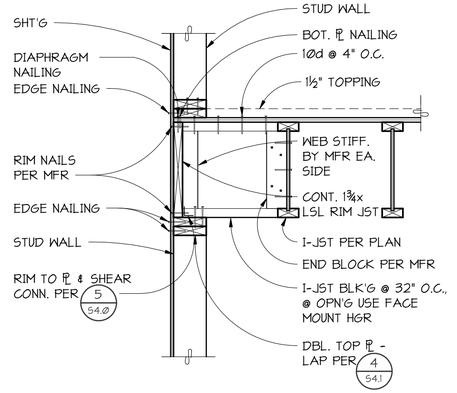


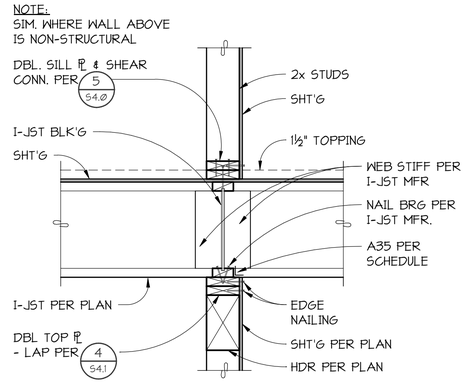
TYPICAL I-JOIST BEARING AT EXTERIOR WALL

1 55.Ø SECTION
NO SCALE



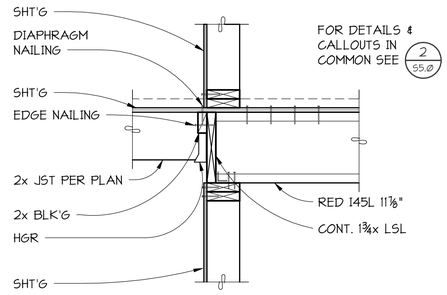
TYPICAL I-JOIST BLOCKING PARALLEL TO EXTERIOR WALL

2 55.Ø SECTION
NO SCALE



TYPICAL I-JOIST AT BEARING STUD WALL

3 55.Ø SECTION
NO SCALE



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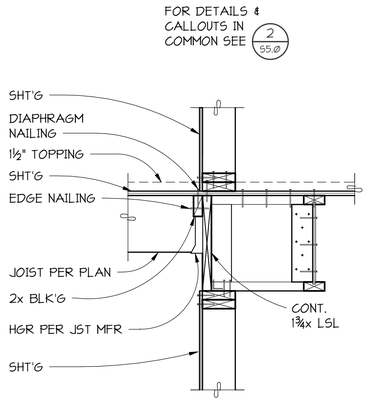

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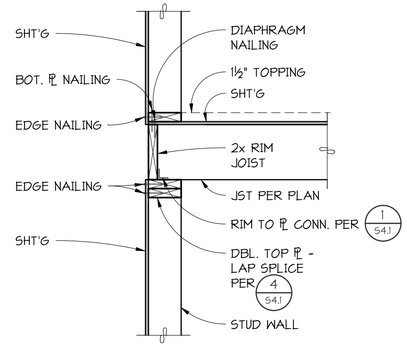

TYPICAL JOIST BEARING AT WALL

6 55.Ø SECTION
1" = 1'-Ø"

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WOOD FRAMING DETAILS

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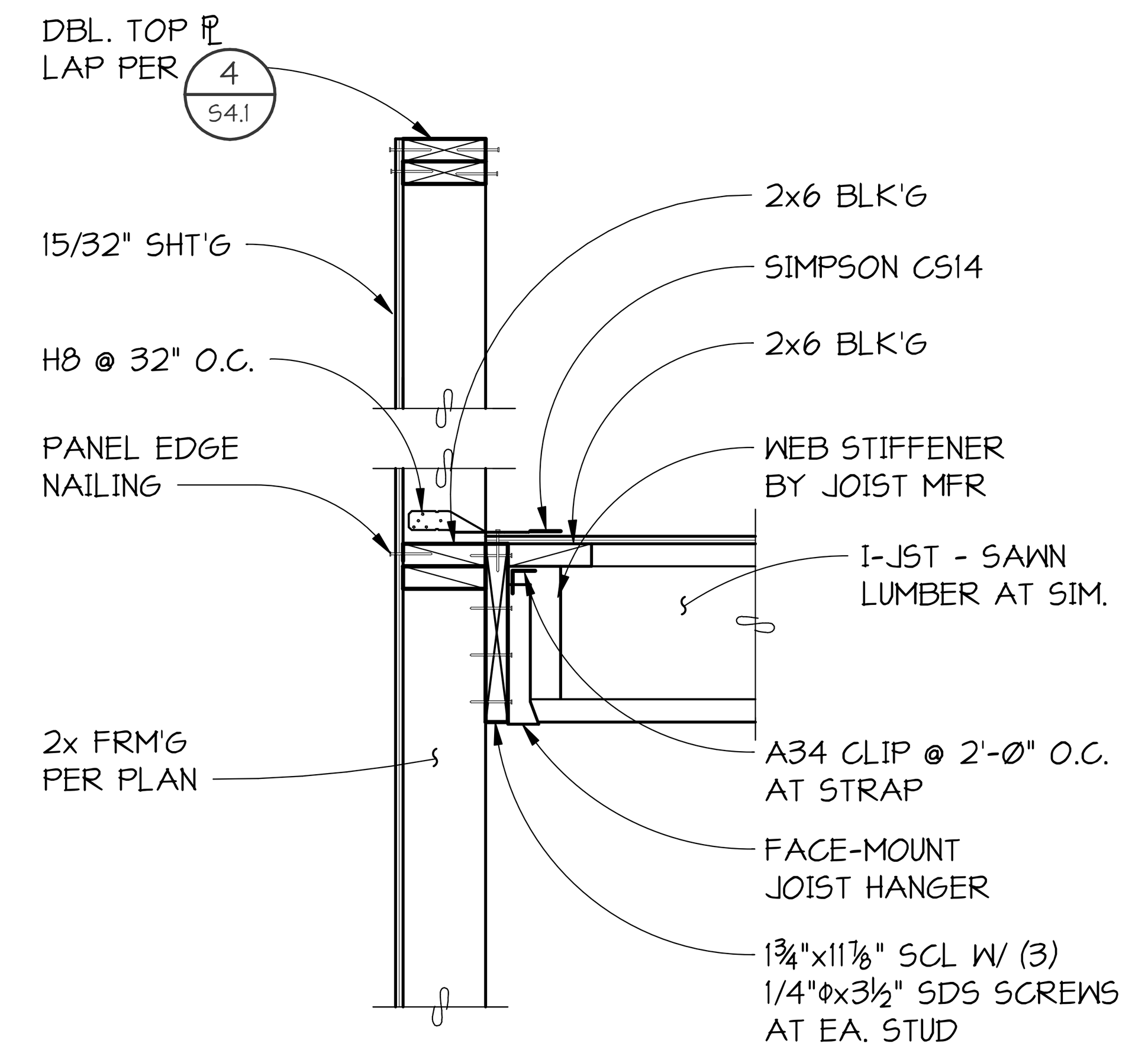
PERMIT SET

C:_Revit Models\18741 Hampton Inn Puyallup v2018 (Central)_dhorne@pcs-structural.com.rvt
1/13/2020 3:17:07 PM



1 S6.0 SECTION
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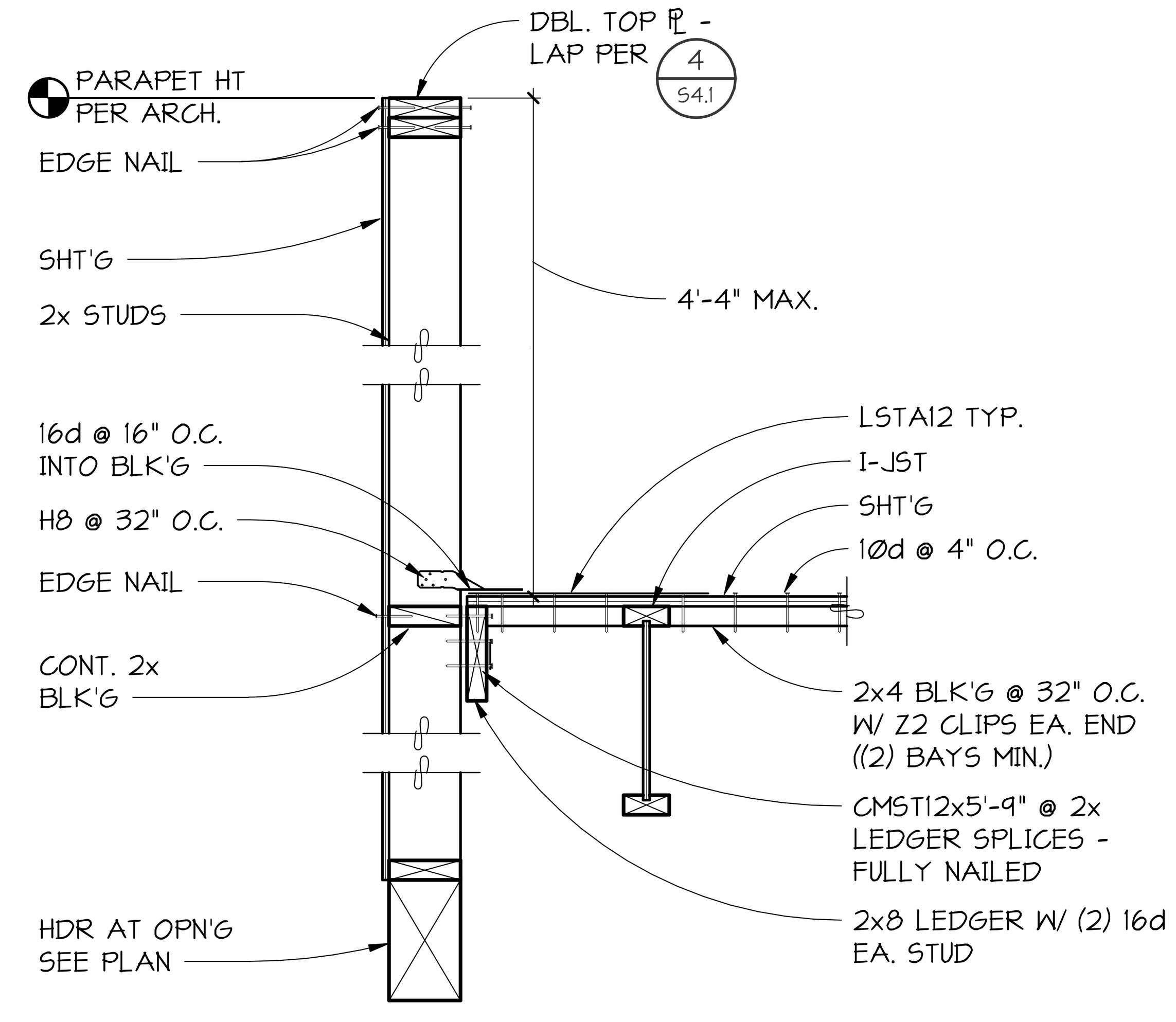
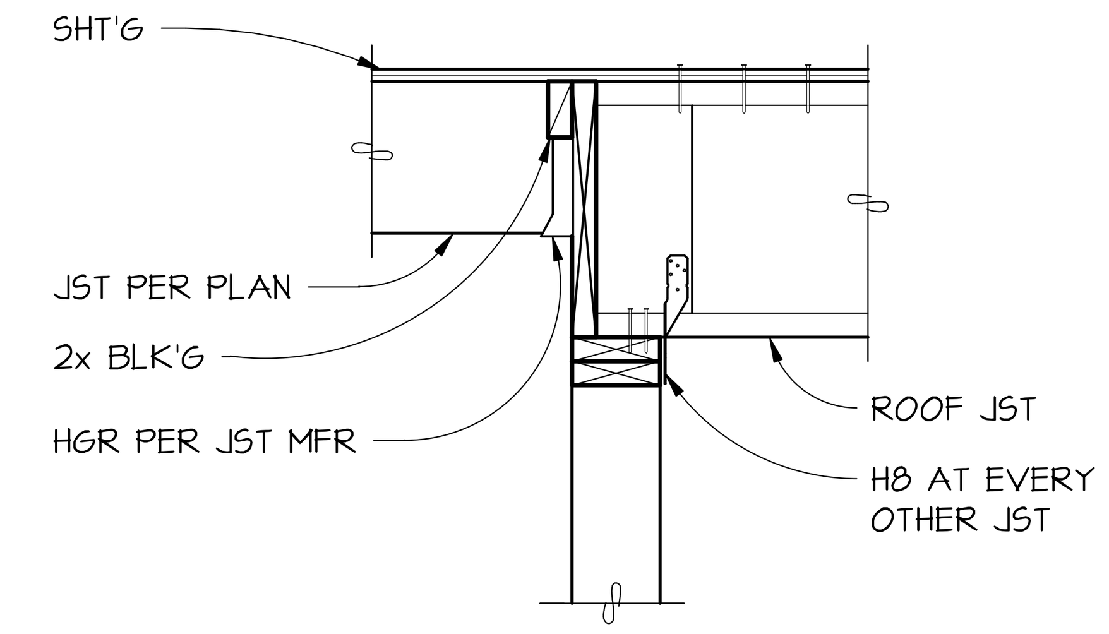


TYPICAL JOIST PARALLEL AT EXTERIOR WALL

2 S6.0 SECTION
NO SCALE



3 S6.0 SECTION
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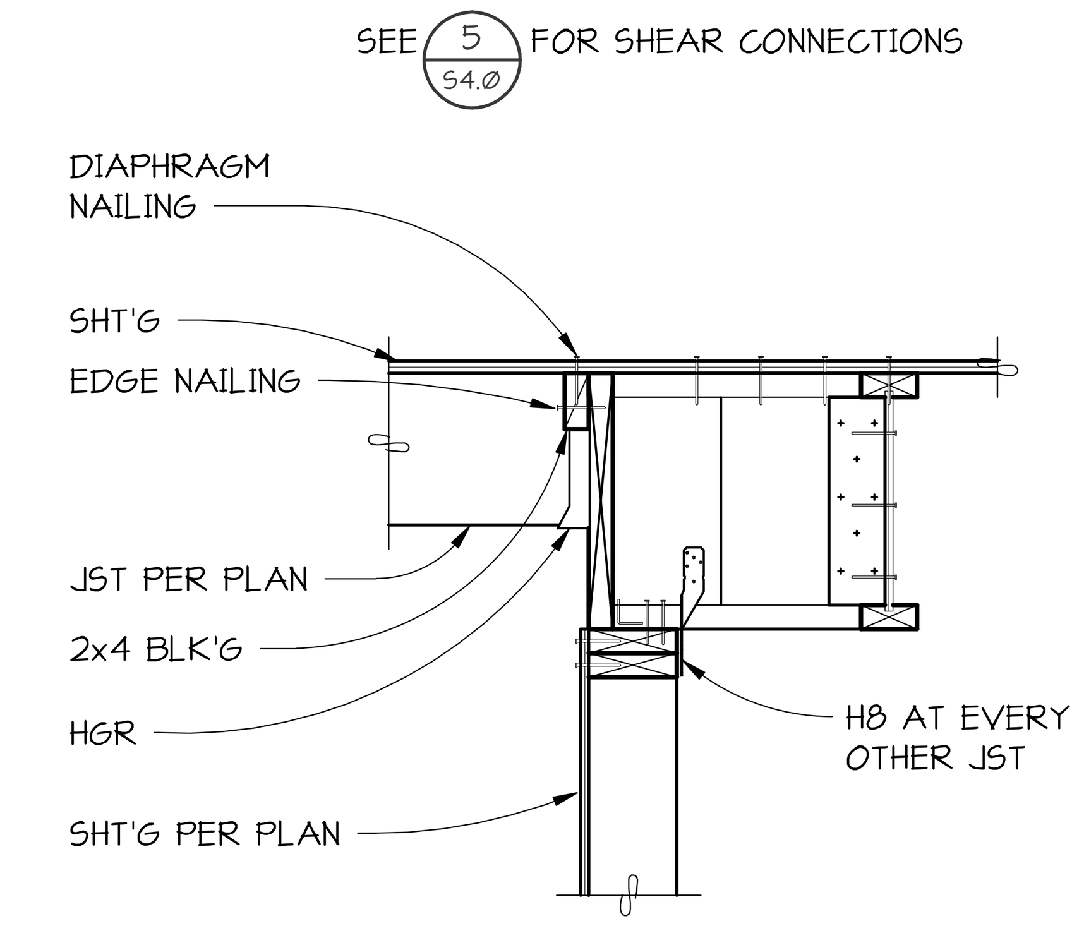

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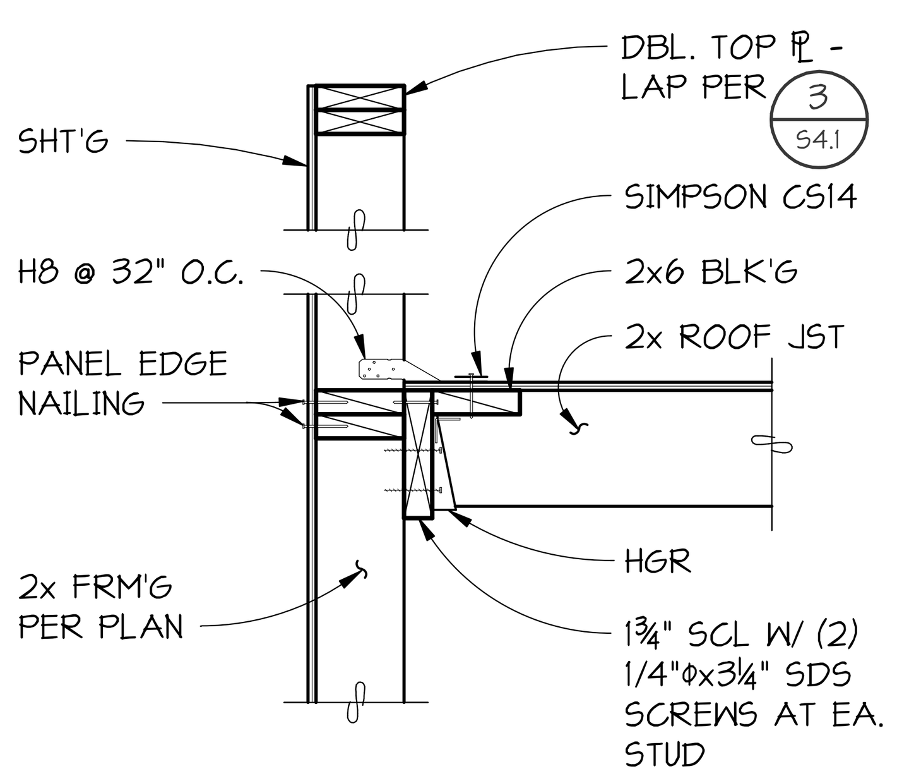
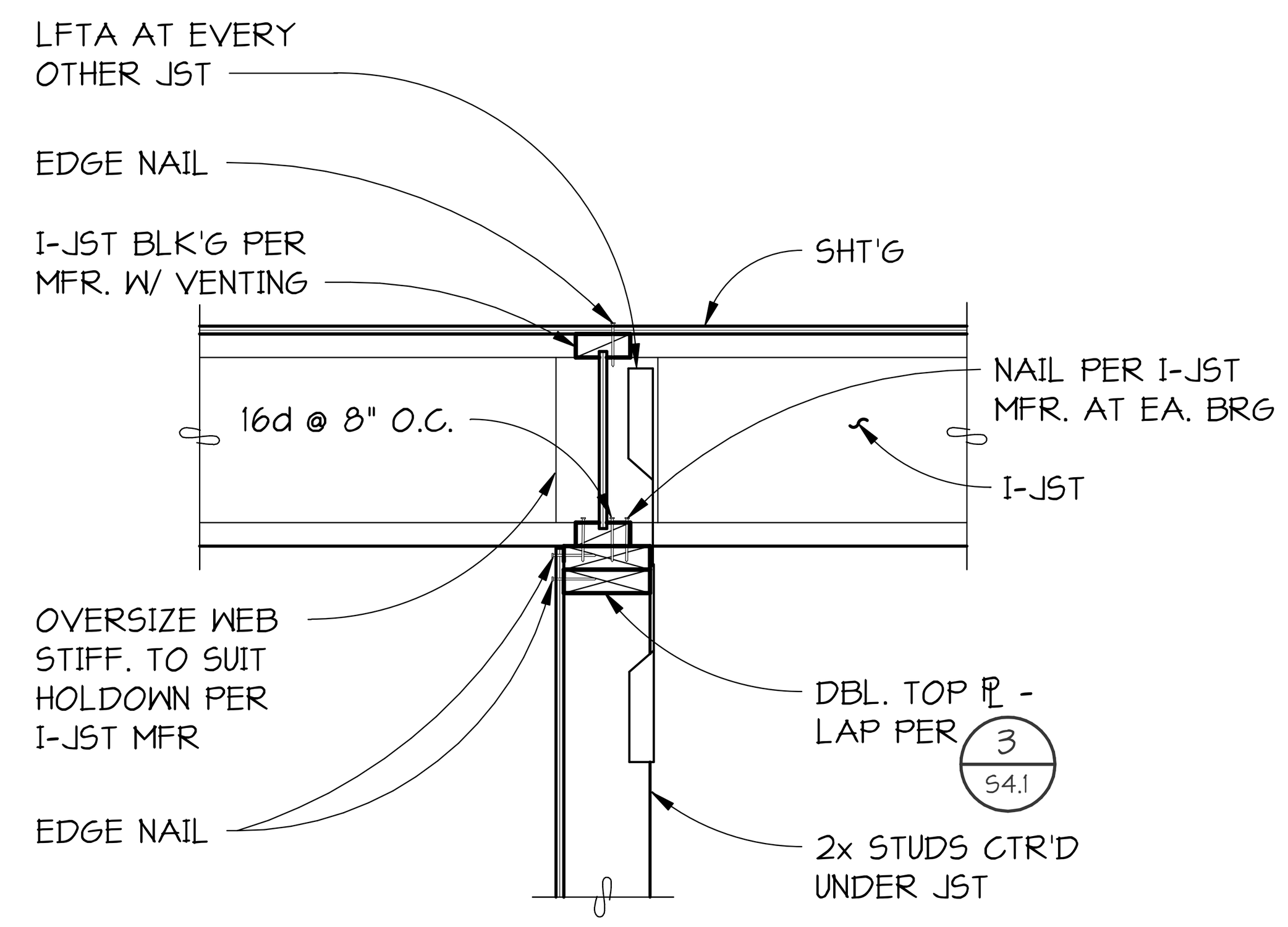


5 S6.0 SECTION
NO SCALE



TYPICAL I-JOIST TO WALL CONNECTION

6 S6.0 SECTION
NO SCALE

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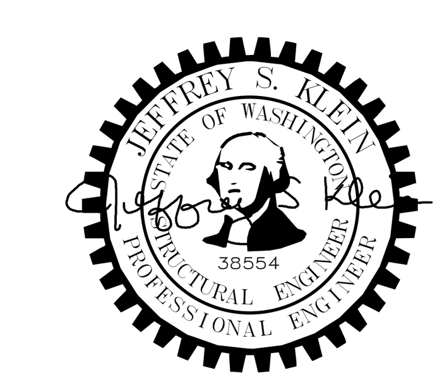

ROOF FRAMING DETAILS

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