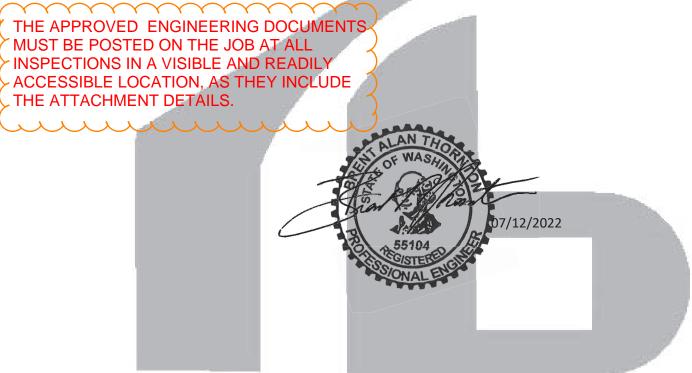
Reviewed 8/04/2022 DLSubject to field inspectors approvals.

Korum Lincoln 100 River Road Puyallup, WA 98371

RBA Job No. 224639.04

CALCULATIONS FOR: LN-DN-12

Designed in accordance with: 2018 International Building Code ASCE 7-16 Wind Velocity = 115 mph Risk Category II



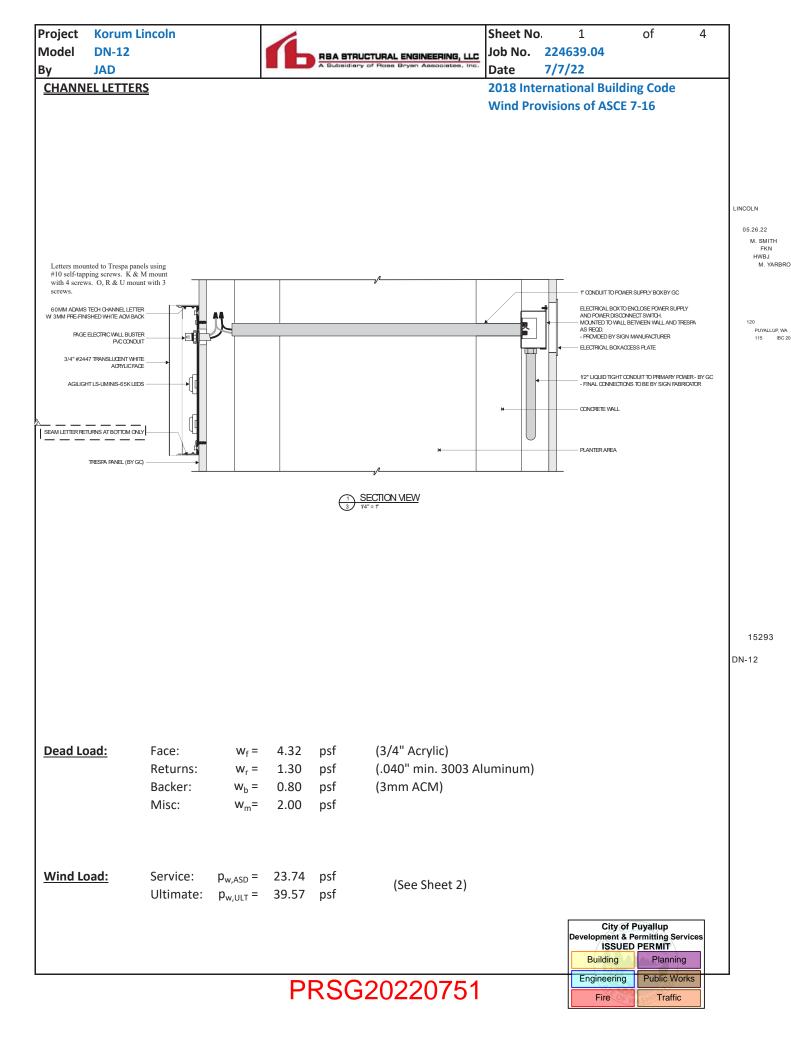
FABRICATOR

Architectural Graphics, Inc. 2655 International Parkway Virginia Beach, Virginia 23452

DESIGN ENGINEER

RBA Structural Engineering, LLC 1 Vantage Way, Suite B-400 Nashville, Tennessee 37228



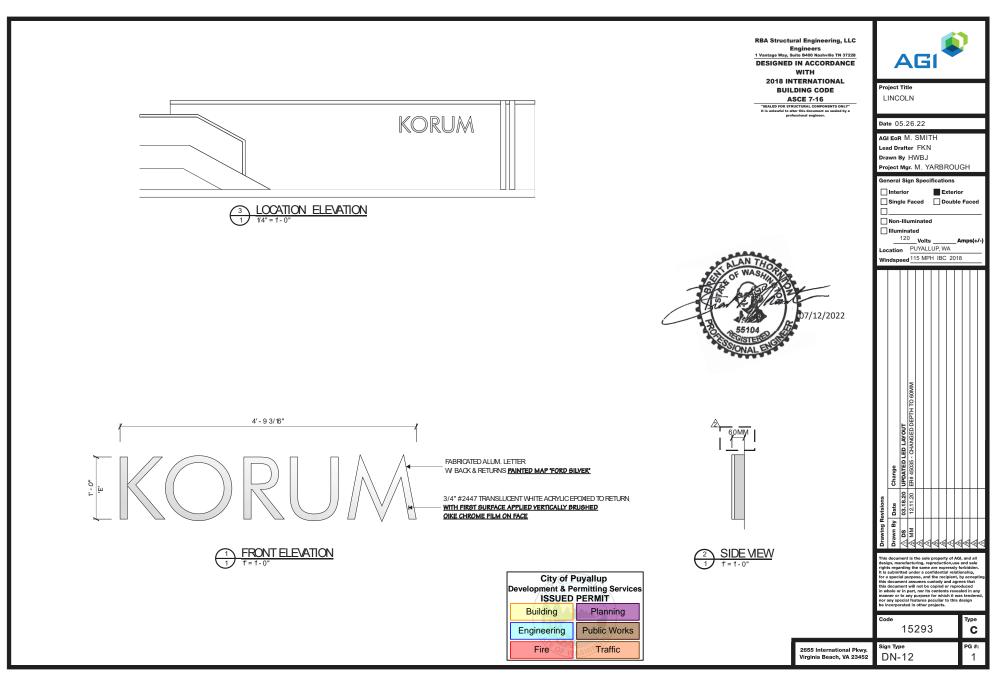


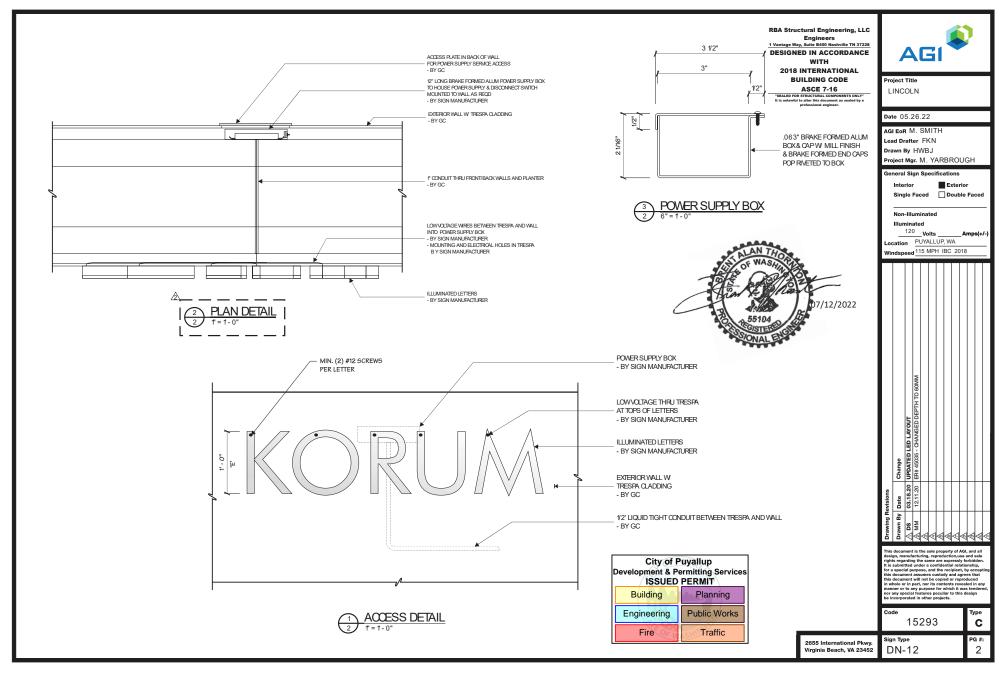
Project Model By	Korum Lincoln DN-12 JAD	1	RBA STRUCT	URAL ENGI Ross Brysn	NEERING, LLC Associates, Inc	Sheet No. Job No. Date	2 224639.04 7/7/2022	of	4
<u>CODES</u>		ovisions of AS	SCE 7-16, Chapt	er 30		Dute	.,,,		
<u>SIGN DI</u>	MENSIONS:					0	AH of Sign, z	= 30	ft.
	Length, B = 4.77	7 ft.	Height, s =	1.00	ft.	OAH	of Building, h	= 30	ft.
	Depth = 0.20	0 ft.	A _{sign} =	1.5	ft ²	Ground	Elevation, z _g	= 0	ft.
<u>W</u>	IND LOADS: (For Eff	fective Wind A	Area $\leq 10 \text{ ft}^2$)						
	Exposure Catego	ory = C		Ri	sk Category	y = 11			
q _h = q _z = q _h =		8 8 0 5 0	Velocity Pre Velocity Pre Topographic Wind Direct Ground Elev	ssure Exp ssure Exp Factor, A ionality F ration Fac	oosure Coe oosure Coe ASCE 7-16, actor, ASCE	ection 26.10.2 fficient @ z, <i>A</i> fficient @ h, <i>J</i> Section 26.8. E 7-16, Table 7-16, Table 26 -16, Figure 26	ASCE 7-16, Ta ASCE 7-16, Ta 2 26.6-1 5.9-1		
p = p+ = p- = p _{max} =	-39.57 lb/ft ²	0 0	Positive Extended	ernal Pres ternal Pre	ssure Coeff essure Coe ⁻	Section 30.3. ficient, ASCE 7 fficient, ASCE SCE 7-16, Sect	7-16, Figure 3 7-16, Figure 3		
<u>LR</u>	FD Loading:								
<u>A</u>	Use wind pressu <u>SD Loading:</u>	re = 39.57	lb/ft ²	for 1.0*W	/ from ASC	E 7-16, Sectio	Г	City of Puy relopment & Perm	
	Use wind pressu	re = 23.74	lb/ft ²	for 0.6*W	/ from ASC	E 7-16, Sectio	n 2 4 1	ISSUED PE Building	

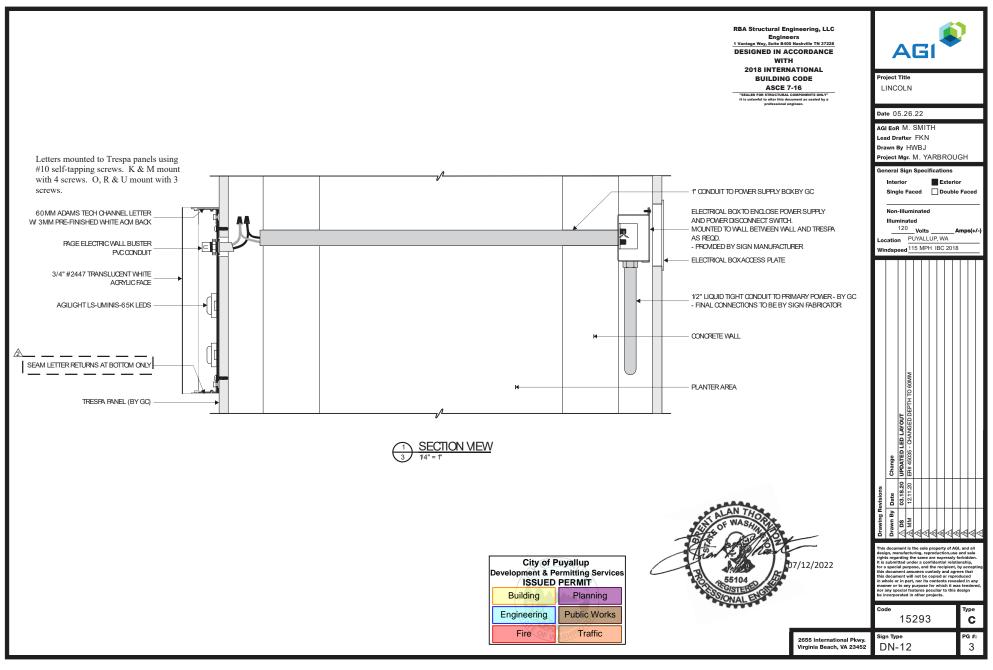
Project Ko Model DN By JAI	-12	coln		1		CTURAL ENGIN of Ross Brysn /		Sheet No. Job No. Date	3 224639.04 7/7/22	of	
Letter Pro	<u>perties</u>	Tota	l Length:	4.77	ft	(Length o	f Entire Lette	erset)			
	-		al Height:	1.00	ft	(Height o	f Entire Lette	erset)			
			n Depth:	2.4	in	(Face-to-l	Backer Depth	ı)			
	Турі	cal Lette	er Stroke:	1.6	in	(Stroke W	/idth of Large	est Letters)			
	Distand	e to Cor	nnection:	0	in	(Approx.	Connection [Distance from	Backer to Anch	iorage)	
Letter Size	es:	(Areas/	/perimete	rs determin	ed by compu	ıter analysi	is)				
	etter:	Area (in ²):	Per. (in):	Total Dead Load (lb):		# of	Vert. Anchor Spacing:	Shear per Anchor (lb):	Tension per Anchor (lb):		
	К	38	62	3.2	6.3	2	10.0	1.6	3.7		
	0	50	40	3.3	8.2	2	10.0	1.7	4.8		
	R	42	50	3.2	6.9	2	10.0	1.6	4.0		
	U	40	59	3.3	6.6	2	10.0	1.6	3.9		
	Μ	52	85	4.4	8.6	2	10.0	2.2	5.1		
								Ē	City of Puyallu Development & Permittin / ISSUED PERM Building	g Services	
<u>Backer:</u>		Constru M _{max} =	$\frac{PL}{4}$	3mm ACM =	2.1	lb-in			Development & Permittin ISSUED PERM Building Pla Engineering Public	g Services IT	
<u>Backer:</u>		M _{max} = P = Ma	$\frac{PL}{4}$ x Anchor	= Tension =	5.1	lb			Development & Permittin ISSUED PERM Building Pla Engineering Public	g Services IT anning c Works	
<u>Backer:</u>		M _{max} = P = Ma	$\frac{PL}{4}$	= Tension =					Development & Permittin ISSUED PERM Building Pla Engineering Public	g Services IT anning c Works	
<u>Backer:</u>		M _{max} = P = Ma L = Typ	$\frac{PL}{4}$ x Anchor	= Tension = Stroke = 	5.1	lb	<	0.083 (Apparent Th	Development & Permittin ISSUED PERM Building Pla Engineering Publi Fire T	g Services IT anning c Works	
<u>Backer:</u>		M _{max} = P = Ma L = Typ	$\frac{PL}{4}$ x Anchor fical Letter $\sqrt{\frac{6 \cdot M_{mo}}{f_y \cdot b}}$ 22000	= Tension = Stroke = <u>x</u> =	5.1 1.6	lb in in		0.083	Development & Permittin ISSUED PERM Building Pla Engineering Publi Fire T	g Services IT c Works raffic	
Backer:		M _{max} = P = Ma L = Typ t _{min} = fy =	$\frac{PL}{4}$ x Anchor - ical Letter $\sqrt{\frac{6 \cdot M_{mo}}{f_y \cdot b}}$ 22000 2.8	= Tension = \cdot Stroke = $\frac{1}{2}$ = psi in	5.1 1.6 0.014	lb in in 30° angle to		0.083	Development & Permittin ISSUED PERM Building Pla Engineering Publi Fire T	g Services IT c Works raffic	
		M _{max} = P = Ma L = Typ t _{min} = fy = b = Constru	$\frac{PL}{4}$ x Anchor - ical Letter $\sqrt{\frac{6 \cdot M_{mo}}{f_y \cdot b}}$ 22000 2.8	= Tension = \cdot Stroke = $\frac{1}{2}$ = psi in	5.1 1.6 0.014 (Based on 3	lb in in 30° angle to		0.083 (Apparent Th	bevelopment & Permittin ISSUED PERM Building Pla Engineering Public Fire T in hickness) 60.00 Pepth =	g Services IT c Works raffic	i
		M _{max} = P = Ma L = Typ t _{min} = fy = b = Constru M _{max} =	$\frac{PL}{4}$ x Anchor - ical Letter $\sqrt{\frac{6 \cdot M_{mo}}{f_y \cdot b}}$ 22000 2.8 uction: $\frac{wL^2}{8}$	$=$ Tension = Stroke = $\frac{1}{2x} =$ psi in .040" 3003	5.1 1.6 0.014 (Based on 3 Aluminum (I	lb in in 30° angle to Jnwelded)		0.083 (Apparent The b/t = λ = b = Return D	bevelopment & Permittin ISSUED PERM Building Pla Engineering Public Fire T in hickness) 60.00 Pepth =	g Services IT c Works raffic <u>O.K.</u> 2.4	i
		$M_{max} =$ $P = Ma$ $L = Typ$ $t_{min} =$ $fy =$ $b =$ $Construction M_{max} = w = Ha$	$\frac{PL}{4}$ x Anchor ical Letter $\sqrt{\frac{6 \cdot M_{mo}}{f_y \cdot b}}$ 22000 2.8 uction: $\frac{wL^2}{8}$ If Face Wi	= Tension = Stroke = $\frac{1}{2}$ = psi in .040" 3003 =	5.1 1.6 0.014 (Based on 3 Aluminum (U 0.2 1.63	lb in in 30° angle to Jnwelded) lb-ft		0.083 (Apparent The b/t = λ = b = Return D t = Return T	bevelopment & Permittin ISSUED PERM Building Pla Engineering Public Fire T in hickness) 60.00 Pepth =	g Services IT c Works raffic <u>O.K.</u> 2.4 0.040	i
	220	$M_{max} = M_{max} = M_{max} = M_{max} = M_{min} = M_{min} = M_{min} = M_{max} = M_{m$	$\frac{PL}{4}$ x Anchor - ical Letter $\sqrt{\frac{6 \cdot M_{mo}}{f_y \cdot b}}$ 22000 2.8 uction: $\frac{wL^2}{8}$ If Face With Vert. And	= Tension = Stroke = $\frac{1}{2X}$ = psi in .040" 3003 = nd Load =	5.1 1.6 0.014 (Based on 3 Aluminum (U 0.2 1.63	lb in 30° angle to Jnwelded) lb-ft plf		0.083 (Apparent The b/t = λ = b = Return D t = Return T	bevelopment & Permittin ISSUED PERM Building Pla Engineering Public Fire T hickness) 60.00 Pepth = hickness =	g Services IT c Works raffic <u>O.K.</u> 2.4 0.040	

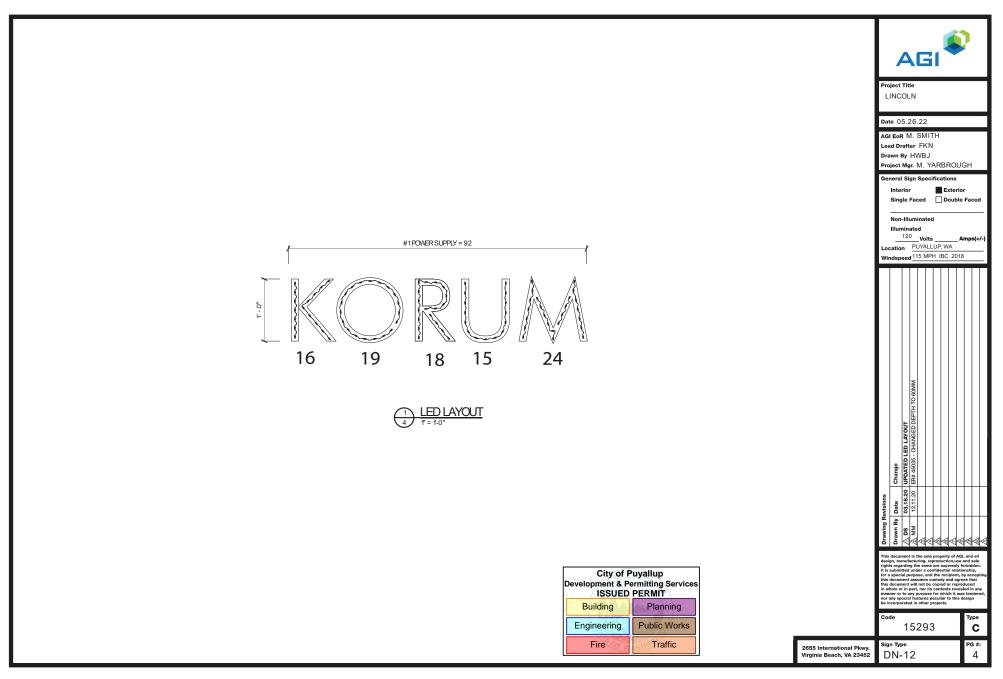
Project	Korum Lincoln	1			•	Sheet No.		4	of	4
Model	DN-12		RBA STRUCTURA	L ENGINEER	ING. LLC	Job No.	224639	.04		
Ву	JAD		A Subsidiary of Ros			Date	07/07/2	22		
CHECK	<u>LETTER ANCHORS</u> :									
	Anchor Loads:									
	T May Angles Tangian	F 4	п.							
	T _{max} = Max Anchor Tension =	5.1	lb							
	V _{max} = Max Anchor Shear =	2.2	lb							
	#12 Screws to Trespa Panel									
	T _{all} = Allowable Tension =	124	lb	>	5.1	lb			<u>O.K.</u>	
	V _{all} = Allowable Shear =	50	lb	>	2.2	lb			<u>O.K.</u>	

City of Puyallup Development & Permitting Services ISSUED PERMIT								
Building	Planning							
Engineering	Public Works							
Fire OF W	Traffic							









Korum Lincoln 100 River Road Puyallup, WA 98371

RBA Job No. 224639.02

CALCULATIONS FOR: LN-CL-32

Designed in accordance with: 2018 International Building Code ASCE 7-16 Wind Velocity = 115 mph Risk Category II



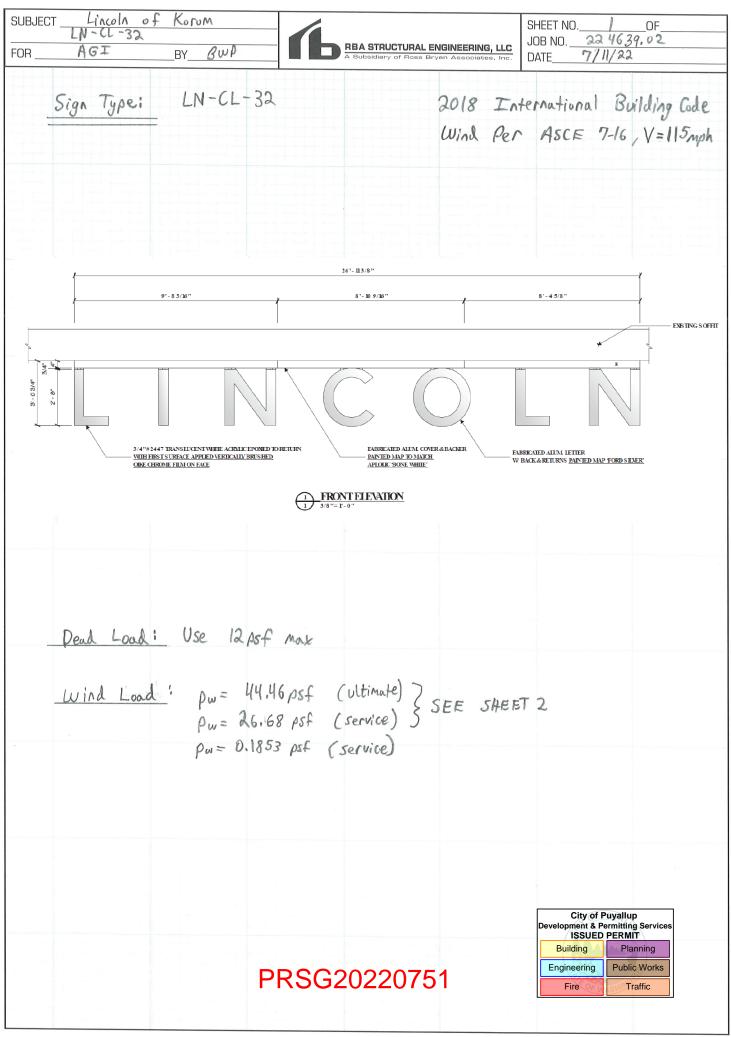
FABRICATOR

Architectural Graphics, Inc. 2655 International Parkway Virginia Beach, Virginia 23452

DESIGN ENGINEER

RBA Structural Engineering, LLC 1 Vantage Way, Suite B-400 Nashville, Tennessee 37228

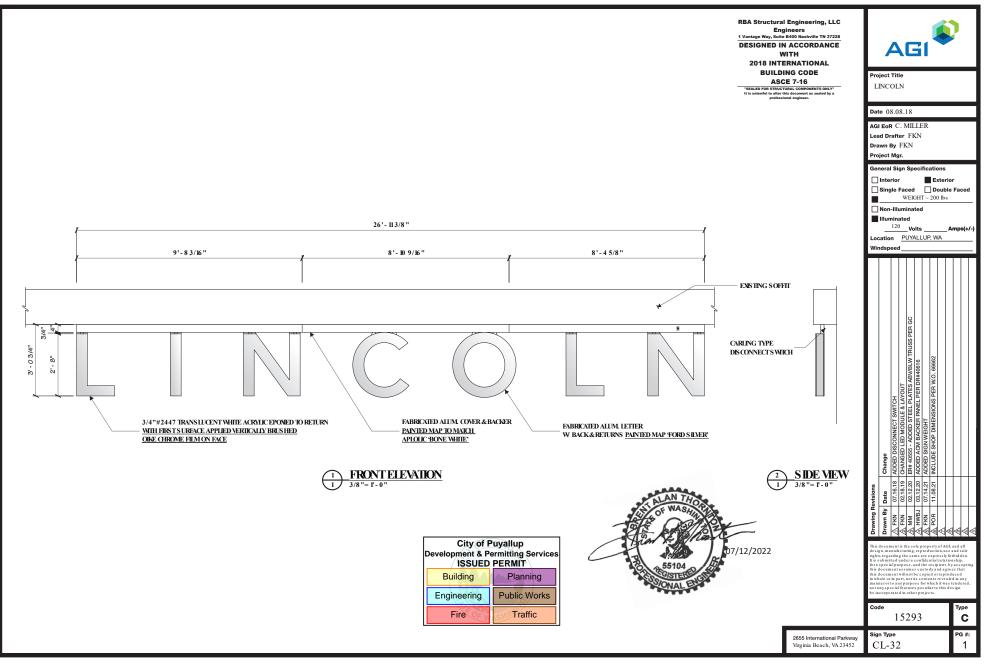


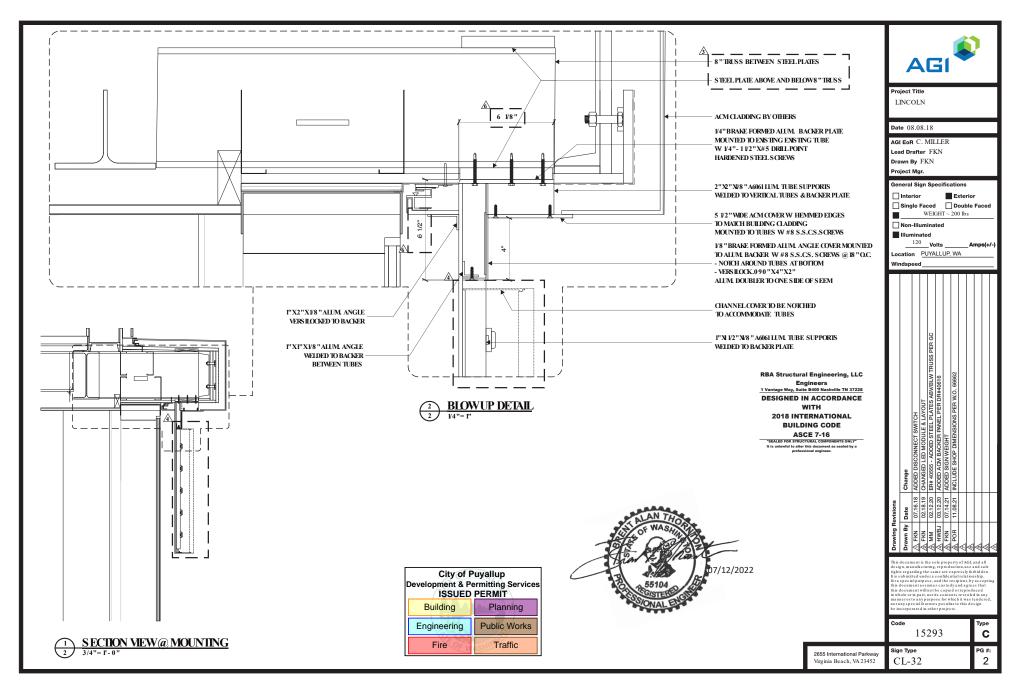


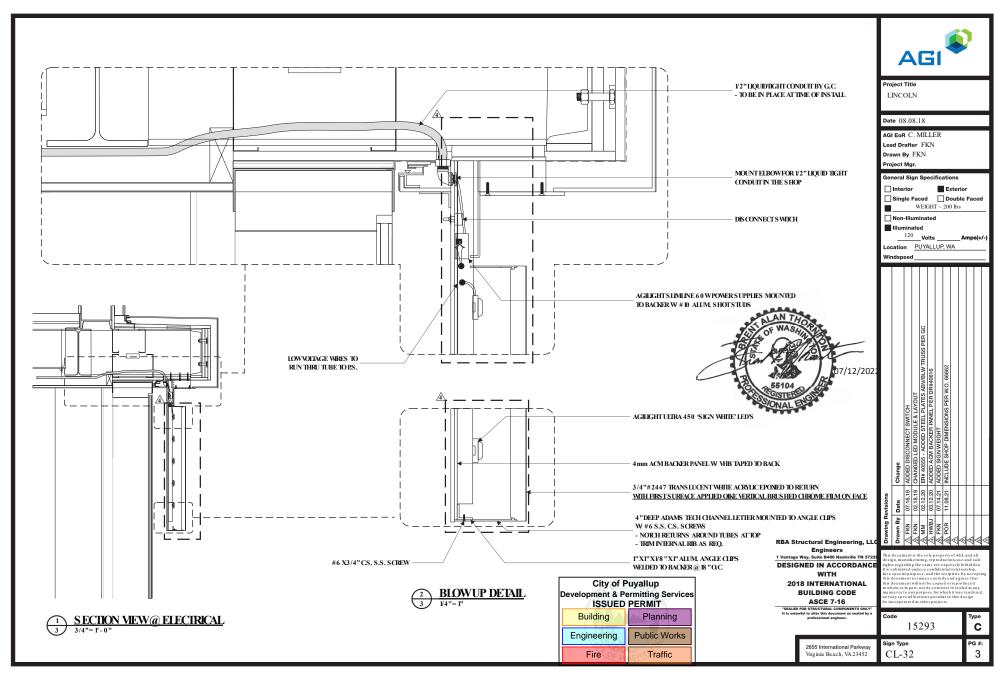
Project Model By	Lincoln of Ko LN-CL-32 BWP	orum	1	CONSU		CIATES, IN IGINEERS	C. Sheet No. Job No. Date	2 224639.02 7/11/2022		
CODES						,				
	Wind Loads	per provi	sions of AS	CE 7-16, Chap	oter 29					
<u>SIGN DI</u>	MENSIONS:									
	Length, B =	26.95	ft.	Height, s =	2.67	ft.	OAH Abo	ove Grade, h =	30 ft.	
	Depth, t =	0.33	ft.	A _{sign} =	72.0	ft ²	Ground	Elevation, z _g =	0 ft.	
<u>w</u>	IND LOADS:									
	Natural Fre	equency =	1		<u>RIGID ST</u>	<u>RUCTURE</u>				
	Exposure C	ategory =	C		Ris	sk Category	/= 11			
q _h =	• 0.00256 * K _z	* K _{zt} * K _d	* K _e * V ²	Velocity Pro	essure, AS	SCE 7-16, Se	ection 26.10.2	2		
	K _z =	0.98		Velocity Pre	essure Exp	oosure Coe	fficient, ASCE	7-16, Table 26	.10-1	
	K _{zt} =	1.0		Topograph	ic Factor,	ASCE 7-16,	Section 26.8	.2		
	K _d =	0.85		Wind Direc	tionality F	actor, ASC	E 7-16, Table	26.6-1		
	K _e =	1.00		Ground Ele	vation Fac	ctor, ASCE	7-16, Table 20	5.9-1		
	V =	115		Basic Wind	Speed, m	ph, ASCE 7	-16, Figure 26	6.5-1B		
q _h =	= 28.27	b/ft ²								
F/A =	• q _h * G * C _f			Design Win	d Loads, A	ASCE 7-16,	Section 29.3.	1		
	G =	0.85		U U	-	-	ection 26.11			
	B/s =	10.09		Length of S	ign/Depth	n of Sign				
	s/h =	0.09		Depth of Si	gn/Overal	ll Height				
	C _f =	1.85		Force Coeff	icient, AS	CE 7-16, Fi	gure 29.3-1			
F/A =	= 44.46	b/ft ²		CASE A: res	ultant act	s normal to	o sign face th	rough the geon	netric center	
				CASE B: res toward the			-	a distance from	n the geometric ce	enter
				CASE C load	ding applie	es				
LR	FD Loading:									
	Use wind p	oressure =	44.46	lb/ft ²	for 1.0*V	V from ASC	E 7-16, Sectio	on 2.3.1 De	City of Puyallup velopment & Permitting Ser ISSUED PERMIT Building Planning	
<u>A</u>	<u>SD Loading:</u>								Engineering Public Woo Fire Traffic	rks
	Use wind p	ressure =	26.68	lb/ft ²	for 0.6*V	V from ASC	CE 7-16, Section	on 2.4.1		

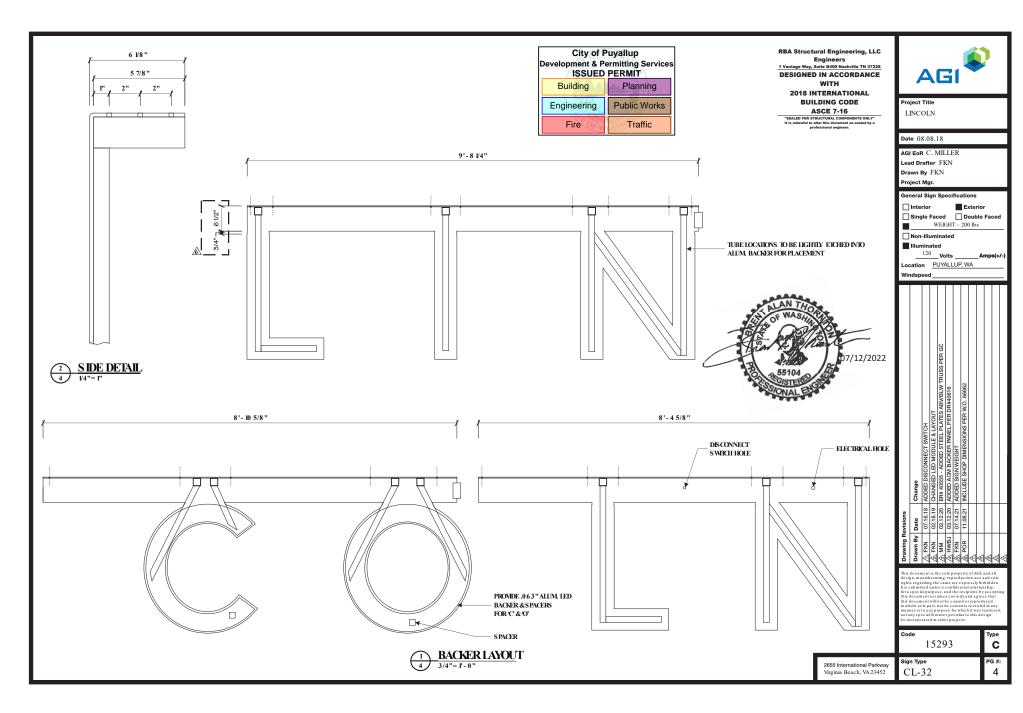
BLEET
$$\frac{LI_{1}O_{2}I_{1} \rightarrow g^{2} - K_{0}OVM}{M_{0} - 224 (16.3^{2}) OZ}$$

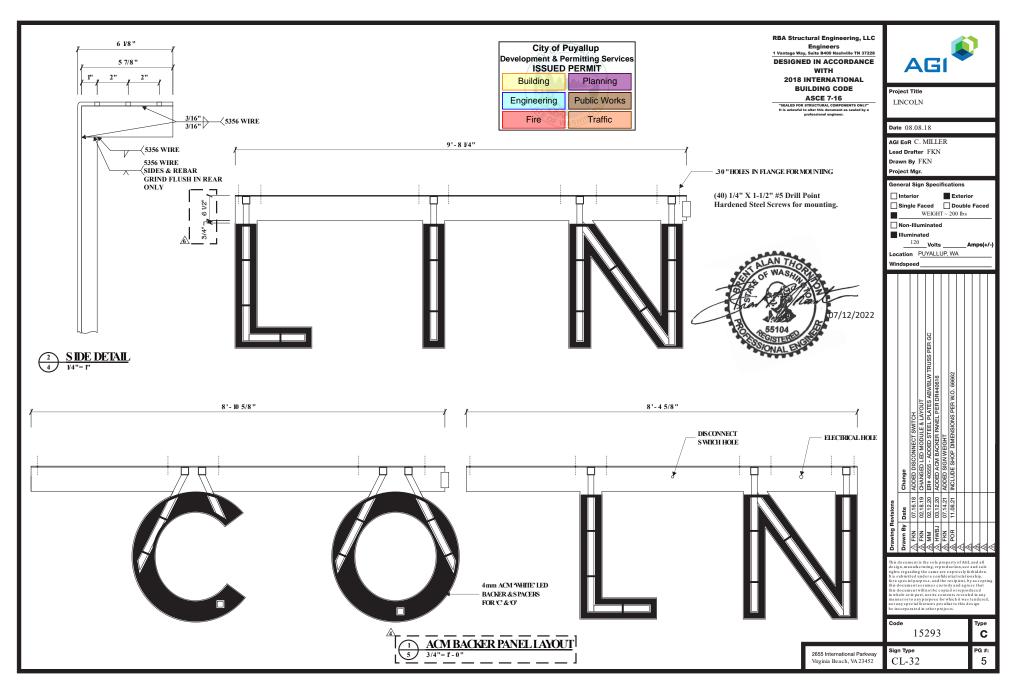
FOR AGI BY BWP
Check Additional Torssion:
Two (0.1853 psi) (140 in²) (121n) = 311 ± in (Significant Torsion Only @ Gardenel Seeby
Check Torsional Capacity of Plate (Only Worst Case) 6061 Aluminum Welded
 $T = \frac{3}{8 \text{ cu}^{3}} \left[1 + 0.5075 (M_{0}) + 0.8265 (M_{0})^{2} + 1.8023 (M_{0})^{2} + 0.919 (M_{0})^{4} \right]$
 $M_{0} = \frac{0.5 (0.55i)}{0.5 (0.55i)} = 0.09544$
 $T = \frac{3(311 \text{ win})}{g(2.156A)(0.55A)^{5}} \left[1 + 0.6095(0.0457) + 0.8055(0.0089)^{2} - 1.8023 (0.0089)^{3} + 0.910 (0.0459)^{4} \right]$
 $T = 3(311 \text{ win})$
 $T = 5500 \text{ psi} > 27944 \text{ psi} (M_{0}) \Rightarrow Taucow = \frac{5500 \text{ psi}}{2.7944 \text{ psi}} (311 \pm in) = 612 \pm in$
 $T_{V} = 5500 \text{ psi} > 27944 \text{ psi} (M_{0}) \Rightarrow Taucow = \frac{5500 \text{ psi}}{2.7944 \text{ psi}} (311 \pm in) = 612 \pm in$
 $T_{V} = 5500 \text{ psi} > 27944 \text{ psi} (M_{0}) \Rightarrow Taucow = \frac{5500 \text{ psi}}{2.7944 \text{ psi}} (311 \pm in) = 612 \pm in$
 $T_{V} = 5500 \text{ psi} > 27944 \text{ psi} (M_{0}) \Rightarrow Taucow = \frac{5500 \text{ psi}}{2.7944 \text{ psi}} (311 \pm in) = 612 \pm in$
 $T_{V} = 5500 \text{ psi} > 2.0944 \text{ psi} (M_{0}) \Rightarrow Taucow = \frac{5500 \text{ psi}}{2.7944 \text{ psi}} (311 \pm in) = 612 \pm in$
 $T_{V} = 5500 \text{ psi} > 2.0944 \text{ psi} (M_{0}) \Rightarrow 1.655 \pm in > 311 \pm in (M_{0}) = 0.301 \text{ in}^{3}$
 $(For Correr Radius "F" = 0)$
 $Taucow = (5500 \text{ psi}) (0.301 \text{ in})^{2} + 1655 \pm in > 311 \pm in (M_{0}) = 0.301 \text{ in}^{3}$
 $(For Correr Radius "F" = 0)$
 $Taucow = (5500 \text{ psi}) (0.301 \text{ in})^{2} + 1655 \pm in > 311 \pm in (M_{0}) = 0.301 \text{ in}^{3}$
 $(For Correr Radius "F" = 0)$
 $Taucow = (5500 \text{ psi}) (10.901 \text{ in}) = 1655 \pm in > 311 \pm in (M_{0}) = 0.301 \text{ in}^{3}$
 $(For Correr Radius "F" = 0)$
 $Taucow = (500 \text{ psi}) (10.901 \text{ in}) = 1655 \pm in > 311 \pm in (M_{0}) = 0.301 \text{ in}^{3}$
 M_{0} M_{0} $M_{0} = \frac{1372 \pm in}{(520 \text{ in})^{2}} (12i_{0}) = 4100.2 \pm in$
 M_{0} $M_{0} = \frac{1372 \pm in}{(520 \text{ in})^{2}} (12i_{0}) = 4100.2 \pm in$
 M_{0} $M_{0} = 5 \text{ cu} = 13(260 \text{ psi}) (1557/144 \text{ in}^{3}) = 10024$
 $T_{0} = 1000 \pm (6 \text{ screws} = 16.7 \pm 777 \pm 7$

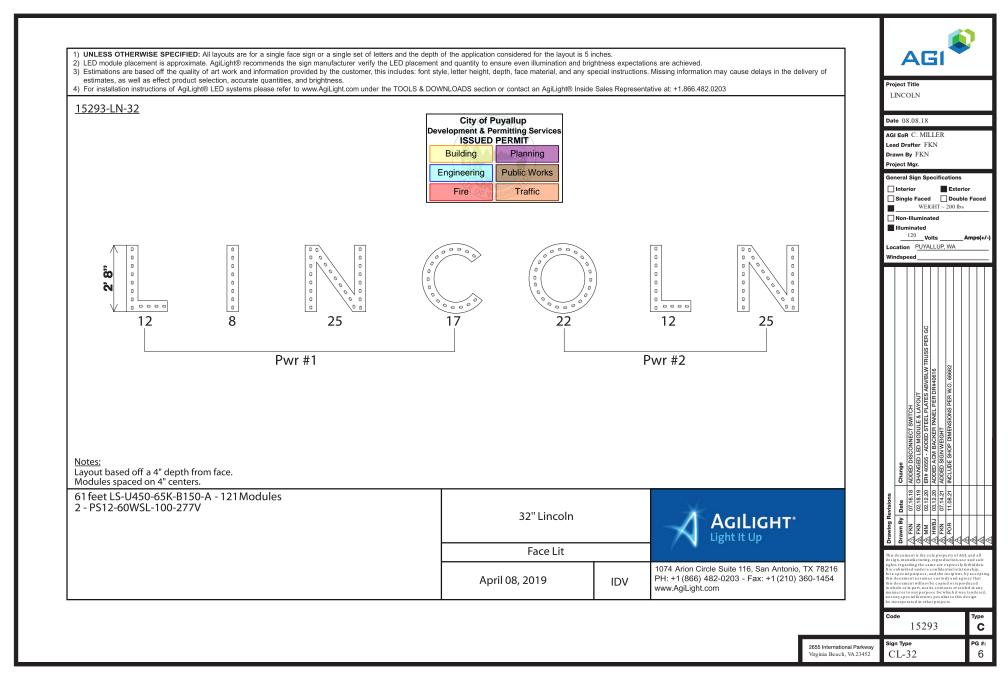












Korum Lincoln 100 River Road Puyallup, WA 98371

RBA Job No. 224639.05

CALCULATIONS FOR: LN-WT-76

Designed in accordance with: 2018 International Building Code ASCE 7-16 Wind Velocity = 115 mph Risk Category II



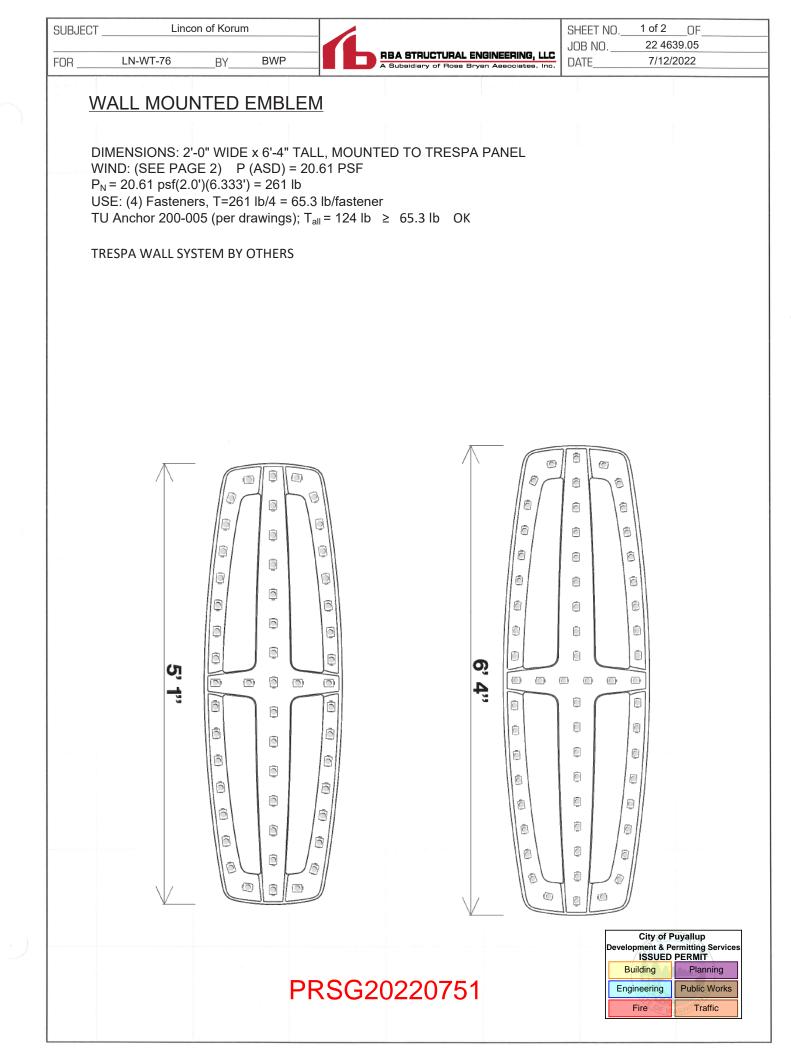
FABRICATOR

Architectural Graphics, Inc. 2655 International Parkway Virginia Beach, Virginia 23452

DESIGN ENGINEER

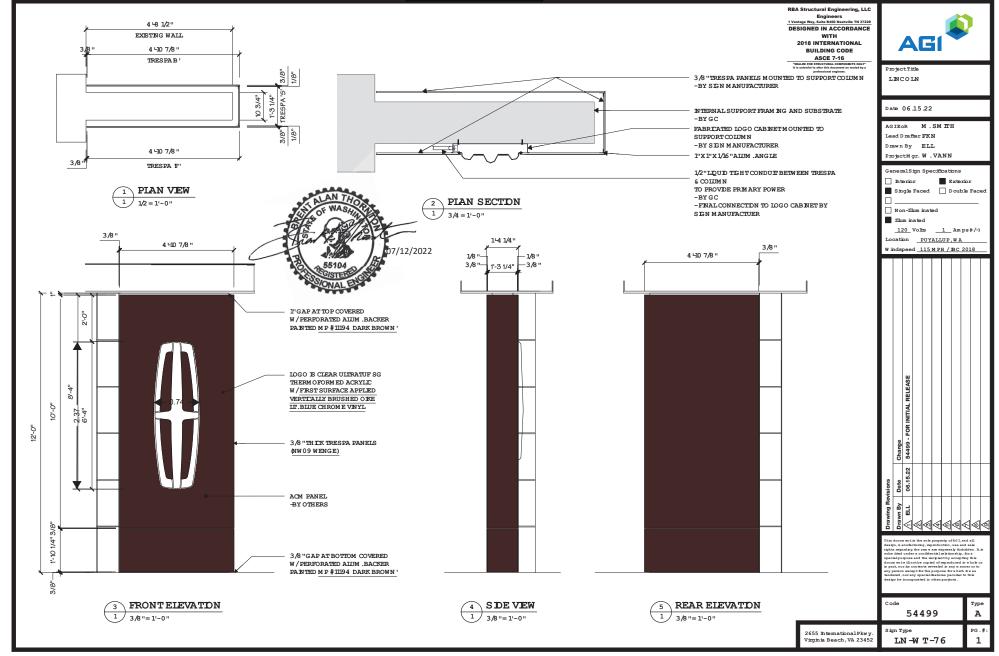
RBA Structural Engineering, LLC 1 Vantage Way, Suite B-400 Nashville, Tennessee 37228

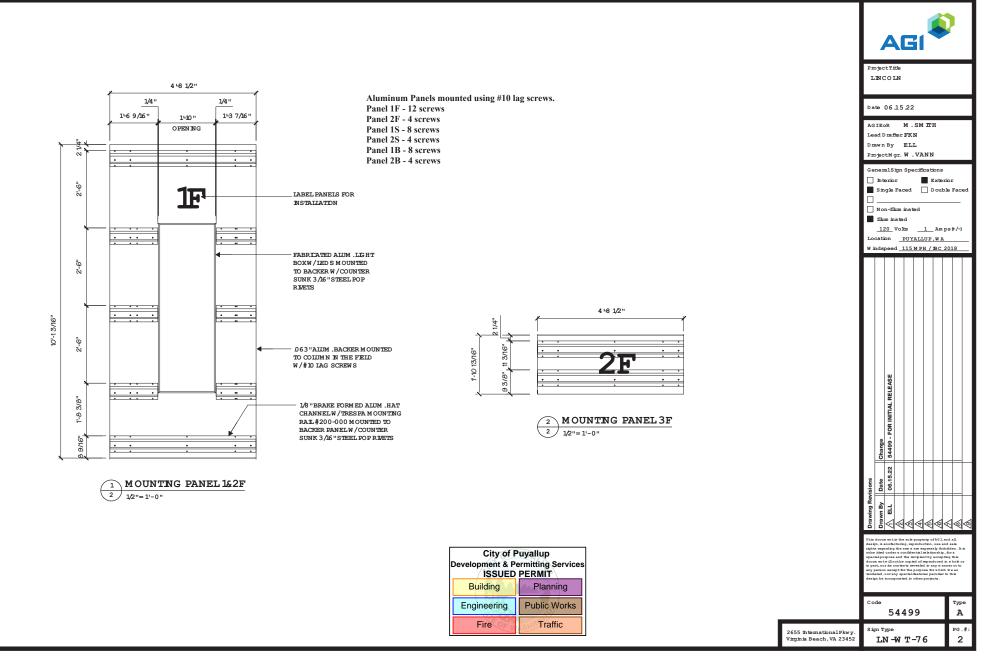
City of Puyallup Development & Permitting Service ISSUED PERMIT								
Building	Planning							
Engineering	Public Works							
Fire	Traffic							

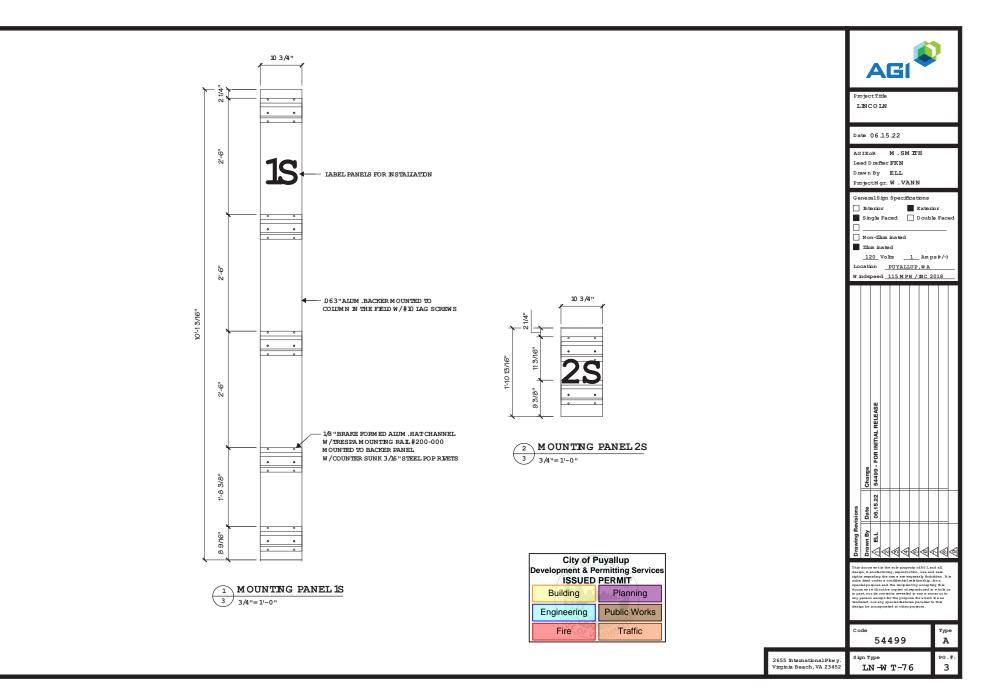


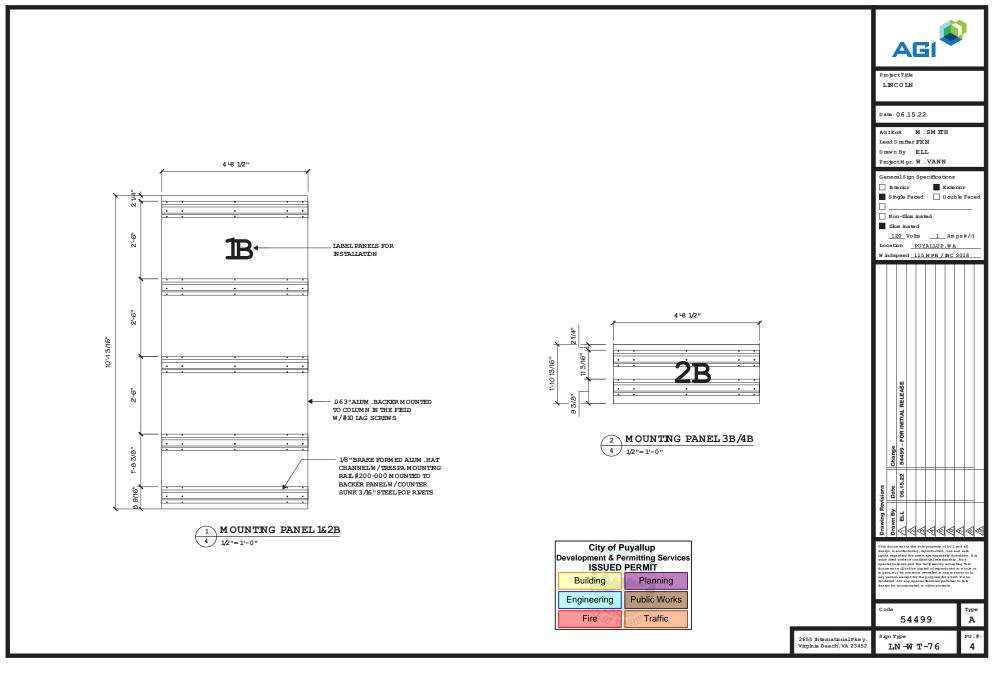
Project Model By <u>CODES</u> :	Lincoln of Korum WT-72 BWP	A Subeidiary of Rose Bryan Associates, Inc.	Sheet No. 2 of 2 Job No. 22 4639.05 2 2 Date 7/12/22 7 2
<u>CODES</u> .	Wind Loads per provisions of AS	CE 7-16, Chapter 30	
<u>SIGN DI</u>	<u>//ENSIONS</u> :		
	Length, B = 2.00 ft.	Height, s = 6.33 ft.	OAH of Sign, z = 13.33 ft.
	Depth = 0.33 ft.	$A_{sign} = 12.7 \text{ ft}^2$	OAH of Wall, h = 15.33 ft.
<u>wi</u>	<u>ND LOADS</u> : (For Effective Wind A	Area $\leq 10 \text{ ft}^2$)	
	Exposure Category = C	Risk Category =	= II
q _z = q _h =	$0.00256 * K_{z} * K_{zt} * K_{d} * V^{2}$ $K_{z} = 0.85$ $K_{h} = 0.85$ $K_{zt} = 1.0$ $K_{d} = 0.85$ $V = 115$ $24.46 lb/ft^{2}$ $24.54 lb/ft^{2}$ $qh * [(GCp) - (GCpi)]$		cient @ z, ASCE 7-16, Table 26.10-1 cient @ h, ASCE 7-16, Table 26.10-1 ection 26.8.2 7-16, Table 26.6-1 6, Figure 26.5-1B
F	$GC_{p+} = 1.00$	Positive External Pressure Coeffic	
	$GC_{p_{-}} = -1.40$ $GC_{p_{i}} = 0.0$	Negative External Pressure Coeffi Internal Pressure Coefficient, ASC	-
p+ = p- = p _{max} = <u>LRF</u>	24.54 lb/ft ² -34.36 lb/ft ²	internal Fressure Coefficient, ASC	L /-10, SECUUII 23.3.2
	Use wind pressure = 34.36	lb/ft ² for 1.0*W from ASCE	7-16 Section 2.3.1 City of Puyallup
AS	<u>D Loading:</u>	INFIL TOF 1.0 W HOIT AGE	Development & Permitting Services ISSUED PERMIT Building Planning Engineering Public Works Fire Traffic
	Use wind pressure = 20.61	lb/ft ² for 0.6*W from ASCE	7-16, Section 2.4.1

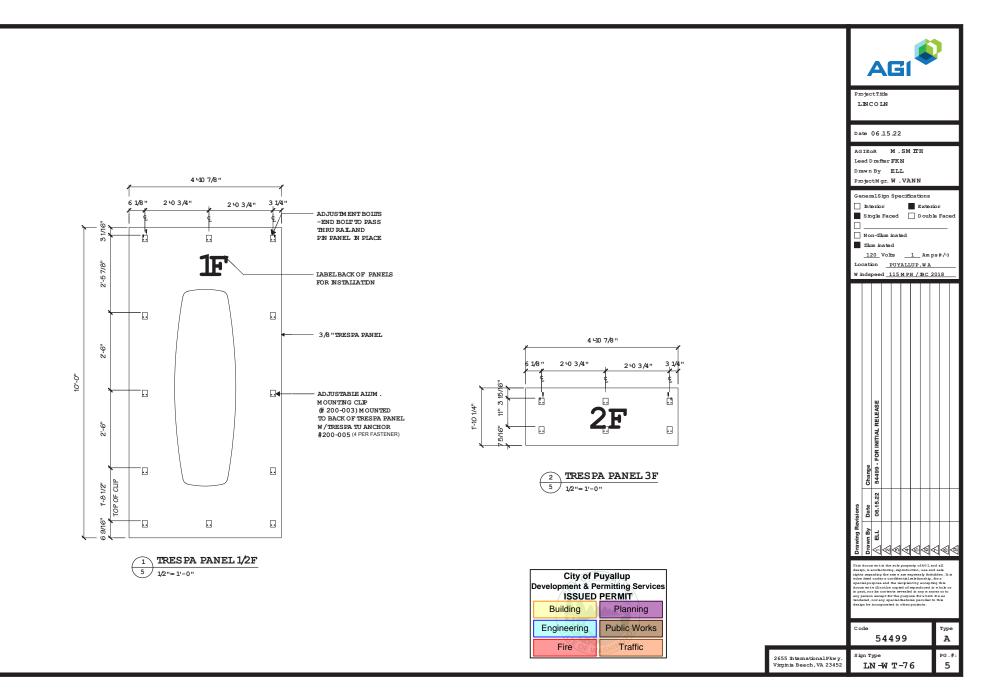


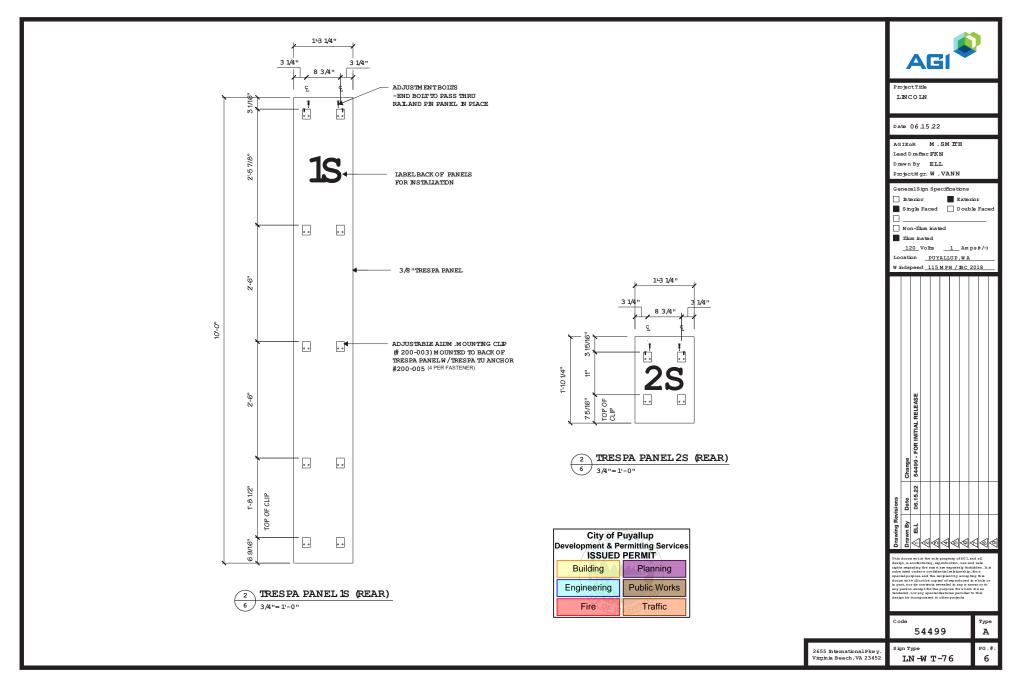




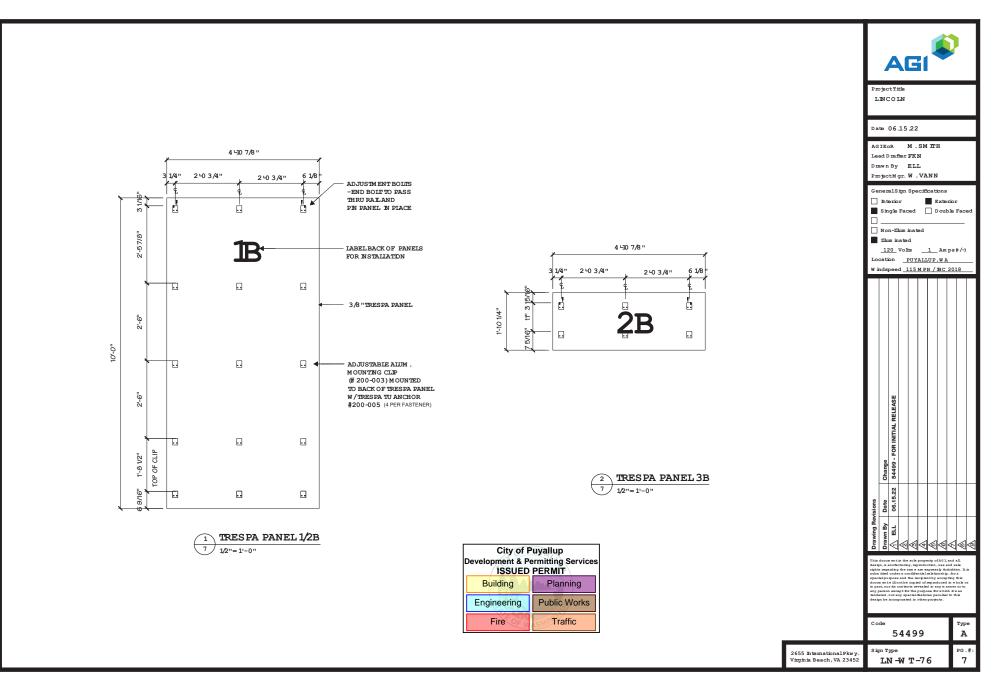


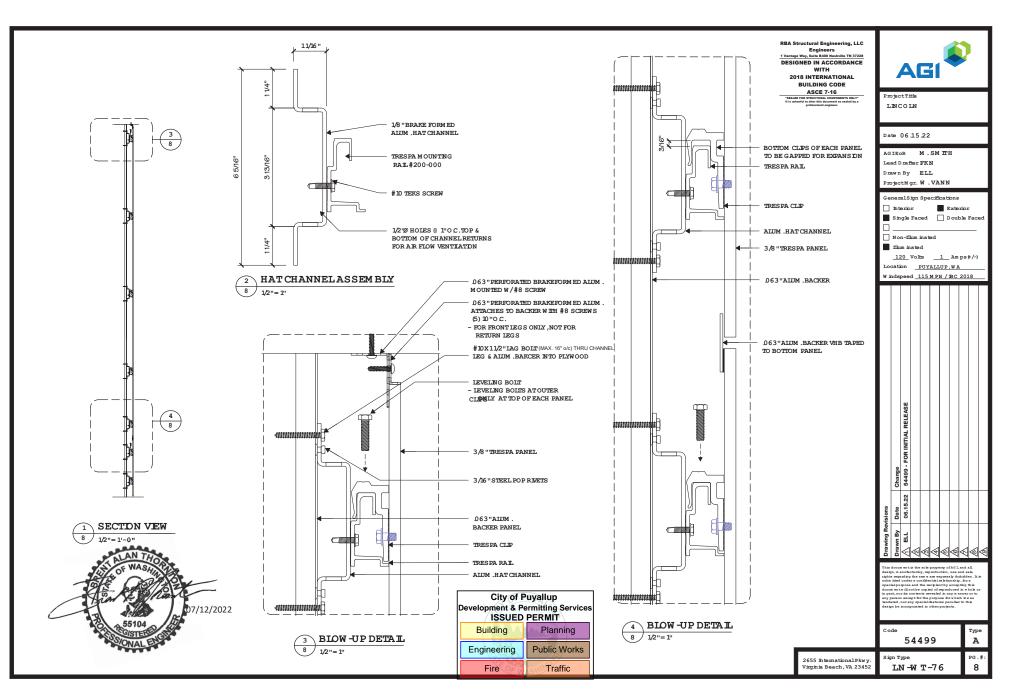


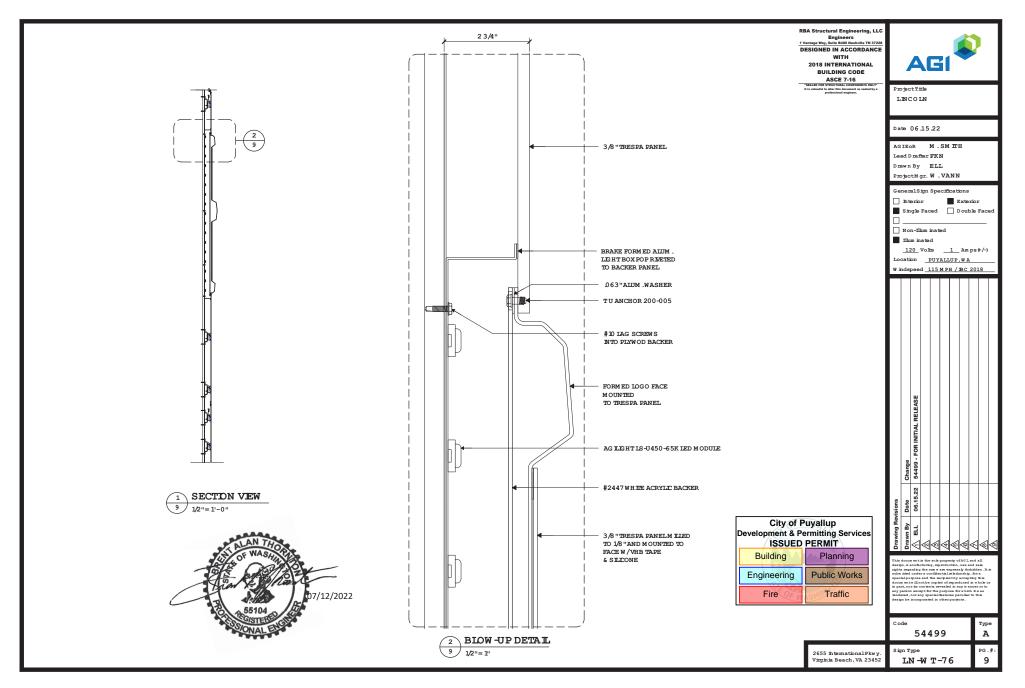


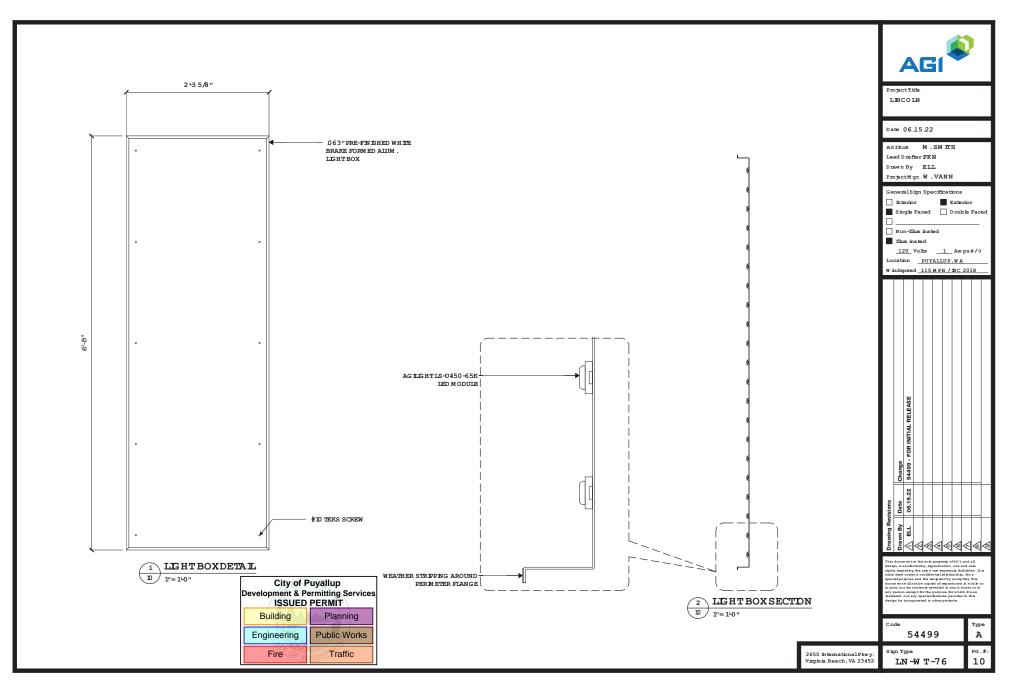


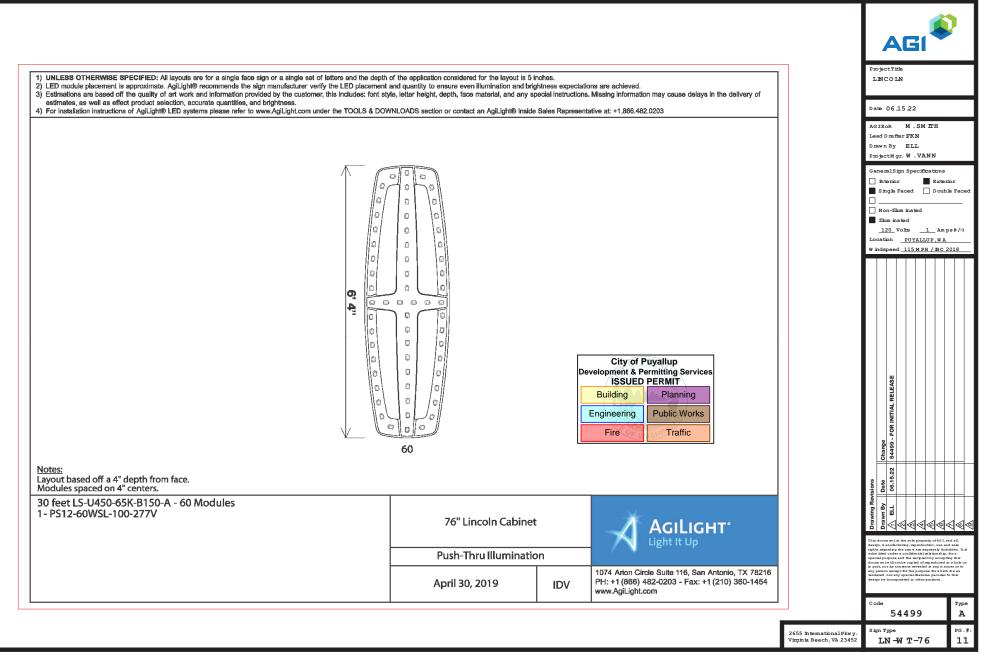
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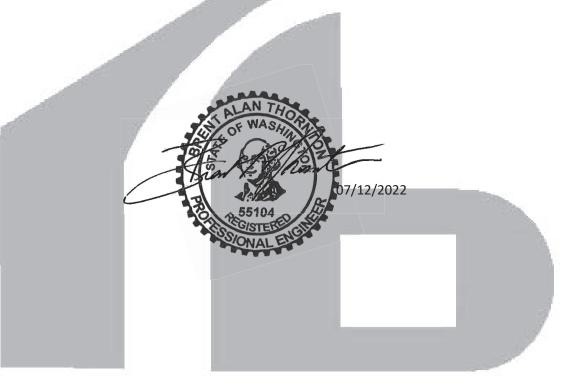


Korum Lincoln 100 River Road Puyallup, WA 98371

RBA Job No. 224639.01

CALCULATIONS FOR: LN-SV-18

Designed in accordance with: 2018 International Building Code ASCE 7-16 Wind Velocity = 115 mph Risk Category II



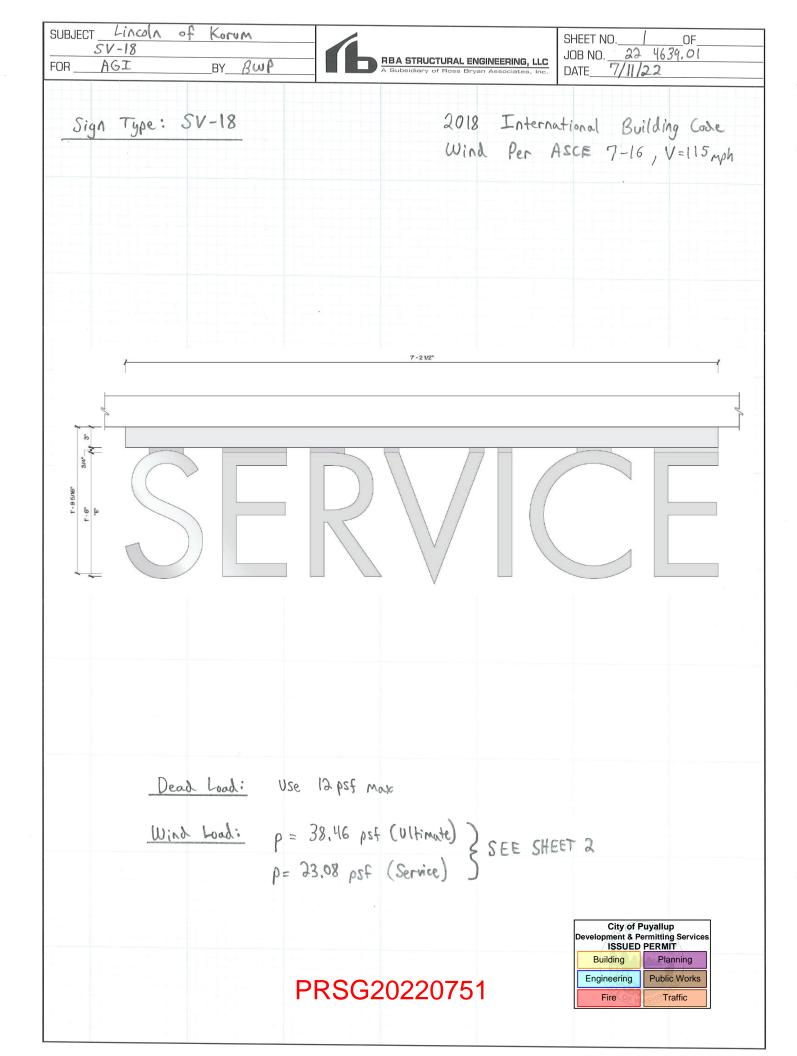
FABRICATOR

Architectural Graphics, Inc. 2655 International Parkway Virginia Beach, Virginia 23452

DESIGN ENGINEER

RBA Structural Engineering, LLC 1 Vantage Way, Suite B-400 Nashville, Tennessee 37228

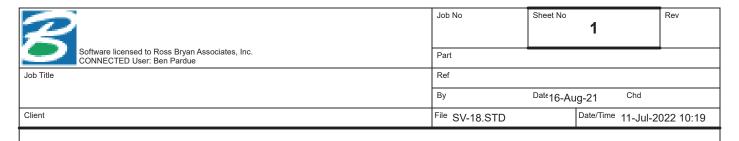




Project Model By	Korum Lincoln SV-18 BWP	1	CONSU		CIATES, INC. IGINEERS . TN	. Sheet No Job No. Date	224639.01 7/11/2022	of	4
<u>CODES</u> :						1			
	Wind Loads per pro	visions of AS	CE 7-16, Chap	ter 29					
<u>SIGN DI</u>	MENSIONS:								
	Length, B = 7.21	ft.	Height, s =	1.78	ft.	OAH Ab	ove Grade, h =	14 ft.	
	Depth, t = 0.33	ft.	A _{sign} =	12.8	ft ²	Ground	Elevation, z _g =	0 ft.	
<u>w</u>	IND LOADS:								
	Natural Frequency	/= 1	<u> </u>	RIGID STI	RUCTURE				
	Exposure Category	/ = C		Ris	k Category	= II			
g _b =	• 0.00256 * K _z * K _{zt} * F	<, * K. * V ²	Velocity Pre	ssure. AS	CE 7-16. Sec	ction 26.10.	2		
-10	$K_z = 0.85$	a ne					- E 7-16, Table 26	5.10-1	
	$K_{zt} = 1.0$		Topographic						
	$K_{d} = 0.85$		Wind Direct						
	$K_{e} = 1.00$		Ground Elev						
	V = 115		Basic Wind S						
q _h =	2			,p e e e e, j	p.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				
	10,10								
F/A =	• q _h * G * C _f		Design Wind	l Loads, A	ASCE 7-16, S	ection 29.3.	1		
,	G = 0.85		Gust Effect F						
	B/s = 4.05		Length of Sig	-	-				
	s/h = 0.13		Depth of Sig		-				
	C _f = 1.85		Force Coeffi		-	ure 29.3-1			
F/A =	38.46 lb/ft ²						rough the geor	netric center	-
,			CASE B: resu toward the v			0	a distance fror	n the geome	tric center
			CASE C load	ing applie	es				
LR	<u>FD Loading:</u>			-					
	Use wind pressure	e = 38.46	lb/ft ²	for 1.0*V	V from ASCE	7-16, Secti		City of Puyal velopment & Permit ISSUED PER	ting Services
<u>A</u>	SD Loading:							Engineering Pu	Planning blic Works Traffic
	Use wind pressure	23.08	lb/ft ²	for 0.6*V	V from ASCE	7-16, Secti	^{on 24} 1 PRSC	620220	0751

SUBLUT Lincoln of Korum

$$\frac{SUP_{18}}{SV-18}$$
 $\frac{1}{SV-18}$ $\frac{1}{SV-18$



1 WIND "LETTERS" : Node Loads

Node	FX	FY	FZ	MX	MY	MZ
	(kip)	(kip)	(kip)	(kip⁻ft)	(kip⁻ft)	(kip⁻ft)
2	-	-	0.023	0.02514	0.02108	-
3	-	-	0.023	0.02514	0.01058	-
4	-	-	0.028	0.02814	0.01309	-
5	-	-	0.0115	0.0107	-	-
6	-	-	0.0115	0.0107	-	-
7	-	-	0.012	0.00917	-	-
8	-	-	0.023	0.0266	0.0249	-
9	-	-	0.023	0.02514	0.01058	-

1 WIND "LETTERS" : Beam Loads

Beam	T	уре	Direction	Fa	Da	Fb	Db	Ecc.
					(ft)			(ft)
1	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
2	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
3	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
4	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
5	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
6	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
7	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
8	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-
9	UNI	lbf/ft	GZ	11.100	-	-	-	-
	UMO	lb-in/in	GX	1.375	-	-	-	-

3 DEAD "LETTERS" : Node Loads

Node	FX	FY	FZ	MX	MY	MZ
	(kip)	(kip)	(kip)	(kip⁻ft)	(kip⁻ft)	(kip⁻ft)
2	-	-0.00612	-	-	-	-
3	-	-0.0066	-	-	-	-
4	-	-0.00744	-	-	-	-
5	-	-0.003	-	-	-	-
6	-	-0.003	-	-	-	-
7	-	-0.00312	-	-	-	-
8	-	-0.00612	-	-	-	-
9	-	-0.0066	-	-	-	-

Development & Pe	City of Puyallup Development & Permitting Services ISSUED PERMIT						
Building	Planning						
Engineering	Public Works						
Fire OF M	Traffic						

Print Time/Date: 11/07/2022 11:04

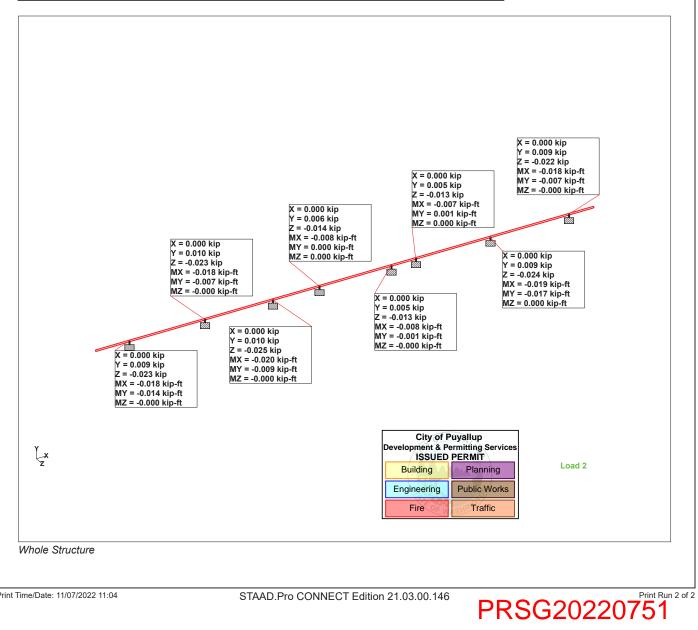
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Print Run 1 of 2

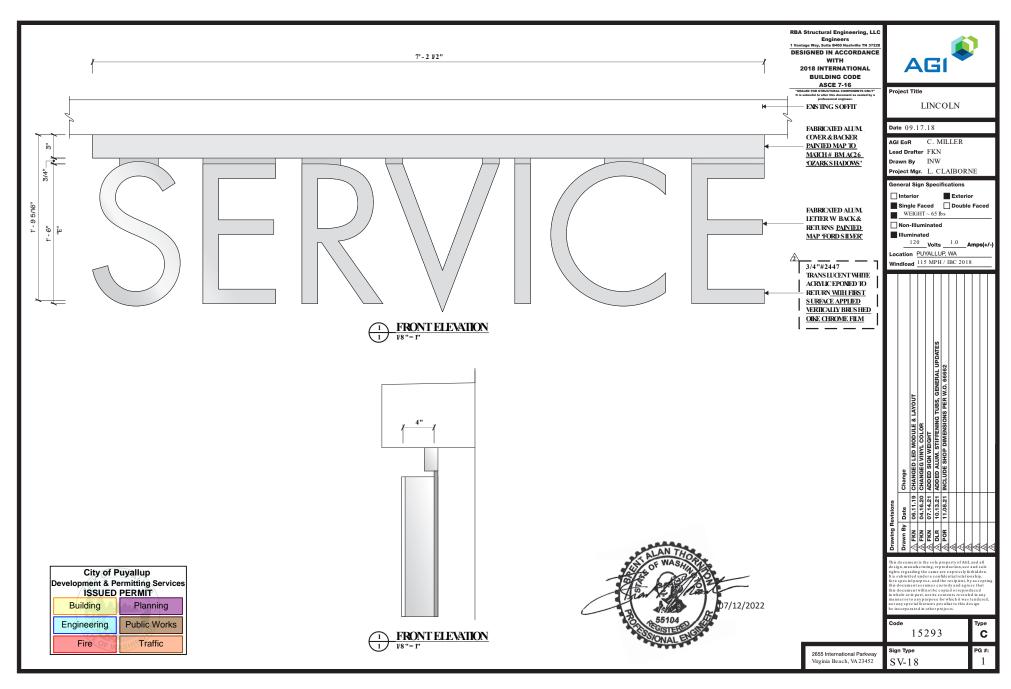
	Job No	Sheet No	2	Rev
	Part			
Job Title	Ref			
	Ву	^{Dat∈} 16-Au	g-21 ^{Chd}	
Client	^{File} SV-18.STD		Date/Time 11-Jul-20	022 10:19

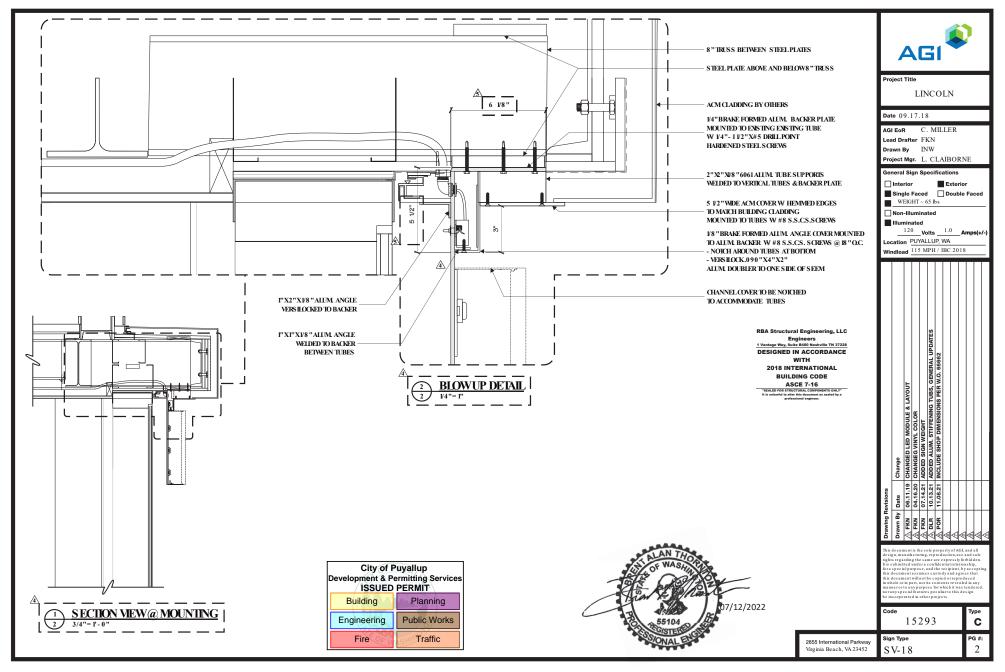
3 DEAD "LETTERS" : Beam Loads

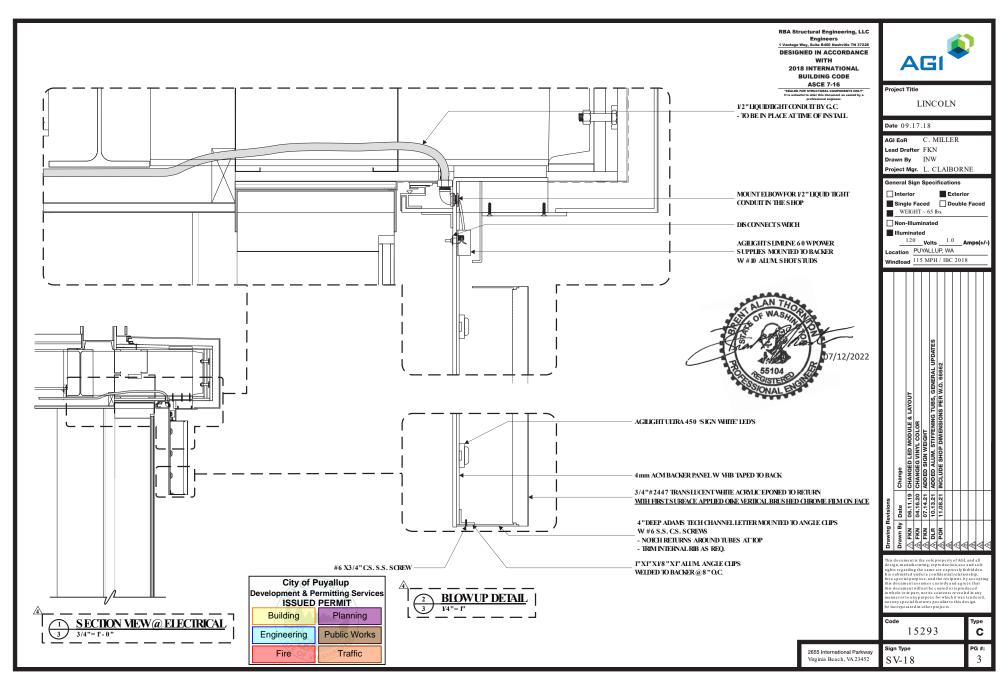
Beam	T	уре	Direction	Fa	Da	Fb	Db	Ecc.
					(ft)			(ft)
1	UNI	lbf/ft	GY	-3.000	-	-	-	-
2	UNI	lbf/ft	GY	-3.000	-	-	-	-
3	UNI	lbf/ft	GY	-3.000	-	-	-	-
4	UNI	lbf/ft	GY	-3.000	-	-	-	-
5	UNI	lbf/ft	GY	-3.000	-	-	-	-
6	UNI	lbf/ft	GY	-3.000	-	-	-	-
7	UNI	lbf/ft	GY	-3.000	-	-	-	-
8	UNI	lbf/ft	GY	-3.000	-	-	-	-
9	UNI	lbf/ft	GY	-3.000	-	-	-	-

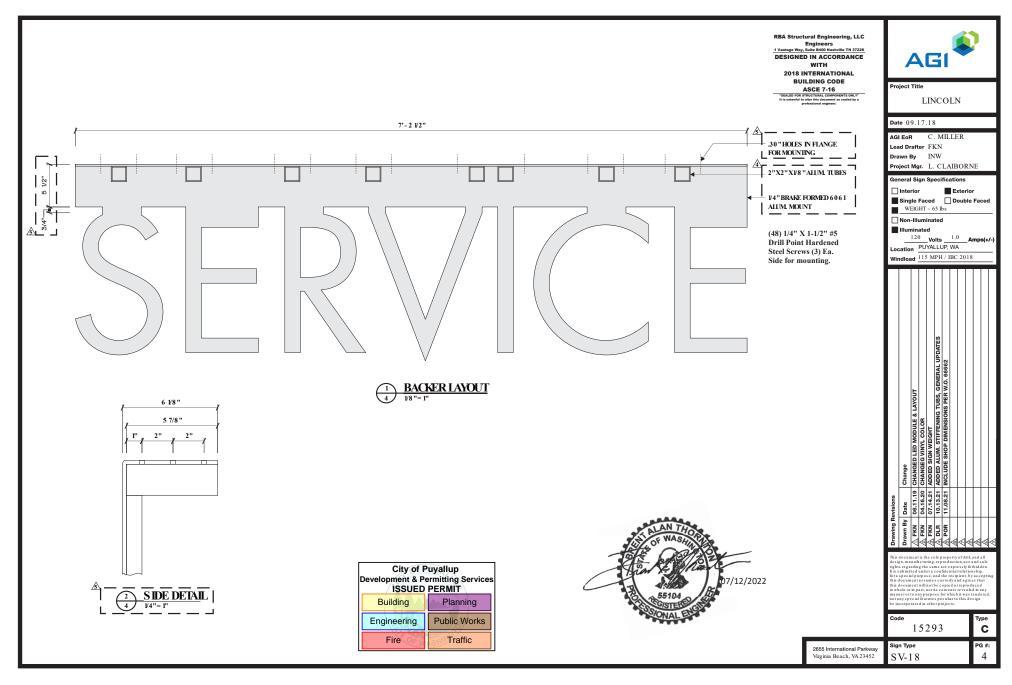


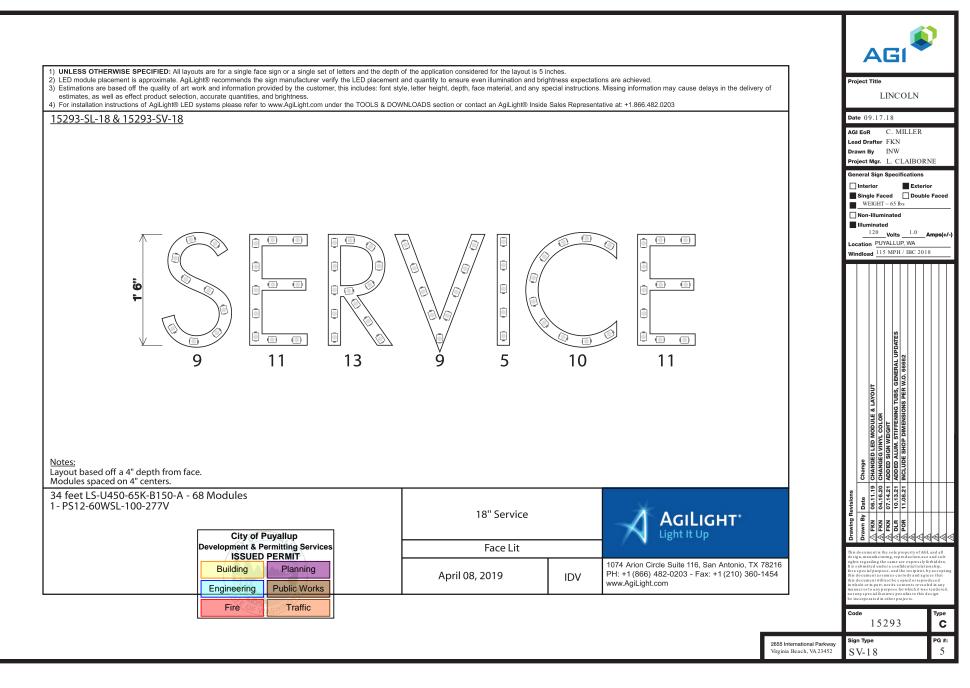
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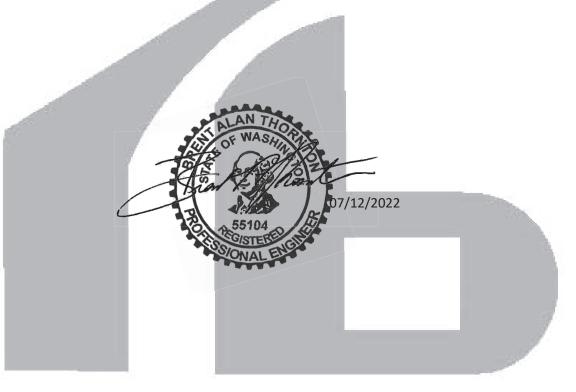


Korum Lincoln 100 River Road Puyallup, WA 98371

RBA Job No. 224639.03

CALCULATIONS FOR: LN-WLL

Designed in accordance with: 2018 International Building Code ASCE 7-16 Wind Velocity = 100 mph Risk Category II



FABRICATOR

Architectural Graphics, Inc. 2655 International Parkway Virginia Beach, Virginia 23452

DESIGN ENGINEER

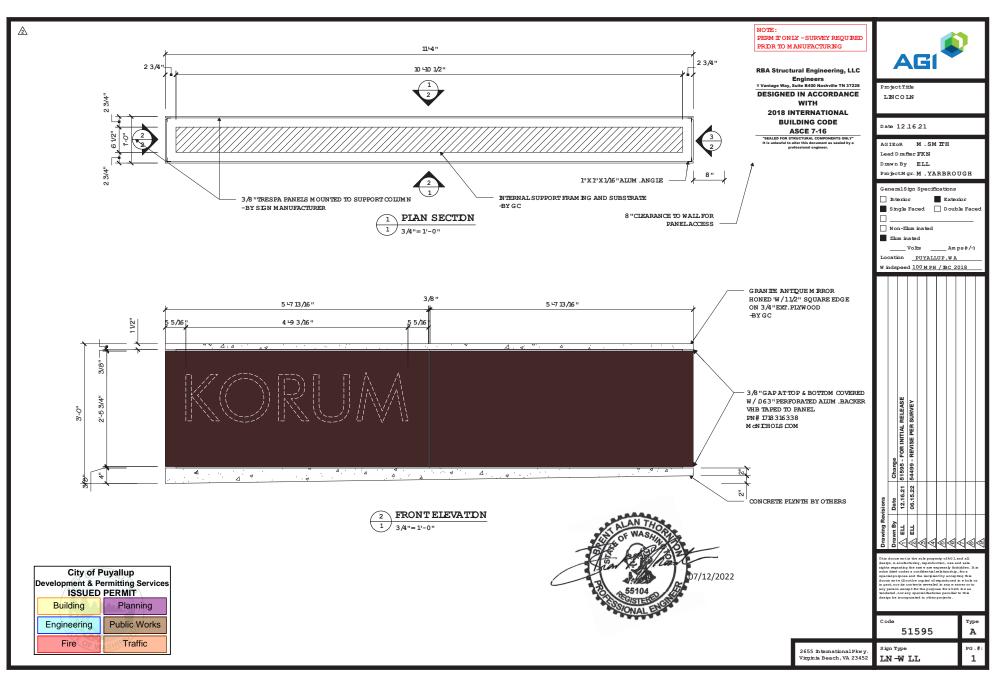
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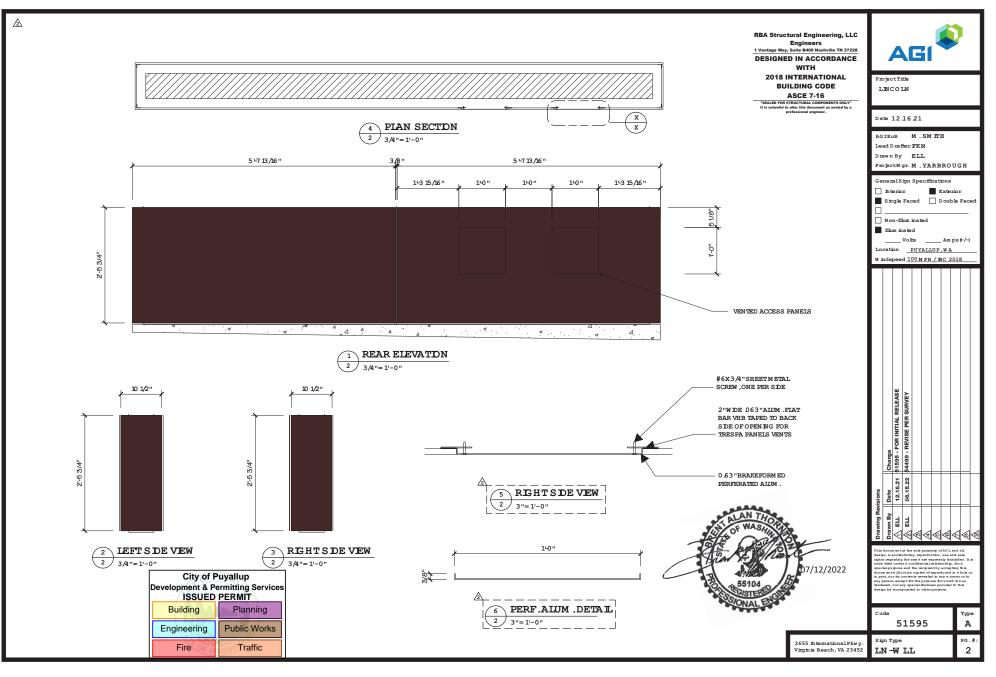


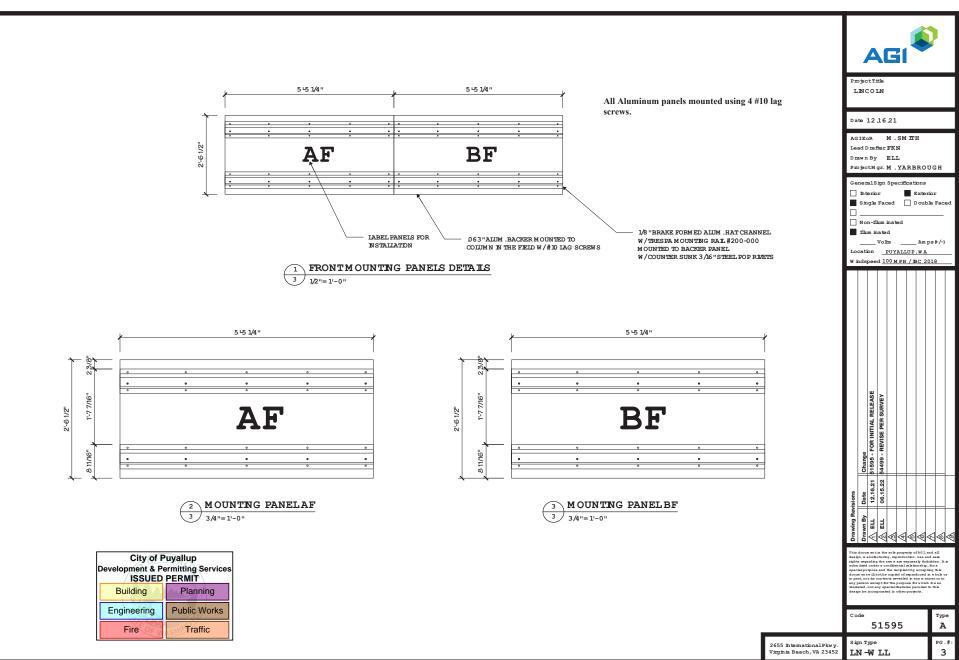
BUBLET Lincoln of Karvan
LN-Wall.
FOR AGI BY QUP
Nox Span for Moment =
$$\left(\frac{4375psi \times 0.25125 in^{2} \times 8 \times 121}{14 \text{ psf}}\right)^{V_{2}}$$

 $= 91.9 in > 2.7.5 in QK$
Max Span for Deflection = $\left(\frac{384 \times 0.0527 in^{4} \times 1305000 \text{ psi}}{5 \times (14\text{ psf}/12\text{ in}) \times 175}\right)^{V_{3}}$
 $= 29.6 in > 27.5 in QK$
Max Span for L Degree Deflection = $\left(\frac{384 \times 0.0527 in^{4} \times 1305000 \text{ psi}}{2 \times 5 \times (14\text{ psf}/12\text{ in}) \times 175}\right)^{V_{3}}$
 $= 29.6 in > 27.5 in QK$
Max Span for L Degree Deflection = $\left(\frac{387 \times 0.0527 in^{4} \times 1305000 \text{ psi} \times \tan(17)}{2 \times 5 \times (14\text{ psf}/12\text{ in}) \times 175}\right)^{V_{3}}$
 $= 34.1 in > 27.5 in QK$
Max Span for Fasteners = $\left(\frac{155.0 \text{ H}}{14 \text{ psf}}\right)^{V_{2}}$
 $= 39.9 in > 27.5 in QK$
Tributary Area of Fastener at widdle = $\frac{27.4385in \times 22.719}{1444 \text{ in}^{2}}$
Tributary Area of Fastener at widdle = $\frac{27.4385in \times 14.875in}{1444 \text{ in}^{2}}$
Tributary Load of Fastener at widdle = $\frac{27.4385in \times 14.875in}{1444 \text{ in}^{2}}$
Tributary Label of V_{2}
 V_{3}
Screw Design load = $275 \text{ H} \times \frac{1}{6} \times 1.1 \times 1.6 = 80.7 \text{ H} > 39.6 \text{ H}$
 $(410 \text{ Lag Screw in Vet Pywed)$
 $N_{Rd} = \frac{NAR}{3n} \times \alpha_{F0} \times \alpha_{S} \times \alpha_{res} = \frac{433.9 \text{ H} (\alpha_{C}(x_{3})(1)(1)}{(18)} = 155.0 \text{ H}$
 $Allowable Fastener Load = 155.4 > 39.6 \text{ H}$
 (18)

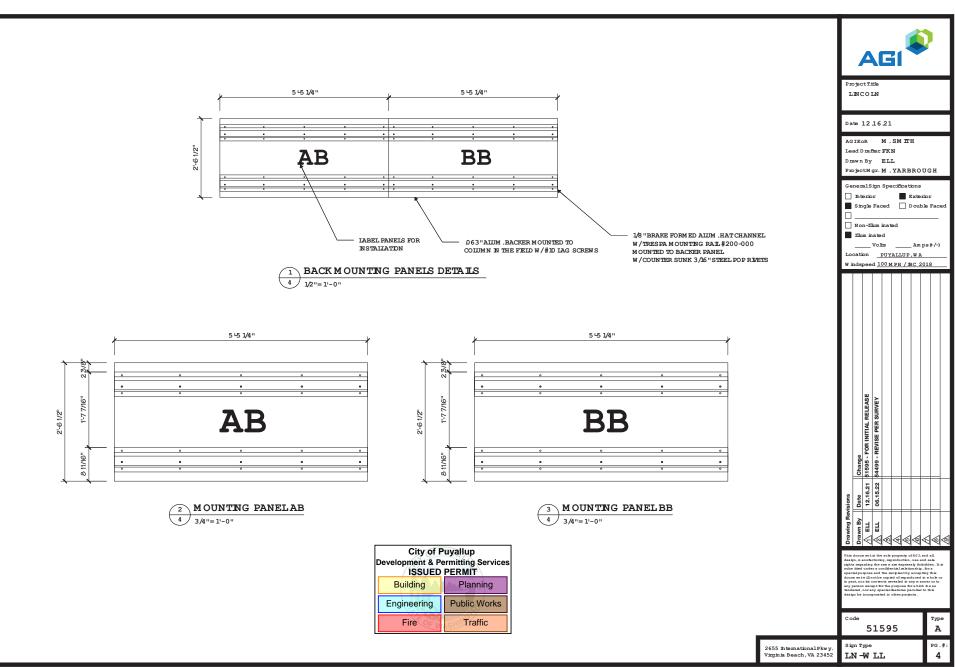
Project Model	Lincoln of Ko	orum	1		AN ASSO	CIATES, INC. IGINEERS	Sheet No Job No.	. 3 22 4639.03		
Ву	BWP					, TN	Date	7/12/2022		
CODES:										
	Wind Loads	per provisio	ons of ASC	E 7-16, Chap	ter 29					
<u>SIGN DII</u>	MENSIONS:									
	Length, B =	11.33	ft.	Height, s =	2.48	ft.	OAH Ab	ove Grade, h =	= 3 ft.	
	Depth, t =	0.875	ft.	A _{sign} =	28.1	ft ²	Ground	Elevation, z _g =	= 0 ft.	
<u>wi</u>	<u>ND LOADS</u> :									
	Natural Fr	equency =	1		<u>RIGID STR</u>	RUCTURE				
	Exposure (Category =	С		Ris	sk Category	= 11			
q _h =	0.00256 * K _z	* K _{7t} * K ₄ *	K _e * V ²	Velocity Pro	essure, AS	CE 7-16, Sec	tion 26.10.2	2		
	K _z =	0.85	e					7-16, Table 2	6.10-1	
	K _{zt} =	1.0				ASCE 7-16, Se				
	K _d =	0.85				actor, ASCE				
	K _e =	1.00				tor, ASCE 7-:				
	v =	100				oh, ASCE 7-1				
q _h =						- ,		-		
F/A =	q _h * G * C _f			Design Win	d Loads, A	SCE 7-16, Se	ection 29.3.	1		
	G =	0.85		•		SCE 7-16, Sec				
	B/s =	4.57		Length of S						
	s/h =	0.83		Depth of Si	• ·	•				
	C _f =	1.48			-	CE 7-16, Figu	re 29.3-1			
F/A =	23.26	b/ft ²		CASE A: res	ultant acts	s normal to s	sign face th	rough the geo	metric center	
						s normal to s l edge equal	-	a distance fro	m the geometr	ic center
				CASE C load	ding applie	S				
LRI	D Loading:				5 TF 5					
	Use wind	pressure =	23.26	lb/ft ²	for 1.0*W	/ from ASCE	7-16, Sectic		City of Puyallup lopment & Permitting S / ISSUED PERMIT Building Plann	
<u>A</u>	D Loading:								ngineering Public V Fire Traf	Works
	Use wind	pressure =	13.95	lb/ft ²	for 0.6*W	/ from ASCE	7-16, Sectio	on 2.4.1		
<u> </u>								PRS	G20220)751







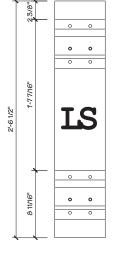
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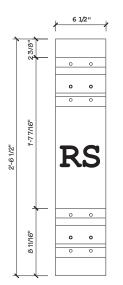






6 1/2"

k



2 MOUNTING PANEL RS 5 1-1/2"= 1'-0"

	AGI					
	Project Title LINCOLN					
	Date 121621					
	AGIEOR M.SMITH Lead Drafter FKN Drawn By ELL Project Mgr. M.YARBROUGH					
	GenezalSim Specifications http:// Extension Single Faced Double Face 					
	Change 5.21 51995 - FOR INITIAL RELEASE 5.22 51499 - REVISE PER SURVEY					
	rawing Revisions rawn By Date ELL 12.10.21 ELL 06.15.32	_				
	The duration of the series of	187				
	^{Соде} ^{Туре} 51595 А					
2655 International Pkwy. Virginia Beach, VA 23452	Sign Type PG.# LN-WLL 5	:				

