1. STRUCTURAL NOTES

1.1. ANY DISCREPANCY FOUND AMONG THE DRAWINGS, SPECIFICATIONS, THESE NOTES, AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT AND THE STRUCTURAL ENGINEER, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S RISK. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION BRACING, FORMWORK AND TEMPORARY CONSTRUCTION SHORING.

1.2. BY THE ACT OF SUBMITTING A BID FOR THE PROPOSED CONTRACT, THE CONTRACTOR WARRANTS THAT:

1.2.1. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE (INCLUDING AGENTS AND SUPPLIERS) HAVE CAREFULLY AND THOROUGHLY REVIEWED THE DRAWINGS AND STRUCTURAL NOTES AND HAVE FOUND THEM COMPLETE AND FREE FROM AMBIGUITIES AND SUFFICIENT FOR THE PURPOSE INTENDED.

1.2.2. THE CONTRACTOR HAS CAREFULLY EXAMINED THE SITE OF THE WORK AND FROM THEIR OWN INVESTIGATIONS, THEY HAVE SATISFIED THEMSELF AS TO THE NATURE AND LOCATION OF THE WORK, AS TO THE CHARACTER, QUALITY, AND QUANTITIES OF MATERIAL AND DIFFICULTIES TO BE ENCOUNTERED, AS TO THE EXTENT OF EQUIPMENT AND OTHER FACILITIES NEEDED FOR THE PERFORMANCE OF THE WORK AND AS TO THE GENERAL AND LOCAL CONDITIONS, AND OTHER ITEMS WHICH MAY IN ANY WAY AFFECT THE WORK OR ITS PERFORMANCE.

1.2.3. THE CONTRACTOR AND ALL WORKERS THEY INTEND TO USE ARE SKILLED AND EXPERIENCED IN THE TYPE OF CONSTRUCTION REPRESENTED BY THE DRAWINGS AND DOCUMENTS BID UPON.

1.2.4. NEITHER THE CONTRACTOR NOR ANY OF THEIR EMPLOYEES, AGENTS, INTENDED SUPPLIERS, OR SUBCONTRACTORS HAVE RELIED UPON ANY VERBAL REPRESENTATIONS ALLEGEDLY AUTHORIZED OR UNAUTHORIZED FROM THE OWNER OR THEIR EMPLOYEES OR AGENTS, INCLUDING THE ARCHITECT OR ENGINEERS, IN ASSEMBLING THE BID FIGURES.

1.2.5. THE REQUIREMENTS CONTAINED WITHIN THIS SECTION SUPERSEDE REQUIREMENTS AND/OR RECOMMENDATIONS CONTAINED IN THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES", AS WELL AS CASE DOCUMENT 962-D "A GUIDELINE ADDRESSING COORDINATION AND COMPLETENESS OF STRUCTURAL CONSTRUCTION DOCUMENTS"

1.2.6. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE ARE AWARE OF AND ACKNOWLEDGE THAT CLOSE COORDINATION AMONG ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS IS REQUIRED.

1.2.7. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE SHALL RECOGNIZE THAT THE PROJECT CONTRACT DOCUMENTS INCLUDE THE ARCHITECTURAL, STRUCTURAL, MECHANICAL AND ELECTRICAL AND OTHER TRADE DRAWINGS AND SPECIFICATIONS

1.2.8. CONTRACTOR AND ALL SUBCONTRACTORS ACKNOWLEDGE THAT CLOSE COORDINATION BETWEEN DISCIPLINES INCLUDED WITHIN THE CONTRACT DOCUMENTS IS NECESSARY. ELEMENTS THAT WILL REQUIRE CLOSE COORDINATION BY THE CONTRACTOR INCLUDE (BUT ARE NOT LIMITED TO):

A. VERIFICATION OF ALL DIMENSIONS INDICATED ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS

B. DETERMINATION OF ALL COLUMN LOCATIONS

C. DETERMINATION OF TOP OF FLOOR, TOP OF STEEL, WALL PLATE AND/OR TOP OF BEAM ELEVATIONS

D. DETERMINATION OF TOP OF FOOTING ELEVATIONS AND FOOTING STEP LOCATIONS

E. MECHANICAL/ELECTRICAL EQUIPMENT LOCATIONS AND WEIGHTS

F. LOCATION AND SIZE OF ALL MECHANICAL/ ELECTRICAL PENETRATIONS THROUGH WALLS AND FLOORS/ ROOFS

G. COORDINATION WITH DESIGNERS/ SUPPLIERS OF PRE-ENGINEERED COMPONENTS (JOISTS, TRUSSES, STAIRS, ETC.)

1.2.9. THE CONTRACTOR ACKNOWLEDGES THAT TEMPORARY SHORING AND/OR BRACING MAY BE REQUIRED TO COMPLETE THE PROJECT. DESIGN AND IMPLEMENTATION OF TEMPORARY SHORING AND/OR BRACING DURING CONSTRUCTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

1.2.10. THE CONTRACTOR AND ALL SUBCONTRACTORS THEY INTEND TO USE SHALL MAKE CONSIDERATION FOR, AND INCLUDE MONIES FOR THE ABOVE IN THE PREPARATION OF THEIR BIDS.

1.2.11. THE CONTRACTOR SHALL NOT SCALE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR LOCATIONS OF ELEMENTS NOTED ABOVE.

1.2.12. ELECTRONIC COPIES OF THE STRUCTURAL DRAWINGS (PDF'S, CAD DRAWINGS OR BIM MODELS) MAY BE PROVIDED TO THE CONTRACTOR FOR THEIR USE. THESE FILES MAY BE PROVIDED AT THE REQUEST OF THE CONTRACTOR FOR THEIR CONVENIENCE ONLY. THE CONTRACTOR AGREES THAT THESE FILES SHALL NOT SUPERSEDE INFORMATION SHOWN ON THE ORIGINAL BID/ CONSTRUCTION DOCUMENTS. THE CONTRACTOR AGREES TO HOLD THE STRUCTURAL ENGINEER HARMLESS FOR ANY ERRORS OR DISCREPANCIES CONTAINED WITHIN THESE ELECTRONIC FILES.

1.2.13. THE BID FIGURE IS BASED SOLELY UPON THE CONSTRUCTION CONTRACT DOCUMENTS AND PROPERLY ISSUED WRITTEN OR VERBAL REPRESENTATIONS.

1.3. EXISTING BUILDING CONDITIONS

1.3.1. STRUCTURAL DESIGN IS BASED ON EXISTING FRAMING CONDITIONS OBSERVED AND FIELD MEASURED AND/OR DESCRIBED IN ORIGINAL CONSTRUCTION DRAWINGS. FIELD OBSERVATIONS DURING DESIGN ARE LIMITED TO AREAS OPEN TO VIEW AND ACCESSIBLE.

1.3.2. GENERAL CONTRACTOR SHALL FIELD VERIFY ALL EXISTING FRAMING CONDITIONS FOR COMPLIANCE WITH THE INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS PRIOR TO DEMOLITION AND CONSTRUCTION. AS-BUILT DEVIATIONS FROM THE INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO EXECUTION OF WORK IN THE AREAS AFFECTED BY THE DISCREPANCY.

1.3.3. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD LOCATE REINFORCING IN EXISTING CONCRETE OR MASONRY CONSTRUCTION PRIOR TO SAW-CUTTING OR CORE-DRILLING. CUTTING OF EXISTING REINFORCING SHALL NOT BE PERMITTED UNLESS DIRECTED BY THE ENGINEER.

1.3.4. WHEN SAW-CUTTING EXISTING CONCRETE OR MASONRY CONSTRUCTION, OVER-CUTTING OF CORNERS SHALL NOT BE PERMITTED.

1.4. CODES

1.4.1. ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2018 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING AUTHORITY.

1.4.2. ALL REFERENCES TO OTHER CODES, STANDARDS AND SPECIFICATIONS, (ACI, ASTM, ETC.), SHALL BE FOR THE EDITION CURRENTLY REFERENCED BY IBC AS AMENDED AND ADOPTED BY THE LOCAL BUILDING AUTHORITY.

1.5. DESIGN CRITERIA

1.5.1. UNIFORM LOADS:

LOCATION	LIVE LOAD	DEAD LOAD
OFFICE (WITH PARTITIONS)	50 PSF + 15 PSF (PARTITIONS)	ACTUAL
STAIRS AND EXITS	100 PSF	ACTUAL
EXISTING SLAB ON GRADE	350 PSF	ACTUAL

1.5.2. WIND LOADS (PER IBC SECTION 1609 AND ASCE 7 CHAPTERS 26 THRU 30):

ULTIMATE DESIGN WIND SPEED (V_{ult}):	98 MPH
RISK CATEGORY	II
WIND EXPOSURE:	B
APPLICABLE INTERNAL PRESSURE COEFFICIENT:	+/-0.18
TOPOGRAPHIC FACTOR (K_{zt})	1.0

1.5.3. SEISMIC LOADS (PER IBC SECTION 1613 AND ASCE 7 CHAPTERS 11 THRU 13):

RISK CATEGORY:	II
SEISMIC IMPORTANCE FACTOR (I_e):	1.0
S_s:	1.258
S_1:	0.433
SITE CLASS:	D – DEFAULT *
S_{DS}:	1.006
S_{D1}:	0.539
SEISMIC DESIGN CATEGORY:	D
DESIGN BASE SHEAR:	$V = C_S \times W$
SEISMIC RESPONSE COEFFICIENT (C_S):	0.201
ANALYSIS PROCEDURE USED:	EQUIVALENT LATERAL FORCE PROCEDURE

* SITE CLASS D IS SELECTED AS THE DEFAULT SITE CLASS PER ASCE 7 SECTION 11.4.3 AND THE VALUE OF F_a = 1.2

SEISMIC FORCE-RESISTING SYSTEM COEFFICIENT, R	RESPONSE MODIFICATION	OVERSTRENGTH FACTOR, Ω_0
A. BEARING WALL SYSTEMS:		
1. SPECIAL REINFORCED CONCRETE SHEAR WALLS	5	2

NOTE: TABULATED OVERSTRENGTH FACTOR HAS BEEN REDUCED IN ACCORDANCE WITH ASCE 7 TABLE 12.2-1 FOOTNOTE B FOR STRUCTURES WITH FLEXIBLE DIAPHRAGMS.

1.6. STATEMENT OF SPECIAL INSPECTIONS

SEE STATEMENT OF SPECIAL INSPECTION AND TESTING SHEET S0.4

1.7. SHOP DRAWINGS

1.7.1. SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR THE FOLLOWING:

A. CONCRETE MIX DESIGN SUBMITTALS

B. REINFORCING STEEL

C. STRUCTURAL AND MISCELLANEOUS STEEL INCLUDING WELD INSERTS AND ANCHORS

D. GLUED-LAMINATED/PARALLAM/LSL MEMBERS

E. WOOD I JOISTS

1.7.2. SHOP DRAWING REVIEW NOTES

A. ENGINEER OF RECORD SHALL REVIEW SHOP DRAWINGS FOR GENERAL CONFORMANCE WITH THE PROJECT CONSTRUCTION DOCUMENTS (PLANS AND SPECIFICATIONS).

B. ENGINEER OF RECORD REVIEW OF SHOP DRAWINGS SHALL NOT RELIEVE THE GENERAL CONTRACTOR OF THEIR RESPONSIBILITY FOR REVIEW OF THE SHOP DRAWINGS FOR COMPLIANCE WITH THE PROJECT REQUIREMENTS.

C. APPROVAL OF THE SHOP DRAWINGS BY THE ENGINEER OF RECORD SHALL NOT BE CONSIDERED AS A GUARANTEE BY THE ENGINEER THAT THE SHOP DRAWINGS COMPLY WITH ALL PROJECT REQUIREMENTS.

D. CONCURRENT SHOP DRAWING REVIEW SHALL ONLY BE PERMITTED IF APPROVED BY THE ARCHITECT/ENGINEER OF RECORD PRIOR TO THE START OF SHOP DRAWING REVIEW.

1.8. MISCELLANEOUS

1.8.1. VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD.

1.8.2. VERIFY SIZE AND LOCATION OF ALL OPENINGS IN THE FLOORS, ROOF AND WALLS WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.

1.8.3. CONSTRUCTION DETAILS NOT SPECIFICALLY SHOWN ON THE DRAWINGS SHALL FOLLOW SIMILAR DETAILS OF SECTIONS OF THIS PROJECT AS APPROVED BY THE ARCHITECT/ ENGINEER.

1.8.4. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR DIMENSIONS AND LOCATIONS OF OPENINGS NOT DIMENSIONED OR SHOWN ON STRUCTURAL PLANS.

1.8.5. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND WEIGHTS OF ALL MECHANICAL AND ELECTRICAL EQUIPMENT INCLUDING HOUSEKEEPING PADS.

1.8.6. FOR PIPES, CONDUITS, DUCTS AND MECHANICAL EQUIPMENT SUPPORTED OR BRACED FROM STRUCTURE: CONFORM TO SHEET METAL AND AIR CONDITIONING CONTRACTORS NATIONAL ASSOCIATION, INC., PUBLICATION "APPENDIX E: SEISMIC RESTRAINT MANUAL GUIDELINES FOR MECHANICAL SYSTEMS." ALL BRACING AND SUPPORTS SHALL BE DESIGNED FOR SEISMIC HAZARD LEVEL (SHL) B. SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA PAMPHLET 13.

1.8.7. THE STRUCTURE HAS BEEN DESIGNED TO RESIST CODE REQUIRED VERTICAL AND LATERAL FORCES AFTER THE CONSTRUCTION OF ALL STRUCTURAL ELEMENTS HAS BEEN COMPLETED. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO JOB SITE SAFETY: ERECTION MEANS, METHODS, AND SEQUENCES; TEMPORARY SHORING, FORMWORK, AND BRACING; USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES.

2. SITE PREPARATION/SOIL REMEDIATION

2.1. SOIL DATA

ALLOWABLE SOIL PRESSURE 2500 PSF. ALLOW 33-1/3% INCREASE FOR LOADS FROM WIND OR SEISMIC ORIGIN. SEE GEOTECHNICAL ENGINEERING REPORT BY TERRA ASSOCIATES DATED SEPTEMBER 27, 2019. SEE GEOTECH REPORT FOR ALL SUBGRADE PREPARATION REQUIREMENTS AS WELL AS CAPILLARY BREAK AND VAPOR BARRIER RECOMMENDATIONS.

2.2. EXCAVATION

EXCAVATE TO DEPTH SHOWN AND TO FIRM UNDISTURBED MATERIAL. OVER-EXCAVATIONS SHALL BE BACKFILLED WITH LEAN CONCRETE (f'_c=500-1200 PSI) OR STRUCTURAL FILL AT THE CONTRACTOR'S EXPENSE. EXERCISE EXTREME CARE DURING EXCAVATION TO AVOID DAMAGE TO BURIED LINES, TANKS, AND OTHER CONCEALED ITEMS. UPON DISCOVERY, DO NOT PROCEED WITH WORK UNTIL RECEIVING WRITTEN INSTRUCTIONS FROM THE ARCHITECT. A COMPETENT REPRESENTATIVE OF THE OWNER SHALL INSPECT ALL FOOTING EXCAVATIONS FOR SUITABILITY OF BEARING SURFACES PRIOR TO PLACEMENT OF REINFORCING STEEL. PROVIDE DRAINAGE AS NECESSARY TO AVOID WATER-SOFTENED SUBGRADE.

2.3. FILL, BACKFILL AND COMPACTION

BACKFILL AGAINST WALLS SHALL NOT BE PLACED UNTIL AFTER THE REMOVAL OF ALL MATERIAL SUBJECT TO ROT OR CORROSION. ALL FILL PLACED AGAINST RETAINING WALLS OR BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL. STRUCTURAL FILL OTHER THAN PEA GRAVEL SHALL BE GRANULAR PLACED IN 12-INCH LOOSE LIFTS AND COMPACTED TO AT LEAST 95% OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698 (STANDARD PROCTOR). PEA GRAVEL FILL SHALL HAVE A MAXIMUM PARTICLE SIZE OF 3/8" DIAMETER.

3. STRUCTURAL CONCRETE

3.1. GENERAL

ALL CONCRETE SHALL BE HARD ROCK CONCRETE MEETING THE REQUIREMENTS OF ACI-301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS." PROPORTIONING OF INGREDIENTS FOR EACH CONCRETE MIX SHALL BE BY METHOD 2 OR THE ALTERNATE PROCEDURE GIVEN IN ACI-301. PLACE CONCRETE PER ACI-304 AND CONFORM TO ACI-604 (306) FOR WINTER CONCRETING AND ACI-605 (305) FOR HOT WEATHER CONCRETING. USE INTERIOR MECHANICAL VIBRATORS WITH 7,000 RPM MINIMUM FREQUENCY. DO NOT OVER-VIBRATE. CONCRETE SHALL BE PLACED MONOLITHICALLY BETWEEN CONSTRUCTION OR CONTROL JOINTS. PROTECT ALL CONCRETE FROM PREMATURE DRYING, EXCESSIVE HOT OR COLD TEMPERATURE FOR SEVEN DAYS AFTER PLACING.

3.2. STRENGTH

TWENTY-EIGHT DAY COMPRESSIVE STRENGTHS (f'_c) SHALL BE AS FOLLOWS WITH EXPOSURE CATEGORY AND CLASS PER ACI TABLE 19.3.1.1 GIVEN IN PARENTHESIS:

SLABS ON GRADE (F0/S0/W0/C0)	4000 PSI
FOOTINGS (F0/S0/W0/C1)	3000 PSI

CONCRETE SUPPLIER TO PROVIDE TEST RECORDS PER SECTION 26.4 OF ACI 318. WHEN NO PRIOR EXPERIENCE OR TRIAL MIXTURE DATA ARE AVAILABLE, THE WATER/CEMENT RATIO FROM THE TABLE BELOW MAY BE USED, BUT ONLY WHEN SPECIAL PERMISSION IS GIVEN BY ENGINEER.

MAXIMUM ABSOLUTE WATER/CEMENT RATIO BY WEIGHT FOR CONCRETE MIXES WITHOUT TEST RECORDS SHALL BE AS FOLLOWS:

SPECIFIED COMPRESSIVE STRENGTH	NON-AIR ENTRAINED CONCRETE	AIR-ENTRAINED CONCRETE
3000 PSI	0.58	0.46
4000 PSI	0.44	0.35

3.3. MATERIALS

3.3.1. CEMENT: ASTM C150, TYPE I OR TYPE II. ENGINEER'S APPROVAL IS NEEDED FOR USE OF TYPE III CEMENT.

3.3.2. COARSE AND FINE AGGREGATE: ASTM C33.

3.3.3. WATER SHALL BE CLEAN AND POTABLE.

3.3.4. FLYASH: ASTM C618 CLASS C (CLASS F MAY BE ALLOWED IF APPROVED BY THE STRUCTURAL ENGINEER)

3.3.5. GROUND GRANULATED BLAST FURNACE SLAG (GGBFS): ASTM C989 GRADE 100 OR 120. GGBFS SHALL NOT BE PERMITTED UNLESS REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER. MIX DESIGNS SUBMITTED INCLUDING GGBFS SHALL INCLUDE SHRINKAGE TEST RESULTS AT 28 DAYS.

3.4. ADMIXTURES

3.4.1. WATER REDUCING ADMIXTURE: ASTM C494. ADMIXTURES SHALL BE USED IN EXACT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

3.4.2. WATER REDUCING ADMIXTURES SHALL BE USED AT ALL HEAVILY CONGESTED AREAS (I.E. CONCRETE BEAMS, COLUMNS AND WALLS WITH REINFORCING SPACING OF 4" OR LESS)

3.4.3. CONCRETE USING ADMIXTURES TO PRODUCE FLOWABLE CONCRETE MAY BE USED SUBJECT TO ENGINEER'S APPROVAL.

3.4.4. AIR ENTRAINMENT: ASTM C260 AND ASTM C494 ENTRAIN 5% PLUS/MINUS 1.5% BY VOLUME IN ALL CONCRETE EXPOSED TO WEATHER.

3.4.5. NO OTHER ADMIXTURES PERMITTED UNLESS APPROVED BY THE ENGINEER.

3.5. FORMWORK AND SHORING

3.5.1. FOLLOW RECOMMENDED PRACTICE FOR CONCRETE FORMWORK (ACI-347).

3.6. REINFORCING STEEL:

3.6.1. DETAIL, FABRICATE, AND PLACE PER ACI-315 AND ACI-318. SUPPORT REINFORCEMENT WITH APPROVED CHAIRS, SPACERS, OR TIES.

3.6.2. DEFORMED BAR REINFORCEMENT: ASTM A615 GR 60

3.6.3. WELDED WIRE FABRIC: ASTM A185 & ASTM A82 F_y=65 KSI

3.6.4. HEADED SHEAR STUD REINFORCEMENT: ASTM A1044

3.6.5. EXCEPT AS NOTED SPECIFICALLY ON THE DRAWINGS, ALL CONCRETE REINFORCEMENT SHALL BE LAP-SPLICED AS FOLLOWS:

#6 AND SMALLER 48 X BAR DIAMETER
#7 AND LARGER 56 X BAR DIAMETER

NO MORE THAN 50% HORIZONTAL OR VERTICAL BARS SHALL BE SPLICED AT ONE LOCATION

3.6.6. EXCEPT AS NOTED SPECIFICALLY ON THE DRAWINGS, PROVIDE CORNER BARS TO MATCH QUANTITY AND DIAMETER OF HORIZONTAL REINFORCEMENT AND LAP WITH HORIZONTAL REINFORCEMENT AS FOLLOWS:

#6 AND SMALLER 48 X BAR DIAMETER
#7 AND LARGER 56 X BAR DIAMETER

THESE CORNER BARS SHALL BE PLACED AT ALL CORNERS AND INTERSECTIONS IN CONCRETE FOOTINGS AND WALLS.

3.6.7. LAP WELDED WIRE FABRIC 12" OR ONE SPACING PLUS 2", WHICHEVER IS MORE.

3.7. CONCRETE COVER ON REINFORCING SHALL BE AS FOLLOWS (UNLESS SHOWN OTHERWISE):

BOTTOM OF FOOTINGS	3"
FORMED EARTH FACE AND SLAB ON GRADE	2"
WALLS, WEATHER FACE	1-1/2"
WALLS, INSIDE FACE	1"
COLUMNS TO TIES	1-1/2"
BEAMS TO STIRRUPS	1-1/2"
BOTTOM OF ELEVATED STRUCTURAL SLAB	3/4"

3.8. CONDUIT AND PIPING EMBEDDED IN CONCRETE

3.8.1. ELECTRICAL CONDUIT SHALL NOT BE PLACED WITHIN A SLAB ON GRADE BUT PLACED BELOW THE SLAB IN THE SUB-BASE.

3.9. GROUT FOR BEARING PLATES

THE NON-SHRINK GROUT SHALL MEET ASTM C1107 GRADE B OR EQUIVALENT (MASTERFLOW 928 BY BASF OR APPROVED EQUIVALENT). GROUT SHALL BE A PRE-PACKAGED HYDRAULIC CEMENT BASED MINERAL AGGREGATE GROUT, MIXED, PLACED AND CURED AS RECOMMENDED BY THE MANUFACTURER. COMPRESSIVE STRENGTH SHALL EXCEED 6000 PSI AT 28 DAYS.

5. METALS

5.1. STRUCTURAL STEEL GENERAL REQUIREMENTS

5.1.1. ALL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO AISC 360-16 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", AISC 341-16 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS" AND AISC 303-16 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" EXCEPT AS AMENDED BY THESE STRUCTURAL NOTES.

5.2. STRUCTURAL STEEL

5.2.1. STEEL W SHAPES SHALL BE ASTM A992 F_y=50 KSI. OTHER SHAPES AND PLATES SHALL BE ASTM A36 F_y=36 KSI. WHERE INDICATED, 50 KSI PLATES SHALL BE ASTM A572 GRADE 50.

5.2.2. STEEL PIPE SECTIONS (PIPE) SHALL BE ASTM A53 TYPE E OR S, GRADE B, F_y=35 KSI.

5.2.3. RECTANGULAR AND ROUND HOLLOW STEEL SECTIONS (HSS) OR TUBE STEEL SECTIONS (TS) SHALL BE ASTM A500, GRADE C, F_y=50 KSI (F_y=46 KSI FOR ROUND SECTIONS)

5.2.4. BOLTS

A. MACHINE BOLTS NOT SPECIFIED AS HIGH STRENGTH SHALL BE ASTM A307 GRADE A.

B. HIGH STRENGTH BOLTS SHALL BE ASTM F3125 GRADE A325 OR GRADE A490 AS INDICATED ON STRUCTURAL DRAWINGS. ALL BOLTS SHALL BE CONSIDERED BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE (CONNECTION TYPE N) UNLESS NOTED OTHERWISE. ALL HIGH STRENGTH BOLTED CONNECTIONS SHALL BE INSTALLED WITH NUTS CONFORMING TO ASTM A563 AND HARDENED WASHERS CONFORMING TO ASTM F436.

C. ALL HIGH STRENGTH BOLTS SHALL BE INSTALLED PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS (LATEST EDITION) BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (WWW.BOLTCOUNCIL.ORG).

5.2.5. STEEL ANCHORAGE ELEMENTS:

A. THREADED RODS SHALL BE ALL-THREAD ASTM A36 (F_y=36 KSI) UNLESS NOTED OTHERWISE.

B. WELDED HEADED STUDS: "NELSON STUDS" SHALL BE BY NELSON STUD WELDING, INC. OR APPROVED EQUIVALENT COMPLYING WITH ASTM A108. STUDS SHALL HAVE A MINIMUM F_u OF 65 KSI.

C. ANCHOR RODS: ANCHOR RODS SHALL BE ASTM F1554, F_y=36 KSI WITH HOOKED, HEADED OR THREADED AND NUTTED ENDS AS INDICATED. AT COLUMN LOCATIONS ANCHOR RODS SHALL BE ASTM F1554, F_y=36 KSI WITH HEADED OR THREADED/NUTTED END. TACK WELD NUT TO ANCHOR ROD UNLESS NOTED OTHERWISE. WHERE NOTED, HIGH STRENGTH ANCHOR RODS SHALL BE ASTM F1554, F_y=105 KSI WITH DOUBLE NUTTED PLATE WASHER.

D. EXPANSION ANCHORS SHALL BE CARBON STEEL AS NOTED IN THE FOLLOWING TABLE. ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND/OR ICC-ES AC193 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. ANCHORS SHALL HAVE A CURRENT CODE REPORT THAT COMPLIES WITH THE CURRENT EDITION OF THE IBC AND SHALL BE RATED FOR USE IN THE SEISMIC DESIGN CATEGORY NOTED IN THE DESIGN CRITERIA SECTION OF THESE NOTES.

EXPANSION ANCHORS IN CONCRETE	CODE REPORT
HILTI KWIK BOLT TZ	ICC ESR-1917
SIMPSON STRONG-BOLT 2	ICC ESR-3037
DEWALT POWER-STUD+ SD2	ICC ESR-2502

E. HEAVY DUTY CONCRETE/MASONRY SCREW ANCHORS SHALL BE USED IN DRY INTERIOR CONDITIONS AND SHALL BE AS NOTED IN THE FOLLOWING TABLE:

HEAVY DUTY CONCRETE/ MASONRY SCREW ANCHORS	CODE REPORT
HILTI KWIK HUS-EZ	ICC ESR-3027(CONC) ICC ESR-3056 (CMU)
SIMPSON TITEN HD	ICC ESR-2713 (CONC) ICC ESR-1056 (CMU)
DEWALT SCREW BOLT+	ICC ESR-3889 (CONC) ICC ESR-4042 (CMU)

F. ADHESIVE ANCHORS SHALL BE THREADED ANCHOR RODS OR REBAR DOWELS USING AN INJECTABLE ADHESIVE AS NOTED IN THE FOLLOWING TABLE. ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND/OR ICC-ES AC-308 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. ANCHORS SHALL HAVE A CURRENT CODE REPORT THAT COMPLIES WITH THE CURRENT EDITION OF THE IBC AND SHALL BE RATED FOR USE IN THE SEISMIC DESIGN CATEGORY NOTED IN THE DESIGN CRITERIA SECTION OF THESE NOTES.

ADHESIVE ANCHORS IN CONCRETE (1) (2)	CODE REPORT
HILTI HIT HY-200 SAFE SET	ICC ESR-3187
SIMPSON AT-XP (3)	IAPMO ER-263
DEWALT AC200+ DUST-X	ICC ESR-4027

(1) ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI, OR AN APPROVED ALTERNATE WHEN SUBMITTED AND APPROVED BY THE ENGINEER. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.

(2) ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS.

(3) SIMPSON SET-XP MAY BE USED WHERE BASE MATERIAL TEMPERATURE IS ABOVE 50 DEGREES FAHRENHEIT OR FOR EMBEDMENT GREATER THAN 12-INCHES FOR LONGER GEL TIME. SEE ICC ESR-2508 (CONC) AND IAPMO ER-265 (MASONRY).

G. POWDER ACTUATED FASTENERS: PDF'S OR PAF'S SHALL BE A MINIMUM 0.157" DIA KNURLED SHANK FASTENER AS NOTED IN THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. FASTENERS DRIVEN INTO STEEL SHALL BE DRIVEN SO THAT THE POINT OF THE FASTENER COMPLETELY PENETRATES THE STEEL BASE MATERIAL. AT TOPPING SLABS, PT SLABS OR SLABS WITH RADIANT HEAT TUBES EMBEDDED WITHIN THE SLAB, LIMIT THE PDF PENETRATION TO 3/4" MAXIMUM AND COORDINATE WITH TENDON/TUBE PLACEMENT AND COVER.

POWDER ACTUATED FASTENERS	CODE REPORT
HILTI X-U	ICC ESR-2269
SIMPSON PDPA	ICC ESR-2138
DEWALT CSI PIN	ICC ESR-2024

H. CONCRETE/MASONRY SCREWS SHALL BE AS NOTED IN THE FOLLOWING TABLE:

CONCRETE/MASONRY SCREWS	CODE REPORT
HILTI KWIK CON II+	-
SIMPSON TITEN	-
DEWALT TAPPER+	ICC ESR-3068 (CONC) ICC ESR-3196 (MAS)

5.2.6. METAL PROTECTION: ALL STEEL EXPOSED TO WEATHER, MOISTURE, SOIL, OR AS NOTED SHALL BE GALVANIZED PER ASTM A123 OR A153 AS APPLICABLE. ALL OTHER STEEL SURFACES SHALL BE SHOP PRIMED AFTER FABRICATION.

REPAIR ALL DAMAGED AREAS OF GALVANIZED PARTS SUCH AS FIELD WELDS, ETC. APPLY REPAIR COATING THICKNESS GREATER THAN OR EQUAL TO ORIGINAL ZINC COATING THICKNESS.

NOTES CONTINUE ON SHEET S0.02

Approval of submitted plans is not an approval of omissions or oversights by this office or noncompliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PRCTI20220873

DATE: April 19, 2022 FILENAME: Q:\2022\2220189\20_STR\CAD\2220189S-00.dwg

NELSON

Nelco Architecture, Inc.

1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA · SEATTLE · SPOKANE · TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

THE APPROVED CONSTRUCTION PLANS, DOCUMENTS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

FULL SIZED LEDGIBLE COLOR PLANS ARE REQUIRED TO BE PROVIDED BY THE PERMITEE ON SITE FOR INSPECTION

SEAL:

DANIEL L. BOOTH
STATE OF WASHINGTON
28466
STRUCTURAL ENGINEER
PROFESSIONAL ENGINEER

CITY STAMP:

City of Puyallup
Building
ACCEPTED
JMontgomery
08/16/2022
9:58:42 AM

STRUCTURAL NOTES

Proj. No: 2220189.20 Reviewed By: DLB

S0.1

5.3. WELDING

5.3.1. ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE," AWS D1.1, AWS D1.4 AND AWS D1.8 AS APPROPRIATE.

5.3.2. ALL WELDING SHALL BE BY CERTIFIED WELDERS; USE 70 KSI LOW HYDROGEN FILLER METAL AND SHALL BE PROTECTED PER AWS D1.1 UNTIL USE. FOR ALL FULL PENETRATION WELDS, FILLER METAL SHALL BE NOTCH TOUGH TO MEET CHARPY V-NOTCH OF 20 FOOT-POUND AT -20°F.

5.3.3. NO WELDING OF REINFORCING STEEL SHALL BE ALLOWED.

5.3.4. ALL FULL PENETRATION FIELD AND SHOP WELDS SHALL BE FULL TIME INSPECTED AND TESTED BY NON-DESTRUCTIVE PROCEDURES. RESULTS OF TESTS SHALL BE SUBMITTED FOR REVIEW BY THE STRUCTURAL ENGINEER.

5.4. WELDING PROCEDURE SPECIFICATION (WPS)

5.4.1. FOR ALL WELDING OF REINFORCING STEEL, NON-PREQUALIFIED WELDS AND ALL WELDING OF COMPONENTS WHICH ARE PART OF THE SEISMIC FORCE RESISTING SYSTEM, CONTRACTOR SHALL SUBMIT A WELDING PROCEDURE SPECIFICATION (WPS) TO ENGINEER FOR APPROVAL. PRIOR TO WELDING, EACH WPS SHALL INCLUDE ALL NECESSARY INFORMATION REQUIRED BY AWS D1.1, AWS D1.4 AND AWS D1.8 AND AS FOLLOWS:

A. APPLICABLE BASE METAL TYPES AND THICKNESSES.

B. SKETCH OF JOINT INDICATING APPLICABLE DIMENSIONS. INDIVIDUAL PASSES SHALL BE IDENTIFIED AND NUMBERED TO IDENTIFY THE SEQUENCE. THE SKETCH SHALL IDENTIFY THE MAXIMUM THICKNESS AND BEAD WIDTH. IN NO CASE SHALL THE LAYER THICKNESS EXCEED 1/4" NOR THE BEAD WIDTH EXCEED 5/8."

C. PREHEAT REQUIREMENTS.

D. ELECTRICAL CHARACTERISTICS (I.E., CURRENT, VOLTAGE, TRAVEL SPEED, ETC.).

E. ELECTRODE REQUIREMENTS SHALL MEET THE REQUIREMENTS OF AWS A5.1, AWS A5.5, AWS A5.17, AWS A5.23, AWS A5.18, AWS A5.20, AWS A5.28, AND AWS A5.29, AS APPLICABLE FOR WELDING METHOD USED.

6. CARPENTRY

DIMENSION LUMBER SHALL BE DF No.2 SAWN LUMBER BEAMS, HEADERS AND COLUMNS SHALL BE DF No.1 OR AS SHOWN ON THE DRAWINGS. ALL 2" NOMINAL LUMBER SHALL BE KILN DRIED (KD). EACH PIECE OF LUMBER SHALL BEAR STAMP OF WEST COAST LUMBER INSPECTION BUREAU (WCLIB) AND/OR WESTERN WOOD PRODUCTS ASSOCIATION (WWPA) SHOWING GRADE MARK.

6.1. PRESSURE-PRESERVATIVE TREATMENT IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1, LATEST EDITION TO THE USE CATEGORY AS FOLLOWS:

6.1.1. TREAT ALL WOOD IN CONTACT WITH CONCRETE, MORTAR, GROUT, MASONRY AND WITHIN 12" OF EARTH TO THE REQUIREMENTS OF USE CATEGORY UC2 (INTERIOR/DAMP).

6.1.2. WHERE POSSIBLE, PRECUT MATERIAL PRIOR TO TREATMENT. ALL FIELD CUTS AND DRILLED HOLES SHALL BE FIELD TREATED IN ACCORDANCE WITH AWPA M-4.

6.2. CARPENTRY HARDWARE

6.2.1. MACHINE BOLTS SHALL BE ASTM A307.

6.2.2. PROVIDE MALLEABLE IRON WASHERS (MIW) OR HEAVY PLATE CUT WASHERS WHERE BOLT HEADS, NUTS OR LAG SCREWS BEAR ON WOOD.

6.2.3. NAILS SHALL BE COMMON, AMERICAN OR CANADIAN MANUFACTURER ONLY WITH MIN. DIAMETERS AS FOLLOWS:

NAIL SIZE	MINIMUM NAIL SHANK DIAMETER	MINIMUM NAIL LENGTH
8d	0.131"	2 1/2"
10d	0.148"	3"
12d	0.148"	3 1/4"
16d SINKER	0.148"	3 1/4"
16d	0.162"	3 1/2"
20d	0.192"	4"

6.2.4. LAG SCREWS SHALL MEET THE REQUIREMENTS OF ANSI/ASME B18.2.1. WOOD SCREWS SHALL MEET THE REQUIREMENTS OF ANSI/ASME B18.6.1.

6.2.5. ANCHORS AND CONNECTIONS SHALL BE SIMPSON, USP, OR ICC (INTERNATIONAL CODE COUNCIL) APPROVED. ALL FASTENERS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS UNLESS OTHERWISE SHOWN. SUBSTITUTED CONNECTIONS SHALL HAVE A TABULATED CAPACITY EQUAL TO OR GREATER THAN THE SPECIFIED CONNECTOR.

6.2.6. CORROSION RESISTANT HARDWARE AND FASTENERS:

A. FASTENERS AND HARDWARE EXPOSED TO WEATHER OR IN UNHEATED PORTIONS OF THE BUILDING SHALL BE MECHANICALLY OR HOT DIPPED GALVANIZED PER ASTM B695 - CLASS 55 OR ASTM A153 - CLASS D. HARDWARE IN CONTACT WITH TREATED WOOD SHALL CONFORM TO A MINIMUM GALVANIZED COATING OF G185 OR AS NOTED BELOW.

B. IF PRESERVATIVE TREATMENT USED IS ACZA (AMMONIACAL COPPER ZINC ARSENATE), IF THE CHEMICAL RETENTION LEVEL IS AWPA USE CATEGORY UC4A OR GREATER, OR IF THE PRESERVATIVE TREATMENT USED IS NOT KNOWN, HARDWARE SHALL BE TYPE 316L STAINLESS STEEL. FASTENERS SHALL BE TYPE 304 OR 305 STAINLESS STEEL.

C. IN THE EVENT OF A CONFLICT BETWEEN THE HARDWARE MANUFACTURER'S RECOMMENDATIONS FOR SELECTING CORROSION-RESISTANT HARDWARE AND FASTENERS, THESE NOTES, AND THE SPECIFICATIONS, THE MOST STRINGENT REQUIREMENT SHALL BE USED UNLESS APPROVED BY THE ENGINEER.

6.3. MINIMUM NAILING: PER IBC TABLE 2304.10.1 FASTENING SCHEDULE.

6.4. COORDINATION AT HOLES IN WOOD STUD WALLS

6.4.1. PIPES IN INTERIOR NONBEARING WALLS: STUD PARTITIONS CONTAINING PIPES SHALL BE FRAMED, AND THE JOISTS SHALL BE SPACED, SO AS TO GIVE PROPER CLEARANCE FOR THE PIPING. WHERE A PARTITION CONTAINING PIPING RUNS PARALLEL TO THE JOISTS, THE JOISTS SHALL BE DOUBLED AND SPACED SO AS TO PERMIT THE PASSAGE OF SUCH PIPING AND SHALL BE BRIDGED. WHERE PIPES ARE PLACED IN, OR PARTIALLY IN, A PARTITION NECESSITATING THE CUTTING OF THE SOLES OR PLATES, A SIMPSON RPS STRAP SHALL BE FASTENED TO EACH PLATE ACROSS AND TO EACH SIDE OF THE OPENING WITH NOT LESS THAN SIX 16d NAILS.

6.4.2. CUTTING AND NOTCHING SAWN LUMBER: IN EXTERIOR WALLS AND BEARING PARTITIONS, ANY WOOD STUD IS PERMITTED TO BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 15 PERCENT OF ITS WIDTH. CUTTING OR NOTCHING OF STUDS TO A DEPTH NOT GREATER THAN 40 PERCENT OF THE WIDTH OF THE STUD IS PERMITTED IN NONBEARING PARTITIONS SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE PARTITION.

6.4.3. CUTTING AND NOTCHING ENGINEERED LUMBER: CUTTING AND NOTCHING SHALL NOT BE PERMITTED IN ENGINEERED LUMBER (LSL) STUDS WITHOUT APPROVAL FROM THE ENGINEER OF RECORD.

6.4.4. BORED HOLES IN SAWN LUMBER: A HOLE NOT GREATER IN DIAMETER THAN 33 PERCENT OF THE STUD WIDTH IS PERMITTED TO BE BORED IN ANY WOOD STUD WITHOUT ENGINEERING VERIFICATION. BORED HOLES NOT GREATER THAN 60 PERCENT OF THE WIDTH OF THE STUD ARE PERMITTED IN NONBEARING PARTITIONS, PROVIDED NOT MORE THAN ANY TWO ADJACENT STUDS ARE SO BORED. IN NO CASE SHALL THE EDGE OF THE BORED HOLE BE NEARER THAN 5/8-INCH FROM THE EDGE OF THE STUD. BORED HOLES SHALL NOT BE LOCATED AT THE SAME SECTION OF STUD AS A NOTCH OR CUT AND SHALL NOT BE LOCATED WITHIN 8-INCHES OF THE END OF THE STUD.

6.4.5. BORED HOLES IN ENGINEERED LUMBER: BORED HOLES SHALL NOT BE PERMITTED IN ENGINEERED LUMBER (LSL) STUDS WITHOUT APPROVAL FROM THE ENGINEER OF RECORD.

6.5. SHEATHING (PLYWOOD/ORIENTED STRAND BOARD)

EACH SHEET SHALL BEAR THE TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION; ALL SHEATHING SHALL CONFORM TO STANDARD PS 2 OR PRP-108. THICKNESS, NUMBER OF PLIES AND LAY-UP AS SHOWN. ALL PLYWOOD SHALL BE C-D INTERIOR WITH EXTERIOR GLUE OR AS NOTED ON THE DRAWINGS AND SHALL BE GROUP I OR II SPECIES. EXCEPT AS OTHERWISE SHOWN, PROVIDE THE FOLLOWING MINIMUM NAILING: PANEL EDGES 10d AT 6" ON CENTER, INTERMEDIATE SUPPORT 10d AT 12" ON CENTER. GAP SHEETS 1/8" FOR 4'x8' SHEETS AND 1/4" FOR 8'x8' AND LARGER SHEETS. THE MOISTURE CONTENT SHALL NOT BE GREATER THAN 15% AT TIME OF ROOFING.

6.6. GLUED-LAMINATED TIMBER

MATERIALS, MANUFACTURE AND QUALITY CONTROL PER ANSI/AITC A190 "STRUCTURAL GLUED LAMINATED TIMBER." CAMBER 1-1/2 TIMES DEAD LOAD DEFLECTION WHERE NOT INDICATED ON DRAWINGS. ALL BEAM MEMBERS SHALL BE COMBINATION 24F-V4 FOR SIMPLE SPANS AND 24F-V8 FOR CONTINUOUS OR CANTILEVERED SPANS AND HAVE EXTERIOR GLUE. ALL COLUMN MEMBERS SHALL BE 24F-V8 UNLESS NOTED OTHERWISE. ALL MEMBERS EXPOSED TO VIEW SHALL BE ARCHITECTURAL APPEARANCE GRADE UNLESS NOTED OTHERWISE. ALL MEMBERS CONCEALED FROM VIEW SHALL BE INDUSTRIAL APPEARANCE UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

6.6.1. ADHESIVES SHALL MEET THE REQUIREMENTS FOR WET CONDITIONS OF SERVICE.

6.6.2. MEMBERS SHALL BE MARKED INDICATING CONFORMANCE WITH ANSI/AITC 190.1. IN ADDITION, A CERTIFICATE OF SUCH CONFORMANCE SHALL BE PROVIDED TO THE BUYER.

6.7. MANUFACTURED STRUCTURAL WOOD MEMBERS

6.7.1. PARALLAM PSL POSTS AND COLUMNS SHALL BE PARALLAM 1.8E AS MANUFACTURED BY WEYERHAEUSER OR APPROVED EQUIVALENT. PARALLAM PSL BEAMS SHALL BE PARALLAM 2.0E AS MANUFACTURED BY WEYERHAEUSER OR APPROVED EQUIVALENT.

6.7.2. TIMBERSTRAND LSL STUDS SHALL BE TIMBERSTRAND 1.5E AS MANUFACTURED BY WEYERHAEUSER OR APPROVED EQUIVALENT.

6.7.3. MICROLLAM LVL MEMBERS SHALL BE MICROLLAM 1.9E AS MANUFACTURED BY WEYERHAEUSER OR APPROVED EQUIVALENT.

6.8. WOOD I-JOISTS

6.8.1. WOOD I-JOISTS SHALL BE AS MANUFACTURED BY TRUS JOIST BY WEYERHAEUSER OR APPROVED EQUAL.

6.8.2. GEOMETRY AND SPACING SHALL BE AS SHOWN. THE MANUFACTURER SHALL PROVIDE ADDITIONAL FRAMING MEMBERS AS SHOWN OR AS NECESSARY TO SUPPORT MECHANICAL EQUIPMENT, WALLS AND/OR PARTITIONS, SNOW DRIFT LOADS, ETC.

6.8.3. WHERE NOTED, PRECUT BLOCKING, BRIDGING, BRACING AND/OR FILLER PIECES SHALL BE FURNISHED BY THE MANUFACTURER. WHERE APPLICABLE, WIND UPLIFT BRACING SHALL BE PROVIDED BY MANUFACTURER.

6.8.4. PROPRIETARY COMPONENTS SHALL HAVE ICC (INTERNATIONAL CODE COUNCIL) APPROVAL.

6.8.5. SHOP DRAWINGS SHALL INDICATE ALL REQUIRED PERMANENT BRACING (INCLUDING BOTTOM CHORD AND WEB BRACING SYSTEM TO RESIST WIND UPLIFT FORCES).

6.8.6. UNLESS NOTED OTHERWISE, THE JOIST MANUFACTURER SHALL SPECIFY AND FURNISH CONNECTION HARDWARE NECESSARY FOR INSTALLATION OF THEIR SYSTEM.

6.8.7. OPEN WEB JOISTS AND I-JOISTS THAT SPAN GREATER THAN 25-FEET SHALL BE CAMBERED.

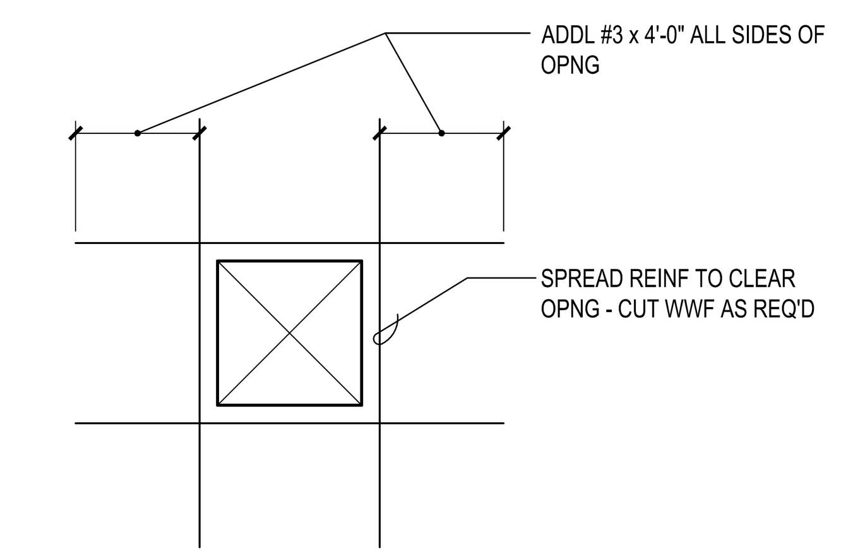
6.8.8. DELIVERED COMPONENTS SHALL BE ACCOMPANIED BY FABRICATOR'S CERTIFICATE OF CONFORMANCE TO THE REFERENCED STANDARDS.

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

KEY TO ABBREVIATIONS

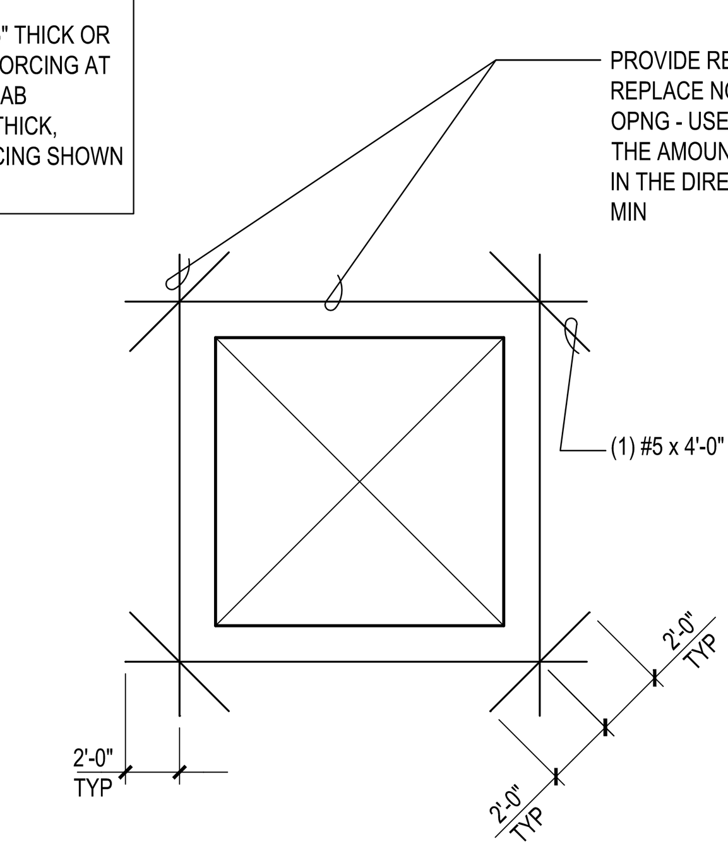
AB	ANCHOR BOLT	L	ANGLE
ABV	ABOVE	LLH	LONG LEG HORIZONTAL
ADDL	ADDITIONAL	LLV	LONG LEG VERTICAL
ADJ	ADJACENT	LOC	LOCATION
AFF	ABOVE FINISH FLOOR	LONGIT	LONGITUDINAL
ARCH	ARCHITECTURAL, ARCHITECT	MAX	MAXIMUM
ASD	ALLOWABLE STRESS DESIGN	MB	MACHINE BOLT
BEL	BELOW	MECH	MECHANICAL
BLKG	BLOCKING	MFR	MANUFACTURER
BM	BEAM	MIN	MINIMUM
BNDY	BOUNDARY	MIW	MALLEABLE IRON WASHER
BOT	BOTTOM	NS	NEAR SIDE
BRG	BEARING	NTS	NOT TO SCALE
BS	BOTH SIDES	NWT	NORMAL WEIGHT
BTWN	BETWEEN	O/	OVER
BU	BUILT UP	OC	ON CENTER
CIP	CAST IN PLACE	O.F.	OUTSIDE FACE
CJ	CONSTRUCTION/CONTROL JOINT	OPP	OPPOSITE HAND
CL	CENTERLINE	OPNG	OPENING
CLG	CEILING	OSB	ORIENTED STRAND BOARD
CLR	CLEAR	PC	PRE-CAST
CMU	CONCRETE MASONRY UNIT	PDF	POWER DRIVEN FASTENERS, PAF
COL	COLUMN	PAF	POWER ACTUATED FASTENERS, PDF
CONC	CONCRETE	PERP	PERPENDICULAR
CONN	CONNECT, CONNECTION	PL	PLATE
CONT	CONTINUOUS	PLF	POUNDS PER LINEAR FOOT
COORD	COORDINATE	PNL	PANEL
CSK	COUNTERSINK	PRE-ENGR	PRE-ENGINEERED
CTR	CENTER	PROV	PROVIDE
CVR	COVER	PT	POST TENSIONED
DEG	DEGREE	PW	PLYWOOD
DIA	DIAMETER	REF	REFERENCE
DBL	DOUBLE	REINF	REINFORCE, REINFORCEMENT
EA	EACH	REQ'D	REQUIRED
EF	EACH FACE	RF	ROOF
ELEV	ELEVATION, ELEVATOR	SCHED	SCHEDULE
EMB	EMBEDMENT	SFRS	SEISMIC FORCE RESISTING SYSTEM
ENGR	ENGINEER	SHTG	SHEATHING
EQ	EQUAL/EQUIVALENT	SIM	SIMILAR
EQUIV	EQUIVALENT	SIMP	SIMPSON STRONG-TIE
ES	EACH SIDE	SOG	SLAB ON GRADE
EW	EACH WAY	SPCG	SPACING
(E)	EXISTING	SQ	SQUARE
EXP	EXPANSION	STD	STANDARD
EXT	EXTERIOR	STIFF	STIFFENER
FDN	FOUNDATION	SW	SHEARWALL
FF	FINISH FLOOR	T&G	TONGUE AND GROOVE
FFE	FINISH FLOOR ELEVATION	THK	THICK
FOC	FACE OF CONCRETE	THRD	THREADED
FOM	FACE OF MASONRY	T.O.	TOP OF
FOS	FACE OF STUD	TOC	TOP OF CONCRETE
FS	FAR SIDE	TOF	TOP OF FOOTING
FTG	FOOTING	TOPL	TOP OF PLATE
GA	GAGE	TOS	TOP OF STEEL
GALV	GALVANIZED	T.O.W.	TOP OF WALL
GC	GENERAL CONTRACTOR	TRANSV	TRANSVERSE
GL	GLUE LAMINATED	TRTD	TREATED
GWB	GYPSUM WALL BOARD	TYP	TYPICAL
HGR	HANGER	UNO	UNLESS NOTED OTHERWISE
HORIZ	HORIZONTAL	VFY	VERIFY
HSS	HOLLOW STEEL SECTION	VERT	VERTICAL
HT	HEIGHT	W/	WITH
I.F.	INSIDE FACE	W/O	WITHOUT
INT	INTERIOR	WF	WIDE FLANGE
JNT	JOINT	WHS	WELDED HEADED STUD
JST	JOIST	WTS	WELDED THREADED STUD
K, KIPS	KIPS=1000 LBS	WWF	WELDED WIRE FABRIC



TYPICAL REINFORCING AT OPENINGS
LESS THAN 12" IN CONC WALL OR SLAB

NOTE:

AT WALL OR SLAB 6" THICK OR LESS, PLACE REINFORCING AT CL - AT WALL OR SLAB GREATER THAN 6" THICK, PROVIDE REINFORCING SHOWN EACH FACE



TYPICAL REINFORCING AT OPENINGS
GREATER THAN 12" IN CONC WALL OR SLAB

TYPICAL 1

NTS

NELSON

Nelco Architecture, Inc.

1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA · SEATTLE · SPOKANE · TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

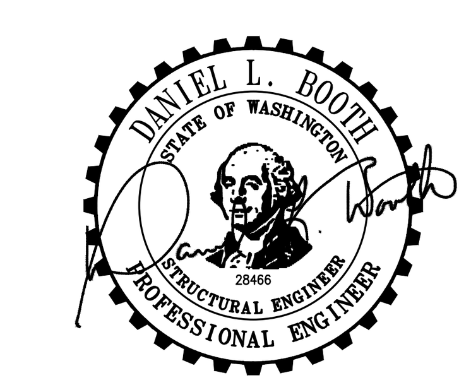
PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:

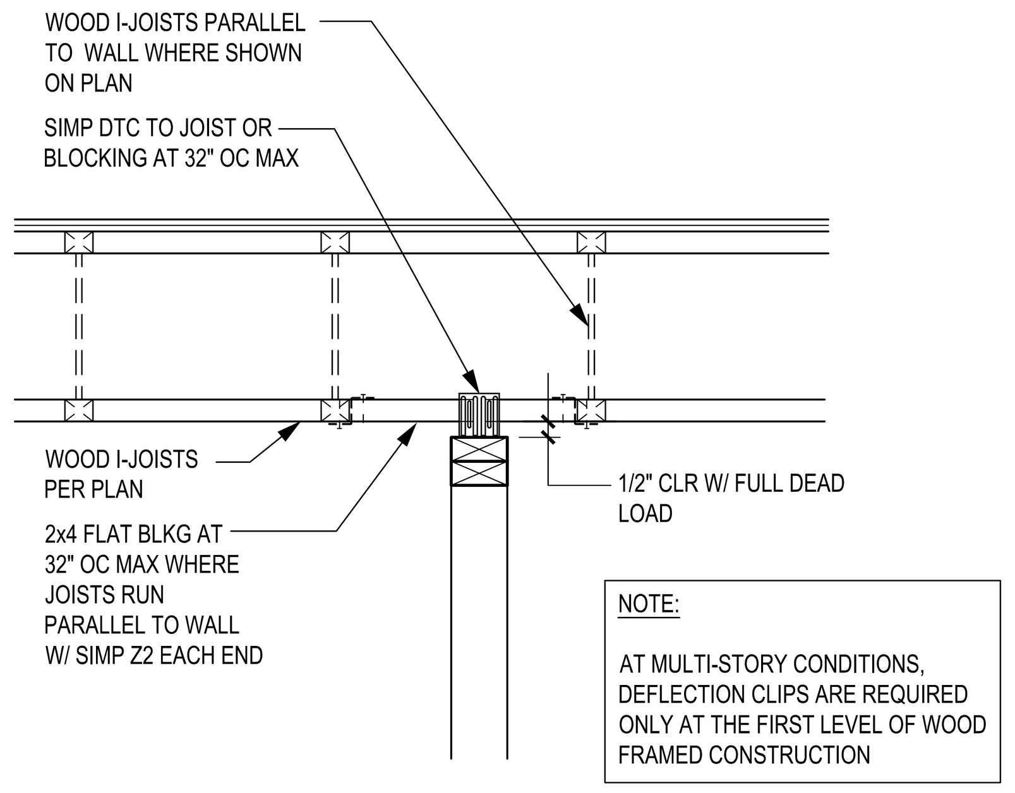


CITY STAMP:

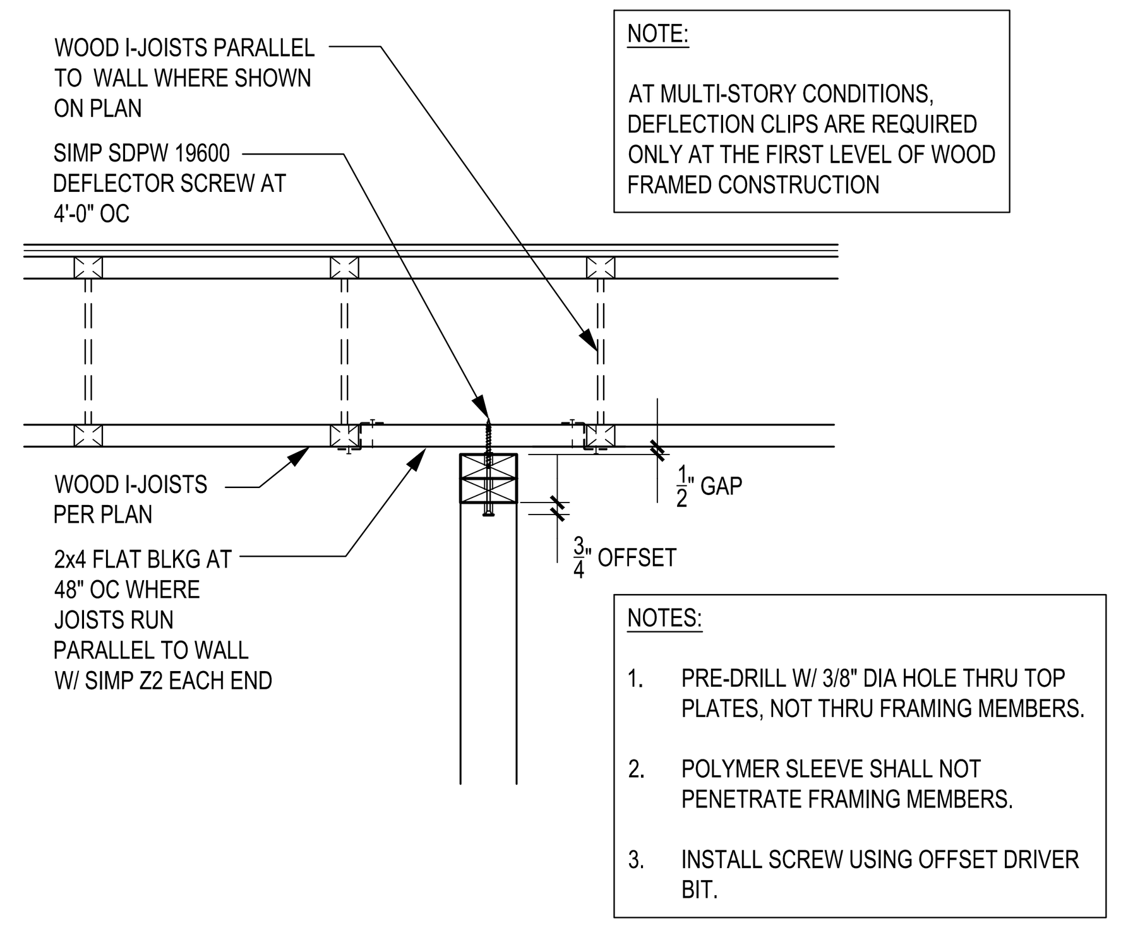
STRUCTURAL NOTES

Proj. No: 2220189.20 Reviewed By: DLB

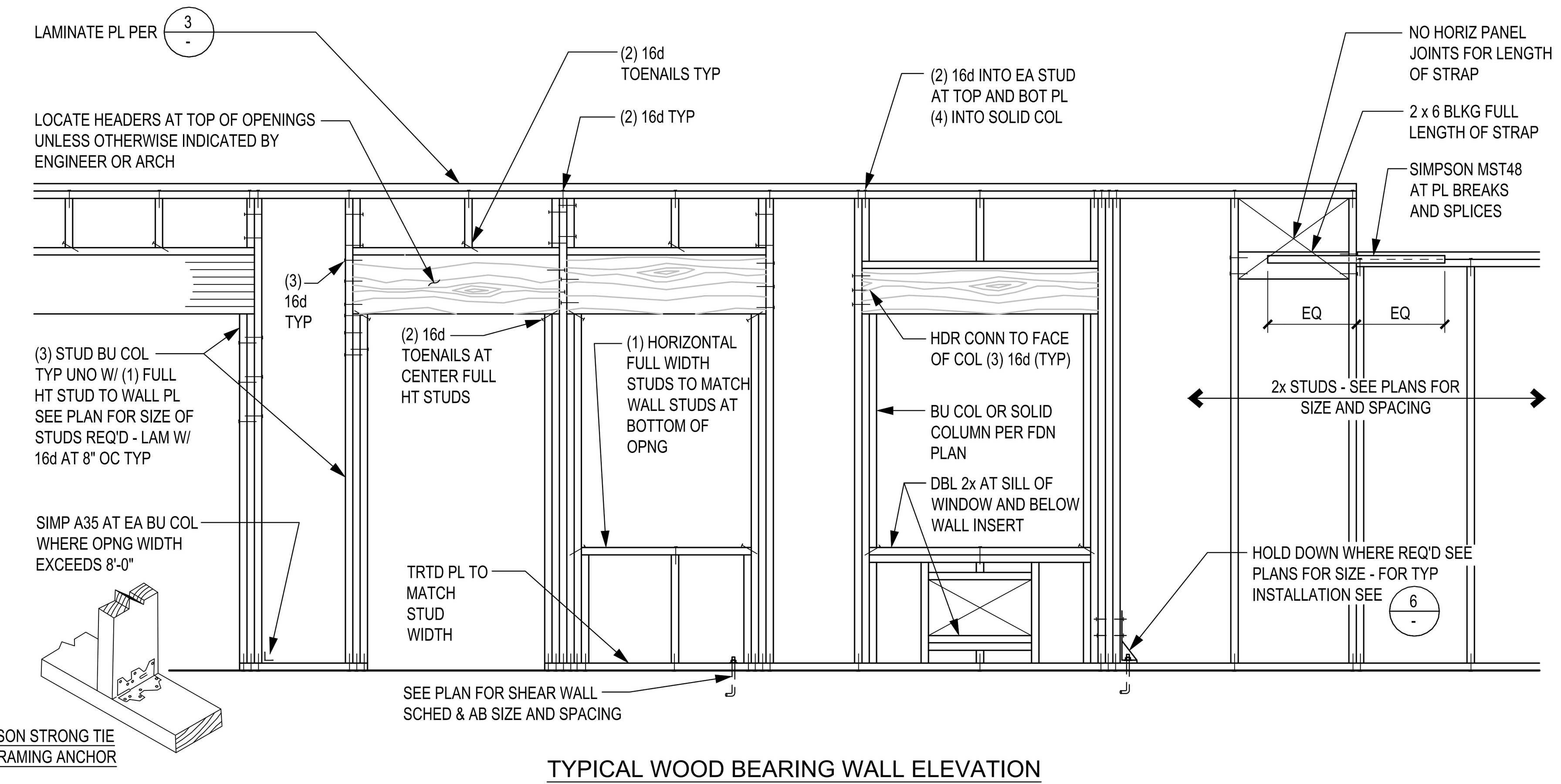
S0.2


WOOD I-JOISTS PARALLEL TO WALL WHERE SHOWN ON PLAN
SIMP DTC TO JOIST OR BLOCKING AT 32" OC MAX
WOOD I-JOISTS PER PLAN
2x4 FLAT BLKG AT 32" OC MAX WHERE JOISTS RUN PARALLEL TO WALL W/ SIMP Z2 EACH END
1/2" CLR W/ FULL DEAD LOAD
NOTE:
AT MULTI-STORY CONDITIONS, DEFLECTION CLIPS ARE REQUIRED ONLY AT THE FIRST LEVEL OF WOOD FRAMED CONSTRUCTION

INTERIOR NON-LOAD BEARING PARTITION WALL CONNECTION TO FLOOR JOISTS


WOOD I-JOISTS PARALLEL TO WALL WHERE SHOWN ON PLAN
SIMP SDPW 19600 DEFLECTOR SCREW AT 4'-0" OC
NOTE:
AT MULTI-STORY CONDITIONS, DEFLECTION CLIPS ARE REQUIRED ONLY AT THE FIRST LEVEL OF WOOD FRAMED CONSTRUCTION
WOOD I-JOISTS PER PLAN
2x4 FLAT BLKG AT 48" OC WHERE JOISTS RUN PARALLEL TO WALL W/ SIMP Z2 EACH END
1/2" GAP
3/4" OFFSET
NOTES:
1. PRE-DRILL W/ 3/8" DIA HOLE THRU TOP PLATES, NOT THRU FRAMING MEMBERS.
2. POLYMER SLEEVE SHALL NOT PENETRATE FRAMING MEMBERS.
3. INSTALL SCREW USING OFFSET DRIVER BIT.

ALTERNATE SCREW CONNECTION


LAMINATE PL PER 3
LOCATE HEADERS AT TOP OF OPENINGS UNLESS OTHERWISE INDICATED BY ENGINEER OR ARCH
(2) 16d TOENAILS TYP
(2) 16d TYP
(2) 16d INTO EA STUD AT TOP AND BOT PL (4) INTO SOLID COL
NO HORIZ PANEL JOINTS FOR LENGTH OF STRAP
2 x 6 BLKG FULL LENGTH OF STRAP
SIMPSON MST48 AT PL BREAKS AND SPLICES
(3) 16d TYP
(3) STUD BU COL TYP UNO W/ (1) FULL HT STUD TO WALL PL SEE PLAN FOR SIZE OF STUDS REQ'D - LAM W/ 16d AT 8" OC TYP
(2) 16d TOENAILS AT CENTER FULL HT STUDS
(1) HORIZONTAL FULL WIDTH STUDS TO MATCH WALL STUDS AT BOTTOM OF OPNG
HDR CONN TO FACE OF COL (3) 16d (TYP)
BU COL OR SOLID COLUMN PER FDN PLAN
DBL 2x AT SILL OF WINDOW AND BELOW WALL INSERT
EQ
EQ
2x STUDS - SEE PLANS FOR SIZE AND SPACING
SIMP A35 AT EA BU COL WHERE OPNG WIDTH EXCEEDS 8'-0"
TRTD PL TO MATCH STUD WIDTH
HOLD DOWN WHERE REQ'D SEE PLANS FOR SIZE - FOR TYP INSTALLATION SEE 6
SEE PLAN FOR SHEAR WALL SCHED & AB SIZE AND SPACING
SIMPSON STRONG TIE A35 FRAMING ANCHOR

TYPICAL WOOD BEARING WALL ELEVATION

SECTION

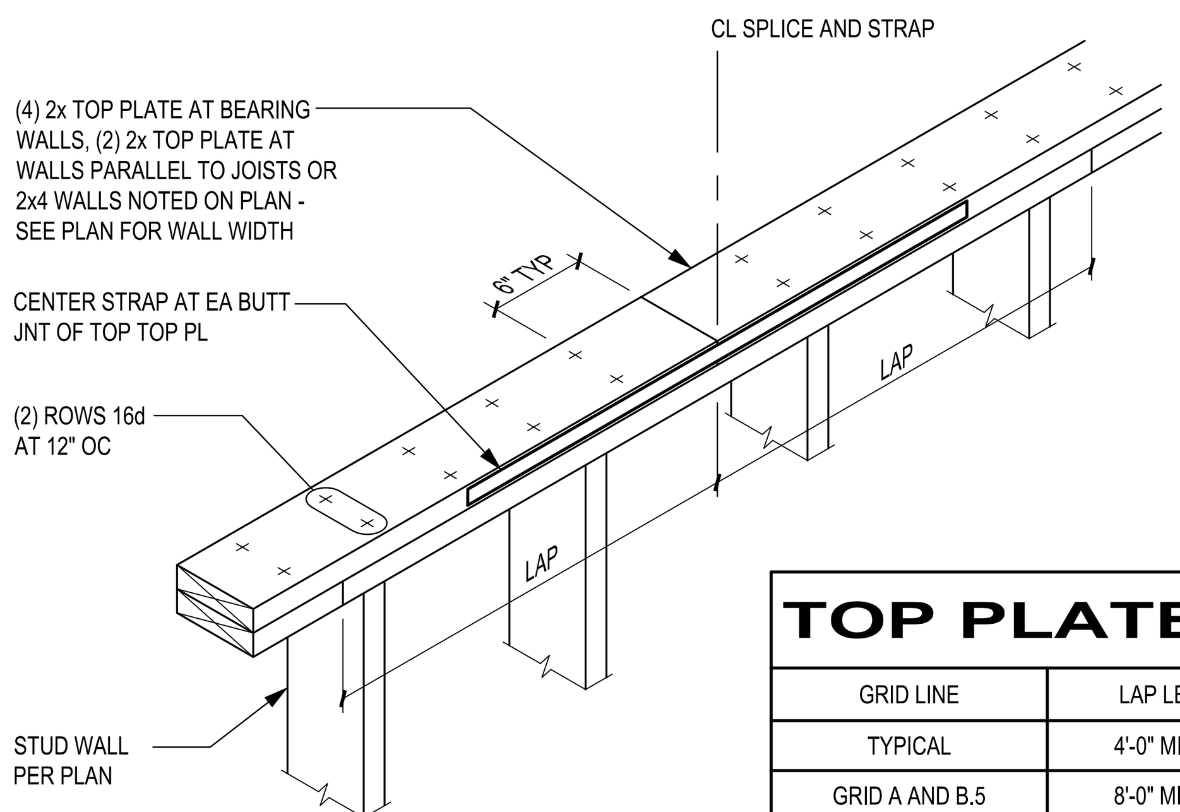
1" = 1'-0"

1

TYPICAL

NTS

2



TOP PLATE SPLICE SCHEDULE

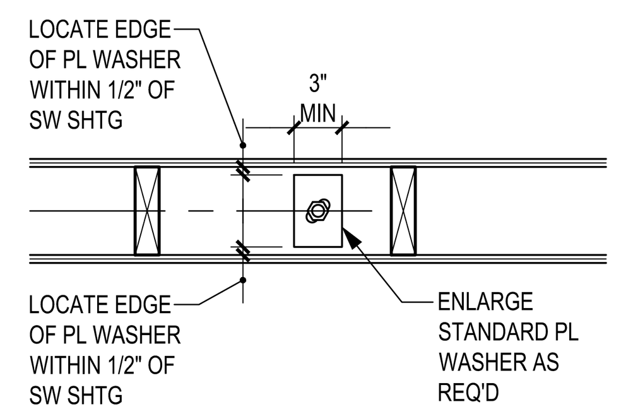
GRID LINE	LAP LENGTH	STRAP
TYPICAL	4'-0" MINIMUM	NONE
GRID A AND B.5	8'-0" MINIMUM	SIMPSON MSTA 36
GRID 1.8 AND 1.9	8'-0" MINIMUM	(2) SIMPSON MSTA36

TYPICAL DOUBLE TOP PLATE SPLICE

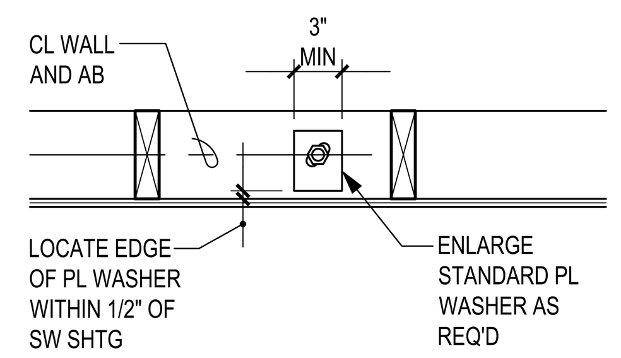
TYPICAL

NTS

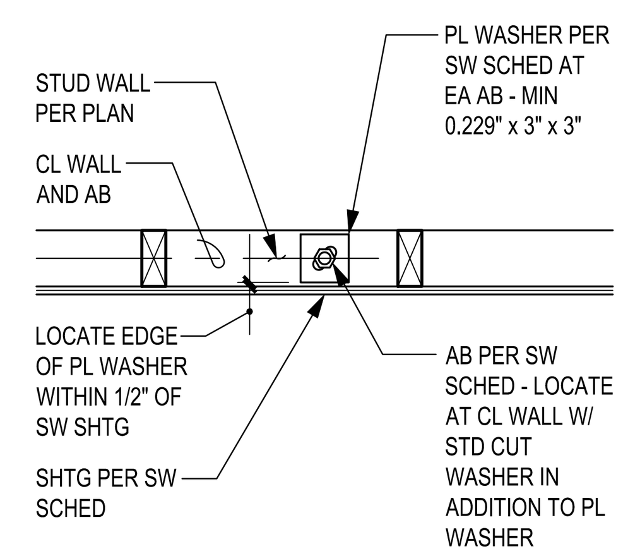
3



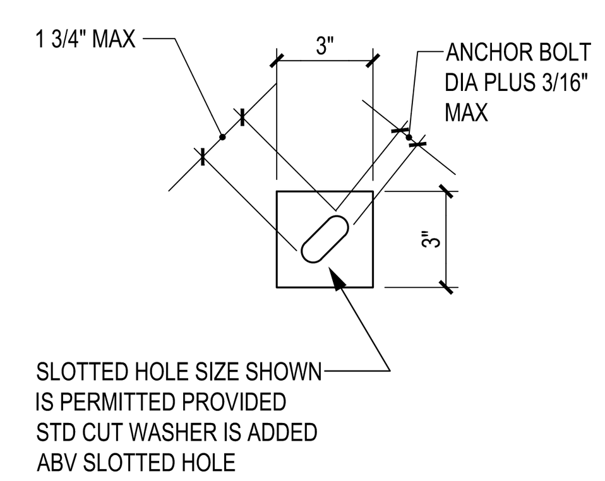
CONDITION AT WALLS SHEATHED BOTH SIDES



CONDITION AT 2x6 AND LARGER WALLS



TYPICAL CONDITION

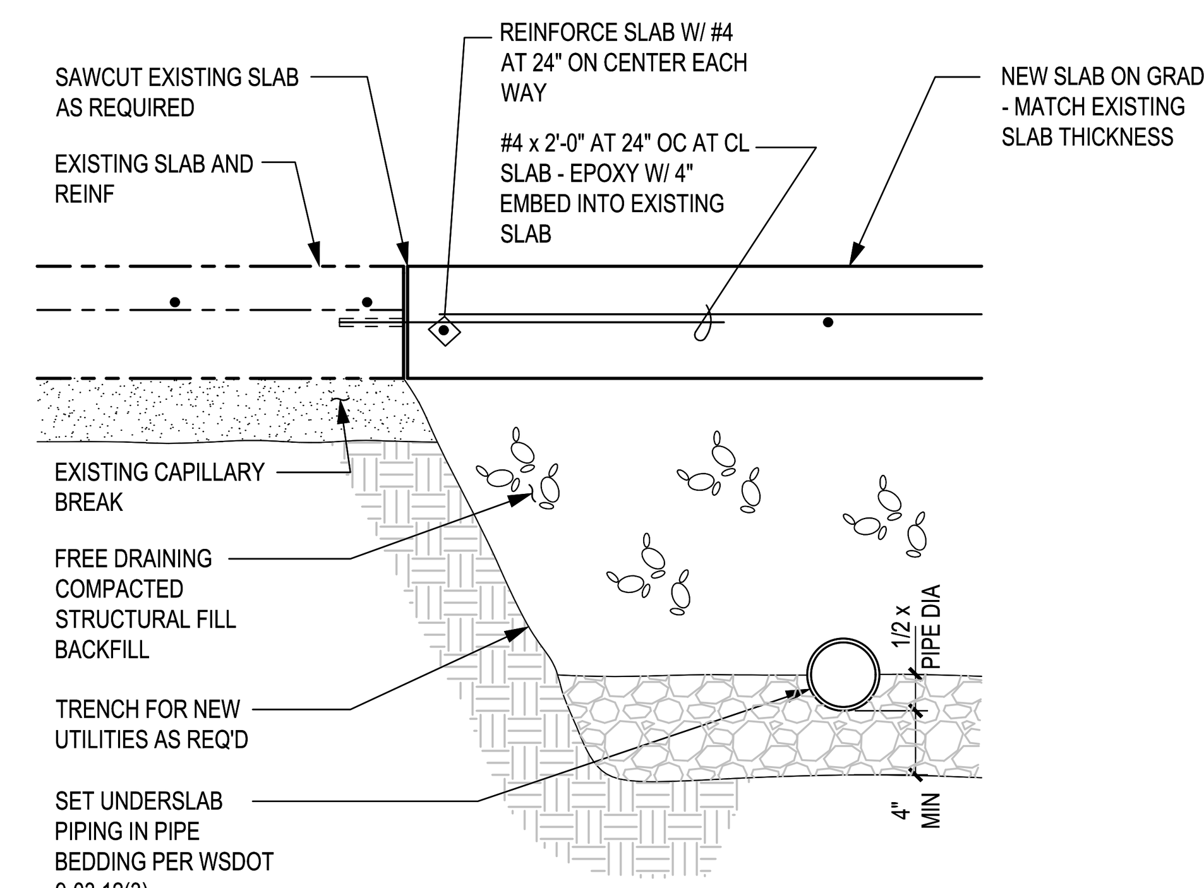


STANDARD (MINIMUM) PL WASHER

TYPICAL

NTS

4


SAWCUT EXISTING SLAB AS REQUIRED
EXISTING SLAB AND REINF
REINFORCE SLAB W/ #4 AT 24" ON CENTER EACH WAY
#4 x 2'-0" AT 24" OC AT CL SLAB - EPOXY W/ 4" EMBED INTO EXISTING SLAB
NEW SLAB ON GRADE - MATCH EXISTING SLAB THICKNESS
EXISTING CAPILLARY BREAK
FREE DRAINING COMPACTED STRUCTURAL FILL BACKFILL
TRENCH FOR NEW UTILITIES AS REQ'D
SET UNDERSLAB PIPING IN PIPE BEDDING PER WSDOT 9-03.12(3)
1/2 x PIPE DIA
4" MIN

NOTES:

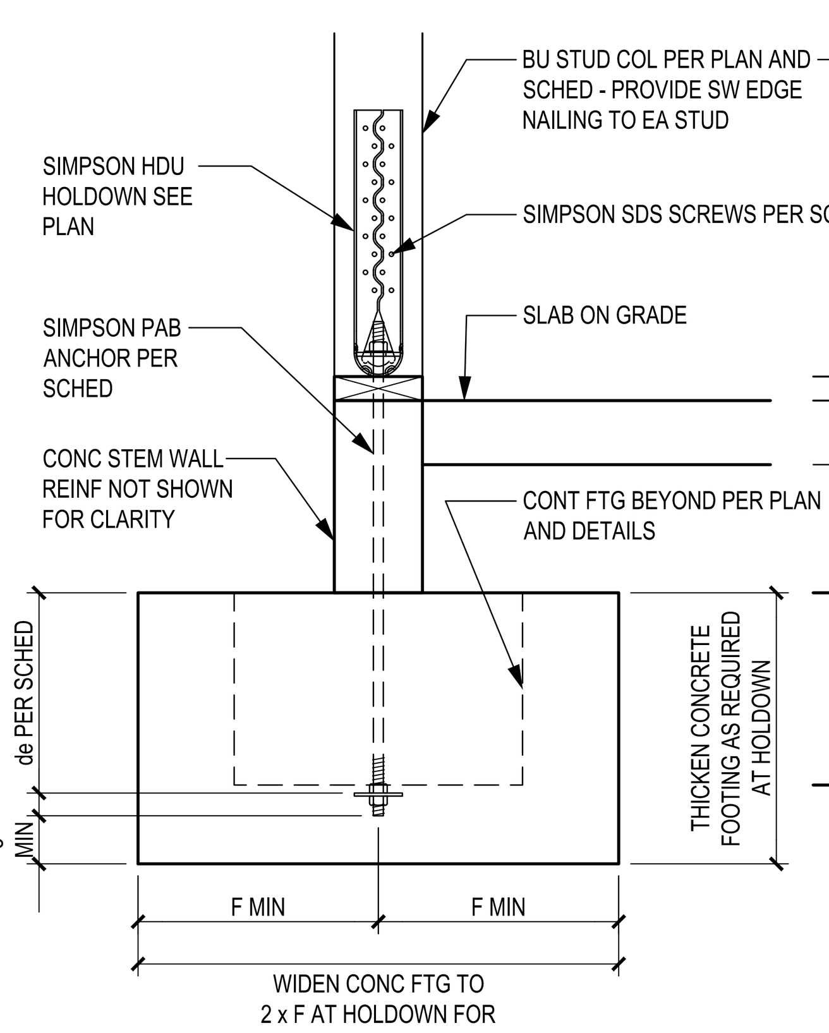
1. IF EDGE OF (E) SLAB IS UNDERMINED BY MORE THAN 6" HORIZONTAL FROM SAWCUT - BACKFILL UPPER 8-INCHES BELOW SLAB WITH LEAN CONCRETE
2. IF VAPOR BARRIER IS PRESENT BELOW EXISTING SLAB - PROVIDE (N) 10 MIL VAPOR BARRIER AND CONNECT WITH SEAM TAPE TO (E) VAPOR BARRIER

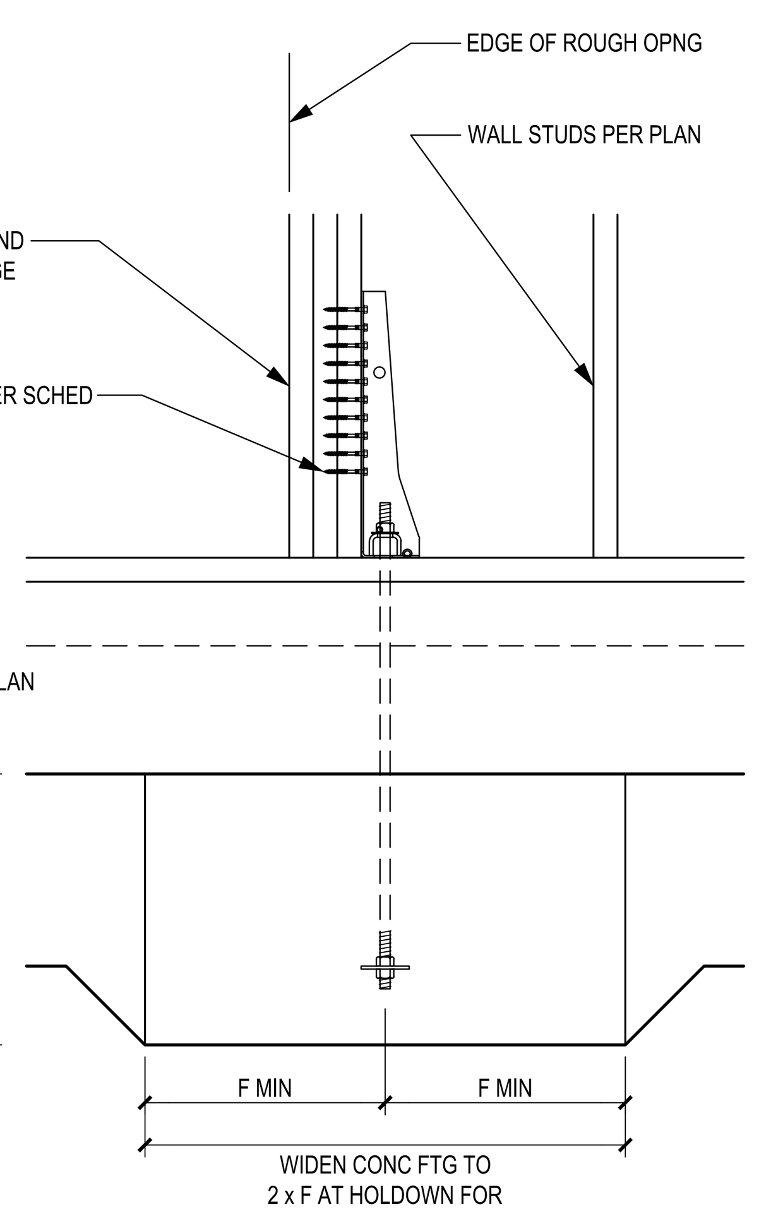
TYPICAL SLAB REPAIR AT TRENCHING

TYPICAL

NTS

5


SIMPSON HDU HOLDOWN SEE PLAN
BU STUD COL PER PLAN AND SCHED - PROVIDE SW EDGE NAILING TO EA STUD
SIMPSON SDS SCREWS PER SCHED
SIMPSON PAB ANCHOR PER SCHED
SLAB ON GRADE
CONC STEM WALL REINF NOT SHOWN FOR CLARITY
CONT FTG BEYOND PER PLAN AND DETAILS
de PER SCHED
3" MIN
F MIN
F MIN
WIDEN CONC FTG TO 2 x F AT HOLDOWN FOR A LENGTH OF 2 x F MIN
THICKEN CONCRETE FOOTING AS REQUIRED AT HOLDOWN



HOLDOWN SCHEDULE

HOLDOWN	FOUNDATION ANCHOR	de	F	FASTENERS	MINIMUM WOOD MEMBER THICKNESS	ALLOWABLE TENSION LOAD
SIMP HDU2	SIMP PAB5	6"	9"	(6) SIMP SDS 1/4" x 2 1/2"	3"	3075#
SIMP HDU4	SIMP PAB5	6"	9"	(10) SIMP SDS 1/4" x 2 1/2"	3"	4565#

HOLDOWN SCHEDULE NOTES:

1. ALLOWABLE LOADS ARE VALID FOR HOLDOWNS FLUSH OR RAISED OFF SILL PLATE.
2. TABULATED LOADS MAY BE DOUBLED WHEN THE HDU IS INSTALLED ON OPPOSITE SIDES OF THE WOOD MEMBER PROVIDED EITHER THE POST IS LARGE ENOUGH TO PREVENT OPPOSING HOLDOWN SCREW INTERFERENCE, OR THE HOLDOWNS ARE OFFSET TO ELIMINATE SCREW INTERFERENCE.

TYPICAL HOLDOWN REQUIREMENTS

TYPICAL

NTS

6

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

NELSON
Nelco Architecture, Inc.
1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA · SEATTLE · SPOKANE · TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:
IDI Logistics
IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:
RED DOT OFFICE TI
2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:
DANIEL L. BOOTH · STATE OF WASHINGTON · 28468 · STRUCTURAL ENGINEER · PROFESSIONAL ENGINEER

CITY STAMP:

TYPICAL DETAILS

Proj. No: 2220189.20 Reviewed By: DLB

S0.3

11. STATEMENT OF SPECIAL INSPECTIONS

IBC	SI	SO	TITLE
1705.2	✓	N/R	STEEL CONSTRUCTION (SEE TABLES 15A, 15B, 15C, AND 15D)
1705.3	✓	N/R	CONCRETE CONSTRUCTION (SEE TABLE 13)
1705.6	✓	N/R	SOILS (SEE TABLE 12A)
1705.12.2	✓	N/R	STRUCTURAL WOOD - SEISMIC FORCE RESISTING SYSTEM (SEE TABLE 18)

SI = SPECIAL INSPECTION

SO = STRUCTURAL OBSERVATION

✓ = ITEM IS REQUIRED

N/R = ITEM IS NOT REQUIRED

SPECIAL INSPECTIONS INDICATED ARE FOR STRUCTURAL ELEMENTS ONLY. SEE ARCH, MECH AND ELEC DRAWINGS FOR ADDITIONAL SPECIAL INSPECTIONS.

11.

11.1. INSPECTION/TESTING REQUIREMENTS:

SEE DRAWINGS, SPECIFICATIONS, AND IBC SECTIONS 110, AND CHAPTER 17.

11.2. INSPECTIONS BY THE BUILDING OFFICIAL (IBC SECTION 110):

11.2.1. FOOTING AND FOUNDATION INSPECTIONS SHALL BE MADE AFTER EXCAVATIONS ARE COMPLETE AND ANY REQUIRED REINFORCING IS IN PLACE. ANY REQUIRED FORMS SHALL BE IN PLACE PRIOR TO INSPECTION.

11.2.2. CONCRETE SLAB AND UNDER FLOOR INSPECTIONS SHALL BE MADE AFTER ALL IN SLAB OR UNDER FLOOR REINFORCING, CONDUIT, PIPING AND OTHER ANCILLARY EQUIPMENT ITEMS AND ACCESSORIES ARE IN PLACE BUT PRIOR TO CONCRETE PLACEMENT OR FLOOR SHEATHING INSTALLATION.

11.2.3. FRAMING INSPECTIONS SHALL BE MADE AFTER ALL SHEATHING, FRAMING, BLOCKING AND BRACING ARE COMPLETE AND ALL PIPES, DUCTS, ELECTRICAL, PLUMBING, ETC., ARE INSTALLED AND APPROVED PRIOR TO COVER.

11.2.4. IN ADDITION TO THE INSPECTIONS SPECIFIED ABOVE, THE BUILDING OFFICIAL IS AUTHORIZED TO MAKE OR REQUIRE OTHER INSPECTIONS OF ANY CONSTRUCTION WORK TO ASCERTAIN COMPLIANCE WITH THE PROVISIONS OF THE IBC OR OTHER LAWS ENFORCED BY THE BUILDING OFFICIAL.

11.3. STRUCTURAL TESTS AND SPECIAL INSPECTIONS (IBC CHAPTER 17):

11.3.1. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

11.3.2. STRUCTURAL TESTS AND SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE IBC AS WELL AS ANY ADDITIONAL REQUIREMENTS OF THE BUILDING OFFICIAL. OMISSION FROM THE LIST BELOW OF TESTING AND INSPECTION REQUIREMENTS SHALL NOT RELIEVE THE CONTRACTOR FROM PROVIDING TESTING AND INSPECTION REQUIRED BY THE SPECIFICATIONS, THE IBC AND THE BUILDING OFFICIAL.

11.3.3. TESTING AND SPECIAL INSPECTIONS SHALL BE COMPLETED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE IBC FOR THE ITEMS LISTED IN THIS SECTION.

11.4. STRUCTURAL OBSERVATION

11.4.1. STRUCTURAL OBSERVATION MAY BE PERFORMED DURING CONSTRUCTION IN A MANNER AS REQUIRED TO BECOME GENERALLY FAMILIAR WITH THE IN-PLACE CONSTRUCTION.

11.4.2. STRUCTURAL OBSERVATION EXTENT SHALL BE AS INDICATED ABOVE. TIMING AND DURATION OF OBSERVATIONS SHALL BE COORDINATED WITH THE GENERAL CONTRACTOR DURING CONSTRUCTION.

11.4.3. CONSTRUCTION OBSERVATION REPORTS AND FINDINGS SHALL NOT BE VIEWED AS A WARRANTY OR GUARANTEE BY THE STRUCTURAL ENGINEER.

11.5. SPECIAL INSPECTOR: SHALL BE CURRENTLY WABO CERTIFIED.

11.5.1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.

11.5.2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, ENGINEER OF RECORD, ARCHITECT OF RECORD, AND OTHER DESIGNATED PERSONS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE GENERAL CONTRACTOR FOR CORRECTION, THEN, IF NOT IN CONFORMANCE, TO THE PROPER DESIGN AUTHORITY AND BUILDING OFFICIAL.

11.5.3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.

12A. REQUIRED SPECIAL INSPECTIONS AND TEST OF SOILS

IBC TABLE 1705.6

SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	N/R	✓
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	N/R	✓
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIAL	N/R	✓
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	✓	N/R
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	N/R	✓

12.

12.1. SPECIAL INSPECTIONS AND TESTS FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT, AND LOAD-BEARING REQUIREMENTS PER IBC 1705.6., AS NOTED IN TABLE 12A.

12.1.1. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONALS SHALL BE USED TO DETERMINE COMPLIANCE.

13. REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

IBC TABLE 1705.3

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	IBC REFERENCE
1.	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	N/R	✓	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2.	REINFORCING BAR WELDING:				
A.	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	N/R	✓	AWS D1.4 ACI 318:26.6.4	
B.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	N/R	✓		
C.	INSPECT ALL OTHER WELDS	✓	N/R		
3.	INSPECT ANCHORS CAST IN CONCRETE	N/R	□	ACI 318: 17.8.2	
4.	INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS				
A.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	✓	N/R	ACI 318: 17.8.2.4	
B.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4A	N/R	✓	ACI 318: 17.8.2	
5.	VERIFY USE OF REQUIRED DESIGN MIX	N/R	✓	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6.	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	✓	N/R	ASTM C 172 ASTM C 31 ACI318:26.4, 26.12	1908.10
7.	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	✓	N/R	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8.	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	N/R	✓	ACI 318: 26.5.3-26.5.5	1908.9
9.	INSPECT PRESTRESSED CONCRETE FOR:				
A.	APPLICATION OF PRESTRESSING FORCES	✓	N/R	ACI 318: 26.10	
B.	GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE RESISTING SYSTEM	✓	N/R		
10.	INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	N/R	✓	ACI 318: 26.9	
11.	VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	N/R	✓	ACI 318: 26.10.2	
12.	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	N/R	✓	ACI 318: 26.11.1.2(b)	

13.

13.1. CONCRETE: SPECIAL INSPECTION AND TESTING PER IBC TABLE 1705.3 AS NOTED IN TABLE 13, INCLUDING:

13.1.1. CONTINUOUS SPECIAL INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.

13.1.2. CONTINUOUS SPECIAL INSPECTION OF BOLTS INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.

13.1.3. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IAPMO, ETC.).

13.1.4. CONTINUOUS SPECIAL INSPECTION FOR CONCRETE REINFORCING BARS, CONCRETE MATERIALS OR PLACEMENT OF CONCRETE FOR COMPOSITE MEMBERS.

13.2. SPECIAL INSPECTIONS AND TESTS SHALL NOT BE REQUIRED FOR THE FOLLOWING:

13.2.1. ISOLATED SPREAD FOOTINGS OF BUILDINGS THREE STORIES OR LESS ABOVE THE GRADE PLANE THAT ARE FULLY SUPPORTED BY EARTH OR ROCK.

13.2.2. NON-STRUCTURAL CONCRETE SLABS ON GRADE.

15.A REQUIRED SPECIAL INSPECTION AND TESTS OF STRUCTURAL STEEL CONSTRUCTION – INSPECTION OF WELDING

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
	AISC TABLE N5.4-1			
1.	PRIOR TO WELDING, VERIFY AND INSPECT THE FOLLOWING:	N/R	✓	
A.	WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	✓	N/R	
B.	WELDING PROCEDURE SPECIFICATIONS (WPS)	✓	N/R	
C.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES	✓	N/R	AISC 360 A3.5
C.	MATERIAL IDENTIFICATION OF STRUCTURAL STEEL MEMBERS	N/R	✓	AISC 360 A3.1
E.	WELDER IDENTIFICATION SYSTEM	N/R	✓	
F.	FIT-UP OF GROOVE WELDS, INCLUDING JOINT GEOMETRY			
	1) JOINT PREPARATION	N/R	✓	
	2) DIMENSIONS: ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL	N/R	✓	
	3) CLEANLINESS: CONDITION OF STEEL SURFACES	N/R	✓	
	4) TACKING: TACK WELD QUALITY AND LOCATION	N/R	✓	
	5) BACKING TYPE AND FIT (IF APPLICABLE)	N/R	✓	
G.	FIT-UP OF CJP GROOVE WELDS OF HSS T-,Y- AND K-JOINTS WITHOUT BACKING, INCLUDING JOINT GEOMETRY.			
	1) JOINT PREPARATION	✓	N/R	
	2) DIMENSIONS: ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL	✓	N/R	
	3) CLEANLINESS: CONDITION OF STEEL SURFACES	✓	N/R	
	4) TACKING: TACK WELD QUALITY AND LOCATION	✓	N/R	
H.	CONFIGURATION AND FINISH OF ACCESS HOLES	N/R	✓	
H.	FIT-UP OF FILLET WELDS			
	1) DIMENSIONS: ALIGNMENT, GAPS AT ROOT	N/R	✓	
	2) CLEANLINESS: CONDITION OF STEEL SURFACES	N/R	✓	
	3) TACKING: TACK WELD QUALITY AND LOCATION	N/R	✓	
I.	CHECK WELDING EQUIPMENT	N/R	✓	
	AISC 360 TABLE N5.4-2			
2.	DURING WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	USE OF QUALIFIED WELDERS	N/R	✓	
B.	CONTROL AND HANDLING OF WELDING CONSUMABLES			
	1) PACKAGING	N/R	✓	
	2) EXPOSURE CONTROL	N/R	✓	
C.	NO WELDING OVER CRACKED TACK WELDS	N/R	✓	
D.	ENVIRONMENTAL CONDITIONS			
	1) WIND SPEED WITHIN LIMITS	N/R	✓	
	2) PRECIPITATION AND TEMPERATURE	N/R	✓	
E.	WELDING PROCEDURE SPECIFICATIONS FOLLOWED			
	1) SETTINGS ON WELDING EQUIPMENT	N/R	✓	
	2) TRAVEL SPEED	N/R	✓	
	3) SELECTED WELDING MATERIALS	N/R	✓	
	4) SHIELDING GAS TYPE AND FLOW RATE	N/R	✓	
	5) PREHEAT APPLIED	N/R	✓	
	6) INTERPASS TEMPERATURE MAINTAINED	N/R	✓	
	7) PROPER POSITION	N/R	✓	
F.	WELDING TECHNIQUES			
	1) INTERPASS AND FINAL CLEANING	N/R	✓	
	2) EACH PASS WITHIN PROFILE LIMITATIONS	N/R	✓	
	3) EACH PASS MEETS QUALITY REQUIREMENTS	N/R	✓	
G.	PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	✓	N/R	
	AISC 360 TABLE N5.4-3			
3.	AFTER WELDING, VERIFY AND INSPECT THE FOLLOWING:			
A.	WELDS CLEANED	N/R	✓	
B.	SIZE, LENGTH, AND LOCATION OF WELDS	✓	N/R	
C.	WELDS MEET VISUAL ACCEPTANCE CRITERIA			
	1) CRACK PROHIBITION	✓	N/R	
	2) WELD TO BASE METAL FUSION	✓	N/R	
	3) CRATER CROSS SECTION	✓	N/R	
	4) WELD PROFILES	✓	N/R	
	5) WELD SIZE	✓	N/R	
	6) UNDERCUT	✓	N/R	
	7) POROSITY	✓	N/R	
D.	ARC STRIKES	✓	N/R	
E.	k-AREA	✓	N/R	
F.	BACKING REMOVED AND WELD TABS REMOVED, IF REQUIRED	✓	N/R	
G.	REPAIR ACTIVITIES	✓	N/R	
H.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	✓	N/R	
I.	NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE ENGINEER OF RECORD	N/R	✓	

NOTES CONTINUE ON SHEET S0.05

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

NELSON

Nelco Architecture, Inc.

1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA · SEATTLE · SPOKANE · TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

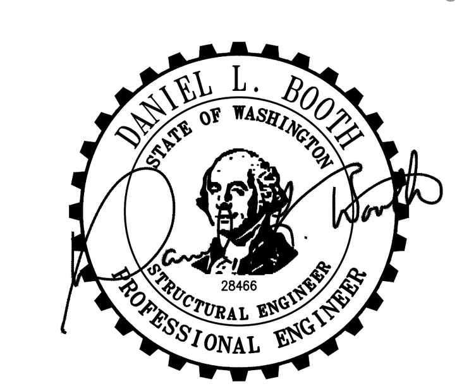
PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:



CITY STAMP:

SPECIAL INSPECTION

Proj. No: 2220189.20 Reviewed By: DLB

S0.4

15.B REQUIRED SPECIAL INSPECTION AND TESTS OF STRUCTURAL STEEL CONSTRUCTION – INSPECTION OF BOLTING

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
	AISC 360 TABLE N5.6-1			
1.	PRIOR TO BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
A.	MANUFACTURER'S CERTIFICATIONS FOR FASTENER MATERIALS	✓	N/R	
B.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	N/R	✓	
C.	PROPER FASTENER SELECTED FOR JOINT DETAIL	N/R	✓	AISC 360 A3.1
D.	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	N/R	✓	
E.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITIONS AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	N/R	✓	
F.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	✓	N/R	
G.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, AND OTHER FASTENER COMPONENTS	N/R	✓	
	AISC 360 TABLE N5.6-2			
2.	DURING BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
A.	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	N/R	✓	
B.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	N/R	✓	
C.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	N/R	✓	
D.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	N/R	✓	
	AISC 360 TABLE N5.6-3			
3.	AFTER BOLTING, VERIFY AND INSPECT THE FOLLOWING:			
A.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	✓	N/R	

15.

15.1. STRUCTURAL STEEL CONSTRUCTION:

SPECIAL INSPECTION AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS SHALL BE IN ACCORDANCE WITH THE QUALITY CONTROL AND QUALITY ASSURANCE REQUIREMENTS OF AISC 360, AS NOTED IN TABLES 15A, 15B, AND AWS D1.1, INCLUDING:

15.1.1. INSPECTION OF ERECTED STEEL SYSTEM.

15.1.2. REVIEW OF MATERIAL TEST REPORTS AND CERTIFICATIONS FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.

15.1.3. OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROCESS AND COMPLETED WELDS SHALL BE AS FOLLOWS:

A. VERIFY THAT WELD FILLER MATERIAL AND MANUFACTURER'S CERTIFICATE OF COMPLIANCE CONFORM TO AWS SPECIFICATION SPECIFIED. VERIFY WELDERS ARE CERTIFIED BY WABO, THAT PROPER ELECTRODES IN OVEN DRY CONDITIONS ARE USED, AND THAT PROPER METHODS AND PREPARATIONS ARE USED.

B. PERIODIC SPECIAL INSPECTION OF WELDING SHALL BE PERFORMED FOR SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16" AND FLOOR AND DECK WELDS.

C. CONTINUOUS SPECIAL INSPECTION OF WELDING SHALL BE PERFORMED ON COMPLETE AND PARTIAL PENETRATION GROOVE WELDS AND FILLET WELDS GREATER THAN 5/16".

D. ALL WELDS SHALL BE CHECKED VISUALLY.

E. ALL SHOP AND FIELD WELDING SHALL BE SUBJECT TO INSPECTION BY A WABO CERTIFIED WELDING INSPECTOR EMPLOYED BY THE OWNER. THE INSPECTOR SHALL UTILIZE RADIOGRAPHIC, ULTRASONIC, OR MAGNETIC PARTICLE TESTING AND ANY OTHER AID TO VISUAL INSPECTION THAT MAY BE DEEMED NECESSARY TO ASSURE THE ADEQUACY OF WELDING. THE OWNER SHALL CARRY OUT TESTING AND INTERPRETATION AT ANY STAGE AFTER WELDING.

F. 10% OF ALL FILLET WELDS SHALL BE CHECKED BY MAGNETIC PARTICLE TESTING.

G. 100% OF ALL COMPLETE PENETRATION WELDS SHALL BE CHECKED BY ULTRASONIC TESTING.

H. ALL WELDS FOUND DEFECTIVE AND REPAIRED SHALL BE REINSPECTED BY THE SAME METHOD ORIGINALLY USED. THE COST OF REPAIR AND REINSPECTION SHALL BE BORNE BY THE CONTRACTOR.

I. STANDARDS FOR ACCEPTANCE SHALL BE AS GIVEN IN AWS D1.1.

15.1.4. OBSERVATION OF BOLTING OPERATIONS.

15.1.5. CONTINUOUS SPECIAL INSPECTION SHALL BE PERFORMED FOR EACH JOINT OR MEMBER. PERIODIC SPECIAL INSPECTION SHALL BE PERFORMED ON ITEMS ON A RANDOM BASIS. PERIODIC SPECIAL INSPECTION NEED NOT DELAY FABRICATION OR ERECTION OPERATIONS.

15.1.6. EPOXY ANCHORS: SPECIFIC REQUIREMENTS FOR INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE OR MASONRY SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IAPMO, ETC.).

15.1.7. EXPANSION ANCHORS: SPECIFIC REQUIREMENTS FOR INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE OR MASONRY SHALL BE AS DESCRIBED IN THE RESEARCH REPORT ISSUED BY AN APPROVED SOURCE (ICC, IAPMO, ETC.).

18. REQUIRED SPECIAL INSPECTION AND TESTS FOR SEISMIC RESISTANCE

	SPECIAL INSPECTION OR TEST TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1.	STRUCTURAL WOOD IN SEISMIC DESIGN CATEGORY C, D, E OR F:		
A.	FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM.	✓	N/R
B.	NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE MAIN SEISMIC FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLDOWNS.	N/R	✓

18.

18.1. SPECIAL INSPECTIONS AND TESTING FOR SEISMIC RESISTANCE:

18.1.1. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE PER IBC 1705.12 SHALL BE REQUIRED FOR SEISMIC FORCE-RESISTING SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E OR F PER TABLE 18 AND THE FOLLOWING:

18.1.2. SPECIAL INSPECTION IS NOT REQUIRED FOR THE FOLLOWING:

A. STRUCTURAL WOOD WHERE THE FASTENER SPACING OF THE SHEATHING IS GREATER THAN 4 INCHES ON CENTER.

City of Puyallup
Development & Permitting Services
ISSUED PERMIT
Building | Planning
Engineering | Public Works
Fire | Traffic

DATE: April 19, 2022 FILENAME: Q:\2022\2220189\20_STR\CAD\2220189S-00.dwg

PRCTI20220873

FOUNDATION NOTES:

1. SEE SHEETS S0.1 AND S0.2 FOR STRUCTURAL NOTES. SEE SHEET S0.3 FOR TYPICAL DETAILS. SEE SHEETS S0.4 AND S0.5 FOR TESTING AND INSPECTION NOTES.
2. SEE GEOTECHNICAL ENGINEERING REPORT FOR ALL FOUNDATION AND SLAB SUPPORT REQUIREMENTS. THIS INCLUDES ALL EXCAVATION, FILL AND FILL PLACEMENT REQUIREMENTS.
3. SEE ARCHITECTURAL/MECHANICAL DRAWINGS FOR DRAINS, SLOPES, AND OTHER FLOOR DEPRESSIONS NOT SHOWN.
4. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, AND WALLS NOT SHOWN.
5. VERIFY ALL WINDOW AND DOOR WIDTHS AND HEIGHTS WITH ARCHITECTURAL DRAWINGS.
6. LOCATIONS OF COLUMNS LOCATED IN WALLS ARE SHOWN SCHEMATICALLY ON STRUCTURAL DRAWINGS. THE CONTRACTOR IS TO COORDINATE LOCATION OF COLUMNS WITH ARCHITECTURAL DRAWINGS.
7. COLUMNS NOT SPECIFICALLY LOCATED BY DIMENSIONS SHALL BE LOCATED ADJACENT TO OPENINGS AS DIMENSIONED BY THE ARCHITECT. SEE ARCHITECTURAL DRAWINGS FOR DETAILS AT ALL WINDOW AND DOOR JAMBS.
8. SEE ARCHITECTURAL DRAWINGS FOR STUD SIZE, SPACING, AND CALLOUTS AT NON-STRUCTURAL WALLS.
9. FOR TYPICAL CONNECTION OF NON-LOAD BEARING WALLS TO SLAB, USE POWDER ACTUATED FASTENERS AT 16" OC.
10. SEE THE SHEARWALL SCHEDULE FOR SHEATHING, NAILING AND ANCHOR BOLT REQUIREMENTS AT ALL WALLS INDICATED AS SHEARWALLS. EXTENT OF THE SHEARWALL REQUIREMENTS INCLUDE THE TOTAL LENGTH OF THE WALL INCLUDING ABOVE AND BELOW WINDOWS AND DOORS UNLESS NOTED OTHERWISE.
11. ALL LOAD BEARING WALL STUDS SHALL BE COVERED WITH A MIN OF 1/2" SHEATHING (EITHER GWB, OR WOOD SHEATHING AS APPLICABLE) ONE SIDE OF STUDS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL WALL COVERING REQUIREMENTS. SEE SHEARWALL SCHEDULE FOR SHEATHING. ATTACHMENT AT SHEARWALLS.
12. ALL 2X STUDS SHALL BE CONTINUOUS BETWEEN DETAIL CUTS. POSITION BUILT-UP STUDS TO ALIGN WITH THE TRUSSES ABOVE.
13. COLUMNS INDICATED EACH SIDE OF WALL OPENINGS SHALL BE (3) BU STUDS INCLUDING (1) BEARING STUD FOR OPENINGS UP TO 4'-0" WIDE. (4) BU STUDS INCLUDING (1) BEARING STUD FOR OPENINGS UP TO 6'-8" WIDE. PROVIDE MIN OF (3) BU STUDS AT ALL HOLDOWN LOCATIONS, UNLESS NOTED OTHERWISE.

FLOOR FRAMING NOTES:

1. SEE SHEETS S0.1 AND S0.2 FOR STRUCTURAL NOTES. SEE SHEET S0.3 FOR TYPICAL DETAILS. SEE SHEETS S0.4 AND S0.5 FOR TESTING AND INSPECTION NOTES.
2. ALL BEAMS SHALL HAVE 0" CAMBER UNLESS NOTED OTHERWISE.
3. ALIGN JOISTS WITH STUDS BELOW WHERE SPACINGS ARE EQUAL.
4. VERIFY ALL TOP OF BEAM AND TOP OF WALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
5. VERIFY ALL DOOR AND WINDOW WIDTHS AND HEIGHTS WITH ARCHITECTURAL DRAWINGS.
6. VERIFY SIZE AND LOCATION OF ALL MECHANICAL PENETRATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. ALL PRE-ENGINEERED JOIST SPACINGS SHALL BE 2'-0" OC EXCEPT AS SHOWN OR NOTED.
8. SCHEMATIC FLOOR SYSTEM SHOWN HAS BEEN DESIGNED TO MEET OR EXCEED A RED-BUILT FLOOR CHOICE RATING OF 4.5. JOIST MANUFACTURER SHALL SUBMIT CERTIFICATION THAT JOISTS DESIGNED AND INSTALLED AS INDICATED IN THE ARCHITECTURAL, STRUCTURAL, AND SHOP DRAWINGS RESULT IN A FLOOR SYSTEM WITH A VIBRATION PERCEPTIBILITY PERFORMANCE EQUAL TO OR EXCEEDING A RED-BUILT FLOOR CHOICE RATING OF 4.5.
9. JOIST MANUFACTURER SHALL PROVIDE DOUBLE JOISTS BELOW ALL PARTITION WALLS PARALLEL TO JOISTS AS INDICATED ON THE PLANS.
10. ATTACH NON STRUCTURAL WALLS TO FLOOR PER DETAIL 1 / S0.3.
11. UNLESS NOTED OTHERWISE, SHEATHING SHALL BE UNBLOCKED AND ORIENTED WITH LONG EDGE OF PANEL (OR FACE GRAIN IF PLYWOOD IS USED) PERPENDICULAR TO SUPPORTS. PANELS SHALL BE STAGGERED WITH OFFSET JOINTS OCCURRING OVER SUPPORTS. MINIMUM SHEATHING DIMENSION PERPENDICULAR TO SUPPORTS SHALL BE 24" UNLESS EDGES OF PANEL ARE BLOCKED.

FOOTING SCHEDULE

MARK	SIZE	REINFORCING	REMARKS
F2.0	2'-0" x 2'-0" x 1'-0"	(2) #5 EACH WAY AT BOTTOM OF FOOTING	
F3.0	3'-0" x 3'-0" x 1'-0"	(3) #5 EACH WAY AT BOTTOM OF FOOTING	
F4.0	4'-0" x 4'-0" x 1'-0"	(4) #5 EACH WAY AT BOTTOM OF FOOTING	
F2 x 4	2'-0" x 4'-0" x 1'-0"	(2) #5 AT BOTTOM OF FOOTING IN LONGITUDINAL DIRECTION (4) #5 AT BOTTOM OF FOOTING IN TRANSVERSE DIRECTION	

FOOTINGS SCHEDULE NOTES:

1. TOP OF FOOTING ELEVATION = -0'-7" UNLESS NOTED OTHERWISE ON PLAN.
2. FOOTING DESIGN BASED ON 2500 PSF ALLOWABLE SOIL BEARING PRESSURE.
3. EQUALLY SPACE REINFORCING IN EACH DIRECTION.
4. PROVIDE 3" CLEAR TO REINFORCING AT BOTTOM OF FOOTING.

SHEARWALL SCHEDULE

MARK	SHEATHING	NAILING		STUD SIZE AT ADJOINING PANEL EDGES	BLOCKING SIZE	FOUNDATION SILL PL ATTACHMENT	2x BOTTOM PLATE ATTACHMENT TO WOOD BELOW	LAMINATED STUDS AT VERTICAL PANEL JOINT	ASD ALLOWABLE UNIT SHEAR - SEISMIC	ASD ALLOWABLE UNIT SHEAR - WIND
		SIZE	SPACING							
W6	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4" MIN)	6" OC EDGES 12" OC FIELD	2x	2x FLAT OR 2x	3/4" DIA AT 48" OC	16d AT 6" OC STAGGERED	16d AT 6" OC STAGGERED	310 PLF	435 PLF
W2	15/32" APA RATED SHEATHING	10d COMMON (0.148" DIA x 2 1/4" MIN)	2" OC EDGES 12" OC FIELD	3x (12)	2x FLAT OR 3x (12)	3/4" DIA AT 16" OC	(3) ROWS 16d AT 8" OC STAGGERED	(3) ROWS 16d AT 8" OC STAGGERED	770 PLF	1078 PLF

APA RATED SHEATHING SHEARWALL NOTES:

1. NAILS SHALL BE COMMON FROM AN AMERICAN OR CANADIAN MFR ONLY. MINIMUM NAIL PENETRATION INTO WOOD FRAMING SHALL BE 1 1/2" FOR 10d NAILS. UNLESS NOTED OTHERWISE, NAIL DIAMETERS AND LENGTHS SHALL BE AS NOTED IN THE CARPENTRY HARDWARE SECTION OF THE STRUCTURAL NOTES. GALVANIZED NAILS SHALL BE HOT DIPPED OR TUMBLED.
2. APA RATED SHEATHING MATERIAL MAY BE EITHER PLYWOOD OR ORIENTED STRAND BOARD CONFORMING TO DOC PS 1 OR PS 2. SHEATHING MAY BE ORIENTED EITHER HORIZONTALLY OR VERTICALLY.
3. SHEATHING PANELS SHALL NOT BE LESS THAN 4' x 8' EXCEPT AT SHEARWALL BOUNDARIES AND CHANGES IN FRAMING. ALL PANELS EDGES SHALL BE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING.
4. ALL INTERIOR SHEAR WALLS HAVE BEEN DESIGNATED. ALL EXTERIOR WALLS WITHOUT DESIGNATION SHALL BE TYPE W6. WHERE THE SHEARWALL HAS BEEN DESIGNATED ON THE PLANS TO EXTEND ALONG LENGTHS OF WALLS WITH PENETRATIONS, SHEATHING AND NAILING OF THAT TYPE SHALL BE REQUIRED ABOVE AND BELOW WALL OPENINGS. OTHERWISE, SHEATHING AND NAILING ABOVE AND BELOW OPENINGS MAY BE TYPE W6.
5. UNLESS NOTED OTHERWISE, THE SHEARWALL DESIGNATION APPLIES TO FULL EXTENT OF WALL BETWEEN CORNERS OF WALLS.
6. SHEARWALLS SHALL RUN CONTINUOUS THROUGH BREAKS CAUSED BY INTERSECTING WALLS.
7. WHEN SHEATHING IS REQUIRED ON ONE SIDE ONLY, PLACE ON THE SIDE OF THE SYMBOL. WHERE THE SHEATHING IS NOTED ON TWO SIDES OF THE WALL, STAGGER VERTICAL PANEL JOINTS SUCH THAT JOINTS ON OPPOSITE SIDES OF THE WALL DO NOT FALL ON THE SAME FRAMING MEMBER.
8. NAIL SPACING INDICATED ON SCHEDULE APPLIES TO ALL STUDS, TOP AND BOTTOM PLATES AND BLOCKING. NAIL SPACINGS OF 3" ON CENTER OR LESS AT ADJOINING PANEL EDGES SHALL BE STAGGERED. NAILS SHALL BE LOCATED AT LEAST 3/8" FROM PANEL EDGES.
9. PROVIDE SHEATHING EDGE NAILING TO ALL COLUMNS WITH HOLDOWNS AND STUDS ATTACHED TO STEEL TUBE COLUMNS.
10. HOT DIPPED GALVANIZED FASTENERS SHALL BE USED TO ATTACH TO ALL TREATED WOOD MEMBERS. ELECTROPLATED FASTENERS ARE NOT ACCEPTABLE.
11. SPACING OF WALL STUDS SHALL BE AS NOTED ON THE PLANS. SPACING OF STUDS SHALL NOT EXCEED 24" OC.
12. WHERE NOTED, THE WIDTH OF THE NAILED FACE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES SHALL BE 3" NOMINAL. TWO 2" NOMINAL FRAMING MEMBERS SHALL BE PERMITTED TO BE USED IN LIEU OF A SINGLE 3" NOMINAL MEMBER PROVIDED THE 2" NOMINAL MEMBERS ARE NAILED TOGETHER PER "LAMINATED STUDS AT VERTICAL PANEL JOINT" IN THE SCHEDULE ABOVE.
13. ANCHOR BOLTS SHALL NOT BE SPACED GREATER THAN 48" OC, AND SHALL HAVE 7" MIN. EMBED. EXPANSION BOLTS SHALL HAVE 5" MIN EMBED. SEE DETAILS FOR TYPE OF CONNECTION REQUIRED. PROVIDE A MINIMUM OF (2) ANCHOR BOLTS PER PIECE, WITH ONE ANCHOR LOCATED NOT MORE THAN 12" OR LESS THAN 4" FROM EACH END OF EACH PIECE. AT NON-SHEAR WALLS, PROVIDE SPECIFIED ANCHOR BOLTS AT 48" OC MAX, UNLESS NOTED OTHERWISE.
14. FOUNDATION ANCHOR BOLTS SHALL HAVE A STEEL PLATE WASHER AT EA ANCHOR BOLT NO LESS THAN 0.229" x 3" x 3" IN SIZE. THE HOLE IN THE PLATE WASHER SHALL BE PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1 3/4", PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. THE PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE FOUNDATION SILL PLATE. SLOTTED PLATE WASHERS SHALL BE A MINIMUM 3" x 4" FOR 2x6 WALLS, AND 3" x 6" FOR 2x8 WALLS.
15. STANDARD CUT WASHERS MAY BE SUBSTITUTED IN LIEU OF PLATE WASHERS FOR ALL TYPE W6 WALLS LONGER THAN 10 FEET.
16. FOR SHEAR WALLS FRAMED WITH ENGINEERED WOOD STUDS (LSL OR LVL), DF No.2 2x FRAMING THAT MATCHES THE DEPTH OF THE STUDS MAY BE SUBSTITUTED FOR ENGINEERED WOOD AT ALL WALL FOUNDATION SILLS AND WALL TOP PLATES, AS WELL AS BLOCKING.

NELSON
Nelco Architecture, Inc.
1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA • SEATTLE • SPOKANE • TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:
IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:
RED DOT OFFICE TI
2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:
DANIEL L. BOOTH
STATE OF WASHINGTON
20468
STRUCTURAL ENGINEER
PROFESSIONAL ENGINEER

CITY STAMP:

SPECIAL INSPECTION, TYPICAL NOTES AND SCHEDULES

Proj. No: 2220189.20 Reviewed By: DLB

S0.5

LEGEND:

SHEARWALL - REFER TO SHEARWALL SCHEDULE ON SHEET S0.5.

PERFORATED SHEARWALL - SHEATHING, NAILING AND BLOCKING SPECIFIED IN THE SHEARWALL SCHEDULE SHALL BE PROVIDED ABOVE AND BELOW OPENINGS FOR LENGTH INDICATED.

WOOD STUD FRAMED WALL ORIGINATING ON FOUNDATION OR FLOOR FRAMING PLAN ON WHICH IT IS NOTED.

COLUMN ORIGINATING ON FOUNDATION OR FLOOR FRAMING PLAN ON WHICH IT IS NOTED.

WALL BELOW FLOOR OR ROOF FRAMING PLAN ON WHICH IT IS NOTED - SEE PLAN BELOW FOR REQUIREMENTS.

COLUMN BELOW WITH HEADER SUPPORTING FLOOR OR ROOF FRAMING ON THE LEVEL ON WHICH IT IS NOTED - SEE PLAN BELOW FOR COLUMN REQUIREMENTS.

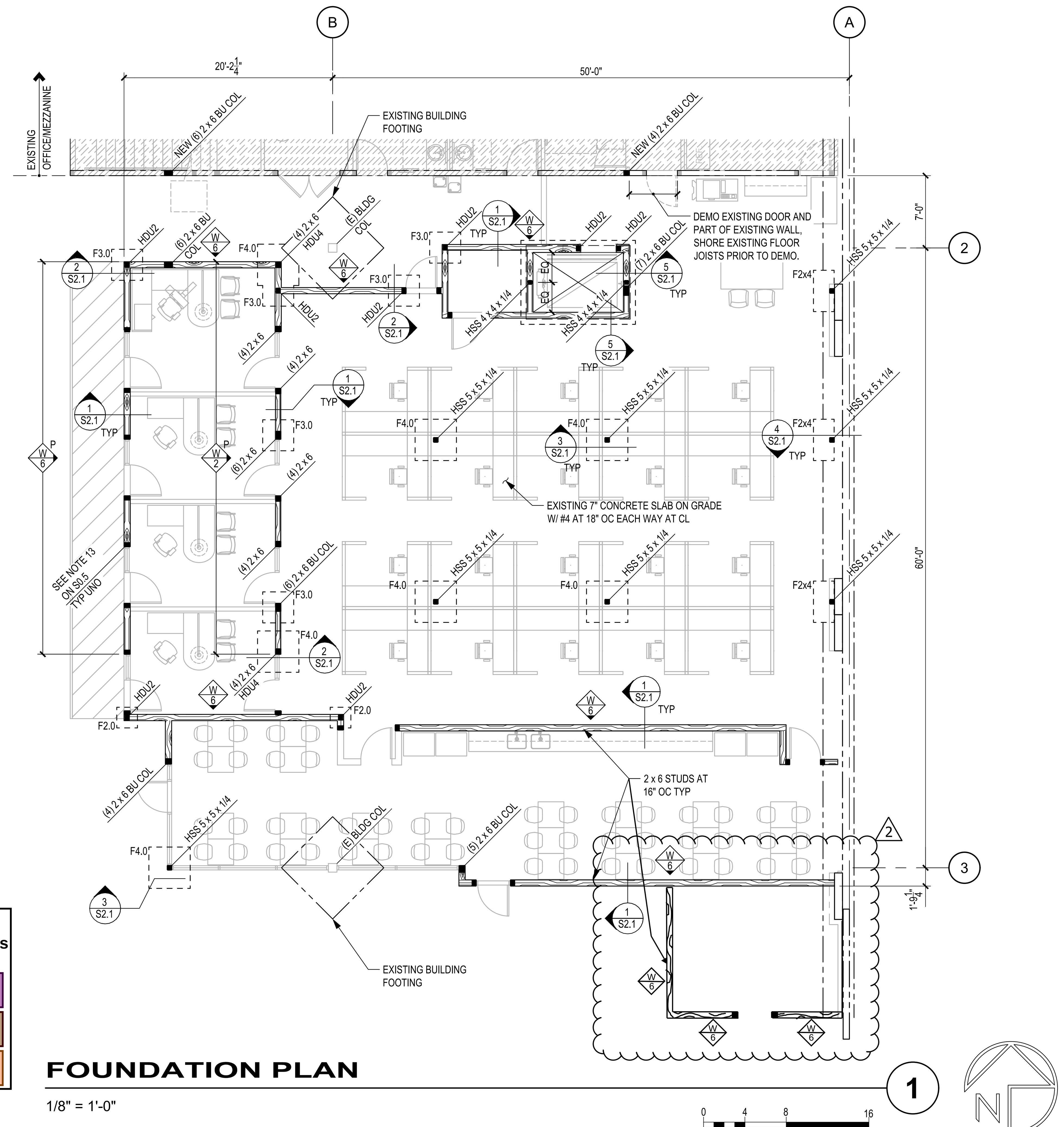
INDICATES COLUMN WITH HOLDOWN - PROVIDE SIMPSON HOLDOWN INDICATED AND SEE DETAIL 6 / S0.3 FOR REQUIREMENTS.

PHANTOM LINE - USED TO DEPICT EXISTING STRUCTURE.

EXISTING STRUCTURAL WALL.

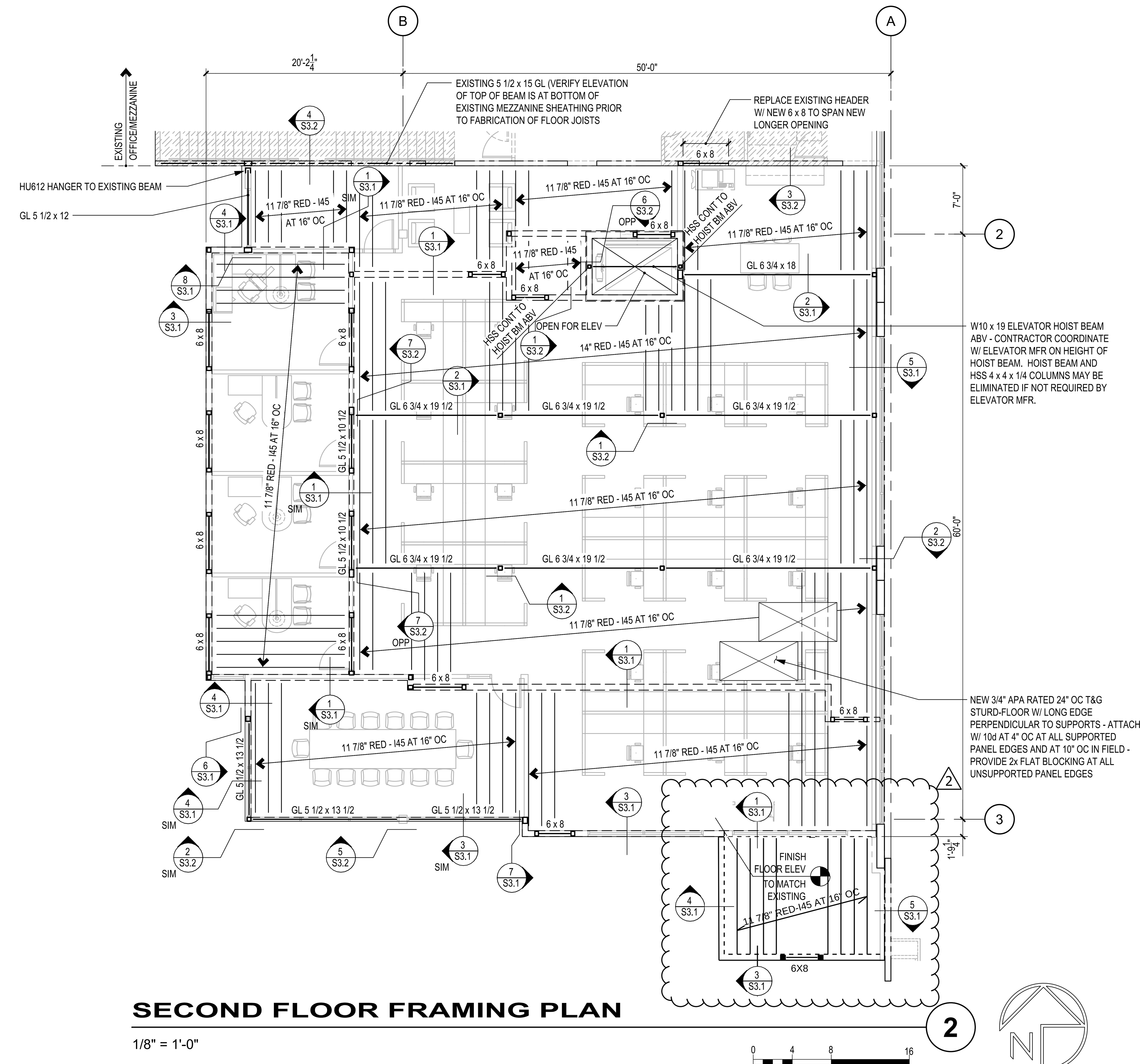
GENERAL PLAN NOTES:

1. SEE SHEETS S0.1 AND S0.2 FOR STRUCTURAL NOTES.
2. SEE SHEET S0.3 FOR TYPICAL DETAILS.
3. SEE SHEETS S0.4 AND S0.5 FOR TESTING AND INSPECTION NOTES.
4. SEE SHEET S0.5 FOR FOUNDATION AND FLOOR FRAMING NOTES, FOOTING AND SHEARWALL SCHEDULES.



FOUNDATION PLAN

1/8" = 1'-0"



SECOND FLOOR FRAMING PLAN

1/8" = 1'-0"

NELSON

Nelco Architecture, Inc.

1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL

TACOMA · SEATTLE · SPOKANE · TRI-CITIES

2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL	2	08/24/2022

SEAL:

DANIEL L. BOOTH · STATE OF WASHINGTON · 28408 · STRUCTURAL ENGINEER · PROFESSIONAL ENGINEER

CITY STAMP:

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

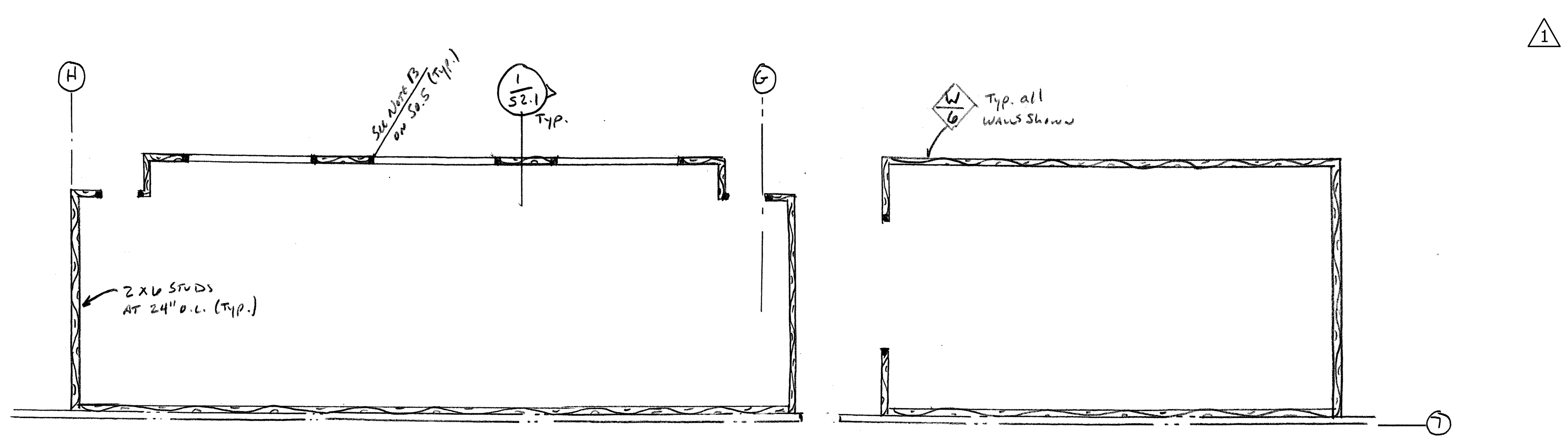
FOUNDATION AND FLOOR FRAMING PLANS

Proj. No: 2220189.20 Reviewed By: DLB

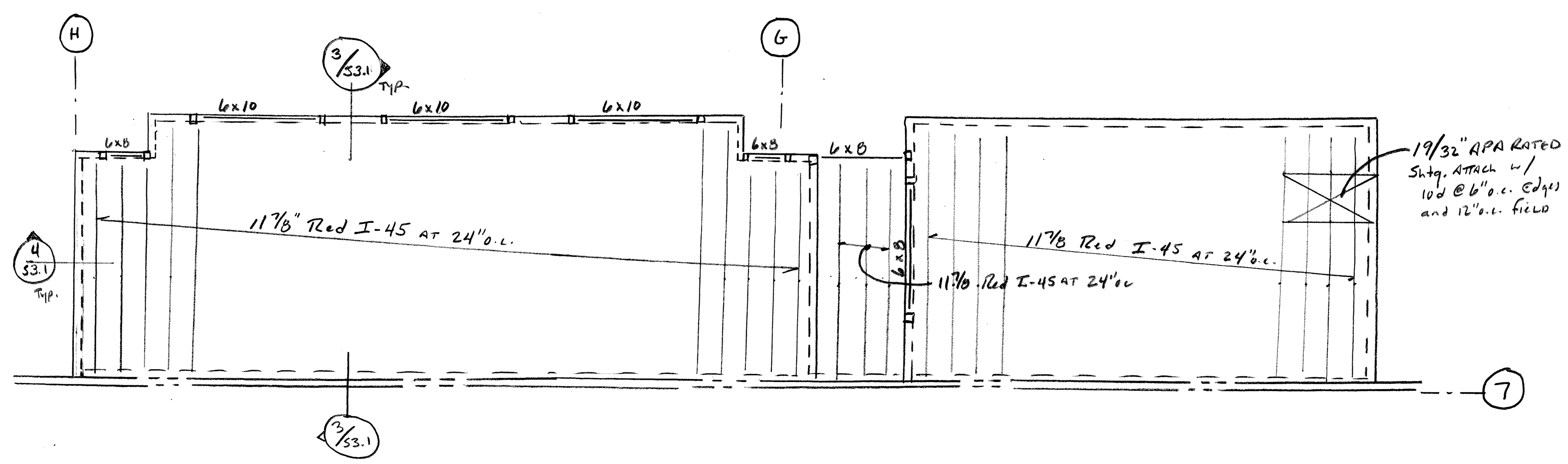
S1.1

DATE: April 19, 2022 FILENAME: Q:\2022\2220189\20_STR\CAD\2220189S-10.dwg

PRCTI20220873



Break Area / Restroom Plan
1/8" = 1'-0"
1



Ceiling Framing Plan
1/8" = 1'-0"
2

NELSON
Nelco Architecture, Inc.
1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA · SEATTLE · SPOKANE · TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:

DANIEL L. BOOTH · STATE OF WASHINGTON · STRUCTURAL ENGINEER · PROFESSIONAL ENGINEER

CITY STAMP:

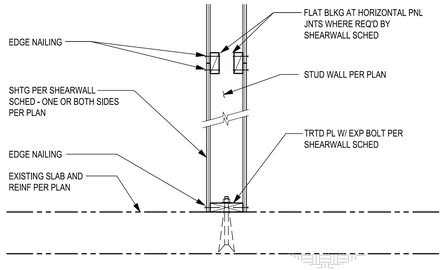
City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

Break Area / Restroom

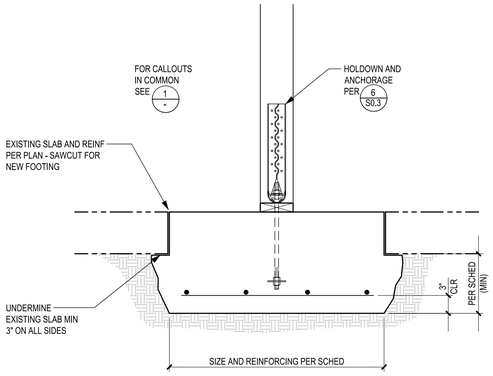
Proj. No: 2220189.20 Reviewed By: DLB

S1.2



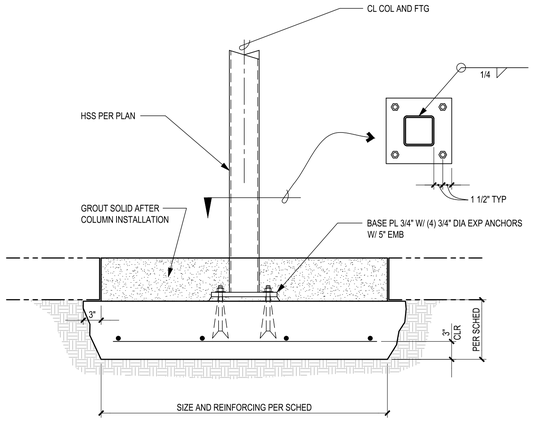
SECTION 1

1" = 1'-0"



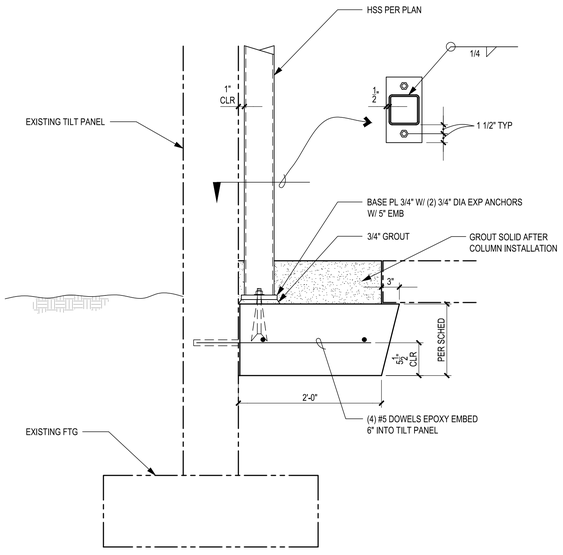
SECTION 2

1" = 1'-0"



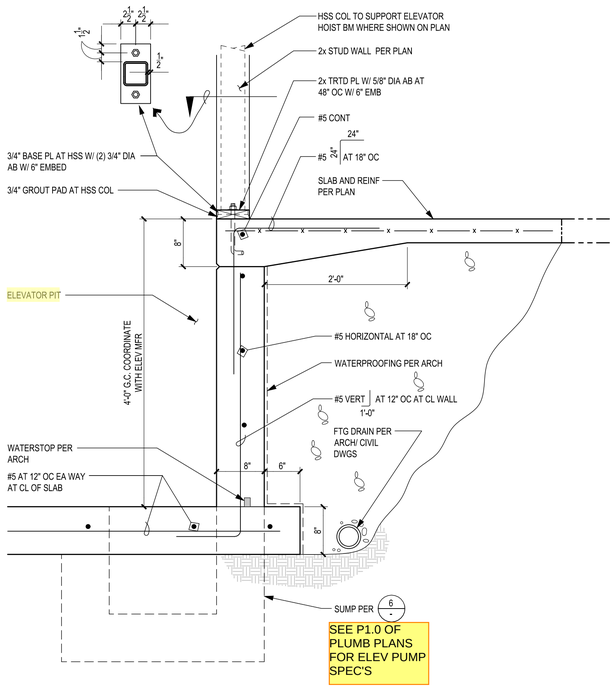
SECTION 3

1" = 1'-0"



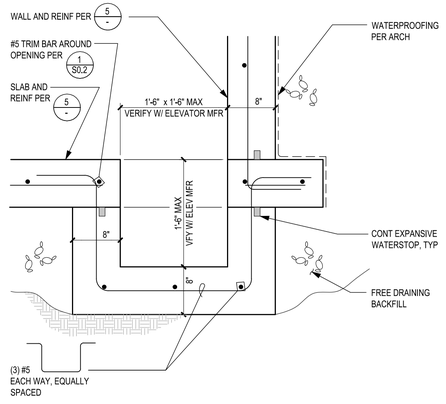
SECTION 4

1" = 1'-0"



SECTION 5

1" = 1'-0"



SECTION 6

1" = 1'-0"

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

DATE: April 19, 2022 FILENAME: Q:\2022\2220189\20_STR\CAD\2220189S-20.dwg

PRCTI20220873

NELSON

Nelco Architecture, Inc.

1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA • SEATTLE • SPOKANE • TRI-CITIES

2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

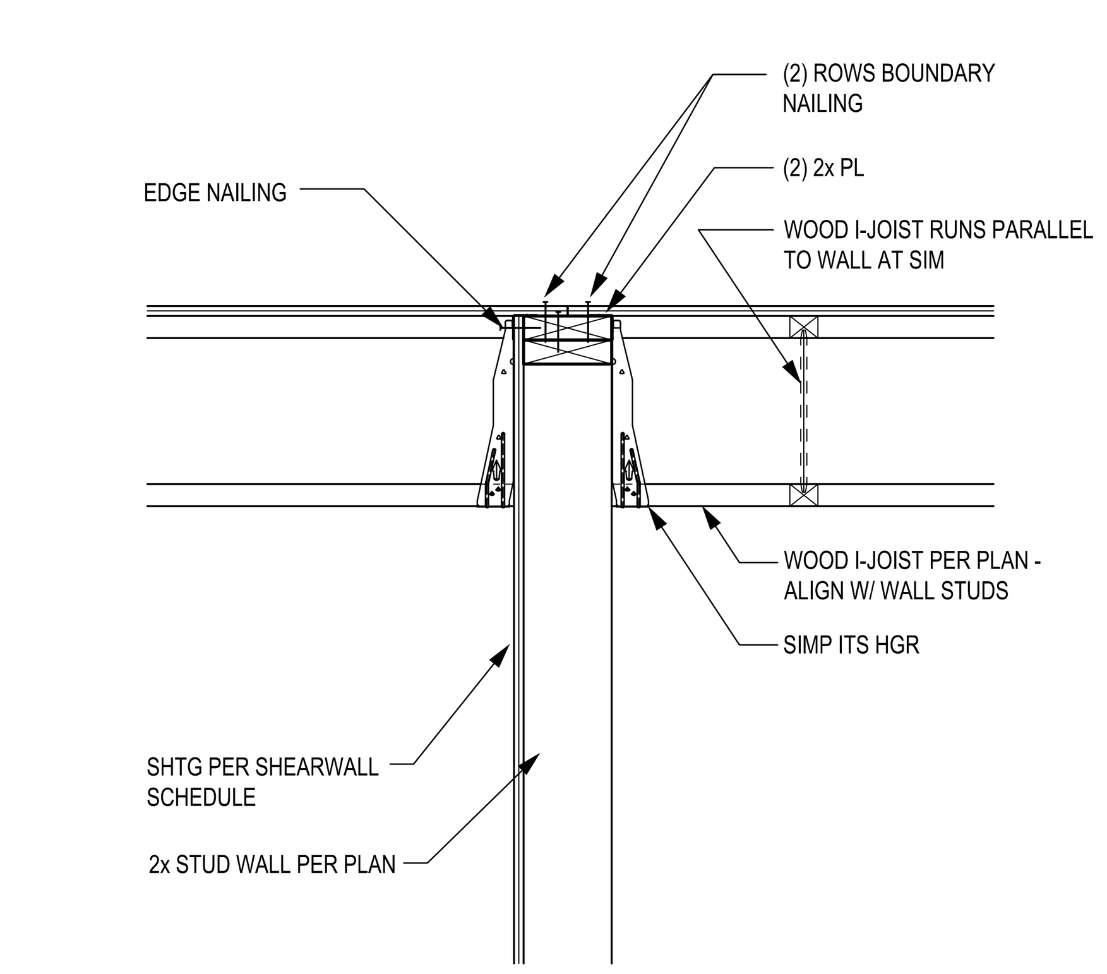
SEAL:

CITY STAMP:

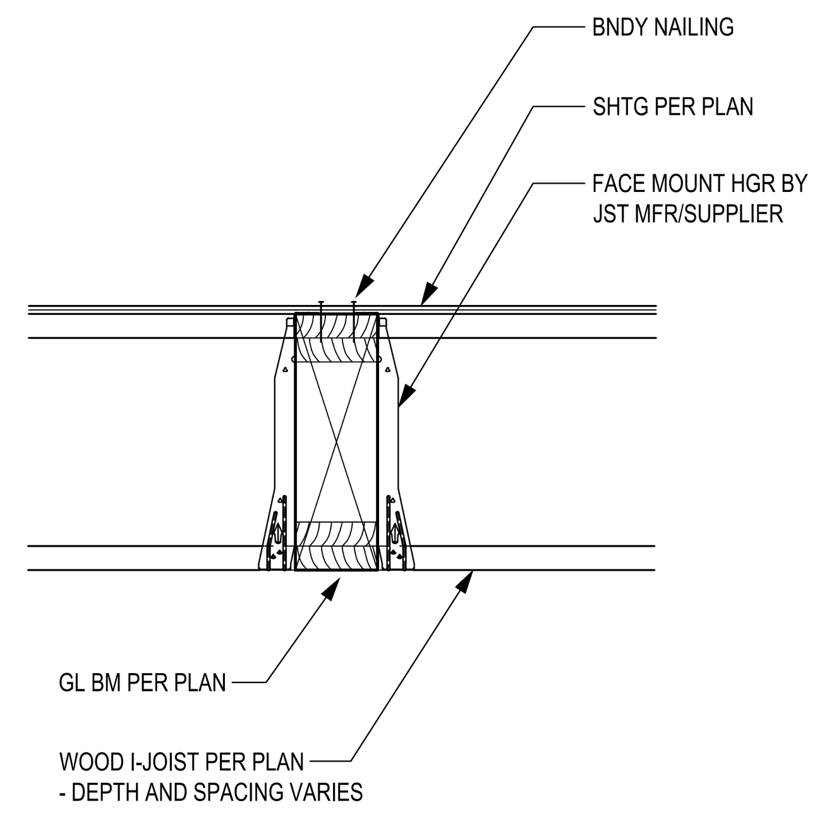
FOUNDATION DETAILS

Proj. No: 2220189.20 Reviewed By: DLB

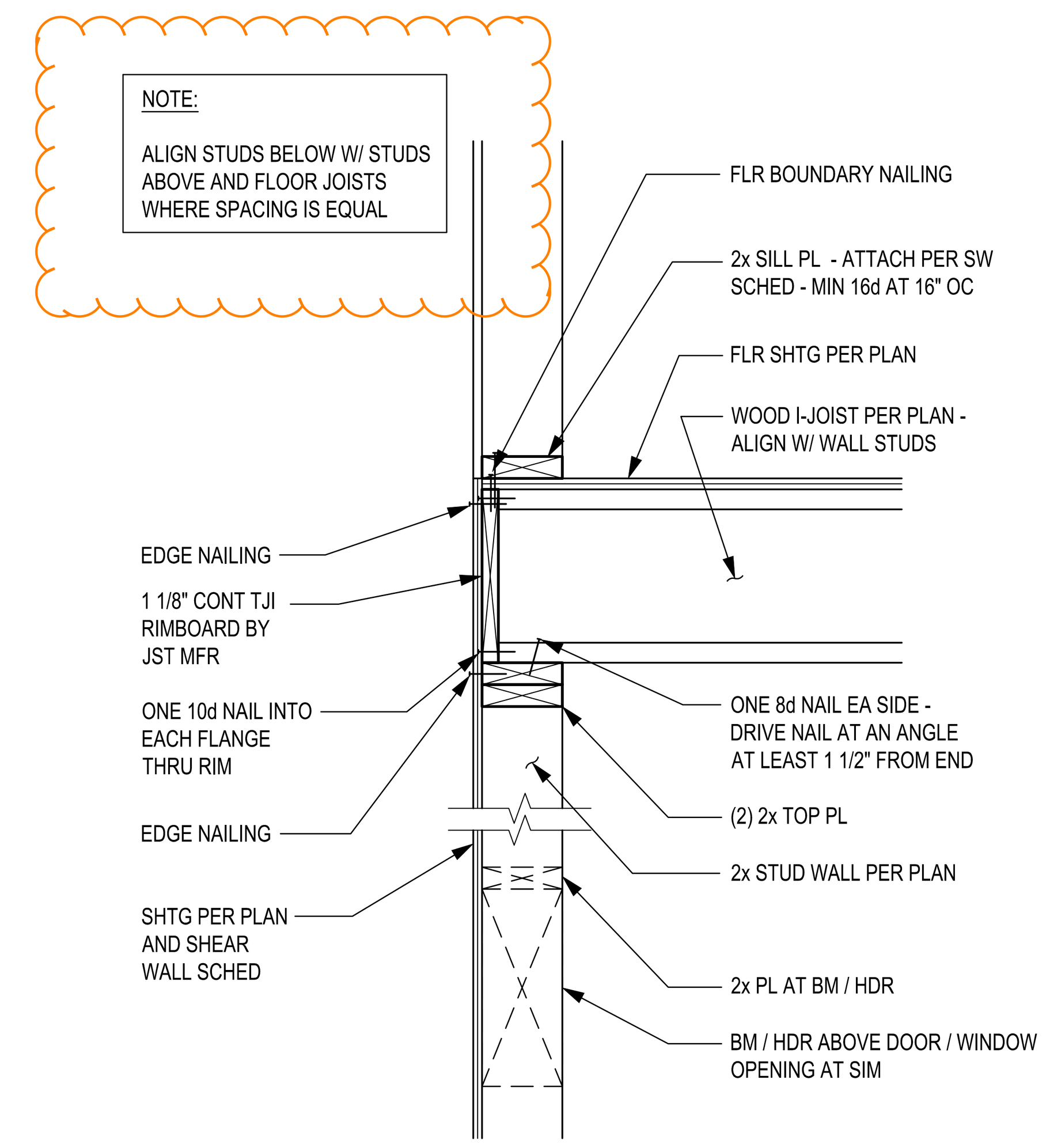
S2.1



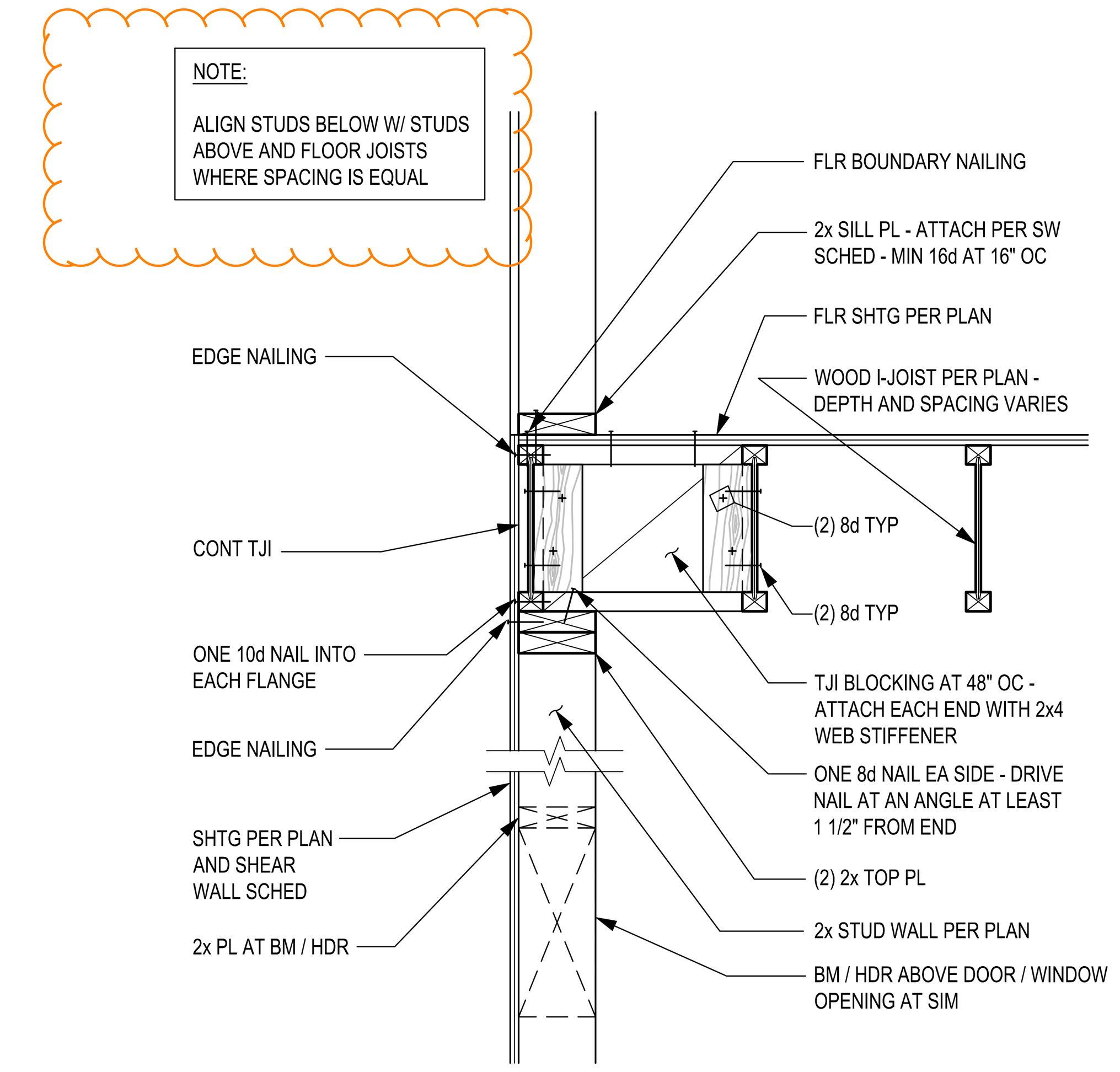
SECTION 1
1" = 1'-0"
02-014C



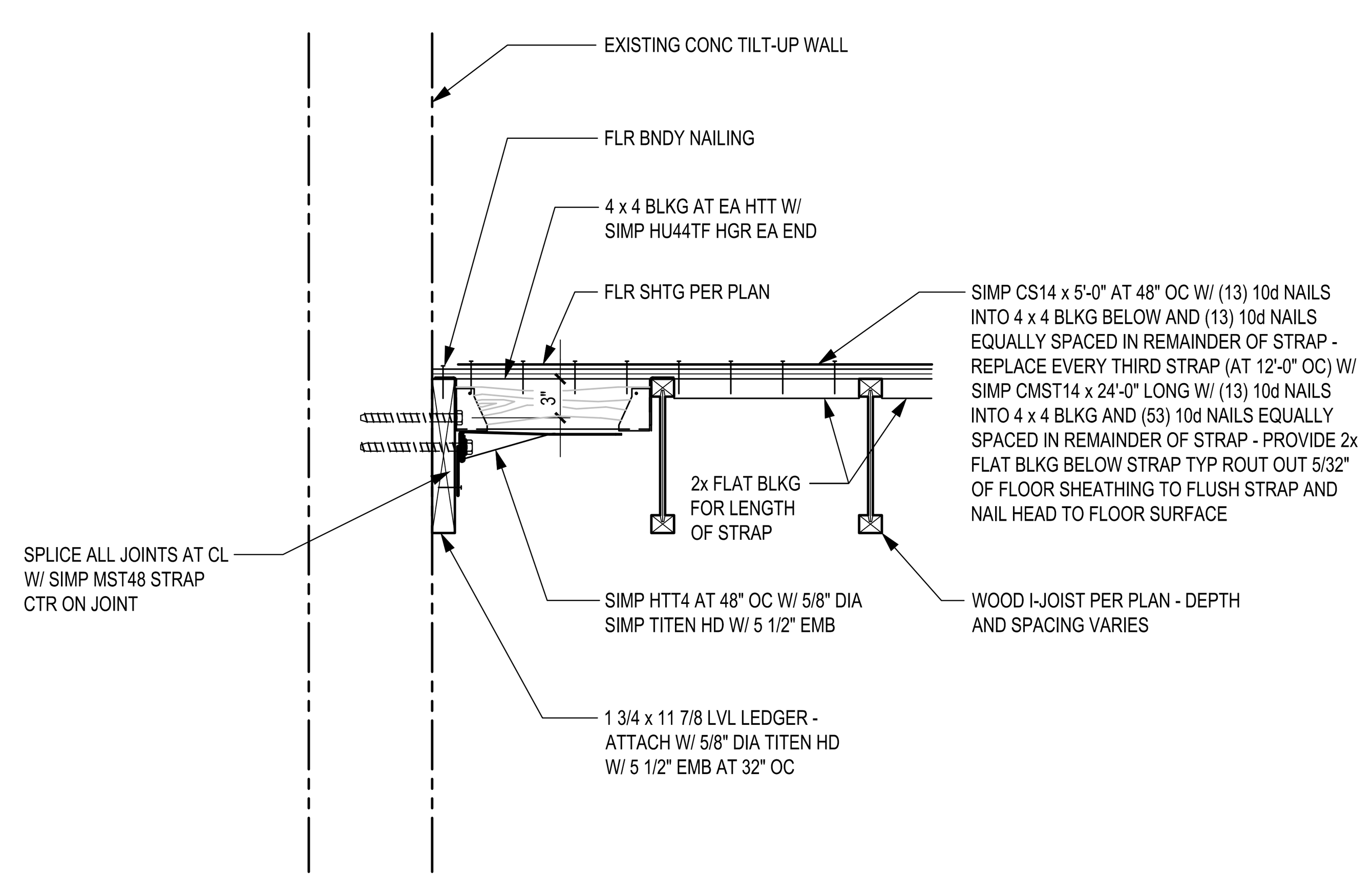
SECTION 2
1" = 1'-0"
02-030A



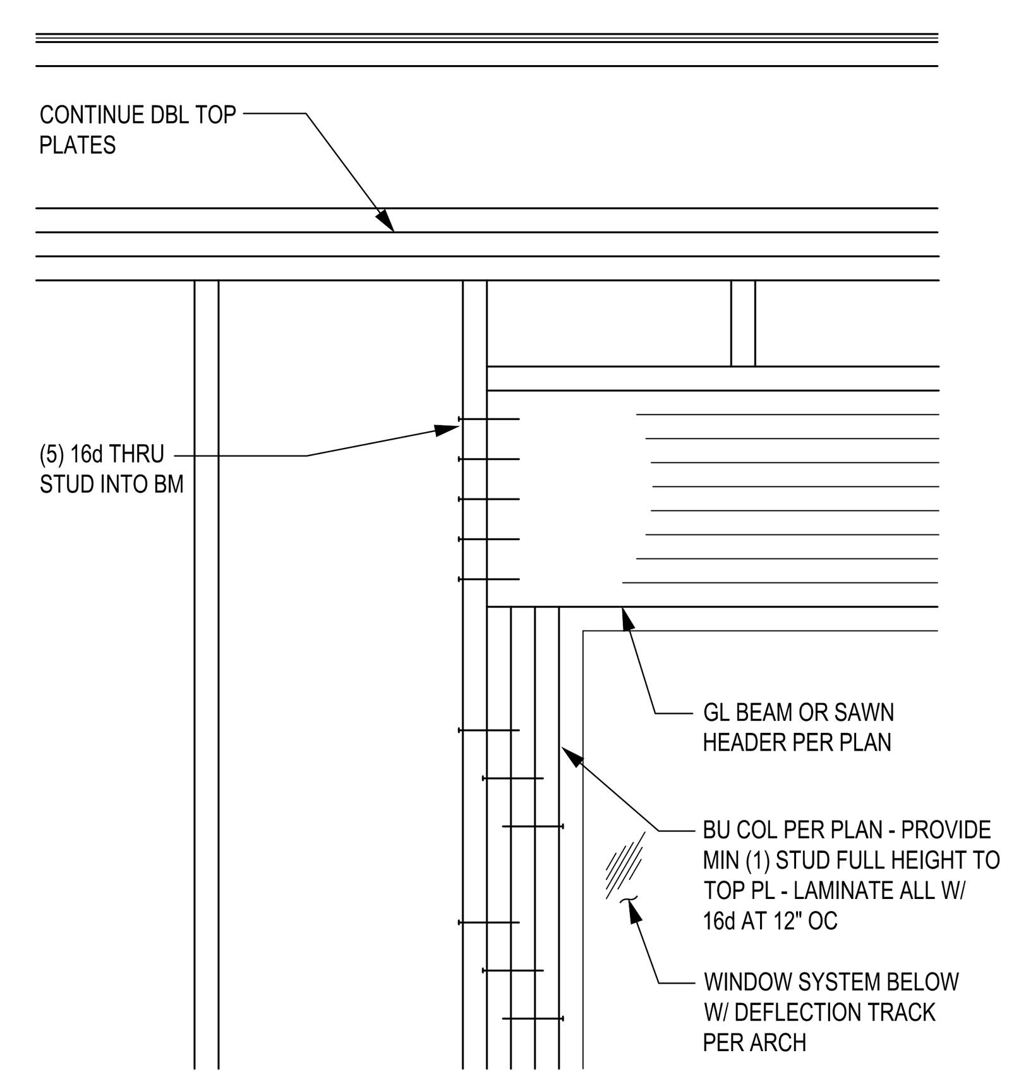
SECTION 3
1" = 1'-0"
02-002



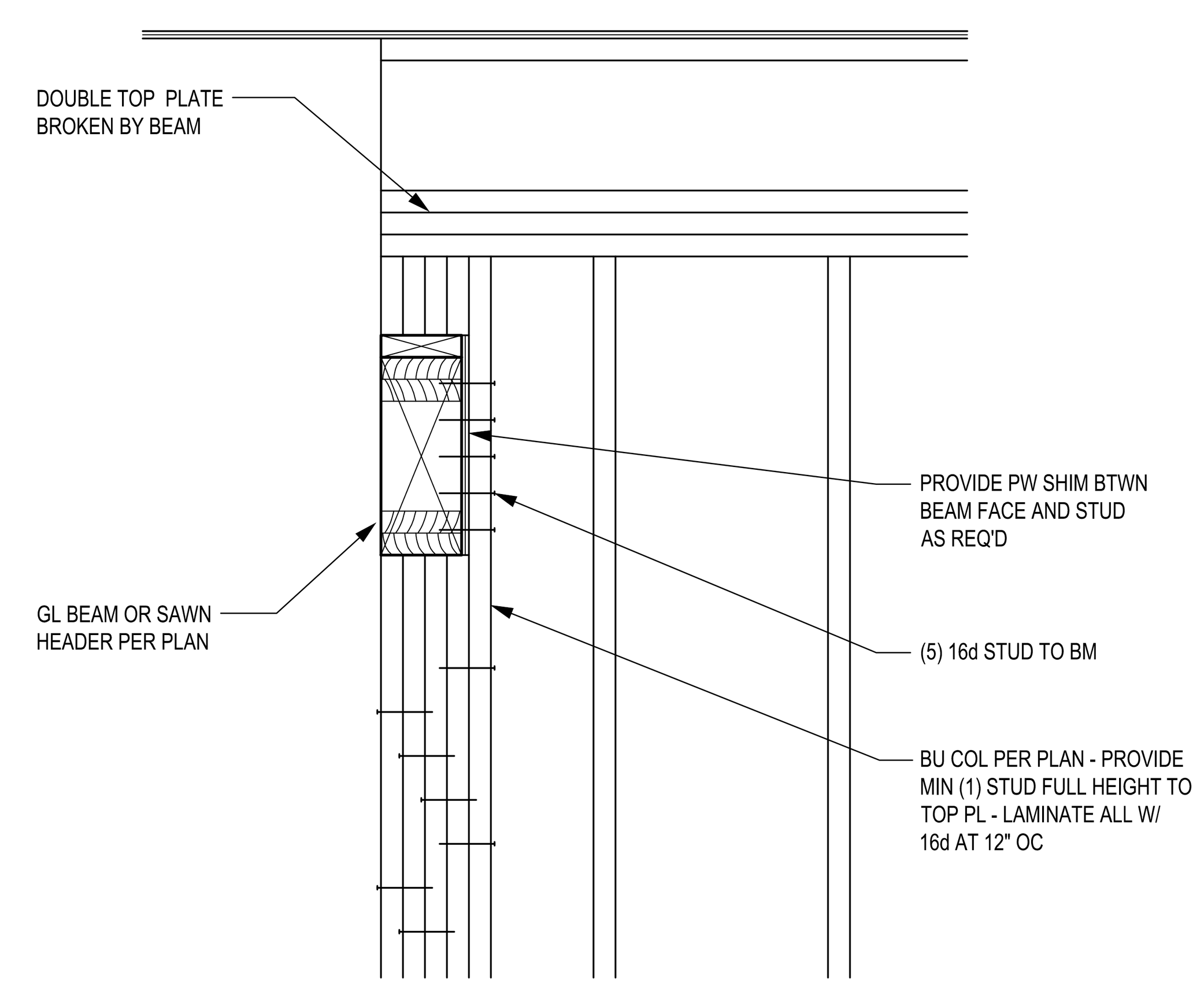
SECTION 4
1" = 1'-0"



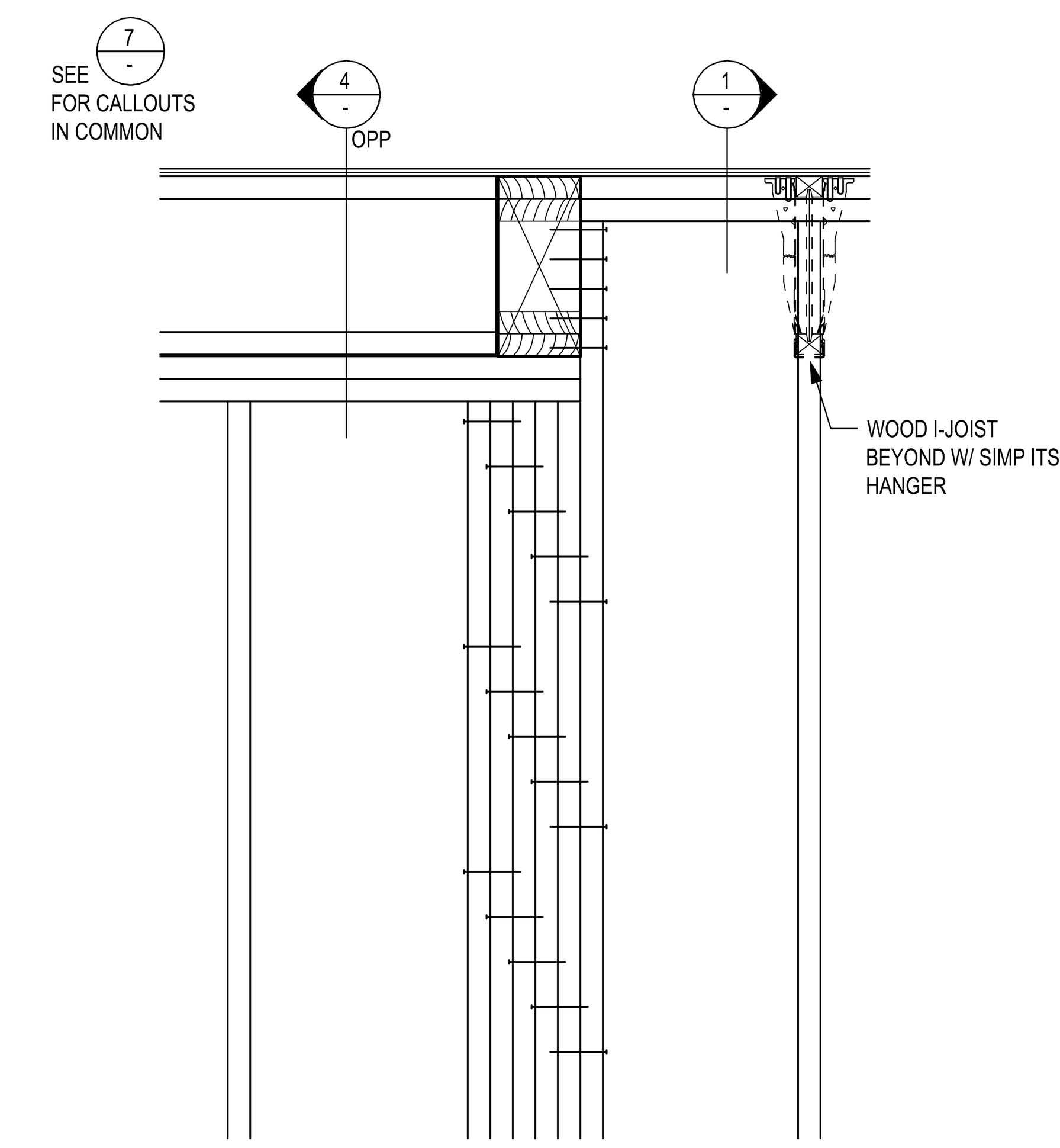
SECTION 5
1" = 1'-0"



SECTION 6
1" = 1'-0"
03-076



SECTION 7
1" = 1'-0"
03-074



SECTION 8
1" = 1'-0"

NELSON
Nelco Architecture, Inc.
1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA · SEATTLE · SPOKANE · TRI-CITIES
2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:
IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:
RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:
DANIEL L. BOOTH · STATE OF WASHINGTON · STRUCTURAL ENGINEER · PROFESSIONAL ENGINEER

CITY STAMP:

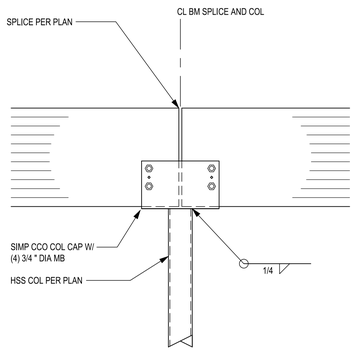
FRAMING DETAILS

Proj. No: 2220189.20 Reviewed By: DLB

S3.1

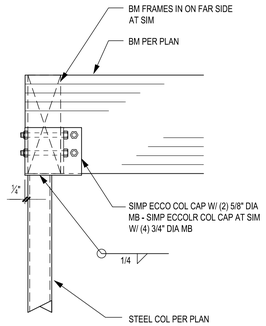
City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic



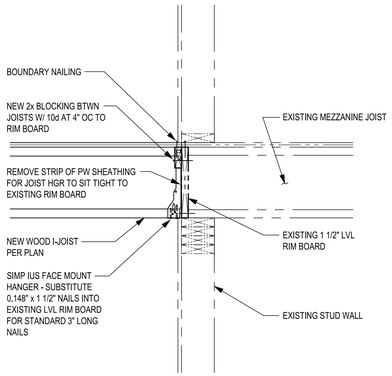
SECTION 1

1" = 1'-0" 03-001



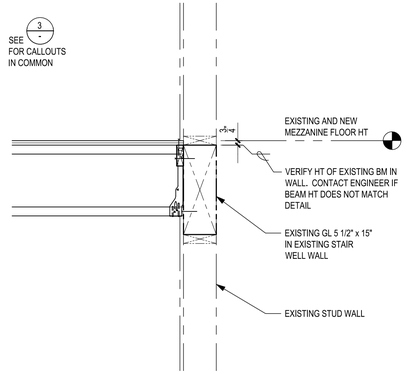
SECTION 2

1" = 1'-0" 03-001



SECTION 3

1" = 1'-0"



SECTION 4

1" = 1'-0"

SECTION 5

1" = 1'-0"

SECTION 6

1" = 1'-0"

SECTION 7

1" = 1'-0"

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

DATE: April 19, 2022 FILENAME: Q:\2022\2220189\20_STR\CAD\2220189S-30.dwg

PRCTI20220873

NELSON

Nelco Architecture, Inc.

1200 Fifth Ave.
Suite 1300
Seattle, WA 98101
Phone: (206) 408-8500
WWW.NELSONWORLDWIDE.COM

AHBL
TACOMA • SEATTLE • SPOKANE • TRI-CITIES

2215 North 30th Street, Suite 300 Tacoma, WA 98403
253.383.2422 TEL 253.383.2572 FAX www.ahbl.com WEB

CLIENT:

IDI Logistics

IDI LOGISTICS
840 APOLLO STREET, SUITE 343
EL SEGUNDO, CA 90245

PROJECT:

RED DOT OFFICE TI

2504 EAST MAIN AVENUE
PUYALLUP, WA 98372

Description:	No:	Date:
PERMIT SET		04/20/2022
PERMIT RESUBMITTAL		08/02/2022
PERMIT RESUBMITTAL		08/24/2022

SEAL:

CITY STAMP:

FRAMING DETAILS

Proj. No: 2220189.20 Reviewed By: DLB

S3.2