

**City of Puyallup
Building
ACCEPTED**

JMontgomery
08/16/2022
10:10:25 AM



THE APPROVED CONSTRUCTION PLANS,
DOCUMENTS AND ALL ENGINEERING MUST
BE POSTED ON THE JOB AT ALL
INSPECTIONS IN A VISIBLE AND READILY
ACCESSIBLE LOCATION.

FULL SIZED LEDGIBLE COLOR PLANS ARE
REQUIRED TO BE PROVIDED BY THE
PERMITEE ON SITE FOR INSPECTION

City of Puyallup Development & Permitting Services ISSUED PERMIT	
Building	Planning
Engineering	Public Works
Fire	Traffic



Approval of submitted plans is not an approval of omissions or oversights by this office or noncompliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

Structural Calculations

PREPARED FOR:

POE Construction
1519 W. Valley Hwy N
Auburn, WA 98001

Clay Johnson

PROJECT:

Red Dot TI - Puyallup Logistics Roof
Mounted VRF Support

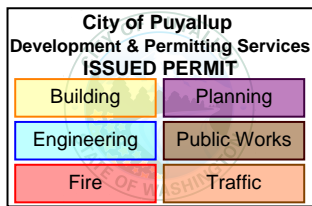
2220554.20

PREPARED BY:

Daniel L Booth, PE, SE
President

DATE:

June 2022



Structural Calculations

For



Red Dot TI - Puyallup Logistics Roof Mounted VRF Support

Puyallup, WA

Project # 2220554.20

Project Principal

Daniel L. Booth, PE, SE

Design Criteria

Design Codes and Standards

Codes and Standards: Structural design and construction shall be in accordance with the applicable sections of the following codes and standards as adopted and amended by the local building authority: International Building Code, 2018 Edition.

Structural Design Criteria:

Live Load Criteria:

Roof (Min Blanket Snow) 25 psf

Wind Load Criteria:

Ultimate Wind Speed 97 MPH

Risk Category II

Wind Exposure B

Topographic Factor 1.00

Seismic Criteria:

Risk Category II

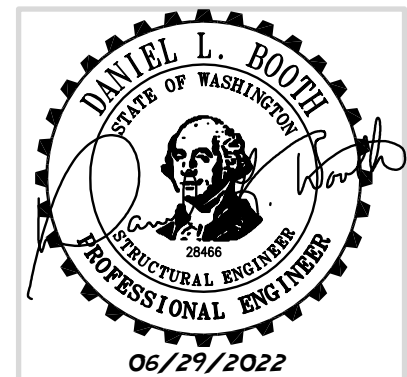
Seismic Importance Factor 1.00

$S_s = 1.258$ $S_{ds} = .433$

Site Class = D

Seismic Design Category = D

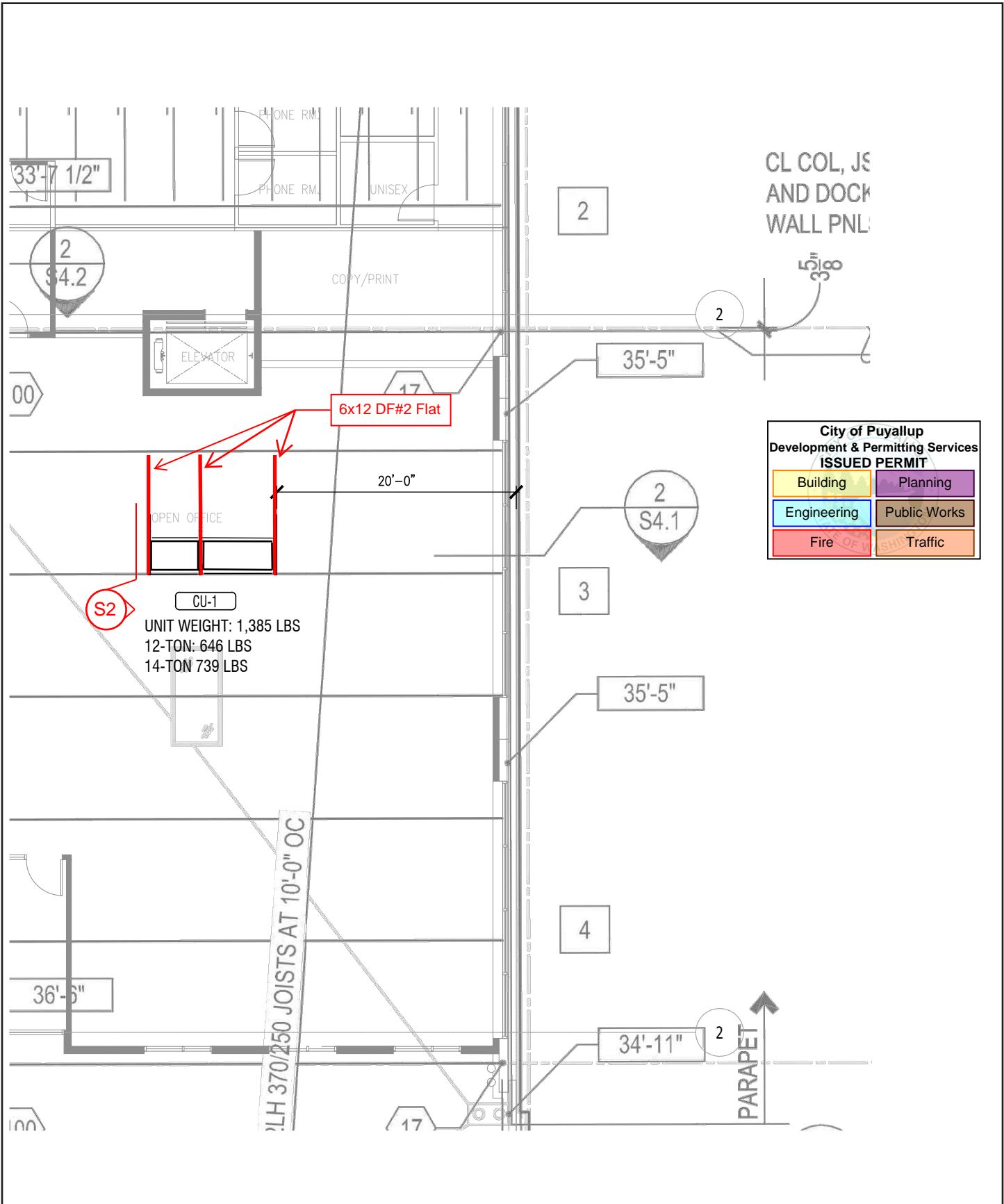
Seismic Design Per ASCE 7-16 Chapter 13



Project Description


The project consists of the design of support framing and anchorage for a 12-ton and 14-ton VRF rooftop mechanical units. Units to be supported on a single stand with a combined equipment weight of 1,385 lbs.

PRCTI20220873

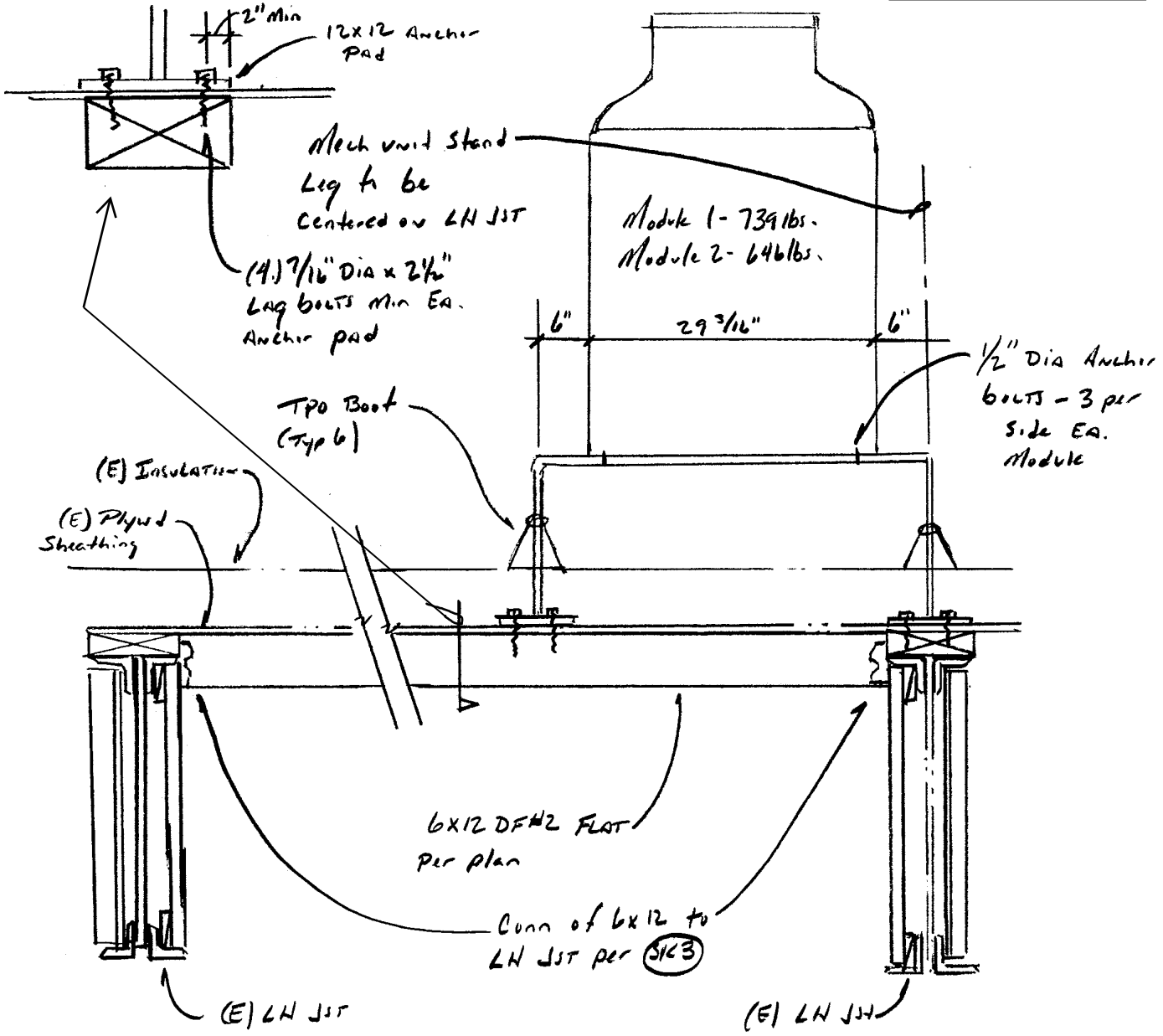


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Building	Planning
Engineering	Public Works
Fire	Traffic

 2215 North 30th Street, Suite 300 Tacoma, WA 98403 253.383.2422 TEL 253.383.2572 FAX	Red Dot TI Roof Mounted VRF Support		S1
	Plan		
	DRAWN BY: DLB	DATE: 6/28/22	

PRCTI20220873



2215 North 30th Street, Suite 300
 Tacoma, WA 98403
 253.383.2422 TEL 253.383.2572 FAX

Red Dot TI Roof Mounted VRF Support

Detail

DRAWN BY: DLB

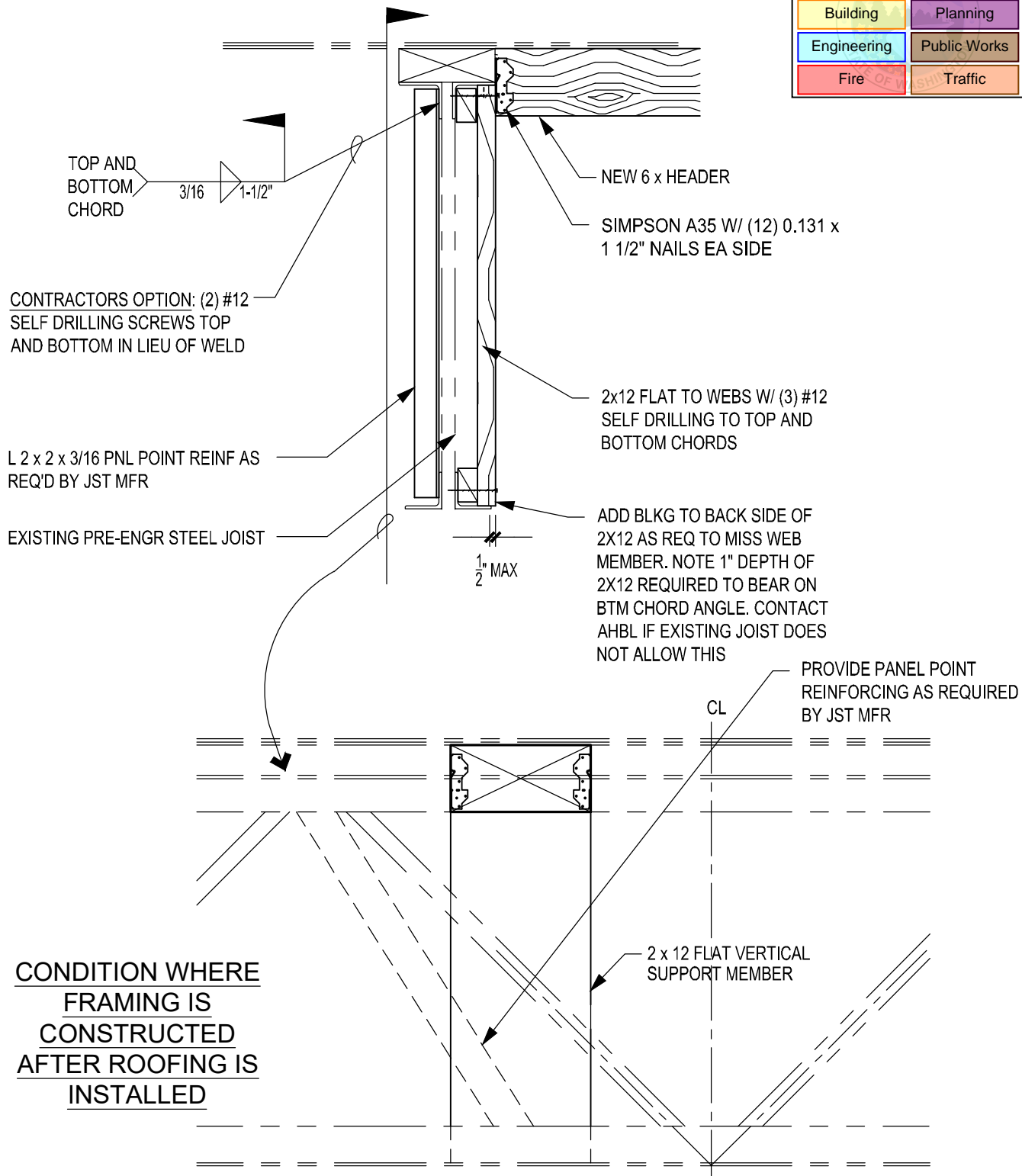
DATE: 6/28/22

JOB NO.: 2220544.20

S2

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Engineering	Public Works
Fire	Traffic



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Red Dot TI Roof Mounted VRF Support

Detail

DRAWN BY: DLB

DATE: 6/28/22

JOB NO.: 2220544.20

S3

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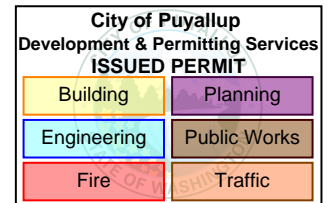
Search Information

Coordinates: 47.1911, -122.2609
Elevation: 55 ft
Timestamp: 2021-05-20T16:43:26.138Z
Hazard Type: Seismic
Reference Document: ASCE7-16
Risk Category: II
Site Class: D-default



Basic Parameters

Name	Value	Description
S _S	1.258	MCE _R ground motion (period=0.2s)
S ₁	0.433	MCE _R ground motion (period=1.0s)
S _{MS}	1.509	Site-modified spectral acceleration value
S _{M1}	* null	Site-modified spectral acceleration value
S _{DS}	1.006	Numeric seismic design value at 0.2s SA
S _{D1}	* null	Numeric seismic design value at 1.0s SA



* See Section 11.4.8

Additional Information

Name	Value	Description
SDC	* null	Seismic design category
F _a	1.2	Site amplification factor at 0.2s
F _v	* null	Site amplification factor at 1.0s
CR _S	0.914	Coefficient of risk (0.2s)
CR ₁	0.898	Coefficient of risk (1.0s)
PGA	0.5	MCE _G peak ground acceleration
F _{PGA}	1.2	Site amplification factor at PGA
PGA _M	0.6	Site modified peak ground acceleration
T _L	6	Long-period transition period (s)
SsRT	1.258	Probabilistic risk-targeted ground motion (0.2s)
SsUH	1.376	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
SsD	1.5	Factored deterministic acceleration value (0.2s)
S1RT	0.433	Probabilistic risk-targeted ground motion (1.0s)
S1UH	0.482	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
S1D	0.6	Factored deterministic acceleration value (1.0s)
PGA _d	0.5	Factored deterministic acceleration value (PGA)

* See Section 11.4.8

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Search Information

Coordinates: 47.1911, -122.2609
Elevation: 55 ft
Timestamp: 2021-05-20T16:46:53.955Z
Hazard Type: Wind



ASCE 7-16

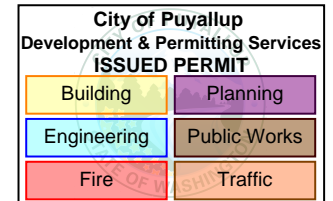
MRI 10-Year 67 mph
 MRI 25-Year 73 mph
 MRI 50-Year 78 mph
 MRI 100-Year 82 mph
 Risk Category I 92 mph
 Risk Category II 97 mph
 Risk Category III 104 mph
 Risk Category IV 108 mph

ASCE 7-10

MRI 10-Year 72 mph
 MRI 25-Year 79 mph
 MRI 50-Year 85 mph
 MRI 100-Year 91 mph
 Risk Category I 100 mph
 Risk Category II 110 mph
 Risk Category III-IV 115 mph

ASCE 7-05

ASCE 7-05 Wind Speed 85 mph



The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area – in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.

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Design Criteria

Unit Weights CU-1 $W_p := 1385 \text{ lbf}$
 Unit Dimensions Width $B := 117.81 \text{ in}$
 Length $L := 29.16 \text{ in}$
 Height $H := 71.625 \text{ in}$

Snow $SL := 25 \text{ psf}$

Seismic Risk Category II
 Site Class D
 Seismic Design Category D

$S_S := 1.258$ $F_a := 1.2$ per ASCE 7-16 Section 11.4.8
 $S_1 := 0.433$

$$S_{MS} := F_a \cdot S_S = 1.51$$

$$S_{DS} := \frac{2}{3} \cdot S_{MS} = 1.006$$

ASCE 7-16 Chapter 13

$a_p := 2.5$
 $R_p := 3.0$
 $I_p := 1.00$

Building Height $h := 34.95 \text{ ft}$
 Attachment Height $z := h = 34.95 \text{ ft}$

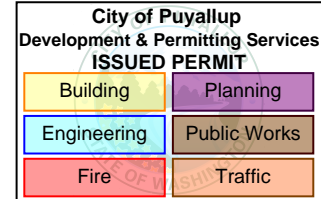
Seismic Force

$$F_{ph} := \frac{0.4 \cdot a_p \cdot S_{DS} \cdot W_p}{\left(\frac{R_p}{I_p}\right)} \cdot \left(1 + 2 \left(\frac{z}{h}\right)\right) = (1.394 \cdot 10^3) \text{ lbf}$$

$$F_{pmin} := 0.3 \cdot S_{DS} \cdot I_p \cdot W_p = 418.159 \text{ lbf}$$

$$F_{pmax} := 1.6 \cdot S_{DS} \cdot I_p \cdot W_p = (2.23 \cdot 10^3) \text{ lbf}$$

$$F_{pv} := 0.2 \cdot W_p = 277 \text{ lbf}$$



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Design Criteria

Wind

ASCE 7-16 Chapter 29.4.1

Exposure Category B

$$V := 97 \text{ mph}$$

$$K_z := 0.74$$

$$K_{zt} := 1.00$$

$$K_d := 0.85$$

$$K_e := 1.00$$

$$q_h := 0.00256 \cdot K_z \cdot K_{zt} \cdot K_d \cdot K_e \cdot V^2 = 15.151$$

$$q_h := q_h \cdot \text{psf} = 15.151 \text{ psf}$$

$$A_{fh} := B \cdot H = 58.598 \text{ ft}^2$$

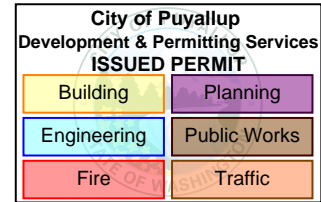
$$GC_{fh} := 1.9$$

$$F_h := q_h \cdot (GC_{fh}) \cdot A_{fh} = (1.687 \cdot 10^3) \text{ lbf}$$

$$A_{fv} := B \cdot L = 23.857 \text{ ft}^2$$

$$GC_{fv} := 1.5$$

$$F_v := q_h \cdot (GC_{fv}) \cdot A_{fv} = 542.166 \text{ lbf}$$



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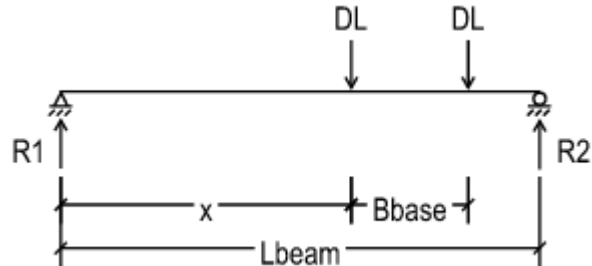
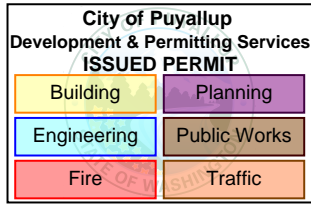
Beam

$$L_{beam} := 10 \text{ ft}$$

$$B_{base} := 3.43 \text{ ft}$$

$$x := 6.57 \text{ ft}$$

$$DL := \frac{W_p}{2} = 692.5 \text{ lbf}$$



Beam Reactions

$$R_2 := \frac{DL \cdot x + DL \cdot (x + B_{base})}{L_{beam}} = (1.147 \cdot 10^3) \text{ lbf}$$

$$R_1 := 2 \cdot DL - R_2 = 237.528 \text{ lbf}$$

Per EOR shell drawings, steel joists designed for additional 1000 lb point load at any panel point along joist top chord

if ($R_1 < 1000 \text{ lbf}$, "OK", "NO GOOD ") = "OK"

if ($R_2 < 1000 \text{ lbf}$, "OK", "NO GOOD ") = "NO GOOD "

See hand calculation analysis - Joist acceptable for point load

Connection

$$h_{cent} := \frac{H}{2} + 18 \text{ in} = 4.484 \text{ ft}$$

$$W_p = (1.385 \cdot 10^3) \text{ lbf}$$

$$F_{pv} = 277 \text{ lbf}$$

$$F_v = 542.166 \text{ lbf}$$

$$F_{ph} = (1.394 \cdot 10^3) \text{ lbf}$$

$$F_h = (1.687 \cdot 10^3) \text{ lbf}$$

0.6D+0.7E

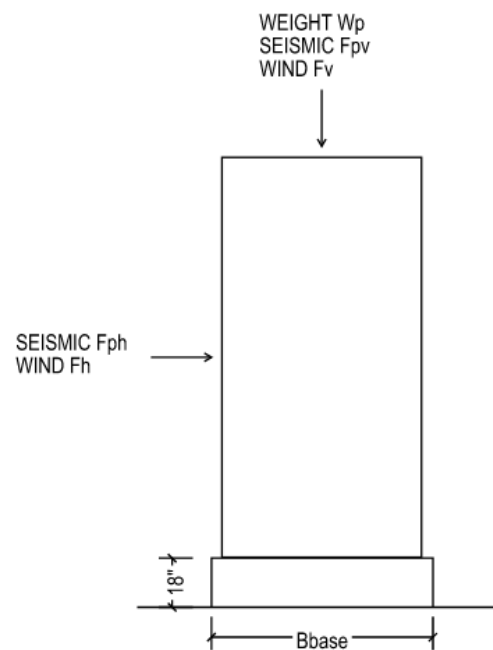
$$M_o := 0.7 \cdot F_{ph} \cdot h_{cent} = (4.375 \cdot 10^3) \text{ lbf} \cdot \text{ft}$$

$$M_r := 0.6 \cdot W_p \cdot \frac{B_{base}}{2} - 0.7 \cdot F_{pv} \cdot \frac{B_{base}}{2} = (1.093 \cdot 10^3) \text{ lbf} \cdot \text{ft}$$

$$M_{net} := M_o - M_r = (3.283 \cdot 10^3) \text{ lbf} \cdot \text{ft}$$

Uplift Force

$$F_u := \frac{M_{net}}{B_{base}} = 957.084 \text{ lbf}$$



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Connection

0.6D+0.6W

$$M_o := F_h \cdot h_{cent} = 7564.39 \text{ lbf} \cdot \text{ft}$$

$$F_u := \frac{M_o}{B_{base}} = (2.205 \cdot 10^3) \text{ lbf}$$

Uplift Force

$$F_{vnet} := 0.6 F_u + \frac{0.6 F_v}{2} - \frac{0.6 W_p}{2} = (1.07 \cdot 10^3) \text{ lbf}$$

Horizontal Force

$$F_{hnet} := \frac{F_h}{2} = 843.416 \text{ lbf}$$

Force Combination

$$F_{net} := \sqrt{F_{vnet}^2 + F_{hnet}^2} = (1.363 \cdot 10^3) \text{ lbf}$$

$$\alpha := \text{atan} \left(\frac{F_{vnet}}{F_{hnet}} \right) = 51.763 \text{ deg}$$

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Curb to Structure Attachment

7/16" Dia Lag Screw - 2" Penetration to Wood Member

Load Duration Factor $C_D := 1.6$

$$\text{Withdrawal } W_{screw} := C_D \cdot 342 \frac{\text{lbf}}{\text{in}} \cdot 2 \text{ in} = 1094.4 \text{ lbf}$$

$$\text{Shear } Z_{perp} := C_D \cdot 190 \text{ lbf} = 304 \text{ lbf}$$

$$Z_a := \frac{W_{screw} \cdot Z_{perp}}{W_{screw} \cdot (\sin(\alpha))^2 + Z_{perp} \cdot (\cos(\alpha))^2} = 420.268 \text{ lbf}$$

Uplift Force at Each Leg

$$F := \frac{F_{net}}{2} = 681.366 \text{ lbf}$$

(2) Screws per Leg

if $(F < 2 \cdot Z_a, \text{"OK"}, \text{"NO GOOD"}) = \text{"OK"}$

Unit to Stand Attachment - 1/2" Dia Bolt

$$A_b := \pi \cdot (0.25 \text{ in})^2 = 0.196 \text{ in}^2$$

$$F_{nt} := 45 \text{ ksi}$$

$$\Omega := 2.00$$

$$R_n := \frac{F_{nt} \cdot A_b}{\Omega} = 4417.865 \text{ lbf}$$

if $(F_{net} < R_n, \text{"OK"}, \text{"NO GOOD"}) = \text{"OK"}$

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Civil Engineers

Structural Engineers

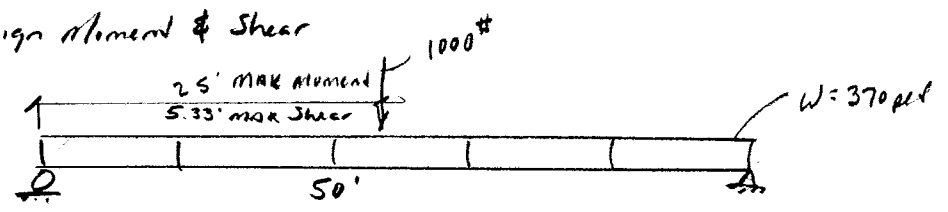
Landscape Architects

Community Planners

Land Surveyors

Review of JST LOADS

Design Moment & Shear

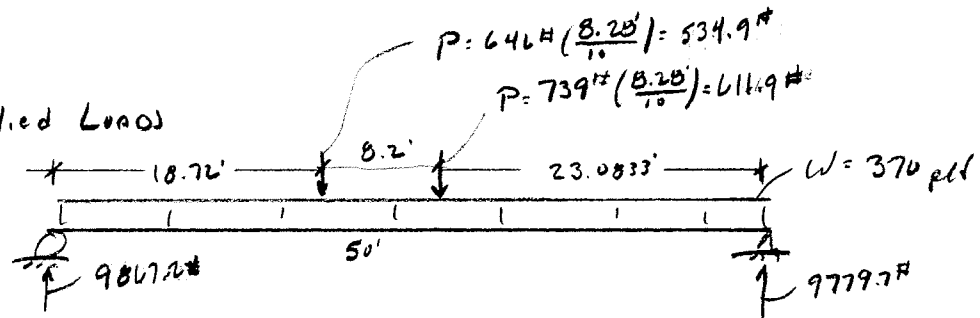


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Max Moment $M = 370 \text{ plf} \left(\frac{50'}{8} \right) + \frac{1000 (50')}{4} = 128.1 \text{ k}'$

Max Shear $V = 370 \text{ plf} \left(\frac{50'}{2} \right) + \frac{1000 (44.67')}{5} = 10.14 \text{ k}$

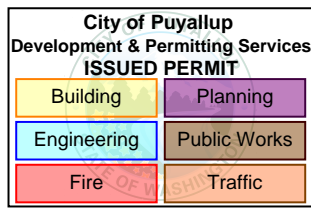
Applied Loads



Max Shear = $9867.2 \text{ lbs} < 10,140 \text{ lbs}$ O.K.

Max Moment = $9779.7 \text{ lbs} (24.778') - 370 \text{ plf} \left(\frac{24.778'}{2} \right)^2 - 611.9 \text{ lbs} (1.695')$
 $= 127,703.6 \text{ ft-lb} < 128,100 \text{ ft-lb}$ O.K.

Existing LH JST Adequate for Support of New Main Unit.



Project Title: Dawn Foods Rooftop Mechanical TI
 Engineer: BN
 Project ID: 2170393.20
 Project Descr:

Wood Beam

Project File: panattoni corp center.ec6

LIC# : KW-06014847, Build:20.22.4.26

AHBL, INC

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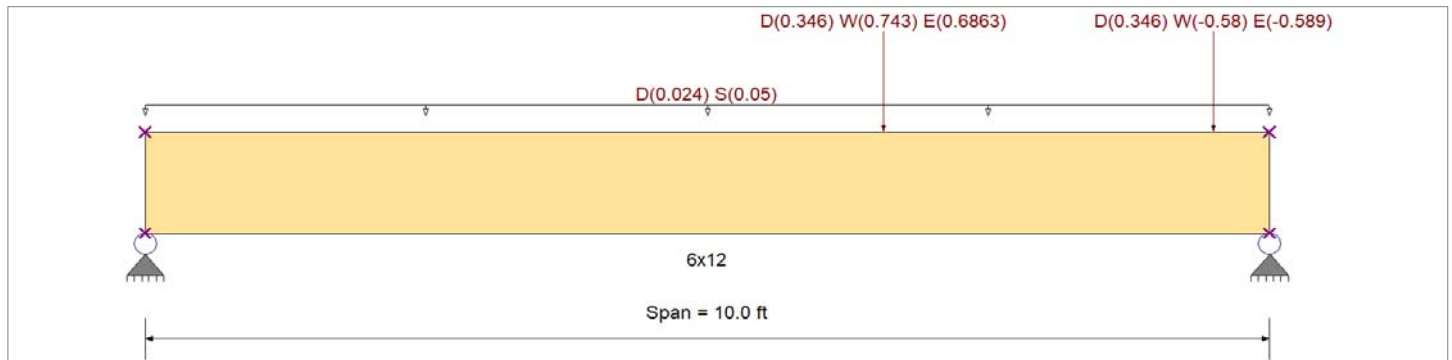
DESCRIPTION: Mechanical Unit Support - CU-1

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16
 Load Combination Set : ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb +	900.0 psi	E : Modulus of Elasticity	
Load Combination : ASCE 7-16	Fb -	900.0 psi	Ebend- xx	1,600.0ksi
	Fc - Prll	1,350.0 psi	Eminbend - xx	580.0ksi
Wood Species : Douglas Fir - Larch	Fc - Perp	625.0 psi		
Wood Grade : No.2	Fv	180.0 psi		
	Ft	575.0 psi	Density	31.210pcf
Beam Bracing : Completely Unbraced				



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
 Point Load : D = 0.3460, W = 0.7430, E = 0.6863 k @ 6.570 ft
 Uniform Load : D = 0.0240, S = 0.050, Tributary Width = 1.0 ft, (Roof Loads)
 Point Load : D = 0.3460, W = -0.580, E = -0.5890 k @ 9.50 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.376	1	Maximum Shear Stress Ratio	=	0.065	: 1
Section used for this span		6x12		Section used for this span		6x12	
fb: Actual	=	389.44	psi	fv: Actual	=	13.41	psi
Fb: Allowable	=	1,035.00	psi	Fv: Allowable	=	207.00	psi
Load Combination		+D+S		Load Combination		+D+S	
Location of maximum on span	=	6.533	ft	Location of maximum on span	=	9.051	ft
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	
Maximum Deflection							
Max Downward Transient Deflection		0.080	in	Ratio =	1505	>=360	Span: 1 : W Only
Max Upward Transient Deflection		0	in	Ratio =	0	<360	n/a
Max Downward Total Deflection		0.155	in	Ratio =	774	>=180	Span: 1 : +D+0.750S+0.5250E
Max Upward Total Deflection		0	in	Ratio =	0	<180	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios								Moment Values			Shear Values						
			M	V	C _d	C _{F/V}	C _i	C _r	C _m	C _t	C _L	M	fb	F'b	V	fv	F'v			
D Only	Length = 10.0 ft	1	0.337	0.053	0.90	1.000	1.00	1.00	1.00	1.00	1.00	1.32	272.83	810.00	0.00	0.00	0.00	0.00	0.00	162.00
+D+S	Length = 10.0 ft	1	0.376	0.065	1.15	1.000	1.00	1.00	1.00	1.00	1.00	1.88	389.44	1035.00	0.00	0.00	0.00	0.00	0.00	0.00
+D+0.750S	Length = 10.0 ft	1	0.348	0.059	1.15	1.000	1.00	1.00	1.00	1.00	1.00	1.74	360.29	1035.00	0.00	0.00	0.00	0.00	0.00	0.00
+D+0.60W	Length = 10.0 ft	1	0.317	0.055	1.60	1.000	1.00	1.00	1.00	1.00	1.00	2.21	457.08	1440.00	0.00	0.00	0.00	0.00	0.00	0.00
+D+0.450W	Length = 10.0 ft	1	0.285	0.049	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.99	411.02	1440.00	0.00	0.00	0.00	0.00	0.00	0.00
+D+0.750S+0.450W	Length = 10.0 ft	1				1.000	1.00	1.00	1.00	1.00	1.00				0.00	0.00	0.00	0.00	0.00	0.00

PRCTI20220873

Wood Beam

Project File: panattoni corp center.ec6

LIC# : KW-06014847, Build:20.22.4.26

AHBL, INC

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DESCRIPTION: Mechanical Unit Support - CU-1

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios									Moment Values			Shear Values		
			M	V	C _d	C _{F/V}	C _i	C _r	C _m	C _t	C _L	M	fb	F'b	V	f _v	F'v
Length = 10.0 ft	1	0.346	0.062	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.00	2.41	498.48	1440.00	0.75	17.73	288.00
+0.60D+0.60W					1.000	1.00	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 10.0 ft	1	0.242	0.043	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.68	347.95	1440.00	0.53	12.50	288.00
+D+0.70E					1.000	1.00	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 10.0 ft	1	0.326	0.057	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.00	2.27	468.85	1440.00	0.70	16.55	288.00
+D+0.750S+0.5250E					1.000	1.00	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 10.0 ft	1	0.352	0.063	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.00	2.45	507.30	1440.00	0.77	18.19	288.00
+0.60D+0.70E					1.000	1.00	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 10.0 ft	1	0.250	0.046	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.74	359.72	1440.00	0.55	13.12	288.00

Overall Maximum Deflections

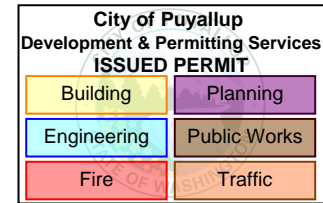
Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+0.750S+0.5250E	1	0.1549	5.255		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	0.620	0.995
Overall MINimum	0.206	-0.063
D Only	0.325	0.745
+D+S	0.575	0.995
+D+0.750S	0.512	0.932
+D+0.60W	0.460	0.707
+D+0.450W	0.426	0.716
+D+0.750S+0.450W	0.614	0.904
+0.60D+0.60W	0.330	0.409
+D+0.70E	0.469	0.669
+D+0.750S+0.5250E	0.620	0.875
+0.60D+0.70E	0.339	0.371
S Only	0.250	0.250
W Only	0.226	-0.063
E Only	0.206	-0.109



Job Name:

System Reference:

Date:

208/230V OUTDOOR VRF HEAT RECOVERY SYSTEM



UNIT OPTION

Standard Model.....**PURY-P312TSNU-A**
 Seacoast (BS) Model.....**PURY-P312TSNU-A-BS**

ACCESSORIES

Twinning Kit (Required).....**CMY-R300NCBK**
 BC Controller (Required).....for details see BC Controller Submittals
 Joint Kit.....for details see Pipe Accessories Submittal
 Panel Heater Kit.....for details see Panel Heater Kit Submittal
 Snow/Hail Guards Kit.....for details see Snow/Hail Guards Kit Submittal

Specifications		System	
Unit Type		PURY-P312TSNU-A(-BS)	
Cooling Capacity (Nominal)	BTU/H	312,000	
Heating Capacity (Nominal)	BTU/H	350,000	
Net Weight	Lbs. [kg]	1,385 [628]	
Refrigerant Piping Diameter	Liquid (High Pressure)	In. [mm]	1-1/8 [28.58] Braze
	Gas (Low Pressure)	In. [mm]	1-5/8 [41.28] Braze
Max. Total Refrigerant Line Length	Ft.	3,116	
Max. Refrigerant Line Length (Between ODU & IDU)	Ft.	541	
Max. Control Wiring Length	Ft.	1,640	
Indoor Unit Connectable	Total Capacity	50.0~150.0% of outdoor unit capacity	
	Model/Quantity	P04~P96/2.0~50.0	
Sound Pressure Levels	dB(A)	67.0/69.0	
Sound Power Levels	dB(A)	87.0/88.5	
Compressor Operating Range		7.5% to 100.0%	
AHRI Ratings (Ducted/Non-ducted)	EER	10.1/10.2	
	IEER	21.4/20.5	
	COP	3.2/3.36	
	SCHE	20.6/23.8	

Specifications		Module 1		Module 2	
Unit Type		PURY-P168TNU-A(-BS)		PURY-P144TNU-A(-BS)	
Cooling Capacity (Nominal)	BTU/H	168,000		144,000	
Heating Capacity (Nominal)	BTU/H	188,000		160,000	
Guaranteed Operating Range ¹	Cooling ²	23~126 [-5.0~52.0]		23~126 [-5.0~52.0]	
	Heating	-13~60 [-25.0~15.5]		-13~60 [-25.0~15.5]	
Extended Operating Range	Heating	-18.0~60 [-18.0~15.5]		-18.0~60 [-18.0~15.5]	
	Heating	-18.0~60 [-18.0~15.5]		-18.0~60 [-18.0~15.5]	
External Dimensions (H x W x D)	In. [mm]	71-5/8 x 68-15/16 x 29-3/16 [1,818 x 1,750 x 740]		71-5/8 x 48-7/8 x 29-3/16 [1,818 x 1,240 x 740]	
Net Weight	Lbs. [kg]	739 [335]		646 [293]	
External Finish		Pre-coated galvanized steel sheet (+powder coating for -BS type) [MUNSELL 5Y 8/1]		Pre-coated galvanized steel sheet (+powder coating for -BS type) [MUNSELL 5Y 8/1]	
Electrical Power Requirements	Voltage, Phase, Hertz, Power Tolerance	208/230V, 3-phase, 60 Hz, ±10%		208/230V, 3-phase, 60 Hz, ±10%	
Minimum Circuit Ampacity	A	61.0/57.0		52.0/48.0	
Maximum Overcurrent Protection	A	100/90		80/70	
Recommended Fuse Size	A	70/70		60/60	
Recommended Minimum Wire Size	AWG [mm]	4/4 [21.2/21.2]		4/4 [21.2/21.2]	
SCCR	kA	5		5	
FAN ⁴	Type x Quantity	Propeller fan x 2		Propeller fan x 2	
	Airflow Rate	CFM	14,850	9,550	
	External Static Pressure	In. WG	0.00, 0.12, 0.24, 0.32, In. WG; factory set to 0 In. WG	0.00, 0.12, 0.24, 0.32, In. WG; factory set to 0 In. WG	
Compressor	Type x Quantity	Inverter scroll hermetic compressor x 1		Inverter scroll hermetic compressor x 1	
Refrigerant	Type x Original Charge	R410A x 23 lbs + 12.0 oz [10.8 kg]		R410A x 23 lbs + 12.0 oz [10.8 kg]	
Protection Devices	High Pressure Protection	High pressure sensor, High pressure switch at 4.15 MPa (601 psi)		High pressure sensor, High pressure switch at 4.15 MPa (601 psi)	
	Inverter Circuit (Comp./Fan)	Over-heat protection, Over-current protection		Over-heat protection, Over-current protection	

NOTES:
 Nominal cooling conditions (Test conditions are based on AHRI 1230)
 Indoor: 80°FDB./67°FWB. (26.7°CDB./19.4°CWB.), Outdoor: 95°FDB. (35°CDB.)
 Nominal heating conditions (Test conditions are based on AHRI 1230)
 Indoor: 70°FDB. (21.1°CDB.), Outdoor: 47°FDB./43°FWB. (8.3°CDB./6.1°CWB.)

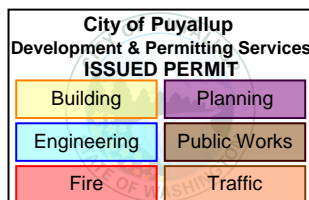
¹Harsh weather environments may demand performance enhancing equipment. Ask your Mitsubishi Electric representative for more details about your region

²For details on extended cooling operation range down to -10° F DB, see Low Ambient Kit Submittal

³When applying product below -4°F, consult your design engineer for cold climate application best practices, including the use of a backup source for heating

⁴Unit will continue to operate in extended operating range, but capacity is not guaranteed

Each individual module requires a separate electrical connection. Refer to electrical data for each individual module.

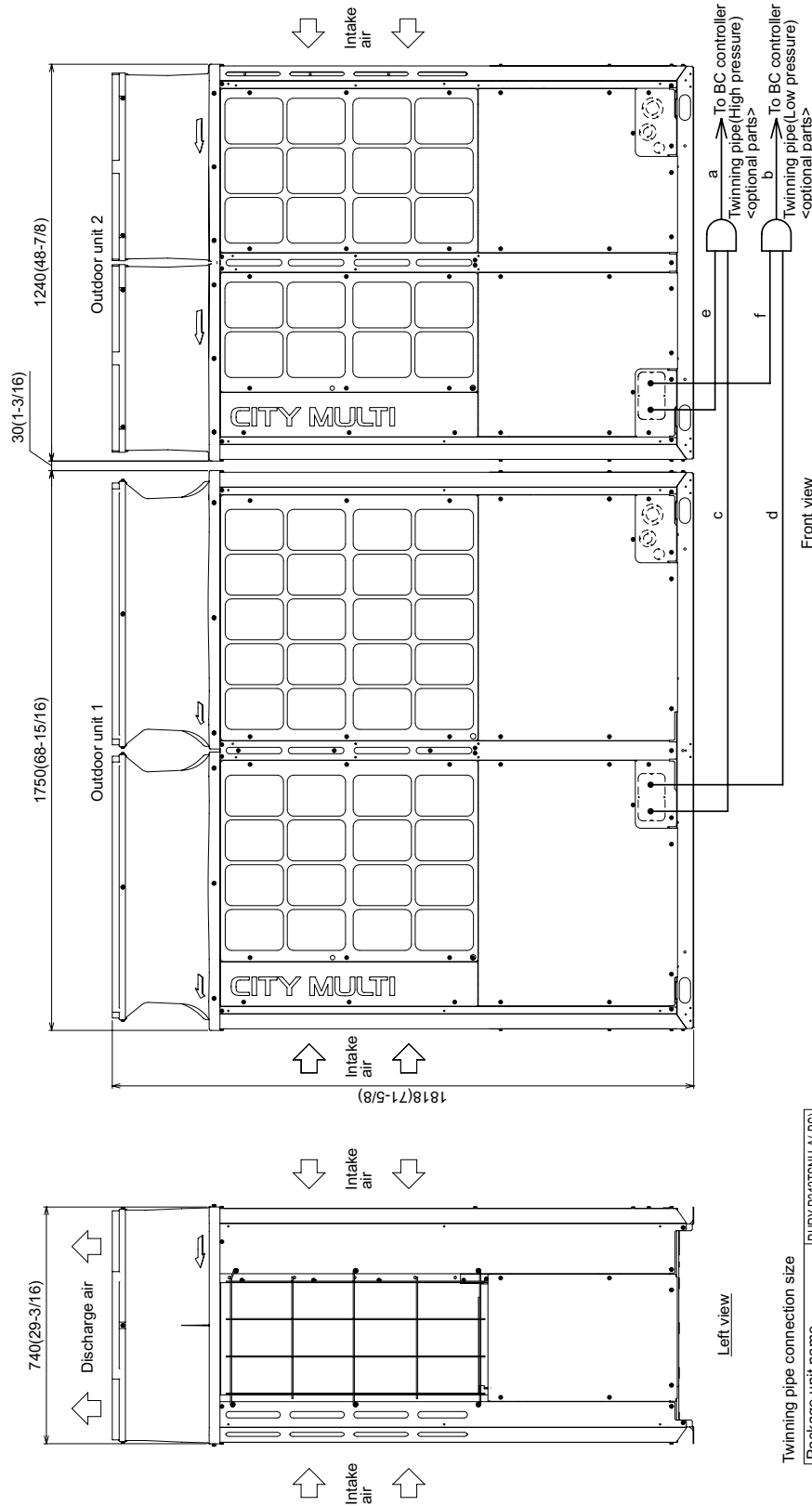


PRCTI20220873

OUTDOOR UNIT: PURY-P312TSNU-A-(BS) – DIMENSIONS

PURY-P312TSNU-A-(BS)

Unit: mm(in)



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Fire	Traffic

Twinning pipe-Outdoor unit	Unit model	High pressure c or e	Low pressure d or f
P144	P144	ø22.2(7/8)	ø28.58(1-1/8)
P168	P168	ø22.2(7/8)	ø28.58(1-1/8)

Package unit name	PURY-P312TSNU-A-(BS)
Outdoor unit 1	PURY-P168TNU-A-(BS)
Outdoor unit 2	PURY-P144TNU-A-(BS)
Outdoor Twinning kit(optional parts)	CNY-R30JNCBK
BC controller	ø28.58(1-1/8)
-Twinning pipe	ø41.28(1-5/8)

- Note 1. Connect the pipes as shown in the figure above. Refer to the table above for the pipe size.
 2. Twinning pipes should not be tilted more than 15 degrees from the horizontal plane.
 Be sure to see the Installation Manual for details of twinning pipe installation.
 3. The pipe section before the Twinning pipe (section "a" and "b" in the figure) must have at least 500mm(19-11/16) of straight section (*including the straight pipe that is supplied with the Twinning pipe).
 4. Only use the Twinning pipe by Mitsubishi (optional parts).

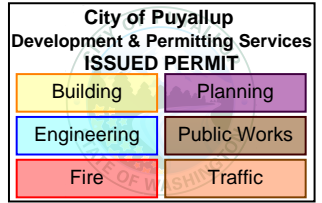
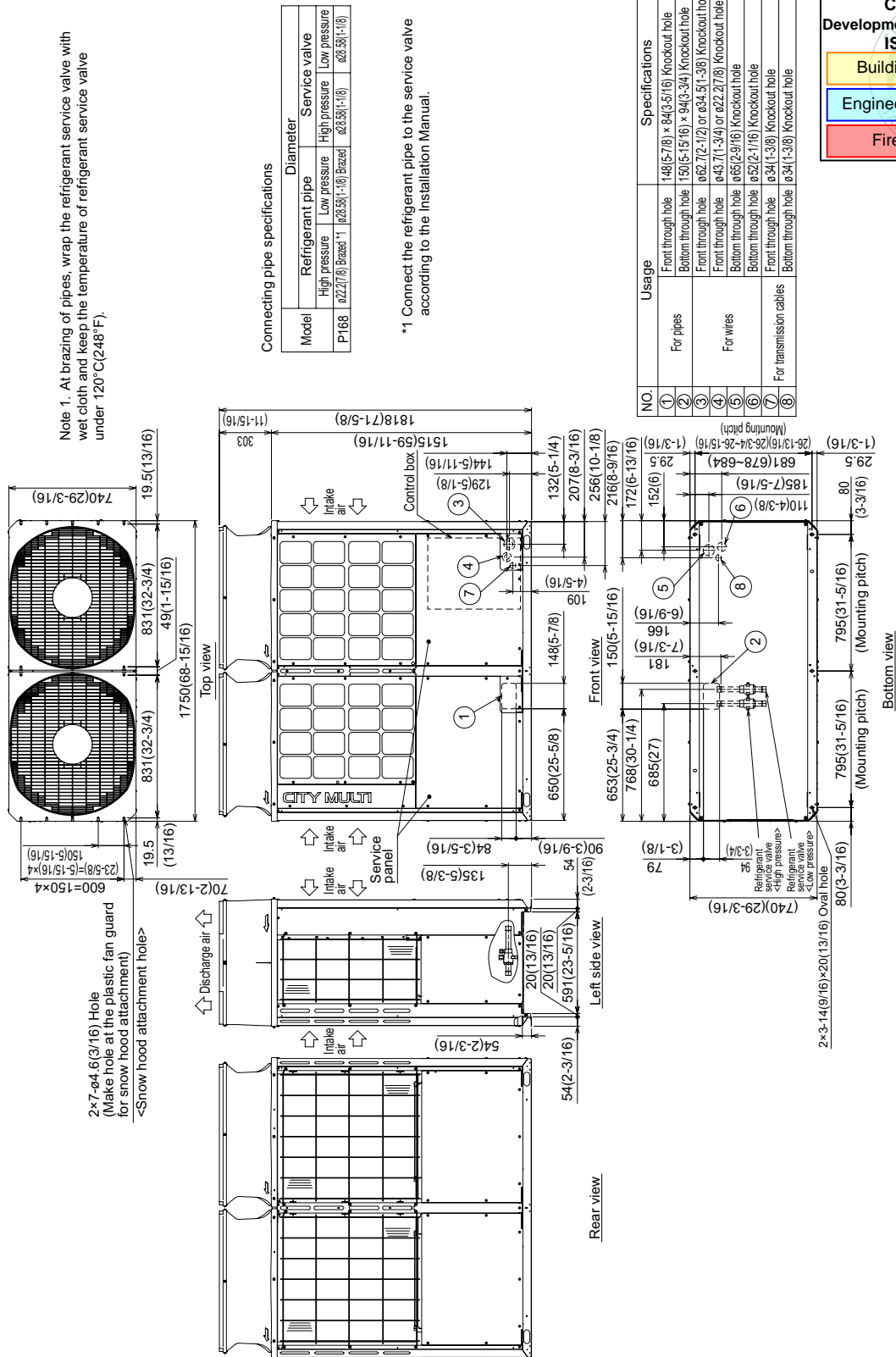
NOTES:
SEACOAST PROTECTION
 Anti-corrosion Protection: A coating treatment is applied to condenser coil for protection from air contaminants.
 Standard: Salt Spray Test Method - no unusual rust development to 480 hours.
 Sea Coast (BS): Salt Spray Test Method (JRA 9002) - no unusual rust development to 960 hours.

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MODULE 1: PURY-P168TNU-A(-BS) – DIMENSIONS

PURY-P168TNU-A(-BS)

Unit: mm(in)



NOTES:
SEACOAST PROTECTION
Anti-corrosion Protection: A coating treatment is applied to condenser coil for protection from air contaminants.
Standard: Salt Spray Test Method - no unusual rust development to 480 hours.
Sea Coast (BS): Salt Spray Test Method (JRA 9002) - no unusual rust development to 960 hours.

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MODULE 2: PURY-P144TNU-A(-BS) – DIMENSIONS

PURY-P96,120,144TNU-A(-BS)

Unit: mm(in)

Note 1. At brazing of pipes, wrap the refrigerant service valve with wet cloth and keep the temperature of refrigerant service valve under 120°C(248°F).

Model	Refrigerant pipe		Service valve	
	High pressure	Low pressure	High pressure	Low pressure
P96	φ19.05(3/4) Brazed*1	φ22.2(7/8) Brazed*1	φ28.5(1-1/8)	φ28.5(1-1/8)
P120	φ19.05(3/4) Brazed*1	φ22.2(7/8) Brazed*1	φ28.5(1-1/8) Brazed	φ28.5(1-1/8)
P144	φ22.2(7/8) Brazed*1	φ28.5(1-1/8) Brazed*1	φ28.5(1-1/8) Brazed	φ28.5(1-1/8)

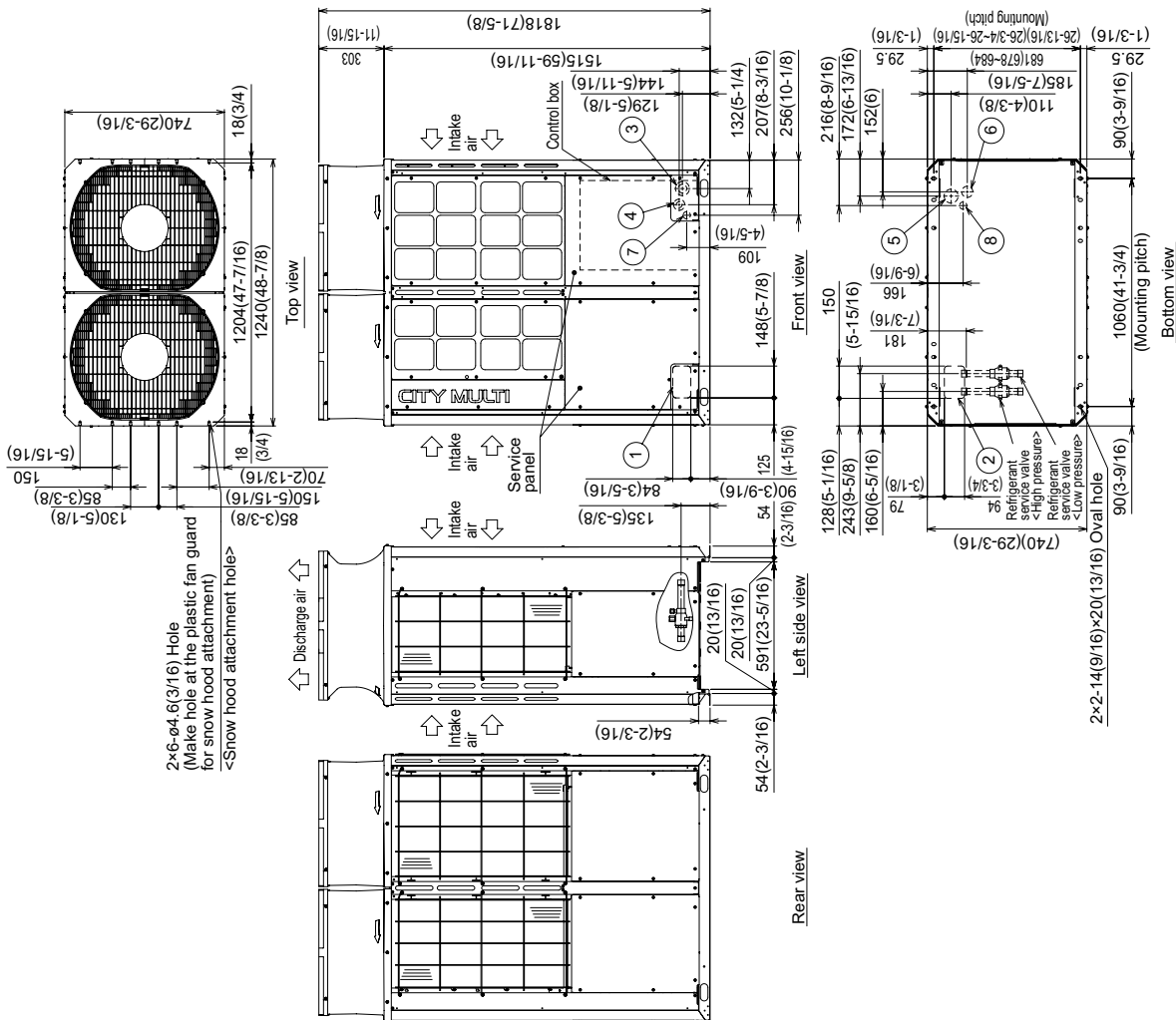
*1 Connect the refrigerant pipe to the service valve according to the Installation Manual.

Connecting pipe specifications

NO.	Usage	Specifications
①	Front through hole	148(5-7/8) × 84(3-5/16) Knockout hole
②	Bottom through hole	150(5-15/16) × 84(3-5/16) Knockout hole
③	Front through hole	φ62.7(2-1/2) or φ34.5(1-3/8) Knockout hole
④	Front through hole	φ43.7(1-3/4) or φ22.2(7/8) Knockout hole
⑤	Bottom through hole	φ65(2-9/16) Knockout hole
⑥	Bottom through hole	φ52(2-1/16) Knockout hole
⑦	Front through hole	φ34(1-3/8) Knockout hole
⑧	Bottom through hole	φ34(1-3/8) Knockout hole

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Fire	Traffic



NOTES:
SEACOAST PROTECTION

Anti-corrosion Protection: A coating treatment is applied to condenser coil for protection from air contaminants.
Standard: Salt Spray Test Method - no unusual rust development to 480 hours.
Sea Coast (BS): Salt Spray Test Method (JRA 9002) - no unusual rust development to 960 hours.

Specifications are subject to change without notice.

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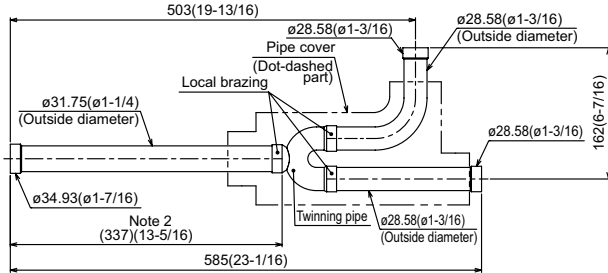
PRCTI20220873

TWINNING KIT: CMY-R300NCBK – DIMENSIONS

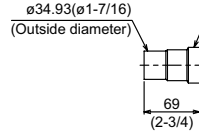
CMY-R300NCBK

Unit: mm (in.)

Low-pressure twinning pipe

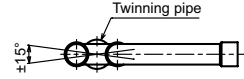


<Deformed pipe(Accessory)>



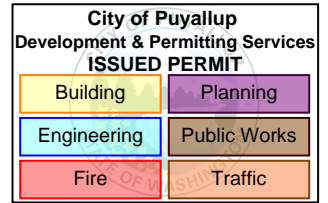
Note:

1. Refer to the figure below for the installation position of the twinning pipe.

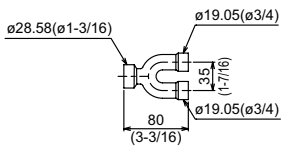


Slope of the twinning pipes are at an angle within $\pm 15^\circ$ to the horizontal plane.

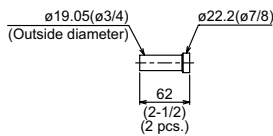
2. Use the attached pipe to braze the port-opening of the twinning pipe.
3. Pipe diameter is indicated by inside diameter.



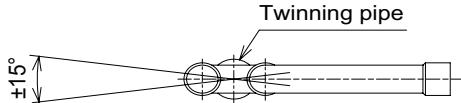
High-pressure twinning pipe



<Deformed pipe(Accessory)>



Note 1. Reference the attitude angle of the twinning pipe below the fig.



The angle of the twinning pipe is within $\pm 15^\circ$ against the horizontal plane.

2. Use the attached pipe to braze the port-opening of the twinning pipe.
3. Pipe diameter is indicated by inside diameter.
4. Only use the Twinning pipe by Mitsubishi (optional parts) .

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Toll Free: 800-433-4822 www.mehvac.com

FORM# PURY-P312TSNU-A - 202204



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