# STORM DRAINAGE REPORT <br> FOR <br> <br> Olson Brothers Storage <br> <br> Olson Brothers Storage Puyallup, Washington 

 Puyallup, Washington}

Revised July 2022 February 2021

Prepared for:<br>Mike Grimit<br>Grimit Architecture<br>Prepared by:<br>Daniel Smith, P.E., Project Manager<br>Approved By:<br>Daniel Smith, P.E., Project Manager



REPORT \#20083
"I hereby state that this Drainage and Erosion/Sediment Control Plan for the Olson Brothers Storage project has been prepared by me or under my supervision and meets the standard of care and expertise which is usual and customary in this community of professional engineers. I understand that City of Puyallup does not and will not assume liability for the sufficiency, suitability or performance of drainage facilities prepared by me."

This analysis is based on data and records either supplied to, or obtained by, C.E.S. NW, Inc. These documents are referenced within the text of the analysis. The analysis has been prepared utilizing procedures and practices within the standard accepted practices of the industry.
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## StORM DRAINAGE

## 1. Project Overview

This report accompanies the civil engineering plans as submitted to the City of Puyallup for review and approval. This document provides site information, and the analysis used to prepare the storm drainage design. The Washington State Department of Ecology Stormwater Management Manual for Western Washington, 2014 (Manual), and the City of Puyallup's modifications to that document establishes the methodology and design criteria used for this project.

The Olson Brothers Storage project proposes the remodel of an existing commercial industrial building and portable building on a 1.86 -acre site comprised of three parcels (2105200180, 2105200191 and 2105200192) zoned Limited Manufacturing (ML). The site is accessed from Inter Ave with a new commercial driveway approach. The project is located in the Puyallup River Water Resources Inventory Area (WRIA 10). The Vicinity Map has been included in Appendix ' A ' of this report. A project summary is as follows:

Permit Applied for - Building Permit and Site Development
Address - 2511 Inter Ave Puyallup, WA 98372
Parcel Numbers - 2105200180, 2105200192 and 2105200192
Legal description - Parcel A, TPN. 2105200191

The west 85 feet of the south 120 feet of the west half of tract 10 of Ackerson's Second Addition to Puyallup, according to the map thereof recorded in volume 8 of plats, page 25 , records of Pierce County, Washington.

Parcel B, TPN. 2105200192

The west half of Block 10 of Ackerson's Second Addition to Puyallup, according to the plat thereof recorded in Volume 8 of Plats, page 25, records of Pierce County, Washington.

Except the south 120 feet of the west 85 feet thereof.

The east one-half of Block 10 of Ackerson's Second Addition to Puyallup, according to the plat thereof recorded in Volume 8 of Plats, page 25, records of Pierce County, Washington.

All situate in the County of Pierce, State of Washington.

The project site has historically been used as a commercial property with an converted single family residence, storage building, portable building and existing gravel lot as depicted on the predeveloped basin map (Appendix ' B '). The existing site is to be redeveloped with landscaping and pave the existing gravel parking with permeable pavement (BMP T5.15) when construction permits are approved. The property has frontage along Inter Ave which provides access with a new commercial driveway approach. Improvements are proposed along Inter Ave which include curb, gutter and sidewalk extended across the property's frontage. The project site proposes approximately 31,632 sq.ft. of paving, that does not include overlaying the existing asphalt, across onsite and offsite improvements and 10,432 sq.ft. of landscaping; therefore, according to Figure 2.4.1 and 2.4.2 of Volume I of the Manual, the project must evaluate all minimum requirements for the new and replaced surfaces; see Section 5 of this report for a detailed discussion of the minimum requirements. The project proposes permeable pavement for flow control of the newly paved and landscaped surfaces. Runoff treatment is provided by the native soils underlying the permeable pavement since they meet the CEC and organic requirements of Section 4.4.2 of Volume V of the Manual. All disturbed areas which are not converted to impervious surface will apply soil amendments per BMP T5.13. native soils meets standards shall be submitted. [R2-03-20083 Storm Drainage Report, Page 4/100]

The existing site's current use is a commercially converted single family residence, commercial storage building and gravel paved storage yard. The site is relatively flat between elevations 6162 (NAVD 88) which gradually slopes towards Inter Avenue. Stormwater runoff from this site is currently collected by an onsite closed conveyance system that outfalls into Inter Avenue's public closed conveyance system. This public closed conveyance system is comprised of 12 -inch concrete pipes and flows west approximately 1,500-feet towards offsite wetlands and Upper Deer Creek. The site is accessed by an existing gravel driveway from Inter Avenue.

Onsite soils have been identified as Briscot loam (6A a Type D soil) determined by the USDA SCS maps of Pierce County, Washington. A description of the USDA soils and a copy of the soil map for this portion of Pierce County have been included in Appendix 'A' of this report. A draft geotechnical engineer's report has been prepared by GeoResources, dated January 21, 2021, with an addendum, dated February 14, 2022, where they documented 0.5 -feet to 1.0 -feet of topsoil over silty alluvium. Mottling was observed at a depth 0.5 to 1.0 -feet with groundwater observed at a depth of 0.5 -feet. An EPA falling head test and small-scale PIT was performed within the native alluvium soils and a 0.6 -inch per hour infiltration design rate is provided. Although, the permeable pavement is sized with an infiltration rate of 0.1-inches per hour. A copy of the geotechnical report is included in Appendix ' D '. Permeable ballast fill is proposed in the paving area so the permeable pavement storage reservoir course meets the separation requirements from the seasonal high groundwater table.

There is an existing gravity sewer main in Inter Avenue which currently serves the property. There are no known aquifer recharge or wellhead protection areas that affect this property. There are no known well or septic systems onsite. If a septic system or well is discovered onsite during construction, it will be decommissioned per Tacoma-Pierce County Health Department standards. The parcel and all the proposed improvements are located within Zone X, which is considered outside of the 100-year floodplain, per FEMA Map \# 53053C0334E. A copy of the FIRM Panel map can be found in Appendix ' B ' of this report.

## 3. Off-site Analysis Report

A quarter mile downstream analysis is required by the City of Puyallup. The project proposes permeable pavement for flow control. The overflow from the permeable pavement is collected and conveyed to the existing public closed conveyance system in Inter Avenue. Based on a field survey and public GIS information, the runoff is conveyed west within Inter Avenue's closed conveyance system towards Upper Deer Creek. The $1 / 4$ mile drainage path ends at the outfall to Upper Deer Creek. Existing wetlands are located at the outfall to Upper Deer Creek. The runoff ultimately outfalls to the Puyallup River which is located approximately 0.9 -miles downstream located northwest of the project site. A downstream map is included in Appendix 'B'. No adverse
impacts are anticipated to the downstream system as a result of the development due to the proposed detention tank and bio-swale.

## 4. Permanent Stormwater Control Plan

## Existing Site Hydrology

The existing site is collected by an existing closed conveyance system that outfalls to the public storm system in Inter Avenue. The existing site is analyzed to demonstrate that the project meets both the flow control and LID performance standards. The pre-developed basin is 0.997 -acres C, Forest, Flat. The following is a summary of the pre-developed site flows:

| 2-year | 0.024-cfs |
| :---: | :---: |
| 10-year | 0.045-cfs |
| 50-year | 0.059-cfs |
| 100-year | 0.063-cfs |



Please refer to the Pre-Developed Basin Map in Appendix 'B' and the WWHM computer results in Appendix ' C '.

## Developed Site Hydrology

Under the developed condition, the project site proposes $31,632 \mathrm{sq} . \mathrm{ft}$. of paving, that does not include overlaying the existing asphalt, across onsite and offsite improvements and $10,432 \mathrm{sq} . \mathrm{ft}$. of landscaping. The landscape and yard areas can be modeled as "pasture" due to soil amendment per Ecology BMP T5.13. For the purpose of sizing the overflow conveyance system the permeable pavement is modelled as C, Lawn, Flat with the WWHM computer program. The post developed basin is summarized in the below:

| Sub-Basin | Land-use | WWHM Description | Area (ac) |
| :---: | :---: | :---: | :---: |
| Onsite A | Yards and Landscape (Amended Soils) | C, Pasture, Flat | 0.174 |
| Onsite B | Curbing | Roadway, Flat | 0.012 |
| Onsite C | Permeable Pavement | C, Lawn, Flat | 0.600 |
| Bypass A | Frontage Improvements Roadway/Sidewalk | Roadway, Flat | 0.054 |
| Bypass B | Onsite Paving | Roadway, Flat | 0.060 |
| Bypass C | Landscaping Planters | C, Pastures, Flat | 0.097 |
| Total |  |  |  |

Table 1 - Post Developed Basin

The following is a summary of the post developed site flows:

| 2-year | 0.078-cfs |
| :---: | :---: |
| 10-year | 0.162-cfs |
| 50-year | 0.272-cfs |
| 100-year | 0.331-cfs |

Please refer to the Post Developed Basin Map in Appendix 'B' and the WWHM computer results in Appendix ' C '.

## Facility Sizing

The project proposes permeable pavement to control the runoff from the newly paved parking lot area. Additionally, planters are constructed as part of the parking lot improvements which are to have their soils amended per BMP T5.13. The areas of the site that are modified as part of this permit are modelled with WWHM computer program to demonstrate the project's compliance with both the flow control and LID performance standards. The permeable pavement's subbasin is summarized in Table 2 below:

| Sub-Basin | Land-use | WWHM Description | Area (ac) |
| :---: | :---: | :---: | :---: |
| Onsite A | Yards and Landscape (Amended Soils) | C, Pasture, Flat | 0.174 |
| Onsite B | Curbing | Roadway, Flat | 0.012 |
| Onsite C | Permeable Pavement | Permeable Pavement <br> WWHM Element | 0.600 |
| Total |  |  | $\mathbf{0 . 7 8 6}$ |

Table 2 - Permeable Pavement Basin
As a factor of safety, the permeable pavement is modelled with an infiltration rate of 0.1 inches per hour and 0.5 effective volume factor. As computed by WWHM, a pavement section comprised of 6.5 -inch thick permeable asphalt over 2.5 -inches of permeable ballast can fully infiltrate the basin summarized in Table 2. Any additional permeable ballast placed below the pavement is for structural purposes and is not needed for stormwater storage. The underlying soils meet the CEC and organic content requirements of Section 4.4.2 of Volume V of the Manual so runoff treatment is provided for the permeable pavement area. The portions of the site that are being improved the project meets the flow control and LID performance standards. A copy of the WWHM computer report is included in Appendix C.

## Conveyance Calculations

The project proposes an extension of the existing conveyance system within Inter Ave to an onsite overflow conveyance system. This system is comprised of catch basins, PVC pipes and ductile iron pipe. The shallowest pipe is analyzed to demonstrate that the system's ability to convey the site's overflow 100-year event as calculated by the WWHM computer program. Computer modeling results are provided in Appendix ' C '. A summary of the calculations is provided below:

- Pipe Reach Name: DIP
- Structure Tributary Area: 0.997-ac
- Pipe Diameter (in): 12-in
- Pipe Length (ft): NA
- Pipe Slope (\%): $\mathbf{0 . 5 0 \%}$
- Manning's Coefficient (n): $\mathbf{0 . 0 1 2}$ (DIP)
- Design Flow (cfs): 0.33-cfs (100-year)
- Pipe-Full Flow (cfs): 2.73-cfs
- Water Depth at Design Flow (in): 2.88-in
- Critical Depth (in): 2.88-in
- Velocity at Design Flow (fps): 2.28-fps
- Velocity at Pipe-Full Flow (fps): 3.47-fps
- Percent full at Design Flow (\%): 24\%
- HGL for each Pipe Reach (elev): 0.24-ft


## 5. Discussion of Minimum Requirements

The project is the redevelopment of two parcels that proposes more than 5,000 sq.ft. of new plus replaced hard surfaces; therefore, as required by Figure 2.4.1 and 2.4.2 of Volume I of the Manual all minimum requirement applies to the new and replaced surfaces.


Figure 2.4.1 - Flow Chart for Determining Requirements for New Development


Figure 2.4.2 - Flow Chart for Determining Requirements for Redevelopment

The following is a summary of the minimum requirements as described in Chapter 2 of Volume I of the Manual.

### 5.1 Minimum Requirement \#1: Preparation of a Stormwater Site Plan

 The Stormwater Site Plan is prepared and is provided with this document.5.2 Minimum Requirement \#2: Construction Stormwater Pollution Prevention (SWPP)

A SWPP Plan has been prepared. It is submitted alongside this report as the erosion control report.

### 5.3 Minimum Requirement \#3: Source Control of Pollution

Permanent source control BMPs are required for the development's daily operations as described in the Pollution Source Control Manual for Commercial/Industrial Activities included as a separate document.
5.4 Minimum Requirement \#4: Preservation of Natural Drainage System and Outfalls Under existing conditions, stormwater runoff is collected onsite and discharged into the public conveyance system in Inter Avenue south of the site.

### 5.5 Minimum Requirement \#5: Onsite Stormwater Management

This project must meet minimum requirements 1-9; therefore, this project must either evaluate List 2 for onsite stormwater management compliance or meet the LID Performance Standard. The project meets the LID Performance Standard with the use of permeable pavement (BMP T5.15) and soil preservation and amendment (Ecology BMP T5.13).

### 5.6 Minimum Requirement \#6: Runoff Treatment

The project provides runoff treatment with the permeable pavement. The underlying soils meet the CEC and organic requirements of Section 4.4.2 of Volume $V$ of the Manual.

### 5.7 Minimum Requirement \#7: Flow Control

The project meets the Flow Control Performance Standard with the use of permeable pavement (BMP T5.15) and soil amendments. (BMP T5.13)

### 5.8 Minimum Requirement \#8: Wetlands Protection

An offsite wetland exists about 1,500-feet downstream of the site at Inter Ave's outfall to Upper Deer Creek. A wetland recharge analysis is performed between the basin summarized in Table 1
of this report. The actual pre-developed land use cover (0.997-acres, Parking Lot, Flat) is used to complete this analysis. Since the project is proposing permeable pavement to mitigate the proposed parking lot improvements the project provides less volume downstream each month of the year. A copy of the analysis is provided in Appendix "C" of this report.
5.9 Minimum Requirement \#9: Operation and Maintenance

An Operation and Maintenance Manual is submitted alongside this report.

## 6. Other Permits

Other necessary permits and approvals include:

- Right of Way
- Sanitary Side Sewer Permits
- Building Remodel Permits


## APPENDIX A

## General Exhibits

Vicinity Map ..... A-1
Soils Map and Description (NRCS) ..... A-2



## MAP LEGEND

| Area of Interest (AOI) |  |
| :--- | :--- |
| $\square$ | Area of Interest (AOI) |
| Soils |  |
| $\square$ | Soil Map Unit Polygons |
| $\square$ | Soil Map Unit Lines |
| $\square$ | Soil Map Unit Points |

Special Point Features
(0) Blowout

B Borrow Pit
䠈 Clay Spot
$\diamond$ Closed Depression
Gravel Pit
$\therefore$ Gravelly Spot
(8) Landfill
A. Lava Flow

Marsh or swamp
Q Mine or Quarry
(-) Miscellaneous Water

- Perennial Water
- Rock Outcrop
$\uparrow$ Saline Spot
$\therefore$ Sandy Spot
Severely Eroded Spot
- Sinkhole

3. Slide or Slip
(2) Sodic Spot

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.
Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)
Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
Soil Survey Area: Pierce County Area, Washington
Survey Area Data: Version 16, Jun 4, 2020
Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 29, 2018—Jul 22, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

| Map Unit Symbol |  | Map Unit Name | Acres in AOI |
| :--- | :--- | ---: | ---: |
| PA | Briscot loam | 2.4 | Percent of AOI |
| Totals for Area of Interest | $\mathbf{2 . 4}$ | $100.0 \%$ |  |

## APPENDIX B

## Basin Exhibits

Predeveloped Basin Map ..... B-1
Post Developed Basin Map ..... B-2
FIRM Panel (\#53053C0334E) ..... B-3
Downstream Drainage Map ..... B-4




POST DEVELOPED BASIN


## National Flood Hazard Layer FIRMette



## Legend

SEe fis report for detailed legend and index map for firm panel layout
 an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 2/1/2021 at 1:21 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.





Date: 2/1/2021 12:34 PM

## APPENDIX C

Computer Modelling Results

## WWHM2012

## PROJECT REPORT

## General Model Information

| Project Name: | 20083 Permeable Pavement |
| :--- | :--- |
| Site Name: | Olson Bros |
| Site Address: | 2511 Inter Ave |
| City: | Puyallup, WA |
| Report Date: | $7 / 18 / 2022$ |
| Gage: | 40 IN EAST |
| Data Start: | $10 / 01 / 1901$ |
| Data End: | $09 / 30 / 2059$ |
| Timestep: | 15 Minute |
| Precip Scale: | 1.000 |
| Version Date: | $2021 / 08 / 18$ |
| Version: | 4.2 .18 |
|  |  |
| POC Thresholds |  |

Low Flow Threshold for POC1: 50 Percent of the 2 Year High Flow Threshold for POC1: 50 Year

## Landuse Basin Data

## Predeveloped Land Use

| Pre-Dev |  |
| :--- | :--- |
| Bypass: | No |
| GroundWater: | No |
| Pervious Land Use | acre |
| C, Forest, Flat | 1.082 |
| Pervious Total | 1.082 |
| Impervious Land Use | acre |
| Impervious Total | 0 |
| Basin Total | 1.082 |

Element Flows To:
Surface
Interflow
Groundwater

Bypass
Bypass: Yes
GroundWater: No
Pervious Land Use acre
C, Pasture, Flat 0.123
Pervious Total 0.123
Impervious Land Use acre
ROADS FLAT 0.084
PARKING FLAT 0.029
Impervious Total 0.113
Basin Total 0.236

Element Flows To:
Surface Interflow

Groundwater

## Landscaping

Bypass: No
GroundWater: No
Pervious Land Use acre
C, Pasture, Flat . 174
Element Flows To:
Surface Interflow
Groundwater
Permeable Parkling Lo®ermeable Parkling Lot

Curbing

Bypass:
Impervious Land Use ROADS FLAT LAT Element Flows To:
Outlet 1
Permeable Parkling Lot

Routing Elements
Predeveloped Routing

## Mitigated Routing

## Permeable Parkling Lot

Pavement Area:0.6600 acre.Pavement Length:169.56 ft.

Pavement Width:
Pavement thickness:
Pour Space of Pavement:
Material thickness of second layer:
Pour Space of material for second layer:
Material thickness of third layer:
Pour Space of material for third layer:
Infiltration On
Infiltration rate:
Infiltration safety factor:
Total Volume Infiltrated (ac-ft.):
Total Volume Through Riser (ac-ft.):
Total Volume Through Facility (ac-ft.):
Percent Infiltrated:
Total Precip Applied to Facility:
Total Evap From Facility:
Element Flows To:
Outlet 1
Outlet 2
169.56 ft .

Pavement slope 1:0.05 To 1
0.54
0.5


Permeable Pavement Hydraulic Table

| Stage(feet) | Area(ac.) | Volume(ac-ft.) | Discharge(cfs) Infilt(cfs) |
| :--- | :--- | :--- | :--- |
| 0.0000 | 0.660 | 0.000 | 0.000 |
| 0.0194 | 0.660 | 0.003 | 0.000 |
| 0.0389 | 0.660 | 0.007 | 0.000 |
| 0.0583 | 0.660 | 0.011 | 0.006 |
| 0.0778 | 0.660 | 0.015 | 0.000 |
| 0.0972 | 0.660 | 0.019 | 0.000 |
| 0.1167 | 0.660 | 0.023 | 0.000 |
| 0.1361 | 0.660 | 0.027 | 0.000 |
| 0.1556 | 0.660 | 0.030 | 0.000 |
| 0.1750 | 0.660 | 0.034 | 0.000 |
| 0.1944 | 0.660 | 0.038 | 0.000 |
| 0.2139 | 0.660 | 0.044 | 0.000 |
| 0.2333 | 0.660 | 0.051 | 0.066 |
| 0.2528 | 0.660 | 0.057 | 0.066 |
| 0.2722 | 0.660 | 0.064 | 0.000 |
| 0.2917 | 0.660 | 0.070 | 0.000 |
| 0.3111 | 0.660 | 0.077 | 0.000 |
| 0.3306 | 0.660 | 0.083 | 0.000 |
| 0.3500 | 0.660 | 0.089 | 0.000 |
| 0.3694 | 0.660 | 0.096 | 0.000 |
| 0.3889 | 0.660 | 0.102 | 0.000 |
| 0.4083 | 0.661 | 0.109 | 0.000 |
| 0.4278 | 0.661 | 0.115 | 0.000 |
| 0.4472 | 0.661 | 0.122 | 0.000 |
| 0.4667 | 0.661 | 0.128 | 0.000 |
| 0.4861 | 0.661 | 0.134 | 0.000 |
| 0.5056 | 0.661 | 0.141 | 0.000 |
| 0.5250 | 0.661 | 0.147 | 0.000 |
| 0.5444 | 0.661 | 0.154 | 0.000 |


|  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| 0.5639 | 0.661 | 0.160 | 0.000 | 0.066 |
| 0.5833 | 0.661 | 0.167 | 0.000 | 0.066 |
| 0.6028 | 0.661 | 0.173 | 0.000 | 0.066 |
| 0.6222 | 0.661 | 0.179 | 0.000 | 0.066 |
| 0.6417 | 0.661 | 0.186 | 0.000 | 0.066 |
| 0.6611 | 0.661 | 0.192 | 0.000 | 0.066 |
| 0.6806 | 0.661 | 0.199 | 0.000 | 0.066 |
| 0.7000 | 0.661 | 0.205 | 0.000 | 0.066 |
| 0.7194 | 0.661 | 0.212 | 0.000 | 0.066 |
| 0.7389 | 0.661 | 0.218 | 0.000 | 0.066 |
| 0.7583 | 0.661 | 0.231 | 0.000 | 0.066 |
| 0.7778 | 0.661 | 0.244 | 0.000 | 0.066 |
| 0.7972 | 0.661 | 0.257 | 0.000 | 0.066 |
| 0.8167 | 0.661 | 0.270 | 0.000 | 0.066 |
| 0.8361 | 0.662 | 0.282 | 0.000 | 0.066 |
| 0.8556 | 0.662 | 0.295 | 0.000 | 0.066 |
| 0.8750 | 0.662 | 0.308 | 0.000 | 0.066 |
| 0.8944 | 0.662 | 0.321 | 0.000 | 0.066 |
| 0.9139 | 0.662 | 0.334 | 0.000 | 0.066 |
| 0.9333 | 0.662 | 0.347 | 0.000 | 0.066 |
| 0.9528 | 0.662 | 0.360 | 0.000 | 0.066 |
| 0.9722 | 0.662 | 0.373 | 0.000 | 0.066 |
| 0.9917 | 0.662 | 0.385 | 0.000 | 0.066 |
| 1.0111 | 0.662 | 0.398 | 0.066 | 0.066 |
| 1.0306 | 0.662 | 0.411 | 0.301 | 0.066 |
| 1.0500 | 0.662 | 0.424 | 0.631 | 0.066 |
| 1.0694 | 0.662 | 0.437 | 1.033 | 0.066 |
| 1.0889 | 0.662 | 0.450 | 1.496 | 0.066 |
| 1.1083 | 0.662 | 0.463 | 2.013 | 0.066 |
| 1.1278 | 0.662 | 0.476 | 2.579 | 0.066 |
| 1.1472 | 0.662 | 0.488 | 3.189 | 0.066 |
| 1.1667 | 0.662 | 0.501 | 3.841 | 0.066 |
| 1.1861 | 0.662 | 0.514 | 4.533 | 0.066 |
| 1.2056 | 0.662 | 0.527 | 5.262 | 0.066 |
| 1.2250 | 0.662 | 0.540 | 6.026 | 0.066 |
| 1.2444 | 0.662 | 0.553 | 6.824 | 0.066 |
| 1.2639 | 0.663 | 0.566 | 7.654 | 0.066 |
| 1.2833 | 0.663 | 0.579 | 8.515 | 0.066 |
| 1.3028 | 0.663 | 0.592 | 9.407 | 0.066 |
| 1.3222 | 0.663 | 0.604 | 10.32 | 0.066 |
| 1.3417 | 0.663 | 0.617 | 11.27 | 0.066 |
| 1.3611 | 0.663 | 0.630 | 12.25 | 0.066 |
| 1.3806 | 0.663 | 0.643 | 13.25 | 0.066 |
| 1.4000 | 0.663 | 0.656 | 14.28 | 0.066 |
| 1.4194 | 0.663 | 0.669 | 15.33 | 0.066 |
| 1.4389 | 0.663 | 0.682 | 16.41 | 0.066 |
| 1.4583 | 0.663 | 0.695 | 17.52 | 0.066 |
| 1.4778 | 0.663 | 0.708 | 18.64 | 0.066 |
| 1.4972 | 0.663 | 0.721 | 19.79 | 0.066 |
| 1.5167 | 0.663 | 0.733 | 20.96 | 0.066 |
| 1.5361 | 0.663 | 0.746 | 22.16 | 0.066 |
| 1.5556 | 0.663 | 0.759 | 23.38 | 0.066 |
| 1.5750 | 0.663 | 0.772 | 24.61 | 0.066 |
| 1.5944 | 0.663 | 0.785 | 25.87 | 0.066 |
| 1.6139 | 0.663 | 0.811 | 27.15 | 0.066 |
| 1.6333 | 0.65288 | 0.663 | 29.45 | 0.066 |
| 1.6722 | 0.633 |  | 0.066 |  |
|  |  |  |  | 0.066 |

1.6917
1.7111
1.7306
1.7500
0.664
0.664 0.664 0.664
0.850
0.863
0.875 0.888
32.48
33.85
35.25
36.67
0.066
0.066
0.066
0.066

## Analysis Results POC 1



Predeveloped Landuse Totals for POC \#1
Total Pervious Area: 1.082

Total Impervious Area:
0
Mitigated Landuse Totals for POC \#1
Total Pervious Area: 0.297
Total Impervious Area: 0.785023
Flow Frequency Method: Log Pearson Type III 17B
Flow Frequency Return Periods for Predeveloped. POC \#1

| Return Period | Flow(cfs) |
| :--- | :--- |
| 2 year | 0.024447 |
| 5 year | 0.037615 |
| 10 year | 0.045226 |
| 25 year | 0.05347 |
| 50 year | 0.058773 |
| 100 year | 0.063374 |

Flow Frequency Return Periods for Mitigated. POC \#1

| Return Period | Flow $(\mathbf{c f s})$ |
| :--- | :--- |
| 2 year | 0.042113 |
| 5 year | 0.056588 |
| 10 year | 0.067116 |
| 25 year | 0.081535 |
| 50 year | 0.093114 |
| 100 year | 0.105434 |

## Annual Peaks

Annual Peaks for Predeveloped and Mitigated. POC \#1

Year
1902
1903
1904
1905
1906
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1954
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1956
1957

Predeveloped Mitigated
$0.020 \quad 0.048$
$0.015 \quad 0.053$
0.026
0.013
0.007
0.038
0.027
0.027
0.038
0.024
0.082
0.038
0.010
0.016
0.024
0.008
0.026
0.020
0.027
0.027
0.022
0.011
0.024
0.017
0.019
0.038
0.024
0.023
0.018
0.050
0.023
0.021
0.033
0.020
0.002
0.022
0.014
0.034
0.017
0.035
0.027
0.016
0.011
0.052
0.045
0.013
0.017
0.067
0.061
0.022
0.019
0.010
0.032
0.066
0.027
0.031
0.045
0.035
0.041
0.042
0.045
0.090
0.032
0.135
0.029
0.051
0.020
0.041
0.027
0.035
0.032
0.049
0.033
0.059
0.026
0.048
0.041
0.031
0.058
0.061
0.031
0.033
0.056
0.028
0.039
0.049
0.029
0.034
0.061
0.067
0.048
0.046
0.066
0.049
0.040
0.030
0.063
0.035
0.053
0.069
0.063
0.035
0.031
0.029
0.034

|  |  |  |
| :--- | :--- | :--- |
| 1958 | 0.065 | 0.046 |
| 1959 | 0.041 | 0.046 |
| 1960 | 0.012 | 0.033 |
| 1961 | 0.041 | 0.092 |
| 1962 | 0.022 | 0.040 |
| 1963 | 0.011 | 0.029 |
| 1964 | 0.011 | 0.086 |
| 1965 | 0.046 | 0.042 |
| 1966 | 0.013 | 0.033 |
| 1967 | 0.020 | 0.048 |
| 1968 | 0.022 | 0.039 |
| 1969 | 0.020 | 0.036 |
| 1970 | 0.031 | 0.042 |
| 1971 | 0.048 | 0.041 |
| 1972 | 0.032 | 0.124 |
| 1973 | 0.041 | 0.071 |
| 1974 | 0.023 | 0.054 |
| 1975 | 0.051 | 0.061 |
| 1976 | 0.027 | 0.062 |
| 1977 | 0.012 | 0.025 |
| 1978 | 0.045 | 0.047 |
| 1979 | 0.013 | 0.045 |
| 1980 | 0.026 | 0.046 |
| 1981 | 0.024 | 0.041 |
| 1982 | 0.011 | 0.034 |
| 1983 | 0.041 | 0.048 |
| 1984 | 0.018 | 0.047 |
| 1985 | 0.029 | 0.054 |
| 1986 | 0.024 | 0.029 |
| 1987 | 0.047 | 0.046 |
| 1988 | 0.029 | 0.029 |
| 1989 | 0.027 | 0.027 |
| 1990 | 0.031 | 0.036 |
| 1991 | 0.025 | 0.049 |
| 1992 | 0.032 | 0.047 |
| 1993 | 0.033 | 0.053 |
| 1994 | 0.048 | 0.040 |
| 1995 | 0.011 | 0.029 |
| 1996 | 0.053 | 0.040 |
| 1997 | 0.022 | 0.035 |
| 1998 | 0.026 | 0.044 |
| 1999 | 0.003 | 0.046 |
| 2000 | 0.019 | 0.040 |
| 2001 | 0.011 | 0.031 |
| 2002 | 0.035 | 0.063 |
| 2003 | 0.030 | 0.034 |
| 2004 | 0.027 | 0.049 |
| 2005 | 0.048 | 0.095 |
| 2006 | 0.015 | 0.044 |
| 2007 | 0.016 | 0.050 |
| 2008 | 0.026 | 0.041 |
| 2009 | 0.031 |  |
| 2010 | 0.015 | 0.040 |
| 2011 | 0.041 |  |
| 2012 | 0.063 |  |
| 2013 | 014 | 0.038 |
| 2015 | 0.036 |  |
|  | 0.031 |  |


| 2016 | 0.009 | 0.039 |
| :--- | :--- | :--- |
| 2017 | 0.037 | 0.059 |
| 2018 | 0.066 | 0.043 |
| 2019 | 0.066 | 0.064 |
| 2020 | 0.021 | 0.046 |
| 2021 | 0.034 | 0.040 |
| 2022 | 0.014 | 0.061 |
| 2023 | 0.028 | 0.076 |
| 2024 | 0.057 | 0.094 |
| 2025 | 0.025 | 0.040 |
| 2026 | 0.040 | 0.045 |
| 2027 | 0.015 | 0.049 |
| 2028 | 0.013 | 0.019 |
| 2029 | 0.027 | 0.034 |
| 2030 | 0.050 | 0.066 |
| 2031 | 0.017 | 0.021 |
| 2032 | 0.010 | 0.033 |
| 2033 | 0.015 | 0.042 |
| 2034 | 0.015 | 0.032 |
| 2035 | 0.057 | 0.047 |
| 2036 | 0.030 | 0.033 |
| 2037 | 0.008 | 0.044 |
| 2038 | 0.025 | 0.046 |
| 2039 | 0.003 | 0.084 |
| 2040 | 0.014 | 0.034 |
| 2041 | 0.019 | 0.042 |
| 2042 | 0.058 | 0.049 |
| 2043 | 0.028 | 0.053 |
| 2044 | 0.037 | 0.038 |
| 2045 | 0.025 | 0.032 |
| 2046 | 0.029 | 0.034 |
| 2047 | 0.021 | 0.041 |
| 2048 | 0.028 | 0.033 |
| 2049 | 0.025 | 0.049 |
| 2050 | 0.018 | 0.039 |
| 2051 | 0.026 | 0.057 |
| 2052 | 0.015 | 0.040 |
| 2053 | 0.027 | 0.034 |
| 2054 | 0.034 | 0.071 |
| 2055 | 0.014 | 0.039 |
| 2056 | 0.012 | 0.053 |
| 2057 | 0.019 | 0.027 |
| 2058 | 0.023 | 0.050 |
| 2059 | 0.040 | 0.063 |
|  | 0 |  |

## Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC \#1

| Rank | Predeveloped Mitigated |  |
| :--- | :--- | :--- |
| 1 | 0.0822 | 0.1353 |
| 2 | 0.0673 | 0.1245 |
| 3 | 0.0663 | 0.0954 |
| 4 | 0.0656 | 0.0939 |
| 5 | 0.0650 | 0.0922 |
| 6 | 0.0611 | 0.0897 |
| 7 | 0.0577 | 0.0863 |
| 8 | 0.0571 | 0.0836 |
| 9 | 0.0568 | 0.0759 |
| 10 | 0.0527 | 0.0714 |


| 11 | 0.0520 | 0.0713 |
| :---: | :---: | :---: |
| 12 | 0.0511 | 0.0692 |
| 13 | 0.0501 | 0.0670 |
| 14 | 0.0498 | 0.0665 |
| 15 | 0.0484 | 0.0664 |
| 16 | 0.0483 | 0.0655 |
| 17 | 0.0482 | 0.0635 |
| 18 | 0.0466 | 0.0629 |
| 19 | 0.0457 | 0.0628 |
| 20 | 0.0452 | 0.0628 |
| 21 | 0.0449 | 0.0626 |
| 22 | 0.0410 | 0.0626 |
| 23 | 0.0407 | 0.0619 |
| 24 | 0.0407 | 0.0612 |
| 25 | 0.0407 | 0.0612 |
| 26 | 0.0403 | 0.0608 |
| 27 | 0.0398 | 0.0605 |
| 28 | 0.0383 | 0.0594 |
| 29 | 0.0379 | 0.0589 |
| 30 | 0.0378 | 0.0581 |
| 31 | 0.0378 | 0.0566 |
| 32 | 0.0371 | 0.0559 |
| 33 | 0.0367 | 0.0543 |
| 34 | 0.0351 | 0.0539 |
| 35 | 0.0348 | 0.0534 |
| 36 | 0.0338 | 0.0534 |
| 37 | 0.0337 | 0.0532 |
| 38 | 0.0336 | 0.0532 |
| 39 | 0.0331 | 0.0525 |
| 40 | 0.0329 | 0.0510 |
| 41 | 0.0324 | 0.0500 |
| 42 | 0.0319 | 0.0499 |
| 43 | 0.0316 | 0.0495 |
| 44 | 0.0314 | 0.0494 |
| 45 | 0.0305 | 0.0492 |
| 46 | 0.0304 | 0.0490 |
| 47 | 0.0300 | 0.0489 |
| 48 | 0.0291 | 0.0488 |
| 49 | 0.0291 | 0.0487 |
| 50 | 0.0288 | 0.0487 |
| 51 | 0.0283 | 0.0485 |
| 52 | 0.0283 | 0.0480 |
| 53 | 0.0276 | 0.0479 |
| 54 | 0.0274 | 0.0479 |
| 55 | 0.0274 | 0.0477 |
| 56 | 0.0273 | 0.0474 |
| 57 | 0.0273 | 0.0468 |
| 58 | 0.0271 | 0.0468 |
| 59 | 0.0271 | 0.0467 |
| 60 | 0.0270 | 0.0461 |
| 61 | 0.0268 | 0.0460 |
| 62 | 0.0267 | 0.0460 |
| 63 | 0.0265 | 0.0459 |
| 64 | 0.0262 | 0.0459 |
| 65 | 0.0261 | 0.0457 |
| 66 | 0.0261 | 0.0457 |
| 67 | 0.0260 | 0.0456 |
| 68 | 0.0258 | 0.0454 |


|  |  |  |
| :--- | :--- | :--- |
| 69 | 0.0258 | 0.0454 |
| 70 | 0.0253 | 0.0449 |
| 71 | 0.0252 | 0.0448 |
| 72 | 0.0251 | 0.0440 |
| 73 | 0.0249 | 0.0439 |
| 74 | 0.0248 | 0.0439 |
| 75 | 0.0245 | 0.0432 |
| 76 | 0.0245 | 0.0423 |
| 77 | 0.0244 | 0.0419 |
| 78 | 0.0243 | 0.0418 |
| 79 | 0.0242 | 0.0416 |
| 80 | 0.0239 | 0.0415 |
| 81 | 0.0237 | 0.0414 |
| 82 | 0.0230 | 0.0413 |
| 83 | 0.0230 | 0.0412 |
| 84 | 0.0226 | 0.0410 |
| 85 | 0.0226 | 0.0410 |
| 86 | 0.0224 | 0.0408 |
| 87 | 0.0221 | 0.0408 |
| 88 | 0.0217 | 0.0406 |
| 89 | 0.0217 | 0.0406 |
| 90 | 0.0216 | 0.0405 |
| 91 | 0.0215 | 0.0403 |
| 92 | 0.0213 | 0.0402 |
| 93 | 0.0208 | 0.0401 |
| 94 | 0.0207 | 0.0401 |
| 95 | 0.0207 | 0.0399 |
| 96 | 0.0205 | 0.0399 |
| 97 | 0.0205 | 0.0398 |
| 98 | 0.0204 | 0.0396 |
| 99 | 0.0203 | 0.0396 |
| 100 | 0.0147 | 0.0153 |
| 101 | 0.0198 | 0.0390 |
| 102 | 0.0195 | 0.0390 |
| 103 | 0.0194 | 0.0389 |
| 104 | 0.0191 | 0.0385 |
| 105 | 0.0190 | 0.0385 |
| 106 | 0.0189 | 0.0380 |
| 107 | 0.0187 | 0.0377 |
| 108 | 0.0185 | 0.0359 |
| 109 | 0.0180 | 0.0356 |
| 110 | 0.0176 | 0.0356 |
| 111 | 0.0174 | 0.0355 |
| 112 | 0.0172 | 0.0351 |
| 113 | 0.0170 | 0.0348 |
| 114 | 0.0170 | 0.0347 |
| 115 | 0.0165 | 0.0346 |
| 116 | 0.0163 | 0.0345 |
| 117 | 0.0160 | 0.0344 |
| 118 | 0.0343 |  |
| 119 | 0.030 | 0.0340 |
| 120 | 0.0338 |  |
| 121 | 0.0337 |  |
| 122 | 0.0336 |  |
| 123 | 0.033 |  |
| 124 | 125 | 0.0334 |
| 126 | 0.0153 |  |
|  | 0 | 0 |

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| 0.0143 | 0.0332 |
| :--- | :--- |
| 0.0139 | 0.0331 |
| 0.0139 | 0.0331 |
| 0.0137 | 0.0329 |
| 0.0136 | 0.0318 |
| 0.0133 | 0.0318 |
| 0.0133 | 0.0315 |
| 0.0132 | 0.0315 |
| 0.0132 | 0.0314 |
| 0.0131 | 0.0313 |
| 0.0125 | 0.0309 |
| 0.0122 | 0.0307 |
| 0.0121 | 0.0307 |
| 0.0120 | 0.0305 |
| 0.0115 | 0.0296 |
| 0.0113 | 0.0293 |
| 0.0112 | 0.0293 |
| 0.0110 | 0.0290 |
| 0.0108 | 0.0289 |
| 0.0106 | 0.0286 |
| 0.0106 | 0.0285 |
| 0.0106 | 0.0285 |
| 0.0100 | 0.0277 |
| 0.0098 | 0.0272 |
| 0.0098 | 0.0271 |
| 0.0085 | 0.0269 |
| 0.0083 | 0.0268 |
| 0.0081 | 0.0263 |
| 0.0065 | 0.0247 |
| 0.0033 | 0.0209 |
| 0.0026 | 0.0202 |
| 0.0017 | 0.0190 |


| Flow(cfs) | Predev | Mit | Percentage | Pass/Fail |
| :--- | :--- | :--- | :--- | :--- |
| 0.0020 | 613285 | 393566 | 64 | Pass |
| 0.0021 | 592787 | 378663 | 63 | Pass |
| 0.0022 | 573397 | 364869 | 63 | Pass |
| 0.0023 | 555114 | 351960 | 63 | Pass |
| 0.0024 | 537719 | 339717 | 63 | Pass |
| 0.0025 | 520988 | 328470 | 63 | Pass |
| 0.0026 | 505088 | 317778 | 62 | Pass |
| 0.0027 | 490019 | 307751 | 62 | Pass |
| 0.0028 | 475448 | 298222 | 62 | Pass |
| 0.0029 | 461543 | 289081 | 62 | Pass |
| 0.0030 | 448025 | 280438 | 62 | Pass |
| 0.0031 | 435061 | 271962 | 62 | Pass |
| 0.0032 | 422430 | 263873 | 62 | Pass |
| 0.0033 | 410242 | 256283 | 62 | Pass |
| 0.0034 | 398552 | 249137 | 62 | Pass |
| 0.0035 | 387250 | 242156 | 62 | Pass |
| 0.0036 | 376503 | 235508 | 62 | Pass |
| 0.0037 | 366198 | 229082 | 62 | Pass |
| 0.0038 | 356337 | 222766 | 62 | Pass |
| 0.0039 | 346697 | 216783 | 62 | Pass |
| 0.0040 | 337390 | 210910 | 62 | Pass |
| 0.0041 | 328470 | 205481 | 62 | Pass |
| 0.0042 | 319828 | 199996 | 62 | Pass |
| 0.0043 | 311407 | 194733 | 62 | Pass |
| 0.0044 | 303263 | 189692 | 62 | Pass |
| 0.0045 | 295396 | 184761 | 62 | Pass |
| 0.0047 | 287806 | 180052 | 62 | Pass |
| 0.0048 | 280438 | 175454 | 62 | Pass |
| 0.0049 | 273513 | 171077 | 62 | Pass |
| 0.0050 | 266643 | 166756 | 62 | Pass |
| 0.0051 | 259940 | 162490 | 62 | Pass |
| 0.0052 | 253569 | 158501 | 62 | Pass |
| 0.0053 | 247253 | 154568 | 62 | Pass |
| 0.0054 | 241214 | 150801 | 62 | Pass |
| 0.0055 | 235342 | 147144 | 62 | Pass |
| 0.0056 | 229691 | 143598 | 62 | Pass |
| 0.0057 | 223985 | 140164 | 62 | Pass |
| 0.0058 | 218500 | 136673 | 62 | Pass |
| 0.0059 | 213237 | 133294 | 62 | Pass |
| 0.0060 | 208029 | 130136 | 62 | Pass |
| 0.0061 | 202988 | 126978 | 62 | Pass |
| 0.0062 | 198113 | 123931 | 62 | Pass |
| 0.0063 | 193404 | 121050 | 62 | Pass |
| 0.0064 | 188750 | 118170 | 62 | Pass |
| 0.0065 | 184263 | 115400 | 62 | Pass |
| 0.0066 | 180052 | 112630 | 62 | Pass |
| 0.0067 | 175952 | 109915 | 62 | Pass |
| 0.0068 | 171908 | 107311 | 62 | Pass |
| 0.0069 | 167975 | 104763 | 62 | Pass |
| 0.0070 | 164152 | 102270 | 62 | Pass |
| 0.0071 | 160496 | 99832 | 62 | Pass |
| 0.0072 | 156895 | 97450 | 62 | Pass |
| 0.0073 | 153404 | 95123 | 62 | Pass |
|  |  |  |  |  |


| 0.0075 | 149970 | 92796 | 61 | Pass |
| :--- | :--- | :--- | :--- | :--- |
| 0.0076 | 146590 | 90635 | 61 | Pass |
| 0.0077 | 143266 | 88475 | 61 | Pass |
| 0.0078 | 140164 | 86425 | 61 | Pass |
| 0.0079 | 137006 | 84431 | 61 | Pass |
| 0.0080 | 133903 | 82492 | 61 | Pass |
| 0.0081 | 130967 | 80663 | 61 | Pass |
| 0.0082 | 128031 | 78724 | 61 | Pass |
| 0.0083 | 125150 | 76951 | 61 | Pass |
| 0.0084 | 122380 | 75123 | 61 | Pass |
| 0.0085 | 119665 | 73406 | 61 | Pass |
| 0.0086 | 116951 | 71688 | 61 | Pass |
| 0.0087 | 114347 | 70082 | 61 | Pass |
| 0.0088 | 111854 | 68364 | 61 | Pass |
| 0.0089 | 109416 | 66813 | 61 | Pass |
| 0.0090 | 107034 | 65317 | 61 | Pass |
| 0.0091 | 104763 | 63822 | 60 | Pass |
| 0.0092 | 102491 | 62381 | 60 | Pass |
| 0.0093 | 100331 | 60941 | 60 | Pass |
| 0.0094 | 98170 | 59500 | 60 | Pass |
| 0.0095 | 96120 | 58115 | 60 | Pass |
| 0.0096 | 94015 | 56730 | 60 | Pass |
| 0.0097 | 92076 | 55401 | 60 | Pass |
| 0.0098 | 90137 | 54232 | 60 | Pass |
| 0.0099 | 88309 | 53041 | 60 | Pass |
| 0.0100 | 86536 | 51883 | 59 | Pass |
| 0.0101 | 84763 | 50764 | 59 | Pass |
| 0.0103 | 82824 | 49567 | 59 | Pass |
| 0.0104 | 81217 | 48470 | 59 | Pass |
| 0.0105 | 79500 | 47318 | 59 | Pass |
| 0.0106 | 77949 | 46287 | 59 | Pass |
| 0.0107 | 76397 | 45240 | 59 | Pass |
| 0.0108 | 74791 | 44116 | 58 | Pass |
| 0.0109 | 73295 | 43135 | 58 | Pass |
| 0.0110 | 71965 | 42199 | 58 | Pass |
| 0.0111 | 70470 | 41157 | 58 | Pass |
| 0.0112 | 69085 | 40265 | 58 | Pass |
| 0.0113 | 67644 | 39340 | 58 | Pass |
| 0.0114 | 66370 | 38509 | 58 | Pass |
| 0.0115 | 65151 | 37656 | 57 | Pass |
| 0.0116 | 63877 | 36764 | 57 | Pass |
| 0.0117 | 62714 | 35938 | 57 | Pass |
| 0.0118 | 61550 | 35174 | 57 | Pass |
| 0.0119 | 60276 | 34321 | 56 | Pass |
| 0.0120 | 59168 | 33573 | 56 | Pass |
| 0.0121 | 58060 | 32869 | 56 | Pass |
| 0.0122 | 56896 | 32121 | 56 | Pass |
| 0 |  |  |  |  |


| Flow(cfs) | Predev | Mit | Percentage | Pass/Fail |
| :--- | :--- | :--- | :--- | :--- |
| 0.0122 | 56896 | 32121 | 56 | Pass |
| 0.0127 | 52442 | 28980 | 55 | Pass |
| 0.0132 | 48387 | 26227 | 54 | Pass |
| 0.0136 | 44636 | 23689 | 53 | Pass |
| 0.0141 | 41240 | 21473 | 52 | Pass |
| 0.0146 | 38260 | 19429 | 50 | Pass |
| 0.0150 | 35501 | 17634 | 49 | Pass |
| 0.0155 | 32980 | 16011 | 48 | Pass |
| 0.0160 | 30537 | 14543 | 47 | Pass |
| 0.0165 | 28448 | 13219 | 46 | Pass |
| 0.0169 | 26509 | 12050 | 45 | Pass |
| 0.0174 | 24759 | 11014 | 44 | Pass |
| 0.0179 | 23141 | 10066 | 43 | Pass |
| 0.0183 | 21689 | 9185 | 42 | Pass |
| 0.0188 | 20332 | 8410 | 41 | Pass |
| 0.0193 | 19080 | 7690 | 40 | Pass |
| 0.0197 | 17867 | 7030 | 39 | Pass |
| 0.0202 | 16731 | 6399 | 38 | Pass |
| 0.0207 | 15617 | 5856 | 37 | Pass |
| 0.0212 | 14626 | 5372 | 36 | Pass |
| 0.0216 | 13728 | 4925 | 35 | Pass |
| 0.0221 | 12892 | 4529 | 35 | Pass |
| 0.0226 | 12105 | 4153 | 34 | Pass |
| 0.0230 | 11396 | 3814 | 33 | Pass |
| 0.0235 | 10670 | 3562 | 33 | Pass |
| 0.0240 | 10000 | 3315 | 33 | Pass |
| 0.0244 | 9368 | 3067 | 32 | Pass |
| 0.0249 | 8759 | 2860 | 32 | Pass |
| 0.0254 | 8205 | 2658 | 32 | Pass |
| 0.0259 | 7734 | 2496 | 32 | Pass |
| 0.0263 | 7246 | 2340 | 32 | Pass |
| 0.0268 | 6798 | 2178 | 32 | Pass |
| 0.0273 | 6426 | 2033 | 31 | Pass |
| 0.0277 | 6111 | 1886 | 30 | Pass |
| 0.0282 | 5828 | 1763 | 30 | Pass |
| 0.0287 | 5551 | 1662 | 29 | Pass |
| 0.0292 | 5265 | 1559 | 29 | Pass |
| 0.0296 | 5003 | 1457 | 29 | Pass |
| 0.0301 | 4782 | 1385 | 28 | Pass |
| 0.0306 | 4531 | 1302 | 28 | Pass |
| 0.0310 | 4338 | 1220 | 28 | Pass |
| 0.0315 | 4154 | 1134 | 27 | Pass |
| 0.0320 | 3937 | 1052 | 26 | Pass |
| 0.0324 | 3713 | 991 | 26 | Pass |
| 0.0329 | 3536 | 940 | 26 | Pass |
| 0.0334 | 3361 | 870 | 25 | Pass |
| 0.0339 | 3227 | 815 | 25 | Pass |
| 0.0343 | 3083 | 765 | 24 | Pass |
| 0.0348 | 2965 | 717 | 24 | Pass |
| 0.0353 | 2850 | 684 | 24 | Pass |
| 0.0357 | 2738 | 638 | 23 | Pass |
| 0.0362 | 2600 | 607 | 23 | Pass |
| 0.0367 | 2477 | 576 | 23 | Pass |
|  |  |  |  |  |


| 0.0371 | 2356 | 548 | 23 | Pass |
| :--- | :--- | :--- | :--- | :--- |
| 0.0376 | 2266 | 517 | 22 | Pass |
| 0.0381 | 2159 | 485 | 22 | Pass |
| 0.0386 | 2057 | 457 | 22 | Pass |
| 0.0390 | 1947 | 433 | 22 | Pass |
| 0.0395 | 1837 | 409 | 22 | Pass |
| 0.0400 | 1749 | 381 | 21 | Pass |
| 0.0404 | 1659 | 362 | 21 | Pass |
| 0.0409 | 1578 | 344 | 21 | Pass |
| 0.0414 | 1510 | 322 | 21 | Pass |
| 0.0418 | 1442 | 301 | 20 | Pass |
| 0.0423 | 1367 | 276 | 20 | Pass |
| 0.0428 | 1296 | 260 | 20 | Pass |
| 0.0433 | 1241 | 252 | 20 | Pass |
| 0.0437 | 1182 | 240 | 20 | Pass |
| 0.0442 | 1129 | 227 | 20 | Pass |
| 0.0447 | 1080 | 218 | 20 | Pass |
| 0.0451 | 1026 | 209 | 20 | Pass |
| 0.0456 | 979 | 198 | 20 | Pass |
| 0.0461 | 922 | 179 | 19 | Pass |
| 0.0465 | 871 | 166 | 19 | Pass |
| 0.0470 | 819 | 159 | 19 | Pass |
| 0.0475 | 771 | 153 | 19 | Pass |
| 0.0480 | 718 | 143 | 19 | Pass |
| 0.0484 | 668 | 138 | 20 | Pass |
| 0.0489 | 629 | 126 | 20 | Pass |
| 0.0494 | 588 | 116 | 19 | Pass |
| 0.0498 | 549 | 113 | 20 | Pass |
| 0.0503 | 507 | 108 | 21 | Pass |
| 0.0508 | 473 | 105 | 22 | Pass |
| 0.0512 | 428 | 101 | 23 | Pass |
| 0.0517 | 392 | 99 | 25 | Pass |
| 0.0522 | 363 | 96 | 26 | Pass |
| 0.0527 | 329 | 93 | 28 | Pass |
| 0.0531 | 300 | 91 | 30 | Pass |
| 0.0536 | 281 | 85 | 30 | Pass |
| 0.0541 | 264 | 81 | 30 | Pass |
| 0.0545 | 248 | 74 | 29 | Pass |
| 0.0550 | 233 | 72 | 30 | Pass |
| 0.0555 | 218 | 69 | 31 | Pass |
| 0.0560 | 205 | 68 | 33 | Pass |
| 0.0564 | 186 | 66 | 35 | Pass |
| 0.0569 | 162 | 65 | 40 | Pass |
| 0.0574 | 142 | 62 | 43 | Pass |
| 0.0578 | 129 | 59 | 45 | Pass |
| 0.0583 | 117 | 56 | 47 | Pass |
| 0.0588 | 105 | 56 | 53 | Pass |
| 0 |  |  |  |  |

## Water Quality

Water Quality BMP Flow and Volume for POC \#1
On-line facility volume: 0 acre-feet
On-line facility target flow: 0 cfs.
Adjusted for $15 \mathrm{~min}: \quad 0$ cfs.
Off-line facility target flow: 0 cfs.
Adjusted for 15 min: 0 cfs.

Wetland Input Volumes


| Wetlands Input Volume for POC 1Average Annual Volume (act) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Average 1: 501 POC 1 Predeveloped flow |  |  |  |  |
|  |  |  |  |  |
| Month | Series 1 | Series 2 | Percent | Pass/Fail |
| Jan | 0.1433 | 0.0647 | 45.1 | Fail |
| Feb | 0.1308 | 0.0551 | 42.1 | Fail |
| Mar | 0.0940 | 0.0438 | 46.6 | Fail |
| Apr | 0.0381 | 0.0241 | 63.3 | Fail |
| May | 0.0097 | 0.0151 | 154.6 | Fail |
| Jun | 0.0021 | 0.0108 | 502.7 | Fail |
| Jul | 0.0002 | 0.0051 | 2226.5 | Fail |
| Aug | 0.0000 | 0.0061 | 24603.9 | Fail |
| Sep | 0.0001 | 0.0126 | 13033.3 | Fail |
| Oct | 0.0030 | 0.0296 | 974.1 | Fail |
| Nov | 0.0465 | 0.0590 | 126.9 | Fail |
| Dec | 0.1218 | 0.0695 | 57.1 | Fail |
| Day | Predevel | Mitigated | Percent | Pass/F |
| Jan1 | 0.0040 | 0.0018 | 44.8 |  |
|  | 0.0044 | 0.0024 | 55.3 | Fail |
| 3 | 0.0052 | 0.0024 | 45.6 | Fail |
| 4 | 0.0046 | 0.0018 | 38.0 | Fail |
| 5 | 0.0042 | 0.0020 | 46.7 | Fail |
| 6 | 0.0044 | 0.0021 | 48.9 | Fail |
| 7 | 0.0043 | 0.0021 | 48.1 | Fail |
| 8 | 0.0038 | 0.0019 | 48.9 | Fail |
| 9 | 0.0041 | 0.0021 | 52.3 | Fail |
| 10 | 0.0042 | 0.0021 | 50.1 | Fail |
| 11 | 0.0044 | 0.0021 | 47.7 | Fail |
| 12 | 0.0041 | 0.0019 | 46.5 | Fail |
| 13 | 0.0048 | 0.0026 | 53.6 | Fail |
| 14 | 0.0056 | 0.0027 | 47.4 | Fail |


| 15 | 0.0055 | 0.0023 | 41.3 Fail |
| :---: | :---: | :---: | :---: |
| 16 | 0.0051 | 0.0023 | 44.3 Fail |
| 17 | 0.0050 | 0.0022 | 44.1 Fail |
| 18 | 0.0059 | 0.0026 | 44.1 Fail |
| 19 | 0.0061 | 0.0024 | 39.6 Fail |
| 20 | 0.0058 | 0.0018 | 31.8 Fail |
| 21 | 0.0044 | 0.0017 | 39.5 Fail |
| 22 | 0.0044 | 0.0023 | 51.8 Fail |
| 23 | 0.0050 | 0.0024 | 48.2 Fail |
| 24 | 0.0052 | 0.0021 | 40.8 Fail |
| 25 | 0.0044 | 0.0018 | 40.2 Fail |
| 26 | 0.0048 | 0.0021 | 43.9 Fail |
| 27 | 0.0045 | 0.0018 | 40.3 Fail |
| 28 | 0.0041 | 0.0015 | 38.0 Fail |
| 29 | 0.0034 | 0.0014 | 42.0 Fail |
| 30 | 0.0036 | 0.0020 | 56.3 Fail |
| 31 | 0.0046 | 0.0022 | 48.9 Fail |
| Feb1 | 0.0051 | 0.0022 | 42.9 Fail |
| 2 | 0.0045 | 0.0018 | 39.5 Fail |
| 3 | 0.0045 | 0.0016 | 36.5 Fail |
| 4 | 0.0036 | 0.0016 | 43.6 Fail |
| 5 | 0.0048 | 0.0024 | 50.3 Fail |
| 6 | 0.0049 | 0.0017 | 34.6 Fail |
| 7 | 0.0047 | 0.0021 | 44.9 Fail |
| 8 | 0.0046 | 0.0018 | 38.4 Fail |
| 9 | 0.0038 | 0.0016 | 42.3 Fail |
| 10 | 0.0039 | 0.0016 | 41.7 Fail |
| 11 | 0.0038 | 0.0020 | 51.8 Fail |
| 12 | 0.0041 | 0.0020 | 48.3 Fail |
| 13 | 0.0042 | 0.0020 | 48.2 Fail |
| 14 | 0.0040 | 0.0017 | 42.0 Fail |
| 15 | 0.0043 | 0.0020 | 47.7 Fail |
| 16 | 0.0057 | 0.0028 | 49.0 Fail |
| 17 | 0.0063 | 0.0027 | 42.4 Fail |
| 18 | 0.0065 | 0.0026 | 40.0 Fail |
| 19 | 0.0064 | 0.0020 | 31.3 Fail |
| 20 | 0.0051 | 0.0016 | 32.4 Fail |
| 21 | 0.0044 | 0.0018 | 40.5 Fail |
| 22 | 0.0041 | 0.0016 | 39.6 Fail |
| 23 | 0.0035 | 0.0015 | 42.9 Fail |
| 24 | 0.0042 | 0.0022 | 51.6 Fail |
| 25 | 0.0046 | 0.0017 | 37.0 Fail |
| 26 | 0.0047 | 0.0021 | 44.1 Fail |
| 27 | 0.0049 | 0.0018 | 36.7 Fail |
| 28 | 0.0047 | 0.0016 | 34.8 Fail |
| 29 | 0.0036 | 0.0013 | 36.7 Fail |
| Mar1 | 0.0034 | 0.0016 | 45.8 Fail |
| 2 | 0.0034 | 0.0015 | 44.6 Fail |
| 3 | 0.0034 | 0.0016 | 48.6 Fail |
| 4 | 0.0034 | 0.0015 | 42.9 Fail |
| 5 | 0.0035 | 0.0017 | 48.6 Fail |
| 6 | 0.0031 | 0.0012 | 39.3 Fail |
| 7 | 0.0027 | 0.0015 | 55.2 Fail |
| 8 | 0.0033 | 0.0019 | 56.6 Fail |
| 9 | 0.0035 | 0.0015 | 42.6 Fail |
| 10 | 0.0032 | 0.0014 | 44.5 Fail |
| 11 | 0.0034 | 0.0016 | 45.9 Fail |
| 12 | 0.0035 | 0.0017 | 47.1 Fail |


| 13 | 0.0033 | 0.0014 | 41.0 Fail |
| :---: | :---: | :---: | :---: |
| 14 | 0.0034 | 0.0015 | 44.5 Fail |
| 15 | 0.0031 | 0.0013 | 42.1 Fail |
| 16 | 0.0027 | 0.0011 | 41.7 Fail |
| 17 | 0.0025 | 0.0012 | 47.9 Fail |
| 18 | 0.0022 | 0.0009 | 42.4 Fail |
| 19 | 0.0024 | 0.0011 | 47.2 Fail |
| 20 | 0.0023 | 0.0011 | 49.6 Fail |
| 21 | 0.0024 | 0.0013 | 54.1 Fail |
| 22 | 0.0033 | 0.0019 | 57.8 Fail |
| 23 | 0.0031 | 0.0014 | 44.2 Fail |
| 24 | 0.0029 | 0.0014 | 47.7 Fail |
| 25 | 0.0027 | 0.0012 | 44.3 Fail |
| 26 | 0.0027 | 0.0016 | 59.6 Fail |
| 27 | 0.0028 | 0.0013 | 45.5 Fail |
| 28 | 0.0027 | 0.0014 | 52.8 Fail |
| 29 | 0.0031 | 0.0015 | 49.5 Fail |
| 30 | 0.0030 | 0.0012 | 39.7 Fail |
| 31 | 0.0028 | 0.0011 | 40.7 Fail |
| Apr1 | 0.0021 | 0.0009 | 41.4 Fail |
| 2 | 0.0014 | 0.0008 | 53.0 Fail |
| 3 | 0.0016 | 0.0009 | 56.2 Fail |
| 4 | 0.0019 | 0.0011 | 57.8 Fail |
| 5 | 0.0020 | 0.0010 | 49.5 Fail |
| 6 | 0.0018 | 0.0008 | 47.3 Fail |
| 7 | 0.0016 | 0.0010 | 61.3 Fail |
| 8 | 0.0019 | 0.0012 | 66.0 Fail |
| 9 | 0.0019 | 0.0011 | 58.4 Fail |
| 10 | 0.0020 | 0.0009 | 45.1 Fail |
| 11 | 0.0016 | 0.0011 | 68.1 Fail |
| 12 | 0.0016 | 0.0009 | 54.3 Fail |
| 13 | 0.0011 | 0.0006 | 57.2 Fail |
| 14 | 0.0008 | 0.0007 | 82.6 Pass |
| 15 | 0.0007 | 0.0005 | 69.3 Fail |
| 16 | 0.0007 | 0.0008 | 107.7 Pass |
| 17 | 0.0009 | 0.0005 | 60.1 Fail |
| 18 | 0.0007 | 0.0006 | 88.2 Pass |
| 19 | 0.0009 | 0.0011 | 111.9 Pass |
| 20 | 0.0011 | 0.0007 | 59.7 Fail |
| 21 | 0.0010 | 0.0006 | 65.1 Fail |
| 22 | 0.0009 | 0.0008 | 89.9 Pass |
| 23 | 0.0013 | 0.0011 | 84.9 Pass |
| 24 | 0.0011 | 0.0007 | 61.4 Fail |
| 25 | 0.0007 | 0.0004 | 52.0 Fail |
| 26 | 0.0006 | 0.0007 | 133.5 Fail |
| 27 | 0.0006 | 0.0005 | 93.7 Pass |
| 28 | 0.0005 | 0.0005 | 107.8 Pass |
| 29 | 0.0004 | 0.0005 | 120.8 Fail |
| 30 | 0.0005 | 0.0007 | 139.1 Fail |
| May1 | 0.0009 | 0.0010 | 103.2 Pass |
| 2 | 0.0008 | 0.0006 | 75.6 Fail |
| 3 | 0.0007 | 0.0006 | 80.4 Pass |
| 4 | 0.0007 | 0.0008 | 116.9 Pass |
| 5 | 0.0007 | 0.0007 | 98.8 Pass |
| 6 | 0.0006 | 0.0005 | 85.9 Pass |
| 7 | 0.0005 | 0.0005 | 87.9 Pass |
| 8 | 0.0004 | 0.0004 | 90.6 Pass |
| 9 | 0.0003 | 0.0003 | 99.7 Pass |


| 10 | 0.0002 | 0.0004 | 193.6 Fail |
| :---: | :---: | :---: | :---: |
| 11 | 0.0002 | 0.0005 | 211.8 Fail |
| 12 | 0.0002 | 0.0005 | 260.3 Fail |
| 13 | 0.0002 | 0.0006 | 270.7 Fail |
| 14 | 0.0002 | 0.0003 | 215.9 Fail |
| 15 | 0.0001 | 0.0004 | 351.5 Fail |
| 16 | 0.0002 | 0.0006 | 266.0 Fail |
| 17 | 0.0002 | 0.0004 | 154.5 Fail |
| 18 | 0.0002 | 0.0003 | 214.4 Fail |
| 19 | 0.0001 | 0.0005 | 414.9 Fail |
| 20 | 0.0002 | 0.0004 | 236.1 Fail |
| 21 | 0.0001 | 0.0003 | 238.7 Fail |
| 22 | 0.0001 | 0.0004 | 278.4 Fail |
| 23 | 0.0002 | 0.0005 | 286.5 Fail |
| 24 | 0.0002 | 0.0004 | 241.4 Fail |
| 25 | 0.0002 | 0.0005 | 225.1 Fail |
| 26 | 0.0002 | 0.0005 | 251.8 Fail |
| 27 | 0.0001 | 0.0004 | 242.2 Fail |
| 28 | 0.0001 | 0.0004 | 379.9 Fail |
| 29 | 0.0001 | 0.0005 | 366.9 Fail |
| 30 | 0.0001 | 0.0004 | 363.7 Fail |
| 31 | 0.0001 | 0.0005 | 625.3 Fail |
| Jun1 | 0.0001 | 0.0006 | 504.6 Fail |
| 2 | 0.0002 | 0.0003 | 203.5 Fail |
| 3 | 0.0001 | 0.0003 | 302.8 Fail |
| 4 | 0.0001 | 0.0005 | 408.2 Fail |
| 5 | 0.0003 | 0.0004 | 173.0 Fail |
| 6 | 0.0002 | 0.0005 | 252.3 Fail |
| 7 | 0.0001 | 0.0004 | 303.8 Fail |
| 8 | 0.0001 | 0.0005 | 416.6 Fail |
| 9 | 0.0002 | 0.0005 | 282.3 Fail |
| 10 | 0.0001 | 0.0003 | 230.3 Fail |
| 11 | 0.0001 | 0.0004 | 408.9 Fail |
| 12 | 0.0001 | 0.0003 | 552.4 Fail |
| 13 | 0.0000 | 0.0003 | 672.0 Fail |
| 14 | 0.0000 | 0.0004 | 1439.0 Fail |
| 15 | 0.0000 | 0.0003 | 1254.6 Fail |
| 16 | 0.0000 | 0.0004 | 1850.3 Fail |
| 17 | 0.0000 | 0.0003 | 1367.2 Fail |
| 18 | 0.0000 | 0.0002 | 712.9 Fail |
| 19 | 0.0000 | 0.0002 | 737.1 Fail |
| 20 | 0.0000 | 0.0004 | 1780.0 Fail |
| 21 | 0.0000 | 0.0003 | 1191.0 Fail |
| 22 | 0.0000 | 0.0002 | 1081.3 Fail |
| 23 | 0.0000 | 0.0006 | 1828.2 Fail |
| 24 | 0.0000 | 0.0003 | 653.6 Fail |
| 25 | 0.0000 | 0.0003 | 1223.6 Fail |
| 26 | 0.0000 | 0.0003 | 1843.5 Fail |
| 27 | 0.0000 | 0.0002 | 2298.0 Fail |
| 28 | 0.0000 | 0.0002 | 2535.8 Fail |
| 29 | 0.0000 | 0.0004 | 2973.6 Fail |
| 30 | 0.0000 | 0.0003 | 2325.5 Fail |
| Jul1 | 0.0000 | 0.0003 | 2615.8 Fail |
| 2 | 0.0000 | 0.0002 | 2882.6 Fail |
| 3 | 0.0000 | 0.0002 | 3028.4 Fail |
| 4 | 0.0000 | 0.0002 | 4925.2 Fail |
| 5 | 0.0000 | 0.0003 | 4015.5 Fail |
| 6 | 0.0000 | 0.0001 | 1753.0 Fail |


| 7 | 0.0000 | 0.0004 | 2834.6 Fail |
| :---: | :---: | :---: | :---: |
| 8 | 0.0000 | 0.0003 | 2914.3 Fail |
| 9 | 0.0000 | 0.0001 | 2089.3 Fail |
| 10 | 0.0000 | 0.0002 | 4795.2 Fail |
| 11 | 0.0000 | 0.0002 | 3231.7 Fail |
| 12 | 0.0000 | 0.0003 | 2349.4 Fail |
| 13 | 0.0000 | 0.0001 | 949.0 Fail |
| 14 | 0.0000 | 0.0002 | 661.9 Fail |
| 15 | 0.0000 | 0.0002 | 1098.8 Fail |
| 16 | 0.0000 | 0.0002 | 1726.9 Fail |
| 17 | 0.0000 | 0.0002 | 3767.9 Fail |
| 18 | 0.0000 | 0.0001 | 801.1 Fail |
| 19 | 0.0000 | 0.0001 | 1220.0 Fail |
| 20 | 0.0000 | 0.0001 | 2884.3 Fail |
| 21 | 0.0000 | 0.0001 | 2508.1 Fail |
| 22 | 0.0000 | 0.0000 | 1520.5 Fail |
| 23 | 0.0000 | 0.0001 | 4866.2 Fail |
| 24 | 0.0000 | 0.0001 | 10892.2 Fail |
| 25 | 0.0000 | 0.0002 | 36437.3 Fail |
| 26 | 0.0000 | 0.0001 | 30675.5 Fail |
| 27 | 0.0000 | 0.0001 | 15668.7 Fail |
| 28 | 0.0000 | 0.0000 | 12434.5 Fail |
| 29 | 0.0000 | 0.0000 | 10027.1 Fail |
| 30 | 0.0000 | 0.0000 | 13800.8 Fail |
| 31 | 0.0000 | 0.0001 | 20376.1 Fail |
| Aug1 | 0.0000 | 0.0001 | 32581.0 Fail |
| 2 | 0.0000 | 0.0002 | 109093.5 Fail |
| 3 | 0.0000 | 0.0002 | 36233.0 Fail |
| 4 | 0.0000 | 0.0001 | 36089.2 Fail |
| 5 | 0.0000 | 0.0001 | 59739.0 Fail |
| 6 | 0.0000 | 0.0001 | 119021.3 Fail |
| 7 | 0.0000 | 0.0001 | 209502.0 Fail |
| 8 | 0.0000 | 0.0001 | 444201.9 Fail |
| 9 | 0.0000 | 0.0001 | 82307.2 Fail |
| 10 | 0.0000 | 0.0002 | 9365.2 Fail |
| 11 | 0.0000 | 0.0001 | 3720.2 Fail |
| 12 | 0.0000 | 0.0002 | 22007.4 Fail |
| 13 | 0.0000 | 0.0001 | 27570.0 Fail |
| 14 | 0.0000 | 0.0003 | 66802.2 Fail |
| 15 | 0.0000 | 0.0002 | 51013.7 Fail |
| 16 | 0.0000 | 0.0003 | 71684.9 Fail |
| 17 | 0.0000 | 0.0003 | 61477.0 Fail |
| 18 | 0.0000 | 0.0001 | 21906.1 Fail |
| 19 | 0.0000 | 0.0002 | 69569.2 Fail |
| 20 | 0.0000 | 0.0002 | 63425.4 Fail |
| 21 | 0.0000 | 0.0002 | 138838.1 Fail |
| 22 | 0.0000 | 0.0002 | 70073.8 Fail |
| 23 | 0.0000 | 0.0004 | 34280.0 Fail |
| 24 | 0.0000 | 0.0003 | 16765.7 Fail |
| 25 | 0.0000 | 0.0003 | 17632.1 Fail |
| 26 | 0.0000 | 0.0004 | 17263.4 Fail |
| 27 | 0.0000 | 0.0004 | 20034.0 Fail |
| 28 | 0.0000 | 0.0005 | 22806.5 Fail |
| 29 | 0.0000 | 0.0002 | 7890.0 Fail |
| 30 | 0.0000 | 0.0003 | 11835.6 Fail |
| 31 | 0.0000 | 0.0006 | 9798.2 Fail |
| Sep1 | 0.0000 | 0.0005 | 10333.5 Fail |
| 2 | 0.0000 | 0.0004 | 10582.4 Fail |


| 3 | 0.0000 | 0.0002 | 8293.5 Fail |
| :---: | :---: | :---: | :---: |
| 4 | 0.0000 | 0.0004 | 21895.6 Fail |
| 5 | 0.0000 | 0.0003 | 28241.2 Fail |
| 6 | 0.0000 | 0.0002 | 32605.1 Fail |
| 7 | 0.0000 | 0.0003 | 20227.3 Fail |
| 8 | 0.0000 | 0.0003 | 12579.7 Fail |
| 9 | 0.0000 | 0.0004 | 20286.8 Fail |
| 10 | 0.0000 | 0.0003 | 23423.3 Fail |
| 11 | 0.0000 | 0.0002 | 20872.0 Fail |
| 12 | 0.0000 | 0.0003 | 45050.1 Fail |
| 13 | 0.0000 | 0.0003 | 81676.7 Fail |
| 14 | 0.0000 | 0.0006 | 85293.6 Fail |
| 15 | 0.0000 | 0.0005 | 60850.7 Fail |
| 16 | 0.0000 | 0.0005 | 18628.4 Fail |
| 17 | 0.0000 | 0.0007 | 27181.0 Fail |
| 18 | 0.0000 | 0.0004 | 14046.8 Fail |
| 19 | 0.0000 | 0.0006 | 21134.7 Fail |
| 20 | 0.0000 | 0.0004 | 14421.3 Fail |
| 21 | 0.0000 | 0.0005 | 9127.6 Fail |
| 22 | 0.0000 | 0.0005 | 4393.3 Fail |
| 23 | 0.0000 | 0.0006 | 6386.7 Fail |
| 24 | 0.0000 | 0.0004 | 5712.7 Fail |
| 25 | 0.0000 | 0.0003 | 6790.6 Fail |
| 26 | 0.0000 | 0.0006 | 19377.1 Fail |
| 27 | 0.0000 | 0.0007 | 19844.9 Fail |
| 28 | 0.0000 | 0.0004 | 11484.7 Fail |
| 29 | 0.0000 | 0.0003 | 7777.2 Fail |
| 30 | 0.0000 | 0.0007 | 8165.5 Fail |
| Oct1 | 0.0000 | 0.0006 | 5676.4 Fail |
| 2 | 0.0000 | 0.0005 | 6766.6 Fail |
| 3 | 0.0000 | 0.0004 | 8577.2 Fail |
| 4 | 0.0000 | 0.0007 | 10351.4 Fail |
| 5 | 0.0000 | 0.0006 | 10894.8 Fail |
| 6 | 0.0002 | 0.0012 | 508.8 Fail |
| 7 | 0.0003 | 0.0009 | 345.3 Fail |
| 8 | 0.0001 | 0.0009 | 604.7 Fail |
| 9 | 0.0001 | 0.0009 | 913.0 Fail |
| 10 | 0.0001 | 0.0008 | 1434.6 Fail |
| 11 | 0.0000 | 0.0007 | 1976.2 Fail |
| 12 | 0.0000 | 0.0007 | 2951.7 Fail |
| 13 | 0.0000 | 0.0007 | 4340.0 Fail |
| 14 | 0.0000 | 0.0008 | 3078.5 Fail |
| 15 | 0.0000 | 0.0006 | 1917.7 Fail |
| 16 | 0.0000 | 0.0008 | 2245.3 Fail |
| 17 | 0.0000 | 0.0010 | 3037.5 Fail |
| 18 | 0.0000 | 0.0010 | 2699.5 Fail |
| 19 | 0.0000 | 0.0011 | 3255.6 Fail |
| 20 | 0.0001 | 0.0015 | 2722.7 Fail |
| 21 | 0.0001 | 0.0011 | 1644.4 Fail |
| 22 | 0.0001 | 0.0009 | 664.9 Fail |
| 23 | 0.0001 | 0.0012 | 1185.7 Fail |
| 24 | 0.0001 | 0.0012 | 1328.9 Fail |
| 25 | 0.0001 | 0.0013 | 1733.7 Fail |
| 26 | 0.0002 | 0.0017 | 1034.4 Fail |
| 27 | 0.0002 | 0.0014 | 617.6 Fail |
| 28 | 0.0003 | 0.0013 | 405.0 Fail |
| 29 | 0.0004 | 0.0011 | 322.1 Fail |
| 30 | 0.0004 | 0.0014 | 410.0 Fail |


| 31 | 0.0007 | 0.0014 | 190.4 Fail |
| :---: | :---: | :---: | :---: |
| Nov1 | 0.0005 | 0.0015 | 280.4 Fail |
| 2 | 0.0004 | 0.0017 | 403.1 Fail |
| 3 | 0.0003 | 0.0017 | 512.2 Fail |
| 4 | 0.0003 | 0.0014 | 445.1 Fail |
| 5 | 0.0003 | 0.0013 | 443.4 Fail |
| 6 | 0.0004 | 0.0016 | 377.5 Fail |
| 7 | 0.0005 | 0.0011 | 251.1 Fail |
| 8 | 0.0005 | 0.0016 | 337.9 Fail |
| 9 | 0.0007 | 0.0017 | 248.2 Fail |
| 10 | 0.0007 | 0.0021 | 282.6 Fail |
| 11 | 0.0008 | 0.0019 | 230.3 Fail |
| 12 | 0.0009 | 0.0019 | 212.4 Fail |
| 13 | 0.0009 | 0.0020 | 218.3 Fail |
| 14 | 0.0011 | 0.0017 | 160.8 Fail |
| 15 | 0.0013 | 0.0019 | 146.8 Fail |
| 16 | 0.0018 | 0.0022 | 124.4 Fail |
| 17 | 0.0014 | 0.0021 | 144.5 Fail |
| 18 | 0.0014 | 0.0020 | 141.0 Fail |
| 19 | 0.0019 | 0.0023 | 121.6 Fail |
| 20 | 0.0019 | 0.0018 | 92.7 Pass |
| 21 | 0.0020 | 0.0024 | 119.3 Pass |
| 22 | 0.0024 | 0.0023 | 95.2 Pass |
| 23 | 0.0035 | 0.0032 | 91.4 Pass |
| 24 | 0.0038 | 0.0030 | 78.7 Fail |
| 25 | 0.0041 | 0.0027 | 66.6 Fail |
| 26 | 0.0033 | 0.0019 | 58.7 Fail |
| 27 | 0.0028 | 0.0021 | 74.3 Fail |
| 28 | 0.0028 | 0.0019 | 66.7 Fail |
| 29 | 0.0030 | 0.0027 | 88.7 Pass |
| 30 | 0.0033 | 0.0024 | 72.4 Fail |
| Dec1 | 0.0032 | 0.0026 | 79.7 Fail |
| 2 | 0.0035 | 0.0028 | 79.3 Fail |
| 3 | 0.0039 | 0.0025 | 63.9 Fail |
| 4 | 0.0040 | 0.0026 | 66.7 Fail |
| 5 | 0.0040 | 0.0025 | 63.1 Fail |
| 6 | 0.0038 | 0.0022 | 57.6 Fail |
| 7 | 0.0038 | 0.0022 | 59.3 Fail |
| 8 | 0.0033 | 0.0018 | 55.2 Fail |
| 9 | 0.0030 | 0.0022 | 74.6 Fail |
| 10 | 0.0036 | 0.0023 | 64.3 Fail |
| 11 | 0.0038 | 0.0025 | 65.4 Fail |
| 12 | 0.0038 | 0.0019 | 50.0 Fail |
| 13 | 0.0038 | 0.0023 | 61.0 Fail |
| 14 | 0.0042 | 0.0022 | 53.3 Fail |
| 15 | 0.0042 | 0.0022 | 51.6 Fail |
| 16 | 0.0046 | 0.0024 | 51.5 Fail |
| 17 | 0.0040 | 0.0019 | 49.1 Fail |
| 18 | 0.0033 | 0.0018 | 54.8 Fail |
| 19 | 0.0037 | 0.0025 | 67.4 Fail |
| 20 | 0.0041 | 0.0024 | 59.2 Fail |
| 21 | 0.0045 | 0.0024 | 54.5 Fail |
| 22 | 0.0043 | 0.0021 | 48.1 Fail |
| 23 | 0.0037 | 0.0020 | 53.8 Fail |
| 24 | 0.0037 | 0.0019 | 53.2 Fail |
| 25 | 0.0042 | 0.0023 | 55.3 Fail |
| 26 | 0.0052 | 0.0023 | 44.6 Fail |
| 27 | 0.0045 | 0.0020 | 43.8 Fail |

48.9 Fail
$0.0047 \quad 0.0024$
51.4 Fail
$0.0040 \quad 0.0016$ $0.0040 \quad 0.0020$

## LID Report

| LID Technique | Used for Treatment? | Total Volume Needs <br> Treatment (ac-ft) | Volume Through Facility (ac-ft) | Infiltration Volume (ac-ft) | Cumulative Volume Infiltration Credit | Percent Volume Infiltrated | Water Quality | $\begin{array}{\|l} \text { Percent } \\ \text { Water Quality } \\ \text { Treated } \end{array}$ | Comment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permeable Parkling Lot POC | V | 257.43 | 282.89 | 282.89 | V | 100.00 | 282.89 | 100.00 | Treat. Credit |
| Total Volume Infiltrated |  | 257.43 | 282.89 | 282.89 |  | 100.00 | 282.89 | $\begin{array}{r} 283 / 283= \\ 100 \% \end{array}$ | Treat. Credit $=100 \%$ |
| Compliance with LID Standard 8\% of 2-yr to 50\% of 2-yr |  |  |  |  |  |  |  |  | Duration Analysis Result = Passed |
|  |  |  |  |  |  |  |  |  |  |

## POC 2

POC \#2 was not reported because POC must exist in both scenarios and both scenarios must have been run.

## Model Default Modifications

Total of 0 changes have been made.

## PERLND Changes

No PERLND changes have been made.

## IMPLND Changes

No IMPLND changes have been made.


## Predeveloped UCI File

RUN
GLOBAL
WWHM4 model simulation
START 19011001 END 20590930
RUN INTERP OUTPUT
RESUME 0 RUN 1
UNIT SYSTEM 1
END GLOBAL
FILES

| <File> | <Un\#> | <-----------File Name-------------------------------------** |
| :--- | ---: | :--- | :--- |
| <-ID-> |  |  |
| WDM | 26 | 20083 Permeable Pavement.wdm |
| MESSU | 25 | Pre20083 Permeable Pavement.MES |
|  | 27 | Pre20083 Permeable Pavement.L61 |
|  | 28 | Pre20083 Permeable Pavement.L62 |
|  | 30 | Poc20083 Permeable Pavementi.dat |

END FILES
OPN SEQUENCE
INGRP INDELT 00:15
PERLND 10
COPY 501

DISPLY 1
END INGRP
END OPN SEQUENCE
DISPLY
DISPLY-INFO1
\# - \#<----------Title----------->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND

END DISPLY-INFO1
END DISPLY
COPY
TIMESERIES
\# - \# NPT NMN ***

501 1 1
END TIMESERIES
END COPY
GENER
OPCODE
\# \# OPCD ***
END OPCODE
PARM
\# \# K ***

END PARM
END GENER
PERLND
GEN-INFO

| <PLS | - NBLKS | Unit-systems | Printer |
| :---: | :---: | :---: | :---: |
| \# - \# |  | User t-series | ngl Metr |
|  |  | in out | *** |

10 C, Forest, Flat $10 \begin{array}{lllllll} & 1 & 1 & 1 & 27 & 0\end{array}$
END GEN-INFO
*** Section PWATER***
ACTIVITY


END ACTIVITY
PRINT-INFO
$<\mathrm{PLS}>* * * * * * * * * * * * * * * * *$ Print-flags ***************************** PIVL PYR
\# - \# ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *********
$10 \begin{array}{llllllllllllllll} & 0 & 0 & 4 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & 9\end{array}$
END PRINT-INFO

```
PWAT-PARM1
    <PLS > PWATER variable monthly parameter value flags ***
    \# - \# CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
    \(10 \begin{array}{llllllllllll}0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0\end{array}\)
END PWAT-PARM1
PWAT-PARM2
```



```
    \(\begin{array}{llllrrrr}10 & 0 & 4.5 & 0.08 & 400 & 0.05 & 0.5 & 0.996\end{array}\)
END PWAT-PARM2
PWAT-PARM3
```



```
    \(\begin{array}{llllllll}10 & 0 & 0 & 2 & 0 & 0 & 0\end{array}\)
END PWAT-PARM3
PWAT-PARM4
\begin{tabular}{rrrrrrrr} 
<PLS \(>\) & PWATER & input info: Part 4 & & & & & \\
\(\#-\) & \(\#\) & CEPSC & UZSN & NSUR & INTFW & IRC & LZETP \\
10 & & 0.2 & 0.5 & 0.35 & 6 & 0.5 & 0.7
\end{tabular}
    60.500 .7
END PWAT-PARM4
PWAT-STATE1
    <PLS > *** Initial conditions at start of simulation
                        ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
    \# - \# *** CEPS SURS UZS \(\quad\) IFWS \(\quad\) LZS AGWS
    \(\begin{array}{llllllll}10 & 0 & 0 & 0 & 0 & 2.5 & 1 & 0\end{array}\)
END PWAT-STATE1
```

```
END PERLND
IMP LND
    GEN-INFO
        <PLS ><-------Name-------> Unit-systems Printer ***
        # - # User t-series Engl Metr ***
    END GEN-INFO
    *** Section IWATER***
    ACTIVITY
        <PLS > ************* Active Sections *****************************
        # - # ATMP SNOW IWAT SLD IWG IQAL ***
    END ACTIVITY
    PRINT-INFO
        <ILS > ******** Print-flags ********* PIVL PYR
        # - # ATMP SNOW IWAT SLD IWG IQAL *********
    END PRINT-INFO
    IWAT-PARM1
        <PLS > IWATER variable monthly parameter value flags ***
        # - # CSNO RTOP VRS VNN RTLI
    END IWAT-PARM1
    IWAT-PARM2
        <PLS > IWATER input info: Part 2 ***
        # - # *** LSUR SLSUR NSUR RETSC
    END IWAT-PARM2
    IWAT-PARM3
        <PLS > IWATER input info: Part 3 ***
        # - # ***PETMAX PETMIN
    END IWAT-PARM3
    IWAT-STATE1
    <PLS > *** Initial conditions at start of simulation
    # - # *** RETS SURS
    END IWAT-STATE1
```

END IMPLND
SCHEMATIC

| <-Source-> | <--Area--> | <-Target-> |  | MBLK | *** |
| :---: | :---: | :---: | :---: | :---: | :---: |
| <Name> \# | <-factor-> | <Name> | \# | Tbl\# | *** |
| Pre-Dev*** |  |  |  |  |  |
| PERLND 10 | 1.082 | COPY | 501 | 12 |  |
| PERLND 10 | 1.082 | COPY | 501 | 13 |  |

```
******Routing*******
```

END SCHEMATIC

NETWORK



END HYDR-PARM1


END HYDR-INIT
END RCHRES
SPEC-ACTIONS
END SPEC-ACTIONS
FTABLES
END FTABLES
EXT SOURCES


| WDM 1 EVAP | ENGL | 1 | PERLND 1 | 999 EXTNL | PETINP |
| :---: | :---: | :---: | :---: | :---: | :---: |
| WDM 1 EVAP | ENGL | 1 | IMPLND 1 | 999 EXTNL | PETINP |
| END EXT SOURCES |  |  |  |  |  |
| EXT TARGETS |  |  |  |  |  |
| <-Volume-> <-Grp> | <-Member-><--Mult-->Tran |  | <-Volume-> | <Member> | Tsys Tgap Amd *** |
| <Name> \# | <Name> | \# \#<-factor->strg | <Name> \# | <Name> | tem strg strg*** |
| COPY 501 OUTPUT | MEAN | 1148.4 | WDM 501 | FLOW E | ENGL REPL |
| END EXT TARGETS |  |  |  |  |  |
| MASS-LINK |  |  |  |  |  |
| <Volume> <-Grp> |  | <-Member-><--Mult--> |  | <Target> | <-Grp> | >-Member->*** |
| ```<Name> MASS-LINK``` | <Name> 12 | \# \#<-factor-> | <Name> |  | <Name> \# \#*** |
| PERLND PWATER | SURO | 0.083333 | COPY | INPUT | MEAN |
| END MASS-LINK | 12 |  |  |  |  |
| MASS-LINK | 13 |  | COPY | INPUT | MEAN |
| PERLND PWATER | IFWO | 0.083333 |  |  |  |
| END MASS-LINK | 13 |  |  |  |  |

END MASS-LINK
END RUN

## Mitigated UCI File

RUN
GLOBAL
WWHM4 model simulation
START 19011001 END 20590930
RUN INTERP OUTPUT
RESUME 0 RUN 1
UNIT SYSTEM 1
END GLOBAL
FILES

```
<File> <Un#> <------------File Name---------------------------------------------
<-ID-> ***
WDM 26 20083 Permeable Pavement.wdm
MESSU 25 Mit20083 Permeable Pavement.MES
27 Mit20083 Permeable Pavement.L61
28 Mit20083 Permeable Pavement.L62
30 POC20083 Permeable Pavement1.dat
```

END FILES
OPN SEQUENCE
INGRP INDELT 00:15
PERLND 13
IMPLND 1
IMPLND 11
PERLND 38
IMPLND 17
IMPLND 16
RCHRES 1
COPY 1
COPY 501
COPY 601
DISPLY 1
END INGRP
END OPN SEQUENCE
DISPLY
DISPLY-INFO1
\# - \#<----------Title----------->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND
1 Permeable Parkling Lot MAX $\quad 1 \quad 2 \quad 30 \quad 9$
END DISPLY-INFO1
END DISPLY
COPY
TIMESERIES

| $\#-$ | $\#$ | NPT | NMN *** |
| ---: | ---: | ---: | ---: |
| 1 |  | 1 | 1 |
| 501 |  | 1 | 1 |
| 601 |  | 1 | 1 |

    END TIMESERIES
    END COPY
GENER
OPCODE
\# \# OPCD ***
END OPCODE
PARM
\# \# K ***
END PARM
END GENER
PERLND
GEN-INFO
<PLS ><-------Name------->NBLKS Unit-systems Printer ***
\# - \# User t-series Engl Metr ***
13 C, Pasture, Flat 1 1 1
38 C, Pasture, Flat $\quad 1 \begin{array}{lllllll}1 & 1 & 1 & 1 & 27 & 0\end{array}$
END GEN-INFO
*** Section PWATER***
ACTIVITY



END PERLND
IMP LND
GEN-INFO

| <PLS | ><-------Name-------> | Unit-systems |  |  | Printer |  | $\star * *$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \# - | \# | User | t-se | ies | Engl |  | *** |
|  |  |  | in | out |  |  | $\star \star \star$ |
| 1 | ROADS / FLAT | 1 | 1 | 1 | 27 | 0 |  |
| 11 | PARKING/FLAT | 1 | 1 | 1 | 27 | 0 |  |
| 17 | ROADS/FLAT LAT | 1 | 1 | 1 | 27 | 0 |  |
| 16 | Porous Pavement | 1 | 1 | 1 | 27 | 0 |  |

END GEN-INFO
*** Section IWATER***
ACTIVITY


```
PRINT-INFO
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|c|}
\hline <ILS & > & **** & ** & Print & lags & **** & **** & \multirow[t]{2}{*}{\[
\underset{\text { PIVL }}{2}
\]} & \multicolumn{2}{|l|}{L PYR} \\
\hline \# - & \# & ATMP & SNOW & IWAT & SLD & IWG & IQAL & & & * \\
\hline 1 & & 0 & 0 & 4 & 0 & 0 & 0 & 1 & 1 & 9 \\
\hline 11 & & 0 & 0 & 4 & 0 & 0 & 0 & 1 & 1 & 9 \\
\hline 17 & & 0 & 0 & 4 & 0 & 0 & 0 & 1 & 1 & 9 \\
\hline 16 & & 0 & 0 & 4 & 0 & 0 & 0 & 1 & 1 & 9 \\
\hline
\end{tabular}
END PRINT-INFO
IWAT-PARM1
    <PLS > IWATER variable monthly parameter value flags ***
        \# - \# CSNO RTOP VRS VNN RTLI ***
\begin{tabular}{rlllll}
1 & 0 & 0 & 0 & 0 & 0 \\
11 & 0 & 0 & 0 & 0 & 0 \\
17 & 0 & 0 & 0 & 0 & 0 \\
16 & 0 & 0 & 0 & 0 & 0
\end{tabular}
END IWAT-PARM1
IWAT-PARM2
    <PLS > IWATER input info: Part \(2 \quad * * *\)
    \# - \# *** LSUR SLSUR NSUR RETSC
\begin{tabular}{rrrrr}
1 & 400 & 0.01 & 0.1 & 0.1 \\
11 & 400 & 0.01 & 0.1 & 0.1 \\
17 & 400 & 0.01 & 0.1 & 0.1 \\
16 & 400 & 0.01 & 0.1 & 0.1
\end{tabular}
END IWAT-PARM2
IWAT-PARM3
    <PLS > IWATER input info: Part 3 ***
    \# - \# ***PETMAX PETMIN
    \(\begin{array}{rlr}1 & 0 & 0 \\ 11 & 0 & 0\end{array}\)
    \(\begin{array}{lll}17 & 0 & 0 \\ 16 & 0 & 0\end{array}\)
    END IWAT-PARM3
IWAT-STATE1
    <PLS > *** Initial conditions at start of simulation
    \# - \# *** RETS SURS
    \(\begin{array}{lll}1 & 0 & 0\end{array}\)
    1100
    \(17 \quad 0 \quad 0\)
    16 0 0
END IWAT-STATE1
```

END IMPLND

| SCHEMATIC |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| <-Source-> | <--Area--> | <-Targe | t-> | MBLK | ** |
| <Name> \# | <-factor-> | <Name> | \# | Tbl\# | *** |
| Landscaping*** |  |  |  |  |  |
| PERLND 38 | 0.2636 | IMPLND | 16 | 54 |  |
| PERLND 38 | 0.2636 | IMPLND | 16 | 55 |  |
| Curbing*** |  |  |  |  |  |
| IMPLND 17 | 0.0182 | IMPLND | 16 | 53 |  |
| IMPLND 16 | 0.66 | RCHRES | 1 | 5 |  |
| Bypass*** |  |  |  |  |  |
| PERLND 13 | 0.123 | COPY | 501 | 12 |  |
| PERLND 13 | 0.123 | COPY | 601 | 12 |  |
| PERLND 13 | 0.123 | COPY | 501 | 13 |  |
| PERLND 13 | 0.123 | COPY | 601 | 13 |  |
| IMPLND 1 | 0.084 | COPY | 501 | 15 |  |
| IMPLND 1 | 0.084 | COPY | 601 | 15 |  |
| IMPLND 11 | 0.029 | COPY | 501 | 15 |  |
| IMPLND 11 | 0.029 | COPY | 601 | 15 |  |
| ******Routing****** |  |  |  |  |  |
| PERLND 38 | 0.174 | COPY | 1 | 12 |  |
| PERLND 38 | 0.174 | COPY | 1 | 13 |  |
| IMPLND 17 | 0.012 | COPY | 1 | 15 |  |
| 20083 Permeable Pavement |  | 2022 5:19:36 | 6 PM |  |  |


| RCHRES 1 | 17 |
| :--- | :--- | :--- | :--- | :--- | :--- |


<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> \# <Name> \# \#<-factor->strg <Name> \# \# <Name> \# \# ***
END NETWORK
RCHRES
GEN-INFO

$\begin{array}{llllllllll}1 & \text { Permeable Parkli-010 } & 2 & 1 & 1 & 1 & 28 & 0 & 1\end{array}$
END GEN-INFO
*** Section RCHRES***
ACTIVITY
<PLS > ************* Active Sections *************************************)
\# - \# HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG ***
$1 \begin{array}{lllllllllll}1 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0\end{array}$
END ACTIVITY
PRINT-INFO

END PRINT-INFO
HYDR-PARM1
RCHRES Flags for each HYDR Section ***
$\begin{aligned} \# & \text { \# } \\ & \text { FG A1 A2 A3 OGFVFG for each } * * * \text { ODGTFG for each }\end{aligned}$
\# - \# VC A1 A2 A3 ODFVFG for each $* * *$ ODGTFG for each $\begin{aligned} & \text { FUNCT for each } \\ & \\ & \\ & \\ & \end{aligned}$
$\begin{array}{llllllllllllll}* & * & * & * & * & * & * & * & * & * & * & * & * & * \\ 0 & 1 & 0 & 0 & 4 & 5 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0\end{array}$

END HYDR-PARM1
HYDR-PARM2
$\left.\begin{array}{cccccccc}\#- & \# & \text { FTABNO } & \text { LEN } & \text { DELTH } & \text { STCOR } & \text { KS } & \text { DB50 }\end{array}\right]$ ***
END HYDR-PARM2
HYDR-INIT
RCHRES Initial conditions for each HYDR section ***
\# - \# *** VOL Initial value of COLIND Initial value of OUTDGT
*** ac-ft for each possible exit for each possible exit
<------><-------->
1 0
END HYDR-INIT
END RCHRES
SPEC-ACTIONS
END SPEC-ACTIONS
FTABLES
FTABLE 1


| 0.116667 | 0.660023 | 0.011550 | 0.000000 | 0.066552 |
| :--- | :--- | :--- | :--- | :--- |
| 0.136111 | 0.660023 | 0.013475 | 0.000000 | 0.066552 |
| 0.155556 | 0.660023 | 0.015401 | 0.000000 | 0.066552 |
| 0.175000 | 0.660023 | 0.017326 | 0.000000 | 0.066552 |
| 0.194444 | 0.660023 | 0.019251 | 0.000000 | 0.066552 |
| 0.213889 | 0.660023 | 0.022459 | 0.000000 | 0.066552 |
| 0.233333 | 0.660023 | 0.025668 | 0.000000 | 0.066552 |
| 0.252778 | 0.660023 | 0.028876 | 0.000000 | 0.066552 |
| 0.272222 | 0.660023 | 0.032084 | 0.000000 | 0.066552 |
| 0.291667 | 0.660023 | 0.035293 | 0.000000 | 0.066552 |
| 0.311111 | 0.660023 | 0.038501 | 0.000000 | 0.066552 |
| 0.330556 | 0.660023 | 0.041710 | 0.000000 | 0.066552 |
| 0.350000 | 0.660023 | 0.044918 | 0.000000 | 0.066552 |
| 0.369444 | 0.660023 | 0.048127 | 0.000000 | 0.066552 |
| 0.388889 | 0.660023 | 0.051335 | 0.000000 | 0.066552 |
| 0.408333 | 0.660023 | 0.054544 | 0.000000 | 0.066552 |
| 0.427778 | 0.660023 | 0.057752 | 0.000000 | 0.066552 |
| 0.447222 | 0.660023 | 0.060960 | 0.000000 | 0.066552 |
| 0.466667 | 0.660023 | 0.064169 | 0.000000 | 0.066552 |
| 0.486111 | 0.660023 | 0.067377 | 0.000000 | 0.066552 |
| 0.505556 | 0.660023 | 0.070586 | 0.000000 | 0.066552 |
| 0.525000 | 0.660023 | 0.073794 | 0.000000 | 0.066552 |
| 0.544444 | 0.660023 | 0.077003 | 0.000000 | 0.066552 |
| 0.563889 | 0.660023 | 0.080211 | 0.000000 | 0.066552 |
| 0.583333 | 0.660023 | 0.083420 | 0.000000 | 0.066552 |
| 0.602778 | 0.660023 | 0.086628 | 0.000000 | 0.066552 |
| 0.622222 | 0.660023 | 0.089836 | 0.000000 | 0.066552 |
| 0.641667 | 0.660023 | 0.093045 | 0.000000 | 0.066552 |
| 0.661111 | 0.660023 | 0.096253 | 0.000000 | 0.066552 |
| 0.680556 | 0.660023 | 0.099462 | 0.000000 | 0.066552 |
| 0.700000 | 0.660023 | 0.102670 | 0.000000 | 0.066552 |
| 0.719444 | 0.660023 | 0.105879 | 0.000000 | 0.066552 |
| 0.738889 | 0.660023 | 0.109087 | 0.000000 | 0.066552 |
| 0.758333 | 0.660023 | 0.121921 | 0.000000 | 0.066552 |
| 0.777778 | 0.660023 | 0.134755 | 0.000000 | 0.066552 |
| 0.797222 | 0.660023 | 0.147588 | 0.000000 | 0.066552 |
| 0.816667 | 0.660023 | 0.160422 | 0.000000 | 0.066552 |
| 0.836111 | 0.660023 | 0.173256 | 0.000000 | 0.066552 |
| 0.855556 | 0.660023 | 0.186090 | 0.000000 | 0.066552 |
| 0.875000 | 0.660023 | 0.198924 | 0.000000 | 0.066552 |
| 0.894444 | 0.660023 | 0.211757 | 0.000000 | 0.066552 |
| 0.913889 | 0.660023 | 0.224591 | 0.000000 | 0.066552 |
| 0.933333 | 0.660023 | 0.237425 | 0.000000 | 0.066552 |
| 1.0 | 0.0 |  |  |  |


| 1.477778 | 0.660023 | 0.596771 | 18.64690 | 0.066552 |
| :---: | :---: | :---: | :---: | :---: |
| 1.497222 | 0.660023 | 0.609604 | 19.79673 | 0.066552 |
| 1.516667 | 0.660023 | 0.622438 | 20.96927 | 0.066552 |
| 1.536111 | 0.660023 | 0.635272 | 22.16409 | 0.066552 |
| 1.555556 | 0.660023 | 0.648106 | 23.38077 | 0.066552 |
| 1.575000 | 0.660023 | 0.660940 | 24.61894 | 0.066552 |
| 1.594444 | 0.660023 | 0.673773 | 25.87823 | 0.066552 |
| 1.613889 | 0.660023 | 0.686607 | 27.15828 | 0.066552 |
| 1.633333 | 0.660023 | 0.699441 | 28.45877 | 0.066552 |
| 1.652778 | 0.660023 | 0.712275 | 29.77938 | 0.066552 |
| 1.672222 | 0.660023 | 0.725108 | 31.11981 | 0.066552 |
| 1.691667 | 0.660023 | 0.737942 | 32.47977 | 0.066552 |
| 1.711111 | 0.660023 | 0.750776 | 33.85898 | 0.066552 |
| 1.730556 | 0.660023 | 0.763610 | 35.25718 | 0.066552 |
| 1.750000 | 0.660023 | 0.776444 | 36.67411 | 0.066552 |
| END FTABLE | 1 |  |  |  |
| END FTABLES |  |  |  |  |



END EXT SOURCES


END MASS-LINK

END RUN

## Disclaimer

## Legal Notice

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Clear Creek Solutions, Inc.
6200 Capitol Blvd. Ste F
Olympia, WA. 98501
Toll Free 1(866)943-0304
Local (360)943-0304
www.clearcreeksolutions.com

WWHM2012

```
Project Name: 20083 Conveyance
Site Name: Olson Bros
Site Address: 2511 Inter Ave
City : Puyallup, WA
Report Date: 2/14/2022
Gage : 40 IN EAST
Data Start : 10/01/1901
Data End : 09/30/2059
Precip Scale: 1.00
Version Date: 2021/08/18
Version : 4.2.18
```

Low Flow Threshold for POC 1 : 50 Percent of the 2 Year
High Flow Threshold for POC 1: 50 year

| PREDEVELOPED LAND USE |  |
| :--- | :---: |
| Name : Pre-Dev |  |
| Bypass: No |  |
| GroundWater: No |  |
| Pervious Land Use |  |
| C, Forest, Flat | $\frac{\text { acre }}{.997}$ |
| Pervious Total | 0.997 |
| Impervious Land Use | acre |
| Impervious Total | 0 |
| Basin Total | 0.997 |

Element Flows To:
Surface Interflow Groundwater

```
MITIGATED LAND USE
Name : Post Dev
Bypass: No
```

GroundWater: No


POC \#2 was not reported because POC must exist in both scenarios and both scenarios must have been run. Perlnd and Implnd Changes
No changes have been made.

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## Channel Report

## 100-year 12-inch DIP @ 0.50\%

## Circular

Diameter (ft) $\quad=1.00$

Invert Elev (ft)
Slope (\%)
N -Value
Calculations
Compute by:
Known Q (cfs)
Known Q
$=0.33 \quad 100$-year flow rate

Highlighted
Depth (ft)
Q (cfs)
Area (sqft)
Velocity (ft/s)
Wetted Perim (ft)
Crit Depth, Yc (ft)
Top Width (ft)
EGL (ft)
$=0.24$
$=0.331$
$=0.15$
= 2.28
$=1.02$
$=0.24$
$=0.85$
$=0.32$


## APPENDIX D

Geotechnical Engineer's Report

December 10, 2021
Olson Brothers Pro Vac, LLC
c/o C.E.S. NW, Inc.
310 - 29 ${ }^{\text {th }}$ Street NE, Suite 101
Puyallup, Washington 98372
(253) 848-4282

Attn: Mr. Craig Deaver<br>cdeaver@cesnwinc.com

Stormwater Soils Report: Infiltration Feasibility Proposed Permeable Pavement 2511 Inter Avenue Puyallup, Washington PN: 2105200-180, -192<br>Doc ID: CES.ProVac.InterAve.SR

## INTRODUCTION

This soils report evaluates the feasibility of the site soils to support shallow infiltration of stormwater runoff from the proposed new hard surfacing to be installed at 2511 Inter Avenue in Puyallup, Washington. The site is currently a gravel surfaced contractor's yard. The approximate site location is shown on the attached Site Location Map, Figure 1.

Our understanding of the project is based on our email correspondence with Mr. Craig Deaver of C.E.S. NW, our review of the provided Cover Sheet by C.E.S NW Inc. dated October 21, 2020 our review of the available geologic and soils data, our December 9, 2020 site visit and subsurface explorations, our groundwater monitoring throughout the 2020/21 wet season, our understanding of the City of Puyallup development codes, and our experience in the area.

We understand the site consists of two separate tax parcels that are currently developed with an existing repair shop, paved and gravel parking areas, and utilities. We further understand that you would like to place an additional 8,771 square feet of new asphalt pavement at the site.

Because of the amount of proposed hard surfacing associated with the project, we understand the City of Puyallup is requiring a Soils Report be prepared in accordance with the 2014 Stormwater Management Manual for Western Washington (SWMMWW), which includes in-situ infiltration testing and wet season groundwater monitoring.

## SCOPE

The purpose of our services was to evaluate the surface and subsurface conditions at the site as a basis for determining the feasibility for onsite stormwater infiltration and providing pertinent conclusions and recommendations relative to stormwater management for the proposed permeable pavement. Specifically, our scope of services for the project included the following:

1. Reviewing the available geologic, hydrogeologic, and geotechnical data for the site area;
2. Exploring surface and subsurface conditions by reconnoitering the site and monitoring the excavation of two test pits to depths of 5.0 feet below existing grades at select locations across the site, and installing two shallow piezometers in each test pit;
3. Performing one EPA falling test in-situ infiltration test;
4. Describing surface and subsurface conditions, including soil type, depth to groundwater, and an estimate of seasonal high groundwater levels;
5. Monitoring groundwater levels at the site during the prescriptive wet season;
6. Providing our opinion about the feasibility of onsite infiltration in accordance with the 2014 SWMMWW, including a preliminary design infiltration rate based on grain size analysis and in-situ testing, as applicable;
7. Preparing this written Soils Report summarizing our site observations and conclusions, and our geotechnical recommendations and design criteria, along with the supporting data; and
8. Preparing a written Addendum Report following the groundwater monitoring period, which ends in April.

The above scope of work was completed in accordance with our Proposal for Geotechnical Engineering Services dated November 18, 2020. We received written authorization to proceed from you on November 19, 2020.

## SITE CONDITIONS

## Surface Conditions

The site consists of two contiguous parcels located at 2511 Inter Avenue within the City of Puyallup, Washington, within an area of existing residential and commercial development. Based on the information provided the Cover Sheet prepared by C.E.S. NW and Pierce County GIS, the west parcel is generally flagpole in shape, and the east parcel is generally rectangular in shape. When combined, these parcels form an irregularly shaped site. The full site measures approximately 115 to 195 feet wide (east to west) by about 408 feet long (north to south) and encompasses about 1.59 acres. The site is bounded by Inter Avenue to the south, by existing commercial development to the east and west, and by land being developed to the north.

The site is generally level, with a slight slope of about 1 percent down to the north. A large office and repair shop building is located in the northwest portion of the site, and the rest of the site is developed with gravel parking stalls for the ProVac trucks and paved or concrete parking stalls for automobiles. Total topographic relief across the site is on the order of about 1 to 2 feet.

Vegetation across the site generally was generally cleared, except for typical landscaping grass lawn surrounding the residence located southwest and adjacent to the site. Standing water was observed throughout the gravel parking area.

## Site Soils

The USDA Natural Resources Conservation Service (NRCS) Web Soil Survey maps the site as being underlain by Briscot silt loam (6A) soils. A copy of the NRCS soils map for the site area is included as Figure 3.

- Briscot silt loam (6A): The Briscot soils are derived from alluvium and are included in hydrologic soils group B/D. These soils typically form on slopes of 0 to 2 percent and are listed as having a "slight" erosion hazard when exposed.


## Site Geology

The draft Geologic Map of the Puyallup 7.5-minute Quadrangle, Washington by K.G. Troost maps the site as being underlain by alluvium (Qal). An excerpt of the above referenced geologic map is included as Figure 4.

- Alluvium (Qal): Alluvial soils generally consist of normally consolidated, stratified deposits of sand, silt, clay, and occasional peat that were deposited along the Puyallup River channel. The existing topography, as well as the surficial and shallow soils in the area, are the result of fluvial action, including down-cutting by the river, channel meandering and migration, and flood deposits. Alluvium typically offers unfavorable infiltration characteristics because of the silty nature of the soils.


## Subsurface Explorations

On December 9, 2020, a representative from GeoResources, LLC (GeoResources) visited the site and monitored the excavation of two test pits at selected locations across the site to depths of about 5.0 feet below the existing ground surface. The test pits were excavated by a licensed earthwork contractor under contract to GeoResources. Piezometers were installed at the termination depth of each test pit.

The specific number, locations, and depths of our explorations were selected based on the configuration of the proposed development and were adjusted in the field based on consideration for underground utilities, existing site conditions, site access limitations, and encountered stratigraphy. Test pit TP-1 was excavated on the adjacent property to the proposed project site because no other areas on the site were clear of utilities. The densities presented in the logs were based on the difficulty of excavation and our experience. Representative soil samples obtained from the test pits were placed in sealed plastic bags then taken to a laboratory for further examination and testing as deemed necessary. The test pits were then backfilled with the excavated soils and bucket tamped, but not otherwise compacted.

The subsurface explorations indicate the subsurface conditions at specific locations only, as actual subsurface conditions can vary across the site. Furthermore, the nature and extent of such variation would not become evident until additional explorations are performed or until construction activities have begun. Based on our experience, it is our opinion that the soils encountered in the explorations are generally representative of the soils at the site.

The approximate locations and numbers of our explorations are shown on the attached Site \& Exploration Plan, Figure 2. The soils encountered were visually classified in accordance with the Unified Soil Classification System (USCS) and ASTM D2488. The USCS is included in Appendix A as Figure A-1, while the descriptive logs of our explorations are included as Figure A-2.

## Subsurface Conditions

Our explorations encountered relatively uniform subsurface conditions that, in our opinion, generally confirmed the mapped stratigraphy. Our test pits generally encountered about $1 / 2$-foot of dark-colored topsoil with roots or 1 foot of crushed rock and reddish brown silty sand with gravel,
consistent with fill. Beneath the topsoil and fill, we observed about 0.8 feet of medium dense brown, orange iron stained silty sand or sandy silt in a moist to wet condition, mantling about 2.5 to 2.8 feet of soft gray, orange iron oxide stained silt in a moist condition. Medium dense gray orange iron oxide stained fine sand with silt was observed beneath the surficial soils to the termination depth of each test pit. We interpret these soils to be consistent with native alluvium soils. Table 1, below, summarizes the approximate thicknesses, depths, and elevations of selected soil layers.

TABLE 1:
APPROXIMATE THICKNESS, DEPTHS, AND ELEVATION OF SOIL TYPES ENCOUNTERED IN EXPLORATIONS

| Exploration <br> Number | Thickness <br> of <br> Topsoil/Fill <br> (feet) | Thickness of <br> Silty <br> Alluvium <br> (feet) | Depth to <br> Mottling <br> (feet) | Depth to <br> Groundwater <br> (feet) | Elevation of <br> Sandy Alluvium <br> (feet) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TP-1 | 0.5 | 3.3 | 0.5 | 2.0 (perched)/5.0 | 57.2 |  |  |
| TP-2 | 1.0 | 3.5 | 1.0 | N/E | 57.5 |  |  |
| Notes: Elevation datum: Provided Cover Sheet by CES NW Inc dated October 21,2020 |  |  |  |  |  |  | N/E: Not encountered |

## Groundwater Conditions

Perched groundwater was encountered at the time of excavation in TP-1 at a depth of 2.0 feet below the existing ground surface on top of the gray, orange iron oxide stained sandy silt alluvium. Additional groundwater seepage was observed at about 5 feet below existing grades in the sandy alluvium. No groundwater seepage was observed in test pit TP-2 at the time of excavation; however, an old drainage pipe with washed rock was encountered in the western portion of the test pit during the over dig of the infiltration test. Orange iron oxide staining, a form of mottling was observed throughout the soils in each of our test pit explorations. Mottling can be indicative of a seasonal or fluctuating groundwater table. We anticipate fluctuations in the local groundwater levels may occur in response to season, precipitation patterns, off-site construction activities, and site utilization.

We returned to the site throughout the prescriptive wet season observe the depth to groundwater within the piezometers installed in each test pit. Both piezometers had seasonal high groundwater at about 0.5 feet below existing grades. Table 2 summarizes the approximate depths and elevations of groundwater and mottling observed at the time of our explorations and our subsequent readings in both piezometers. We were unable to record groundwater levels during some days in TP-2 because a car had parked over the piezometer. The measurements from our groundwater monitoring are attached in Appendix B.

TABLE 2:
APPROXIMATE DEPTHS, AND ELEVATION OF GROUNDWATER ENCOUNTERED IN EXPLORATIONS

| Exploration Number | Depth to Groundwater (feet) | Elevation of Groundwater (feet) | Dated Measured |
| :---: | :---: | :---: | :---: |
| TP-1 | 2(perched), 5 0.5 0.5 0.5 0.5 1.1 1.8 1.7 0.5 1.5 2.4 2.4 1.7 2.1 | 59(perched), 56 60.5 60.5 60.5 60.5 59.9 59.2 59.3 60.5 59.5 58.6 58.6 59.3 58.9 | $12 / 9 / 2020$ $12 / 21 / 2020$ $12 / 31 / 2020$ $1 / 8 / 2021$ $1 / 14 / 2021$ $1 / 29 / 2021$ $2 / 5 / 2021$ $2 / 12 / 2021$ $2 / 16 / 2021$ $3 / 5 / 2021$ $3 / 12 / 2021$ $3 / 19 / 2021$ $3 / 26 / 2021$ $4 / 1 / 2021$ |
| TP-2 | NE 0.5 1.0 0.7 0.5 1.9 1.2 2.3 Inaccessible Inaccessible Inaccessible Inaccessible 2.2 | NE 61.5 <br> 61.0 <br> 61.3 <br> 61.5 <br> 60.1 <br> 60.8 <br> 59.7 <br> 59.8 | $\begin{gathered} \hline 12 / 9 / 2020 \\ 12 / 21 / 2020 \\ 12 / 31 / 2020 \\ 1 / 8 / 2021 \\ 1 / 14 / 2021 \\ 1 / 29 / 2021 \\ 2 / 5 / 2021 \\ 2 / 12 / 2021 \\ 2 / 16 / 2021 \\ 3 / 5 / 2021 \\ 3 / 12 / 2021 \\ 3 / 19 / 2021 \\ 3 / 26 / 2021 \\ 4 / 1 / 2021 \\ \hline \end{gathered}$ |

## Laboratory Testing

Geotechnical laboratory tests were performed on selected samples retrieved from the test pits to estimate index engineering properties of the soils encountered. Laboratory testing included visual soil classification per ASTM D2488 and ASTM D2487. We also submitted representative samples to an independent analytical laboratory for determination of organic content and cation exchange capacity. Organic content was determined per ASTM D2974 and cation exchange capacity was determined per SW846 9081. Test results are included in Appendix B.

## CONCLUSIONS AND RECOMMENDATIONS

Based on our site reconnaissance and subsurface explorations, it is our opinion that the infiltration of stormwater runoff generated onsite by the proposed development is not feasible for the site.

## Infiltration Recommendations

Mottled silty sand was encountered near the surface in both of our subsurface explorations, and groundwater was observed at about 0.5 feet below existing grades during our December 21, 2020 site visit to check the groundwater levels in each piezometer.

The City of Puyallup uses the 2012 Stormwater Management Manual for Western Washington, with 2014 updates ( 2014 SWMMWW). Per the 2014 SWMMWW, Volume V, BMP T5.15, a minimum of 1 foot of separation is required between the bottom of the storage course for permeable pavement and the top of an impermeable layer, such as mottling, or the sandy silt soils encountered at the site. Based on the conditions encountered, permeable pavement appears to be infeasible. We performed an EPA falling head test in the brown mottled silty sand in test pit TP-2 and measured an initial rate of 0.6 inches per hour. An EPA falling head test was chosen for this project because the use of a PIT would interfere with the function of the ProVac yard for that day, and in our opinion, would give an inaccurate rate for the soils encountered in our test pits. Based on the above, a longterm design rate of 0.04 inches per hour is applicable for this project, if the site grades can be adjusted to meet the required vertical separation to the seasonal high groundwater. This would require site grades to be raised on the order of 2 to 3 feet.

Per the 2014 SWMMWW, minimum cation exchange capacity of 5 milliequivalents per 100 milligrams of soil and 1 percent organic content is required for soils to provide adequate water quality treatment to the stormwater. Testing was conducted on the shallow soils at the site located at about 2 feet below existing grades by a third party laboratory. The organic content of the site soils were determined to be 5.79 and 9.94 percent per ASTM D: 2974-13, with a cation exchange capacity of 18.0 and 17.6 milliequivalents per 100 grams as determined by SW-846 Test Method 9081. The shallow onsite soils have the required treatment capacity per the 2014 SWMMWW.

Alternative stormwater management methods, such as detention or dispersion, should be considered for this project in accordance with the 2014 SWMMWW. All minimum setback requirements and infeasibility criteria per the 2014 SWMMWW should be considered prior to the selection of any stormwater facility for the proposed development.

## LIMITATIONS

We have prepared this report for use by CES NW Inc, and other members of the design team, for use in the design of a portion of this project. The data used in preparing this report and this report should be provided to prospective contractors for their bidding or estimating purposes only. Our report, conclusions and interpretations are based on our subsurface explorations, data from others and limited site reconnaissance, and should not be construed as a warranty of the subsurface conditions.

Variations in subsurface conditions are possible between the explorations and may also occur with time. A contingency for unanticipated conditions should be included in the budget and schedule. Sufficient monitoring, testing and consultation should be provided by our firm during construction to confirm that the conditions encountered are consistent with those indicated by the explorations, to


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provide recommendations for design changes should the conditions revealed during the work differ from those anticipated, and to evaluate whether earthwork and foundation installation activities comply with contract plans and specifications.

The scope of our services does not include services related to environmental remediation and construction safety precautions. Our recommendations are not intended to direct the contractor's methods, techniques, sequences or procedures, except as specifically described in our report for consideration in design.

If there are any changes in the loads, grades, locations, configurations or type of facilities to be constructed, the conclusions and recommendations presented in this report may not be fully applicable. If such changes are made, we should be given the opportunity to review our recommendations and provide written modifications or verifications, as appropriate.

We have appreciated the opportunity to be of service to you on this project. If you have any questions or comments, please do not hesitate to call at your earliest convenience.

Respectfully submitted,
GeoResources, LLC


Andrew Schnitger, EIT Engineer in Training


Keith S. Schembs, LEG
Principal


Eric W. Heller, PE, LG
Senior Geotechnical Engineer

AES:KSS:KEB/aes
DocID: CES.ProVac.InterAve.SR
Attachments: Figure 1: Site Location Map Figure 2: Site \& Exploration Plan Figure 3: NRCS Soils Map Figure 4: USGS Geologic Map Appendix A - Subsurface Explorations Appendix B - Laboratory Test Results


## Approximate Site Location

Map created from Pierce County Public GIS (https://matterhornwab.co.pierce.wa.us/publicgis/)


Not to Scale
Site Location Map
Olson Brothers Storage
2511 Inter Avenue
Puyallup, Washington
PN: 2105200-180, -192


Approximate Site Location
Map created from Pierce County Public GIS (https://matterhornwab.co.pierce.wa.us/publicgis/)
$\square$ Approximate test pit and piezometer location
—— Approximate site boundary

-     -         - Approximate locations of property lines


| GEORESOURCES | Site \& Exploration Plan <br> Olson Brothers Storage 2511 Inter Avenue Puyallup, Washington PN: 2105200-180, -192 |  |  |
| :---: | :---: | :---: | :---: |
| 4809 Pacific Hwy. E. \| Fife, WA 98424 | 253.896 .1011 | www. georesources.rocks | DocID: CES.ProVac.InterAve.F | December 2021 | Figure 2 |



## Approximate Site Location

Map created from Web Soil Survey (http://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx)

| Soil <br> Type | Soil Name | Parent Material | Slopes | Erosion Hazard | Hydrologic <br> Soils Group |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 6 A | Briscot silt loam | Alluvium | 0 to 2 | Slight | B/D |



Not to Scale
NRCS Soils Map
Olson Brothers Storage
2511 Inter Avenue
Puyallup, Washington
PN: 2105200-180, -192


## Approximate Site Location

An excerpt from the draft Geologic Map of the Puyallup 7.5-minute Quadrangle, Washington, by K.G. Troost (in review)

| Qal | Alluvium |
| :--- | :--- |



## USGS Geologic Map

Olson Brothers Storage 2511 Inter Avenue Puyallup, Washington
PN: 2105200-180, -192

## Appendix A

Subsurface Explorations

SOIL CLASSIFICATION SYSTEM

| MAJOR DIVISIONS |  |  | GROUP <br> SYMBOL | GROUP NAME |
| :---: | :---: | :---: | :---: | :---: |
| COARSE GRAINED SOILS | GRAVEL | CLEAN GRAVEL | GW | WELL-GRADED GRAVEL, FINE TO COARSE GRAVEL |
|  |  |  | GP | POORLY-GRADED GRAVEL |
|  | More than 50\% Of Coarse Fraction Retained on No. 4 Sieve | GRAVEL WITH FINES | GM | SILTY GRAVEL |
|  |  |  | GC | CLAYEY GRAVEL |
|  | SAND | CLEAN SAND | SW | WELL-GRADED SAND, FINE TO COARSE SAND |
| More than 50\% Retained on No. 200 Sieve | More than 50\% Of Coarse Fraction Passes No. 4 Sieve |  | SP | POORLY-GRADED SAND |
|  |  | SAND <br> WITH FINES | SM | SILTY SAND |
|  |  |  | SC | CLAYEY SAND |
| FINE GRAINED SOILS | SILT AND CLAY | INORGANIC | ML | SILT |
|  |  |  | CL | CLAY |
|  | Liquid Limit Less than 50 | ORGANIC | OL | ORGANIC SILT, ORGANIC CLAY |
|  | SILT AND CLAY | INORGANIC | MH | SILT OF HIGH PLASTICITY, ELASTIC SILT |
| More than 50\% Passes No. 200 Sieve |  |  | CH | CLAY OF HIGH PLASTICITY, FAT CLAY |
|  | Liquid Limit 50 or more | ORGANIC | OH | ORGANIC CLAY, ORGANIC SILT |
| HIGHLY ORGANIC SOILS |  |  | PT | PEAT |

NOTES:

1. Field classification is based on visual examination of soil in general accordance with ASTM D2488-90.
2. Soil classification using laboratory tests is based on ASTM D2487-90.
3. Description of soil density or consistency are based on
 soils, and or test data.

SOIL MOISTURE MODIFIERS:
Dry- Absence of moisture, dry to the touch
Moist- Damp, but no visible water
Wet- Visible free water or saturated, usually soil is obtained from below water table

## Unified Soils Classification System

## Olson Brothers Storage 2511 Inter Avenue <br> Puyallup, Washington <br> PN: 2105200-180, -192

## Test Pit TP-1

Location: South of existing residence, off of proposed permeable area Approximate Elevation: 61'


Excavated on: December 9, 2020

|  | Test Pit Logs <br> Olson Brothers Storage 2511 Inter Avenue Puyallup, Washington PN: 2105200-180, -192 |  |  |
| :---: | :---: | :---: | :---: |
| 4809 Pacific Hwy. E. \| Fife, WA 98424 | 253.896 .1011 | www. georesources.rocks | DocID: CES.ProVac.InterAve.F | December 2021 | Figure A-2 |

## Appendix B

Laboratory Results

2221 Ross Way • Tacoma, WA 98421 • (253) 272-4850 • Fax (253) 572-9838 • www.spectra-lab.com

11/18/2021

GeoResources, LLC
4809 Pacific Hwy E
Fife, WA 98424

| Project: | CES.Provac |
| :--- | :--- |
| Sample Matrix: | Soil |
| Date Sampled: | $11 / 12 / 2021$ |
| Date Received: | $11 / 15 / 2021$ |
| Spectra Project: | 2021110413 |
|  | Rush |



## SPECTRA LABORATORIES


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## Appendix C

Groundwater Monitoring Logs

4809 Pacific Hwy. E. | Fife, Washington 98424 | 253.896 .1011 | www. georesources.rocks

## MONITORING WELL LOGS

Project ID: CES.ProVac.InterAve
***Depths are in reference with ground surface***

| Date: <br> Field Tech: | 12/9/2020 (ATD) AES | Date: <br> Field Tech: | $\begin{aligned} & 12 / 21 / 2020 \\ & \text { AES } \end{aligned}$ |
| :---: | :---: | :---: | :---: |
| Well \# | Depth (ft) | Well \# | Depth (ft) |
| 1 | 2 (perched), 5 | 1 | 0.5 |
| 2 | NE | 2 | 0.5 |
| Date: | 12/31/2020 | Date: | 1/8/2021 |
| Field Tech: | AES | Field Tech: | AES |
| Well \# | Depth (ft) | Well \# | Depth (ft) |
| 1 | 0.5 | 1 | 0.5 |
| 2 | 1.0 | 2 | 0.7 |


| Date: | 1/14/2021 |
| :--- | :--- |
| Field Tech: | AES |
| Well \# | Depth $(\mathrm{ft})$ |
| 1 | 0.5 |
| 2 | 0.5 |


| Date: | $2 / 5 / 2021$ |
| :--- | :--- |
| Field Tech: | AES |
| Well \# | Depth $(\mathrm{ft})$ |
| 1 | 1.8 |
| 2 | 1.2 |


| Date: | $2 / 12 / 2021$ |
| :--- | :--- |
| Field Tech: | AES |
| Well \# | Depth $(\mathrm{ft})$ |
| 1 | 1.7 |
| 2 | 2.3 |

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## MONITORING WELL LOGS

| Project ID: $\quad$ CES.ProVac.InterAve |  |
| :--- | :--- |
|  | $* * *$ Depths are in reference with ground surface*** |


| Date: <br> Field Tech: | $2 / 16 / 2021$ <br> AES | Date: <br> Field Tech: | $3 / 5 / 2021$ <br> AES |
| :---: | :---: | :---: | :---: |
| Well \# | Depth (ft) | Well \# | Depth (ft) |
| 1 | 0.5 | 1 | 1.5 |
| 2 | Unaccessable | 2 | Unaccessable |
| Date: | 3/12/2021 | Date: | 3/19/2021 |
| Field Tech: | CB | Field Tech: | CB |
| Well \# | Depth (ft) | Well \# | Depth (ft) |
| 1 | 2.4 | 1 | 2.4 |
| 2 | Unaccessable | 2 | Unaccessable |
| Date: | 3/26/2021 | Date: | 4/1/2021 |
| Field Tech: | CB | Field Tech: | CB |
| Well \# | Depth (ft) | Well \# | Depth (ft) |
| 1 | 1.7 | 1 | 2.1 |
| 2 | 2.2 | 2 | Unaccessable |

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February 14, 2022
Olson Brothers Pro Vac, LLC
c/o C.E.S. NW, Inc.
$310-29^{\text {th }}$ Street NE, Suite 101
Puyallup, Washington 98372
(253) 848-4282

Attn: Mr. Craig Deaver
cdeaver@cesnwinc.com
Soils Report Addendum: Supplemental Infiltration Testing
Proposed Permeable Pavement
2511 Inter Avenue
Puyallup, Washington
PN: 2105200-180, -192
Doc: CES.ProVac.InterAve.SRa.rev2

## INTRODUCTION

We are pleased to submit this addendum to our previously prepared soils report dated December 10, 2021. On December 21, 2021, we returned to the site to perform one Pilot Infiltration Test (PIT) in the green space on the southern portion of the site, in the front yard of the existing residence. The bottom of the PIT was excavated approximately 1 foot below the existing grades. The soils at the bottom of the PIT were consistent with the native alluvium soils described in our original report of a medium dense brown, orange iron stained silty sand or sandy silt that was in a moist to wet condition.

At the time of our testing, water was being pumped out from beneath the crawl space of the existing residence, and the surface water ponding on the gravel surface and adjacent sod area was flowing towards our PIT. No groundwater was encountered in our PIT, but the surface water was flowing into our PIT. Our excavation slowly started to fill in as the rate of inflow as greater than the infiltration rate of the soils. During the limited time of our testing prior to surface water inflow, the measured rate appeared consistent with the rates provided in our December 2021 report, and those rates are still appropriate. We also monitored groundwater during the winter of 2020/2021. The results of our groundwater monitoring and original infiltration testing are summarized in our December 10, 2021 report.

Based on the Paving \& Utility Plan by C.E.S. NW Inc., dated February 9, 2022, the grades at the site will be raised by 2 feet to meet the vertical separation requirements for permeable pavement. Catch basins and overflows will also be implemented. It is our opinion that vertical separation requirements can be met at the site once the site grades have been raised, and permeable pavement would therefore be feasible for this project.
page | 2

## CLOSURE

We trust that this is sufficient for your needs. If you have any questions regarding the content of this letter, please call.

Yours very truly
GeoResources, LLC


Andrew Schnitger, EIT
Engineer in Training


Keith Schembs, LEG
Principal


Eric W. Heller, PE, LG
Senior Geotechnical Engineer

AES:KSS:EWH/aes
DocID: CES.ProVac.InterAve.SRa.rev2

