

July 16, 2018 ES-4960.02

### Earth Solutions NW LLC

Geotechnical Engineering, Construction
Observation/Testing and Environmental Services

Step by Step Family Support Center c/o Jeff Brown Architecture 12181 C Street South Tacoma, Washington 98444

Attention:

Mr. Jeff Brown

Subject:

**Geotechnical Consulting Services** 

Winter High Groundwater Evaluation and Infiltration Testing

Germaine Korum Center 13407 – 80<sup>th</sup> Street East Puyallup, Washington

Reference:

Earth Solutions NW, LLC

Geotechnical Engineering Study

Project No. ES-4960, dated April 12, 2017

Barghausen Consulting Engineers, Inc. Grading and Storm Drainage Plan

Job No. 17376, dated September 12, 2017

Washington State Department of Ecology

Stormwater Management Manual for Western Washington, December 2014

#### Dear Mr. Brown:

In accordance with your request, Earth Solutions NW, LLC (ESNW) has prepared this letter with results of our recent infiltration testing and winter groundwater monitoring. With respect to the proposed bioretention cell, an allowable infiltration rate of 0.25 inches per hour (iph) was specified for design. The purpose of our recent investigation was to perform in-situ infiltration testing within the infiltration pond area and provide groundwater monitoring throughout the winter season.

#### **Project Description**

ESNW understands the proposed development will be comprised of several one- or two-story structures, two greenhouses, parking areas and drive lanes, a bioretention cell, and related infrastructure improvements. Many of the existing structures will be retained. The determination of the capacity for native soils to accommodate infiltration and seasonal high groundwater monitoring were the primary foci of this evaluation.

#### **Groundwater Monitoring**

Our groundwater monitoring program consisted of installing two piezometers at the approximate locations depicted on the attached test pit location plan (TP-101 and TP-102). The installations occurred in February 2018 and readings have been collected on a bi-weekly basis. The following table summarizes the data collected during the groundwater monitoring period.

	TP-101	TP-102
Date	Groundwa	iter Depth (ft)*
2/21	2.0	2.5
	2.3	2.7
3/6 3/20	2.4	2.8
4/4	2.7	3.1
4/16	1.0	2.7
5/15	5.0	4.8

<sup>\*</sup>Depth measured from existing ground surface

As anticipated, high groundwater readings corresponded with relatively high rainfall events. The readings completed on April 16, 2018 are indicative of seasonal high groundwater elevations.

### Infiltration Evaluation

Our infiltration evaluation was completed in general accordance with the 2014 Surface Water Management Manual for Western Washington (2014 SWMMWW), as adopted by the city of Puyallup.

Native alluvium was encountered in a loose to medium dense condition within the proposed bioretention cell. To determine a long-term design rate for the proposed bioretention cell, we completed small-scale Pilot Infiltration Tests (PITs). In-situ testing was performed at a depth of approximately one to one-and-one-half feet below the ground surface at TP-101 and TP-102, which correlates to the approximate proposed infiltrating surface depth. Following the soak period, the falling head observation was performed and yielded a measured field rate (Ksat initial) of 0.75 iph at TP-101 and 1.0 iph at TP-102. The Ksat obtained from the PITs must be reduced through the application of correction factors to account for site variability, test methods, and degree of influent control to prevent sedimentation. The following factors were used in design and analysis:

Site variability (CF<sub>v</sub>)

0.8

Test Method (CFt)

0.5 (Small-scale PIT)

Degree of influent control to prevent siltation (CF<sub>m</sub>)

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Multiplying our K<sub>sat</sub> initial with the calculated CF<sub>t</sub>, a design infiltration rate of 0.3 iph was determined. Based on the conditions observed during our fieldwork, a design infiltration rate of 0.3 iph is considered applicable to the proposed infiltration location with respect to bioretention applications. We recommend incorporating an overflow provision to the extent practicable.

#### **Discussion**

Our infiltration testing was performed to confirm that native soil present at the proposed bottom of the bioretention cell exhibited an infiltration capacity to support the design infiltration rate of 0.25 iph. Native soil at the proposed bottom of the bioretention cell was recognized as a fine-grained alluvial deposit. It is our professional opinion the infiltration capacity of the native soil within the stormwater management pond is adequate to support the design rate of 0.25 iph, provided adequate separation between the facility base and the seasonal high groundwater table is incorporated into final designs.

If the design assumptions outlined in this letter are incorrect or change, or if construction conditions differ from those encountered during our fieldwork, ESNW should be contacted to review the recommendations and conclusions provided in this letter. This letter has been prepared for the exclusive use of the Step by Step Family Support Center and their representatives. A warranty is neither expressed nor implied. The recommendations and conclusions provided in this letter are professional opinions consistent with the level of care and skill that is typical of other members in the profession currently practicing under similar conditions in this area. Variations in the soil and groundwater conditions encountered at the test pit locations may exist and may not become evident until construction.

Step by Step Family Support Center c/o Jeff Brown Architecture July 16, 2018

We appreciate the opportunity to be of service to you and trust this letter meets your current needs. Should you have questions, or require additional information, please call.

Sincerely,

### **EARTH SOLUTIONS NW, LLC**



Adam Z. Shier, G.I.T. Staff Geologist



Keven D. Hoffmann, P.E. Senior Project Engineer

Attachments: Plate 1 - Vicinity Map

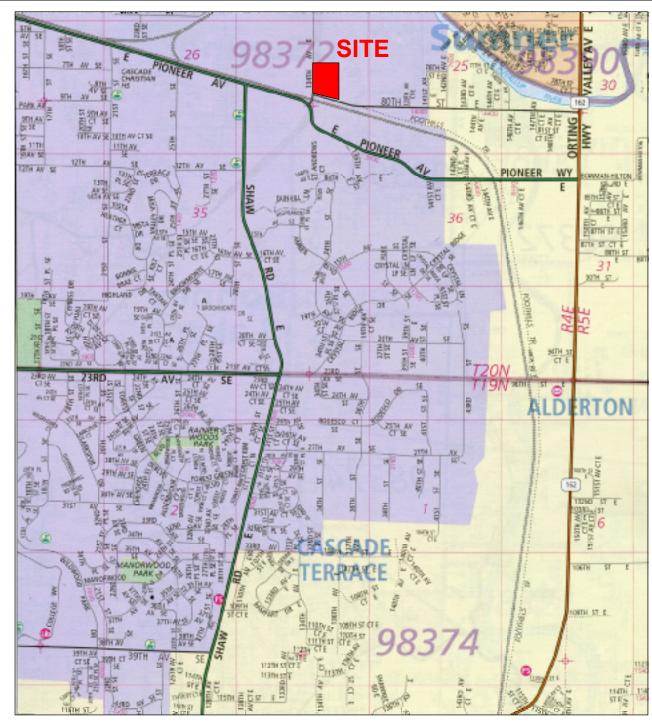
Plate 2 – Test Pit Location Plan

Test Pit Logs Laboratory Data

CC:

Barghausen Consulting Engineers, Inc.

Attention: Mr. Dan Balmelli, P.E. (Email only)



Reference: Pierce County, Washington Map 835 By The Thomas Guide Rand McNally 32nd Edition

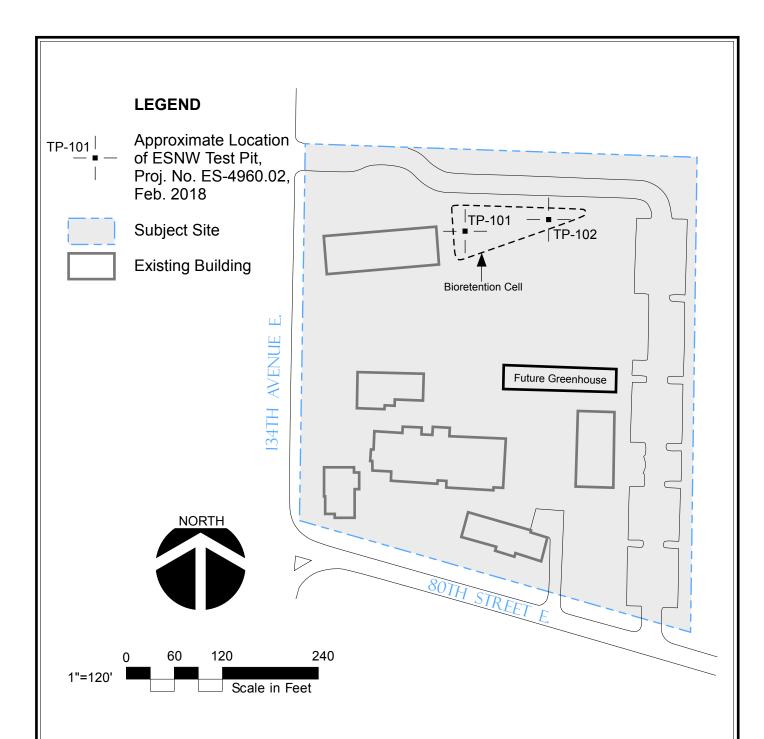


NOTE: This plate may contain areas of color. ESNW cannot be responsible for any subsequent misinterpretation of the information resulting from black & white reproductions of this plate.



Vicinity Map Germaine Korum Center Puyallup, Washington

Drwn. CAM	Date 06/07/2018	Proj. No.	4960.02
Checked AZS	Date June 2018	Plate	1



NOTE: The graphics shown on this plate are not intended for design purposes or precise scale measurements, but only to illustrate the approximate test locations relative to the approximate locations of existing and / or proposed site features. The information illustrated is largely based on data provided by the client at the time of our study. ESNW cannot be responsible for subsequent design changes or interpretation of the data by others.

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Test Pit Location Plan Germaine Korum Center Puyallup, Washington

Drwn. CAM	Date 06/07/2018	Proj. No.	4960.02
Checked AZS	Date June 2018	Plate	2

# Earth Solutions NWLLC SOIL CLASSIFICATION CHART

Í	A 105 DN 401	0110	SYM	BOLS	TYPICAL
M	AJOR DIVISI	ONS	GRAPH	LETTER	DESCRIPTIONS
	GRAVEL AND	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
	GRAVELLY SOILS	(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
COARSE GRAINED SOILS	MORE THAN 50% OF COARSE FRACTION	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
	RETAINED ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
MORE THAN 50% OF MATERIAL IS	SAND AND	CLEAN SANDS		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
LARGER THAN NO. 200 SIEVE SIZE	SANDY SOILS	(LITTLE OR NO FINES)	$\times$	SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION	SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES
	PASSING ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		SC	CLAYEY SANDS, SAND - CLAY MIXTURES
				ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
GOILE				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE				МН	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
SIZE	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		СН	INORGANIC CLAYS OF HIGH PLASTICITY
				ОН	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
н	GHLY ORGANIC	SOILS	70 70 40 40 6 40 40 40 40 70 30 40 40	PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

DUAL SYMBOLS are used to indicate borderline soil classifications.

The discussion in the text of this report is necessary for a proper understanding of the nature of the material presented in the attached logs.



Earth Solutions NW 1805 - 136th Place N.E., Suite 201 Bellevue, Washington 98005 Telephone: 425-449-4704 Fax: 425-449-4711

# TEST PIT NUMBER TP-101 PAGE 1 OF 1

	ROJ	ECT NUN	MBER ES-4960.02				PROJECT NAME Germaine Korum Center	
[	DATE	STARTE	D 2/8/18	CO	MPLE.	TED 2/	/8/18 GROUND ELEVATION TEST PIT SIZE	
							GROUND WATER LEVELS:	
- 1			METHOD					
			AZS					
- 1			ce Conditions: bare so					
F	1				_		A LECTION TO THE PROPERTY OF T	
	0 UEP IH	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	
	-		MC = 29.40% Fines = 66.60%				Brown sandy SILT, loose to medium dense, moist to wet  [USDA Classification: slightly gravelly LOAM]  -iron oxide staining	
=	5		MC = 24.70%	ML				
	-		MC = 38.00% Fines = 64.20%				-becomes gray -light groundwater seepage at 6' [USDA Classification: LOAM]	
	10		MC = 41.90%			10.0	Test pit terminated at 10.0 feet below existing grade. Groundwater seepage encountered at 6.0 feet during excavation. No caving observed.  Bottom of test pit at 10.0 feet.	
GENERAL BH / TP / WELL 4960-2.GPJ GINT US.GDT 6/7/18							Bottom of test pit at 10.0 leet.	



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## TEST PIT NUMBER TP-102 PAGE 1 OF 1

PROJ	IECT NUM	MBER ES-4960.02					PROJECT NAME Germaine Korum Center		
							GROUND ELEVATION TEST PIT SIZE		
							GROUND WATER LEVELS:		
		METHOD							
	NOTES Surface Conditions: bare soil						AT END OF EXCAVATION		
NOTE	1	Ce Conditions, bare so	)II	T	F		AFTER EXCAVATION		
O DEPTH	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG			MATERIAL DESCRIPTION		
		MC = 28.40% Fines = 73.10%				[USDA Classific	a sand, loose to medium dense, moist to wet ation: slightly gravelly LOAM]		
5		MC = 29.40%	ML			-becomes gray	ay .		
		MC = 31.30%			ey	-caving from to	9,		
10		MC = 36.50%			10.0	[USDA Classific			
GENERAL BH 7 TP / WELL 4980-2,GPJ GINT US,GDT 67/18		Fines = 91.60%				Test pit termina feet during exca	ted at 10.0 feet below existing grade. Groundwater seepage encountered at 9.0 vation. Caving observed from 6.0 to 9.0 feet.  Bottom of test pit at 10.0 feet.		

Specimen Identification

1.0ft.

6.0ft.

1.0ft.

10.0ft.

**TP-101** 

TP-101

TP-102

TP-102

GRAIN SIZE USDA

M

D100

4.75

2

4.75

2

D60

D30

D10

LL

PL

PI

%Silt

%Clay

66.6

64.2

73.1

91.6

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#### **GRAIN SIZE DISTRIBUTION**

