

June 28, 2022 ES-8413

Earth Solutions NW LLC

Geotechnical Engineering, Construction Observation/Testing and Environmental Services

American Pride Lending, LLC P.O. Box 1226 Kent, Washington 98035

Attention: Mr. Sikander Sekhon

- Subject: Infiltration Evaluation Proposed Site Improvements 212 Todd Road Northeast Puyallup, Washington
- Reference: Department of Ecology, State of Washington 2014 Stormwater Management Manual for Western Washington (SMMWW)

Schuster, et al. Geologic Map of Tacoma, November 2015

Dear Mr. Sekhon:

As requested, Earth Solutions NW, LLC (ESNW) has prepared this infiltration evaluation for the subject site.

Project Description

We understand low impact development flow control best management practices (BMPs) are being evaluated to control stormwater. Infiltration of stormwater is being evaluated as part of the overall stormwater design.

If the above design assumptions are incorrect or change, ESNW should be contacted to review the recommendations provided in this report. ESNW should review final designs to confirm that our geotechnical recommendations have been incorporated into project plans.

Surface

The subject site is located at 212 Todd Road Northeast in Puyallup, Washington, as illustrated on the attached Vicinity Map (Plate 1). The site consists of one tax parcel (Pierce County parcel number 2354300575). The site is currently developed a single-family residence in the northwest corner of the property; the remainder of the property is covered with gravel. The site topography is relatively level.

<u>Subsurface</u>

An ESNW representative observed, logged, and sampled five test pits, excavated at accessible locations within the property boundaries, on March 7, 2022 using a mini-trackhoe and operator provided by our firm. Shallow groundwater monitoring wells were installed within test pits TP-3, TP-4, and TP-5. The approximate locations of the test pits are depicted on the attached Plate 2 (Test Pit Location Plan). Please refer to the test pit logs provided as attachments for a more detailed description of subsurface conditions. Representative soil samples collected at the test pit locations were evaluated in general accordance with Unified Soil Classification System (USCS) and USDA methods and procedures.

Fill

Existing gravel fill was encountered at all test pit locations extending to about one foot below the existing ground surface (bgs). The gravel fill was associated with the gravel-surfacing material observed throughout the majority of the site.

Native Soil

Underlying surficial existing fill, native soil was encountered primarily as loose to medium dense silty sand and sandy silt (USCS: SM and ML, respectively). Caving within the test pits was observed, beginning at depths of about three and one-half to seven and one-half feet bgs. The native soil was generally observed to be in a wet condition.

Geologic Setting

The referenced geologic map identifies alluvium deposits throughout the site and surrounding area. According to the geologic map resource, alluvium deposits are loose, stratified to massively bedded fluvial silt, sand, and gravel. Based on our field observations, native soil likely to be exposed on site will be consistent with alluvium deposits.

Groundwater

The local groundwater table was observed beginning at depths of about five to five and one-half feet bgs during the fieldwork on March 7, 2022. It is likely that the local groundwater table rises a foot or two throughout the peak of the wet season; ESNW can complete seasonal groundwater level monitoring upon request. Groundwater flow rates and elevations fluctuate depending on many factors, including precipitation duration and intensity, the time of year, and soil conditions. In general, groundwater flow rates are higher during the winter, spring, and early summer months.

Infiltration Evaluation

Our evaluation of site infiltration capacity was completed by excavating a series of test pits throughout the site, and completing two small-scale pilot infiltration tests (PITs). As indicated in the *Subsurface* section of this report, native soils encountered during our fieldwork were characterized primarily as silty sand and sandy silt, with the groundwater table encountered beginning at about five to five and one-half feet bgs.

PITs were performed within TP-1 and TP-2 at a depth of roughly four feet bgs; the measured infiltration rates were 4.2 and 1.4 inches per hour, respectively. For preliminary design purposes, we recommend assuming a measured infiltration rate of 1.4 inches per hour. The measured rate must be reduced by the following correction factors:

•	Measured infiltration rate	1.4 inches per hour		
•	Site variability (CF _v)	0.5		
•	Test method (CFt)	0.5		
•	Degree of influent control (CF _m)	0.9		

The correction factors, along with the measured infiltration rate, were applied to determine the design infiltration rate. Based on our in-situ test results, it is our opinion the following infiltration rate can be used for preliminary design purposes if pursued:

Design infiltration rate
0.3 inches per hour

If infiltration is pursued, the facilities will need to maintain proper separation from the local groundwater table. Depending on total impervious area proposed to be directed to infiltration facilities, additional PITs may be necessary.

ESNW can provide further evaluation and recommendations for site BMPs as plans develop. ESNW should review final stormwater management plans to provide supplementary recommendations, as needed.

Limitations

The recommendations and conclusions provided in this letter are professional opinions consistent with the level of care and skill that is typical of other members in the profession currently practicing under similar conditions in this area. A warranty is not expressed or implied. Variations in the soil and groundwater conditions observed at the test sites may exist and may not become evident until construction. ESNW should reevaluate the conclusions in this letter if variations are encountered.

Additional Services

ESNW can complete additional PITs and seasonal groundwater level monitoring upon request. ESNW should have an opportunity to review the final design with respect to the geotechnical recommendations provided in this letter. ESNW should also be retained to provide testing and consultation services during the earthwork phase of construction.

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We trust this letter meets your current needs. Should you have questions regarding the content herein, or require additional information, please call.

Sincerely,

EARTH SOLUTIONS NW, LLC



Henry T. Wright, P.E. Associate Principal Engineer

Attachments: Plate 1 – Vicinity Map Plate 2 – Test Pit Location Plan Test Pit Logs Grain Size Distribution





Earth Solutions NWLLC SOIL CLASSIFICATION CHART

м		ONS	SYME	BOLS	TYPICAL
141			GRAPH	LETTER	DESCRIPTIONS
	GRAVEL AND	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
	GRAVELLY SOILS	(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
COARSE GRAINED SOILS	MORE THAN 50% OF COARSE	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
	RETAINED ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
MORE THAN 50% OF MATERIAL IS	SAND AND	CLEAN SANDS		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
LARGER THAN NO. 200 SIEVE SIZE	SANDY SOILS	(LITTLE OR NO FINES)		SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE	SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES
	PASSING ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		SC	CLAYEY SANDS, SAND - CLAY MIXTURES
		LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
FINE GRAINED SOILS	SILTS AND CLAYS			CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE		LIQUID LIMIT GREATER THAN 50		МН	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
SIZE	SILTS AND CLAYS			СН	INORGANIC CLAYS OF HIGH PLASTICITY
				ОН	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HI	HIGHLY ORGANIC SOILS			PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

DUAL SYMBOLS are used to indicate borderline soil classifications.

The discussion in the text of this report is necessary for a proper understanding of the nature of the material presented in the attached logs.





GENERAL BH / TP / WELL - 8413.GPJ - GRAPHICS TEMPLATE WITH LAT AND LONG.GDT - 5/24/22

	t Ear Soluti NW	Earth Solu 15365 N.E ONS Redmond, Telephone Fax: 425-4	tions NW, L . 90th Stree Washingtor : 425-449-4 149-4711	LC t, Suite 100 n 98052 i704	TEST PIT NUMBER TP-3 PAGE 1 OF 1
PROJI		BER _ ES-8413			PROJECT NAME _ 212 Todd Road N.E.
DATE	STARTE	D <u>3/7/22</u>		PLETED _ 3/7/22	GROUND ELEVATION
	VATION (CONTRACTOR <u>N</u>	W Excavatir	ng	LATITUDE LONGITUDE
LOGG	ED BY	SES	CHEC	KED BY HTW	∇ AT TIME OF EXCAVATION 5.5 ft
	S Surfac	ce Conditions: grav	/el pad		-
O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S. GRAPHIC LOG		MATERIAL DESCRIPTION
			GP	Gray poorly graded	d GRAVEL, loose, damp (Fill)
 2.5 5.0		MC = 18.9%	SM	5.0	AND, loose to medium dense, wet
 - 7.5 		MC = 37.9%	ML	Dark brown sandy	SILT, loose to medium dense, wet , moderate caving to BOH
		MC = 34.6%	<u> </u>	9.5 Test pit terminated	at 9.5 feet below existing grade. Groundwater table encountered at 5.5 feet
				during excavation.	Caving observed from 5.5 feet to BOH.

GENERAL BH / TP / WELL - 8413.GPJ - GRAPHICS TEMPLATE WITH LAT AND LONG.GDT - 5/24/22

		Ear Soluti NW	th 15365 N.E ions Redmond, Telephone Fax: 425-	itions I 2. 90th Wash 2: 425 449-47	NW, L Stree hingtoi -449-4 711	LC tt, Suite 100 n 98052 4704	TEST PIT NUMBER TP-4 PAGE 1 OF 1
	PROJECT NUMBER ES-8413						PROJECT NAME _212 Todd Road N.E.
	DATE STARTED _3/7/22 COMPLETED _3/7/22				COMF	PLETED _3/7/22	GROUND ELEVATION
	EXCAVATION CONTRACTOR NW Excavating			cavati	ng	LATITUDE LONGITUDE	
	EXCA	VATION					GROUND WATER LEVEL:
	LOGG	ED BY	SES			KED BY HTW	AT TIME OF EXCAVATION <u>5.0 ft</u>
╞		S Surfa	ce Conditions: gra	vei pao		1	
	o DEPTH (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION
ſ	0.0					Gray poorly g	raded GRAVEL, loose, damp (Fill)
ľ	-			GP		1.0	
F	_					Brown silty S	AND, loose to medium dense, wet
F	_		MC = 24.6%			아 레	
F	25						
F	2.0			SM			
F	-						
F	-						
F	_					Dark brown s	andy SILT, loose to medium dense, wet
ŀ	-					mottled textu	Ire
╞	5.0		MC = 41.1%				table
╞	-						
╞	-						
21	-					-slight caving	to BOH
5/24/2	-			ML			
Ľ.	7.5						
ONG.	-						
AD L	-						
- LAT	-						
Ĕ,	-						
"LATE	10.0		MC = 33.6%	<u> </u>		10.0 Test pit termi	nated at 10.0 feet below existing grade. Groundwater table encountered at 5.0 feet
TEMI						during excave	ation. Caving observed from 6.5 feet to BOH.
PHICS							
GRA							
.GPJ -							
- 8413							
VELL -							
TP / V							
- BH /							
VERAL							
ΰ							

	Eart Soluti NW	Earth Solut 15365 N.E ONS Redmond, Telephone Fax: 425-4	tions NW, LL . 90th Street Washington : 425-449-47 149-4711	-C , Suite 100 98052 704	TEST PIT NUMBER TP-5 PAGE 1 OF 1		
PROJ		BER _ ES-8413			PROJECT NAME _ 212 Todd Road N.E.		
DATE	STARTE	D <u>3/7/22</u>		LETED <u>3/7/22</u>	GROUND ELEVATION		
EXCA	VATION		W Excavatin	g			
					GROUND WATER LEVEL: ∇ AT TIME OF EXCAVATION 5.0 ft		
NOTE	S Surfac	ce Conditions: grav	CHECr				
O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S. GRAPHIC LOG		MATERIAL DESCRIPTION		
			GP	Gray poorly graded	I GRAVEL, loose, damp (Fill)		
				1.0			
				Brown silty SAND,	loose to medium dense, wet		
2.5		MC = 23.8%					
			SM	-slight caving to BC	ЭН		
 <u>5.0</u> 				groundwater table 5.5 Dark brown sandy			
 		MC = 37.2%		,			
			ML				
10.0		MC = 24.00		10.0			
		MC = 34.2%		Test pit terminated during excavation.	at 10.0 feet below existing grade. Groundwater table encountered at 5.0 feet Caving observed from 3.0 feet to BOH.		

GENERAL BH / TP / WELL - 8413.GPJ - GRAPHICS TEMPLATE WITH LAT AND LONG.GDT - 5/24/22



Earth Solutions NW, LLC 15365 N.E. 90th Street, Suite 100 Redmond, Washington 98052 Telephone: 425-449-4704 Fax: 425-449-4711

GRAIN SIZE DISTRIBUTION

