



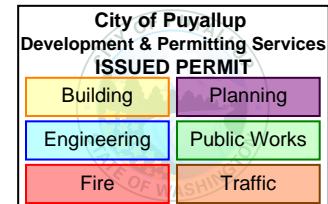
PRPF20241230

October 11, 2024  
ES-4556.03

Earth Solutions NW LLC

Geotechnical Engineering, Construction  
Observation/Testing and Environmental Services

Cascade Christian Schools  
815 – 21<sup>st</sup> Street Southeast  
Puyallup, Washington 98372



Attention: Ray Ossman

**Subject: Bearing Capacity Evaluation  
Cascade Christian School  
819 – 21<sup>st</sup> Street Southeast  
Puyallup, Washington**

Greetings:

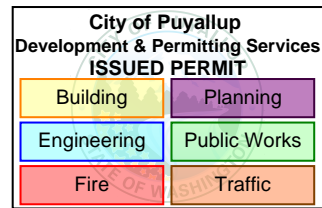
As requested, Earth Solutions NW, LLC (ESNW) has prepared this letter providing foundation design recommendations for the project.

**Project & Site Description**

The subject site is located directly east of the intersection between 21<sup>st</sup> Street Southeast and 9<sup>th</sup> Avenue Southeast in Puyallup, Washington. The approximate site location is illustrated on the attached Vicinity Map (Plate 1). The site consists of a single tax parcel (Pierce County Parcel Number 042026-3083) and totals about 0.37 acres. We understand that the site is currently under construction and will ultimately be developed with six portable classroom structures and associated infrastructure improvements. We anticipate the portables will be manufactured or constructed with light loaded wood framing on a conventional foundation system. Perimeter footing loads are anticipated at 1 to 2 kips and slab on grade loading is anticipated at 150 psf.

**Subsurface Conditions**

An ESNW representative observed, logged, and sampled two test pits on June 17, 2024. The test pits were excavated at accessible areas of the site. Based on our observations, native soils generally consisted of silty sand and localized exposures of sandy silt (USCS: SM and ML, respectively). In-situ soil density was generally characterized as loose to medium dense and in-situ soil moisture contents were characterized as moist to wet at the time of exploration. The maximum exploration depth was approximately nine and one-half feet below the ground surface (bgs). The local groundwater table was observed at test location TP-2 at a depth of approximately nine and one-half feet bgs during the June 2024 fieldwork. The locations of the test pits can be seen on the attached Test Pit Location Plan (Plate 2). The corresponding test pit logs are also attached to this letter.



### **Construction Observations**

ESNW performed three site visits during grading operations in the area to perform density testing and document general construction observations and grading activities. A copy of each field report is provided as an attachment to this letter. At the time of the visits, the contractor was observed placing and compacting an imported gravel as structural fill in the area. To our understanding, approximately two to three feet of material was placed and compacted. Representative density tests indicate that a minimum compaction of at least 95 percent was achieved at the tested locations. Furthermore, when evaluating the placed fill with a one-half inch diameter steel T-Probe, dense conditions were generally observed.

### **Foundations**

In our opinion, the proposed portable classrooms can be constructed on conventional continuous and spread footing foundations that bear upon placed and compacted structural fill soils. Based on our observations, native soil conditions considered suitable for support of the proposed structures will likely be encountered at typical foundation depths. In general, where loose or unsuitable soil conditions are exposed at foundation subgrade elevations, compaction of the soil to the specifications of structural fill or overexcavation and replacement with suitable structural fill will be necessary.

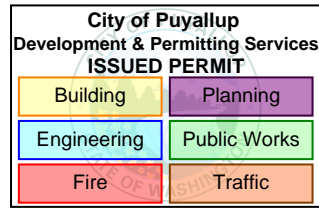
Provided the foundations will be supported as prescribed, the following parameters may be used for the design:

- Allowable soil bearing capacity 2,000 psf
- Passive earth pressure 300 pcf (equivalent fluid)
- Coefficient of friction 0.40

The above passive pressure and friction values include a factor-of-safety (FOS) of 1.5. A one-third increase in the allowable soil bearing capacity may be assumed for short-term wind and seismic loading conditions. With structural loading as expected, total settlement in the range of one to two inches and differential settlement of about one-half to one inch is anticipated. Most settlements should occur during construction when dead loads are applied. ESNW should be contacted to observe and evaluate foundation subgrade areas prior to placing formwork.

### **Limitations**

This report has been prepared for the exclusive use of Cascade Christian Schools and its representatives. The recommendations and conclusions provided in this letter are professional opinions consistent with the level of care and skill that is typical of other members in the profession currently practicing under similar conditions in this area. A warranty is not expressed or implied. Variations in the soil and groundwater conditions observed at the test pit locations may exist and may not become evident until construction. ESNW should reevaluate the conclusions in this report if variations are encountered.



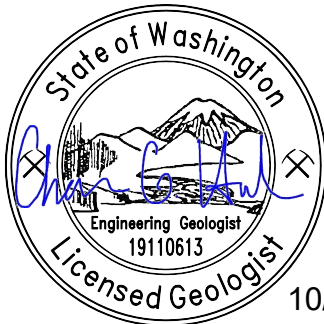
**Additional Services**

ESNW should be retained to provide additional geotechnical services in. sociation with this project, including testing and consulting services during construction. ESNW should have an opportunity to review final project plans with respect to the geotechnical recommendations provided in this letter.

We appreciate the opportunity to be of service to you and trust this letter meets your current needs. Should you have any questions, or require additional information, please call.

Sincerely,

**EARTH SOLUTIONS NW, LLC**



10/11/2024

Chase G. Halsen

Chase G. Halsen, L.G., L.E.G.  
Project Manager



10/11/2024

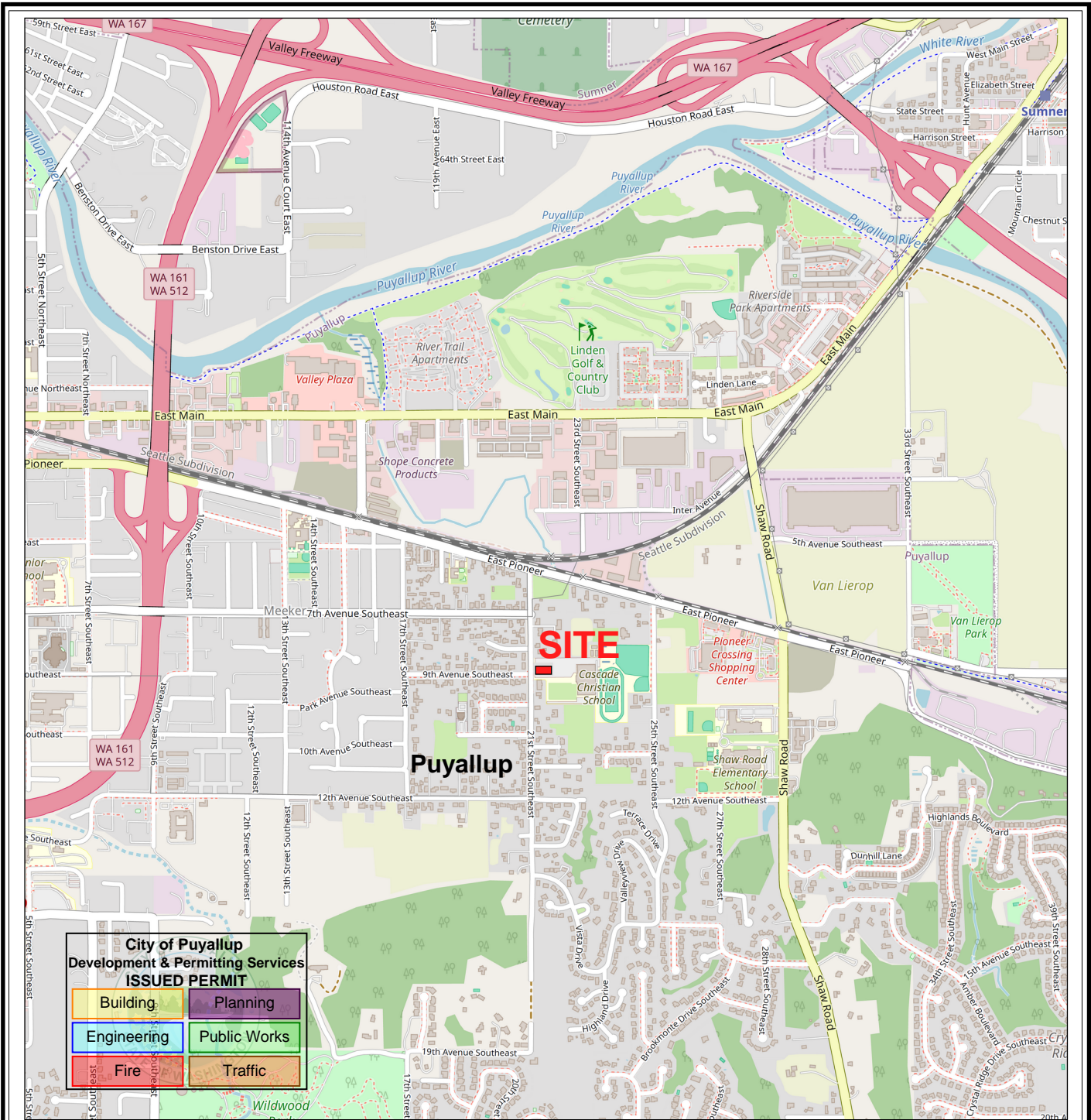
Kyle R. Campbell, P.E.  
Senior Principal Engineer

Attachments: Plate 1 – Vicinity Map  
Plate 2 – Test Pit Location Plan  
Test Pit Logs  
Grain Size Distribution  
Field Reports

cc: JBA, LLC  
Attention: Songyi Cho

References:

- Infiltration Evaluation, prepared by ESNW, ES-4556.02, dated July 31, 2024
- Phase 5 – Site Plan, produced by Vader Engineering, dated May 27, 2024



- City of Puyallup  
Development & Permitting Services  
ISSUED PERMIT**
- Building
  - Planning
  - Engineering
  - Public Works
  - Fire
  - Traffic



Reference:  
Pierce County, Washington  
OpenStreetMap.org

**Earth Solutions NW LLC**

Geotechnical Engineering, Construction  
Observation/Testing and Environmental Services

**Vicinity Map  
Cascade Christian School  
Puyallup, Washington**




Drawn CAM	Date 10/11/2024	Proj. No. 4556.03	
Checked SES	Date Oct. 2024	Plate	1

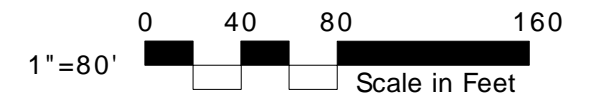
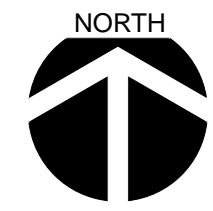
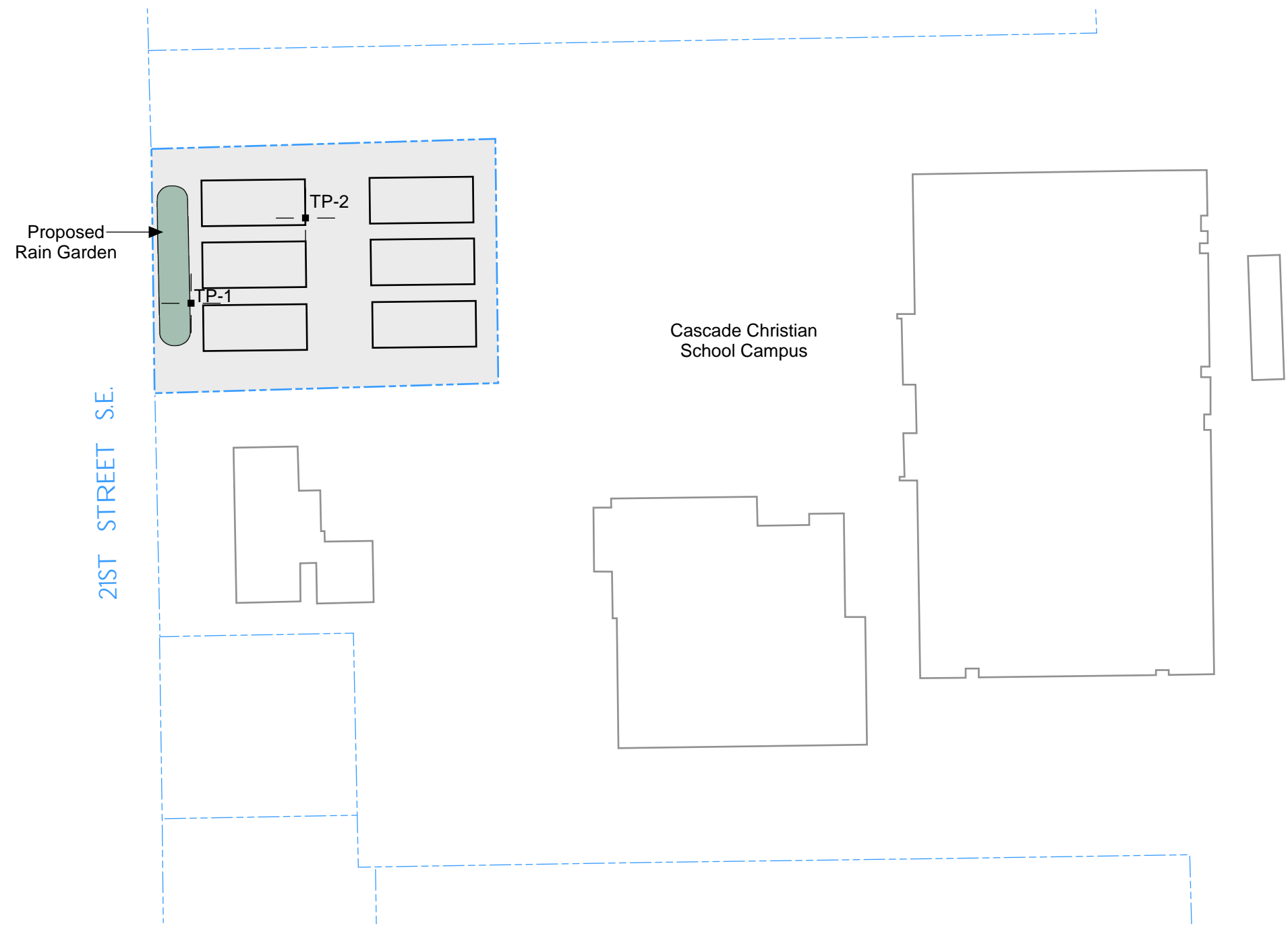
NOTE: This plate may contain areas of color. ESNW cannot be responsible for any subsequent misinterpretation of the information resulting from black & white reproductions of this plate.

**City of Puyallup**  
Development & Permitting Services  
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Building	Planning
Engineering	Public Works
Fire	Traffic

**LEGEND**

- TP-1 | — ■ — | Approximate Location of ESNW Test Pit, Proj. No. ES-4556.02, June 2024
-  Subject Area
-  Proposed Portable Structures
-  Existing Building



NOTE: The graphics shown on this plate are not intended for design purposes or precise scale measurements, but only to illustrate the approximate test locations relative to the approximate locations of existing and / or proposed site features. The information illustrated is largely based on data provided by the client at the time of our study. ESNW cannot be responsible for subsequent design changes or interpretation of the data by others.

NOTE: This plate may contain areas of color. ESNW cannot be responsible for any subsequent misinterpretation of the information resulting from black & white reproductions of this plate.

Test Pit Location Plan  
Cascade Christian School  
Puyallup, Washington

Earth Solutions NW LLC  
Geotechnical Engineering, Construction  
Observation/Testing and Environmental Services



Drawn CAM
Checked SES
Date 10/11/2024
Proj. No. 4556.03
Plate 2

Coarse-Grained Soils - More Than 50% Retained on No. 200 Sieve		Moisture Content		Symbols																							
Gravels - More Than 50% of Coarse Fraction Retained on No. 4 Sieve		<b>GW</b>	Well-graded gravel with or without sand, little to no fines	Dry - Absence of moisture, dusty, dry to the touch																							
		<b>GP</b>	Poorly graded gravel with or without sand, little to no fines	Damp - Perceptible moisture, likely below optimum MC																							
		<b>GM</b>	Silty gravel with or without sand	Moist - Damp but no visible water, likely at/near optimum MC																							
		<b>GC</b>	Clayey gravel with or without sand	Wet - Water visible but not free draining, likely above optimum MC																							
Sands - 50% or More of Coarse Fraction Passes No. 4 Sieve		<b>SW</b>	Well-graded sand with or without gravel, little to no fines	Saturated/Water Bearing - Visible free water, typically below groundwater table																							
		<b>SP</b>	Poorly graded sand with or without gravel, little to no fines																								
		<b>SM</b>	Silty sand with or without gravel																								
		<b>SC</b>	Clayey sand with or without gravel																								
Fine-Grained Soils - 50% or More Passes No. 200 Sieve		Terms Describing Relative Density and Consistency																									
Silt and Clays Liquid Limit Less Than 50		<b>ML</b>	Silt with or without sand or gravel; sandy or gravelly silt	<b>Coarse-Grained Soils:</b> <u>Density</u> <u>SPT blows/foot</u> Very Loose                      < 4 Loose                              4 to 9 Medium Dense                    10 to 29 Dense                                30 to 49 Very Dense                        ≥ 50																							
		<b>CL</b>	Clay of low to medium plasticity; lean clay with or without sand or gravel; sandy or gravelly lean clay	<b>Fine-Grained Soils:</b> <u>Consistency</u> <u>SPT blows/foot</u> Very Soft                         < 2 Soft                                  2 to 3 Medium Stiff                    4 to 7 Stiff                                 8 to 14 Very Stiff                        15 to 29 Hard                                ≥ 30																							
		<b>OL</b>	Organic clay or silt of low plasticity	<b>Test Symbols &amp; Units</b> Fines = Fines Content (%) MC = Moisture Content (%) DD = Dry Density (pcf) Str = Shear Strength (tsf) PID = Photoionization Detector (ppm) OC = Organic Content (%) CEC = Cation Exchange Capacity (meq/100 g) LL = Liquid Limit (%) PL = Plastic Limit (%) PI = Plasticity Index (%)																							
		<b>MH</b>	Elastic silt with or without sand or gravel; sandy or gravelly elastic silt																								
Silt and Clays Liquid Limit 50 or More		<b>CH</b>	Clay of high plasticity; fat clay with or without sand or gravel; sandy or gravelly fat clay	<b>Component Definitions</b> <table border="1"> <thead> <tr> <th>Descriptive Term</th> <th>Size Range and Sieve Number</th> </tr> </thead> <tbody> <tr> <td>Boulders</td> <td>Larger than 12"</td> </tr> <tr> <td>Cobbles</td> <td>3" to 12"</td> </tr> <tr> <td>Gravel</td> <td>3" to No. 4 (4.75 mm)</td> </tr> <tr> <td>Coarse Gravel</td> <td>3" to 3/4"</td> </tr> <tr> <td>Fine Gravel</td> <td>3/4" to No. 4 (4.75 mm)</td> </tr> <tr> <td>Sand</td> <td>No. 4 (4.75 mm) to No. 200 (0.075 mm)</td> </tr> <tr> <td>Coarse Sand</td> <td>No. 4 (4.75 mm) to No. 10 (2.00 mm)</td> </tr> <tr> <td>Medium Sand</td> <td>No. 10 (2.00 mm) to No. 40 (0.425 mm)</td> </tr> <tr> <td>Fine Sand</td> <td>No. 40 (0.425 mm) to No. 200 (0.075 mm)</td> </tr> <tr> <td>Silt and Clay</td> <td>Smaller than No. 200 (0.075 mm)</td> </tr> </tbody> </table>		Descriptive Term	Size Range and Sieve Number	Boulders	Larger than 12"	Cobbles	3" to 12"	Gravel	3" to No. 4 (4.75 mm)	Coarse Gravel	3" to 3/4"	Fine Gravel	3/4" to No. 4 (4.75 mm)	Sand	No. 4 (4.75 mm) to No. 200 (0.075 mm)	Coarse Sand	No. 4 (4.75 mm) to No. 10 (2.00 mm)	Medium Sand	No. 10 (2.00 mm) to No. 40 (0.425 mm)	Fine Sand	No. 40 (0.425 mm) to No. 200 (0.075 mm)	Silt and Clay	Smaller than No. 200 (0.075 mm)
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Silt and Clay	Smaller than No. 200 (0.075 mm)																										
<b>OH</b>	Organic clay or silt of medium to high plasticity																										
Highly Organic Soils		<b>PT</b>	Peat, muck, and other highly organic soils	<b>Modifier Definitions</b> <table border="1"> <thead> <tr> <th>Percentage by Weight (Approx.)</th> <th>Modifier</th> </tr> </thead> <tbody> <tr> <td>&lt; 5</td> <td>Trace (sand, silt, clay, gravel)</td> </tr> <tr> <td>5 to 14</td> <td>Slightly (sandy, silty, clayey, gravelly)</td> </tr> <tr> <td>15 to 29</td> <td>Sandy, silty, clayey, gravelly</td> </tr> <tr> <td>&gt; 30</td> <td>Very (sandy, silty, clayey, gravelly)</td> </tr> </tbody> </table>		Percentage by Weight (Approx.)	Modifier	< 5	Trace (sand, silt, clay, gravel)	5 to 14	Slightly (sandy, silty, clayey, gravelly)	15 to 29	Sandy, silty, clayey, gravelly	> 30	Very (sandy, silty, clayey, gravelly)												
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Fill		<b>FILL</b>	Made Ground	Classifications of soils in this geotechnical report and as shown on the exploration logs are based on visual field and/or laboratory observations, which include density/consistency, moisture condition, grain size, and plasticity estimates, and should not be construed to imply field or laboratory testing unless presented herein. Visual-manual and/or laboratory classification methods of ASTM D2487 and D2488 were used as an identification guide for the Unified Soil Classification System.																							



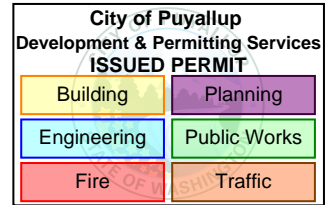


15365 NE 90th Street, Suite 100  
 Redmond, WA 98052  
 Office (425) 449-4704 | esnw.com  
 Branch Office: Pasco, WA

# TEST PIT NUMBER TP-1

PROJECT NUMBER ES-4556.02 PROJECT NAME Cascade Christian School  
 DATE STARTED 6/17/24 COMPLETED 6/17/24 GROUND ELEVATION \_\_\_\_\_  
 EXCAVATION CONTRACTOR Client Provided LATITUDE 47.18441 LONGITUDE -122.26666  
 LOGGED BY SES CHECKED BY CGH GROUND WATER LEVEL:  
 NOTES \_\_\_\_\_  $\nabla$  AT TIME OF EXCAVATION \_\_\_\_\_  
 SURFACE CONDITIONS Grass AFTER EXCAVATION \_\_\_\_\_

DEPTH (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
0.0					
			TPSL		Dark brown TOPSOIL, minimal root intrusions
				1.0	
					Dark brown silty SAND, medium dense, moist
2.5					
	GB	MC = 14.1 Fines = 23.9 OC = 1.4			-infiltration test at 3' [USDA Classification: slightly gravelly loamy SAND] -becomes wet (post-test)
			SM		
5.0					
	GB	MC = 25.2 Fines = 27.4			[USDA Classification: loamy SAND]
7.5					
			ML		Gray sandy SILT, medium dense, wet
				8.5	
	GB	MC = 30.1 Fines = 62.2			[USDA Classification: LOAM]



Test pit terminated at 8.5 feet below existing grade. No groundwater encountered during excavation. No caving observed.

LIMITATIONS: Ground elevation (if listed) is approximate; the test location was not surveyed. Coordinates are approximate and based on the WGS84 datum. Do not rely on this test log as a standalone document. Refer to the text of the geotechnical report for a complete understanding of subsurface conditions.



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# TEST PIT NUMBER TP-2

PROJECT NUMBER ES-4556.02 PROJECT NAME Cascade Christian School  
 DATE STARTED 6/17/24 COMPLETED 6/17/24 GROUND ELEVATION \_\_\_\_\_  
 EXCAVATION CONTRACTOR Client Provided LATITUDE 47.18457 LONGITUDE -122.26647  
 LOGGED BY SES CHECKED BY CGH GROUND WATER LEVEL:  
 NOTES \_\_\_\_\_  $\nabla$  AT TIME OF EXCAVATION 9.5 ft  
 SURFACE CONDITIONS Grass AFTER EXCAVATION \_\_\_\_\_

DEPTH (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
0.0					
			TPSL		Dark brown TOPSOIL, shallow root intrusions
					Dark brown silty SAND, loose to medium dense, damp
2.5	GB	MC = 12.4 Fines = 21.8 OC = 1.0			[USDA Classification: loamy SAND]  -becomes gray
5.0	GB	MC = 35.6	SM		-becomes wet
7.5	GB	MC = 24.3			
					$\nabla$ -groundwater table at 9.5'

City of Puyallup  
 Development & Permitting Services  
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Engineering	Public Works
Fire	Traffic

Test pit terminated at 9.5 feet below existing grade. Groundwater table encountered at 9.5 feet during excavation. No caving observed.

LIMITATIONS: Ground elevation (if listed) is approximate; the test location was not surveyed. Coordinates are approximate and based on the WGS84 datum. Do not rely on this test log as a standalone document. Refer to the text of the geotechnical report for a complete understanding of subsurface conditions.

GENERAL BH / TP / WELL - 4556-2.GPJ - GINT US.GDT - 10/11/24



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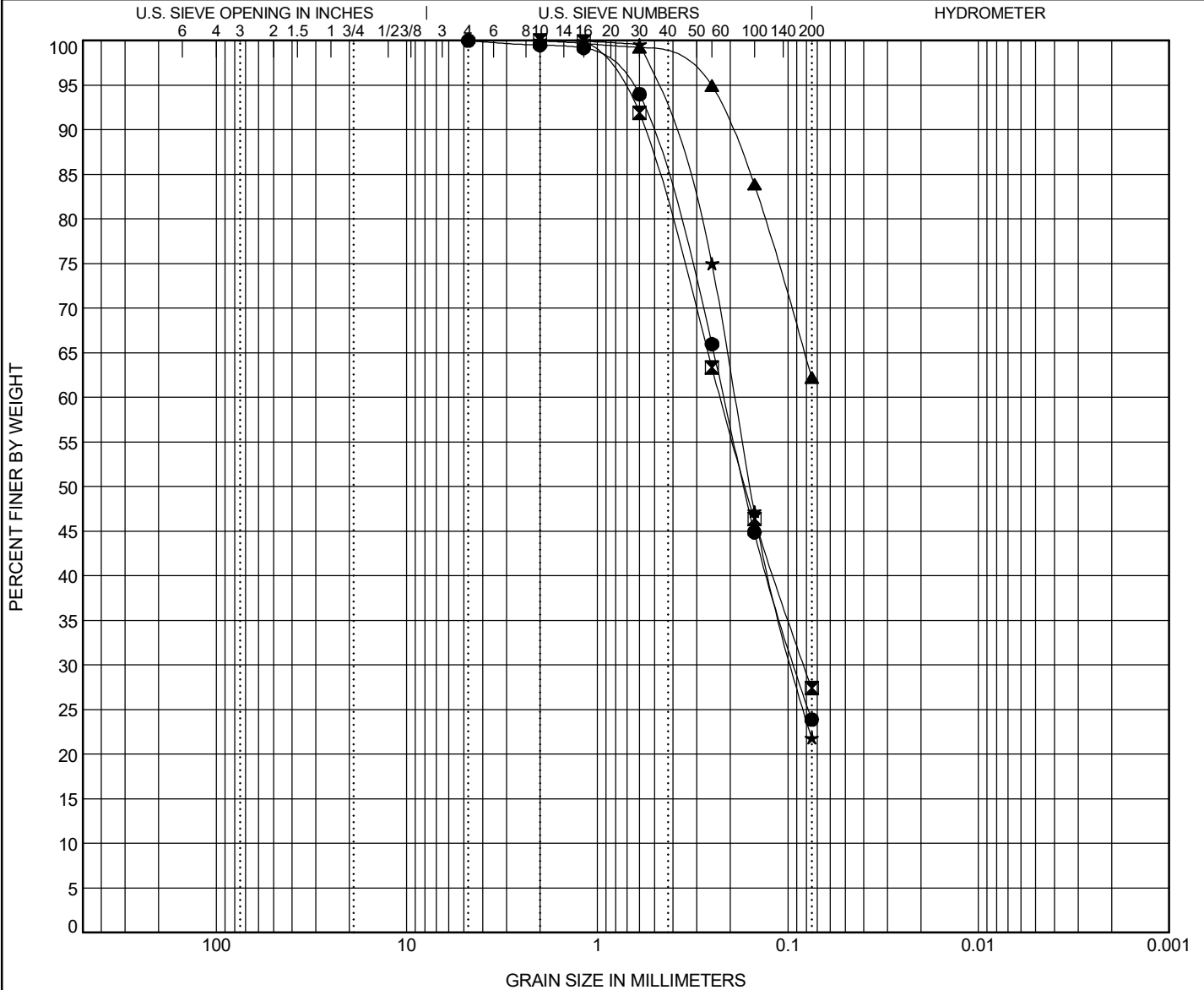
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# GRAIN SIZE DISTRIBUTION

PROJECT NUMBER ES-4556.02

PROJECT NAME Cascade Christian School



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification						Cc	Cu
● TP-01 3.00ft.	<b>USDA: Dark Brown Slightly Gravelly Loamy Sand. USCS: SM.</b>							
☒ TP-01 5.00ft.	<b>USDA: Dark Brown Loamy Sand. USCS: SM.</b>							
▲ TP-01 8.50ft.	<b>USDA: Dark Brown Loam. USCS: Sandy ML.</b>							
★ TP-02 2.50ft.	<b>USDA: Dark Brown Loamy Sand. USCS: SM.</b>							

Specimen Identification	D100	D60	D30	D10	LL	PL	PI	%Silt	%Clay
● TP-01 3.0ft.	4.75	0.216	0.092					23.9	
☒ TP-01 5.0ft.	2	0.226	0.082					27.4	
▲ TP-01 8.5ft.	2							62.2	
★ TP-02 2.5ft.	2	0.19	0.094					21.8	

GRAIN SIZE USDA ES-4556.02 CASCADE CHRISTIAN SCHOOL.GPJ GINT US LAB.GDT 6/25/24





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# FIELD REPORT

Project No.	4556.03	Page	1 of 2	Report ID	4556.03 E 2024 0808
Date	Thu 8/8/2024	Weather	Clear, 70s		
Arrival/Departure Time(s)	11:30-12:00	Travel Time (hr)	0.5		
Project Name	Cascade Christian School				
Location	819 & 911 – 21 <sup>st</sup> St SE, Puyallup				
ESNW Rep. & Phone	Brian Hebert   206-474-9643				
Client Info/Contact	Cascade Christian Schools   Ray Ossman				
General Contractor Info/Contact	Mountain Construction   Ken Schmidt				
Reviewed By	LAC		CGH		
	Field Supervisor	Initials/Date	Project Manager	Initials/Date	
<p><b>Limitations:</b> The presence of our field representative at the site is to provide our client with a source of professional advice, opinions, and recommendations based upon the field representative's observations and testing of the contractor's work. Our services do not include supervision or direction of the contractor, their employees, or agents. Geotechnical recommendations for obtaining project objectives may be made by our representatives; however, direction of the actual work should come from the owner or contractor, as appropriate. The contractor is responsible for complying with the contract documents at all times, regardless of the presence of our field representative. Jobsite safety, including compliance with all applicable state or federal regulations, is the sole responsibility of the contractor. The observations, recommendations, and conclusions provided in this field report are preliminary until reviewed by the ESNW project manager.</p>					

As requested, ESNW rep was onsite to observe fill and test compaction.

While onsite, ESNW observed the contractor grade and fill the area with imported material in approximately 6 inch lifts and compact with a large vibrating single-drum roller. Water was added to material during compaction. **Density test results indicate adequate compaction of at least 95% of the maximum dry density at the tested locations.** ESNW will return if requested.



Test Number	Test Location	Elevation	Reference Proctor	Maximum Dry Density (pcf)	Test Moisture (%)	Test Dry Density (pcf)	% of MDD
1	General Fill	-1.5'	1	133.3	7.0	127.1	95
2	General Fill	-1.5'	1	133.3	6.4	127.8	96



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Fire	Traffic

# FIELD REPORT

Project No.	4556.03	Page	1 of 2	Report ID	4556.03 E 2024 0809
Date	Fri 8/9/2024	Weather	Clear, 70s		
Arrival/Departure Time(s)	11:30-12:00	Travel Time (hr)	1.0		
Project Name	Cascade Christian School				
Location	819 & 911 – 21 <sup>st</sup> St SE, Puyallup				
ESNW Rep. & Phone	Brian Hebert   206-474-9643				
Client Info/Contact	Cascade Christian Schools   Ray Ossman				
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Reviewed By	LAC		CGH		
	Field Supervisor	Initials/Date	Project Manager	Initials/Date	
<small>Limitations: The presence of our field representative at the site is to provide our client with a source of professional advice, opinions, and recommendations based upon the field representative's observations and testing of the contractor's work. Our services do not include supervision or direction of the contractor, their employees, or agents. Geotechnical recommendations for obtaining project objectives may be made by our representatives; however, direction of the actual work should come from the owner or contractor, as appropriate. The contractor is responsible for complying with the contract documents at all times, regardless of the presence of our field representative. Jobsite safety, including compliance with all applicable state or federal regulations, is the sole responsibility of the contractor. The observations, recommendations, and conclusions provided in this field report are preliminary until reviewed by the ESNW project manager.</small>					

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# FIELD REPORT

Project No.	4556.03	Page	1 of 2	Report ID	4556.03 E 2024 0813
Date	Tue 8/13/2024	Weather	Cloudy, 60s		
Arrival/Departure Time(s)	3:45-4:00	Travel Time (hr)	0.75		
Project Name	Cascade Christian School				
Location	819 & 911 – 21 <sup>st</sup> St SE, Puyallup				
ESNW Rep. & Phone	Brian Hebert   206-474-9643				
Client Info/Contact	Cascade Christian Schools   Ray Ossman				
General Contractor Info/Contact	Mountain Construction   Ken Schmidt				
Reviewed By	LAC Field Supervisor	<i>LAC</i> Initials/Date	CGH Project Manager	<i>CGH</i> Initials/Date	
<small>Limitations: The presence of our field representative at the site is to provide our client with a source of professional advice, opinions, and recommendations based upon the field representative's observations and testing of the contractor's work. Our services do not include supervision or direction of the contractor, their employees, or agents. Geotechnical recommendations for obtaining project objectives may be made by our representatives; however, direction of the actual work should come from the owner or contractor, as appropriate. The contractor is responsible for complying with the contract documents at all times, regardless of the presence of our field representative. Jobsite safety, including compliance with all applicable state or federal regulations, is the sole responsibility of the contractor. The observations, recommendations, and conclusions provided in this field report are preliminary until reviewed by the ESNW project manager.</small>					

As requested, ESNW rep was onsite to observe fill and test compaction.

While onsite, ESNW observed the contractor grade and fill the area with imported material in approximately 6 inch lifts and compact with a large vibrating single-drum roller. Water was added to material during compaction. ESNW rep probed the compacted fill with a ½-inch diameter steel T-probe. **ESNW rep observed the area to be dense.** ESNW will return if requested.





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