



2525 Hyacinth Street NE  
 Salem, OR 97301  
 Phone: 503-399-1432

To: EWS - Engineered Wood Solutions

ATT: Engineer of Record  
 Architect of Record

The contents of this submittal are provided for your review and approval as required by the contract documents. We request that you review the attached documents at your earliest convenience.

PROJECT NAME	DATE:
Bradley Park Care Center Pavilion Building	1/21/2026
PROJECT NUMBER	
2501272-Pavilion Building	

ATTACHMENTS	DATE	PAGES	SPECIFICATIONS
PDF	1/21/2026	1	Roof Truss Layout
PDF	1/21/2026	4	Roof Truss Sealed Component Drawings

**SUBMITAL TYPE**

- Shop Drawings
- Submittal
- Resubmittal
- Final Construction Documents
- For View Only
- Other

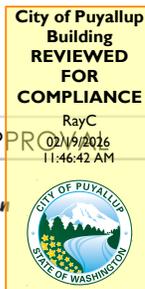
**APPROVAL COMMENTS**

Please acknowledge receipt of this submittal and provide your comments and approvals as per the project schedule.

Note: Sheets S7.1 and A5.5 do not match in their information and contain discrepancies regarding the lower roof at the pavilion building. Essentially, one sheet calls for 2x8 @ 24" O.C., while the other shows mono trusses on one side of the building. Our design is based on Sheet S7.1, as the date on that drawing is more recent.

AUTHORIZED SIGNATURE OF APPROVER

*Graham Orthmann*

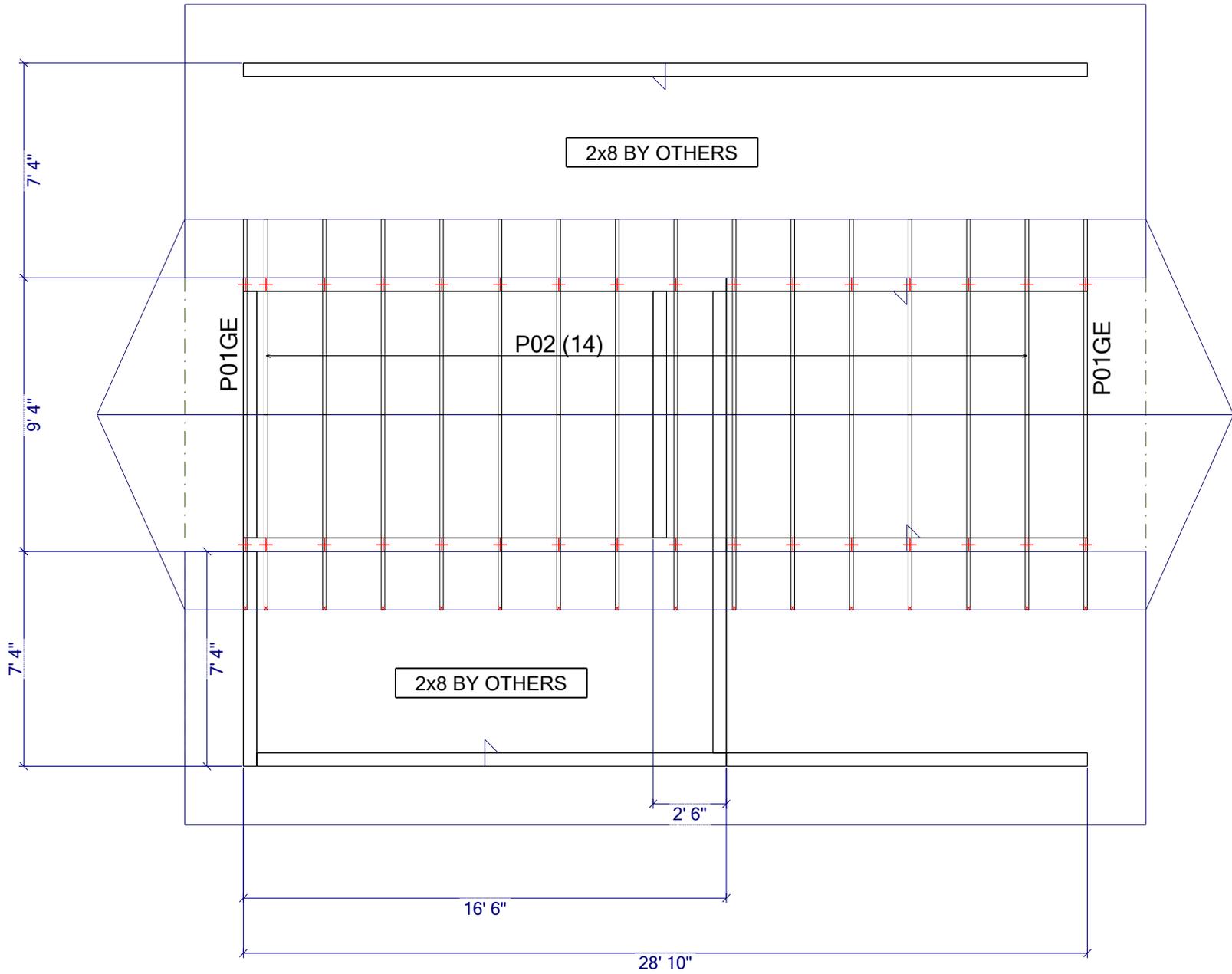


The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permittee on site for inspection.

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for ensuring that the building complies with all applicable codes and regulations of the local government.





Builder: EWS - Engineered Wood Solutions

Job: 2501272 - Pavilion Building  
 707 39th Avenue SE  
 Puyallup, WA

Designer: Gerardo Garcia



706763  
 01/20/2026

LOADING

TCLL: 25 psf  
 TCCL: 15 psf  
 BCLL: 0 psf  
 BCDL: 6 psf

Total Loads: 46 psf

Snow GSL: 25 psf

Mustang Truss  
 2525 Hyacinth St. NE  
 Salem, OR 97301

Office: 503-399-1432

Fax: 503-399-1435

Toll Free: 866-391-8787



The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Mustang Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **2501272 - Pavilion Building - 1256593**  
P01GE, P02

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

01/21/2026



---

Arturo A. Hernandez (WA, 42169)

My license expiration date for the state of WA is 11/07/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

### DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, |, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

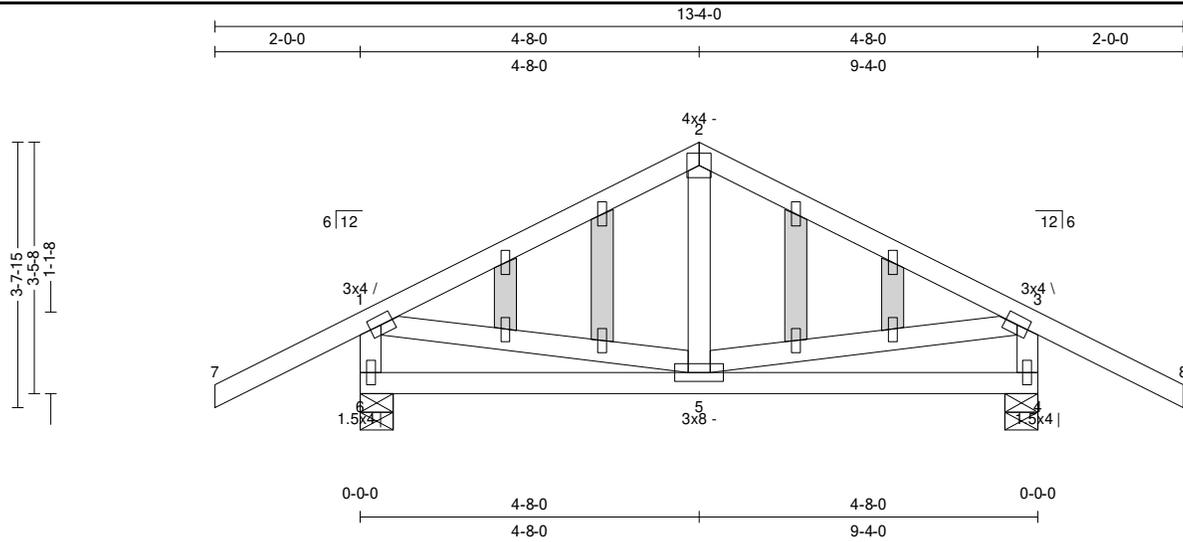
The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS:** National Design Specification for Wood Construction
- ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)

SPAN 9-4-0	PITCH 6/12	QTY 2	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 56 lbs
---------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.78 (7-1)	Vert TL: 0.02 in	L/999	(4-5)	L/180
TCDL: 15	TPI 1-2014	BC: 0.19 (5-6)	Vert LL: 0.01 in	L/999	(4-5)	L/240
BCLL: 0	Rep Mbr: No	Web: 0.13 (1-6)	Horz TL: 0 in		4	
BCDL: 6	Lumber D.O.L.: 115 %					

01/21/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	731 lbs			-141 lbs	-141 lbs	21 lbs
4	1	5.5 in	1.50 in	731 lbs			-141 lbs	-141 lbs	

**Material**

TC: DFL #2 2 x 4  
 BC: DFL #2 2 x 4  
 Web: DFL Stud 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (25 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 25 psf Roof (GSL = 32 psf), Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

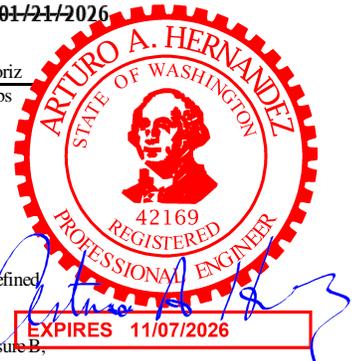
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.579	(-466 lbs)	2-3	0.579	(-466 lbs)
BC						
Web	1-6	0.132	(-707 lbs)	3-5	0.103	308 lbs
	1-5	0.103	308 lbs	3-4	0.132	(-707 lbs)

**Notes**

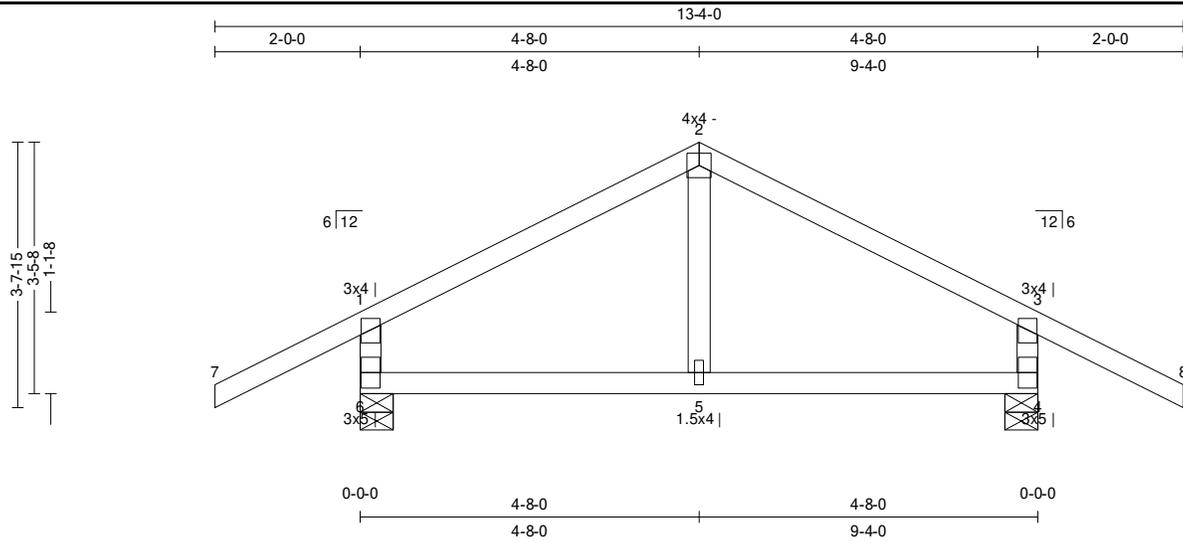
- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 16" OC, U.N.O.
- Attach structural gable blocks with 1.5x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14  
 Eagle Metal Products

SPAN 9-4-0	PITCH 6/12	QTY 14	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 38 lbs
---------------	---------------	-----------	--------------	--------------	-----------------	-----------------	-------------	------------------	-------------------

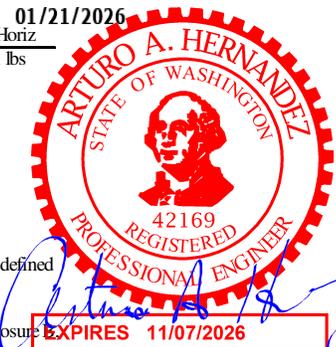


All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.68 (7-1)	Vert TL: 0.05 in	L/999	5	L/180
TCDL: 15	TPI 1-2014	BC: 0.36 (5-6)	Vert LL: 0.03 in	L/999	5	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.65 (1-6)	Horz TL: 0 in		4	
BCDL: 6	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	731 lbs			-141 lbs	-141 lbs	21 lbs
4	1	5.5 in	1.50 in	731 lbs			-141 lbs	-141 lbs	



**Material**

TC: DFL #2 2 x 4  
 BC: DFL #2 2 x 4  
 Web: DFL Stud 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (25 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 25 psf Roof (GSL = 32 psf), Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.536	(-427 lbs)	2-3	0.536	(-427 lbs)
BC						
Web	1-6	0.651	(-662 lbs)	3-4	0.651	(-662 lbs)

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14  
 Eagle Metal Products