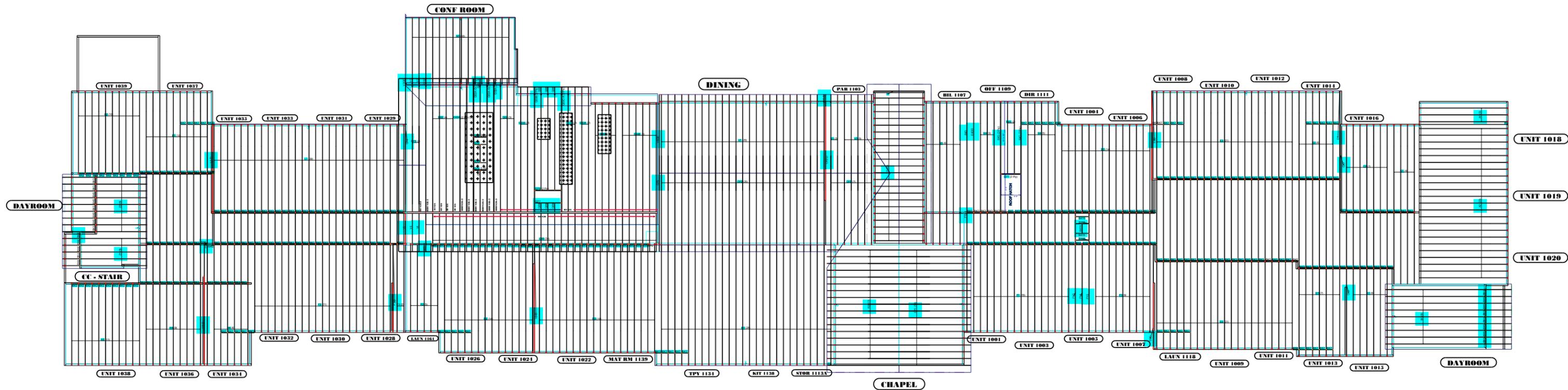
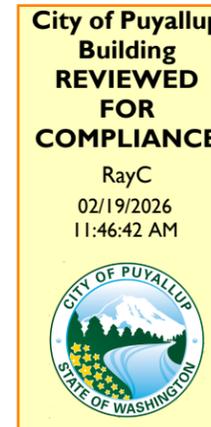
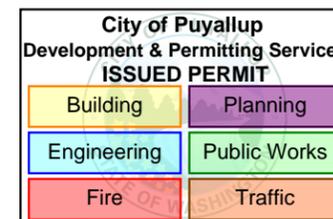


TOP PLATE HEIGHTS	
Height	Color
0"	
1'-0"	
2'-0"	
10'-0"	
12'-0-3/4"	
12'-0"	
13'-9-1/2"	
14'-0-3/4"	
14'-1"	
16'-2"	
17'-10"	
20'-9"	



See Notes on 10/25/25 Truss Layout Rev. (2)



Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permittee on site for inspection.

Builder: EWS - Engineered Wood Solutions

Job: 2501272
707 39th Avenue SE
Puyallup, WA

Designer: Anthony Wagner



343031
01/23/2026

LOADING

TCLL: 25 psf
TCDL: 15 psf
BCLL: 0 psf
BCDL: 6 psf

Total Loads: 46 psf

Snow GSL: 25 psf

Mustang Truss
2525 Hyacinth St. NE
Salem, OR 97301

Office: 503-399-1432
Fax: 503-399-1435
Toll Free: 866-391-8787

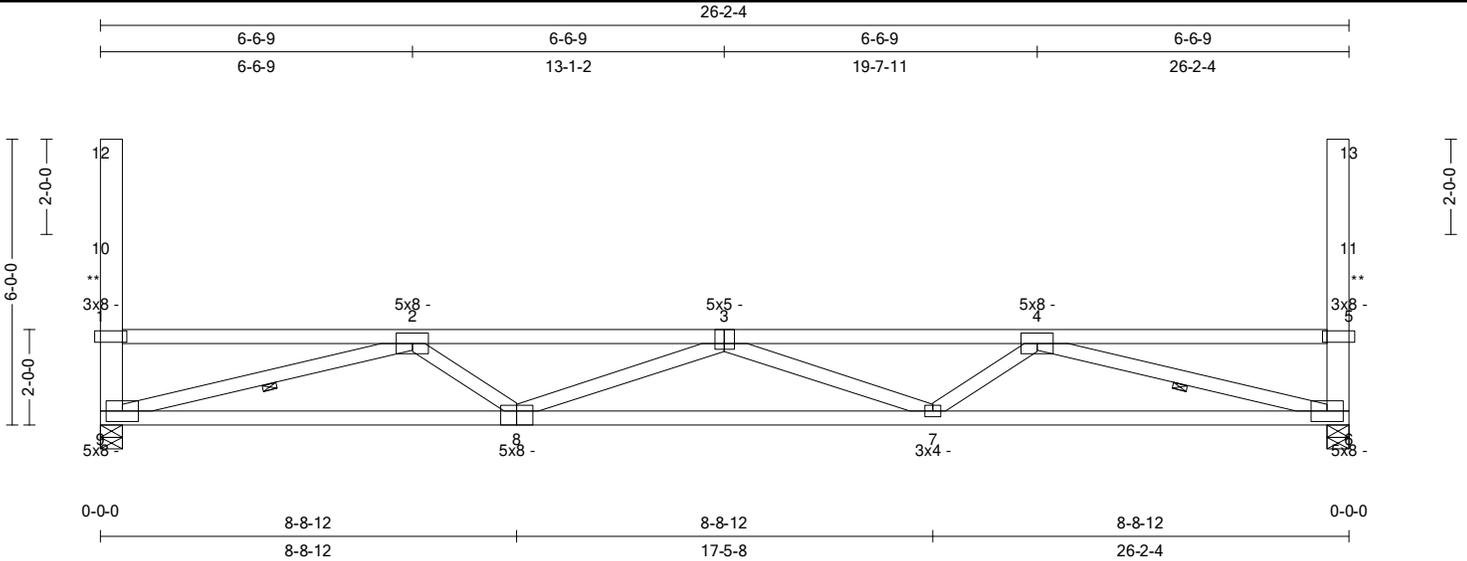




Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **AI**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:12
 Page: 1 of 2

SPAN 26-2-4	PITCH 0/12	QTY 3	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 127 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.70 (1-2)	Vert TL: 0.75 in	L/404	(7-8)	L/180
TCDL: 15	TPI 1-2014	BC: 0.97 (7-8)	Vert LL: 0.4 in	L/756	(7-8)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.81 (4-6)	Horz TL: 0.14 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,205 lbs	.	.	-70 lbs	-70 lbs	-139 lbs
6	1	5.5 in	1.50 in	1,205 lbs	.	.	-70 lbs	-70 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 12-9, 13-6

Bracing

TC: Sheathed or Purlins at 2-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-9, 4-6

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to members 10-1 & 11-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.697	401 lbs	2-3	0.642	599 lbs	(-3,958 lbs)	3-4	0.642	599 lbs	(-3,958 lbs)	4-5	0.697	401 lbs
BC	6-7	0.838	3,492 lbs	(-465 lbs)	7-8	0.975	4,614 lbs	(-524 lbs)	8-9	0.838	3,492 lbs	(-465 lbs)		
Web	2-9	0.805		(-3,618 lbs)	3-7	0.314		(-744 lbs)						
	2-8	0.285	644 lbs		4-7	0.285	644 lbs							
	3-8	0.314		(-744 lbs)	4-6	0.805		(-3,618 lbs)						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: A1
 Job: [2501272](#)
 Designer:Anthony
 Date: 01/23/26 09:23:12
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-2-4	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	127 lbs

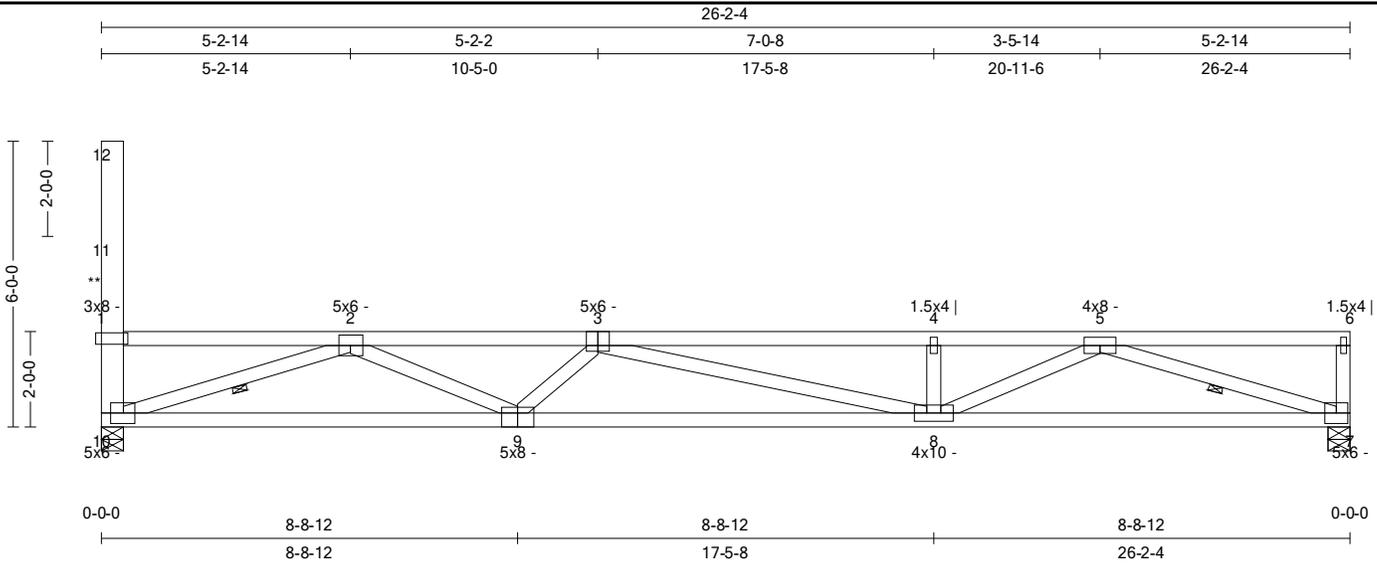
9) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 26-2-4	PITCH 0/12	QTY 23	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 119 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.76 (3-4)	Vert TL: 0.65 in	L/ 468	(8-9)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.98 (8-9)	Vert LL: 0.34 in	L/ 879	(8-9)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.67 (3-8)	Horz TL: 0.13 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	1,199 lbs	.	.	-47 lbs	-47 lbs	134 lbs
7	1	5.5 in	1.50 in	1,210 lbs	.	.	-107 lbs	-107 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 12-10

Bracing

TC: Sheathed or Purlins at 2-4-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-1-0, Purlin design by Others.
 Web: One Midpoint Row: 2-10, 5-7

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 11-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.310	401 lbs	3-4	0.756	385 lbs	(-4,168 lbs)		
	2-3	0.422	487 lbs	4-5	0.399	385 lbs	(-4,168 lbs)		
BC	7-8	0.786	2,876 lbs	8-9	0.980	4,445 lbs	(-493 lbs)	9-10	0.768
									2,861 lbs
Web	2-10	0.511	(-3,023 lbs)	3-8	0.670	320 lbs	(-708 lbs)	5-7	0.530
	2-9	0.560	1,268 lbs	4-8	0.058		(-454 lbs)		301 lbs
	3-9	0.096	(-632 lbs)	5-8	0.635	1,438 lbs			(-3,034 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSL-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **A2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:12
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-2-4	0/12	23	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

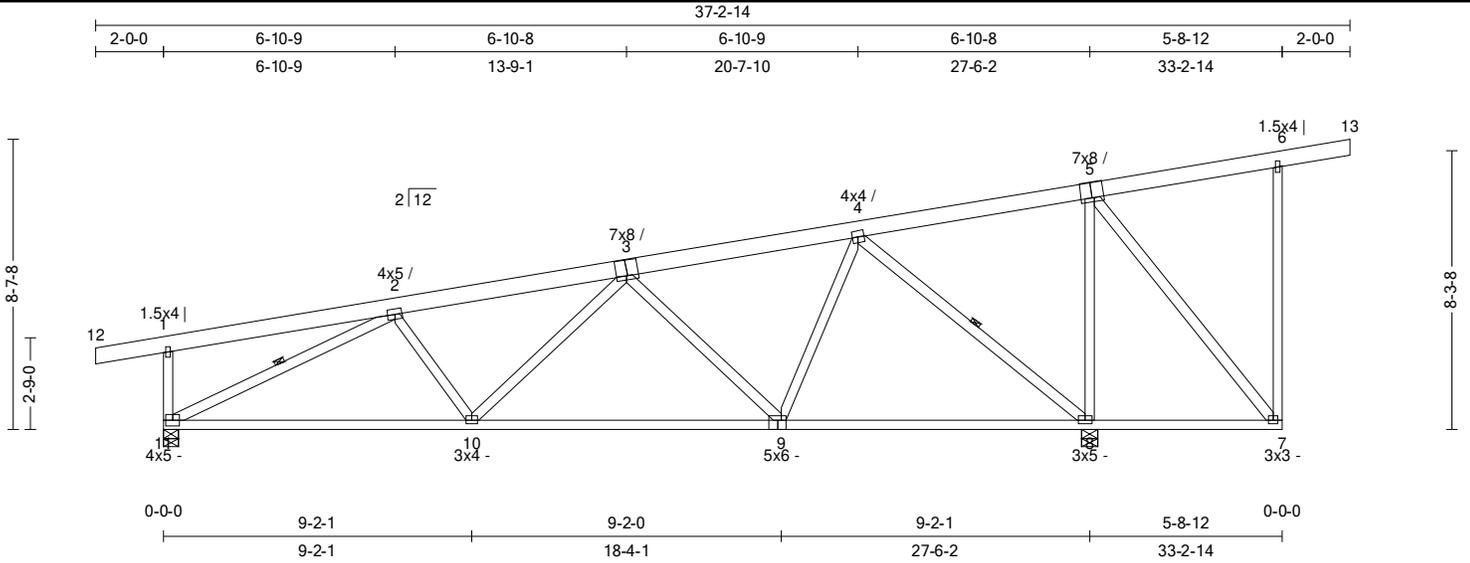
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Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: B1
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:12
 Page: 1 of 1

SPAN 33-2-14	PITCH 2/12	QTY 9	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 200 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.21 (4-5)	Vert TL: 0.35 in	L/932	(10-11)	L/180
TCDL: 15	TPI 1-2014	BC: 0.76 (10-11)	Vert LL: 0.21 in	L/999	(10-11)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.80 (5-8)	Cant / OH TL: 0.04 in UP	2L/999	7	2L/480
BCDL: 6	Lumber D.O.L.: 115 %		Cant / OH LL: 0.03 in UP	2L/999	7	2L/480
			Horz TL: 0.06 in		8	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,344 lbs	154 lbs
8	1	5.5 in	2.17 in	2,034 lbs	.	.	-107 lbs	-107 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 4-8, 5-7

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-11, 4-8

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.152	(-1,906 lbs)	4-5	0.214	369 lbs			
	3-4	0.159	(-1,343 lbs)						
BC	7-8	0.416	(-313 lbs)	8-9	0.662	1,030 lbs	9-10	0.753	1,773 lbs
Web	1-11	0.092	(-427 lbs)	4-9	0.318	720 lbs	5-7	0.164	502 lbs
	2-11	0.574	(-2,029 lbs)	4-8	0.499	(-1,500 lbs)	6-7	0.652	(-372 lbs)
	3-9	0.588	(-775 lbs)	5-8	0.803	(-966 lbs)			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

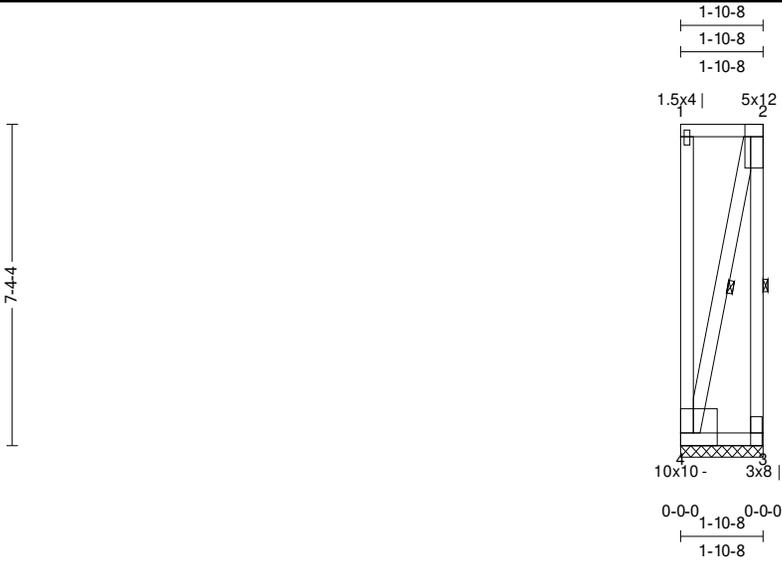
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Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK1
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:12
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 8	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 35 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/ 999	(3-4)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/ 999	(3-4)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (2-4)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	2,548 lbs	-2,469 lbs	-255 lbs	-32 lbs	-2,469 lbs	-102 lbs
1	22.5 in	N/A	2,548 lbs	-2,469 lbs	-255 lbs	-32 lbs	-2,469 lbs	563 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Standard 2 x 4: 2-4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-4, 2-3

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.066	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.816	2,571 lbs	(-2,571 lbs)	2-3	0.527	2,480 lbs	(-2,537 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

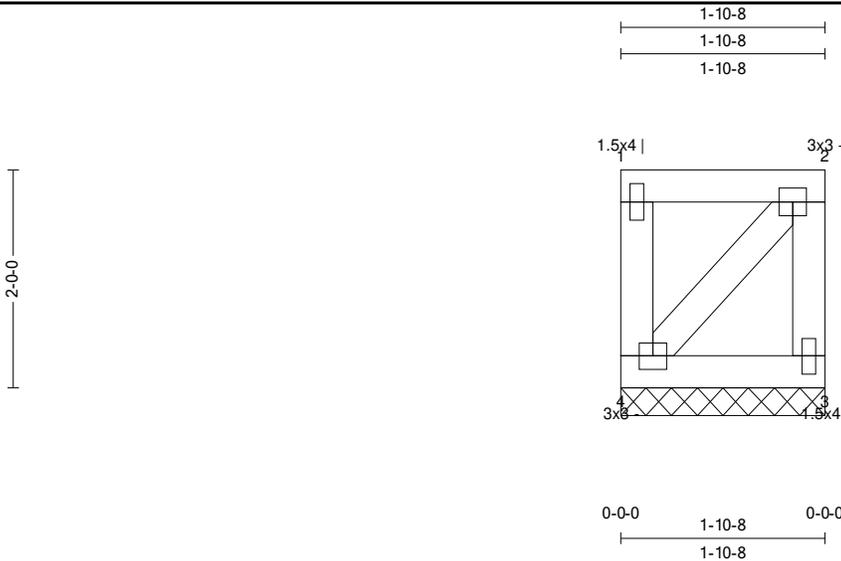
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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK2
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:12
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 257	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 11 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.05 (1-2)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.02 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.06 (1-4)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	86 lbs	.	-8 lbs	-32 lbs	-32 lbs	-28 lbs
1	22.5 in	N/A	86 lbs	.	-8 lbs	-32 lbs	-32 lbs	28 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

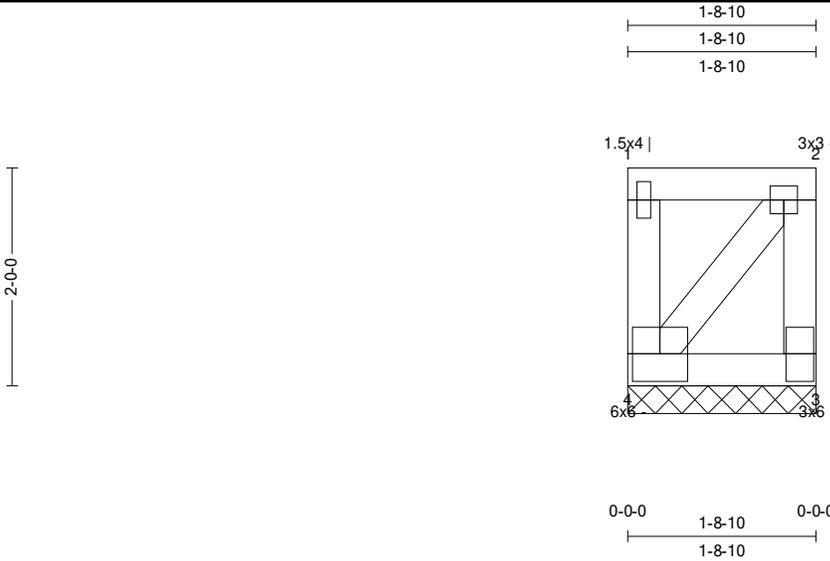
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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK3
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:12
 Page: 1 of 1

SPAN 1-8-10	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 12 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/180
TCDL: 15	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/240
BCLL: 0	Rep Mbr: No	Web: 0.26 (2-4)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	20.625 in	N/A	653 lbs	-581 lbs	-10 lbs	-29 lbs	-581 lbs	-28 lbs
1	20.625 in	N/A	653 lbs	-581 lbs	-10 lbs	-29 lbs	-581 lbs	-516 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 516 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.066	472 lbs	(-472 lbs)
BC	2-4	0.255	804 lbs	(-804 lbs)
Web	2-3	0.188	591 lbs	(-643 lbs)

Notes

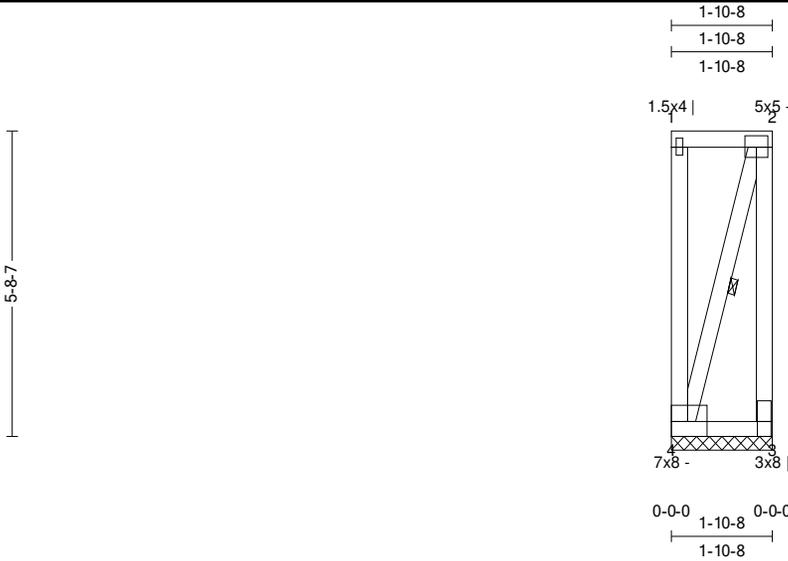
- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 1-10-8	PITCH 0/12	QTY 5	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 27 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/ 999	(3-4)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/ 999	(3-4)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.99 (2-3)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	1,962 lbs	-1,883 lbs	-147 lbs	-32 lbs	-1,883 lbs	-79 lbs
1	22.5 in	N/A	1,962 lbs	-1,883 lbs	-147 lbs	-32 lbs	-1,883 lbs	563 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-4

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7
TC	1-2	0.066	519 lbs	(-519 lbs)			
BC							
Web	2-4	0.636	2,003 lbs	(-2,003 lbs)	2-3	0.988	1,894 lbs (-1,951 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

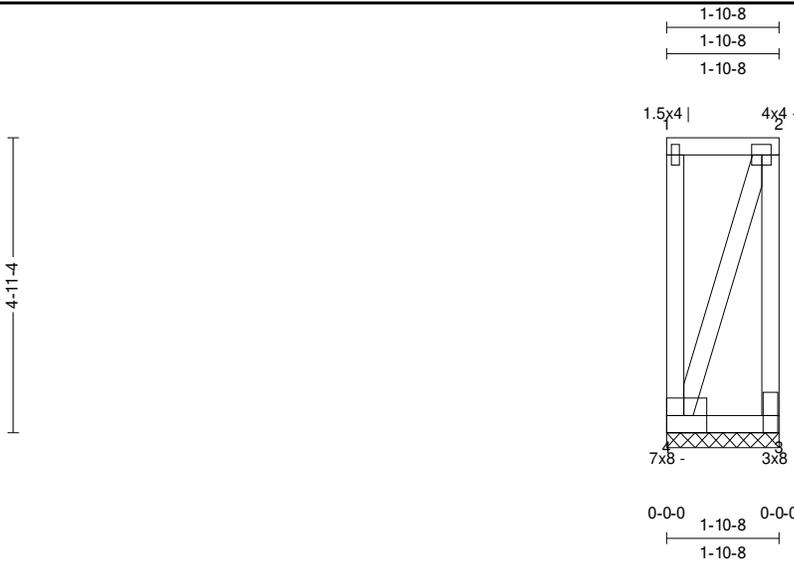
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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 Salem, OR 97301
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Truss: BK5
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:13
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 8	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 24 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/ 999	(3-4)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/ 999	(3-4)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.70 (2-4)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	1,690 lbs	-1,611 lbs	-107 lbs	-32 lbs	-1,611 lbs	-69 lbs
1	22.5 in	N/A	1,690 lbs	-1,611 lbs	-107 lbs	-32 lbs	-1,611 lbs	563 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	Force	Force
TC	1-2	0.066	519 lbs (-519 lbs)
BC			
Web	2-4	0.704	1,744 lbs (-1,744 lbs)
	2-3	0.626	1,622 lbs (-1,679 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

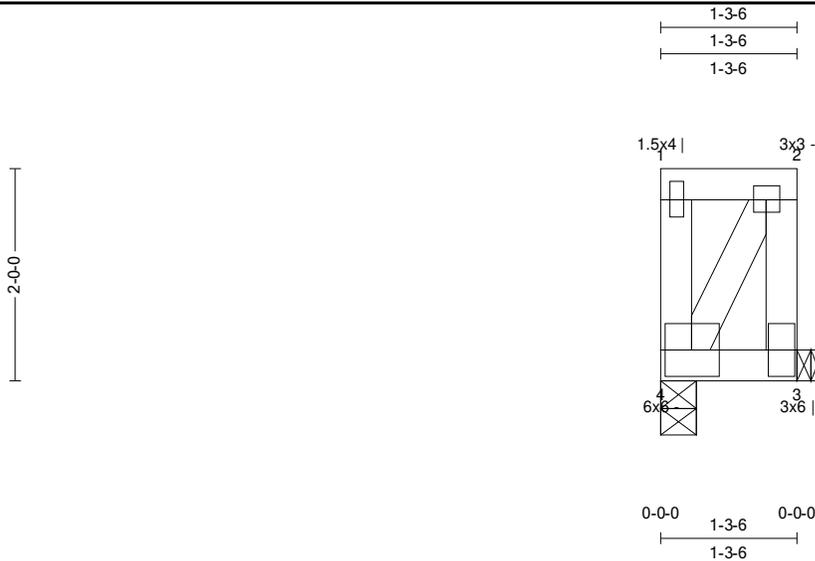
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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
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 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK6
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:13
 Page: 1 of 1

SPAN 1-3-6	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 10lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/180
TCDL: 15	TPI 1-2014	BC: 0.01 (3-4)	Vert LL: 0 in	L/999	3	L/240
BCLL: 0	Rep Mbr: No	Web: 0.24 (2-4)	Horz TL: 0 in		3	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	4 in	1.50 in	690 lbs	-637 lbs	-21 lbs	-22 lbs	-637 lbs	384 lbs
3	1	1.5 in	1.50 in	690 lbs	-637 lbs	-21 lbs	-22 lbs	-637 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 384 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	CSI	Tension (lbs)	Compression (lbs)
TC	1-2	0.048	341 lbs	(-341 lbs)
BC				
Web	2-4	0.243	767 lbs	(-767 lbs)
	2-3	0.205	644 lbs	(-683 lbs)

Notes

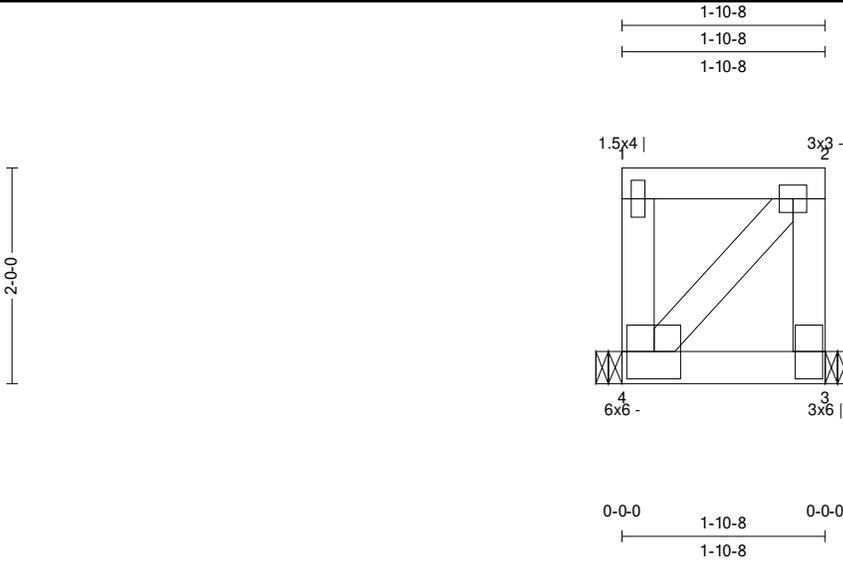
- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Nailing schedule shall be specified by truss manufacturer per NDS.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 1-10-8	PITCH 0/12	QTY 40	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 12 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/180
TCDL: 15	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.26 (2-4)	Horz TL: 0 in		3	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	646 lbs	-568 lbs	-8 lbs	-32 lbs	-568 lbs	563 lbs
3	1	1.5 in	1.50 in	646 lbs	-568 lbs	-8 lbs	-32 lbs	-568 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4
TC	1-2	0.066	519 lbs	(-519 lbs)
BC				
Web	2-4	0.263	827 lbs	(-827 lbs)
	2-3	0.184	579 lbs	(-635 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Nailing schedule shall be specified by truss manufacturer per NDS.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

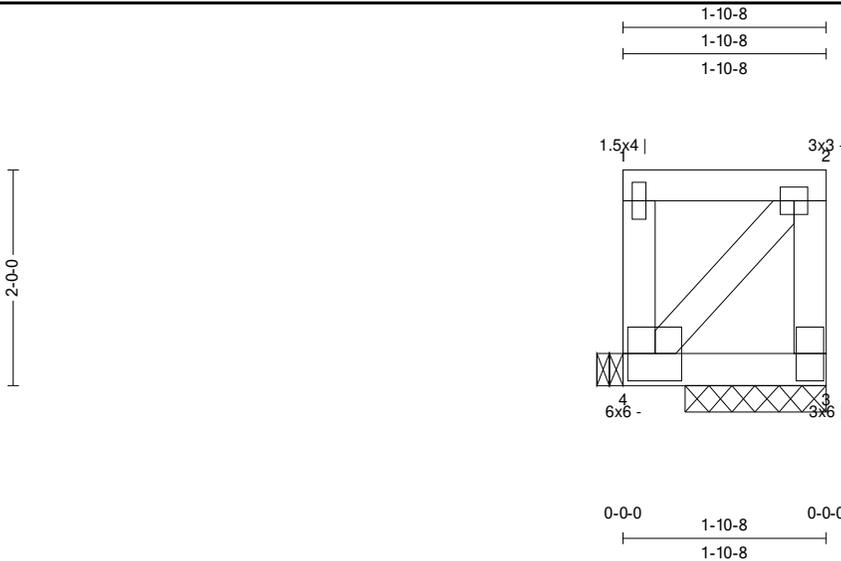
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK8
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:13
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 12 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in UP	L/999	3	L/180
TCDL: 15	TPI 1-2014	BC: 0.01 (3-4)	Vert LL: 0 in	L/999	3	L/240
BCLL: 0	Rep Mbr: No	Web: 0.26 (2-4)	Horz TL: 0 in		3	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	637 lbs	-576 lbs	-13 lbs	-37 lbs	-576 lbs	563 lbs
3	1	15.625 in	N/A	643 lbs	-571 lbs	-10 lbs	-34 lbs	-571 lbs	.
3	1	15.625 in	N/A	33 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Tension	Compression
TC	1-2	0.072	519 lbs (-519 lbs)
BC	2-4	0.263	827 lbs (-827 lbs)
Web	2-3	0.184	579 lbs (-635 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Nailing schedule shall be specified by truss manufacturer per NDS.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

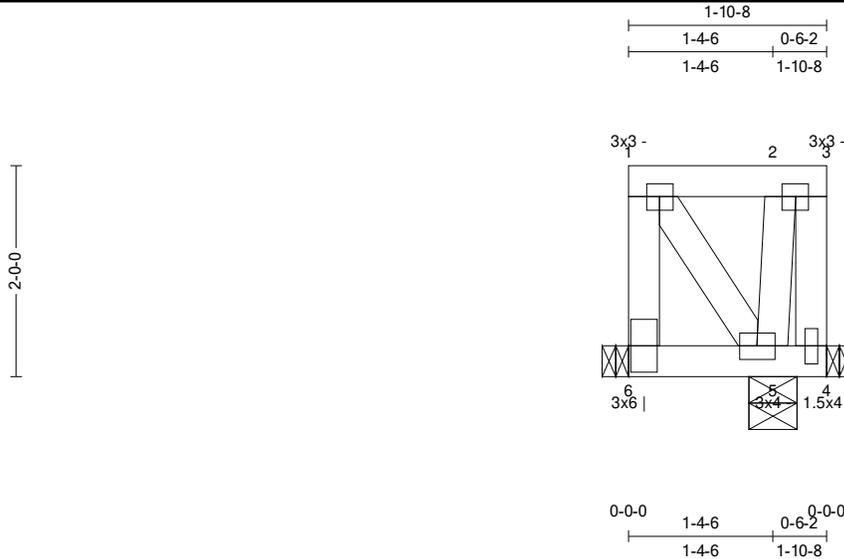
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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK9
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:13
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 13 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.06 (1-3)	Vert TL: 0 in	L/999	(5-6)	L/180
TCDL: 15	TPI 1-2014	BC: 0.08 (5-6)	Vert LL: 0 in	L/999	(5-6)	L/240
BCLL: 0	Rep Mbr: No	Web: 0.27 (1-5)	Horz TL: 0 in		4	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	1.50 in	729 lbs	-666 lbs	-13 lbs	-27 lbs	-666 lbs	-563 lbs
5	1	5.5 in	1.50 in	419 lbs	-353 lbs	-1 lbs	-23 lbs	-353 lbs	.
4	1	1.5 in	1.50 in	326 lbs	-298 lbs	-7 lbs	-15 lbs	-298 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 8-3-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has not been designed for the effects of unbalanced snow loads.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-3	0.063	454 lbs	(-448 lbs)															
BC	5-6	0.081	563 lbs	(-563 lbs)															
Web	1-6	0.214	674 lbs	(-721 lbs)	1-5	0.270	849 lbs	(-861 lbs)	3-5	0.096	303 lbs	(-335 lbs)	3-4	0.095					(-325 lbs)

Notes

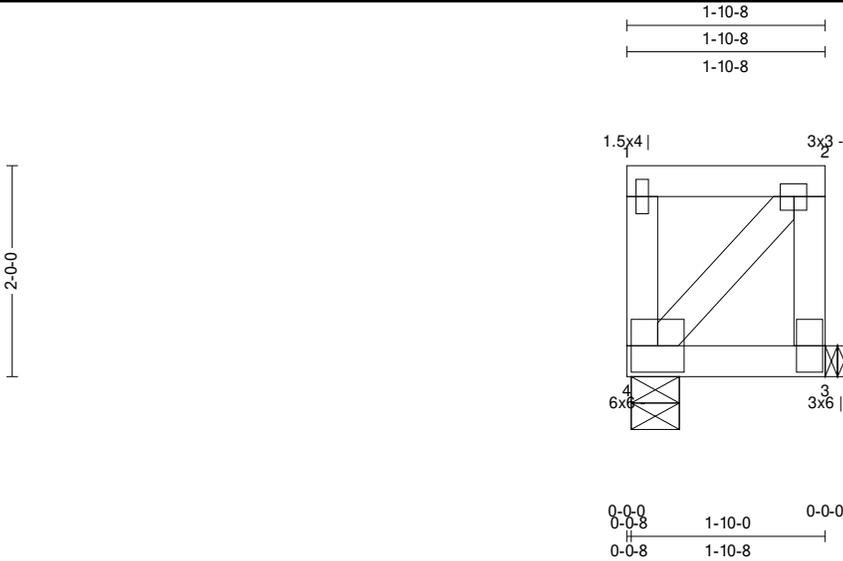
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 6, 5, 4 may need to be considered.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 1-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-8	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 12 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/180
TCDL: 15	TPI 1-2014	BC: 0.16 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/240
BCLL: 0	Rep Mbr: No	Web: 0.26 (2-4)	Cant / OH TL: 0 in UP	2L/999	(4-4)	2L/480
BCDL: 6	Lumber D.O.L.: 115 %		Cant / OH LL: 0 in UP	2L/999	4	2L/480
			Horz TL: 0 in		3	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	598 lbs	-513 lbs	-6 lbs	-36 lbs	-513 lbs	563 lbs
3	1	1.5 in	1.50 in	592 lbs	-520 lbs	-7 lbs	-29 lbs	-520 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.072	519 lbs	(-519 lbs)
BC	2-4	0.263	827 lbs	(-827 lbs)
Web	2-3	0.184	579 lbs	(-635 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Nailing schedule shall be specified by truss manufacturer per NDS.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

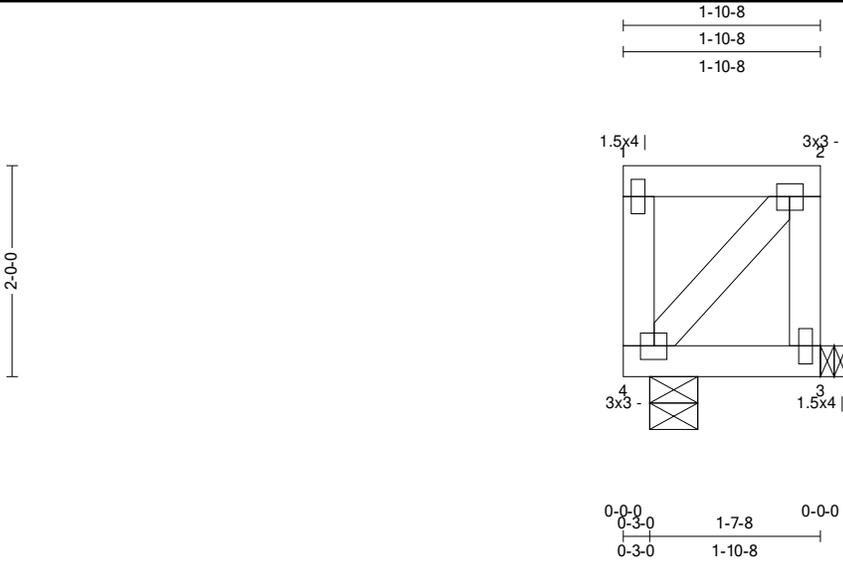
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Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK11
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:13
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-3-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 11 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.06 (1-2)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL: 0	Rep Mbr: No	Web: 0.06 (1-4)	Cant / OH TL: 0 in UP	2L / 999	4	2L / 480
BCDL: 6	Lumber D.O.L.: 115 %		Cant / OH LL: 0 in UP	2L / 999	(4-4)	2L / 480
			Horz TL: 0 in		3	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	100 lbs	.	-8 lbs	-44 lbs	-44 lbs	-32 lbs
3	1	1.5 in	1.50 in	73 lbs	.	-9 lbs	-27 lbs	-27 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Nailing schedule shall be specified by truss manufacturer per NDS.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

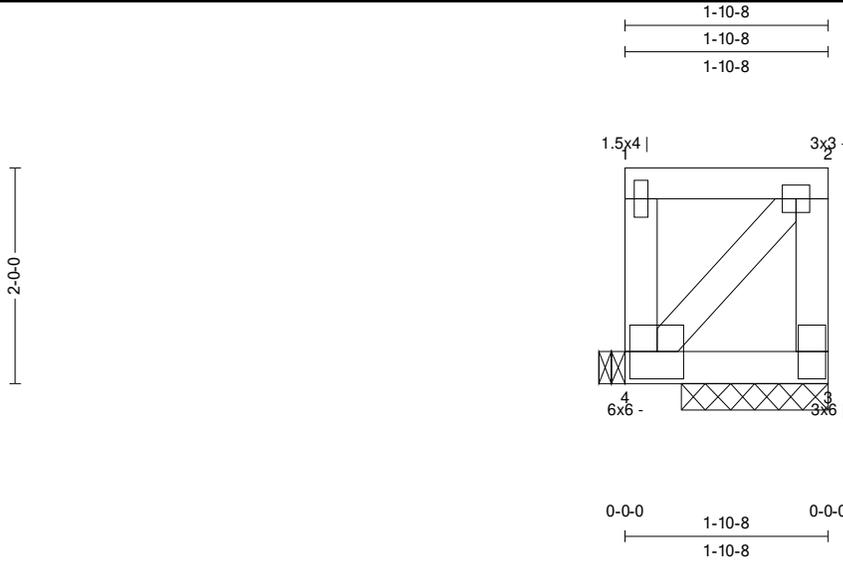
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Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK12
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:13
 Page: 1 of 1

SPAN 1-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 12 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0 in	L/999	(3-3)	L/180
TCDL: 15	TPI 1-2014	BC: 0.01 (3-4)	Vert LL: 0 in	L/999	3	L/240
BCLL: 0	Rep Mbr: No	Web: 0.26 (2-4)	Horz TL: 0 in		3	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	637 lbs	-577 lbs	-13 lbs	-38 lbs	-577 lbs	563 lbs
3	1	16.25 in	N/A	643 lbs	-571 lbs	-9 lbs	-34 lbs	-571 lbs	.
3	1	16.25 in	N/A	34 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has not been designed for the effects of unbalanced snow loads.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	1-2	0.072	519 lbs	(-519 lbs)				
TC								
BC								
Web	2-4	0.263	827 lbs	(-827 lbs)	2-3	0.184	579 lbs	(-635 lbs)

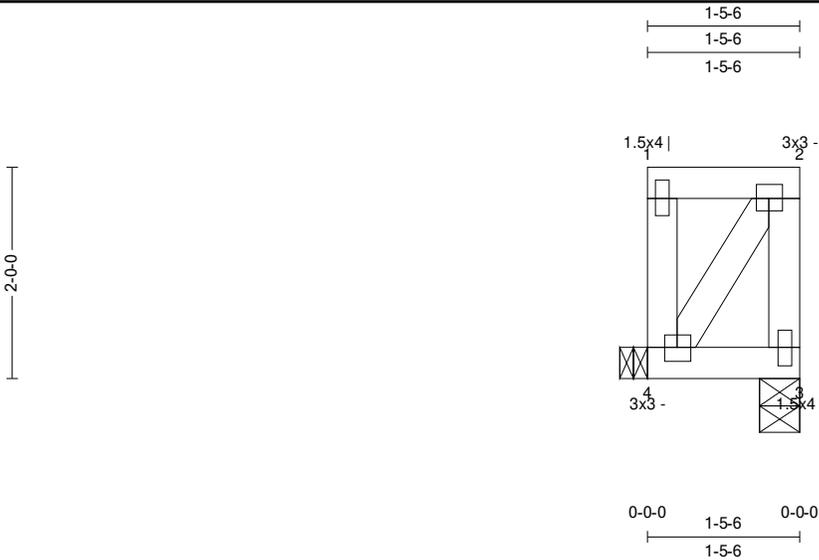
Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products

SPAN 1-5-6	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 10 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.03 (1-2)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.01 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.04 (1-4)	Horz TL: 0 in		3	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	67 lbs	.	-16 lbs	-25 lbs	-25 lbs	32 lbs
3	1	4.625 in	1.50 in	67 lbs	.	-16 lbs	-25 lbs	-25 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

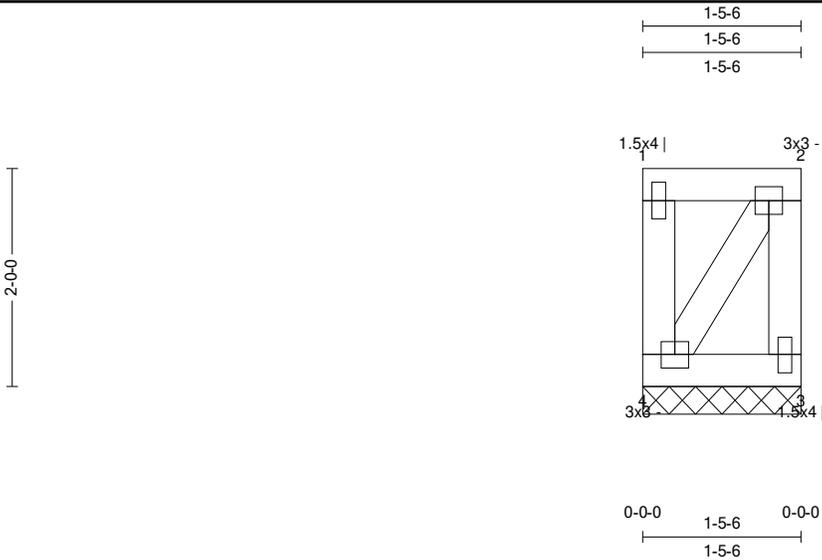
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Max CSI	Max Tension Force	Max Compression Force
TC			
BC			
Web			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 1-5-6	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 10 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.03 (1-2)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.01 (3-4)	Vert LL: 0 in	L / 999	3	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.04 (1-4)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	17.375 in	N/A	67 lbs	-	-16 lbs	-25 lbs	-25 lbs	-28 lbs
1	17.375 in	N/A	67 lbs	-	-16 lbs	-25 lbs	-25 lbs	28 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

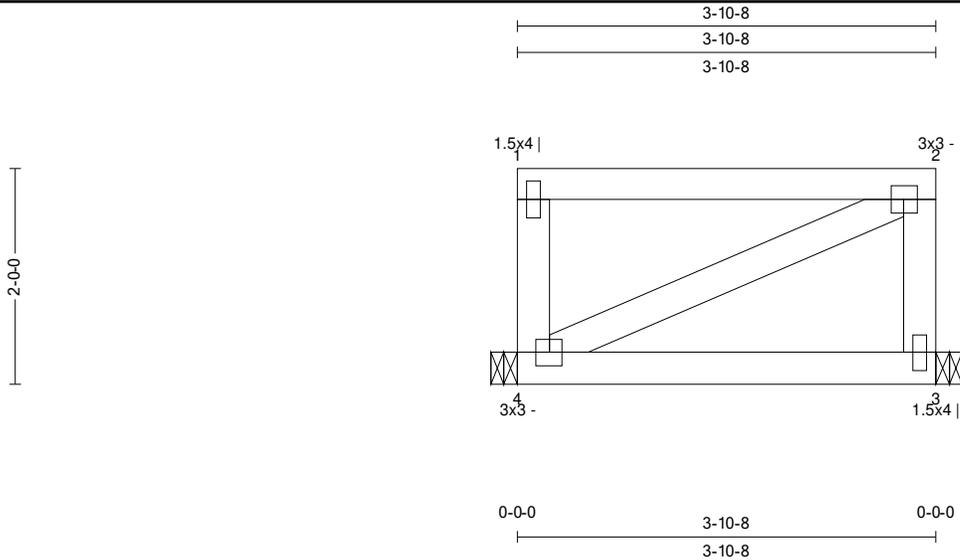
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Max CSI	Max Tension Force	Max Compression Force
TC			
BC			
Web			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 3-10-8	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 18 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.28 (1-2)	Vert TL: 0.01 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.11 (3-4)	Vert LL: 0.01 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.06 (1-4)	Horz TL: 0 in		3	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	178 lbs	.	.	-66 lbs	-66 lbs	-32 lbs
3	1	1.5 in	1.50 in	178 lbs	.	.	-66 lbs	-66 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

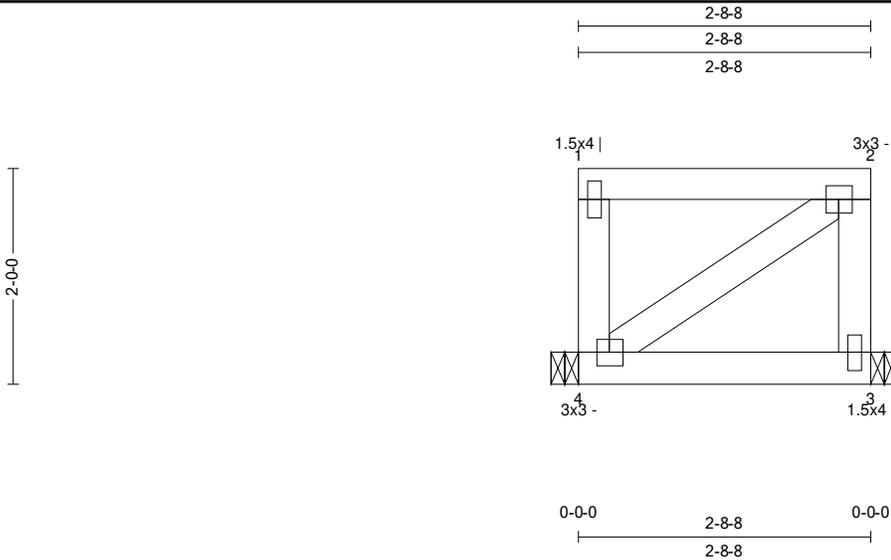
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Max CSI	Max Tension Force	Max Compression Force
TC			
BC			
Web			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 2-8-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 14 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.15 (1-2)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.06 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.05 (1-4)	Horz TL: 0 in		3	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	125 lbs	.	.	-46 lbs	-46 lbs	32 lbs
3	1	1.5 in	1.50 in	125 lbs	.	.	-46 lbs	-46 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Max CSI	Max Tension Force	Max Compression Force
TC			
BC			
Web			

Notes

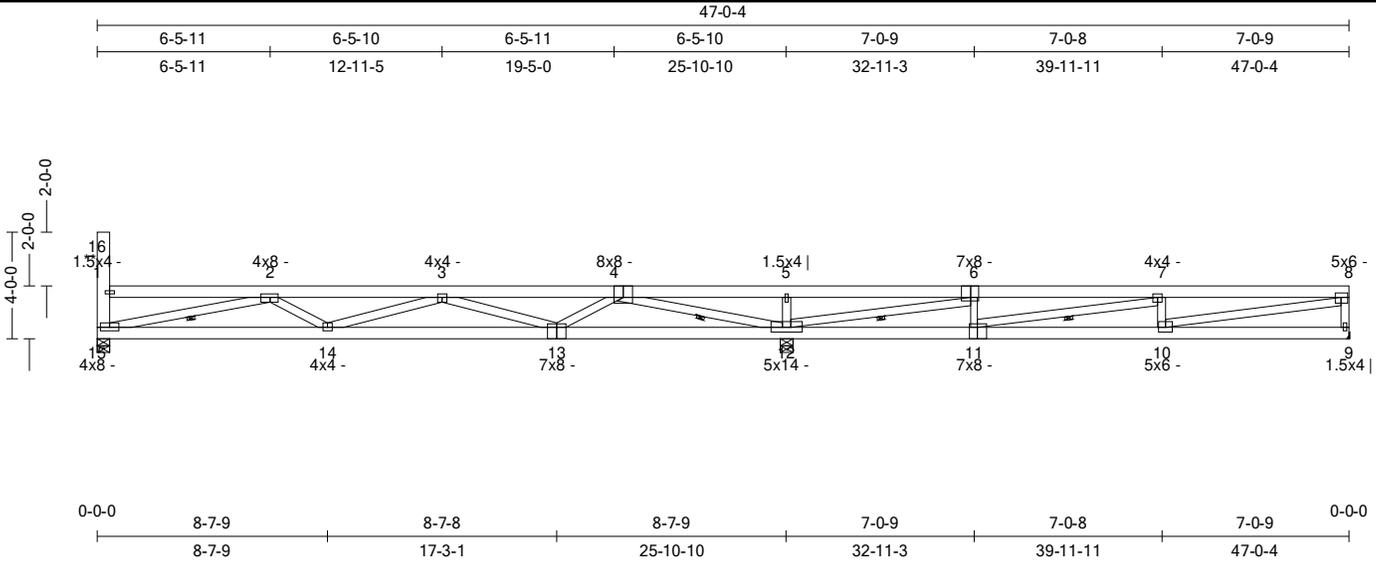
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: C1
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 1 of 2

SPAN 47-0-4	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 266 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.65 (4-5)	Vert TL: 0.43 in	L/ 710	(13-14)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.38 (13-14)	Vert LL: 0.24 in	L/ 999	(13-14)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.86 (6-12)	Horz TL: 0.06 in		9	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	1,023 lbs	96 lbs
12	1	5.5 in	2.79 in	2,616 lbs
9	1	1.5 in	---	816 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 8-10
 DFL SS 2 x 6: 16-15

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-15, 4-12, 6-12, 7-11

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 16-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.177	(-3,270 lbs)	4-5	0.652	2,710 lbs	6-7	0.188	491 lbs	(-1,482 lbs)		
	3-4	0.204	(-2,299 lbs)	5-6	0.652	2,710 lbs	7-8	0.231		(-2,287 lbs)		
BC	10-11	0.275	2,287 lbs	12-13	0.322	1,477 lbs	14-15	0.301	2,969 lbs			
	11-12	0.298	1,443 lbs	13-14	0.381	3,589 lbs						
Web	2-15	0.659	(-3,058 lbs)	4-13	0.456	1,033 lbs	6-11	0.156	353 lbs	8-9	0.094	(-761 lbs)
	2-14	0.205	464 lbs	4-12	0.825	(-3,840 lbs)	7-11	0.317	(-1,255 lbs)			
	3-14	0.135	(-339 lbs)	5-12	0.089	(-721 lbs)	7-10	0.054	(-437 lbs)			
	3-13	0.602	(-1,510 lbs)	6-12	0.865	(-3,424 lbs)	8-10	0.450	2,343 lbs			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



Mustang Truss
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Truss: **CI**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
47-0-4	0/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	266 lbs

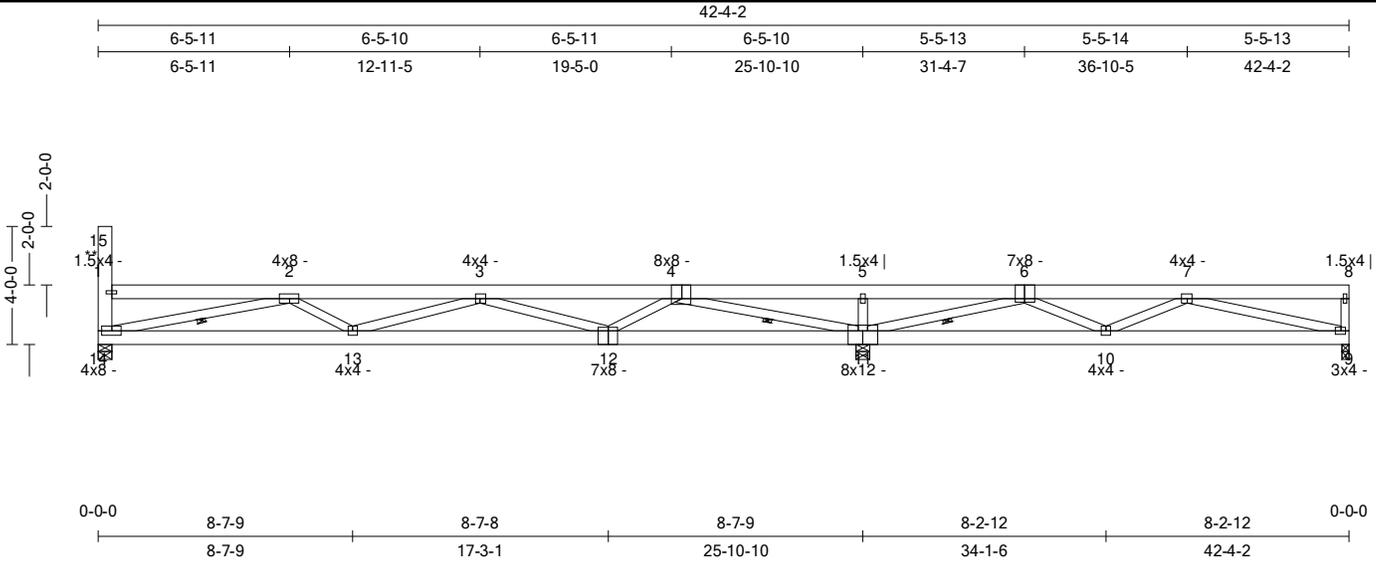
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.07 in, 2L/784 (16-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 42-4-2	PITCH 0/12	QTY 7	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 238 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.58 (4-5)	Vert TL: 0.42 in	L/ 716	(12-13)	L/ 180
TCDL : 15	TPI 1-2014	BC : 0.38 (12-13)	Vert LL: 0.23 in	L/ 999	(12-13)	L/ 240
BCLL : 0	Rep Mbr : Yes	Web : 0.84 (4-11)	Horz TL: 0.05 in		11	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.50 in	1,018 lbs	.	.	-6 lbs	-6 lbs	96 lbs
11	1	5.5 in	2.60 in	2,439 lbs	.	.	-80 lbs	-80 lbs	.
9	1	2.75 in	1.50 in	592 lbs	.	.	-10 lbs	-10 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 15-14

Bracing

TC: Sheathed or Purlins at 4-8-0, Purlin design by Others.
 BC: Sheathed or Purlins at 8-1-0, Purlin design by Others.
 Web: One Midpoint Row: 2-14, 4-11, 6-11

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 15-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.175	(-3,245 lbs)	4-5	0.583	2,612 lbs	6-7	0.111	545 lbs	(-936 lbs)
	3-4	0.210	(-2,255 lbs)	5-6	0.583	2,612 lbs				
BC	9-10	0.162	1,190 lbs	11-12	0.280	1,417 lbs	13-14	0.300	2,950 lbs	
	10-11	0.177	574 lbs	(-1,105 lbs)	12-13	0.375	3,554 lbs			
Web	2-14	0.655	(-3,039 lbs)	4-12	0.453	1,026 lbs	6-10	0.304	688 lbs	
	2-13	0.211	478 lbs	4-11	0.837	(-3,892 lbs)	7-10	0.121	(-588 lbs)	
	3-13	0.131	(-328 lbs)	5-11	0.083	(-671 lbs)	7-9	0.683	(-1,239 lbs)	
	3-12	0.580	(-1,454 lbs)	6-11	0.437	(-2,350 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **C2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
42-4-2	0/12	7	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	238 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.07 in, 2L/789 (15-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

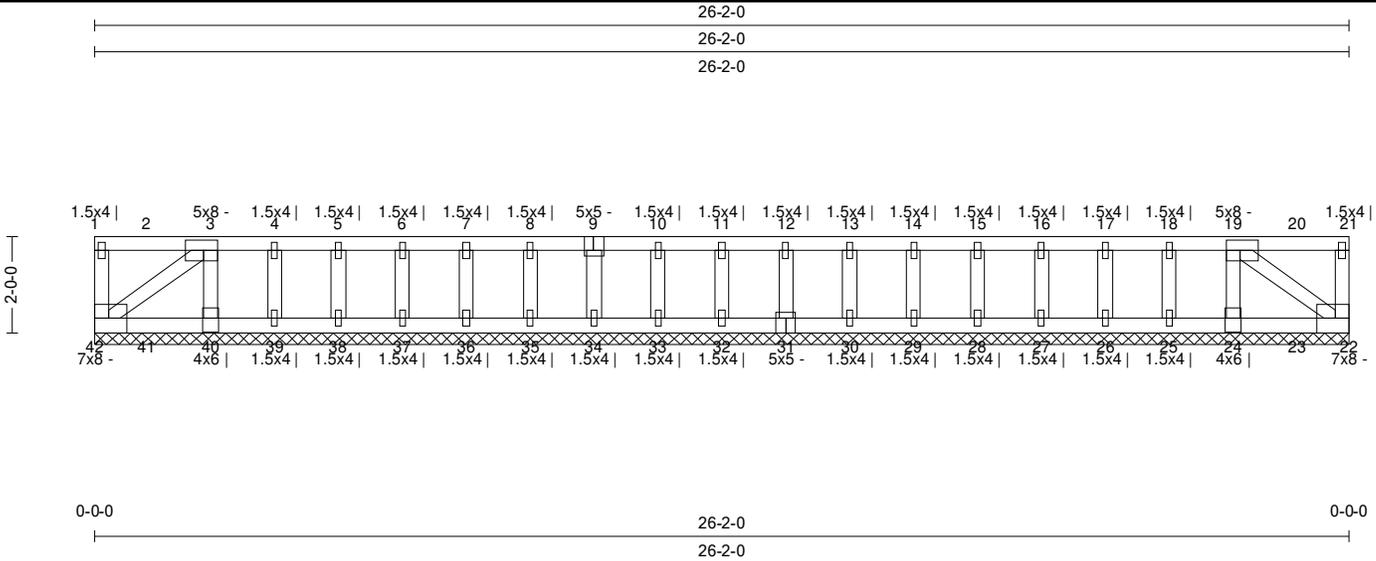
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 Eagle Metal Products



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Truss: C3D
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 1 of 2

SPAN 26-2-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 116 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.47 (18-19)	Vert TL: 0 in UP	L/999	22	L/180
TCDL: 15	TPI 1-2014	BC: 0.03 (22-24)	Vert LL: 0 in	L/999	22	L/240
BCLL: 0	Rep Mbr: No	Web: 0.90 (3-40)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	314 in	N/A	2,992 lbs	-2,888 lbs	.	-4 lbs	-2,888 lbs	-3,932 lbs
1	314 in	N/A	2,981 lbs	-2,810 lbs	.	-23 lbs	-2,810 lbs	.
1	314 in	N/A	119 lbs	-25 lbs	.	-8 lbs	-25 lbs	.
1	314 in	N/A	127 lbs	.	.	-7 lbs	-7 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	126 lbs	.	.	-8 lbs	-8 lbs	.
1	314 in	N/A	127 lbs	.	.	-7 lbs	-7 lbs	.
1	314 in	N/A	119 lbs	-25 lbs	.	-8 lbs	-25 lbs	.
1	314 in	N/A	2,981 lbs	-2,810 lbs	.	-23 lbs	-2,810 lbs	.
1	314 in	N/A	2,992 lbs	-2,888 lbs	.	-4 lbs	-2,888 lbs	3,932 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 3-42, 19-22

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to a 7,850 lbs (300 plf) drag load distributed along the TC rake from each direction.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **C3D**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-2-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	116 lbs

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-3	0.167	695 lbs	(-703 lbs)	7-8	0.228	1,593 lbs	(-1,607 lbs)	12-13	0.118	793 lbs	(-807 lbs)	17-18	0.451	2,793 lbs	(-2,807 lbs)
	3-4	0.472	3,193 lbs	(-3,207 lbs)	8-9	0.173	1,193 lbs	(-1,207 lbs)	13-14	0.173	1,193 lbs	(-1,207 lbs)	18-19	0.472	3,193 lbs	(-3,207 lbs)
	4-5	0.451	2,793 lbs	(-2,807 lbs)	9-10	0.118	793 lbs	(-807 lbs)	14-15	0.228	1,593 lbs	(-1,607 lbs)	19-21	0.167	695 lbs	(-703 lbs)
	5-6	0.332	2,393 lbs	(-2,407 lbs)	10-11	0.060	393 lbs	(-407 lbs)	15-16	0.283	1,993 lbs	(-2,007 lbs)				
	6-7	0.283	1,993 lbs	(-2,007 lbs)	11-12	0.060	393 lbs	(-407 lbs)	16-17	0.332	2,393 lbs	(-2,407 lbs)				
BC																
Web	3-42	0.673	4,876 lbs	(-4,883 lbs)												
	3-40	0.900	2,836 lbs	(-2,954 lbs)												
	19-24	0.900	2,836 lbs	(-2,954 lbs)												
	19-22	0.673	4,876 lbs	(-4,883 lbs)												

Notes

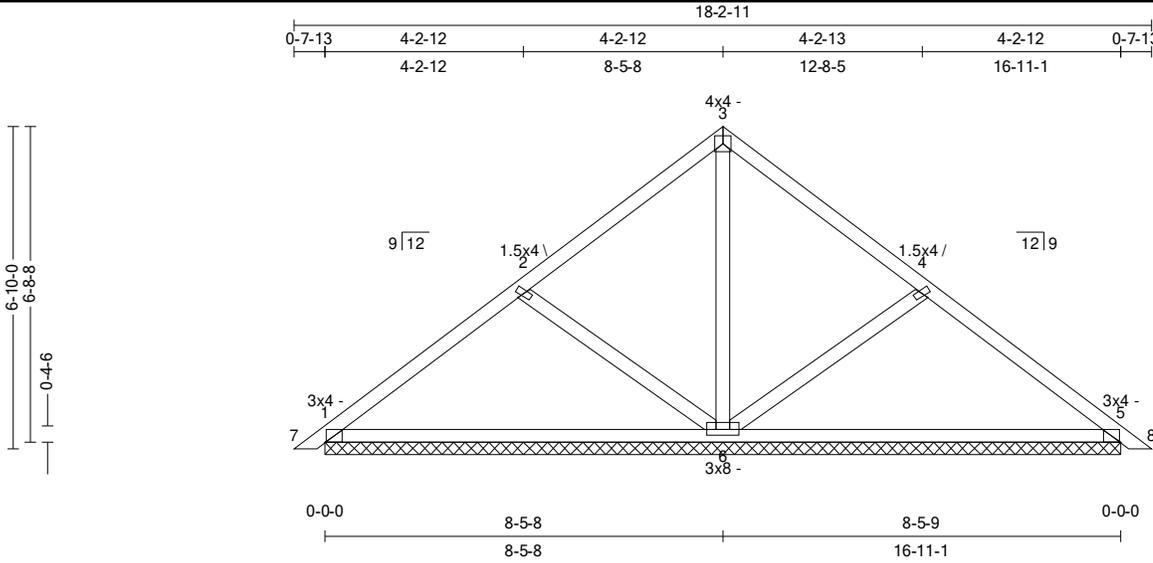
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) Provide adequate drainage to prevent ponding.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 22, 24, 25, 39, 40, 42 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



SPAN 16-11-1	PITCH 9/12	QTY 18	OHL 0-7-13	OHR 0-7-13	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 71 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.26 (2-3)	Vert TL: 0.08 in	L/999	(5-6)	L/180
TCDL: 15	TPI 1-2014	BC: 0.40 (5-6)	Vert LL: 0.05 in	L/999	(5-6)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.23 (3-6)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	203.042 in	N/A	746 lbs	.	.	-57 lbs	-57 lbs	163 lbs
1	203.042 in	N/A	0 lbs	-543 lbs	-117 lbs	-55 lbs	-543 lbs	511 lbs
1	203.042 in	N/A	0 lbs	-543 lbs	-117 lbs	-55 lbs	-543 lbs	-511 lbs
1	203.042 in	N/A	843 lbs	616 lbs
1	203.042 in	N/A	843 lbs	-616 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7		
TC	1-2	0.246	710 lbs	(-430 lbs)					
	4-5	0.246	710 lbs	(-430 lbs)					
BC									
Web	2-6	0.159	(-323 lbs)	3-6	0.232	(-327 lbs)	4-6	0.159	(-323 lbs)

Notes

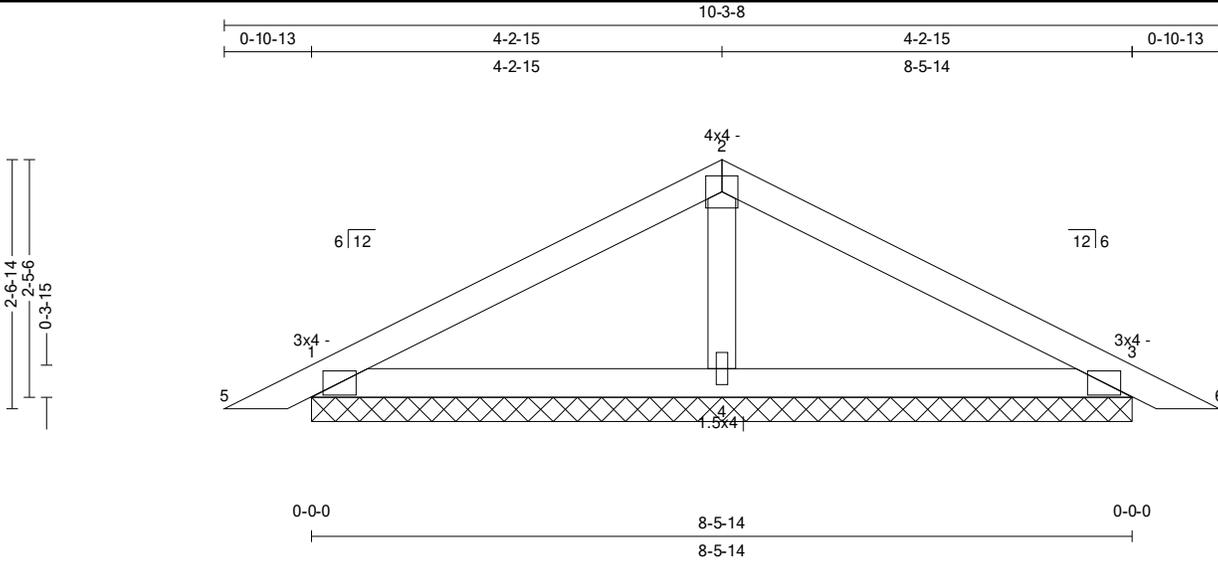
- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 5, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



Mustang Truss
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Truss: CP2
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 1 of 1

SPAN 8-5-14	PITCH 6/12	QTY 32	OHL 0-10-13	OHR 0-10-13	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 28 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.24 (2-3)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.08 (4-1)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.03 (2-4)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		488 lbs	173 plf	-89 lbs	-30 lbs	-91 lbs	-91 lbs	-418 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

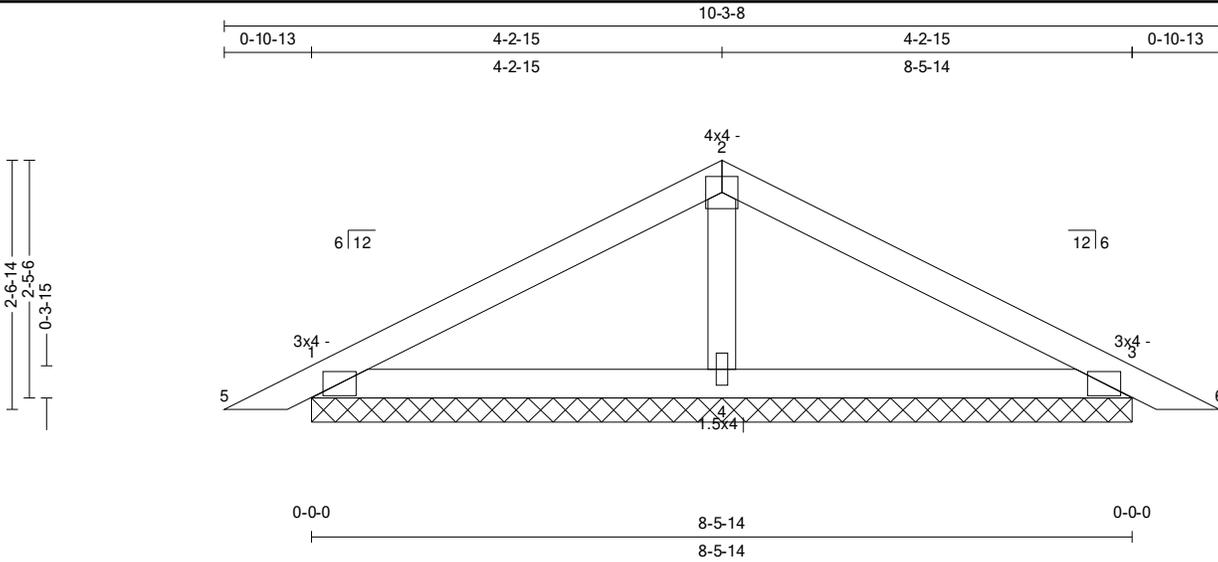
TC	1-2	0.242	392 lbs	2-3	0.242	392 lbs
BC						
Web						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 48" OC, U.N.O.
- 4) Attach gable webs with 3x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 1 may need to be considered.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 8-5-14	PITCH 6/12	QTY 1	OHL 0-10-13	OHR 0-10-13	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 28 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.28 (2-3)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.09 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.05 (2-4)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		488 lbs	173 plf	-89 lbs	-30 lbs	-91 lbs	-91 lbs	-418 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

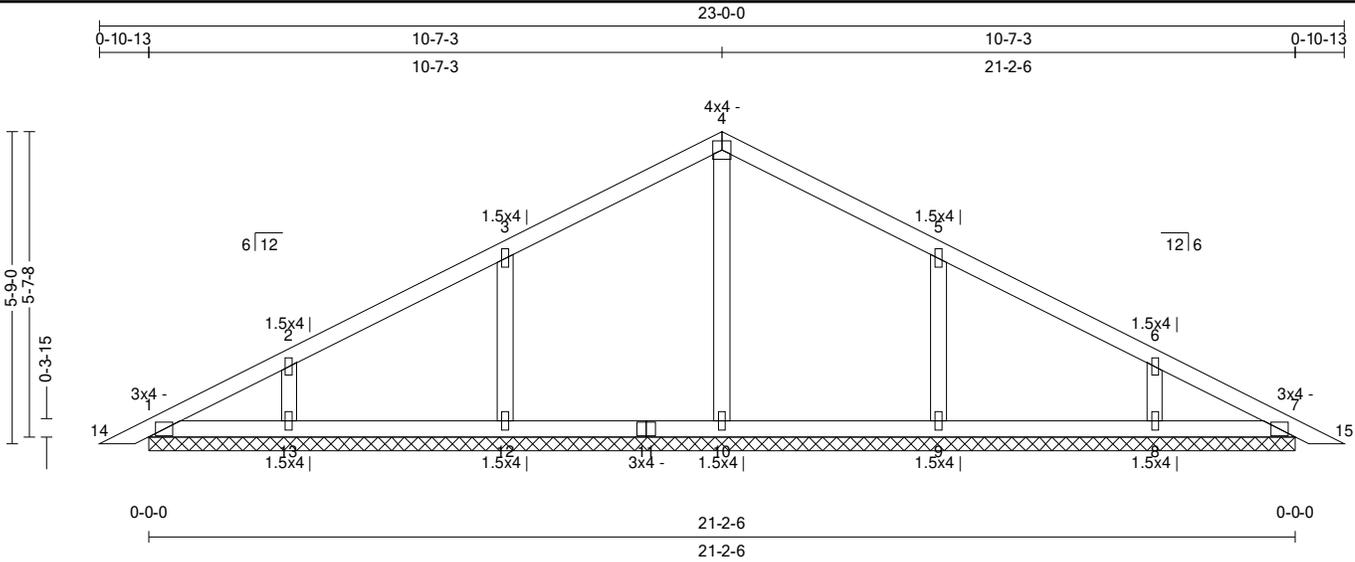
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC 1-2	0.278 392 lbs
TC 2-3	0.278 392 lbs
BC	
Web	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 21-2-6	PITCH 6/12	QTY 2	OHL 0-10-13	OHR 0-10-13	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 78 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.27 (2-3)	Vert TL: 0.01 in	L/999	(12-13)	L/180
TCDL : 15	TPI 1-2014	BC : 0.11 (9-10)	Vert LL: 0.01 in	L/999	(12-13)	L/240
BCLL : 0	Rep Mbr : No	Web : 0.13 (4-10)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		448 lbs	110 plf	-43 lbs		-34 lbs	-43 lbs	74 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

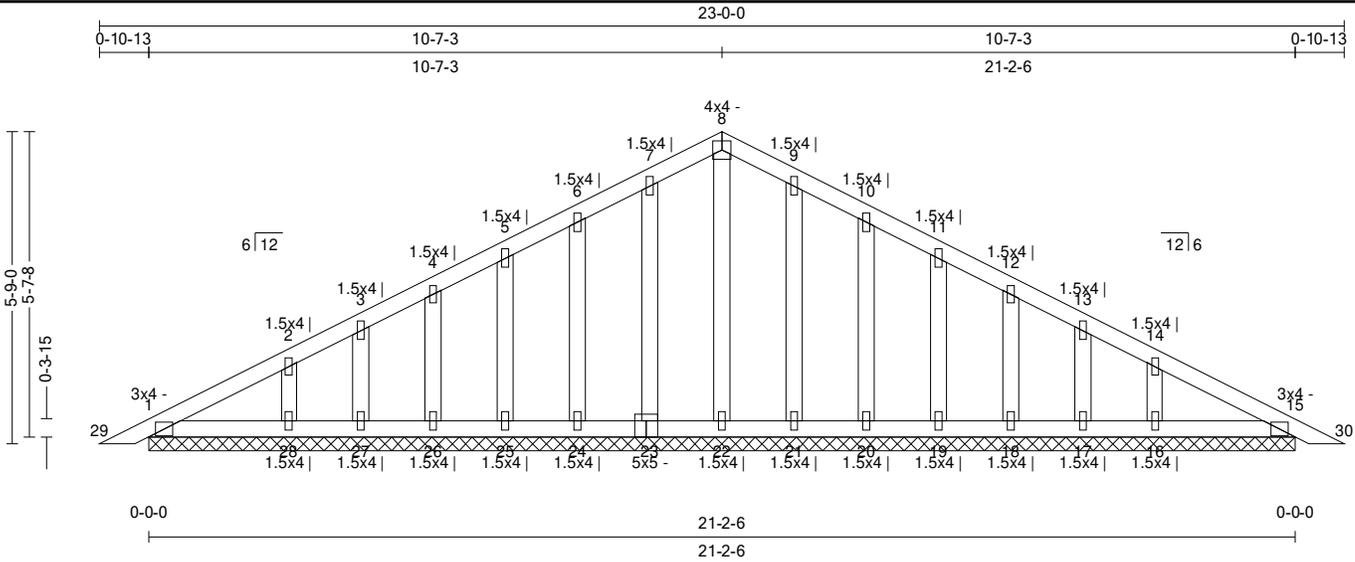
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web	3-12	0.102	(-397 lbs)
	5-9	0.102	(-397 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 48" OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 7 may need to be considered.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 21-2-6	PITCH 6/12	QTY 1	OHL 0-10-13	OHR 0-10-13	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 111 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.09 (1-2)	Vert TL: 0 in	L / 999	15	L / 180
TCDL : 15	TPI 1-2014	BC : 0.02 (15-16)	Vert LL: 0 in	L / 999	15	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.06 (7-23)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		166 lbs	114 plf			-19 lbs	-19 lbs	-94 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

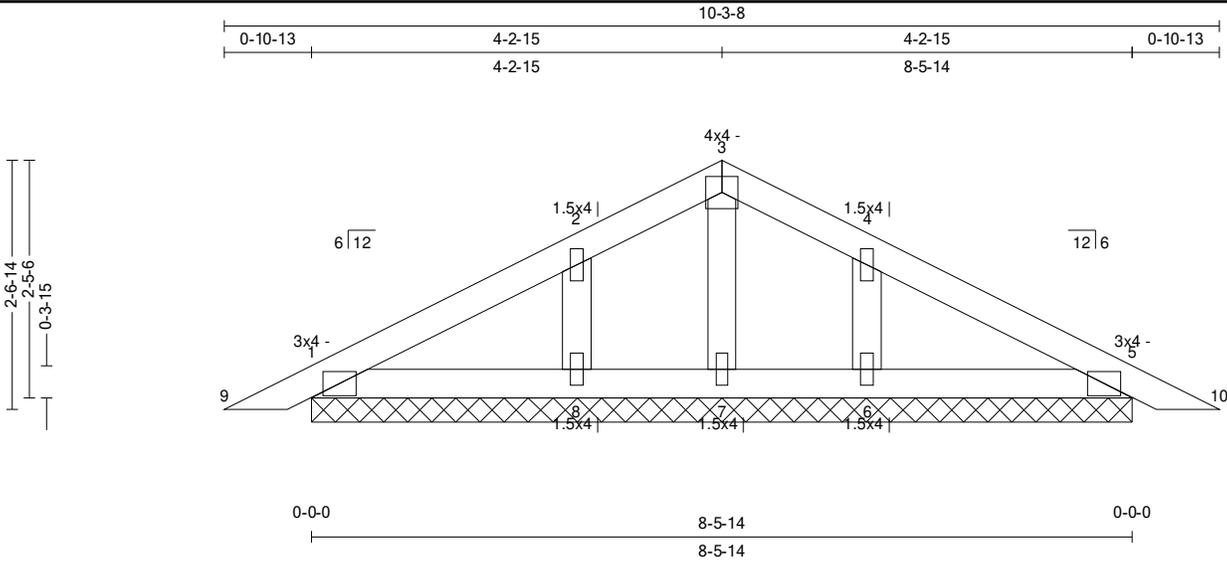
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16" OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 8-5-14	PITCH 6/12	QTY 1	OHL 0-10-13	OHR 0-10-13	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 32 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.11 (1-2)	Vert TL: 0 in	L / 999	(5-6)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.03 (8-1)	Vert LL: 0 in	L / 999	5	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.04 (4-6)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		227 lbs	147 plf		-4 lbs	-58 lbs	-58 lbs	-131 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

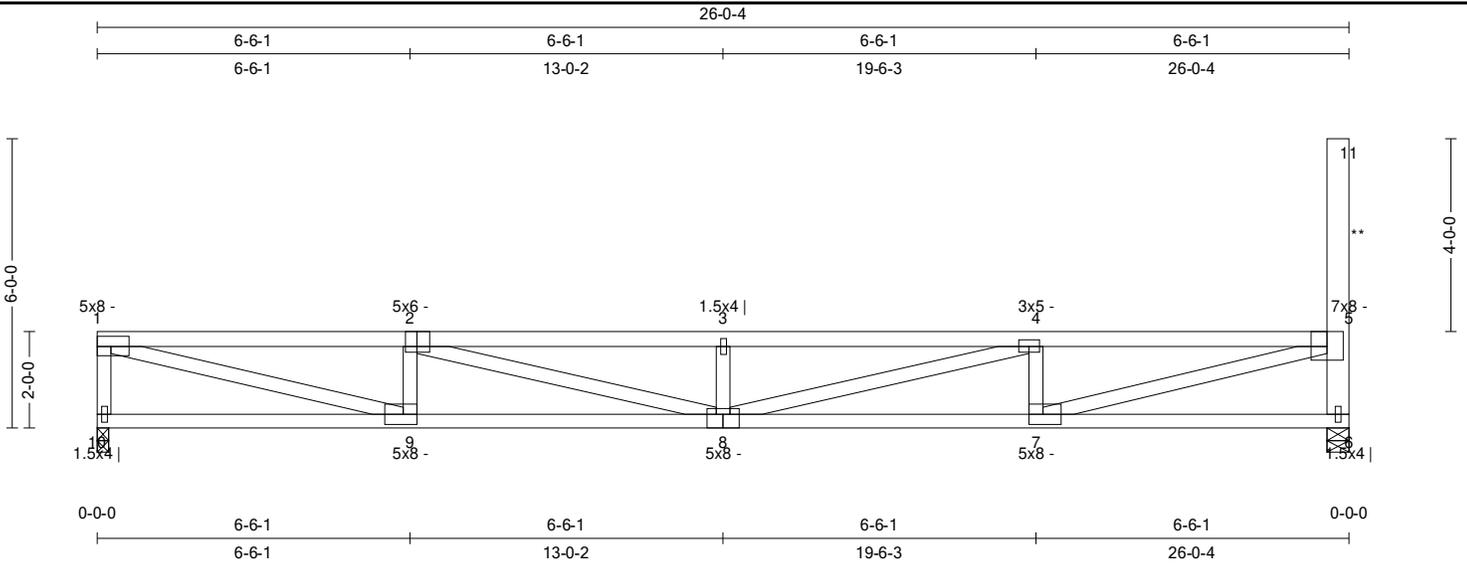
TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 18" OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.



SPAN 26-0-4	PITCH 0/12	QTY 13	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 120 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.95 (1-2)	Vert TL: 0.73 in	L/414	8	L/180
TCDL: 15	TPI 1-2014	BC: 0.77 (8-9)	Vert LL: 0.4 in	L/760	8	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.69 (1-9)	Horz TL: 0.07 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	2.75 in	1.50 in	1,202 lbs	.	.	-112 lbs	-112 lbs	-197 lbs
6	1	5.5 in	1.50 in	1,192 lbs	.	.	-43 lbs	-43 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 1-9, 5-7
 DFL SS 2 x 6: 11-6

Bracing

TC: Sheathed
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 11-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.953	349 lbs	(-3,486 lbs)	2-3	0.718	523 lbs	(-4,566 lbs)	3-4	0.749	523 lbs	(-4,566 lbs)	4-5	0.804	575 lbs	(-3,480 lbs)
BC	7-8	0.755	3,480 lbs	(-339 lbs)	8-9	0.770	3,559 lbs									
Web	1-10	0.149		(-1,163 lbs)	2-8	0.512	1,160 lbs		4-7	0.111		(-867 lbs)				
	1-9	0.693	3,609 lbs	(-341 lbs)	3-8	0.062		(-483 lbs)	5-7	0.693	3,607 lbs					
	2-9	0.110		(-859 lbs)	4-8	0.515	1,165 lbs		5-6	0.219		(-1,152 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) A creep factor of 2.00 has been applied for this truss analysis.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
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Truss: **DI**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-0-4	0/12	13	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	120 lbs

7) Listed wind uplift reactions based on MWFRS & C&C loading.
 8) Parapet TL: 0.26 in, 2L/385 (11-5), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

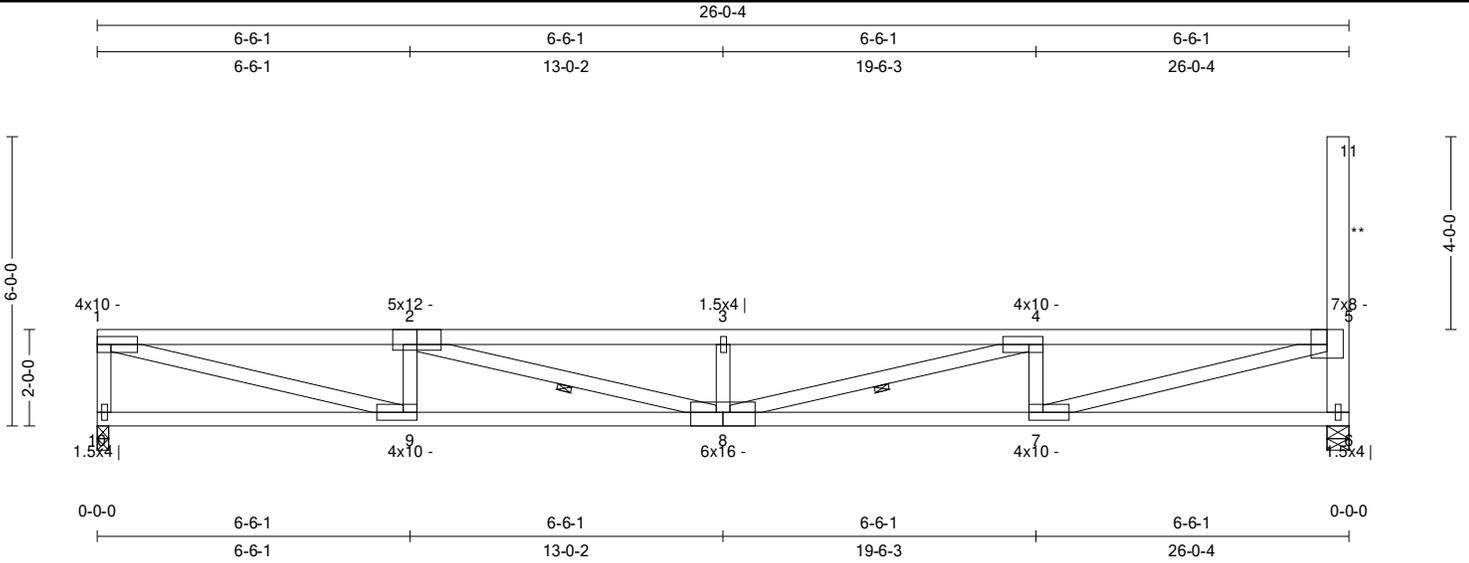
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Truss: D1D
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:14
 Page: 1 of 2

SPAN 26-0-4	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 123 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.84 (1-2)	Vert TL: 0.67 in	L/ 452	8	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.86 (9-10)	Vert LL: 0.37 in	L/ 830	8	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.82 (4-8)	Horz TL: 0.15 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	2.75 in	1.50 in	1,202 lbs	.	.	-112 lbs	-112 lbs	7,806 lbs
6	1	5.5 in	1.50 in	1,192 lbs	.	.	-43 lbs	-43 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #1B 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 1-9, 5-7
 DFL SS 2 x 6: 11-6

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 2-1-0, Purlin design by Others.
 Web: One Midpoint Row: 2-8, 4-8

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 7,806 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 11-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	Force (lbs)	Force (lbs)	ID	Force (lbs)	Force (lbs)
TC	1-2	0.841	349 lbs (-3,520 lbs)	2-3	0.628	523 lbs (-4,564 lbs)
BC	7-8	0.610	3,478 lbs (-338 lbs)	8-9	0.808	7,407 lbs (-4,168 lbs)
Web	1-10	0.149	(-1,163 lbs)	2-8	0.772	2,431 lbs (-1,475 lbs)
	1-9	0.693	(-398 lbs)	3-8	0.062	(-483 lbs)
	2-9	0.110	(-865 lbs)	4-8	0.823	2,591 lbs (-1,566 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Mustang Truss

2525 Hyacinth Street NE

Salem, OR 97301

Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **DID**

Job: **2501272**

Designer: Anthony

Date: 01/23/26 09:23:15

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-0-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	123 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.23 in, 2L/433 (11-5), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

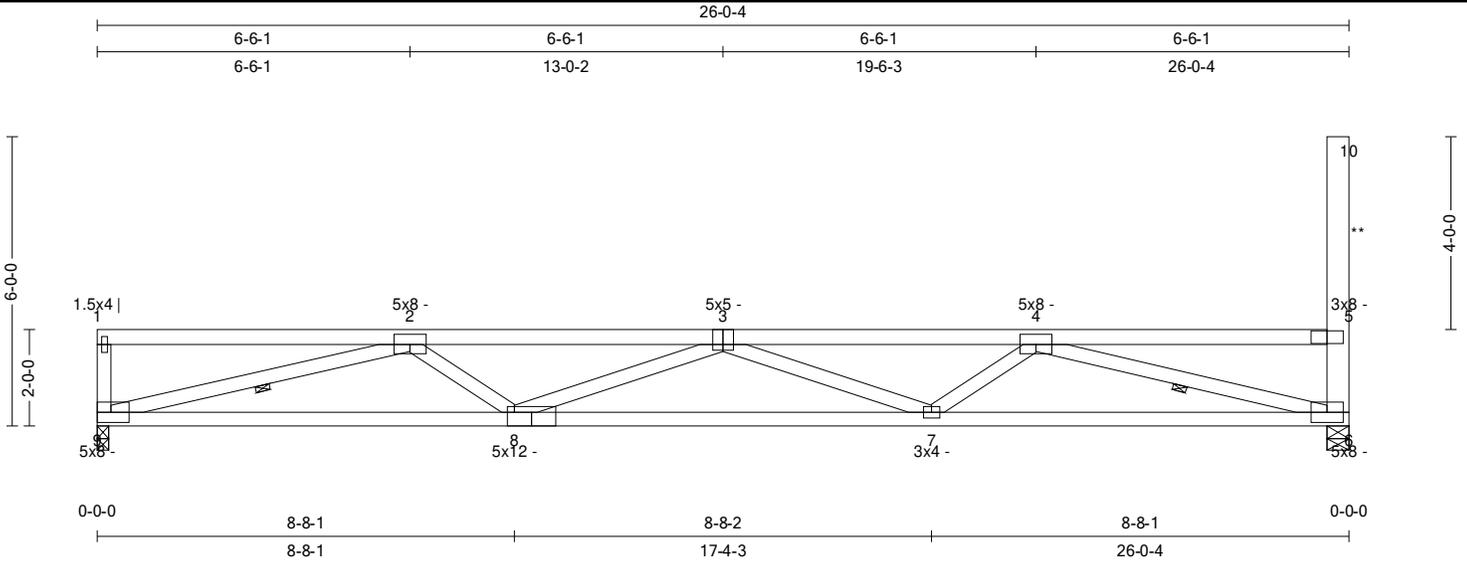
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Truss: D2
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 1 of 2

SPAN 26-0-4	PITCH 0/12	QTY 21	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 118 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.70 (1-2)	Vert TL: 0.72 in	L/421	(7-8)	L/180
TCDL: 15	TPI 1-2014	BC: 0.98 (7-8)	Vert LL: 0.39 in	L/774	(7-8)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (2-9)	Horz TL: 0.14 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	2.75 in	1.50 in	1,202 lbs	.	.	-112 lbs	-112 lbs	-197 lbs
6	1	5.5 in	1.50 in	1,192 lbs	.	.	-43 lbs	-43 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-9, 4-6

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.639	383 lbs	(-3,979 lbs)	3-4	0.628	526 lbs	(-3,893 lbs)	4-5	0.686	465 lbs
BC	6-7	0.825	3,433 lbs	(-364 lbs)	7-8	0.979	4,552 lbs	(-310 lbs)	8-9	0.870	3,458 lbs
Web	2-9	0.820	374 lbs	(-3,580 lbs)	3-7	0.306		(-734 lbs)			
	2-8	0.304	688 lbs		4-7	0.281	637 lbs				
	3-8	0.259		(-682 lbs)	4-6	0.785		(-3,558 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Mustang Truss
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Truss: **D2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-0-4	0/12	21	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs

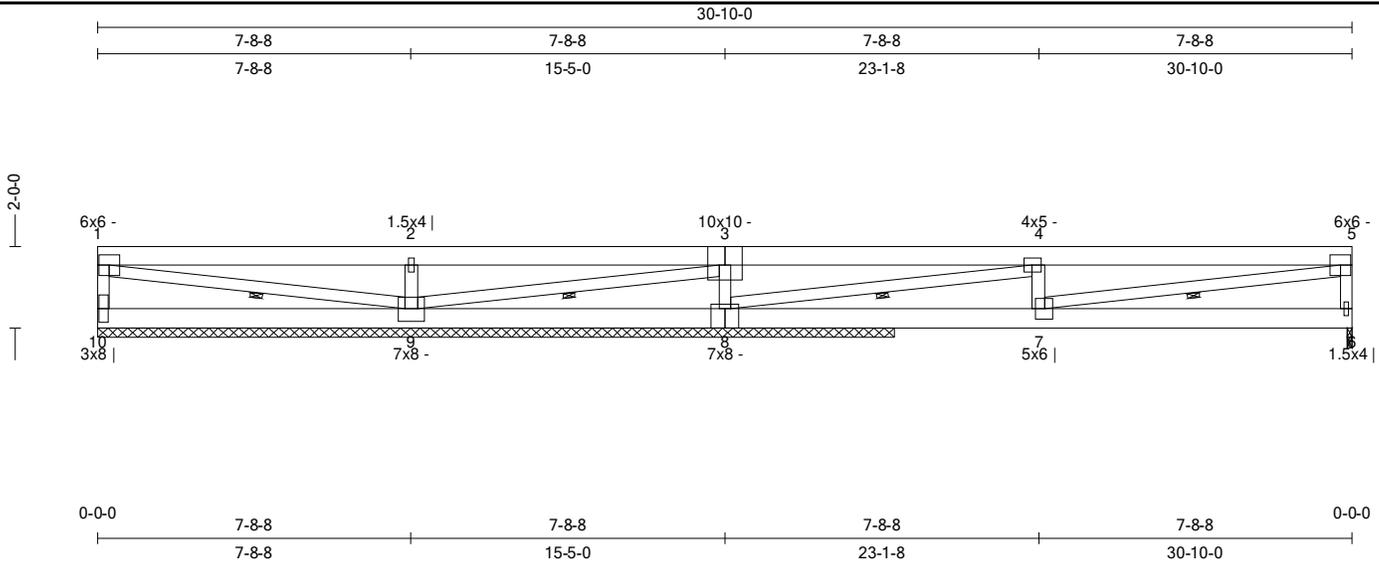
9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.26 in, 2L/385 (10-5), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 30-10-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 173 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.38 (3-4)	Vert TL: 0.15 in	L/ 875	(6-7)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.26 (7-8)	Vert LL: 0.08 in	L/ 999	(6-7)	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.92 (3-9)	Horz TL: 0.01 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	1.50 in	684 lbs	-106 lbs	.	-34 lbs	-106 lbs	.
8	1	234.999 in	N/A	1,211 lbs	.	.	-59 lbs	-59 lbs	4,121 lbs
9	1	234.999 in	N/A	789 lbs	.	.	-38 lbs	-38 lbs	-5,470 lbs
10	1	234.999 in	N/A	644 lbs	-400 lbs	.	-15 lbs	-400 lbs	18 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 5-4-0, Purlin design by Others.
 BC: Sheathed or Purlins at 7-7-0, Purlin design by Others.
 Web: One Midpoint Row: 1-9, 3-9, 4-8, 5-7

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 9,250 lbs (300 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has not been designed for the effects of unbalanced snow loads.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.359	2,590 lbs	(-2,461 lbs)	2-3	0.360	2,120 lbs	(-1,992 lbs)	3-4	0.379	1,808 lbs	(-1,467 lbs)	4-5	0.317	1,185 lbs	(-2,493 lbs)
BC	7-8	0.259	2,537 lbs	(-1,229 lbs)												
Web	1-10	0.140	440 lbs	(-605 lbs)	3-9	0.918	2,893 lbs	(-2,677 lbs)	4-7	0.107	336 lbs	(-461 lbs)				
	1-9	0.812	2,556 lbs	(-2,688 lbs)	3-8	0.092		(-830 lbs)	5-7	0.822	2,589 lbs	(-1,254 lbs)				
	2-9	0.087		(-703 lbs)	4-8	0.828	1,267 lbs	(-2,949 lbs)	5-6	0.071		(-629 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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Truss: **D2D**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
30-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	173 lbs

- 8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 6, 10 may need to be considered.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

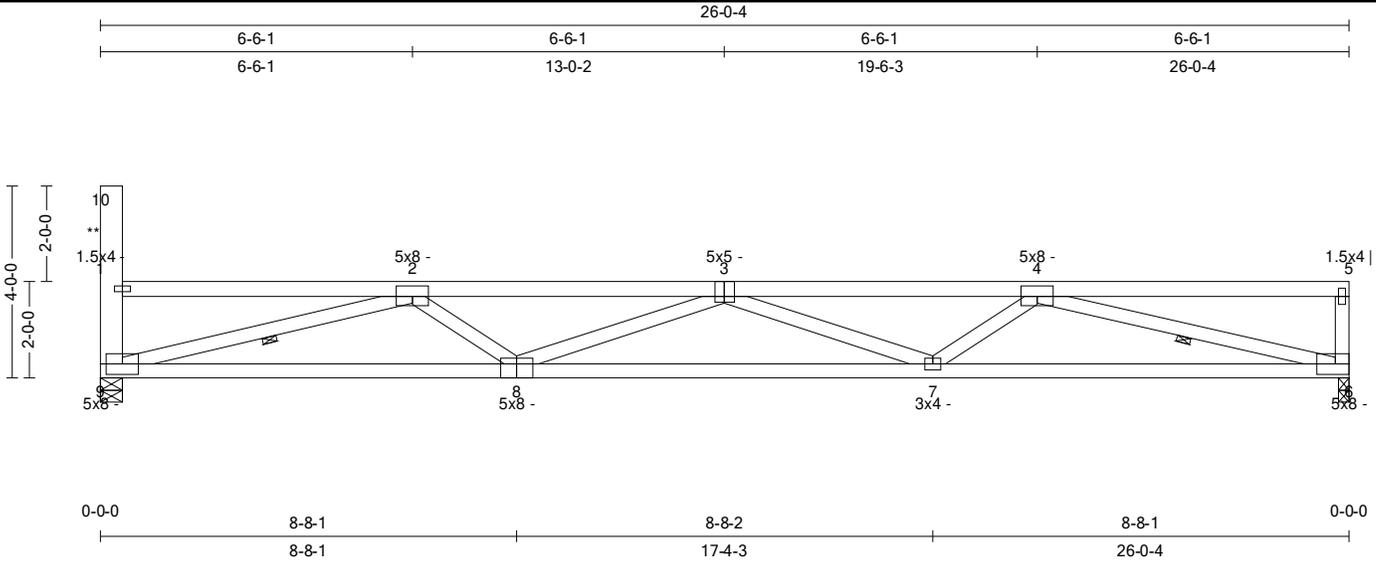
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Mustang Truss
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Truss: D3
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 1 of 2

SPAN 26-0-4	PITCH 0/12	QTY 14	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 113 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.70 (4-5)	Vert TL: 0.74 in	L/ 413	(7-8)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.96 (7-8)	Vert LL: 0.39 in	L/ 772	(7-8)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (4-6)	Horz TL: 0.14 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,192 lbs	.	.	-62 lbs	-62 lbs	96 lbs
6	1	2.75 in	1.50 in	1,202 lbs	.	.	-93 lbs	-93 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 10-9

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-9, 4-6

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 10-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.627	334 lbs	(-3,896 lbs)	3-4	0.636	(-3,917 lbs)
BC	6-7	0.837	3,466 lbs		7-8	0.965	4,547 lbs (-367 lbs)
Web	2-9	0.785		(-3,559 lbs)	3-7	0.309	(-740 lbs)
	2-8	0.283	640 lbs		4-7	0.280	634 lbs
	3-8	0.303		(-725 lbs)	4-6	0.822	(-3,589 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
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 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **D3**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-0-4	0/12	14	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	113 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.13 in, 2L/384 (10-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

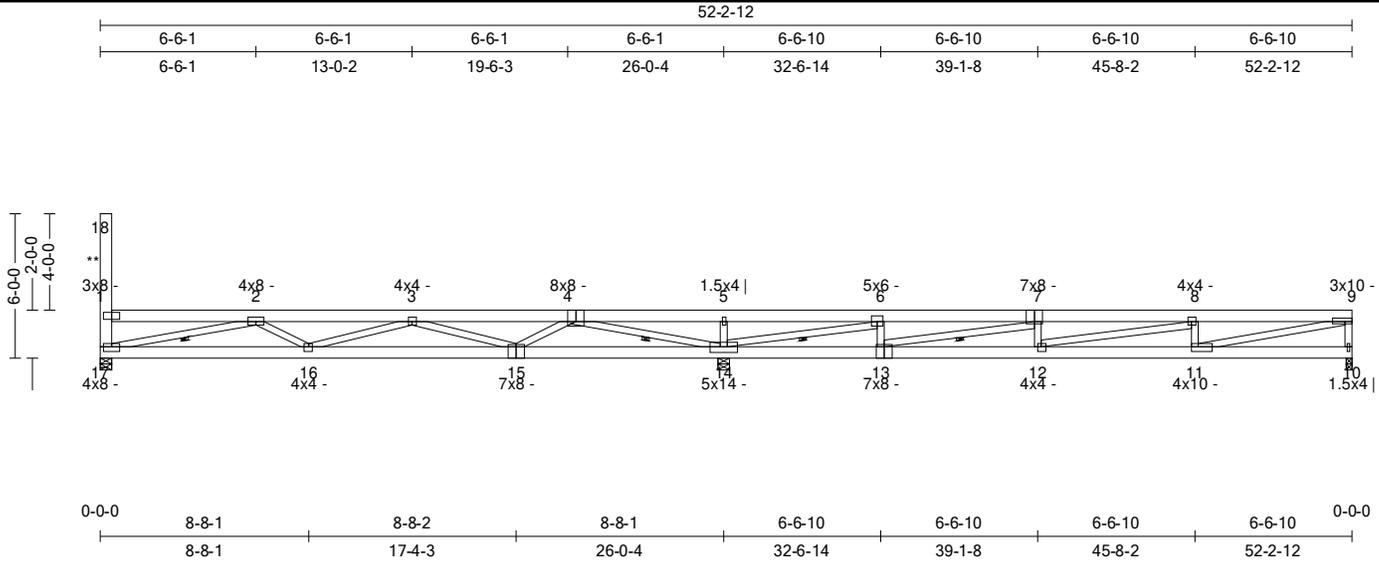
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Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E1
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 1 of 2

SPAN 52-2-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 299 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.84 (5-6)	Vert TL: 0.46 in	L/ 666	12	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.41 (15-16)	Vert LL: 0.28 in	L/ 999	12	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.95 (6-14)	Horz TL: 0.07 in		10	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.50 in	1,008 lbs	197 lbs
14	1	5.5 in	3.10 in	2,909 lbs
10	1	2.75 in	1.50 in	1,027 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 9-11
 DFL SS 2 x 6: 18-17

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-17, 4-14, 6-14, 7-13

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 18-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.234	500 lbs	3-4	0.230	(-2,108 lbs)	5-6	0.843	3,517 lbs	7-8	0.239	(-3,510 lbs)
	2-3	0.191	(-3,202 lbs)	4-5	0.843	3,517 lbs	6-7	0.199	591 lbs	8-9	0.256	(-3,050 lbs)
BC	11-12	0.362	3,050 lbs	13-14	0.379	1,449 lbs	15-16	0.415	3,469 lbs			
	12-13	0.410	3,512 lbs	14-15	0.358	1,249 lbs	16-17	0.331	2,924 lbs			
Web	2-17	0.654	(-3,011 lbs)	5-14	0.091	(-736 lbs)	8-11	0.083	(-674 lbs)			
	2-16	0.182	411 lbs	6-14	0.948	(-4,203 lbs)	9-11	0.603	3,138 lbs			
	3-15	0.676	(-1,681 lbs)	6-13	0.247	560 lbs	9-10	0.120	(-971 lbs)			
	4-15	0.498	1,128 lbs	7-13	0.542	(-2,400 lbs)						
	4-14	0.862	(-3,983 lbs)	8-12	0.209	472 lbs						

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

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Truss: **E1**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
52-2-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	299 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.14 in, 2L/746 (18-1), Allowable 2L/120.

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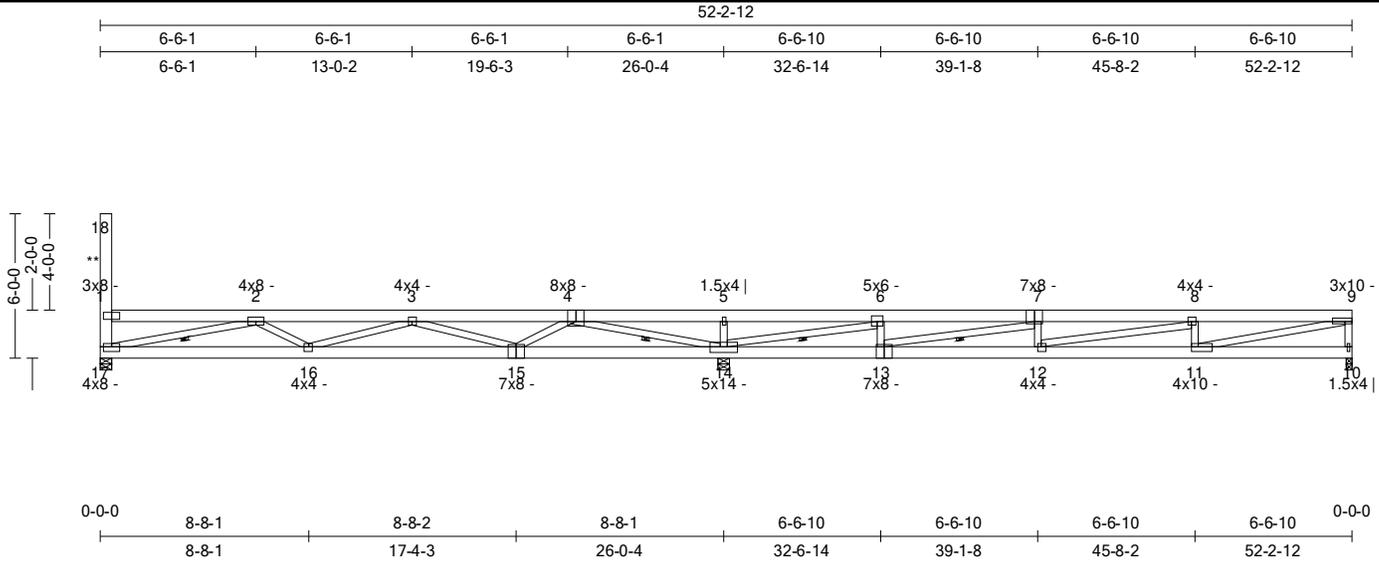
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Mustang Truss
 2525 Hyacinth Street NE
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Truss: E2
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 1 of 2

SPAN 52-2-12	PITCH 0/12	QTY 6	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 299 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.74 (5-6)	Vert TL: 0.46 in	L/ 666	12	L/ 180
TCDL : 15	TPI 1-2014	BC : 0.37 (15-16)	Vert LL: 0.28 in	L/ 999	12	L/ 240
BCLL : 0	Rep Mbr : Yes	Web : 0.95 (6-14)	Horz TL: 0.07 in		10	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.50 in	1,008 lbs	197 lbs
14	1	5.5 in	3.10 in	2,909 lbs
10	1	2.75 in	1.50 in	1,027 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 9-11
 DFL SS 2 x 6: 18-17

Bracing

TC: Sheathed or Purlins at 4-5-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-6-0, Purlin design by Others.
 Web: One Midpoint Row: 2-17, 4-14, 6-14, 7-13

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 18-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.203	500 lbs	3-4	0.199	(-2,108 lbs)	5-6	0.744	3,517 lbs	7-8	0.205	(-3,510 lbs)
	2-3	0.173	(-3,202 lbs)	4-5	0.744	3,517 lbs	6-7	0.172	591 lbs	8-9	0.220	(-3,050 lbs)
BC	11-12	0.325	3,050 lbs	13-14	0.335	1,449 lbs	15-16	0.372	3,469 lbs			
	12-13	0.367	3,512 lbs	14-15	0.313	1,249 lbs	16-17	0.297	2,924 lbs			
Web	2-17	0.654	(-3,011 lbs)	5-14	0.091	(-736 lbs)	8-11	0.083	(-674 lbs)			
	2-16	0.182	411 lbs	6-14	0.948	(-4,203 lbs)	9-11	0.603	3,138 lbs			
	3-15	0.676	(-1,681 lbs)	6-13	0.247	560 lbs	9-10	0.120	(-971 lbs)			
	4-15	0.498	1,128 lbs	7-13	0.542	(-2,400 lbs)						
	4-14	0.862	(-3,983 lbs)	8-12	0.209	472 lbs						

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

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Truss: **E2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
52-2-12	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	299 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.14 in, 2L/746 (18-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

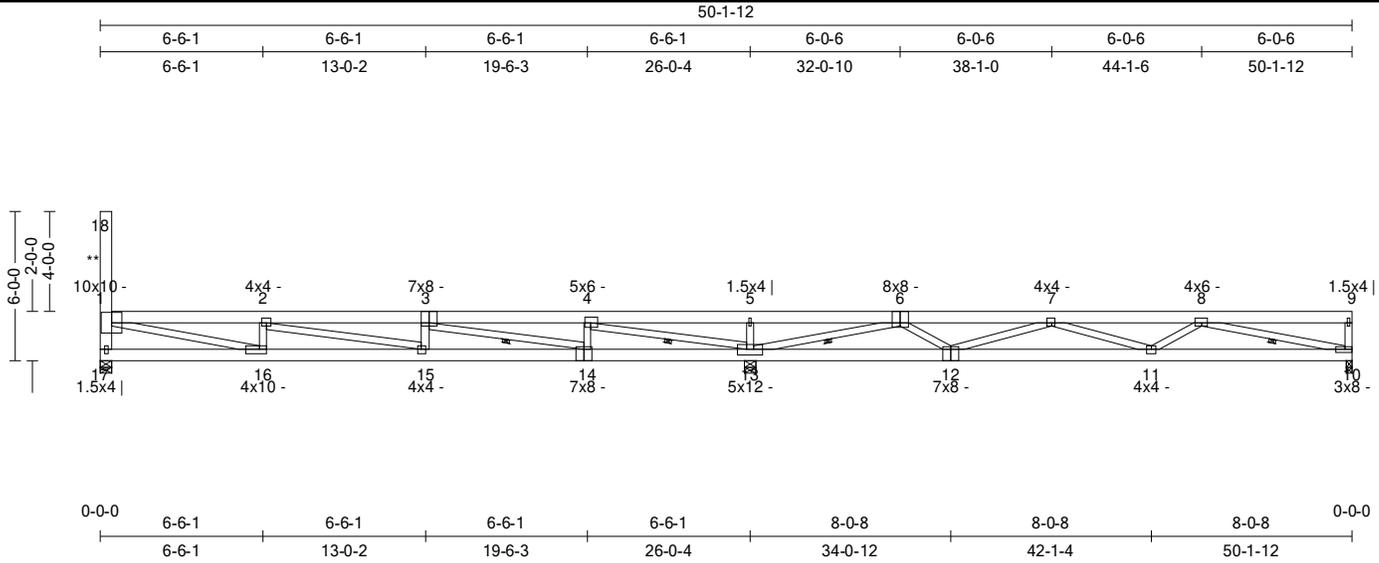
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Truss: E3
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 1 of 2

SPAN 50-1-12	PITCH 0/12	QTY 21	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 289 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.70 (5-6)	Vert TL: 0.45 in	L/ 674	15	L/ 180
TCDL : 15	TPI 1-2014	BC : 0.37 (14-15)	Vert LL: 0.27 in	L/ 999	15	L/ 240
BCLL : 0	Rep Mbr : Yes	Web : 0.91 (4-13)	Horz TL: 0.06 in		10	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.50 in	1,018 lbs	197 lbs
13	1	5.5 in	2.98 in	2,789 lbs
10	1	2.75 in	1.50 in	940 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 1-16
 DFL SS 2 x 6: 18-17

Bracing

TC: Sheathed or Purlins at 4-5-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-4-0, Purlin design by Others.
 Web: One Midpoint Row: 3-14, 4-13, 6-13, 8-10

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 18-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.209	(3,013 lbs)	3-4	0.171	337 lbs	(-1,562 lbs)	5-6	0.698	3,177 lbs	7-8	0.147	(-2,744 lbs)
	2-3	0.204	(-3,512 lbs)	4-5	0.698	3,177 lbs		6-7	0.154	327 lbs	(-1,777 lbs)		
BC	10-11	0.256	2,518 lbs	12-13	0.295	1,148 lbs	(-788 lbs)	14-15	0.367	3,515 lbs	16-17	0.086	(-466 lbs)
	11-12	0.321	2,938 lbs	13-14	0.334	1,517 lbs	(-369 lbs)	15-16	0.321	3,013 lbs			
Web	1-7	0.230	(-961 lbs)	4-14	0.236	535 lbs		7-12	0.528		(-1,491 lbs)		
	1-6	0.596	3,102 lbs	4-13	0.909		(-4,072 lbs)	8-11	0.152	345 lbs			
	2-6	0.084	(-680 lbs)	5-13	0.090		(-728 lbs)	8-10	0.528		(-2,603 lbs)		
	2-15	0.227	513 lbs	6-13	0.744		(-3,553 lbs)						
	3-14	0.508	(-2,274 lbs)	6-12	0.400	906 lbs							

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

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Truss: **E3**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
50-1-12	0/12	21	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	289 lbs

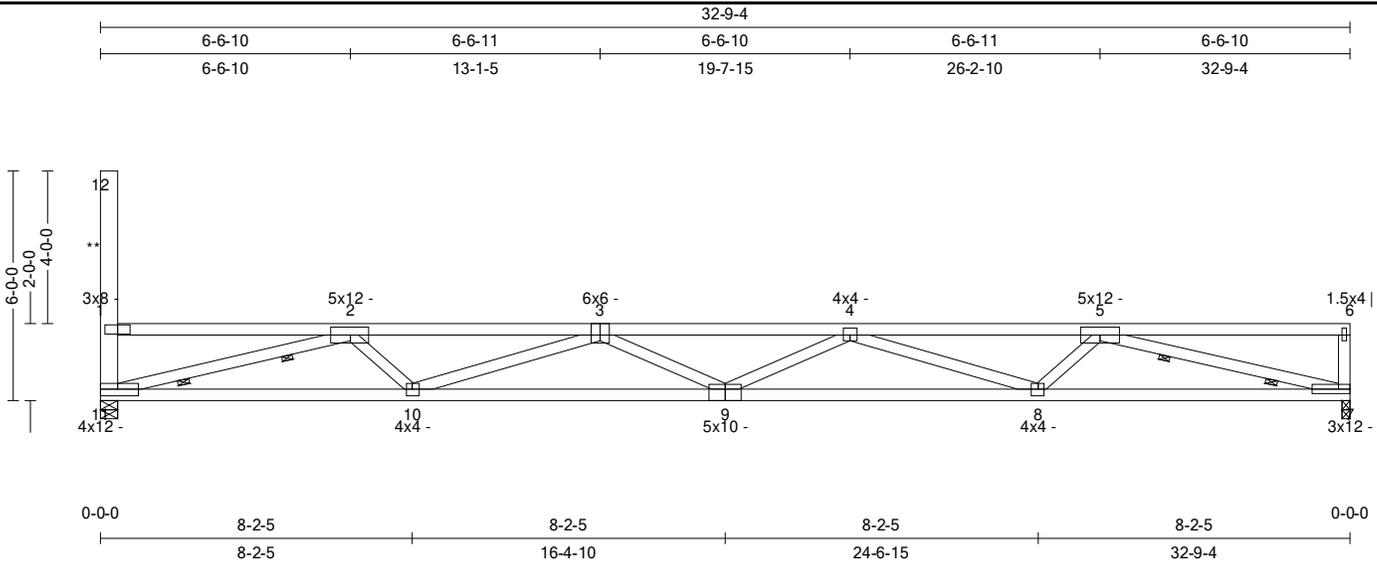
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.14 in, 2L/716 (18-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 32-9-4	PITCH 0/12	QTY 8	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 146 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.96 (3-4)	Vert TL: 1.38 in	L/280	9	L/180
TCCL: 15	TPI 1-2014	BC: 0.66 (8-9)	Vert LL: 0.75 in	L/514	9	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.90 (5-7)	Horz TL: 0.21 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,502 lbs	.	.	-34 lbs	-34 lbs	197 lbs
7	1	2.75 in	1.51 in	1,513 lbs	.	.	-89 lbs	-89 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL 2400/2.0 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: Two Third Point Rows: 2-11, 5-7

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.470	463 lbs	3-4	0.957	497 lbs	(-6,981 lbs)			
	2-3	0.580	538 lbs	4-5	0.580	316 lbs	(-5,277 lbs)			
BC	7-8	0.478	4,641 lbs	8-9	0.663	6,897 lbs	(-469 lbs)	9-10	0.662	6,883 lbs
										(-558 lbs)
Web	2-11	0.870	(-4,760 lbs)	3-9	0.218	494 lbs		5-8	0.406	919 lbs
	2-10	0.411	930 lbs	4-9	0.213	482 lbs		5-7	0.904	309 lbs
	3-10	0.897	(-1,741 lbs)	4-8	0.883	(-1,715 lbs)				(-4,803 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSL-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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Truss: **E4**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
32-9-4	0/12	8	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	146 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.44 in, 2L/228 (12-1), Allowable 2L/120.

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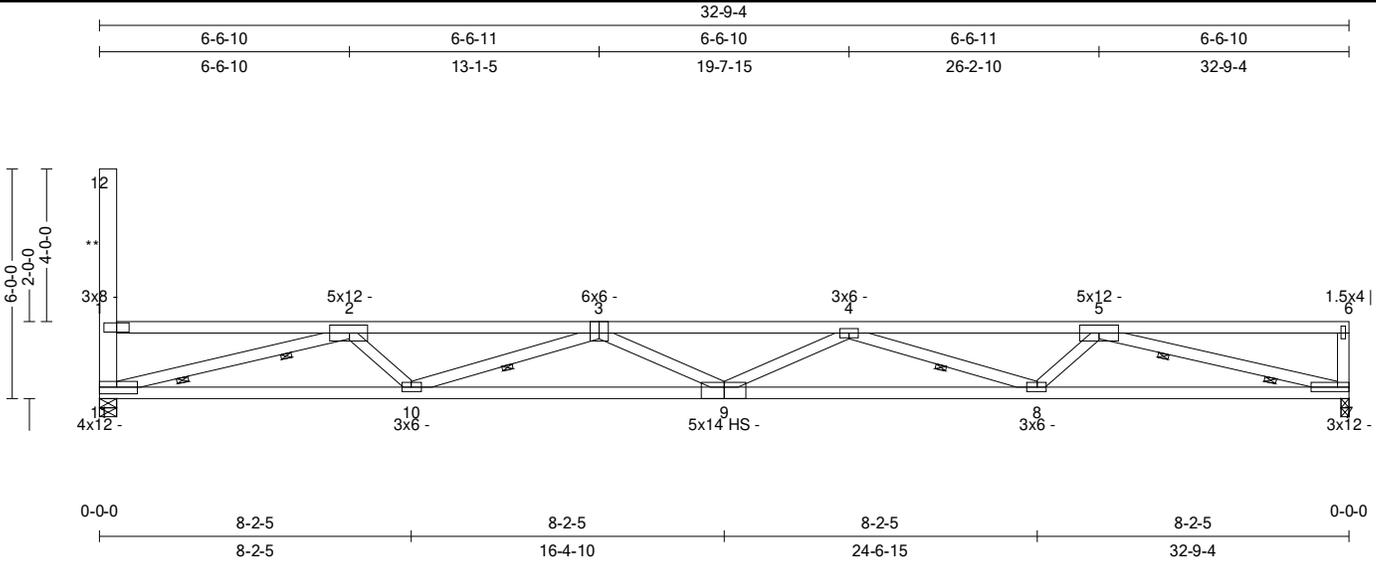
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Truss: E4D
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 1 of 2

SPAN 32-9-4	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 146 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.82 (3-4)	Vert TL: 1.33 in	L/ 290	9	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.73 (8-9)	Vert LL: 0.72 in	L/ 532	9	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.90 (5-7)	Horz TL: 0.28 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,502 lbs	.	.	-34 lbs	-34 lbs	-9,832 lbs
7	1	2.75 in	1.51 in	1,513 lbs	.	.	-89 lbs	-89 lbs	.

Material

TC: DFL 2400/2.0 2 x 4
 BC: DFL 2400/2.0 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed or Purlins at 2-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
 Web: One Midpoint Row: 3-10, 4-8
 Two Third Point Rows: 2-11, 5-7

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 9,832 lbs (300 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has not been designed for the effects of unbalanced snow loads.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 7) ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.398	1,911 lbs	(-1,911 lbs)	3-4	0.815	497 lbs	(-6,980 lbs)	5-6	0.409	1,980 lbs	(-1,980 lbs)
	2-3	0.497	538 lbs	(-5,237 lbs)	4-5	0.502	316 lbs	(-5,275 lbs)				
BC	7-8	0.530	4,639 lbs		8-9	0.732	7,033 lbs	(-787 lbs)	9-10	0.730	9,006 lbs	(-2,773 lbs)
					10-11	0.710	9,943 lbs	(-5,785 lbs)				
Web	2-11	0.869		(-4,757 lbs)	3-9	0.387	1,219 lbs	(-1,009 lbs)	5-8	0.406	1,194 lbs	
	2-10	0.411	1,206 lbs		4-9	0.386	1,217 lbs	(-1,021 lbs)	5-7	0.904	309 lbs	(-4,801 lbs)
	3-10	0.351	817 lbs	(-2,305 lbs)	4-8	0.351	842 lbs	(-2,306 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.

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Truss: **E4D**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:15
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
32-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	146 lbs

- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.51 in, 2L/194 (12-1), Allowable 2L/120.

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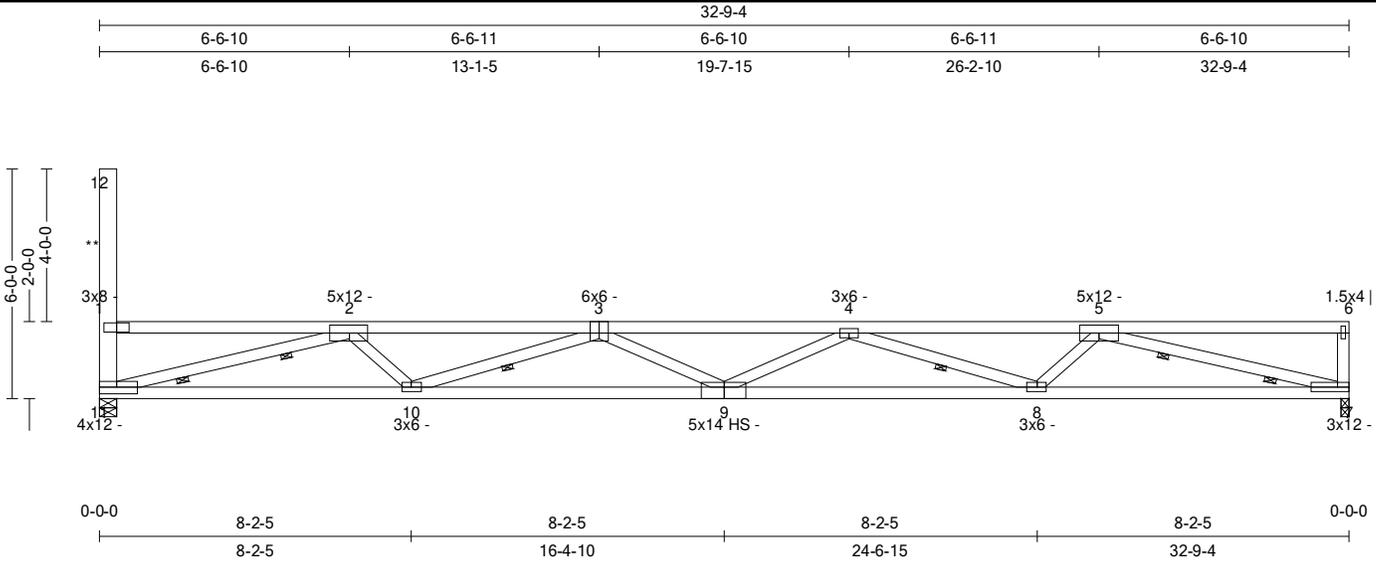
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Truss: E4DD
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 1 of 2

SPAN 32-9-4	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 146 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.82 (3-4)	Vert TL: 1.33 in	L/ 290	9	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.73 (8-9)	Vert LL: 0.72 in	L/ 532	9	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.90 (5-7)	Horz TL: 0.28 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,502 lbs	.	.	-34 lbs	-34 lbs	-9,832 lbs
7	1	2.75 in	1.51 in	1,513 lbs	.	.	-89 lbs	-89 lbs	.

Material

TC: DFL 2400/2.0 2 x 4
 BC: DFL 2400/2.0 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed or Purlins at 2-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
 Web: One Midpoint Row: 3-10, 4-8
 Two Third Point Rows: 2-11, 5-7

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 9,832 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.398	1,911 lbs	(-1,911 lbs)	3-4	0.815	497 lbs	(-6,980 lbs)	5-6	0.409	1,980 lbs	(-1,980 lbs)
	2-3	0.497	538 lbs	(-5,237 lbs)	4-5	0.502	316 lbs	(-5,275 lbs)				
BC	7-8	0.530	4,639 lbs		8-9	0.732	7,033 lbs	(-787 lbs)	9-10	0.730	9,006 lbs	(-2,773 lbs)
					10-11	0.710	9,943 lbs	(-5,785 lbs)				
Web	2-11	0.869		(-4,757 lbs)	3-9	0.387	1,219 lbs	(-1,009 lbs)	5-8	0.406	1,194 lbs	
	2-10	0.411	1,206 lbs		4-9	0.386	1,217 lbs	(-1,021 lbs)	5-7	0.904	309 lbs	(-4,801 lbs)
	3-10	0.351	817 lbs	(-2,305 lbs)	4-8	0.351	842 lbs	(-2,306 lbs)				

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.

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Truss: **E4DD**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
32-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	146 lbs

- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.51 in, 2L/194 (12-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

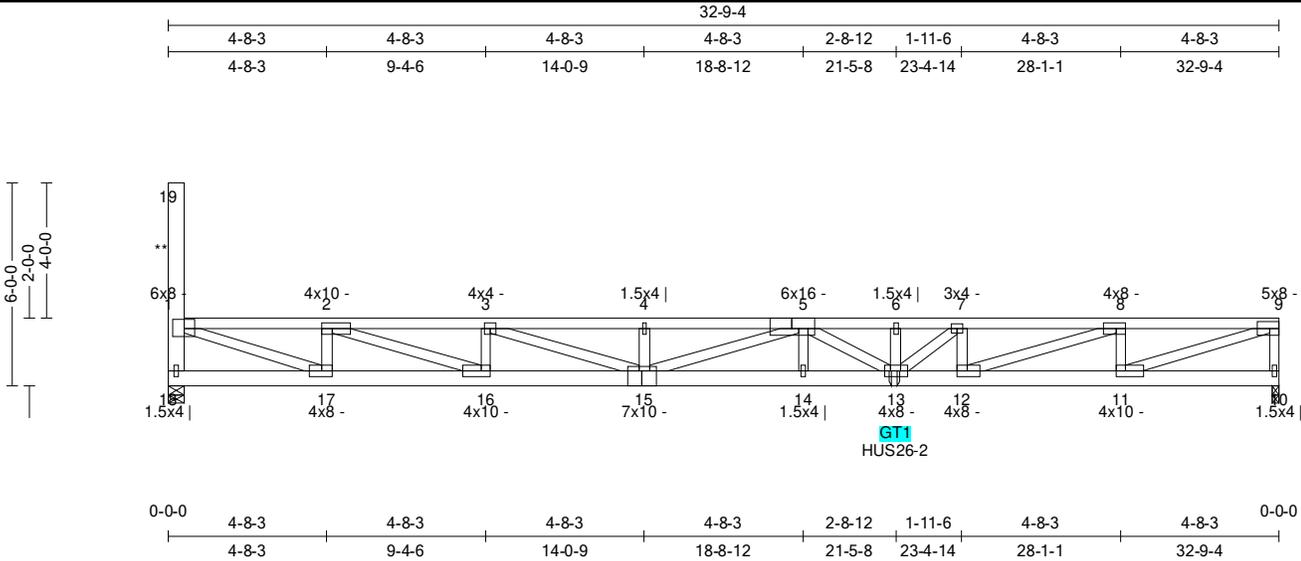
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Truss: E4G
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 1 of 2

SPAN 32-9-4	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 47.63 in	WGT/PLY 181 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.82 (7-8)	Vert TL: 1.19 in	L/ 324	(14-15)	L/ 180
TCLL: 25	TPI 1-2014	BC: 0.80 (13-14)	Vert LL: 0.65 in	L/ 595	(14-15)	L/ 240
TCDL: 15	Rep Mbr: No	Web: 0.94 (2-16)	Horz TL: 0.09 in		10	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	5.5 in	1.50 in	2,066 lbs	392 lbs
10	1	2.75 in	1.65 in	3,097 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 1-17, 8-12, 9-11
 DFL SS 2 x 6: 19-18

Bracing

TC: Sheathed or Purlins at 3-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 19-1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	28-8-2	Down	Proj	24.22 plf	24.22 plf	
Top	28-8-2	32-9-4	Down	Proj	24.22 plf	24.22 plf	
Top	0-0-0	21-5-0	Down	Proj	25.78 plf	25.78 plf	
Top	21-5-0	32-9-4	Down	Proj	75 plf	75 plf	

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Truss: **E4G**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

SPAN 32-9-4	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 47.63 in	WGT/PLY 181 lbs
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Load Case **DI**: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	28-8-2	Down	Proj	14.53 plf	14.53 plf	
Top	28-8-2	32-9-4	Down	Proj	14.53 plf	14.53 plf	
Top	0-0-0	21-5-0	Down	Proj	15.47 plf	15.47 plf	
Top	21-5-0	32-9-4	Down	Proj	45 plf	45 plf	
Bot	0-0-0	28-8-2	Down	Proj	5.81 plf	5.81 plf	
Bot	28-8-2	32-9-4	Down	Proj	5.81 plf	5.81 plf	
Bot	0-0-0	21-5-0	Down	Proj	6.19 plf	6.19 plf	
Bot	21-5-0	32-9-4	Down	Proj	18 plf	18 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.296	341 lbs	(-2,542 lbs)	3-4	0.700	(-5,853 lbs)	5-6	0.765	(-6,788 lbs)	7-8	0.822	(-6,158 lbs)
	2-3	0.518		(-4,547 lbs)	4-5	0.661	(-5,861 lbs)	6-7	0.732	(-6,788 lbs)	8-9	0.522	(-3,733 lbs)
BC	11-12	0.510	3,733 lbs		13-14	0.803	6,795 lbs	15-16	0.571	4,547 lbs	17-18	0.050	(-436 lbs)
	12-13	0.751	6,158 lbs		14-15	0.751	6,765 lbs	16-17	0.304	2,542 lbs			(-305 lbs)
Web	1-18	0.220		(-1,002 lbs)	3-15	0.612	1,385 lbs	8-11	0.152				(-1,269 lbs)
	1-17	0.520	2,706 lbs		5-15	0.151	(-963 lbs)	9-11	0.761	3,965 lbs			
	2-17	0.108		(-902 lbs)	7-13	0.362	820 lbs	9-10	0.179				(-1,495 lbs)
	2-16	0.937	2,122 lbs		7-12	0.097	(-809 lbs)						
	3-16	0.078		(-650 lbs)	8-12	0.493	2,567 lbs						

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
GT1	BC	21-5-0	1120	0	0	HUS26-2	4x SD10212, 4x SD10212

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental Head Side - FastenMaster FlatLOK (2 - Ply) Screws within 12" along the chord or into converging webs at point load:

BC: 21-5-0,(3)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

- 9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 10) Lateral bracing shall be attached to each ply.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.
- 12) Parapet TL: 0.39 in, 2L/258 (19-1), Allowable 2L/120.

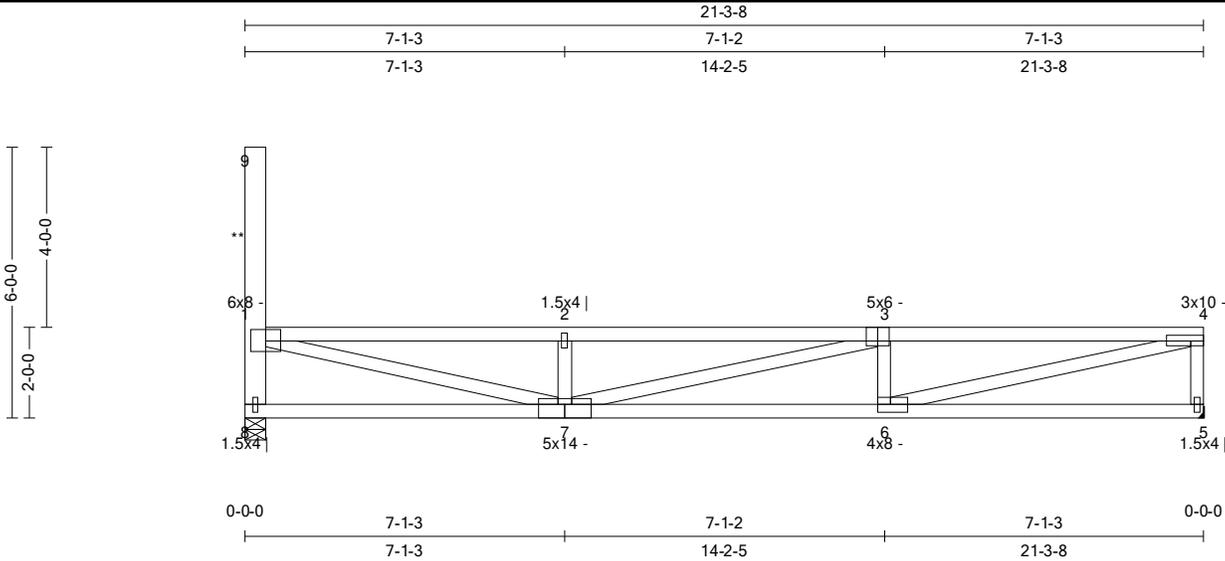
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 21-3-8	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 100 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.97 (3-4)	Vert TL: 0.4 in	L/ 628	(6-7)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.70 (6-7)	Vert LL: 0.21 in	L/ 999	(6-7)	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.56 (1-7)	Horz TL: 0.03 in		5	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	5.5 in	1.50 in	974 lbs	.	.	-44 lbs	-44 lbs	197 lbs
5	1	1.5 in	--	985 lbs	.	.	-127 lbs	-127 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 1-7, 4-6
 DFL SS 2 x 6: 9-8

Bracing

TC: Sheathed
 BC: Sheathed or Purlins at 9-5-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 9-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.872	550 lbs	(-2,830 lbs)	2-3	0.587	550 lbs	(-2,830 lbs)	3-4	0.974	398 lbs	(-2,827 lbs)
BC	6-7	0.700	2,880 lbs	(-370 lbs)	7-8	0.327		(-425 lbs)				
Web	1-8	0.219		(-933 lbs)	2-7	0.077		(-605 lbs)	3-6	0.078		(-611 lbs)
	1-7	0.560	2,916 lbs		3-7	0.472	438 lbs	(-511 lbs)	4-6	0.559	2,912 lbs	(-389 lbs)
									4-5	0.121		(-944 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) A creep factor of 2.00 has been applied for this truss analysis.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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Truss: **E4S**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
21-3-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	100 lbs

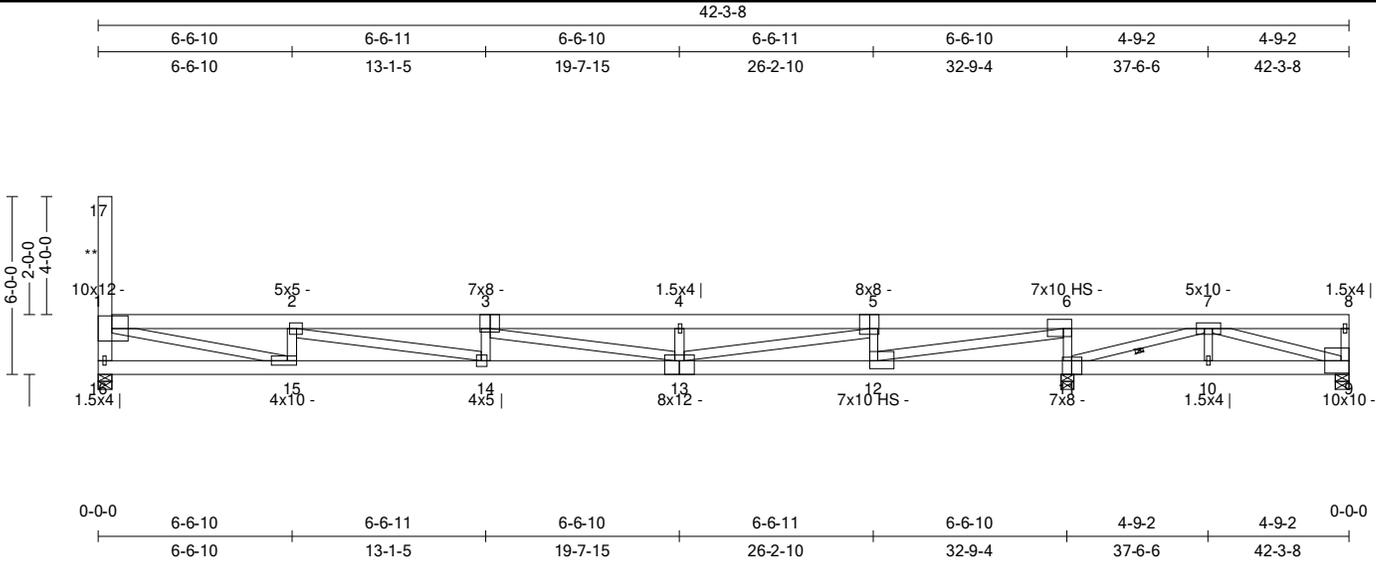
8) Listed wind uplift reactions based on MWFRS & C&C loading.
 9) Parapet TL: 0.18 in, 2L/565 (9-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 42-3-8	PITCH 0/12	QTY 6	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 254 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.74 (6-7)	Vert TL: 0.9 in	L/ 428	(13-14)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.54 (13-14)	Vert LL: 0.49 in	L/ 786	(13-14)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.90 (3-13)	Horz TL: 0.03 in		11	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.50 in	1,280 lbs	197 lbs
11	1	5.5 in	3.22 in	3,015 lbs	.	.	-125 lbs	-125 lbs	.
9	1	5.5 in	1.50 in	46 lbs	-630 lbs	-87 lbs	.	-630 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #1B 2 x 4: 6-12
 DFL #2 2 x 4: 1-15, 5-13, 7-9
 DFL SS 2 x 6: 17-16

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
 BC: Sheathed or Purlins at 3-11-0, Purlin design by Others.
 Web: One Midpoint Row: 7-11

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 17-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.243	481 lbs	(-4,089 lbs)
	2-3	0.305	409 lbs	(-5,614 lbs)
BC	9-10	0.117	(-2,193 lbs)	11-12 0.281
	10-11	0.246	(-2,193 lbs)	12-13 0.212 1,398 lbs
	13-14	0.542	5,643 lbs	(-377 lbs)
	14-15	0.428	4,089 lbs	(-447 lbs)
Web	1-16	0.229	(-1,222 lbs)	3-13 0.900
	1-15	0.808	4,208 lbs	4-13 0.064
	2-15	0.115	(-929 lbs)	5-13 0.670 3,489 lbs
	2-14	0.692	1,567 lbs	5-12 0.158
	6-12	0.763	5,528 lbs	
	6-11	0.238	(-1,927 lbs)	
	7-11	0.406	(-2,577 lbs)	
	7-9	0.444	2,313 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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Truss: **E5**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
42-3-8	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	254 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 9 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.26 in, 2L/386 (17-1), Allowable 2L/120.

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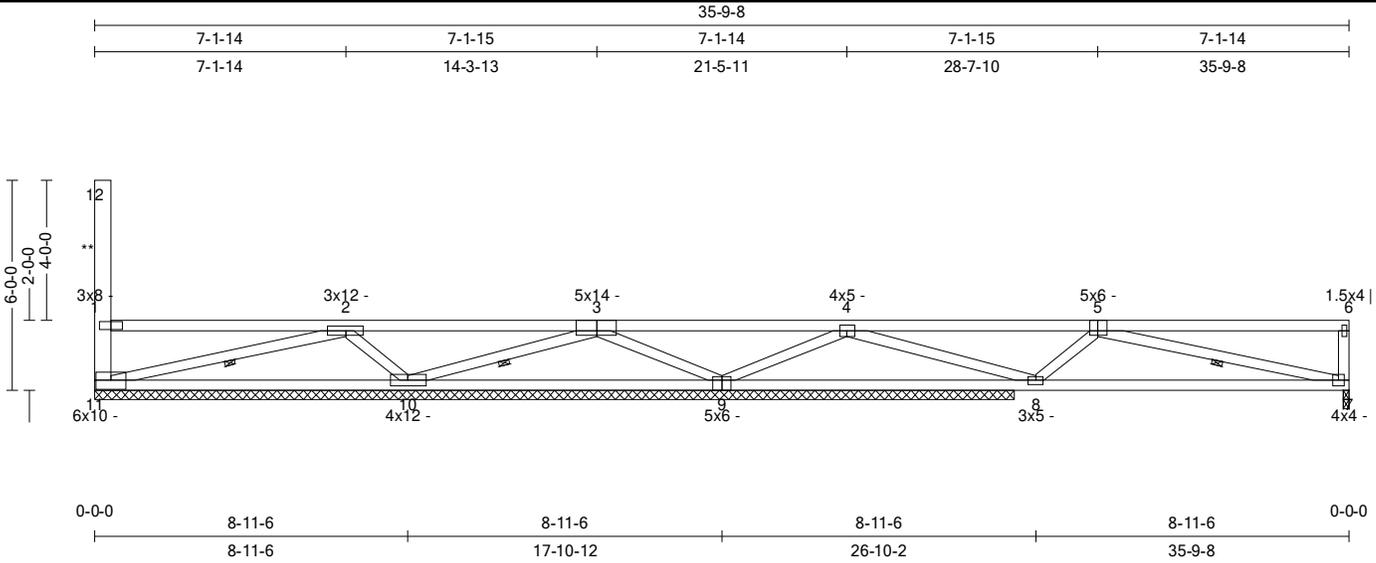
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Truss: E6D
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 1 of 2

SPAN 35-9-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 156 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.87 (3-4)	Vert TL: 0.32 in	L/ 348	(7-8)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.81 (7-8)	Vert LL: 0.18 in	L/ 625	(7-8)	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.88 (3-10)	Horz TL: 0.06 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	2 in	1.50 in	683 lbs	-21 lbs	.	-27 lbs	-27 lbs	.
9	1	315 in	N/A	1,590 lbs	.	.	-81 lbs	-81 lbs	5,450 lbs
10	1	315 in	N/A	803 lbs	.	.	-46 lbs	-46 lbs	-3,485 lbs
11	1	315 in	N/A	709 lbs	-435 lbs	.	.	-435 lbs	2,364 lbs

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 3-8-0, Purlin design by Others.
 Web: One Midpoint Row: 2-11, 3-10, 5-7

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 10,738 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.791	2,092 lbs	(-2,092 lbs)	3-4	0.873	3,735 lbs	(-2,317 lbs)	5-6	0.727	2,161 lbs	(-2,161 lbs)
	2-3	0.847	1,532 lbs	(-1,139 lbs)	4-5	0.576	1,962 lbs	(-3,282 lbs)				
BC	7-8	0.812	2,077 lbs	(-634 lbs)	8-9	0.637	2,856 lbs	(-2,637 lbs)				
Web	2-11	0.745	2,347 lbs	(-2,435 lbs)	3-9	0.733	1,422 lbs	(-2,466 lbs)	5-8	0.124	392 lbs	(-562 lbs)
	2-10	0.157	494 lbs	(-1,156 lbs)	4-9	0.659		(-2,130 lbs)	5-7	0.538	652 lbs	(-2,138 lbs)
	3-10	0.883	2,780 lbs	(-2,693 lbs)	4-8	0.532	1,675 lbs	(-520 lbs)				

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

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Truss: **E6D**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-9-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	156 lbs

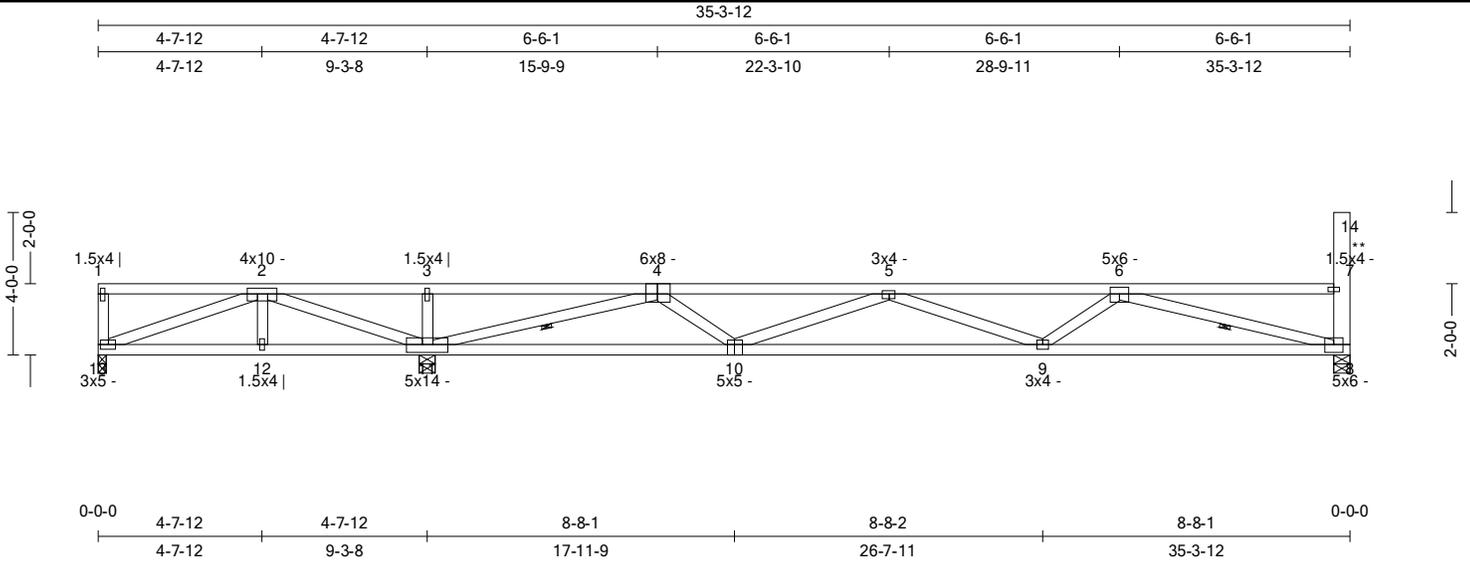
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 11 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.16 in, 2L/642 (12-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 35-3-12	PITCH 0/12	QTY 27	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 152 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.87 (2-3)	Vert TL: 0.56 in	L/ 545	(9-10)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.83 (9-10)	Vert LL: 0.29 in	L/ 999	(9-10)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.98 (4-11)	Horz TL: 0.08 in		8	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.50 in	177 lbs	-325 lbs	-42 lbs	-3 lbs	-325 lbs	-96 lbs
11	1	5.5 in	2.50 in	2,344 lbs	.	.	-86 lbs	-86 lbs	.
8	1	5.5 in	1.50 in	1,011 lbs	.	.	-19 lbs	-19 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 5-10-0, Purlin design by Others.
 Web: One Midpoint Row: 4-11, 6-8

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.871	2,605 lbs	4-5	0.371	(-2,098 lbs)		
	3-4	0.871	2,605 lbs	5-6	0.429	(-3,022 lbs)		
BC	8-9	0.786	2,781 lbs	10-11	0.590	1,424 lbs	12-13	0.141 (-1,106 lbs)
	9-10	0.834	3,219 lbs	11-12	0.342	(-1,106 lbs)		
Web	2-13	0.523	1,183 lbs	4-10	0.413	934 lbs		
	2-11	0.819	(-1,950 lbs)	5-10	0.552	(-1,323 lbs)		
	3-11	0.065	(-508 lbs)	6-9	0.197	447 lbs		
	4-11	0.975	(-4,123 lbs)	6-8	0.636	(-2,882 lbs)		

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **F1**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

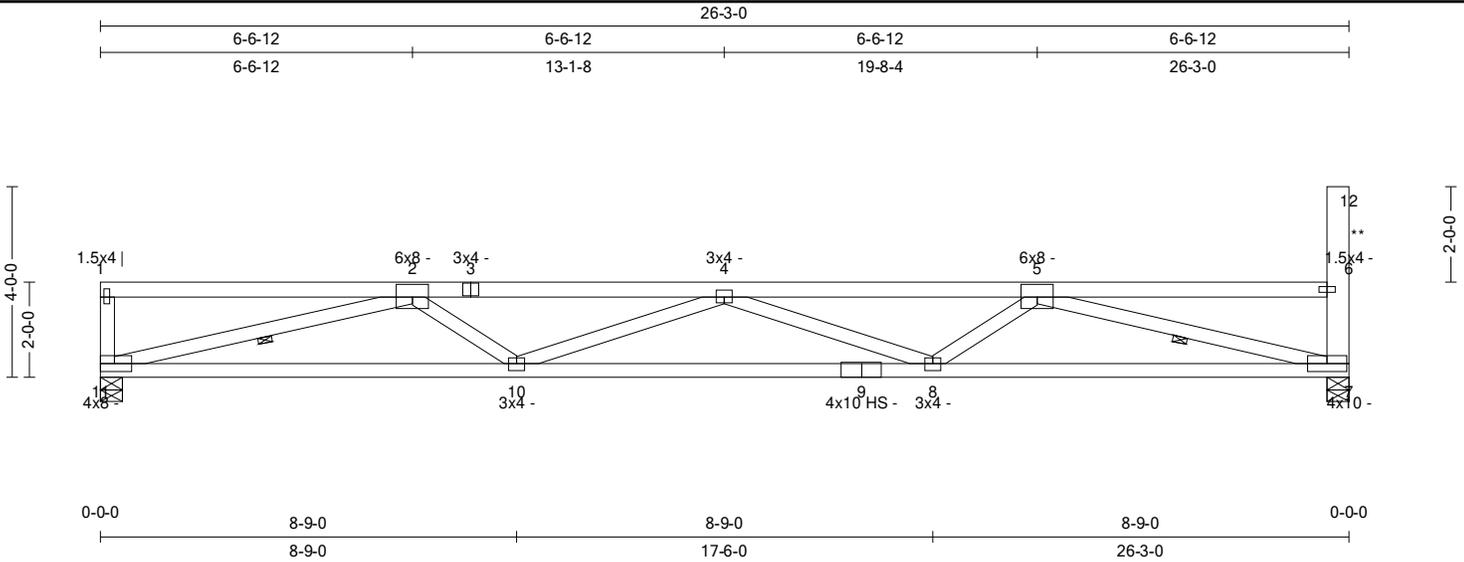
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-3-12	0/12	27	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.1 in, 2L/524 (14-7), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 26-3-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 114 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.82 (1-2)	Vert TL: 0.72 in	L / 421	(9-10)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.80 (8-10)	Vert LL: 0.39 in	L / 787	(9-10)	L / 240
BCLL : 0	Rep Mbr : No	Web : 1.00 (2-11)	Horz TL: 0.13 in		7	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,213 lbs	.	.	-93 lbs	-93 lbs	-96 lbs
7	1	5.5 in	1.50 in	1,202 lbs	.	.	-62 lbs	-62 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #1B 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL SS 2 x 6: 12-7

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-11, 5-7

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 12-6

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-4	0.761	(-3,987 lbs)	4-5	0.763	335 lbs	(-3,965 lbs)			
BC	7-8	0.691	3,496 lbs	8-10	0.804	4,628 lbs		10-11	0.701	3,528 lbs
Web	2-11	0.998	(-3,651 lbs)	4-8	0.331		(-737 lbs)			
	2-10	0.215	643 lbs	5-8	0.217	649 lbs				
	4-10	0.337	(-752 lbs)	5-7	0.964		(-3,621 lbs)			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Truss: **FIA**
Job: **2501272**
Date: 01/23/26 09:23:16
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-3-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	114 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.13 in, 2L/404 (12-6), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

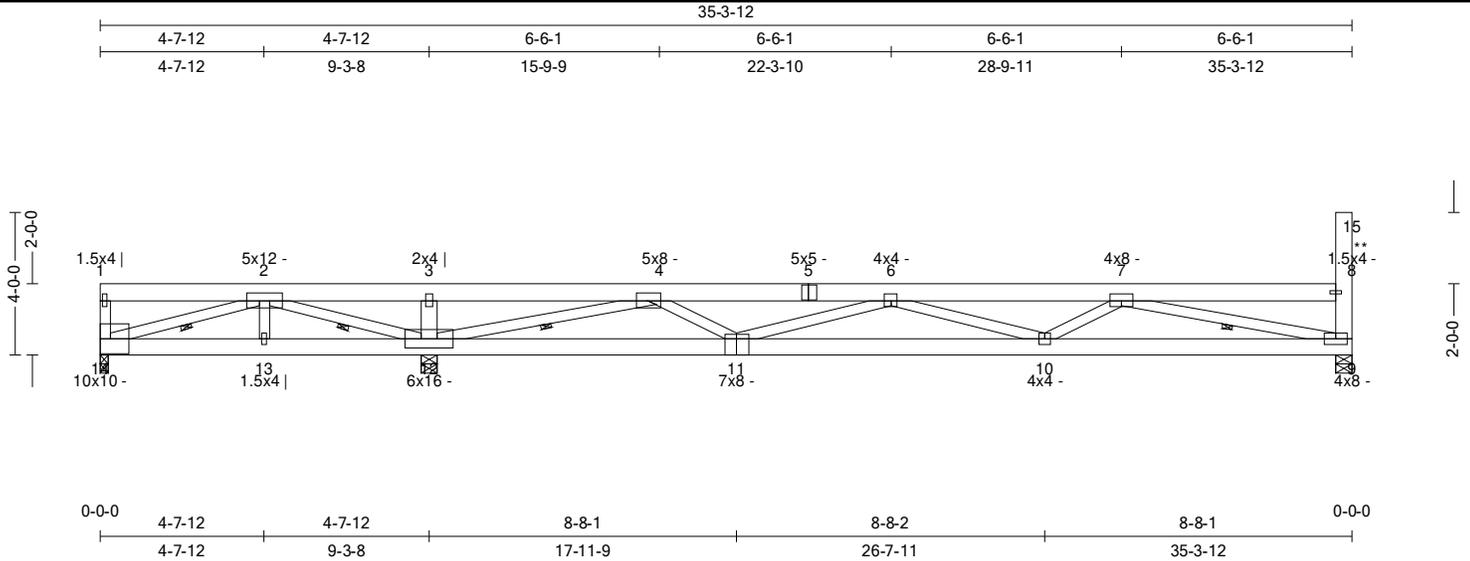
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Truss: F1D
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 1 of 2

SPAN 35-3-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 205 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.59 (3-4)	Vert TL: 0.43 in	L/ 713	(10-11)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.56 (12-13)	Vert LL: 0.23 in	L/ 999	(10-11)	L/ 240
BCLL: 0	Rep Mbr: No	Web: 0.83 (4-12)	Horz TL: 0.11 in		9	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	2.75 in	1.50 in	1,023 lbs	-1,083 lbs	-36 lbs	-4 lbs	-1,083 lbs	-10,594 lbs
12	1	5.5 in	2.45 in	2,297 lbs	.	.	-85 lbs	-85 lbs	.
9	1	5.5 in	1.50 in	1,023 lbs	.	.	-20 lbs	-20 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 2-14
 DFL SS 2 x 6: 3-12, 15-9

Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
 Web: One Midpoint Row: 2-14, 2-12, 4-12, 7-9

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 10,594 lbs (300 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 15-8

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

	1-2	0.228	1,403 lbs	(-1,403 lbs)	3-4	0.590	4,301 lbs	(-2,114 lbs)	6-7	0.195	936 lbs	(-3,972 lbs)
TC	2-3	0.590	5,704 lbs	(-3,516 lbs)	4-6	0.242	2,093 lbs	(-4,181 lbs)	7-8	0.235	1,895 lbs	(-1,895 lbs)
BC	9-10	0.338	2,979 lbs		11-12	0.394	3,867 lbs	(-2,722 lbs)	13-14	0.533	6,801 lbs	(-7,561 lbs)
	10-11	0.423	3,728 lbs	(-538 lbs)	12-13	0.565	6,801 lbs	(-7,561 lbs)				
Web	2-14	0.553	4,009 lbs	(-3,206 lbs)	4-11	0.493	1,117 lbs		7-9	0.666		(-3,068 lbs)
	2-12	0.631	1,988 lbs		6-11	0.597		(-1,486 lbs)				
	3-12	0.036		(-614 lbs)	6-10	0.335	690 lbs	(-854 lbs)				
	4-12	0.825		(-3,884 lbs)	7-10	0.221	689 lbs					

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Mustang Truss
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Truss: **FID**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:16
 Page: 2 of 2

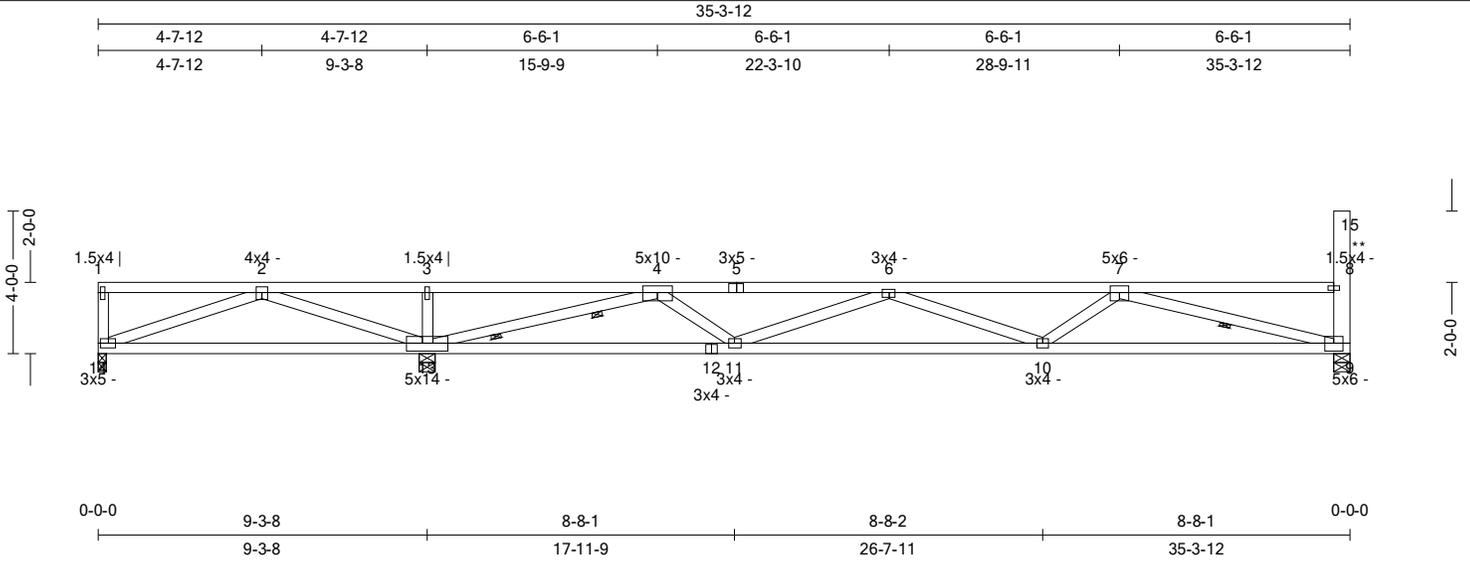
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-3-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	205 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 14 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.07 in, 2L/785 (15-8), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products

SPAN 35-3-12	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 150 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.98 (2-3)	Vert TL: 0.27 in	L / 389	(13-14)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.96 (10-11)	Vert LL: 0.18 in	L / 588	(13-14)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.99 (4-13)	Horz TL: 0.08 in		9	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	2.75 in	1.50 in	176 lbs	-326 lbs	-43 lbs	-3 lbs	-326 lbs	-96 lbs
13	1	5.5 in	2.50 in	2,344 lbs	.	.	-85 lbs	-85 lbs	.
9	1	5.5 in	1.50 in	1,011 lbs	.	.	-20 lbs	-20 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL SS 2 x 6: 15-9

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
 BC: Sheathed or Purlins at 5-6-0, Purlin design by Others.
 Web: One Midpoint Row: 7-9
 Two Third Point Rows: 4-13

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 15-8

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.982	2,594 lbs	4-6	0.428	(-2,092 lbs)			
	3-4	0.982	2,594 lbs	6-7	0.496	(-3,022 lbs)			
BC	9-10	0.906	2,781 lbs	10-11	0.957	3,215 lbs	11-13	0.945	1,423 lbs
							13-14	0.647	(-1,141 lbs)
Web	2-14	0.408	1,220 lbs	4-13	0.991	(-4,108 lbs)	7-10	0.150	447 lbs
	2-13	0.893	(-1,908 lbs)	4-11	0.310	927 lbs	7-9	0.761	(-2,882 lbs)
	3-13	0.099	(-507 lbs)	6-11	0.587	(-1,327 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Mustang Truss
2525 Hyacinth Street NE
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Truss: **FIG**
Job: **2501272**
Date: 01/23/26 09:23:17
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-3-12	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	150 lbs

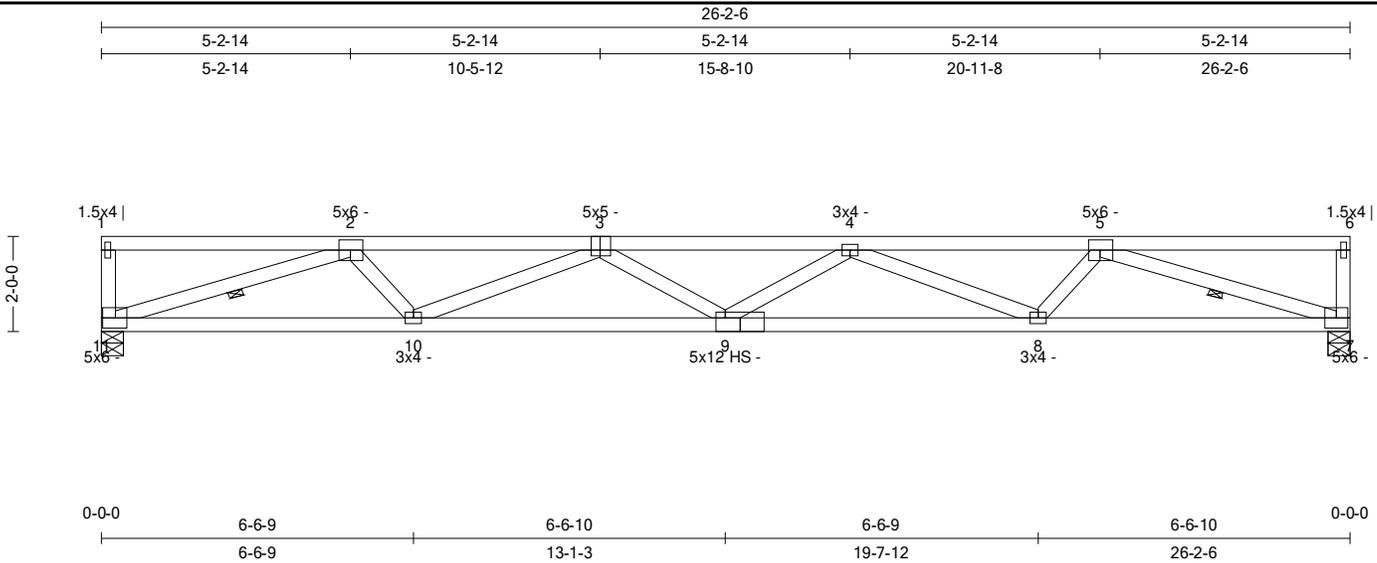
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 14 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.1 in, 2L/524 (15-8), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 26-2-6	PITCH 0/12	QTY 5	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 110 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.56 (3-4)	Vert TL: 0.66 in	L/ 460	(9-10)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.91 (9-10)	Vert LL: 0.36 in	L/ 850		L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.54 (2-11)	Horz TL: 0.14 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,205 lbs	.	.	-84 lbs	-84 lbs	32 lbs
7	1	5.5 in	1.50 in	1,205 lbs	.	.	-84 lbs	-84 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 2-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-11, 5-7

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.400	(-3,345 lbs)	4-5	0.400	(-3,342 lbs)			
	3-4	0.556	(-4,436 lbs)						
BC	7-8	0.641	2,936 lbs	8-9	0.889	4,382 lbs	9-10	0.908	4,375 lbs
	2-11	0.541	(-3,099 lbs)	3-9	0.156	353 lbs	5-8	0.295	668 lbs
Web	2-10	0.296	669 lbs	4-9	0.136	307 lbs	5-7	0.541	(-3,096 lbs)
	3-10	0.400	(-1,123 lbs)	4-8	0.404	(-1,134 lbs)			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

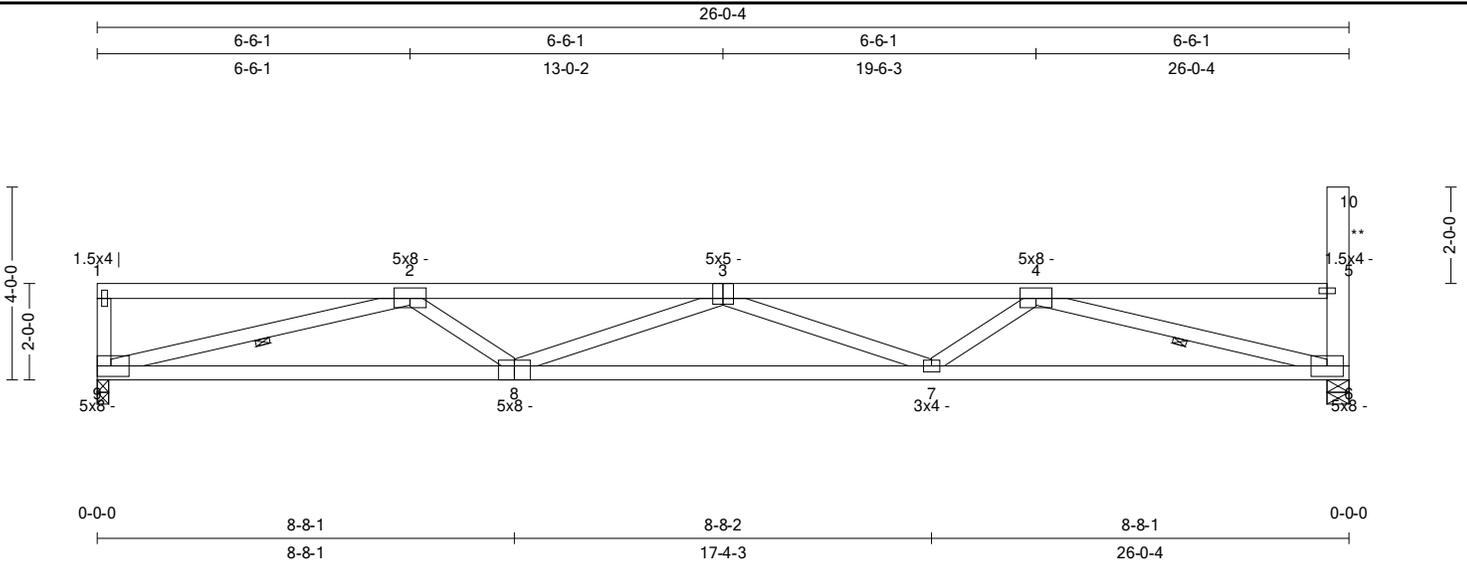
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
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Truss: F3
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:17
 Page: 1 of 2

SPAN 26-0-4	PITCH 0/12	QTY 22	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 113 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.70 (1-2)	Vert TL: 0.74 in	L/413	(7-8)	L/180
TCDL: 15	TPI 1-2014	BC: 0.96 (7-8)	Vert LL: 0.39 in	L/772	(7-8)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (2-9)	Horz TL: 0.14 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	2.75 in	1.50 in	1,202 lbs	.	.	-93 lbs	-93 lbs	-96 lbs
6	1	5.5 in	1.50 in	1,192 lbs	.	.	-62 lbs	-62 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-9, 4-6

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.636	(-3,917 lbs)	3-4	0.627	334 lbs	(-3,896 lbs)		
BC	6-7	0.825	3,434 lbs	7-8	0.965	4,547 lbs	8-9	0.837	3,466 lbs
Web	2-9	0.822	(-3,589 lbs)	3-7	0.303		(-725 lbs)		
	2-8	0.280	634 lbs	4-7	0.283	640 lbs			
	3-8	0.309	(-740 lbs)	4-6	0.785		(-3,559 lbs)		

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Truss: **F3**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:17
 Page: 2 of 2

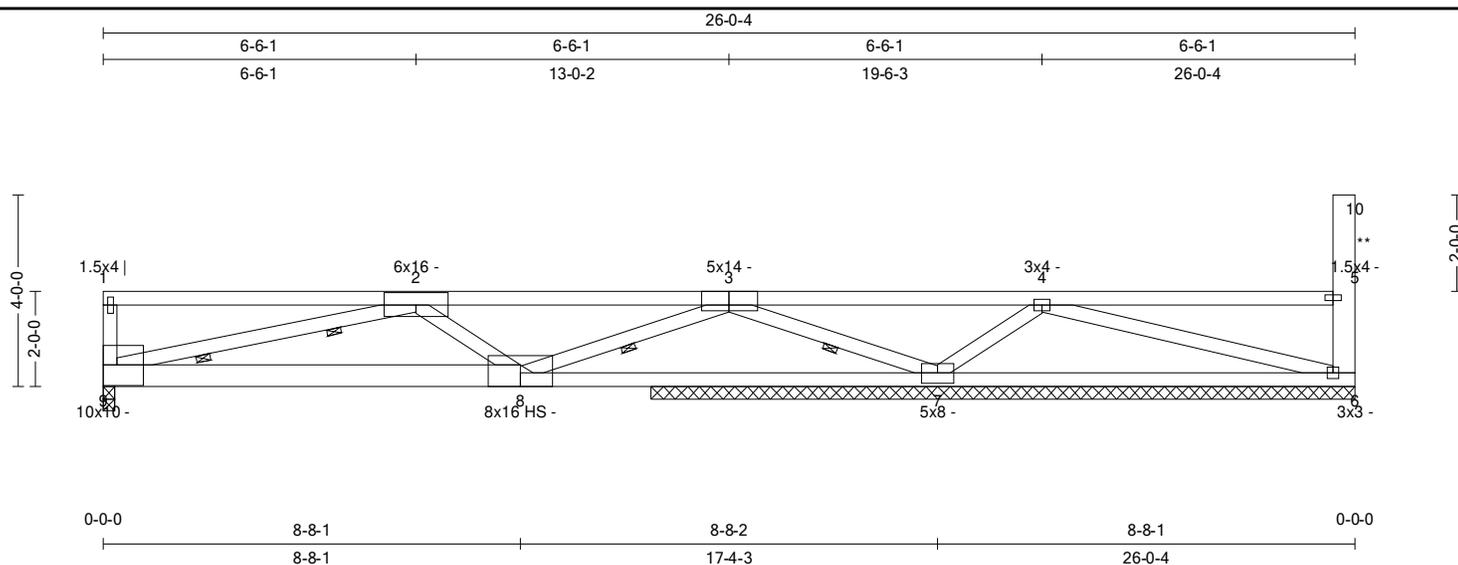
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-0-4	0/12	22	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	113 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.13 in, 2L/384 (10-5), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 26-0-4	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 124 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.86 (3-4)	Vert TL: 0.21 in	L / 650	(7-8)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.95 (7-8)	Vert LL: 0.15 in	L / 909	(7-8)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.90 (2-9)	Horz TL: 0.15 in		6	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	2.75 in	1.92 in	1,796 lbs	-1,165 lbs	.	-57 lbs	-1,165 lbs	16,783 lbs
6	1	175.498 in	N/A	285 lbs	-144 lbs	.	.	-144 lbs	.
7	1	175.498 in	N/A	1,973 lbs	-559 lbs	.	-108 lbs	-559 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4 except:
 DFL SS 2 x 6: 8-9
 Web: DFL Stud 2 x 4 except:
 DFL #1B 2 x 4: 2-9
 DFL #2 2 x 4: 3-8
 DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-9-0, Purlin design by Others.
 BC: Sheathed or Purlins at 2-6-0, Purlin design by Others.
 Web: One Midpoint Row: 3-8, 3-7
 Two Third Point Rows: 2-9

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 16,783 lbs (645 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.649	4,233 lbs (-4,233 lbs)	2.3
TC	3-4	0.856	7,733 lbs (-6,735 lbs)	4.5
BC	6-7	0.533	659 lbs (-980 lbs)	7.8
BC	8-9	0.725	11,466 lbs (-10,031 lbs)	8.9
Web	2-9	0.902	5,489 lbs (-6,970 lbs)	3.7
Web	2-8	0.595	2,474 lbs (-2,588 lbs)	4.7
Web	3-8	0.628	4,551 lbs (-3,790 lbs)	4.6

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

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Truss: **F3-D**
Job: **2501272**
Date: 01/23/26 09:23:17
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
26-0-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	124 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 9, 6, 7 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.03 in, 2L/999 (10-5), Allowable 2L/120.

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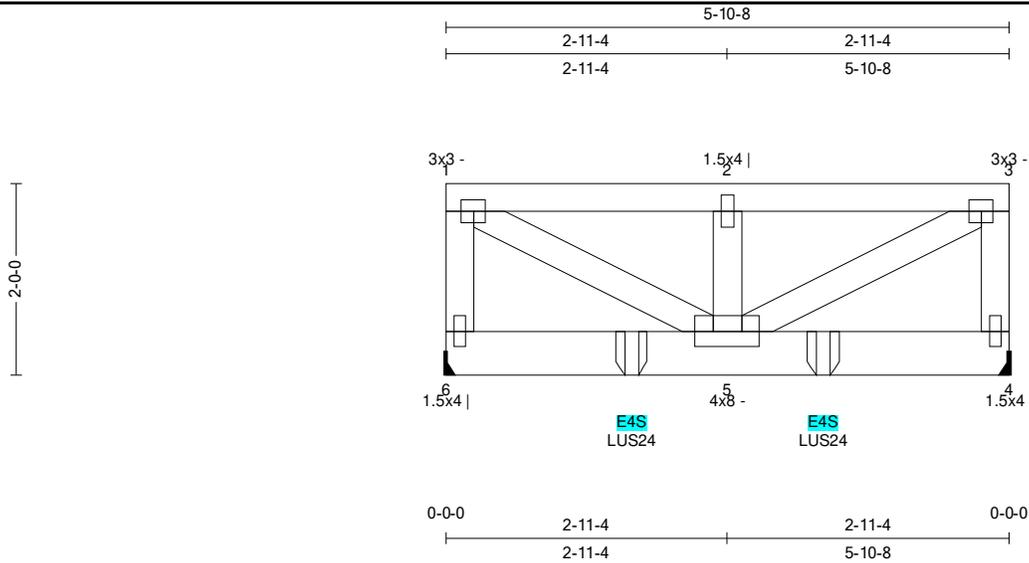
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Truss: GT1
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:17
 Page: 1 of 2

SPAN 5-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 12 in	WGT/PLY 32 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0.02 in	L/999	(4-5)	L/180
TCLL: 25	TPI 1-2014	BC: 0.12 (4-5)	Vert LL: 0.01 in	L/999	(4-5)	L/240
TCDL: 15	Rep Mbr: No	Web: 0.33 (1-5)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	1,120 lbs	-16 lbs
4	1	1.5 in	---	1,120 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	max CSI	max tension force	max compression force		
TC	1-2	0.071	(-645 lbs)	2-3	0.071	(-645 lbs)
BC	1-6	0.049	(-408 lbs)	3-5	0.329	746 lbs
Web	1-5	0.329	746 lbs	3-4	0.049	(-408 lbs)

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
E4S	BC	1-11-4	985	0	0	LUS24	2x SD9212, 4x SD9112
E4S	BC	3-11-4	985	0	0	LUS24	2x SD9212, 4x SD9112

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Truss: **GT1**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:17
 Page: 2 of 2

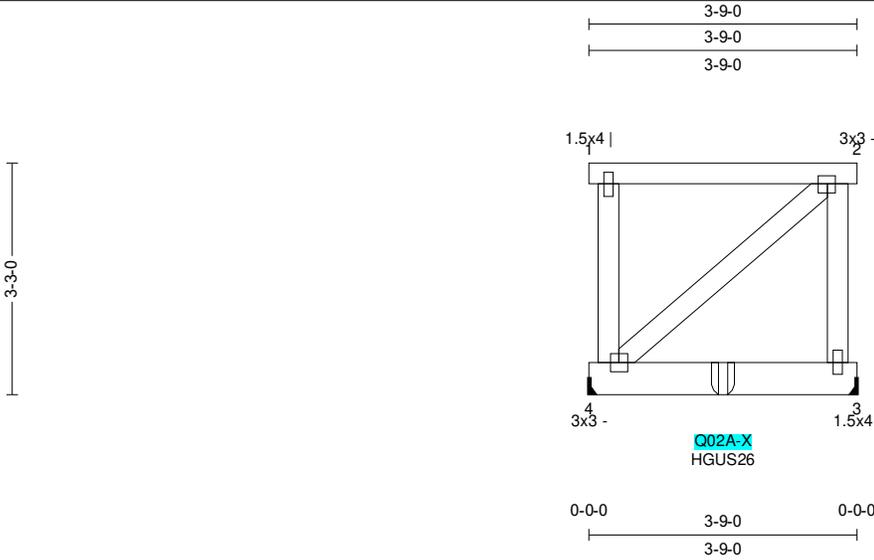
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
5-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	12in	32lbs

- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row @ 2-0-0 oc.
- 8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 9) Lateral bracing shall be attached to each ply.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

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SPAN 3-9-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 13.13 in	WGT/PLY 24lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0.05 in	L/933	(3-4)	L/180
TCLL: 25	TPI 1-2014	BC: 0.67 (3-4)	Vert LL: 0.01 in	L/999	(3-4)	L/240
TCDL: 15	Rep Mbr: No	Web: 0.04 (2-3)	Horz TL: 0 in		3	
BCLL: 0	Lumber D.O.L.: 115%					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	--	1,182 lbs	29 lbs
3	1	1.5 in	--	1,182 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	27.34 plf	27.34 plf	

Load Case Df: Std Dead Load

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	16.41 plf	16.41 plf	
Bot	0-0-0	3-9-0	Down	Proj	6.56 plf	6.56 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC	
BC	
Web	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
Q02A-X	BC	1-10-8	2261	0	0	HGUS26	8x 16d, 20x 16d

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
3-9-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	13.13 in	24lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc - Z(TSLZ278, 2 - ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row 0.131"x3" Nails @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies; in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2 - ply) Screws as follows within 24" of the location shown:

BC: 1-10-8,(7)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

9) Lateral bracing shall be attached to each ply.

10) Install screws per manufacturer recommendations.

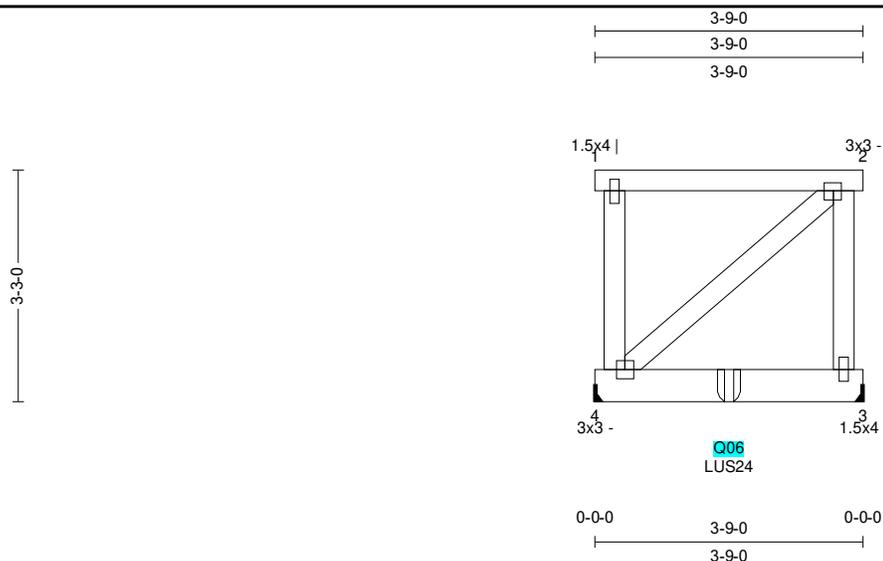
11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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Eagle Metal Products

SPAN 3-9-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 13.13 in	WGT/PLY 24lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.14 (1-2)	Vert TL: 0.03 in	L / 999	(3-4)	L / 180
TCLL: 25	TPI 1-2014	BC: 0.46 (3-4)	Vert LL: 0.01 in	L / 999	(3-4)	L / 240
TCDL: 15	Rep Mbr: No	Web: 0.07 (2-3)	Horz TL: 0 in		3	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	435 lbs	.	.	-2 lbs	-2 lbs	29 lbs
3	1	1.5 in	---	435 lbs	.	.	-2 lbs	-2 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	27.34 plf	27.34 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	16.41 plf	16.41 plf	
Bot	0-0-0	3-9-0	Down	Proj	6.56 plf	6.56 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC	
BC	
Web	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
Q06	BC	1-10-8	767	0	0	LUS24	2x SD9212, 4x SD9112

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Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **GT3**
Job: **2501272**
Date: 01/23/26 09:23:17
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
3-9-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	13.13 in	24 lbs

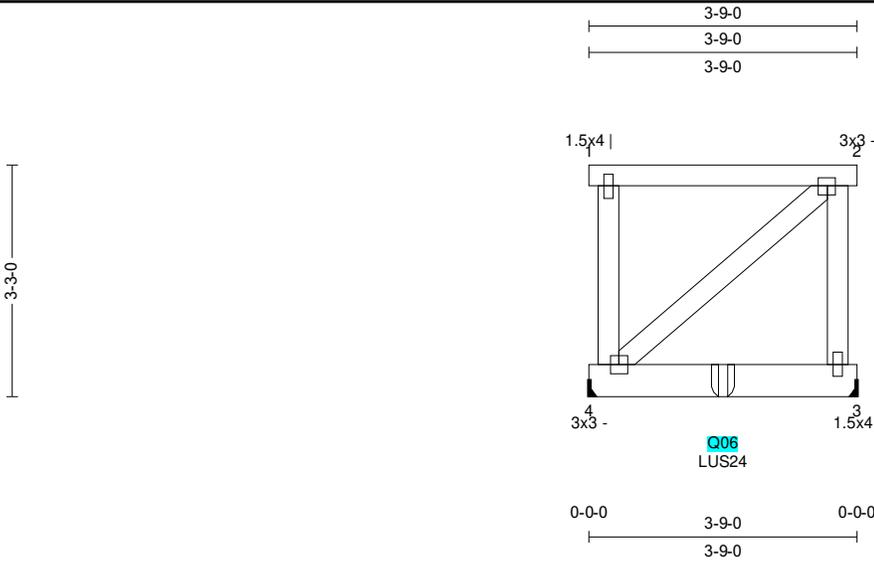
Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

SPAN 3-9-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 13.13 in	WGT/PLY 24lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.14 (1-2)	Vert TL: 0.03 in	L / 999	(3-4)	L / 180
TCLL: 25	TPI 1-2014	BC: 0.46 (3-4)	Vert LL: 0.01 in	L / 999	(3-4)	L / 240
TCDL: 15	Rep Mbr: No	Web: 0.07 (2-3)	Horz TL: 0 in		3	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	435 lbs	.	.	-2 lbs	-2 lbs	29 lbs
3	1	1.5 in	---	435 lbs	.	.	-2 lbs	-2 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	27.34 plf	27.34 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	16.41 plf	16.41 plf	
Bot	0-0-0	3-9-0	Down	Proj	6.56 plf	6.56 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC	
BC	
Web	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
Q06	BC	1-10-8	767	0	0	LUS24	2x SD9212, 4x SD9112

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Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **GT4**
Job: **2501272**
Date: 01/23/26 09:23:17
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
3-9-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	13.13 in	24 lbs

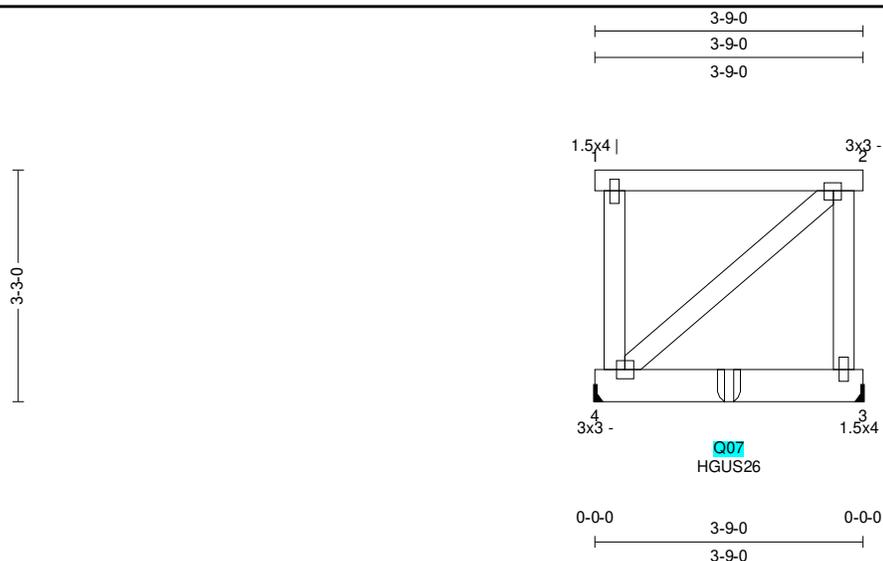
Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

SPAN 3-9-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 13.13 in	WGT/PLY 24lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.07 (1-2)	Vert TL: 0.04 in	L / 999	(3-4)	L / 180
TCLL: 25	TPI 1-2014	BC: 0.55 (3-4)	Vert LL: 0.01 in	L / 999	(3-4)	L / 240
TCDL: 15	Rep Mbr: No	Web: 0.04 (2-3)	Horz TL: 0 in		3	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	961 lbs	29 lbs
3	1	1.5 in	---	961 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	27.34 plf	27.34 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-9-0	Down	Proj	16.41 plf	16.41 plf	
Bot	0-0-0	3-9-0	Down	Proj	6.56 plf	6.56 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC	
BC	
Web	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
Q07	BC	1-10-8	1820	0	0	HGUS26	8x 16d, 20x 16d

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SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
3-9-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	13.13 in	24lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc - Z(TSLZ278, 2 - ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row 0.131"x3" Nails @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies; in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2 - ply) Screws as follows within 24" of the location shown:

BC: 1-10-8,(6)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

9) Lateral bracing shall be attached to each ply.

10) Install screws per manufacturer recommendations.

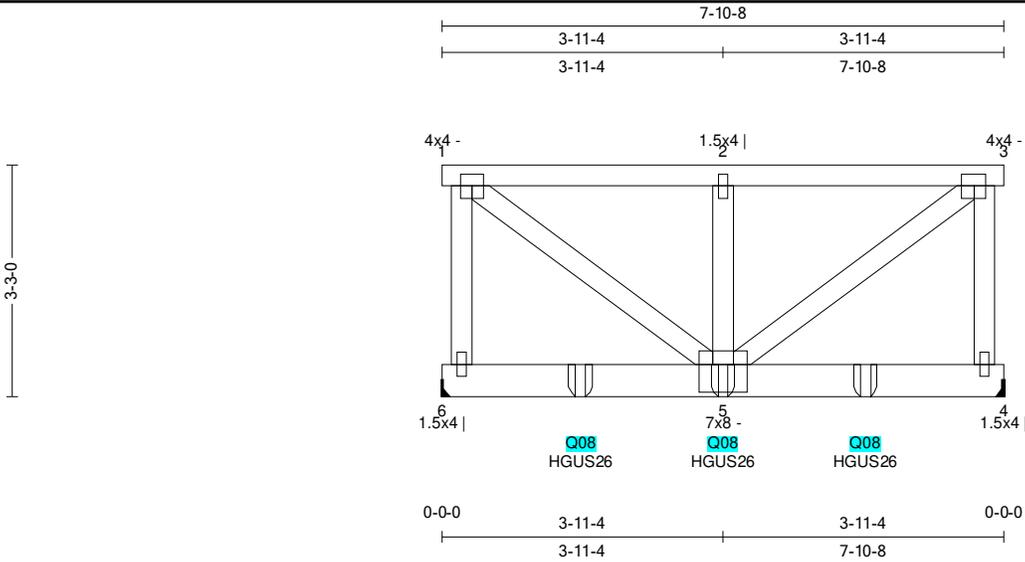
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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TrueBuild® Truss Software v5.8.14
Eagle Metal Products

SPAN 7-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 22.25 in	WGT/PLY 48 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.12 (1-2)	Vert TL: 0.04 in	L/999	(4-5)	L/180
TCLL: 25	TPI 1-2014	BC: 0.31 (4-5)	Vert LL: 0.02 in	L/999	(4-5)	L/240
TCDL: 15	Rep Mbr: No	Web: 0.46 (1-5)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 115%					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	--	2,516 lbs	49 lbs
4	1	1.5 in	--	2,510 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
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- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-9-0	Down	Proj	46.35 plf	46.35 plf	

Load Case Df: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-9-0	Down	Proj	27.81 plf	27.81 plf	
Bot	0-0-0	7-9-0	Down	Proj	11.12 plf	11.12 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.124		(-1,089 lbs)
BC	2-3	0.124		(-1,089 lbs)
Web	1-6	0.173	(900 lbs)	3-5 0.463 1,384 lbs
	1-5	0.463	1,384 lbs	3-4 0.172 (-897 lbs)

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 7-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 22.25 in	WGT/PLY 48 lbs
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Truss to Truss Connection Summary

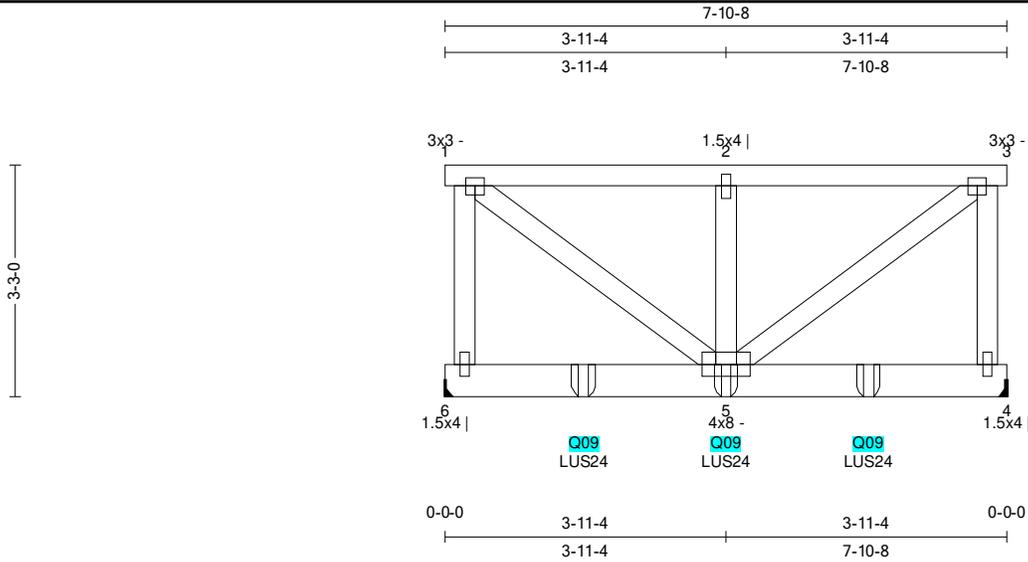
Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
Q08	BC	1-11-4	1555	0	0	HGUS26	8x 16d, 20x 16d
Q08	BC	3-11-4	1555	0	0	HGUS26	8x 16d, 20x 16d
Q08	BC	5-11-4	1555	0	0	HGUS26	8x 16d, 20x 16d

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc - Z(TSLZ278, 2 - ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 1-4-4 oc, Webs - 1 row 0.131"x3" Nails @ 2-0-0 oc.
- 8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 9) Lateral bracing shall be attached to each ply.
- 10) Install screws per manufacturer recommendations.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 7-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 22.25 in	WGT/PLY 47 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.22 (1-2)	Vert TL: 0.01 in	L/999	(4-5)	L/180
TCLL: 25	TPI 1-2014	BC: 0.06 (4-5)	Vert LL: 0.01 in	L/999	(4-5)	L/240
TCDL: 15	Rep Mbr: No	Web: 0.16 (1-5)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 115%					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	511 lbs	.	-4 lbs	-44 lbs	-44 lbs	49 lbs
4	1	1.5 in	---	499 lbs	.	.	-41 lbs	-41 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-9-0	Down	Proj	46.35 plf	46.35 plf	

Load Case Df: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-9-0	Down	Proj	27.81 plf	27.81 plf	
Bot	0-0-0	7-9-0	Down	Proj	11.12 plf	11.12 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.216		(-387 lbs)
BC	2-3	0.216		(-387 lbs)
Web	1-6	0.105	492 lbs	(-430 lbs)
	1-5	0.165	492 lbs	(-420 lbs)

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 7-10-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 22.25 in	WGT/PLY 47 lbs
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Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
Q09	BC	1-11.4	116	21	0	LUS24	2x SD9212, 4x SD9112
Q09	BC	3-11.4	116	21	0	LUS24	2x SD9212, 4x SD9112
Q09	BC	5-11.4	116	21	0	LUS24	2x SD9212, 4x SD9112

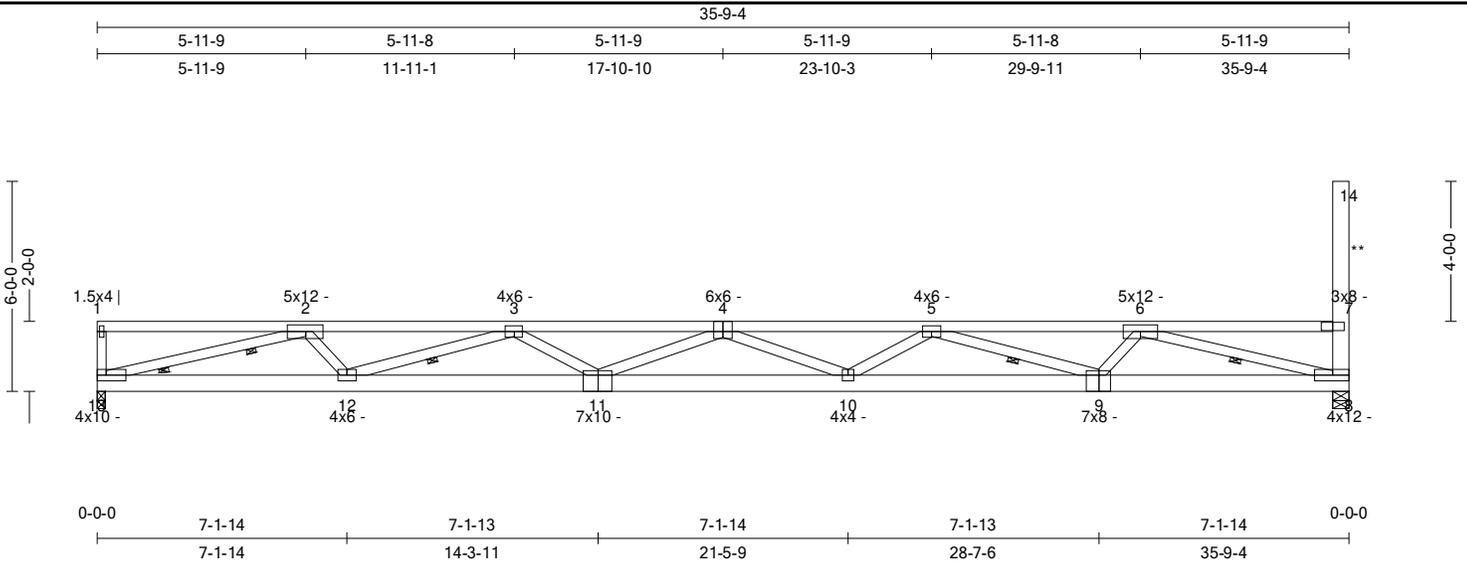
Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



SPAN 35-9-4	PITCH 0/12	QTY 26	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 185 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.76 (3-4)	Vert TL: 1.6 in	L/264	(10-11)	L/180
TCDL: 15	TPI 1-2014	BC: 0.82 (10-11)	Vert LL: 0.87 in	L/486	(10-11)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.98 (6-8)	Horz TL: 0.19 in		8	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.76 in	1,651 lbs	.	.	-78 lbs	-78 lbs	-197 lbs
8	1	5.5 in	1.75 in	1,640 lbs	.	.	-27 lbs	-27 lbs	.

Material

TC: DFL 2400/2.0 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 2-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 3-12, 5-9, 6-8
 Two Third Point Rows: 2-13

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.458	(-5,538 lbs)	4-5	0.761	544 lbs	(-8,314 lbs)	6-7	0.286	476 lbs
	3-4	0.764	(-8,331 lbs)	5-6	0.454	544 lbs	(-5,489 lbs)			
BC	8-9	0.473	4,858 lbs	10-11	0.822	8,856 lbs	(-334 lbs)	12-13	0.480	4,913 lbs
	9-10	0.740	7,832 lbs	11-12	0.743	7,859 lbs				
Web	2-13	0.861	(-5,101 lbs)	4-11	0.237		(-787 lbs)	6-9	0.471	1,067 lbs
	2-12	0.467	1,057 lbs	4-10	0.233		(-773 lbs)	6-8	0.980	(-5,049 lbs)
	3-12	0.410	(-2,451 lbs)	5-10	0.317	719 lbs				
	3-11	0.322	729 lbs	5-9	0.414		(-2,475 lbs)			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **H1**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:18
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-9-4	0/12	26	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	185 lbs

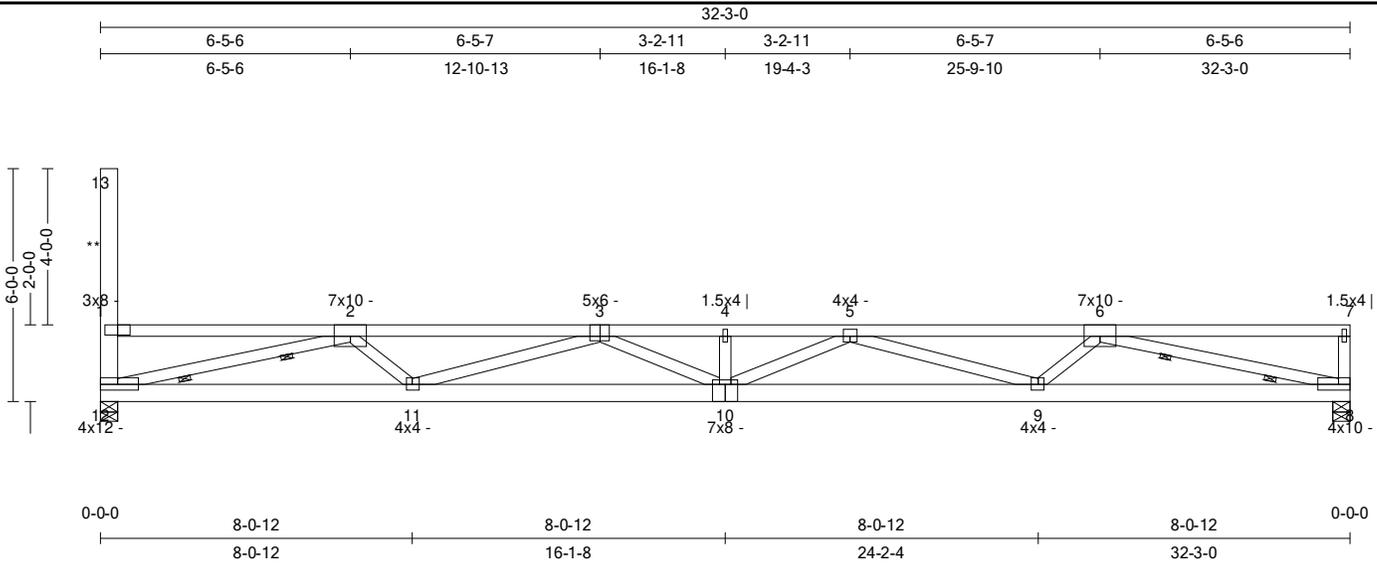
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.48 in, 2L/208 (14-7), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 32-3-0	PITCH 0/12	QTY 32	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 167 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.79 (5-6)	Vert TL: 1.25 in	L / 301	10	L / 180
TCDL : 15	TPI 1-2014	BC : 0.67 (9-10)	Vert LL: 0.68 in	L / 554	10	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.88 (6-8)	Horz TL: 0.14 in		8	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	5.5 in	1.58 in	1,478 lbs	.	.	-35 lbs	-35 lbs	197 lbs
8	1	5.5 in	1.59 in	1,489 lbs	.	.	-91 lbs	-91 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 13-12

Bracing

TC: Sheathed
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: Two Third Point Rows: 2-12, 6-8

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 13-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.645	476 lbs	3-4	0.754	524 lbs	(-7,088 lbs)	5-6	0.788	324 lbs	(-5,257 lbs)
	2-3	0.788	549 lbs	4-5	0.752	524 lbs	(-7,088 lbs)				
BC	8-9	0.469	4,644 lbs	9-10	0.675	6,883 lbs	(-477 lbs)	10-11	0.673	6,870 lbs	(-567 lbs)
				3-10	0.252	570 lbs		6-9	0.384	870 lbs	
Web	2-11	0.390	882 lbs	5-10	0.257	581 lbs		6-8	0.880	319 lbs	(-4,796 lbs)
	3-11	0.858	(-1,741 lbs)	5-9	0.846	(-1,715 lbs)					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSL-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **H2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:18
 Page: 2 of 2

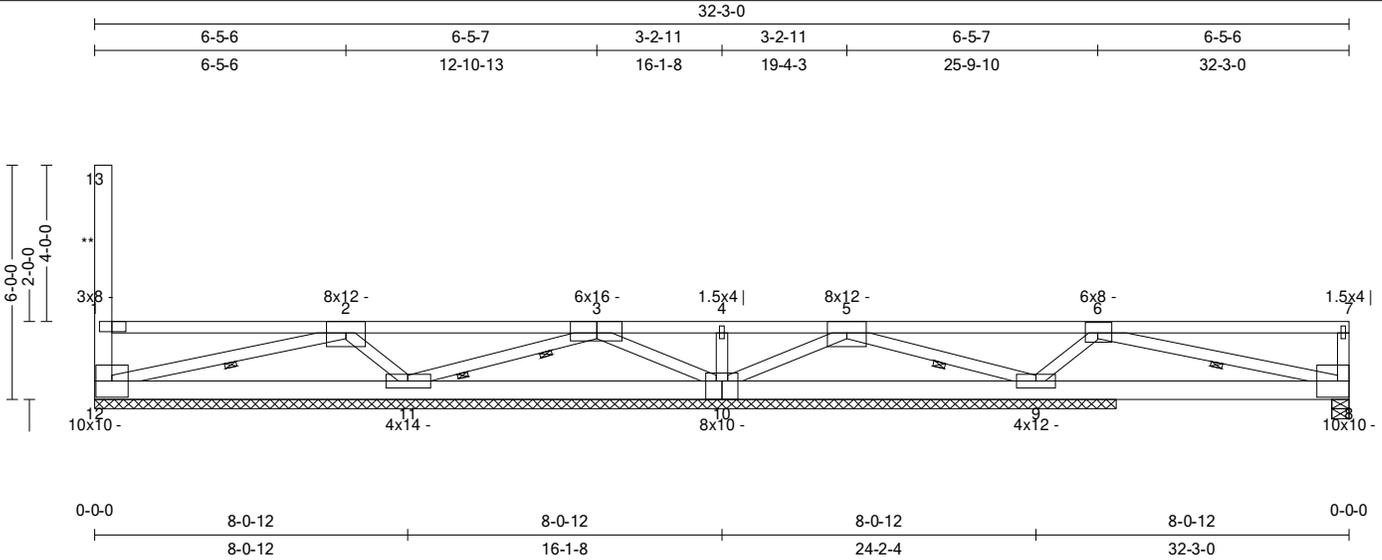
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
32-3-0	0/12	32	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	167 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.39 in, 2L/257 (13-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products

SPAN 32-3-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 176 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.97 (6-7)	Vert TL: 0.04 in	L / 999	(8-9)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.30 (8-9)	Vert LL: 0.02 in	L / 999	(8-9)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.98 (3-11)	Horz TL: 0.03 in		8	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	5.5 in	1.50 in	1,275 lbs	-983 lbs	.	-20 lbs	-983 lbs	.
9	1	314.998 in	N/A	835 lbs	-101 lbs	.	-35 lbs	-101 lbs	-10,361 lbs
10	1	314.998 in	N/A	763 lbs	.	.	-55 lbs	-55 lbs	-5,883 lbs
11	1	314.998 in	N/A	754 lbs	.	.	-70 lbs	-70 lbs	-5,490 lbs
12	1	314.998 in	N/A	1,587 lbs	-1,288 lbs	.	.	-1,288 lbs	5,642 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL SS 2 x 6
 Web: DFL Stud 2 x 4 except:
 DFL #1B 2 x 4: 2-12
 DFL #2 2 x 4: 5-9, 6-8
 DFL SS 2 x 6: 13-12

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
 BC: Sheathed or Purlins at 4-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-12, 5-9, 6-8
 Two Third Point Rows: 3-11

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects due to a 27,090 lbs (840 plf) drag load distributed along the TC rake from each direction.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 13-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.958	5,263 lbs	(-5,263 lbs)	3-4	0.556	3,027 lbs	(-2,875 lbs)	5-6	0.951	5,502 lbs	(-5,338 lbs)
	2-3	0.918	3,835 lbs	(-3,753 lbs)	4-5	0.503	2,581 lbs	(-2,430 lbs)	6-7	0.970	5,457 lbs	(-5,457 lbs)
BC	8-9	0.302	4,482 lbs	(-4,258 lbs)								
Web	2-12	0.908	5,536 lbs	(-5,831 lbs)	3-10	0.817	2,805 lbs	(-3,247 lbs)	6-9	0.317	1,319 lbs	(-1,870 lbs)
	2-11	0.424	1,763 lbs	(-2,286 lbs)	5-10	0.942	3,340 lbs	(-3,745 lbs)	6-8	0.849	4,397 lbs	(-4,629 lbs)
	3-11	0.984	4,093 lbs	(-4,437 lbs)	5-9	0.681	4,826 lbs	(-5,221 lbs)				

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **H2-D**
Job: **2501272**
Date: 01/23/26 09:23:18
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
32-3-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	176 lbs

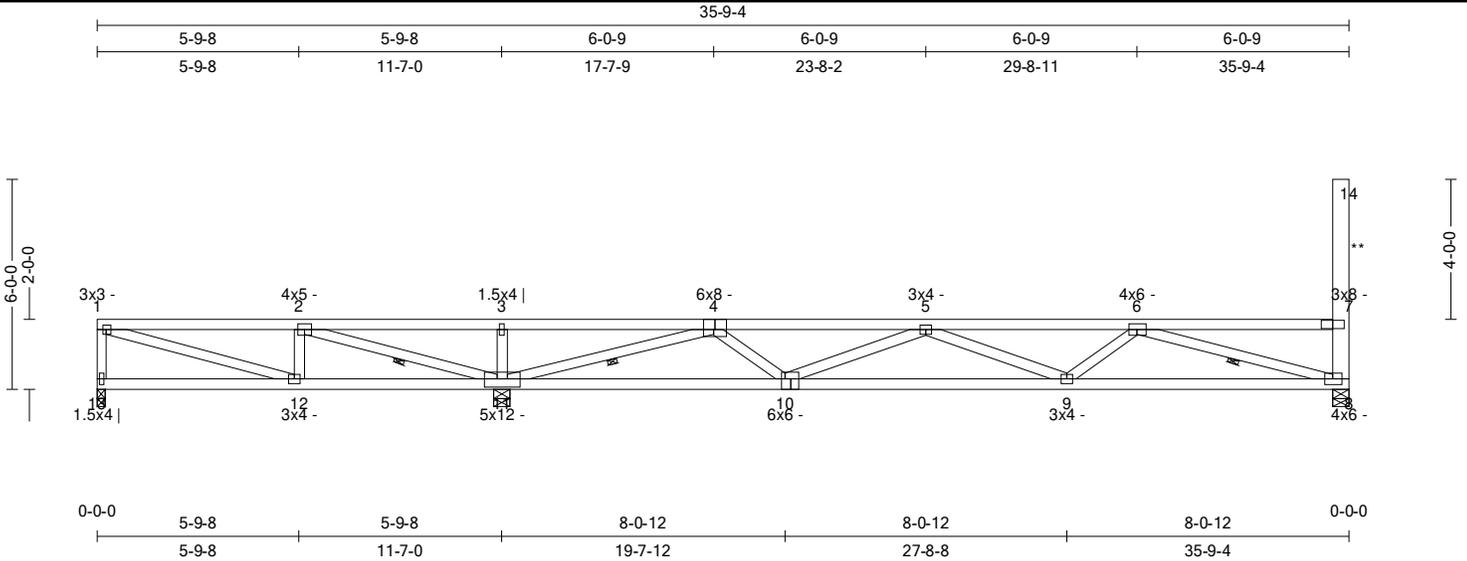
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 9, 12 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.16 in, 2L/606 (13-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products



SPAN 35-9-4	PITCH 0/12	QTY 16	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 158 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.80 (2-3)	Vert TL: 0.42 in	L/ 666	(9-10)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.72 (9-10)	Vert LL: 0.23 in	L/ 999	(9-10)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.76 (4-11)	Horz TL: 0.07 in		8	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.50 in	358 lbs	-89 lbs	.	-9 lbs	-89 lbs	-197 lbs
11	1	5.5 in	2.31 in	2,163 lbs	.	.	-103 lbs	-103 lbs	.
8	1	5.5 in	1.50 in	952 lbs

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 3-10-0, Purlin design by Others.
 BC: Sheathed or Purlins at 7-8-0, Purlin design by Others.
 Web: One Midpoint Row: 2-11, 4-11, 6-8

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.402	658 lbs	(-430 lbs)	3-4	0.797	2,124 lbs	5-6	0.362	375 lbs	(-2,660 lbs)
	2-3	0.797	2,124 lbs		4-5	0.342		6-7	0.405	463 lbs	
BC	8-9	0.669	2,437 lbs		10-11	0.550	1,364 lbs				
	9-10	0.715	2,864 lbs		11-12	0.318	430 lbs				
Web	1-13	0.053		(-324 lbs)	4-11	0.755		6-8	0.507		(-2,540 lbs)
	1-12	0.411	449 lbs	(-688 lbs)	4-10	0.369	836 lbs				
	2-11	0.402		(-2,056 lbs)	5-10	0.390					
	3-11	0.066		(-517 lbs)	6-9	0.179	405 lbs				

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **H4**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:18
 Page: 2 of 2

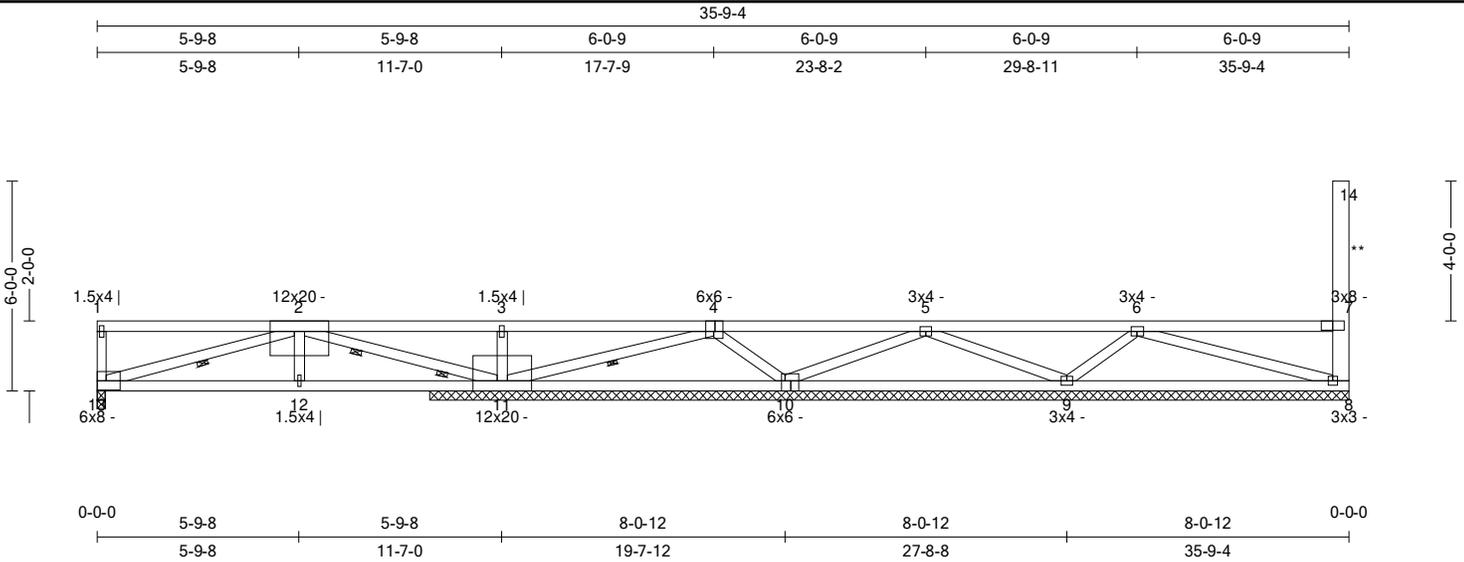
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-9-4	0/12	16	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	158 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.16 in, 2L/610 (14-7), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products

SPAN 35-9-4	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 167 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.73 (2-3)	Vert TL: 0.27 in	L/ 417	(12-13)	L/ 180
TCDL : 15	TPI 1-2014	BC : 0.86 (11-12)	Vert LL: 0.24 in	L/ 457	(12-13)	L/ 240
BCLL : 0	Rep Mbr : No	Web : 0.96 (2-11)	Horz TL: 0.22 in		8	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.57 in	1,580 lbs	-1,136 lbs	.	-30 lbs	-1,136 lbs	15,560 lbs
8	1	315 in	N/A	440 lbs	-176 lbs	.	.	-176 lbs	.
9	1	315 in	N/A	825 lbs	.	.	-71 lbs	-71 lbs	.
10	1	315 in	N/A	750 lbs	.	.	-33 lbs	-33 lbs	.
11	1	315 in	N/A	1,136 lbs	-253 lbs	.	-27 lbs	-253 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL 2400/2.0 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL #2 2 x 4: 2-13
 DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 2-4-0, Purlin design by Others.
 BC: Sheathed
 Web: One Midpoint Row: 2-13, 4-11
 Two Third Point Rows: 2-11

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 15,560 lbs (435 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has not been designed for the effects of unbalanced snow loads.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 7) ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.574	2,536 lbs	(-2,536 lbs)	3-4	0.665	3,981 lbs	(-3,823 lbs)	5-6	0.606	3,911 lbs	(-3,644 lbs)
	2-3	0.730	6,517 lbs	(-6,358 lbs)	4-5	0.558	4,733 lbs	(-4,432 lbs)	6-7	0.557	2,547 lbs	(-2,547 lbs)
BC	8-9	0.296	1,129 lbs	(-973 lbs)	10-11	0.417	4,141 lbs	(-4,062 lbs)	12-13	0.844	11,275 lbs	(-10,509 lbs)
	9-10	0.294	2,422 lbs	(-2,297 lbs)	11-12	0.858	11,275 lbs	(-10,509 lbs)				
Web	2-13	0.794	4,477 lbs	(-5,277 lbs)	4-10	0.237	843 lbs	(-1,327 lbs)	6-8	0.764	1,014 lbs	(-1,176 lbs)
	2-11	0.964	4,007 lbs	(-4,970 lbs)	5-10	0.445	741 lbs	(-1,206 lbs)				
	3-11	0.097	(-497 lbs)		5-9	0.466	811 lbs	(-1,237 lbs)				
	4-11	0.602	2,459 lbs	(-2,706 lbs)	6-9	0.165	(-756 lbs)					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	167 lbs

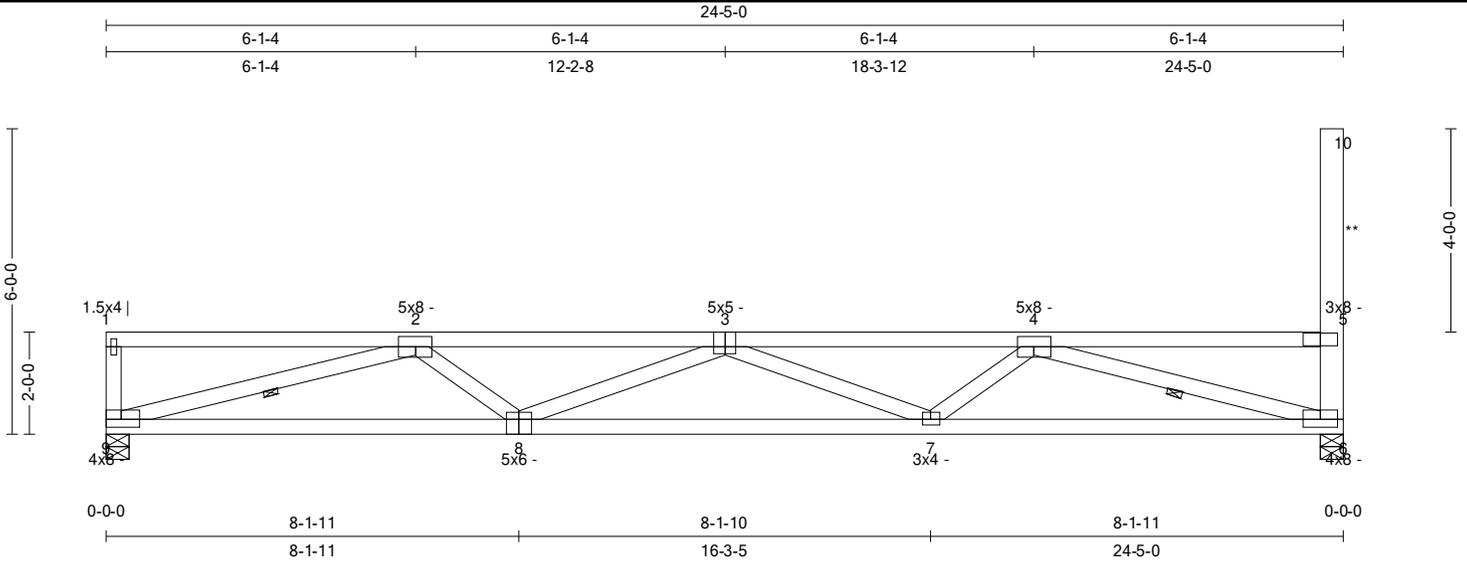
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 13, 8, 11 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.15 in, 2L/651 (14-7), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.
This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products



SPAN 24-5-0	PITCH 0/12	QTY 11	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 111 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.61 (1-2)	Vert TL: 0.57 in	L/494	(7-8)	L/180
TCDL: 15	TPI 1-2014	BC: 0.85 (7-8)	Vert LL: 0.31 in	L/924	(7-8)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.66 (2-9)	Horz TL: 0.12 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,129 lbs	.	.	-117 lbs	-117 lbs	-197 lbs
6	1	5.5 in	1.50 in	1,118 lbs	.	.	-44 lbs	-44 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-9, 4-6

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.543	372 lbs	(-3,444 lbs)	3-4	0.535	521 lbs	(-3,423 lbs)	4-5	0.600	465 lbs
BC	6-7	0.724	3,016 lbs	(-353 lbs)	7-8	0.848	3,998 lbs		8-9	0.735	3,046 lbs
Web	2-9	0.664	360 lbs	(-3,169 lbs)	3-7	0.242		(-644 lbs)			
	2-8	0.253	572 lbs		4-7	0.255	578 lbs				
	3-8	0.248		(-658 lbs)	4-6	0.634		(-3,141 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **H5**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:18
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
24-5-0	0/12	11	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	111 lbs

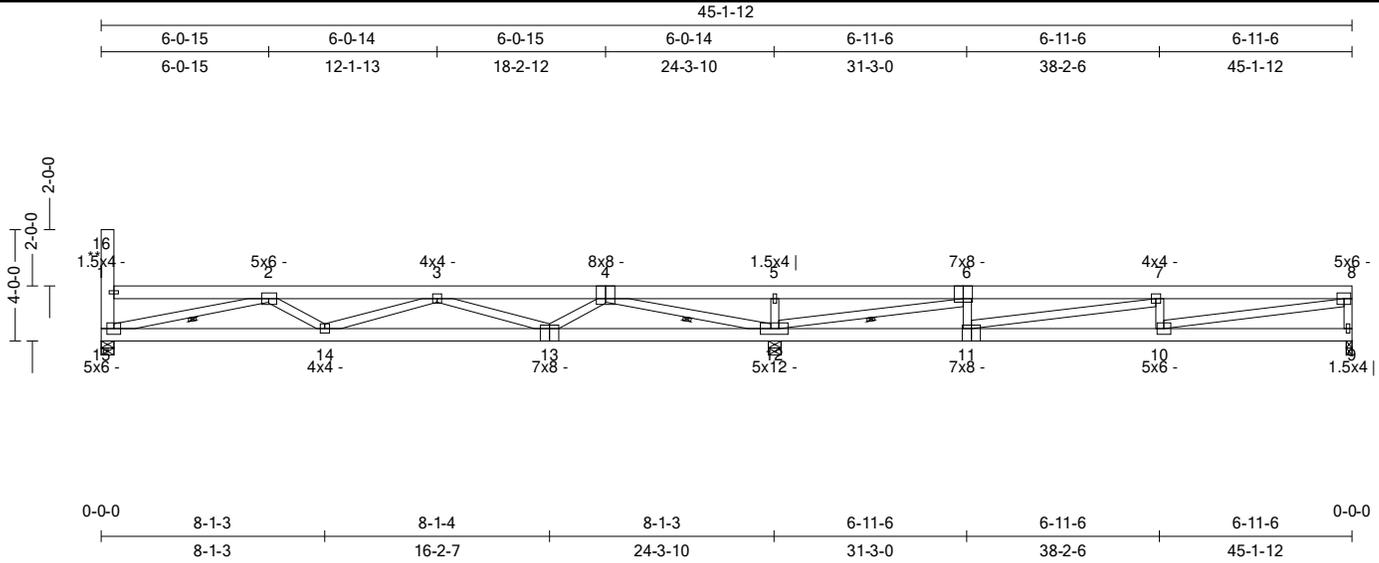
9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.21 in, 2L/466 (10-5), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



SPAN 45-1-12	PITCH 0/12	QTY 10	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 255 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.61 (4-5)	Vert TL: 0.34 in	L/ 827	(13-14)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.34 (13-14)	Vert LL: 0.2 in	L/ 999	(13-14)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.98 (7-11)	Horz TL: 0.05 in		9	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	961 lbs	.	.	-4 lbs	-4 lbs	96 lbs
12	1	5.5 in	2.66 in	2,497 lbs	.	.	-81 lbs	-81 lbs	.
9	1	2.75 in	1.50 in	813 lbs	.	.	-18 lbs	-18 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 8-10
 DFL SS 2 x 6: 16-15

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-15, 4-12, 6-12

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- ** - Indicates parapet wind loading has been applied to member 16-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.155	(-2,884 lbs)	4-5	0.608	2,388 lbs	6-7	0.183	(-1,508 lbs)
	3-4	0.162	(-2,047 lbs)	5-6	0.608	2,388 lbs	7-8	0.224	(-2,257 lbs)
BC	10-11	0.271	2,257 lbs	12-13	0.308	1,439 lbs	14-15	0.264	2,619 lbs
	11-12	0.297	1,471 lbs	13-14	0.340	3,157 lbs			
Web	2-15	0.535	(-2,709 lbs)	4-12	0.734	(-3,469 lbs)	7-11	0.977	(-1,134 lbs)
	2-14	0.183	414 lbs	5-12	0.088	(-713 lbs)	7-10	0.054	(-439 lbs)
	3-13	0.477	(-1,331 lbs)	6-12	0.804	(-3,253 lbs)	8-10	0.444	2,314 lbs
	4-13	0.365	826 lbs	6-11	0.149	336 lbs	8-9	0.094	(-758 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **K1**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:19
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
45-1-12	0/12	10	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	255 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.06 in, 2L/921 (16-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

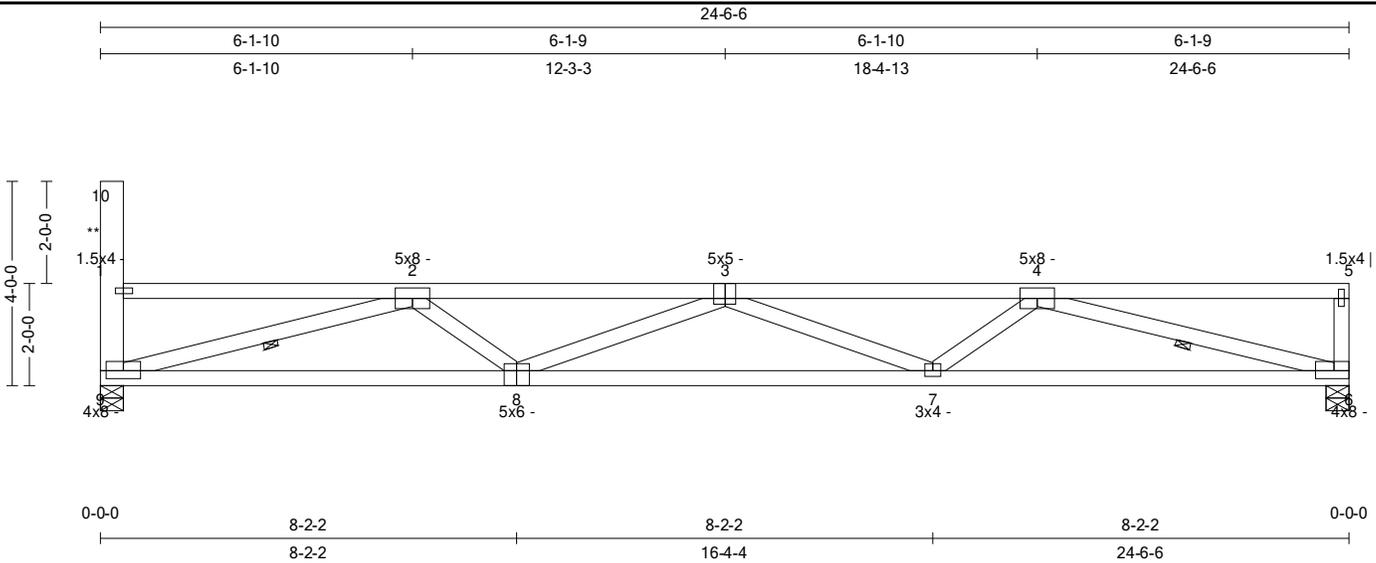
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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: K2
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:19
 Page: 1 of 2

SPAN 24-6-6	PITCH 0/12	QTY 10	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 107 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.62 (4-5)	Vert TL: 0.58 in	L/ 487	(7-8)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.86 (7-8)	Vert LL: 0.31 in	L/ 912	(7-8)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.67 (4-6)	Horz TL: 0.12 in		6	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,123 lbs	.	.	-64 lbs	-64 lbs	96 lbs
6	1	5.5 in	1.50 in	1,134 lbs	.	.	-97 lbs	-97 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 10-9

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-9, 4-6

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 10-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.541	330 lbs	(-3,456 lbs)	3-4	0.550	(-3,476 lbs)			
BC	6-7	0.742	3,075 lbs		7-8	0.856	4,036 lbs (-355 lbs)	8-9	0.731	3,045 lbs (-336 lbs)
Web	2-9	0.644		(-3,170 lbs)	3-7	0.252	(-664 lbs)			
	2-8	0.257	582 lbs		4-7	0.255	576 lbs			
	3-8	0.246		(-650 lbs)	4-6	0.674	(-3,198 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **K2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:19
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
24-6-6	0/12	10	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	107 lbs

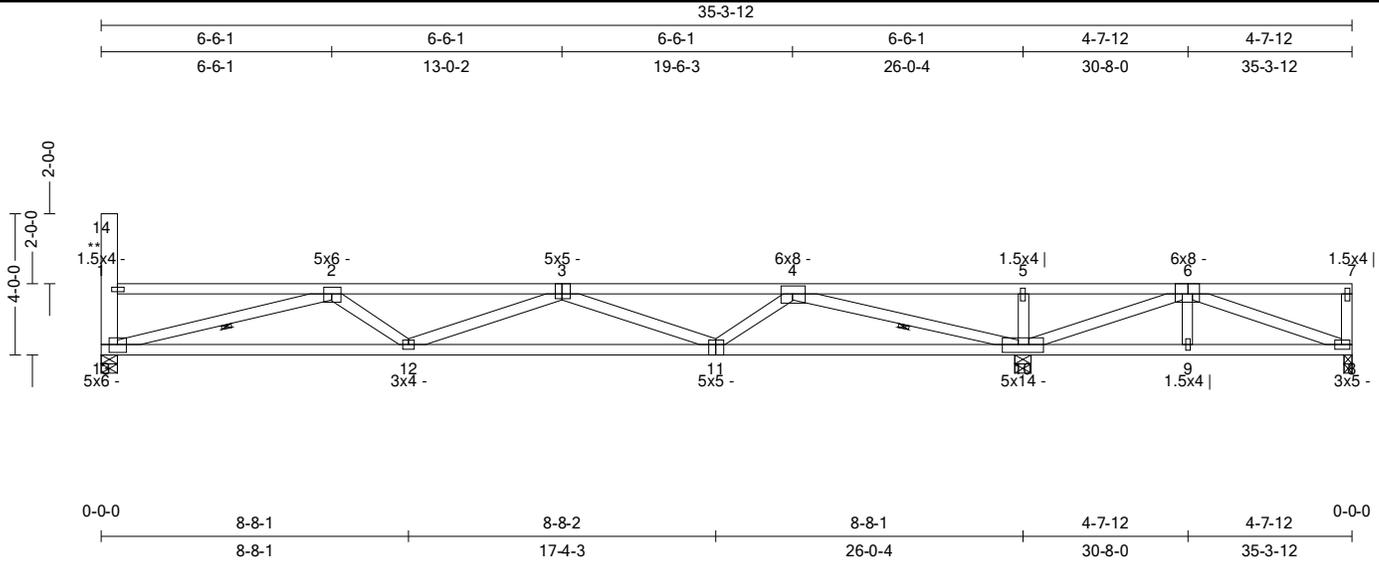
9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.11 in, 2L/460 (10-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 35-3-12	PITCH 0/12	QTY 29	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 153 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.87 (5-6)	Vert TL: 0.56 in	L/ 545	(11-12)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.83 (11-12)	Vert LL: 0.29 in	L/ 999	(11-12)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.98 (4-10)	Horz TL: 0.09 in		10	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	5.5 in	1.50 in	1,011 lbs	.	.	-15 lbs	-15 lbs	96 lbs
10	1	5.5 in	2.50 in	2,344 lbs	.	.	-101 lbs	-101 lbs	.
8	1	2.75 in	1.50 in	177 lbs	-325 lbs	-35 lbs	.	-325 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 14-13

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 5-10-0, Purlin design by Others.
 Web: One Midpoint Row: 2-13, 4-10

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 14-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.428	(-3,022 lbs)	4-5	0.871	2,605 lbs			
	3-4	0.371	(-2,098 lbs)	5-6	0.871	2,605 lbs			
BC	8-9	0.141	(-1,106 lbs)	10-11	0.590	1,424 lbs	12-13	0.786	2,781 lbs
	9-10	0.342	(-1,106 lbs)	11-12	0.834	3,219 lbs			
Web	2-13	0.636	(-2,882 lbs)	4-10	0.975	(-4,123 lbs)			
	2-12	0.197	447 lbs	5-10	0.065	(-508 lbs)			
	3-11	0.552	(-1,323 lbs)	6-10	0.819	(-1,950 lbs)			
	4-11	0.413	934 lbs	6-8	0.523	1,183 lbs			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **L1**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:19
 Page: 2 of 2

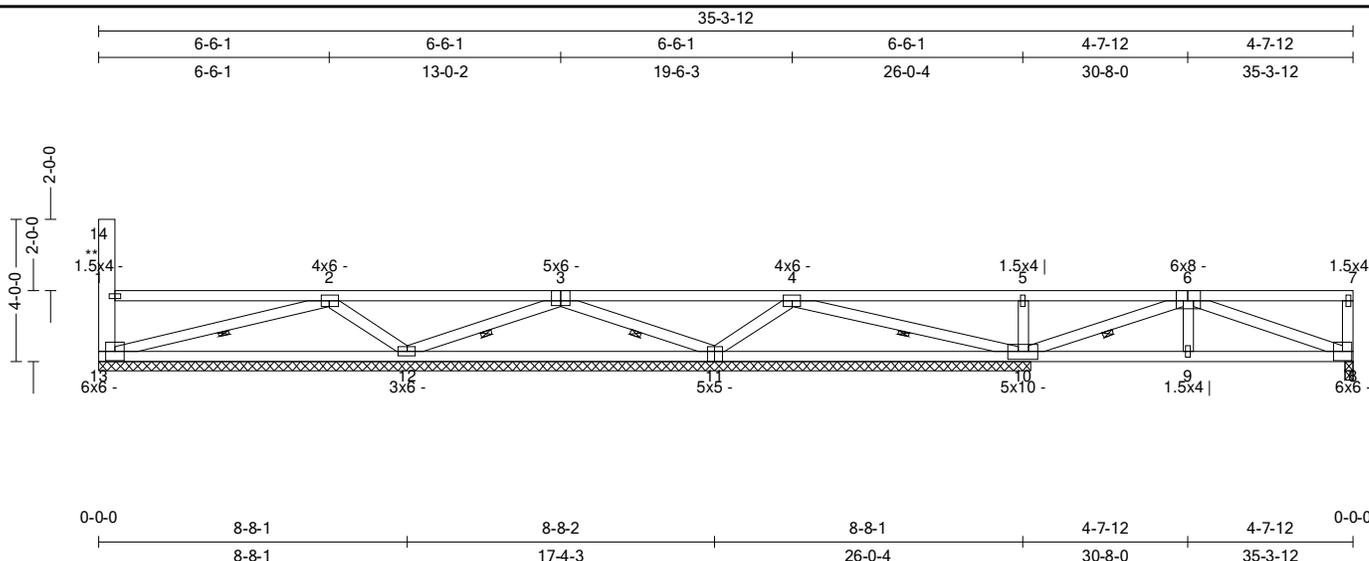
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-3-12	0/12	29	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	153 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 8 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.1 in, 2L/524 (14-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 35-3-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 152 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.91 (2-3)	Vert TL: 0.22 in	L / 999	(12-13)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.61 (11-12)	Vert LL: 0.14 in	L / 999	(12-13)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.95 (6-8)	Horz TL: 0.03 in		8	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	2.75 in	1.50 in	937 lbs	-581 lbs	-	-17 lbs	-581 lbs	-
10	1	315 in	N/A	868 lbs	-	-	-29 lbs	-29 lbs	-6,851 lbs
11	1	315 in	N/A	801 lbs	-	-	-36 lbs	-36 lbs	-3,020 lbs
12	1	315 in	N/A	855 lbs	-	-	-40 lbs	-40 lbs	3,011 lbs
13	1	315 in	N/A	875 lbs	-564 lbs	-	-	-564 lbs	2,782 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL SS 2 x 6: 14-13

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
 Web: One Midpoint Row: 2-13, 3-12, 3-11, 4-10, 6-10

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 15,361 lbs (435 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has not been designed for the effects of unbalanced snow loads.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 7) ** - Indicates parapet wind loading has been applied to member 14-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.865	2,748 lbs	(-2,748 lbs)	3-4	0.809	2,228 lbs	(-2,043 lbs)	5-6	0.640	2,077 lbs	(-2,022 lbs)
	2-3	0.909	2,128 lbs	(-1,955 lbs)	4-5	0.773	2,861 lbs	(-2,805 lbs)	6-7	0.359	2,034 lbs	(-2,034 lbs)
BC	8-9	0.376	2,203 lbs	(-1,731 lbs)								
	9-10	0.462	2,203 lbs	(-1,731 lbs)								
Web	2-13	0.660	2,589 lbs	(-2,883 lbs)	4-11	0.262	872 lbs	(-1,405 lbs)	6-8	0.949	1,852 lbs	(-2,356 lbs)
	2-12	0.272	875 lbs	(-1,455 lbs)	4-10	0.707	2,592 lbs	(-2,890 lbs)				
	3-12	0.478	1,987 lbs	(-2,498 lbs)	5-10	0.092		(-475 lbs)				
	3-11	0.477	1,985 lbs	(-2,509 lbs)	6-10	0.475	1,974 lbs	(-2,536 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
35-3-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs

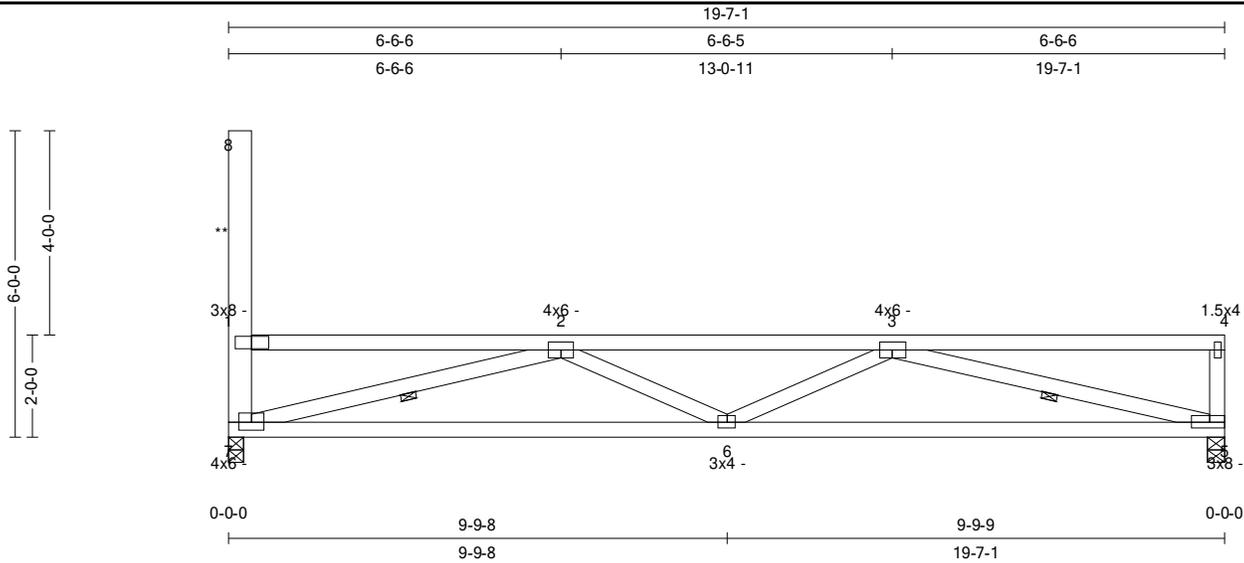
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 13 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.
- 11) Parapet TL: 0.05 in, 2L/999 (14-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 19-7-1	PITCH 0/12	QTY 16	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 89 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.66 (3-4)	Vert TL: 0.44 in	L/ 522	(5-6)	L/ 180
TCDL: 15	TPI 1-2014	BC: 1.00 (5-6)	Vert LL: 0.23 in	L/ 981	(5-6)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.56 (3-5)	Horz TL: 0.06 in		5	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	896 lbs	.	.	-43 lbs	-43 lbs	197 lbs
5	1	4 in	1.50 in	906 lbs	.	.	-133 lbs	-133 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Standard 2 x 4 except:
 DFL SS 2 x 6: 8-7

Bracing

TC: Sheathed or Purlins at 3-8-0, Purlin design by Others.
 BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.
 Web: One Midpoint Row: 2-7, 3-5

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) ** - Indicates parapet wind loading has been applied to member 8-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.641	466 lbs	2-3	0.562	407 lbs	(-2,484 lbs)
BC	5-6	0.999	2,364 lbs	6-7	0.997	2,347 lbs	(-525 lbs)
Web	2-7	0.539	(-2,431 lbs)	3-6	0.177	402 lbs	
	2-6	0.182	412 lbs	3-5	0.563	409 lbs	(-2,448 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **NI**
 Job: **2501272**
 Designer:Anthony
 Date: 01/23/26 09:23:19
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
19-7-1	0/12	16	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	89lbs

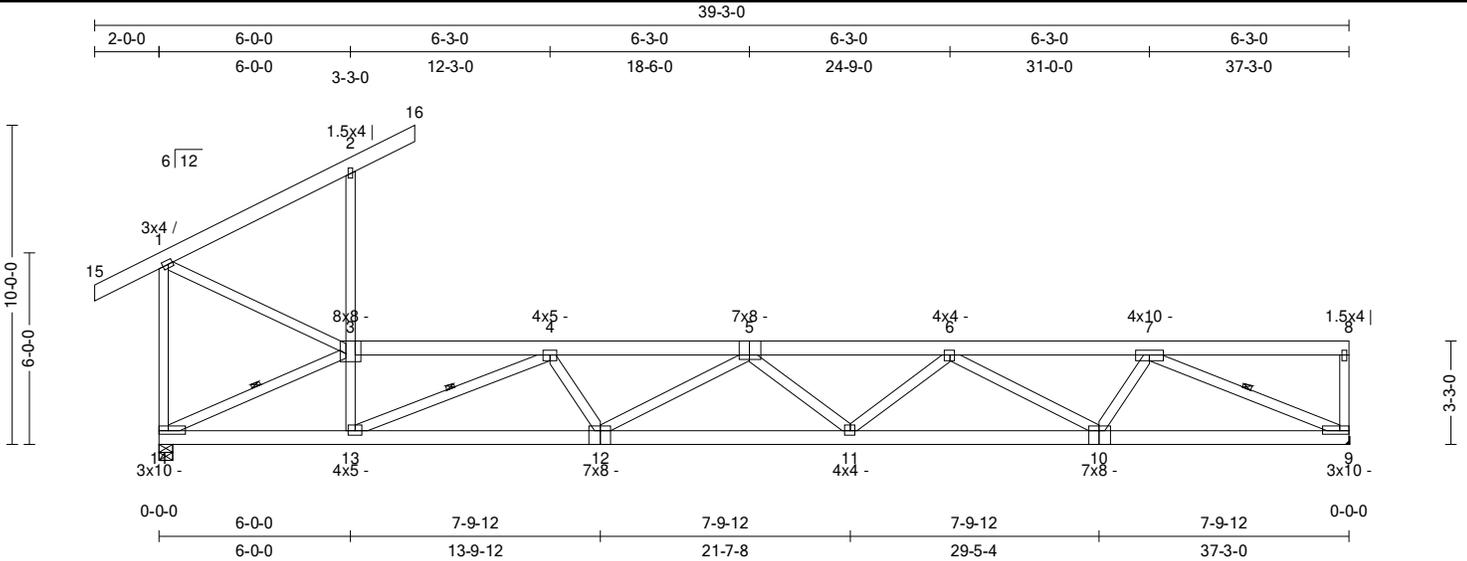
9) Listed wind uplift reactions based on MWFRS & C&C loading.
 10) Parapet TL: 0.17 in, 2L/572 (8-1), Allowable 2L/120.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 37-3-0	PITCH 6/12	QTY 7	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 247 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.30 (5-6)	Vert TL: 0.62 in	L / 712	(11-12)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.50 (11-12)	Vert LL: 0.33 in	L / 999	(11-12)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.88 (6-10)	Horz TL: 0.13 in		9	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	2.14 in	2,009 lbs	.	.	-214 lbs	-214 lbs	243 lbs
9	1	1.5 in	---	1,738 lbs	.	.	-133 lbs	-133 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 2-13

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-6-0, Purlin design by Others.
 Web: One Midpoint Row: 3-14, 4-13, 7-9

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.211	942 lbs	(-3,267 lbs)	5-6	0.298	610 lbs	(-5,530 lbs)
	4-5	0.291	833 lbs	(-5,404 lbs)	6-7	0.201	340 lbs	(-3,732 lbs)
BC	9-10	0.295	3,210 lbs		11-12	0.501	5,812 lbs	(-681 lbs)
	10-11	0.447	5,142 lbs	(-487 lbs)	12-13	0.457	5,217 lbs	(-838 lbs)
					13-14	0.279	3,255 lbs	(-882 lbs)
Web	1-14	0.322		(-416 lbs)	4-13	0.507		(-2,135 lbs)
	3-14	0.768	765 lbs	(-3,606 lbs)	4-12	0.233	527 lbs	
	2-3	0.216		(-388 lbs)	5-12	0.386		(-717 lbs)
	3-13	0.269	950 lbs		5-11	0.159		(-504 lbs)
					6-11	0.285	645 lbs	
					6-10	0.883		(-1,641 lbs)
					7-10	0.472	1,068 lbs	
					7-9	0.813	306 lbs	(-3,530 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Provide adequate drainage to prevent ponding.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **Q02**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:19
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
37-3-0	6/12	7	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	247 lbs

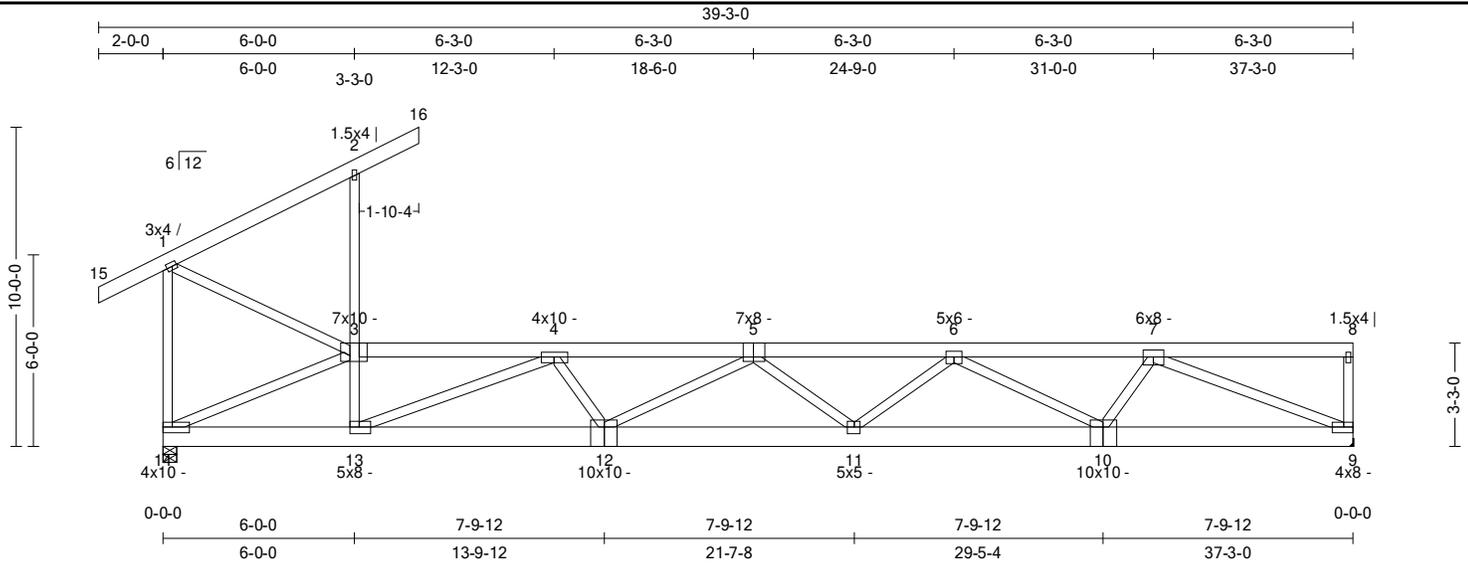
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 37-3-0	PITCH 6/12	QTY 6	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 24 in	WGT/PLY 273 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.41 (5-6)	Vert TL: 0.75 in	L/ 586	(11-12)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.61 (11-12)	Vert LL: 0.16 in	L/ 999	(11-12)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.95 (7-9)	Horz TL: 0.13 in		9	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	2.23 in	4,182 lbs	.	.	-214 lbs	-214 lbs	243 lbs
9	1	1.5 in	---	3,698 lbs	.	.	-133 lbs	-133 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 8
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 5-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	37-3-0	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	37-3-0	Down	Proj	6 psf	6 psf	24 in
Top	7-8-0	28-0-0	Down	Proj	100 psf	100 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **Q02A**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:19
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
37-3-0	6/12	6	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	273 lbs
7	D1 + S3				1.000				
8	D2 + S1				1.000				
9	D2 + S2				1.000				
10	D2 + S3				1.000				
11	0.60 D1 + 0.60 W1 [Uplift]				1.000				
12	0.60 D1 + 0.60 W2 [Uplift]				1.000				
13	0.60 D1 + 0.60 W4 [Uplift]				1.000				
14	0.60 D1 + 0.60 W8 [Uplift]				1.000				
15	0.60 D2 + 0.60 W1 [Uplift]				1.000				
16	0.60 D2 + 0.60 W2 [Uplift]				1.000				
17	0.60 D2 + 0.60 W4 [Uplift]				1.000				
18	0.60 D2 + 0.60 W8 [Uplift]				1.000				
19	D1 + L10*1				1.000				
20	D2 + L10*1				1.000				
21	D1 + I1				1.000				
22	D2 + I1				1.000				
23	D1 + UR1				1.000				
24	D1 + UR2				1.000				
25	D2 + UR1				1.000				
26	D2 + UR2				1.000				
27	D1 + AS10*1				1.000				
28	D2 + AS10*1				1.000				

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.313	474 lbs	(-3,993 lbs)	5-6	0.406	310 lbs	(-7,510 lbs)
	4-5	0.397	425 lbs	(-7,342 lbs)	6-7	0.247		(-4,593 lbs)
BC	9-10	0.295	3,801 lbs		11-12	0.612	8,049 lbs	(-347 lbs)
	10-11	0.517	6,917 lbs		12-13	0.540	7,092 lbs	(-427 lbs)
Web	3-14	0.923	384 lbs	(-4,388 lbs)	5-12	0.165		(-939 lbs)
	3-13	0.859	1,381 lbs		5-11	0.109		(-777 lbs)
	4-13	0.794		(-3,379 lbs)	6-11	0.375	850 lbs	
	4-12	0.253	574 lbs		6-10	0.472		(-2,686 lbs)

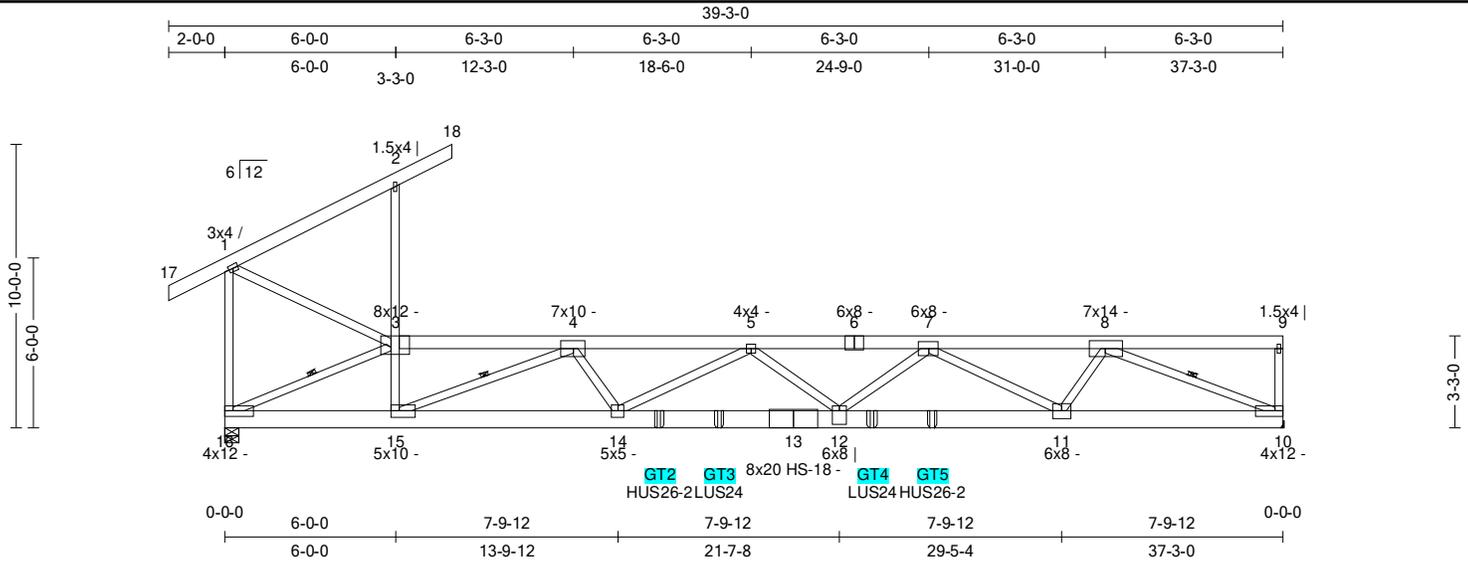
Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row @ 2-0-0 oc.
- 9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 10) Lateral bracing shall be attached to each ply.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products

SPAN 37-3-0	PITCH 6/12	QTY 2	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 23.63 in	WGT/PLY 288 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.84 (5-7)	Vert TL: 1.14 in	L/386	(13-14)	L/180
TCLL: 25	TPI 1-2014	BC: 0.87 (12-14)	Vert LL: 0.22 in	L/999	(13-14)	L/240
TCDL: 15	Rep Mbr: No	Web: 0.96 (3-16)	Horz TL: 0.19 in		10	
BCLL: 0	Lumber D.O.L.: 115%					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	2.38 in	5,752 lbs			-16 lbs	-16 lbs	231 lbs
10	1	1.5 in	--	5,473 lbs					

Material

TC: DFL SS 2 x 6
 BC: SYP 2400/2.0 2 x 8
 Web: DFL Stud 2 x 4 except:
 DFL #1B 2 x 4: 8-10
 DFL #2 2 x 4: 3-16, 2-15, 4-15

Bracing

TC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 3-16, 4-15, 8-10

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	37-3-0	Down	Proj	25 plf	25 plf	
Top	-2-0-0	15-2-8	Down	Proj	24.22 plf	24.22 plf	
Top	17-4-12	22-9-4	Down	Proj	24.22 plf	24.22 plf	
Top	24-11-8	37-3-0	Down	Proj	24.22 plf	24.22 plf	

SPAN 37-3-0	PITCH 6/12	QTY 2	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 23.63 in	WGT/PLY 288 lbs
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Load Case **D1**: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	37-3-0	Down	Proj	15 plf	15 plf	
Top	-2-0-0	15-2-8	Down	Proj	14.53 plf	14.53 plf	
Top	17-4-12	22-9-4	Down	Proj	14.53 plf	14.53 plf	
Top	24-11-8	37-3-0	Down	Proj	14.53 plf	14.53 plf	
Bot	0-0-0	37-3-0	Down	Proj	6 plf	6 plf	
Bot	0-0-0	15-2-8	Down	Proj	5.81 plf	5.81 plf	
Bot	17-4-12	22-9-4	Down	Proj	5.81 plf	5.81 plf	
Bot	24-11-8	37-3-0	Down	Proj	5.81 plf	5.81 plf	

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	37-3-0	Down	Proj	15 plf	15 plf	
Top	-2-0-0	15-2-8	Down	Proj	14.53 plf	14.53 plf	
Top	17-4-12	22-9-4	Down	Proj	14.53 plf	14.53 plf	
Top	24-11-8	37-3-0	Down	Proj	14.53 plf	14.53 plf	
Bot	0-0-0	37-3-0	Down	Proj	6 plf	6 plf	
Bot	0-0-0	15-2-8	Down	Proj	5.81 plf	5.81 plf	
Bot	17-4-12	22-9-4	Down	Proj	5.81 plf	5.81 plf	
Bot	24-11-8	37-3-0	Down	Proj	5.81 plf	5.81 plf	
Top	7-8-0	28-0-0	Down	Proj	100 psf	100 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	15-3-4	Down	216 lbs	
Bot	17-4-12	Down	98 lbs	
Bot	22-9-4	Down	98 lbs	
Bot	24-10-12	Down	174 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000
7	D1 + S3	1.000
8	D2 + S1	1.000
9	D2 + S2	1.000
10	D2 + S3	1.000
11	0.60 D1 + 0.60 W1 [Uplift]	1.000
12	0.60 D1 + 0.60 W2 [Uplift]	1.000
13	0.60 D1 + 0.60 W4 [Uplift]	1.000
14	0.60 D1 + 0.60 W8 [Uplift]	1.000
15	0.60 D2 + 0.60 W1 [Uplift]	1.000
16	0.60 D2 + 0.60 W2 [Uplift]	1.000
17	0.60 D2 + 0.60 W4 [Uplift]	1.000
18	0.60 D2 + 0.60 W8 [Uplift]	1.000
19	D1 + L10*1	1.000
20	D2 + L10*1	1.000
21	D1 + I1	1.000
22	D2 + I1	1.000
23	D1 + UR1	1.000
24	D1 + UR2	1.000
25	D2 + UR1	1.000
26	D2 + UR2	1.000
27	D1 + AS10*1	1.000
28	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.459	(-5,643 lbs)	5-7	0.840	(-12,048 lbs)
	4-5	0.758	(-11,391 lbs)	7-8	0.426	(-7,167 lbs)
BC	10-11	0.263	5,852 lbs	12-14	0.875	12,365 lbs
	11-12	0.813	10,636 lbs	14-15	0.539	10,646 lbs
Web	3-16	0.961	(-6,201 lbs)	5-14	0.271	(-1,183 lbs)
	3-15	0.567	2,139 lbs	5-12	0.110	(-539 lbs)
	4-15	0.920	(-5,455 lbs)	7-12	0.657	1,871 lbs
	4-14	0.526	1,495 lbs	7-11	0.920	(-4,011 lbs)
				8-11	0.938	2,640 lbs
				8-10	0.929	(-6,406 lbs)

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 37-3-0	PITCH 6/12	QTY 2	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 23.63 in	WGT/PLY 288 lbs
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Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
GT2	BC	15-3-4	1182	0	0	HUS26-2	4x SD10212, 4x SD10212
GT3	BC	17-4-12	435	0	0	LUS24	2x SD9212, 4x SD9112
GT4	BC	22-9-4	435	0	0	LUS24	2x SD9212, 4x SD9112
GT5	BC	24-10-12	961	0	0	HUS26-2	4x SD10212, 4x SD10212

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 10) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 11) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc - Z(TSLZ278, 2 - ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row 0.131"x3" Nails @ 2-0-0 oc.

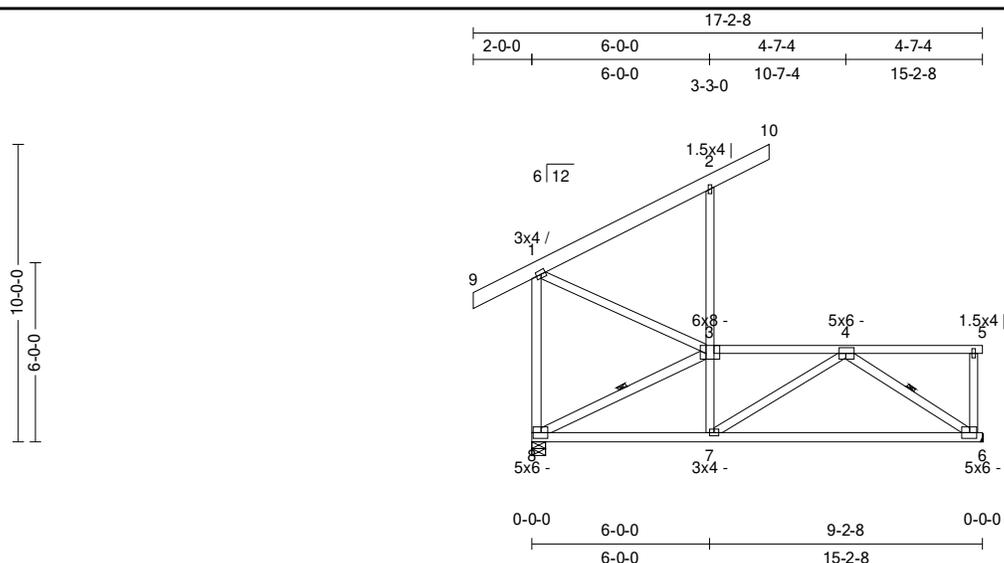
Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2 - ply) Screws as follows within 24" of the location shown:

- BC: 15-3-4,(4)Connectors
- BC: 17-4-12,(2)Connectors
- BC: 22-9-4,(2)Connectors
- BC: 24-10-12,(4)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

- 12) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 13) Lateral bracing shall be attached to each ply.
- 14) Install screws per manufacturer recommendations.
- 15) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 15-2-8	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 105 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.95 (4-5)	Vert TL: 0.37 in	L / 473	(6-7)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.98 (7-8)	Vert LL: 0.19 in	L / 935	(6-7)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.77 (3-8)	Horz TL: 0.05 in		6	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	5.5 in	1.81 in	1,700 lbs	.	.	-98 lbs	-98 lbs	275 lbs
6	1	1.5 in	--	2,261 lbs	.	.	-315 lbs	-315 lbs	.

Material

TC: DFL 2400/2.0 2 x 4 except:
 DFL SS 2 x 6: 9-10
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL #2 2 x 4: 2-7

Bracing

TC: Sheathed or Purlins at 3-10-0, Purlin design by Others.
 BC: Sheathed or Purlins at 7-8-0, Purlin design by Others.
 Web: One Midpoint Row: 3-8, 4-6

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	15-2-8	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	15-2-8	Down	Proj	6 psf	6 psf	24 in
Top	6-0-0	15-2-8	Down	Proj	100 psf	100 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
15-2-8	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	105 lbs
7	D1 + S3				1.000				
8	D2 + S1				1.000				
9	D2 + S2				1.000				
10	D2 + S3				1.000				
11	0.60 D1 + 0.60 W1 [Uplift]				1.000				
12	0.60 D1 + 0.60 W2 [Uplift]				1.000				
13	0.60 D1 + 0.60 W4 [Uplift]				1.000				
14	0.60 D1 + 0.60 W8 [Uplift]				1.000				
15	0.60 D2 + 0.60 W1 [Uplift]				1.000				
16	0.60 D2 + 0.60 W2 [Uplift]				1.000				
17	0.60 D2 + 0.60 W4 [Uplift]				1.000				
18	0.60 D2 + 0.60 W8 [Uplift]				1.000				
19	D1 + L10*1				1.000				
20	D2 + L10*1				1.000				
21	D1 + I1				1.000				
22	D2 + I1				1.000				
23	D1 + UR1				1.000				
24	D1 + UR2				1.000				
25	D2 + UR1				1.000				
26	D2 + UR2				1.000				
27	D1 + AS10*1				1.000				
28	D2 + AS10*1				1.000				

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

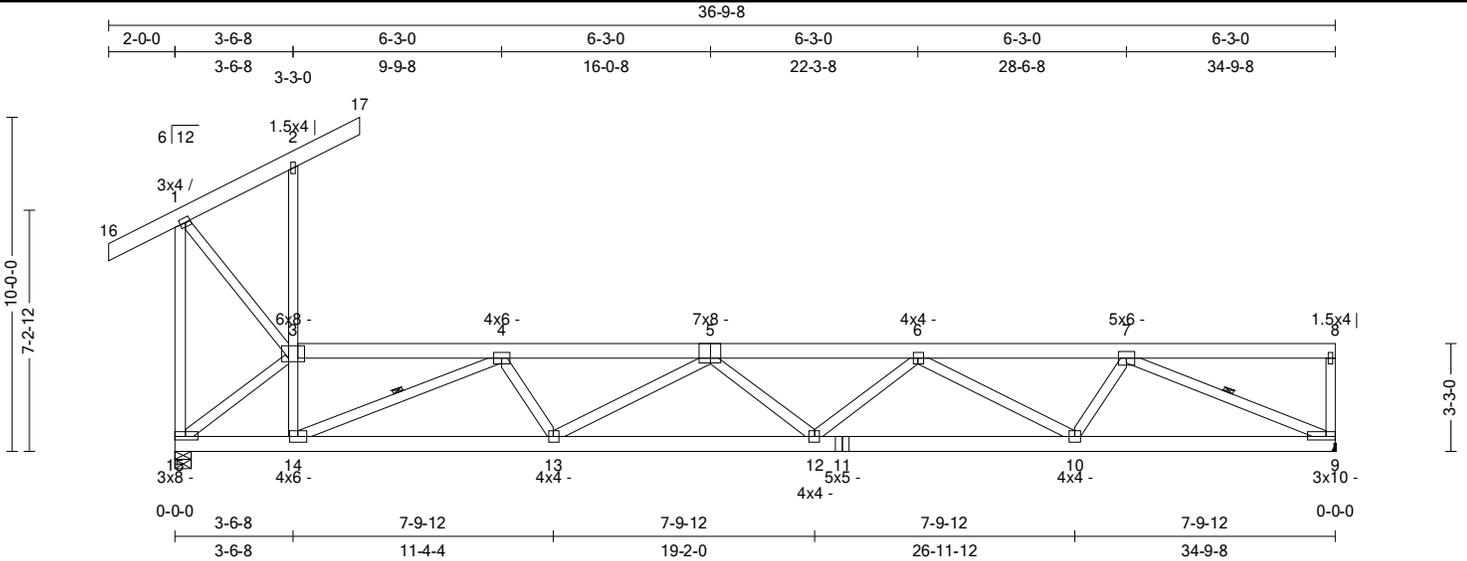
TC	3-4	0.784	723 lbs	(-2,634 lbs)											
BC	6-7	0.945	2,451 lbs	(-397 lbs)	7-8	0.979	2,622 lbs	(-648 lbs)							
Web	1-8	0.316		(-427 lbs)	3-8	0.770	492 lbs	(-2,938 lbs)	4-7	0.161	350 lbs	(-302 lbs)	5-6	0.127	(-543 lbs)
	1-3	0.075	310 lbs		2-3	0.472		(-870 lbs)	4-6	0.665	526 lbs	(-2,977 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



SPAN 34-9-8	PITCH 6/12	QTY 5	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 232 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.26 (5-6)	Vert TL: 0.47 in	L/875	(12-13)	L/180
TCDL: 15	TPI 1-2014	BC: 0.44 (12-13)	Vert LL: 0.25 in	L/999	(12-13)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.78 (3-15)	Horz TL: 0.1 in		9	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.03 in	1,905 lbs	.	.	-237 lbs	-237 lbs	238 lbs
9	1	1.5 in	---	1,616 lbs	.	.	-134 lbs	-134 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 2-14

Bracing

TC: Sheathed or Purlins at 3-9-0, Purlin design by Others.
 BC: Sheathed or Purlins at 9-11-0, Purlin design by Others.
 Web: One Midpoint Row: 4-14, 7-9

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9	Force 10
TC	3-4 0.182 845 lbs (-1,920 lbs)	5-6 0.261 585 lbs (-4,856 lbs)								
	4-5 0.236 777 lbs (-4,386 lbs)	6-7 0.183 334 lbs (-3,399 lbs)								
BC	9-10 0.274 2,946 lbs	12-13 0.437 5,000 lbs (-645 lbs)	14-15 0.162 1,914 lbs (-784 lbs)							
	10-12 0.402 4,605 lbs (-471 lbs)	13-14 0.362 4,134 lbs (-775 lbs)								
Web	1-15 0.538 (-319 lbs)	4-14 0.576 (-2,425 lbs)	6-12 0.229 518 lbs							
	1-3 0.113 356 lbs	4-13 0.264 599 lbs	6-10 0.755 (-1,404 lbs)							
	3-15 0.777 791 lbs (-2,478 lbs)	5-13 0.456 (-848 lbs)	7-10 0.410 928 lbs							
	3-14 0.246 1,074 lbs	5-12 0.119 (-375 lbs)	7-9 0.746 303 lbs (-3,239 lbs)							

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Provide adequate drainage to prevent ponding.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **Q03A**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
34-9-8	6/12	5	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	232 lbs

- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

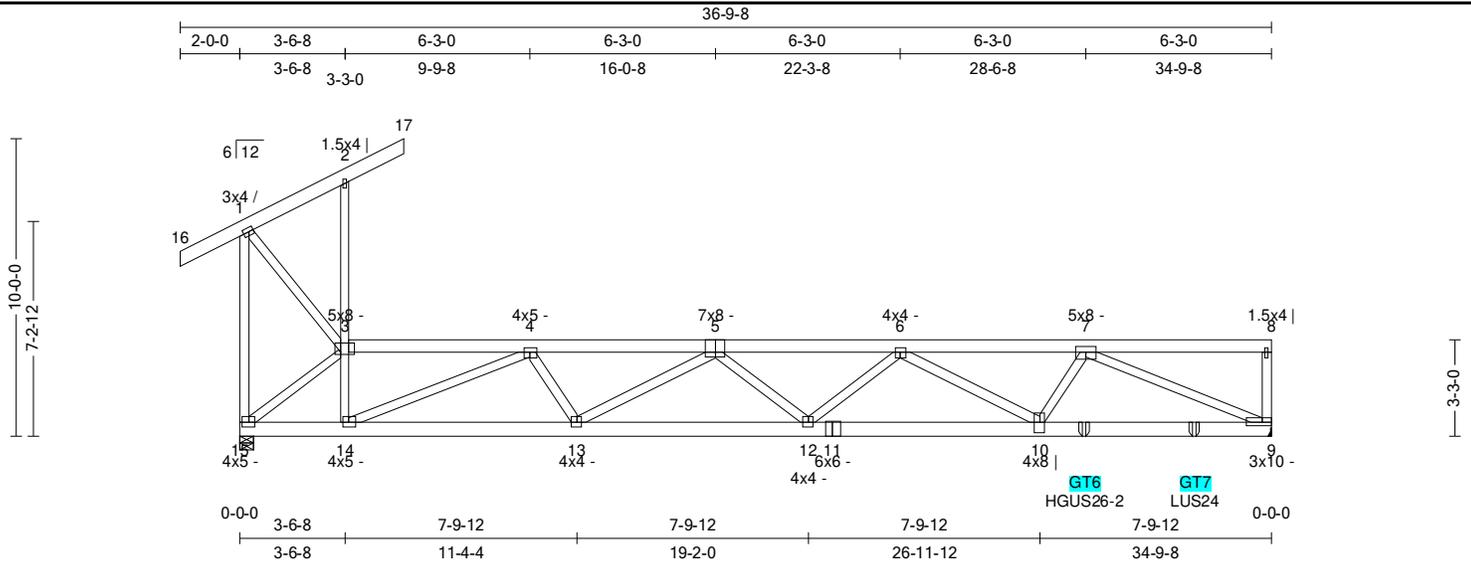
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 Eagle Metal Products



Mustang Truss
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Truss: Q03B
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 1 of 3

SPAN 34-9-8	PITCH 6/12	QTY 3	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 24 in	WGT/PLY 239 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.26 (5-6)	Vert TL: 0.47 in	L/ 865	(12-13)	L/ 180
TCLL: 25	TPI 1-2014	BC: 0.80 (9-10)	Vert LL: 0.24 in	L/ 999	(9-10)	L/ 240
TCDL: 15	Rep Mbr: No	Web: 0.91 (7-9)	Horz TL: 0.11 in		9	
BCLL: 0	Lumber D.O.L.: 115 %	Shear: 0.97 (9-10)				
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	2,774 lbs	.	.	-114 lbs	-114 lbs	238 lbs
9	1	1.5 in	---	4,050 lbs

Material

TC: DFL SS 2 x 6
 BC: SYP 2400/1.8 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 2-14, 7-10

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	34-9-8	Down	Proj	25.78 plf	25.78 plf	
Top	-2-0-0	28-5-14	Down	Proj	24.22 plf	24.22 plf	
Top	32-2-6	34-9-8	Down	Proj	24.22 plf	24.22 plf	

Load Case DL: Std Dead Load

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	34-9-8	Down	Proj	15.47 plf	15.47 plf	
Top	-2-0-0	28-5-14	Down	Proj	14.53 plf	14.53 plf	
Top	32-2-6	34-9-8	Down	Proj	14.53 plf	14.53 plf	
Bot	0-0-0	34-9-8	Down	Proj	6.19 plf	6.19 plf	
Bot	0-0-0	28-5-14	Down	Proj	5.81 plf	5.81 plf	
Bot	32-2-6	34-9-8	Down	Proj	5.81 plf	5.81 plf	

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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
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Truss: **Q03B**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 2 of 3

SPAN 34-9-8	PITCH 6/12	QTY 3	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 24 in	WGT/PLY 239 lbs
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User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	34-9-8	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	34-9-8	Down	Proj	6 psf	6 psf	24 in
Top	8-0-0	14-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000
7	D1 + S3	1.000
8	D2 + S1	1.000
9	D2 + S2	1.000
10	D2 + S3	1.000
11	0.60 D1 + 0.60 W1 [Uplift]	1.000
12	0.60 D1 + 0.60 W2 [Uplift]	1.000
13	0.60 D1 + 0.60 W4 [Uplift]	1.000
14	0.60 D1 + 0.60 W8 [Uplift]	1.000
15	0.60 D2 + 0.60 W1 [Uplift]	1.000
16	0.60 D2 + 0.60 W2 [Uplift]	1.000
17	0.60 D2 + 0.60 W4 [Uplift]	1.000
18	0.60 D2 + 0.60 W8 [Uplift]	1.000
19	D1 + L10*1	1.000
20	D2 + L10*1	1.000
21	D1 + I1	1.000
22	D2 + I1	1.000
23	D1 + UR1	1.000
24	D1 + UR2	1.000
25	D2 + UR1	1.000
26	D2 + UR2	1.000
27	D1 + AS10*1	1.000
28	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.113	349 lbs	(-1,485 lbs)	5-6	0.263	(-4,436 lbs)
	4-5	0.225		(-3,805 lbs)	6-7	0.259	(-4,381 lbs)
BC	9-10	0.798	3,580 lbs		12-13	0.365	4,401 lbs
	10-12	0.662	4,582 lbs		13-14	0.220	3,565 lbs
Web	3-15	0.271	300 lbs	(-1,916 lbs)	5-13	0.128	(-718 lbs)
	3-14	0.218	966 lbs		6-12	0.074	(-338 lbs)
	4-14	0.541		(-2,278 lbs)	6-10	0.093	(-395 lbs)
	4-13	0.226	511 lbs		7-10	0.315	1,642 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
G16	BC	28-5-14	2516	0	0	HGUS26-2	8x 16d, 20x 16d
G17	BC	32-2-6	511	143	0	LUS24	2x SD9212, 4x SD9112

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **Q03B**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 3 of 3

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
34-9-8	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	239 lbs

9) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental Head Side - FastenMaster FlatLOK (2 - Ply) Screws within 12" along the chord or into converging webs at point load:

- BC: 28-5-14,(7)Connectors
- BC: 32-2-6,(2)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

10) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

11) Lateral bracing shall be attached to each ply.

12) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

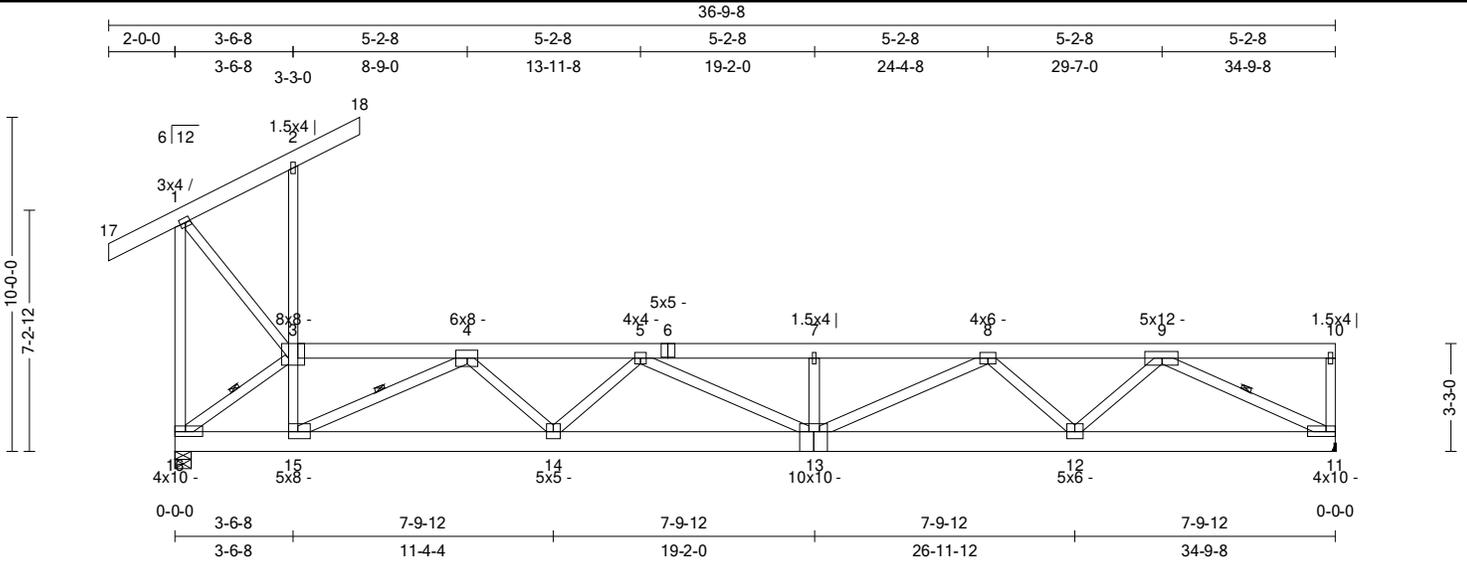
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 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
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 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q03C
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 1 of 2

SPAN 34-9-8	PITCH 6/12	QTY 3	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 259 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.51 (7-8)	Vert TL: 0.78 in	L/ 526	(13-14)	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.71 (13-14)	Vert LL: 0.23 in	L/ 999	(13-14)	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.91 (9-11)	Horz TL: 0.14 in		11	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	3.21 in	3,010 lbs	.	.	-238 lbs	-238 lbs	238 lbs
11	1	1.5 in	---	2,611 lbs	.	.	-134 lbs	-134 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 8
 Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 2-15, 9-12

Bracing

TC: Sheathed or Purlins at 2-5-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 3-16, 4-15, 9-11

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	34-9-8	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	34-9-8	Down	Proj	6 psf	6 psf	24 in
Top	6-0-0	27-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
34-9-8	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	259 lbs
7	D1 + S3				1.000				
8	D2 + S1				1.000				
9	D2 + S2				1.000				
10	D2 + S3				1.000				
11	0.60 D1 + 0.60 W1 [Uplift]				1.000				
12	0.60 D1 + 0.60 W2 [Uplift]				1.000				
13	0.60 D1 + 0.60 W4 [Uplift]				1.000				
14	0.60 D1 + 0.60 W8 [Uplift]				1.000				
15	0.60 D2 + 0.60 W1 [Uplift]				1.000				
16	0.60 D2 + 0.60 W2 [Uplift]				1.000				
17	0.60 D2 + 0.60 W4 [Uplift]				1.000				
18	0.60 D2 + 0.60 W8 [Uplift]				1.000				
19	D1 + L10*1				1.000				
20	D2 + L10*1				1.000				
21	D1 + I1				1.000				
22	D2 + I1				1.000				
23	D1 + UR1				1.000				
24	D1 + UR2				1.000				
25	D2 + UR1				1.000				
26	D2 + UR2				1.000				
27	D1 + AS10*1				1.000				
28	D2 + AS10*1				1.000				

Member Forces

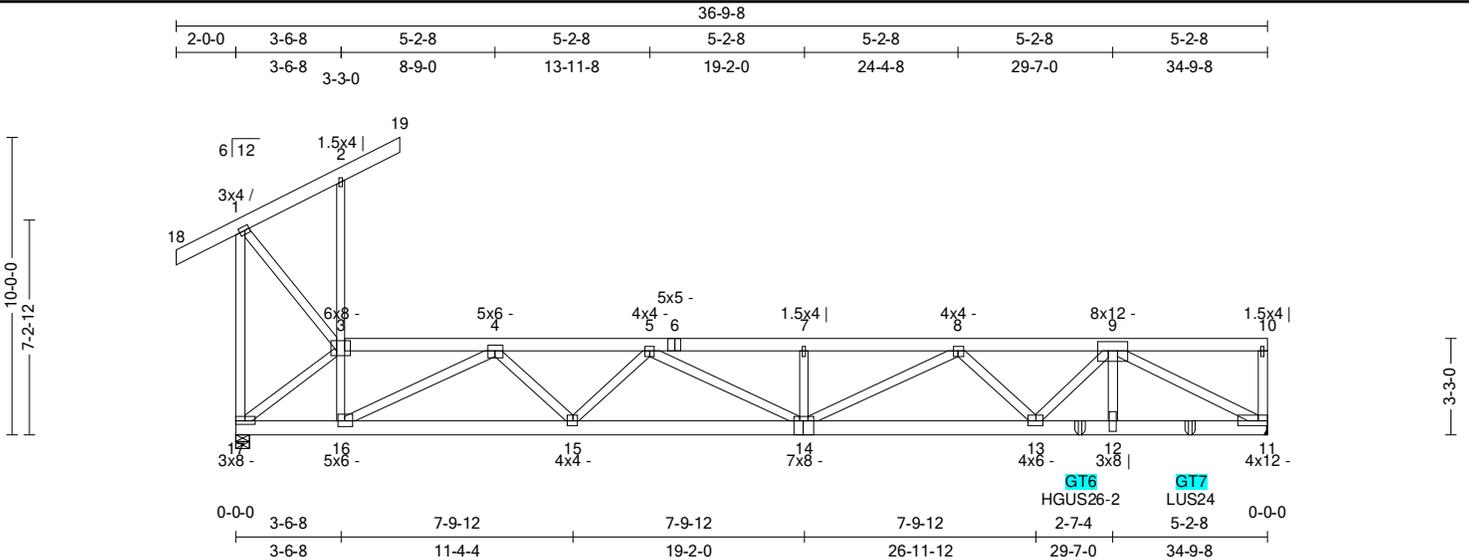
Table indicates: Member ID, max CSI max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.225	851 lbs	(-3,321 lbs)	7-8	0.507	610 lbs	(-9,580 lbs)				
	4-5	0.443	785 lbs	(-8,362 lbs)	8-9	0.328	337 lbs	(-6,195 lbs)				
	5-7	0.507	610 lbs	(-9,573 lbs)								
BC	11-12	0.333	4,375 lbs		13-14	0.707	9,392 lbs	(-699 lbs)	15-16	0.220	3,311 lbs	(-790 lbs)
	12-13	0.604	7,926 lbs	(-406 lbs)	14-15	0.542	7,263 lbs	(-804 lbs)				
Web	1-16	0.528		(-323 lbs)	4-15	0.839		(-4,447 lbs)	7-13	0.146		(-960 lbs)
	1-3	0.114	358 lbs		4-14	0.702	1,590 lbs		8-13	0.834	1,888 lbs	
	3-16	0.594	789 lbs	(-4,242 lbs)	5-14	0.376		(-1,488 lbs)	8-12	0.632		(-2,503 lbs)
	3-15	0.479	2,136 lbs		5-13	0.263	596 lbs	(-312 lbs)	9-12	0.505	2,630 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 34-9-8	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 24 in	WGT/PLY 247 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2021/	TC: 0.40 (7-8)	Vert TL: 0.63 in	L/ 656	(14-15)	L/ 180
TCLL: 25	TPI 1-2014	BC: 0.82 (12-13)	Vert LL: 0.2 in	L/ 999	(14-15)	L/ 240
TCDL: 15	Rep Mbr: No	Web: 0.90 (9-11)	Horz TL: 0.15 in		11	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 6						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.52 in	3,659 lbs			-86 lbs	-86 lbs	229 lbs
11	1	1.5 in	--	5,847 lbs					

Material

TC: DFL SS 2 x 6
 BC: SYP 2400/1.8 2 x 6
 Web: DFL Stud 2 x 4 except:
 DFL #2 2 x 4: 2-16, 9-11

Bracing

TC: Sheathed or Purlins at 5-9-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	28-5-14	Down	Proj	25.78 plf	25.78 plf	
Top	32-2-6	34-9-8	Down	Proj	25.78 plf	25.78 plf	
Top	-2-0-0	34-9-8	Down	Proj	24.22 plf	24.22 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	28-5-14	Down	Proj	15.47 plf	15.47 plf	
Top	32-2-6	34-9-8	Down	Proj	15.47 plf	15.47 plf	
Top	-2-0-0	34-9-8	Down	Proj	14.53 plf	14.53 plf	
Bot	0-0-0	28-5-14	Down	Proj	6.19 plf	6.19 plf	
Bot	32-2-6	34-9-8	Down	Proj	6.19 plf	6.19 plf	
Bot	0-0-0	34-9-8	Down	Proj	5.81 plf	5.81 plf	

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 34-9-8	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 2	SPACING 24 in	WGT/PLY 247 lbs
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User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	28-5-14	Down	Proj	15.47 plf	15.47 plf	
Top	32-2-6	34-9-8	Down	Proj	15.47 plf	15.47 plf	
Top	-2-0-0	34-9-8	Down	Proj	14.53 plf	14.53 plf	
Bot	0-0-0	28-5-14	Down	Proj	6.19 plf	6.19 plf	
Bot	32-2-6	34-9-8	Down	Proj	6.19 plf	6.19 plf	
Bot	0-0-0	34-9-8	Down	Proj	5.81 plf	5.81 plf	
Top	6-0-0	27-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	28-5-14	Down	1,052 lbs	
Bot	32-2-6	Down	228 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000
7	D1 + S3	1.000
8	D2 + S1	1.000
9	D2 + S2	1.000
10	D2 + S3	1.000
11	0.60 D1 + 0.60 W1 [Uplift]	1.000
12	0.60 D1 + 0.60 W2 [Uplift]	1.000
13	0.60 D1 + 0.60 W4 [Uplift]	1.000
14	0.60 D1 + 0.60 W8 [Uplift]	1.000
15	0.60 D2 + 0.60 W1 [Uplift]	1.000
16	0.60 D2 + 0.60 W2 [Uplift]	1.000
17	0.60 D2 + 0.60 W4 [Uplift]	1.000
18	0.60 D2 + 0.60 W8 [Uplift]	1.000
19	D1 + L10*1	1.000
20	D2 + L10*1	1.000
21	D1 + I1	1.000
22	D2 + I1	1.000
23	D1 + UR1	1.000
24	D1 + UR2	1.000
25	D2 + UR1	1.000
26	D2 + UR2	1.000
27	D1 + AS10*1	1.000
28	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.136	314 lbs	(-2,019 lbs)	7-8	0.401	(-6,880 lbs)			
	4-5	0.314		(-5,391 lbs)	8-9	0.359	(-6,171 lbs)			
	5-7	0.401		(-6,878 lbs)						
BC	11-12	0.399	4,923 lbs		13-14	0.432	6,703 lbs	15-16	0.292	4,545 lbs
	12-13	0.822	4,923 lbs		14-15	0.414	6,192 lbs	16-17	0.117	2,012 lbs
Web	3-17	0.534		(-2,605 lbs)	5-15	0.235		8-13	0.163	(-814 lbs)
	3-16	0.311	1,392 lbs		5-14	0.284	849 lbs	9-13	0.612	1,829 lbs
	4-16	0.691		(-2,866 lbs)	7-14	0.094	(-490 lbs)	9-12	0.419	1,251 lbs
	4-15	0.415	1,241 lbs		8-14	0.147	440 lbs	9-11	0.901	(-5,622 lbs)

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset	Load (Max down lbs)	Load (Max up lbs)	Angle (deg)	Hanger (or equal)	Fastener
G16	BC	28-5-14	2510	0	0	HGUS26-2	8x 16d, 20x 16d
G17	BC	32-2-6	499	123	0	LUS24	2x SD9212, 4x SD9112

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
34-9-8	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	247 lbs

9) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc - Z(TSLZ278, 2 - ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row 0.131"x3" Nails @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2 - ply) Screws as follows within 24" of the location shown:

- BC: 28-5-14,(11)Connectors
- BC: 32-2-6,(2)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

10) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

11) Lateral bracing shall be attached to each ply.

12) Install screws per manufacturer recommendations.

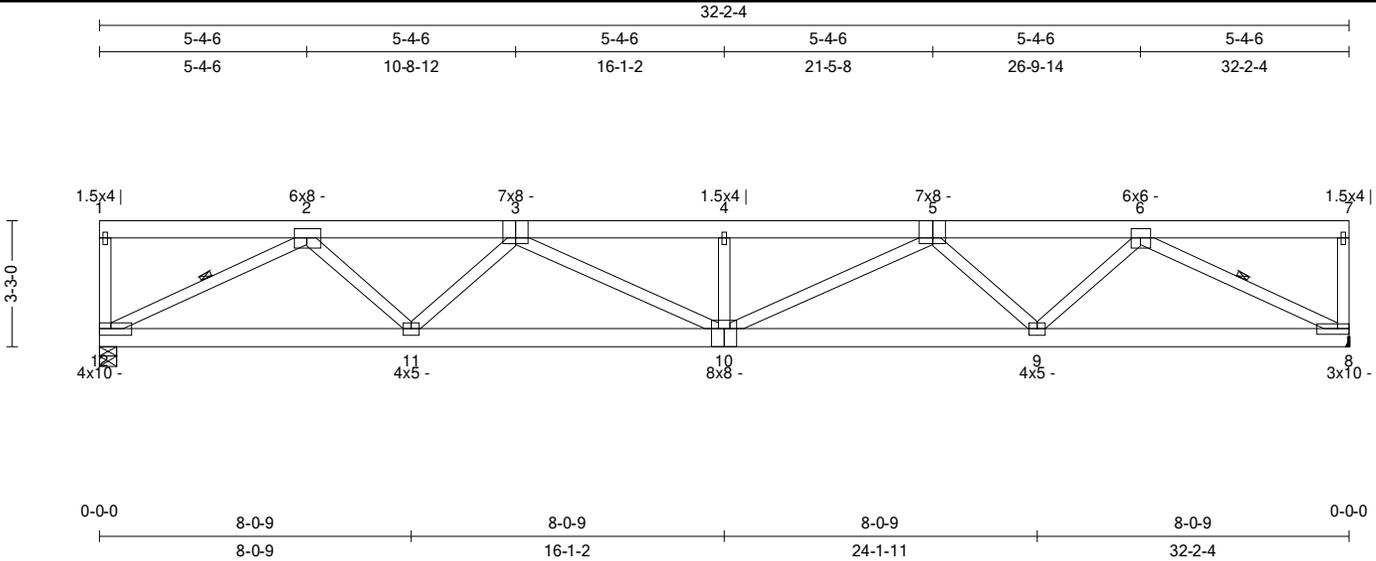
13) Listed wind uplift reactions based on MWFRS & C&C loading.



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Truss: Q04A
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 1 of 2

SPAN 32-2-4	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 190 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.33 (3-4)	Vert TL: 0.49 in	L/777	(10-11)	L/180
TCDL: 15	TPI 1-2014	BC: 0.53 (10-11)	Vert LL: 0.18 in	L/999	10	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.84 (2-12)	Horz TL: 0.11 in		8	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	5.5 in	2.50 in	2,348 lbs	.	.	-70 lbs	-70 lbs	-52 lbs
8	1	1.5 in	--	1,814 lbs	.	.	-70 lbs	-70 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-12, 6-8

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	32-2-4	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	32-2-4	Down	Proj	6 psf	6 psf	24 in
Top	3-0-0	15-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D2 + S1	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **Q04A**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
32-2-4	0/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	190 lbs
7	0.60 D1 + 0.60 W1 [Uplift]				1.000				
8	0.60 D1 + 0.60 W2 [Uplift]				1.000				
9	0.60 D1 + 0.60 W4 [Uplift]				1.000				
10	0.60 D1 + 0.60 W8 [Uplift]				1.000				
11	0.60 D2 + 0.60 W1 [Uplift]				1.000				
12	0.60 D2 + 0.60 W2 [Uplift]				1.000				
13	0.60 D2 + 0.60 W4 [Uplift]				1.000				
14	0.60 D2 + 0.60 W8 [Uplift]				1.000				
15	D1 + L10*1				1.000				
16	D2 + L10*1				1.000				
17	D1 + UR1				1.000				
18	D1 + UR2				1.000				
19	D2 + UR1				1.000				
20	D2 + UR2				1.000				
21	D1 + AS10*1				1.000				
22	D2 + AS10*1				1.000				

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.265	(-4,996 lbs)	4-5	0.326	(-6,138 lbs)						
	3-4	0.326	(-6,138 lbs)	5-6	0.213	(-4,013 lbs)						
BC	8-9	0.272	2,935 lbs	9-10	0.438	4,992 lbs	10-11	0.526	6,025 lbs	11-12	0.356	3,879 lbs
Web	2-12	0.843	(-4,399 lbs)	3-10	0.314	711 lbs	5-9	0.383	(-1,414 lbs)			
	2-11	0.712	1,612 lbs	4-10	0.090	(-564 lbs)	6-9	0.687	1,555 lbs			
	3-11	0.403	(-1,486 lbs)	5-10	0.645	1,460 lbs	6-8	0.638	(-3,329 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

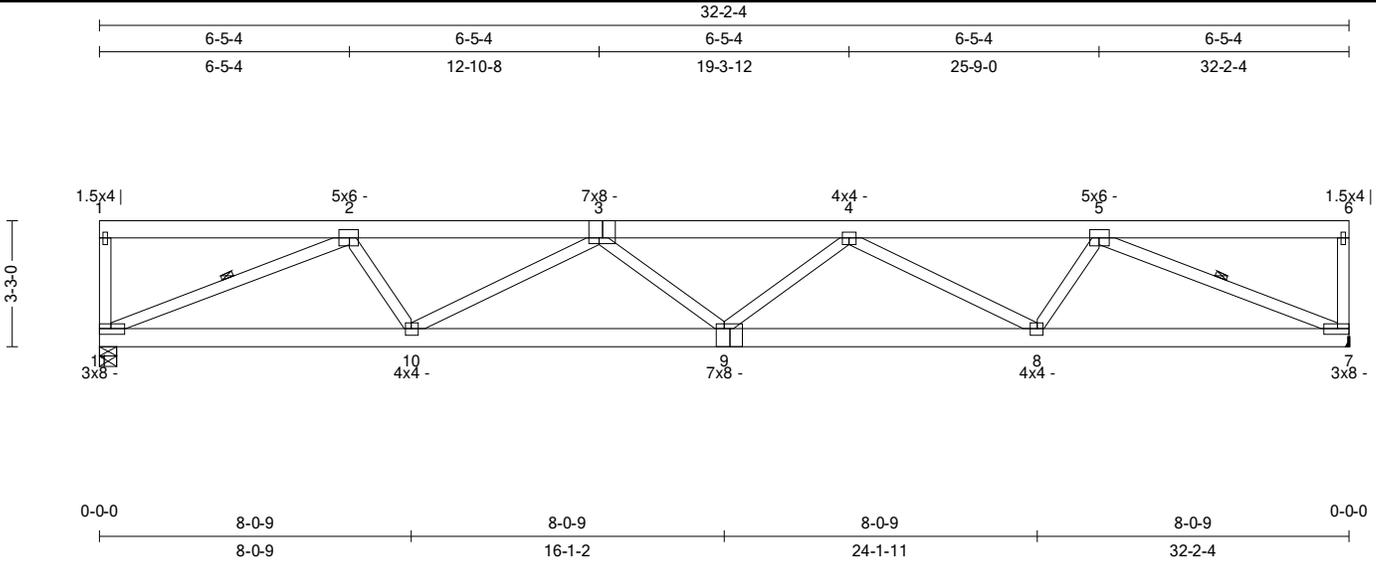
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 Eagle Metal Products



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 2525 Hyacinth Street NE
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Truss: Q04B
 Job: 2501272
 Designer: Anthony
 Date: 01/23/26 09:23:20
 Page: 1 of 1

SPAN 32-2-4	PITCH 0/12	QTY 6	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 185 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.22 (3-4)	Vert TL: 0.36 in	L/999	(9-10)	L/180
TCDL: 15	TPI 1-2014	BC: 0.36 (9-10)	Vert LL: 0.2 in	L/999	9	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.71 (2-11)	Horz TL: 0.08 in		7	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.58 in	1,481 lbs	.	.	-70 lbs	-70 lbs	-52 lbs
7	1	1.5 in	--	1,481 lbs	.	.	-70 lbs	-70 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-11, 5-7

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.168	(-3,098 lbs)	4-5	0.167	(-3,097 lbs)			
	3-4	0.222	(-4,118 lbs)						
BC	7-8	0.258	2,721 lbs	8-9	0.358	4,059 lbs	9-10	0.362	4,068 lbs
	2-11	0.714	(-2,980 lbs)	3-9	0.149	338 lbs	5-8	0.332	752 lbs
Web	2-10	0.331	749 lbs	4-9	0.149	338 lbs	5-7	0.714	(-2,977 lbs)
	3-10	0.628	(-1,116 lbs)	4-8	0.626	(-1,112 lbs)			

Notes

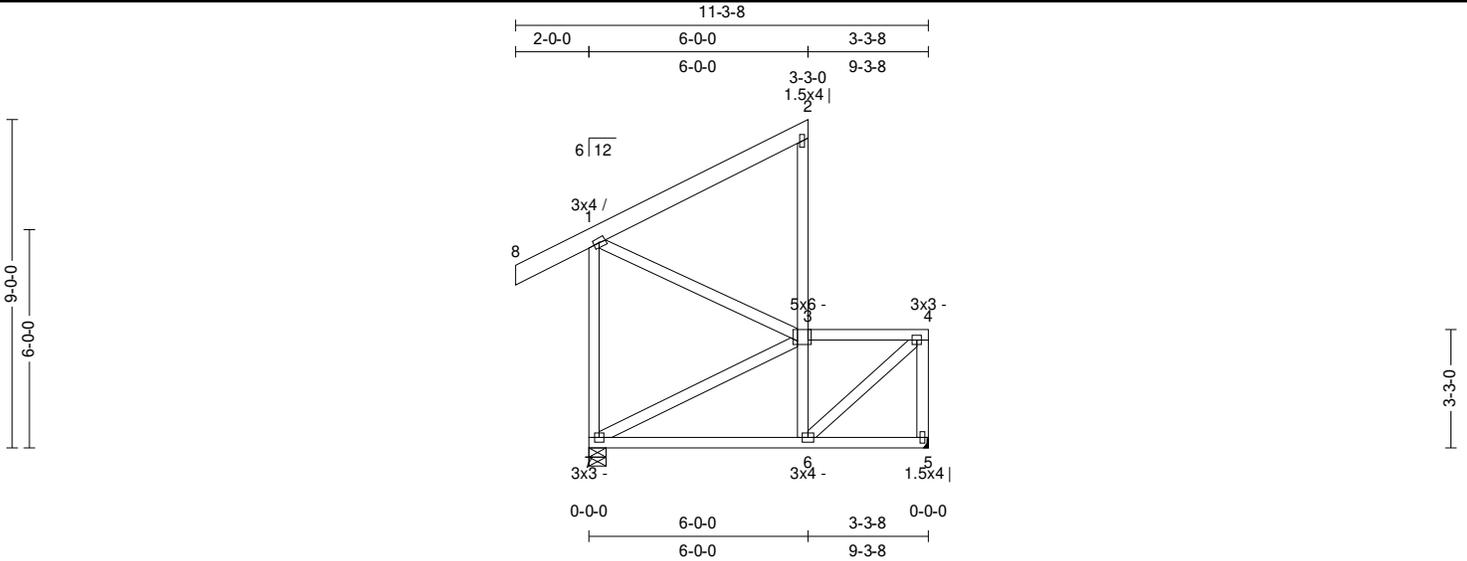
- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products



SPAN 9-3-8	PITCH 6/12	QTY 34	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 76 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.27 (3-4)	Vert TL: 0.06 in	L / 999	(6-7)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.24 (6-7)	Vert LL: 0.03 in	L / 999	(6-7)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.47 (1-7)	Horz TL: 0 in		5	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	5.5 in	1.50 in	639 lbs	.	.	-88 lbs	-88 lbs	156 lbs
5	1	1.5 in	--	462 lbs	.	-20 lbs	-156 lbs	-156 lbs	.

Material

TC: DFL #2 2 x 4 except:
 DFL SS 2 x 6: 8-2

BC: DFL #2 2 x 4

Web: DFL Standard 2 x 4 except:
 DFL #2 2 x 4: 2-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

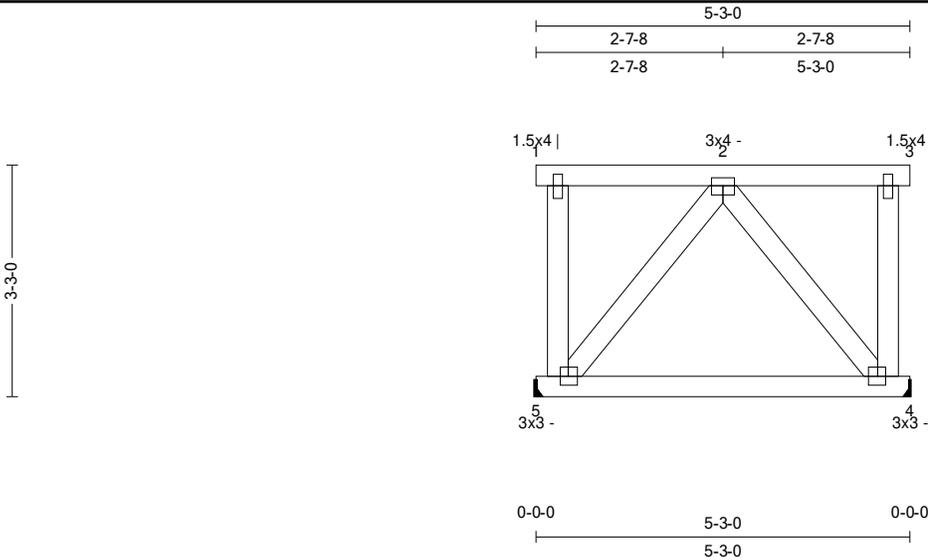
TC	3-4	0.275	(-315 lbs)				
BC	6-7	0.239	314 lbs				
Web	1-7	0.473	(-503 lbs)	4.6	0.187	424 lbs	
	3-7	0.249	(-354 lbs)	4.5	0.179	(-449 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 5-3-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 29 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.43 (2-3)	Vert TL: 0.04 in	L / 999	(4-5)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.26 (4-5)	Vert LL: 0.02 in	L / 999	(4-5)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.15 (2-5)	Horz TL: 0 in		4	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	1.5 in	--	767 lbs	.	.	-90 lbs	-90 lbs	-52 lbs
4	1	1.5 in	--	767 lbs	.	.	-90 lbs	-90 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	5-3-0	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	5-3-0	Down	Proj	6 psf	6 psf	24 in
Top	0-0-0	5-3-0	Down	Proj	100 psf	100 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D2 + S1	1.000
7	0.60 D1 + 0.60 W1 [Uplift]	1.000
8	0.60 D1 + 0.60 W2 [Uplift]	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
5-3-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	29 lbs
9	0.60 D1 + 0.60 W4 [Uplift]				1.000				
10	0.60 D1 + 0.60 W8 [Uplift]				1.000				
11	0.60 D2 + 0.60 W1 [Uplift]				1.000				
12	0.60 D2 + 0.60 W2 [Uplift]				1.000				
13	0.60 D2 + 0.60 W4 [Uplift]				1.000				
14	0.60 D2 + 0.60 W8 [Uplift]				1.000				
15	D1 + L10*1				1.000				
16	D2 + L10*1				1.000				
17	D1 + UR1				1.000				
18	D1 + UR2				1.000				
19	D2 + UR1				1.000				
20	D2 + UR2				1.000				
21	D1 + AS10*1				1.000				
22	D2 + AS10*1				1.000				

Member Forces

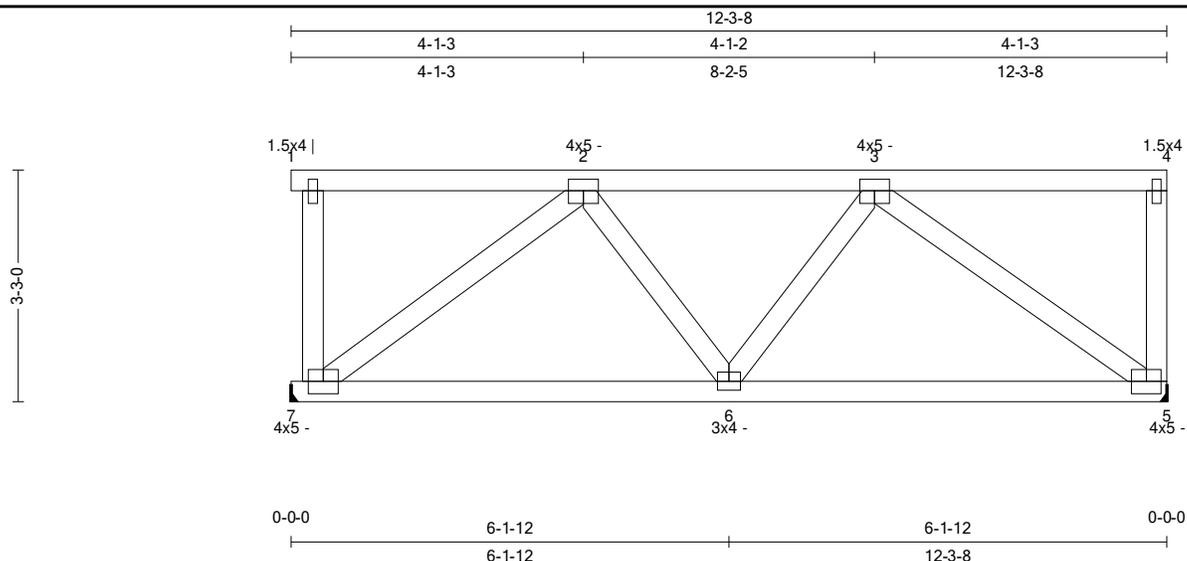
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC												
BC	4.5	0.260	304 lbs									
Web	1.5	0.118	(-353 lbs)	2.5	0.145	(-494 lbs)	2.4	0.145	(-494 lbs)	3.4	0.118	(-353 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 12-3-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 60 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.86 (3-4)	Vert TL: 0.08 in	L / 999	(5-6)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.58 (5-6)	Vert LL: 0.03 in	L / 999	(5-6)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.95 (3-5)	Horz TL: 0.03 in		5	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	1.5 in	--	1,820 lbs	.	.	-102 lbs	-102 lbs	52 lbs
5	1	1.5 in	--	1,769 lbs	.	.	-99 lbs	-99 lbs	.

Material

TC: DFL #1B 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	12-3-8	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	12-3-8	Down	Proj	6 psf	6 psf	24 in
Top	0-0-0	12-3-8	Down	Proj	100 psf	100 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D2 + S1	1.000
7	0.60 D1 + 0.60 W1 [Uplift]	1.000
8	0.60 D1 + 0.60 W2 [Uplift]	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
12-3-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	60 lbs
9	0.60 D1 + 0.60 W4 [Uplift]				1.000				
10	0.60 D1 + 0.60 W8 [Uplift]				1.000				
11	0.60 D2 + 0.60 W1 [Uplift]				1.000				
12	0.60 D2 + 0.60 W2 [Uplift]				1.000				
13	0.60 D2 + 0.60 W4 [Uplift]				1.000				
14	0.60 D2 + 0.60 W8 [Uplift]				1.000				
15	D1 + L10*1				1.000				
16	D2 + L10*1				1.000				
17	D1 + UR1				1.000				
18	D1 + UR2				1.000				
19	D2 + UR1				1.000				
20	D2 + UR2				1.000				
21	D1 + AS10*1				1.000				
22	D2 + AS10*1				1.000				

Member Forces

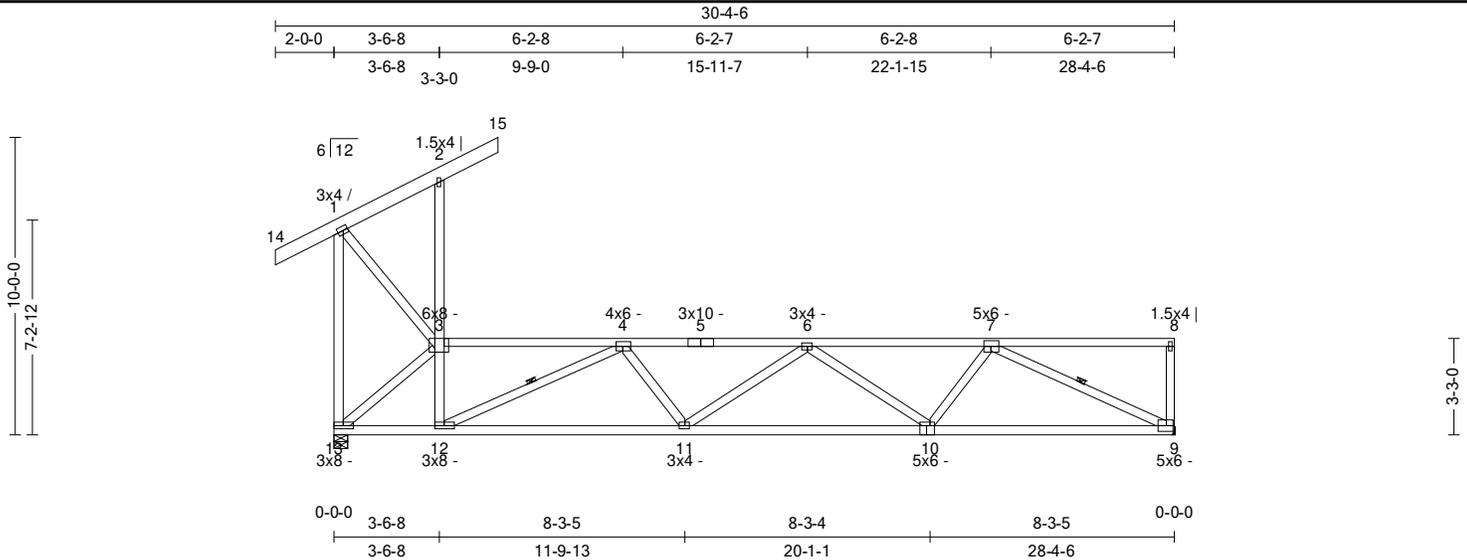
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.689	(-1,676 lbs)				
BC	5-6	0.581	1,671 lbs	6-7	0.574	1,626 lbs	
Web	1-7	0.121	(-516 lbs)	3-5	0.952	(-2,088 lbs)	
	2-7	0.897	(-2,064 lbs)	4-5	0.114	(-487 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 28-4-6	PITCH 6/12	QTY 3	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 160 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.79 (4-6)	Vert TL: 0.45 in	L/739	(10-11)	L/180
TCDL : 15	TPI 1-2014	BC : 0.99 (10-11)	Vert LL: 0.18 in	L/999	(10-11)	L/240
BCLL : 0	Rep Mbr : Yes	Web : 0.92 (3-13)	Horz TL: 0.14 in		9	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	5.5 in	2.11 in	1,974 lbs	.	.	-209 lbs	-209 lbs	236 lbs
9	1	1.5 in	--	1,555 lbs	.	.	-163 lbs	-163 lbs	.

Material

TC: DFL #1B 2 x 4 except:
 DFL SS 2 x 6: 14-15
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL #2 2 x 4: 2-12

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 7-8-0, Purlin design by Others.
 Web: One Midpoint Row: 4-12, 7-9

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

User-defined Load Case **D2**: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	28-4-6	Down	Proj	15 psf	15 psf	24 in
Bot	0-0-0	28-4-6	Down	Proj	6 psf	6 psf	24 in
Top	8-0-0	14-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
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Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
28-46	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs

7	D1 + S3				1.000				
8	D2 + S1				1.000				
9	D2 + S2				1.000				
10	D2 + S3				1.000				
11	0.60 D1 + 0.60 W1 [Uplift]				1.000				
12	0.60 D1 + 0.60 W2 [Uplift]				1.000				
13	0.60 D1 + 0.60 W4 [Uplift]				1.000				
14	0.60 D1 + 0.60 W8 [Uplift]				1.000				
15	0.60 D2 + 0.60 W1 [Uplift]				1.000				
16	0.60 D2 + 0.60 W2 [Uplift]				1.000				
17	0.60 D2 + 0.60 W4 [Uplift]				1.000				
18	0.60 D2 + 0.60 W8 [Uplift]				1.000				
19	D1 + L10*1				1.000				
20	D2 + L10*1				1.000				
21	D1 + I1				1.000				
22	D2 + I1				1.000				
23	D1 + UR1				1.000				
24	D1 + UR2				1.000				
25	D2 + UR1				1.000				
26	D2 + UR2				1.000				
27	D1 + AS10*1				1.000				
28	D2 + AS10*1				1.000				

Member Forces

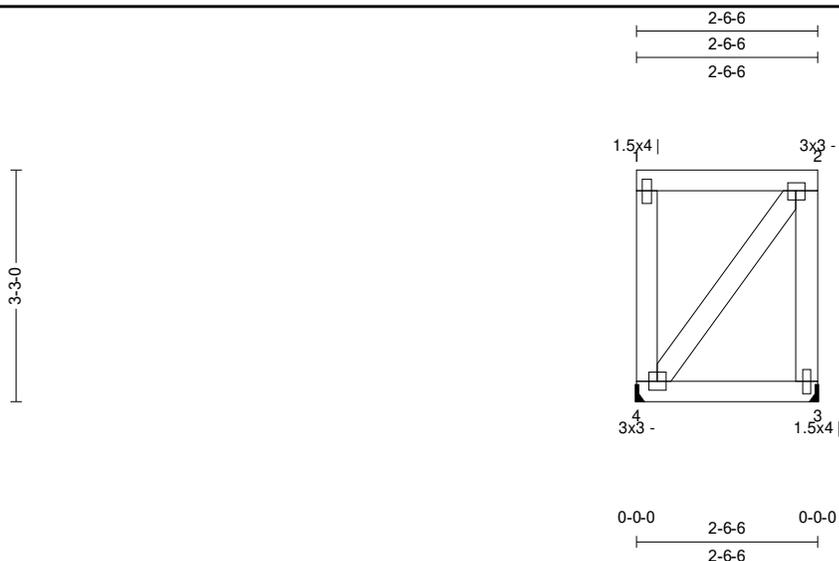
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.675	766 lbs	(-1,896 lbs)	6-7	0.513	385 lbs	(-3,232 lbs)
	4-6	0.790	649 lbs	(-4,209 lbs)				
BC	9-10	0.787	2,663 lbs		10-11	0.991	4,148 lbs	(-500 lbs)
					11-12	0.888	4,173 lbs	(-673 lbs)
Web	1-13	0.447		(-320 lbs)	3-12	0.270	1,171 lbs	
	1-3	0.084	350 lbs		4-12	0.711		(-2,523 lbs)
	3-13	0.916	707 lbs	(-2,506 lbs)	4-11	0.120	360 lbs	
					6-11	0.170	398 lbs	(-339 lbs)
					6-10	0.571		(-1,135 lbs)
					7-10	0.323	965 lbs	
					12-13	0.548	1,889 lbs	(-701 lbs)
					7-9	0.817	344 lbs	(-2,964 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 2-6-6	PITCH 0/12	QTY 3	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 18 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.11 (1-2)	Vert TL: 0 in	L / 999	(3-4)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.04 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.12 (1-4)	Horz TL: 0 in		3	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	116 lbs	.	-21 lbs	-43 lbs	-43 lbs	52 lbs
3	1	1.5 in	---	116 lbs	.	-21 lbs	-43 lbs	-43 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

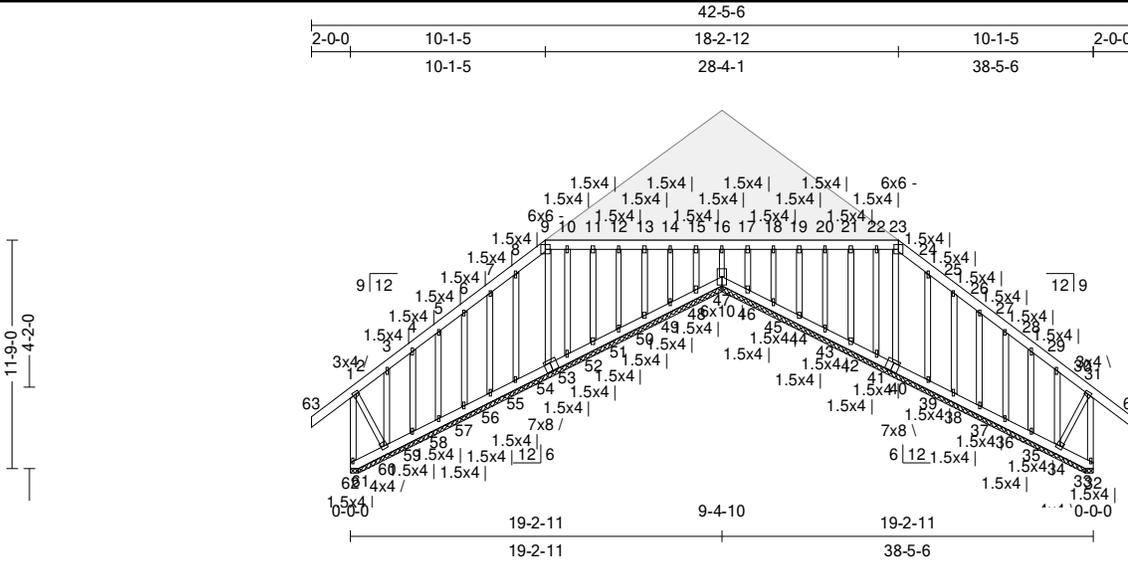
Member	Max CSI	Max Tension Force	Max Compression Force
TC			
BC			
Web			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



SPAN 38-5-6	PITCH 9/12	QTY 1	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 357 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.18 (63-1)	Vert TL: 0 in UP	L/999	32	L/180
TCDL: 15	TPI 1-2014	BC: 0.00 (60-62)	Vert LL: 0 in	L/999	32	L/240
BCLL: 0	Rep Mbr: No	Web: 0.17 (1-62)	Horz TL: 0 in			
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		260 lbs	108 plf		-68 lbs	-105 lbs	-105 lbs	93 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

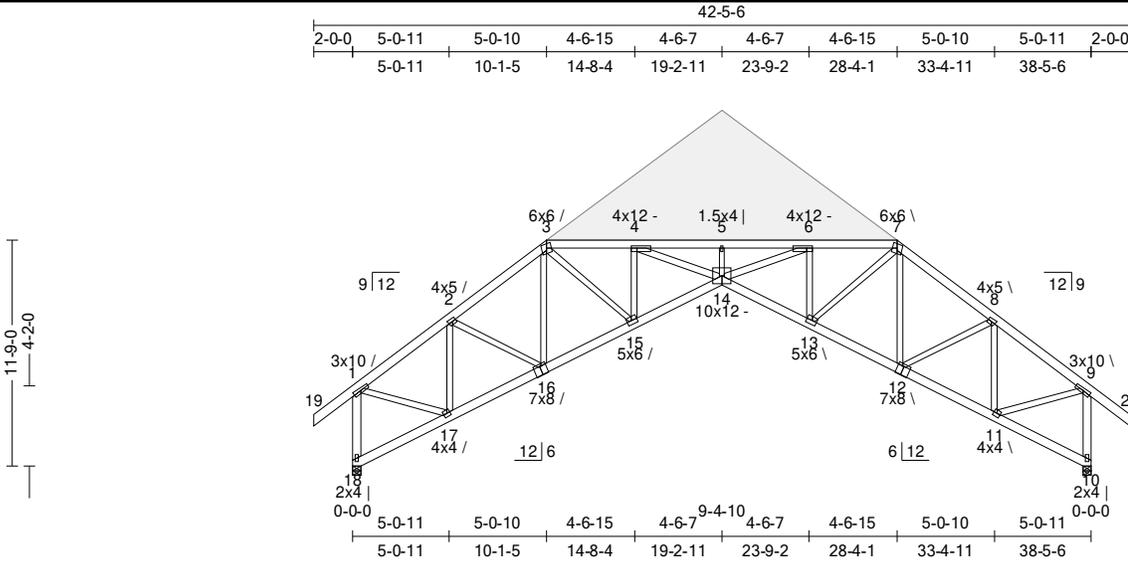
TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16" OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.



SPAN 38-5-6	PITCH 9/12	QTY 17	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 298 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 25	Bldg Code: IBC 2021/	TC: 0.45 (4-5)	Vert TL: 0.83 in	L/ 546	14	L/ 180
TCDL: 15	TPI 1-2014	BC: 0.64 (13-14)	Vert LL: 0.45 in	L/ 999	14	L/ 240
BCLL: 0	Rep Mbr: Yes	Web: 0.92 (4-14)	Horz TL: 0.9 in		10	
BCDL: 6	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	5.5 in	1.75 in	1,929 lbs	.	.	-12 lbs	-12 lbs	-101 lbs
10	1	5.5 in	1.75 in	1,929 lbs	.	.	-12 lbs	-12 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL SS 2 x 6
 Web: DFL Standard 2 x 4 except:
 DFL #2 x 4: 3-15, 4-14, 6-14, 7-13
 DFL SS 2 x 6: 1-18, 9-10

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.127	(-1,909 lbs)	4-5	0.450	(-8,225 lbs)	7-8	0.137	(-2,581 lbs)
	2-3	0.137	(-2,581 lbs)	5-6	0.450	(-8,225 lbs)	8-9	0.127	(-1,909 lbs)
	3-4	0.199	(-3,790 lbs)	6-7	0.199	(-3,790 lbs)			
BC	11-12	0.135	1,606 lbs	13-14	0.644	4,184 lbs	15-16	0.243	2,266 lbs
	12-13	0.243	2,266 lbs	14-15	0.644	4,184 lbs	16-17	0.135	1,606 lbs
Web	1-18	0.223	(-1,900 lbs)	3-15	0.467	2,430 lbs	7-13	0.467	2,430 lbs
	1-17	0.660	1,494 lbs	4-15	0.658	(-2,310 lbs)	7-12	0.375	(-575 lbs)
	2-17	0.457	(-1,091 lbs)	4-14	0.922	4,801 lbs	8-12	0.278	629 lbs
	2-16	0.278	629 lbs	6-14	0.922	4,801 lbs	8-11	0.457	(-1,091 lbs)
	3-16	0.375	(-575 lbs)	6-13	0.658	(-2,310 lbs)	9-11	0.660	1,494 lbs

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **R2**
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:21
 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
38-5-6	9/12	17	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	298 lbs

5) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

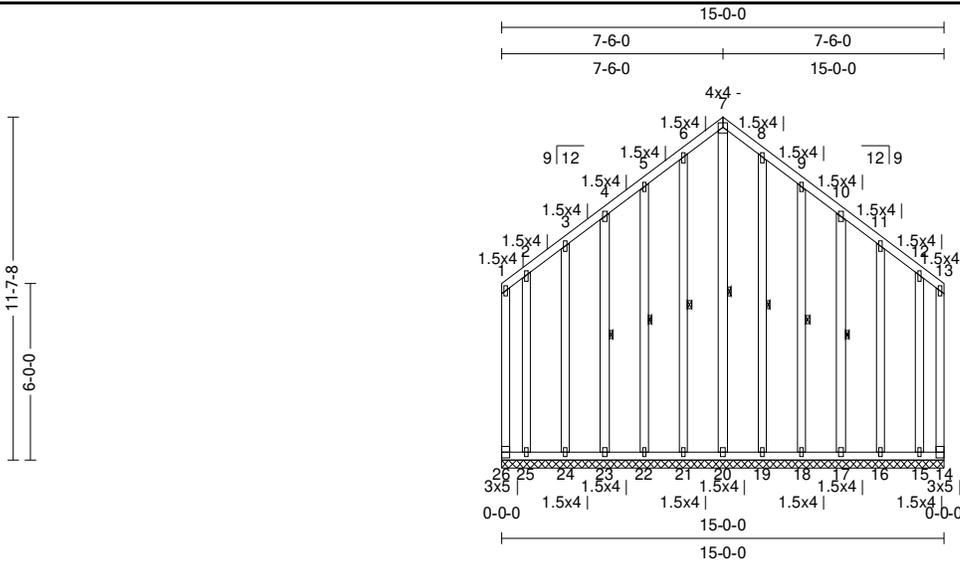
TrueBuild® Truss Software v5.8.14
 Eagle Metal Products



Mustang Truss
 2525 Hyacinth Street NE
 Salem, OR 97301
 Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: T1
 Job: **2501272**
 Designer: Anthony
 Date: 01/23/26 09:23:21
 Page: 1 of 1

SPAN 15-0-0	PITCH 9/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 175 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.44 (12-13)	Vert TL: 0 in UP	L / 999	14	L / 180
TCDL : 15	TPI 1-2014	BC : 0.01 (23-24)	Vert LL: 0 in	L / 999	14	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.47 (13-14)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		288 lbs	170 plf		-286 lbs	-153 lbs	-286 lbs	-66 lbs

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Standard 2 x 4: 2-25, 3-24, 11-16, 12-15

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 4-23, 5-22, 6-21, 7-20, 8-19, 9-18, 10-17

Loads

1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

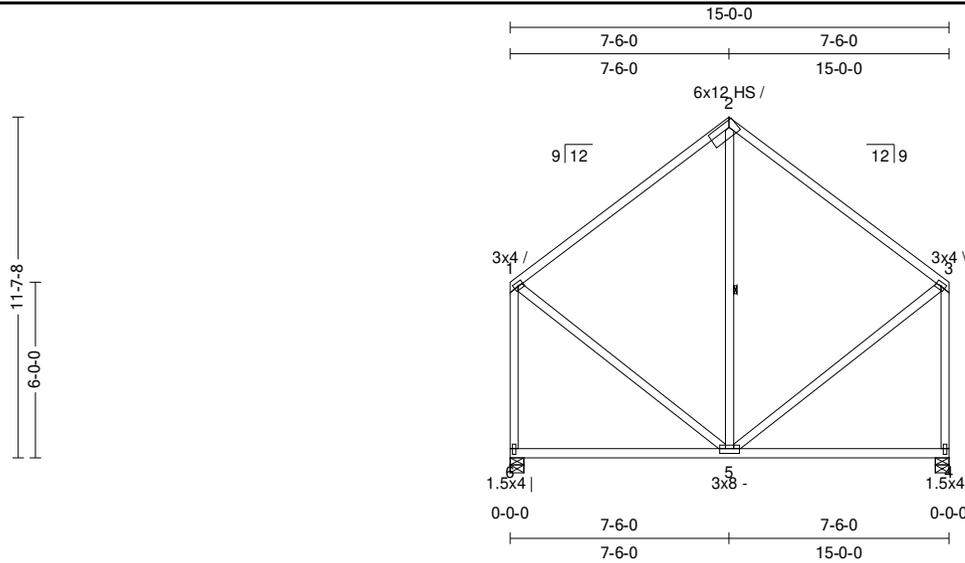
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16" OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) Gable must be sheathed on one side or lateral bracing applied appropriately.
- 8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 9) A creep factor of 2.00 has been applied for this truss analysis.
- 10) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 15-0-0	PITCH 9/12	QTY 22	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 96 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.95 (2-3)	Vert TL: 0.13 in	L/999	(4-5)	L/180
TCDL : 15	TPI 1-2014	BC : 0.53 (4-5)	Vert LL: 0.09 in	L/999	(4-5)	L/240
BCLL : 0	Rep Mbr : Yes	Web : 0.43 (1-6)	Horz TL: 0 in		4	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	690 lbs	.	.	-33 lbs	-33 lbs	127 lbs
4	1	5.5 in	1.50 in	690 lbs	.	.	-33 lbs	-33 lbs	.

Material

TC: DFL #2 2 x 4
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Standard 2 x 4: 1-6, 3-4

Bracing

TC: Sheathed
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-5

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

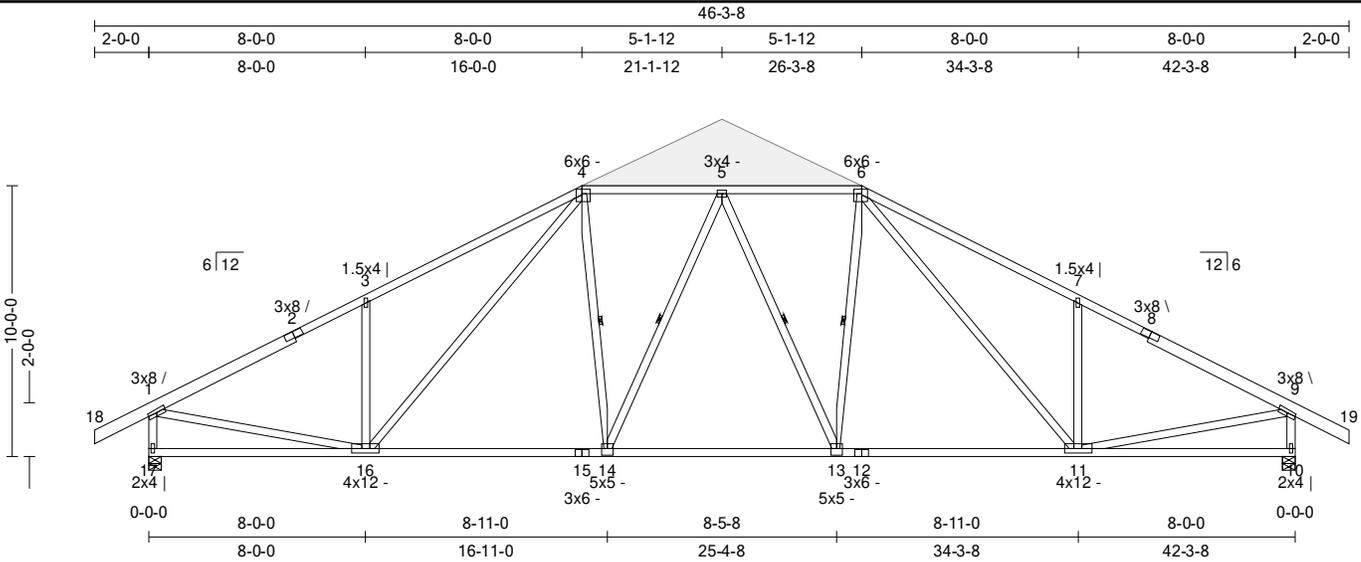
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.952	(476 lbs)	2-3	0.952	(476 lbs)
BC						
Web	1-6	0.430	(-655 lbs)	2-5	0.166	(-322 lbs)
	1-5	0.067	349 lbs	3-5	0.067	349 lbs
				3-4	0.430	(-655 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 42-3-8	PITCH 6/12	QTY 25	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 253 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.84 (7-9)	Vert TL: 0.36 in	L / 999	(11-12)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.95 (13-14)	Vert LL: 0.23 in	L / 999	(11-12)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.44 (9-11)	Horz TL: 0.08 in		10	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	2.25 in	2,105 lbs	.	.	-85 lbs	-85 lbs	56 lbs
10	1	5.5 in	2.25 in	2,105 lbs	.	.	-85 lbs	-85 lbs	.

Material

TC: DFL #2 2 x 4 except:
 DFL SS 2 x 6: 18-2, 8-19
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-17, 3-16, 7-11, 9-10

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 4-14, 5-14, 5-13, 6-13

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

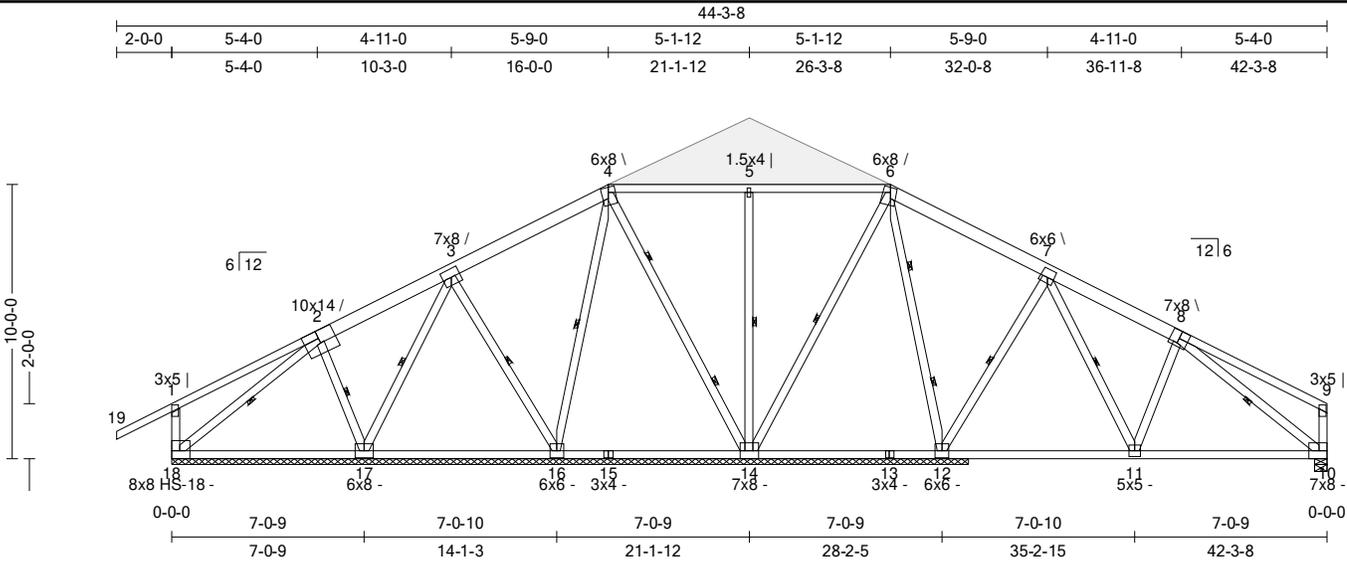
TC	1-3	0.841	(-2,624 lbs)	4-5	0.649	(-2,010 lbs)	6-7	0.798	(-2,629 lbs)
	3-4	0.798	(-2,629 lbs)	5-6	0.649	(-2,010 lbs)	7-9	0.841	(-2,624 lbs)
BC	11-13	0.937	1,987 lbs	14-16	0.937	1,987 lbs			
	13-14	0.948	2,075 lbs						
Web	1-17	0.397	(-2,066 lbs)	4-16	0.138	470 lbs	5-13	0.161	(-350 lbs)
	1-16	0.436	2,270 lbs	4-14	0.087	456 lbs	6-13	0.087	456 lbs
	3-16	0.381	(-648 lbs)	5-14	0.161	(-350 lbs)	6-11	0.138	470 lbs
							7-11	0.381	(-648 lbs)
							9-11	0.436	2,270 lbs
							9-10	0.397	(-2,066 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 42-3-8	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 277 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.82 (8-9)	Vert TL: 0.1 in	L / 999	(10-11)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.80 (11-12)	Vert LL: 0.07 in	L / 999	(11-12)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.93 (2-18)	Horz TL: 0.09 in		10	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	2.68 in	2,515 lbs	-2,023 lbs	.	-16 lbs	-2,023 lbs	.
12	1	350 in	N/A	1,034 lbs	-130 lbs	.	-23 lbs	-130 lbs	7,177 lbs
13	1	350 in	N/A	119 lbs
14	1	350 in	N/A	896 lbs	.	.	-54 lbs	-54 lbs	-3,124 lbs
15	1	350 in	N/A	122 lbs
16	1	350 in	N/A	1,599 lbs	-1,149 lbs	.	-40 lbs	-1,149 lbs	-2,271 lbs
17	1	350 in	N/A	1,038 lbs	-490 lbs	.	.	-490 lbs	-2,625 lbs
18	1	350 in	N/A	3,985 lbs	-3,584 lbs	.	-50 lbs	-3,584 lbs	5,967 lbs

Material

TC: DFL 2400/2.0 2 x 4 except:
 DFL SS 2 x 6: 2-4, 6-8
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL 2400/2.0 2 x 4: 2-18, 4-14, 6-14
 DFL Stud 2 x 4: 1-18, 2-17, 8-11, 9-10

Bracing

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 2-6-0, Purlin design by Others.
 Web: One Midpoint Row: 2-18, 2-17, 3-17, 3-16, 4-16, 5-14, 6-14, 7-12, 7-11, 8-10
 Two Third Point Rows: 4-14, 6-12

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects due to a 21.013 lbs (435 plf) drag load distributed along the TC rake from each direction.
- 3) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 4) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 7) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	19-1	0.560	962 lbs	(-905 lbs)	3-4	0.698	4,676 lbs	(-4,664 lbs)	6-7	0.432	1,453 lbs	(-1,338 lbs)
	1-2	0.673	3,739 lbs	(-3,671 lbs)	4-5	0.502	3,652 lbs	(-3,484 lbs)	7-8	0.394	1,311 lbs	(-1,686 lbs)
	2-3	0.514	3,556 lbs	(-3,578 lbs)	5-6	0.350	1,510 lbs	(-1,342 lbs)	8-9	0.817	2,596 lbs	(-2,513 lbs)
BC	10-11	0.576	3,527 lbs	(-3,177 lbs)								
	11-12	0.803	5,181 lbs	(-5,077 lbs)								
Web	1-18	0.241	1,001 lbs	(-1,288 lbs)	3-16	0.908	3,524 lbs	(-3,738 lbs)	6-14	0.925	3,173 lbs	(-3,305 lbs)
	2-18	0.929	7,682 lbs	(-7,754 lbs)	4-16	0.704	1,716 lbs	(-1,868 lbs)	6-12	0.877	2,786 lbs	(-3,101 lbs)
	2-17	0.650	2,704 lbs	(-2,948 lbs)	4-14	0.785	3,475 lbs	(-3,731 lbs)	7-12	0.718	2,403 lbs	(-2,954 lbs)
	3-17	0.858	3,673 lbs	(-3,812 lbs)	5-14	0.158		(-407 lbs)	7-11	0.469	2,495 lbs	(-2,082 lbs)

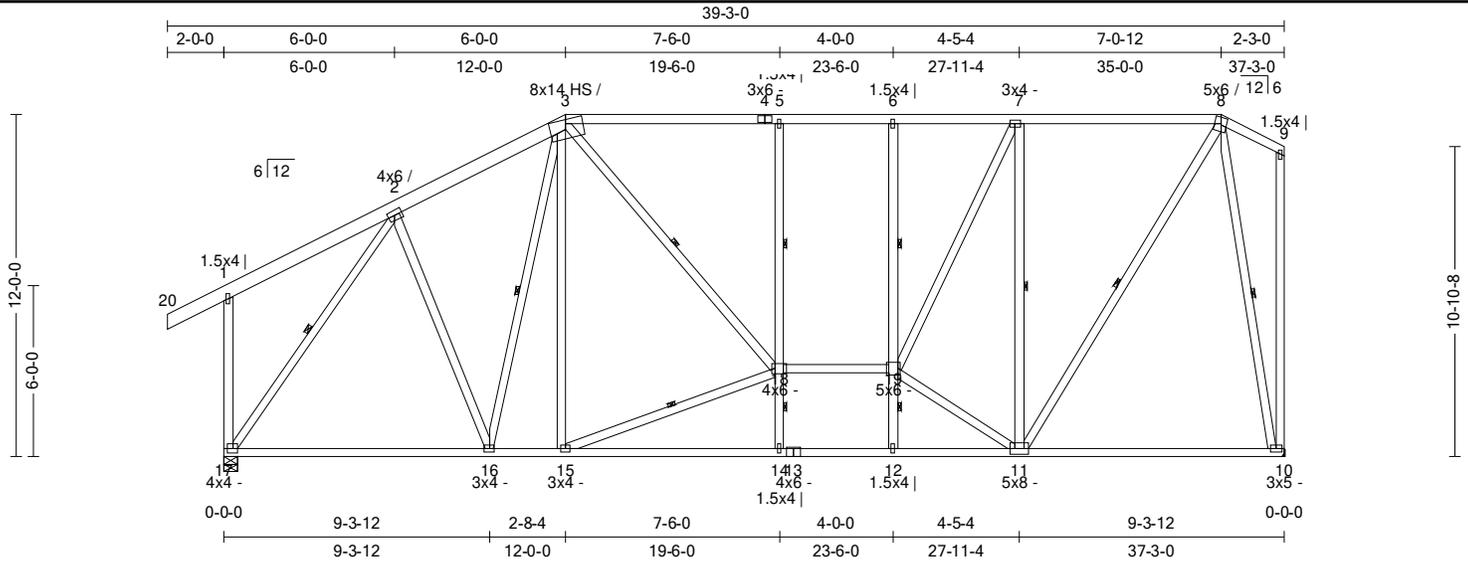
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SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
42-3-8	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	277 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 10, 12, 16, 17, 18 may need to be considered.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 37-3-0	PITCH 6/12	QTY 2	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 311 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 1.00 (3-5)	Vert TL: 0.61 in	L/720	(14-15)	L/180
TCDL : 15	TPI 1-2014	BC : 0.95 (11-12)	Vert LL: 0.36 in	L/999	(10-11)	L/240
BCLL : 0	Rep Mbr : No	Web : 0.97 (8-10)	Horz TL: 0.04 in		10	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	2.00 in	1,878 lbs	.	.	-82 lbs	-82 lbs	194 lbs
10	1	1.5 in	--	1,709 lbs	.	.	-83 lbs	-83 lbs	.

Material

TC: DFL #1B 2 x 4 except:
 DFL SS 2 x 6: 20-3
 BC: DFL #1B 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-17, 18-19, 19-11

Bracing

TC: Sheathed
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-17, 3-16, 3-18, 18-15, 5-14, 6-12, 7-11, 8-11, 8-10

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.332	(-1,356 lbs)	5-6	0.667	(-1,337 lbs)	7-8	0.491	(-1,018 lbs)
	3-5	1.000	(-1,337 lbs)	6-7	0.667	(-1,336 lbs)			
BC	10-11	0.721	309 lbs	12-14	0.951	1,213 lbs	15-16	0.807	1,226 lbs
	11-12	0.952	1,215 lbs	14-15	0.735	1,212 lbs	16-17	0.781	960 lbs
Web	1-17	0.273	(-404 lbs)	5-18	0.117	(-341 lbs)	8-11	0.265	1,377 lbs
	2-17	0.743	(-1,720 lbs)	6-19	0.170	(-496 lbs)	8-10	0.971	(-1,720 lbs)
	2-16	0.090	466 lbs	19-12	0.074	332 lbs			
	3-16	0.210	(-386 lbs)	7-19	0.136	707 lbs			
	3-18	0.070	365 lbs	7-11	0.573	(-1,033 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.

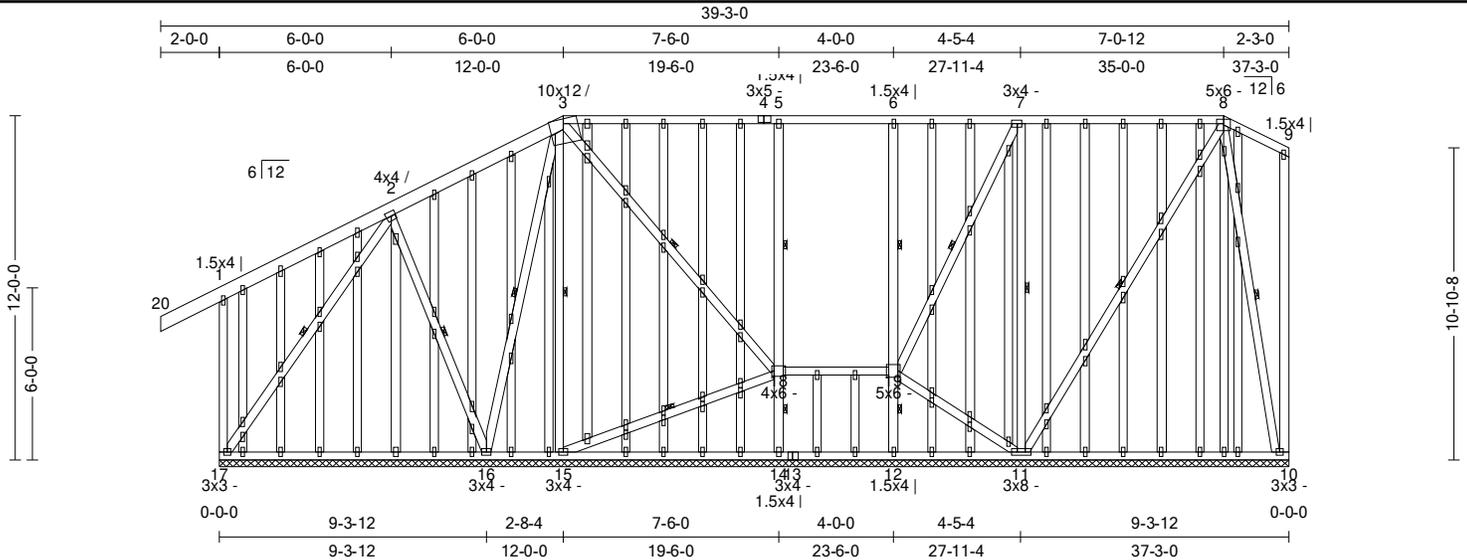
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
37-3-0	6/12	2	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	311 lbs

- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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SPAN 37-3-0	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 630 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25 TCDL : 15 BCLL : 0 BCDL : 6	Bldg Code : IBC 2021/ TPI 1-2014 Rep Mbr : No Lumber D.O.L. : 115 %	TC : 0.85 (3-5) BC : 0.95 (16-17) Web : 0.58 (9-10)	Vert TL: 0.52 in Vert LL: 0.39 in Horz TL: 0 in	L / 865 L / 999	(10-11) (10-11)	L / 180 L / 240

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	447 in	N/A	385 lbs	.	.	-27 lbs	-27 lbs	-96 lbs
1	447 in	N/A	641 lbs	.	.	-4 lbs	-4 lbs	42 lbs
1	447 in	N/A	398 lbs	.	.	-30 lbs	-30 lbs	.
1	447 in	N/A	0 lbs	-145 lbs	-33 lbs	-33 lbs	-145 lbs	.
1	447 in	N/A	745 lbs	.	.	-33 lbs	-33 lbs	.
1	447 in	N/A	314 lbs	50 lbs
1	447 in	N/A	634 lbs	.	.	-20 lbs	-20 lbs	-73 lbs
1	447 in	N/A	606 lbs	.	.	-37 lbs	-37 lbs	112 lbs

Material

TC: DFL #1B 2 x 4 except:
 DFL SS 2 x 6: 20-3
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-17, 18-19, 19-11

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-17, 2-16, 3-16, 3-15, 3-18, 18-15, 5-14, 6-12, 7-19, 7-11, 8-11, 8-10

Loads

- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC						
BC						
Web	1-17	0.273	(-395 lbs)	7-11	0.206	(-372 lbs)
	2-16	0.118	(-340 lbs)	8-10	0.210	(-371 lbs)
	5-18	0.169	(-493 lbs)			
	18-14	0.070	(-606 lbs)			
	19-12	0.044	(-381 lbs)			

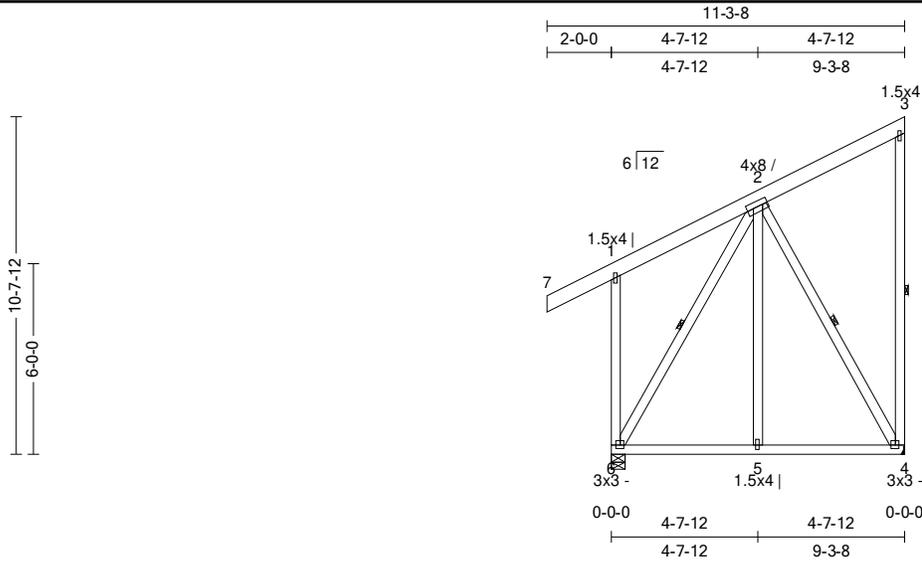
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
37-3-0	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	630 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 16 "OC, U.N.O.
- 3) Attach structural gable blocks with 1.5x4 20ga plates, U.N.O.
- 4) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 5) Provide adequate drainage to prevent ponding.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 9-3-8	PITCH 6/12	QTY 2	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 88 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.18 (7-1)	Vert TL: 0.02 in	L / 999	(4-5)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.28 (5-6)	Vert LL: 0.02 in	L / 999	(4-5)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.64 (3-4)	Horz TL: 0 in		4	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	630 lbs	.	.	-53 lbs	-53 lbs	185 lbs
4	1	1.5 in	--	407 lbs	.	-38 lbs	-149 lbs	-149 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-6, 2-4, 3-4

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

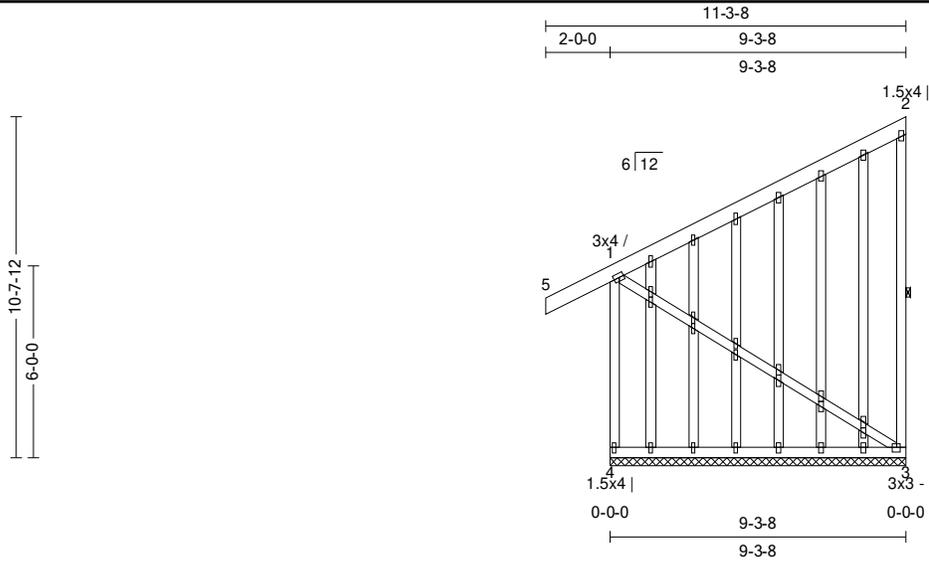
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	TC	BC	Web
1-6	0.390	(-436 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 9-3-8	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 128 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.51 (1-2)	Vert TL: 0.51 in	L/ 219	(3-4)	L/ 180
TCDL : 15	TPI 1-2014	BC : 0.52 (3-4)	Vert LL: 0.34 in	L/ 326	(3-4)	L/ 240
BCLL : 0	Rep Mbr : No	Web : 0.63 (2-3)	Horz TL: 0 in			
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	111.5 in	N/A	407 lbs		-38 lbs	-149 lbs	-149 lbs	159 lbs
1	111.5 in	N/A	630 lbs			-53 lbs	-53 lbs	75 lbs

Material

TC: DFL SS 2 x 6
 BC: DFL 2400/2.0 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-3

Loads

- This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

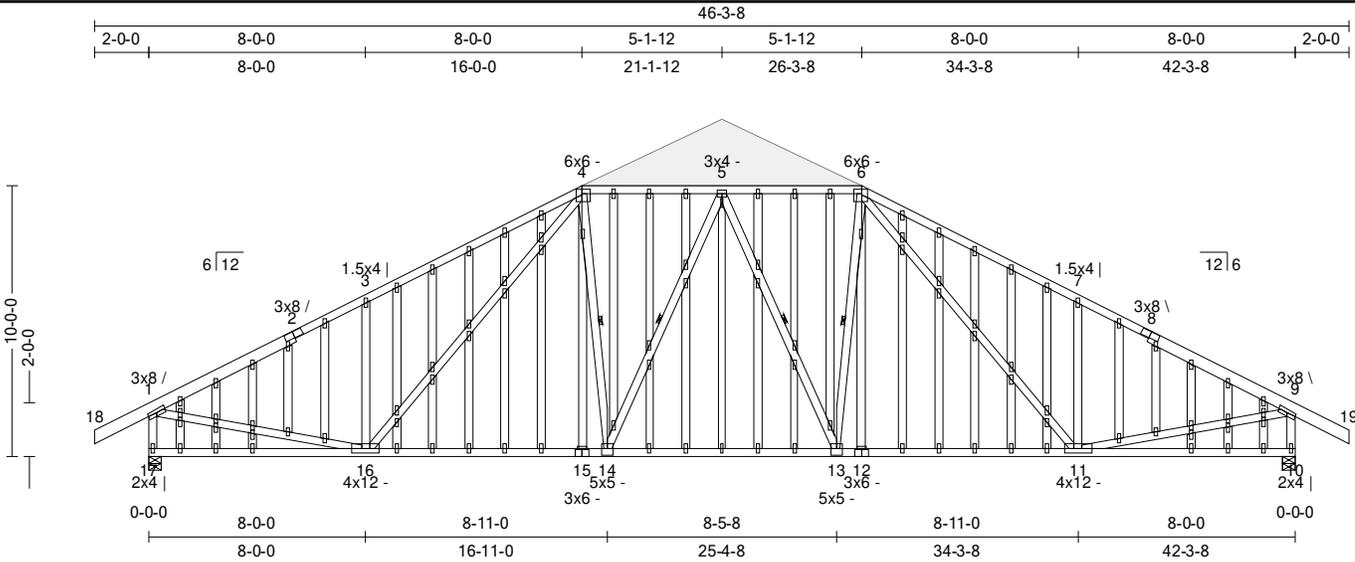
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		1-4
		0.358
		(-574 lbs)
		2-3
		0.635
		(-351 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 16" OC, U.N.O.
- Attach structural gable blocks with 1.5x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Gable must be sheathed on one side or lateral bracing applied appropriately.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 42-3-8	PITCH 6/12	QTY 1	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 496 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.97 (7-9)	Vert TL: 0.34 in	L / 999	(11-12)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.80 (13-14)	Vert LL: 0.21 in	L / 999	(11-12)	L / 240
BCLL : 0	Rep Mbr : No	Web : 0.44 (9-11)	Horz TL: 0.07 in		10	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	2.25 in	2,105 lbs			-85 lbs	-85 lbs	56 lbs
10	1	5.5 in	2.25 in	2,105 lbs			-85 lbs	-85 lbs	

Material

TC: DFL #2 2 x 4 except:
 DFL SS 2 x 6: 18-2, 8-19
 BC: DFL #1B 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-17, 3-16, 7-11, 9-10

Bracing

TC: Sheathed or Purlins at 2-5-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 4-14, 5-14, 5-13, 6-13

Loads

- 1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (C_e = 1.0), Thermal (C_t = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- 5) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-3	0.970	(-2,624 lbs)	4-5	0.749	(-2,010 lbs)	6-7	0.922	(-2,628 lbs)
	3-4	0.922	(-2,628 lbs)	5-6	0.749	(-2,010 lbs)	7-9	0.970	(-2,624 lbs)
BC	11-13	0.789	1,987 lbs	14-16	0.789	1,987 lbs			
	13-14	0.798	2,075 lbs						
Web	1-17	0.397	(-2,065 lbs)	4-16	0.138	469 lbs	5-13	0.161	(-350 lbs)
	1-16	0.436	2,269 lbs	4-14	0.088	456 lbs	6-13	0.088	456 lbs
	3-16	0.381	(-648 lbs)	5-14	0.161	(-350 lbs)	6-11	0.138	469 lbs
							7-11	0.381	(-648 lbs)
							9-11	0.436	2,269 lbs
							9-10	0.397	(-2,065 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 16" OC, U.N.O.
- 3) Attach structural gable blocks with 1.5x4 20ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 20 % (C_q = 0.80).
- 6) Gable must be sheathed on one side or lateral bracing applied appropriately.
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: **V6GE**
Job: **2501272**
Date: 01/23/26 09:23:22
Page: 2 of 2

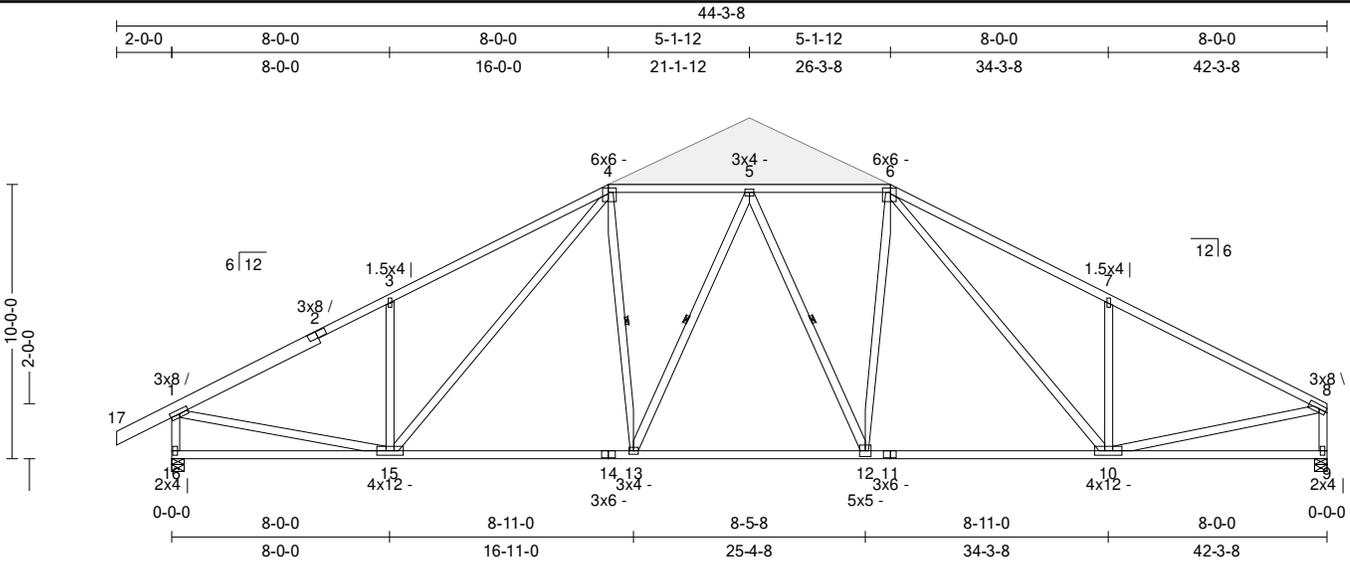
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
42-3-8	6/12	1	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	496 lbs

10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14
Eagle Metal Products

SPAN 42-3-8	PITCH 6/12	QTY 7	OHL 2-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 244 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.86 (7-8)	Vert TL: 0.36 in	L / 999	(10-11)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.95 (12-13)	Vert LL: 0.23 in	L / 999	(10-11)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.45 (8-10)	Horz TL: 0.08 in		9	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	2.25 in	2,110 lbs	.	.	-86 lbs	-86 lbs	70 lbs
9	1	5.5 in	2.07 in	1,941 lbs	.	.	-53 lbs	-53 lbs	.

Material

TC: DFL #1B 2 x 4 except:
 DFL SS 2 x 6: 17-2
 BC: DFL #2 2 x 4
 Web: DFL #2 2 x 4 except:
 DFL Stud 2 x 4: 1-16, 3-15, 7-10, 8-9

Bracing

TC: Sheathed or Purlins at 2-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 4-13, 5-13, 5-12

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1
- Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

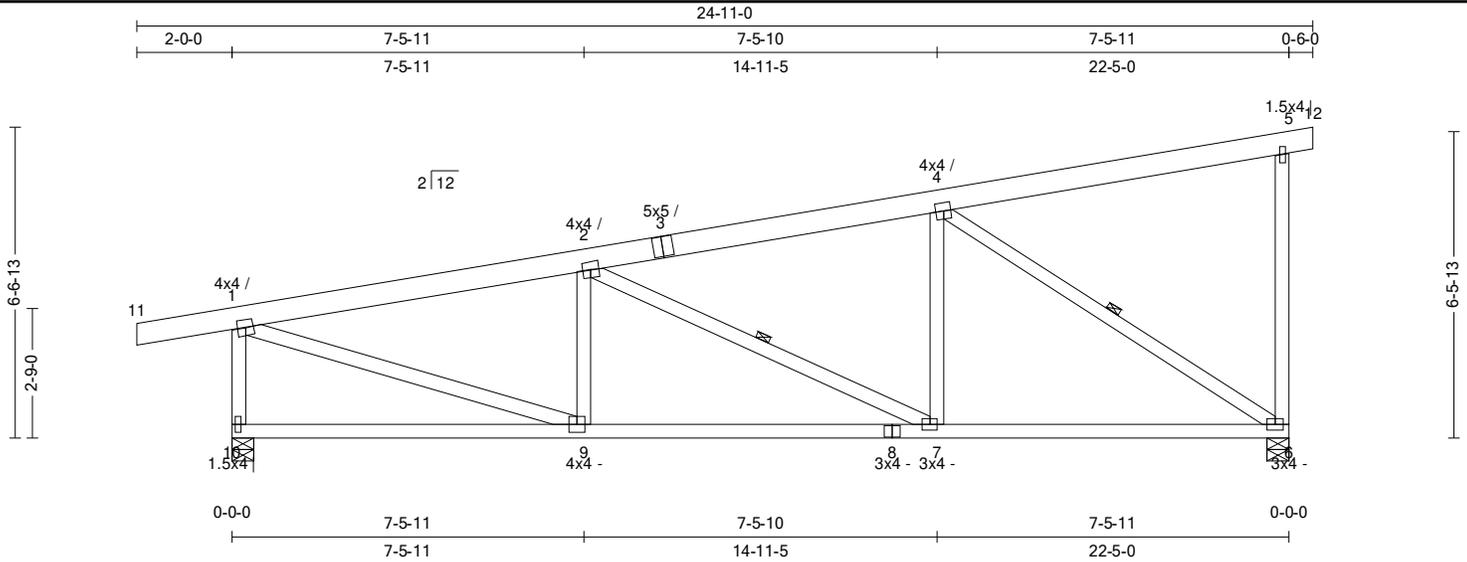
TC	1-3	0.632	(-2,631 lbs)	4-5	0.484	(-2,018 lbs)	6-7	0.729	(-2,690 lbs)
	3-4	0.602	(-2,636 lbs)	5-6	0.438	(-2,019 lbs)	7-8	0.864	(-2,645 lbs)
BC	10-12	0.938	1,995 lbs	13-15	0.937	1,994 lbs			
	12-13	0.950	2,085 lbs						
Web	1-16	0.398	(-2,070 lbs)	4-15	0.137	481 lbs	5-12	0.148	(-323 lbs)
	1-15	0.437	2,276 lbs	4-13	0.088	458 lbs	6-12	0.088	457 lbs
	3-15	0.382	(-650 lbs)	5-13	0.175	(-381 lbs)	6-10	0.186	522 lbs
							7-10	0.428	(-729 lbs)
							8-10	0.446	2,323 lbs
							8-9	0.371	(-1,901 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

SPAN 22-5-0	PITCH 2/12	QTY 5	OHL 2-0-0	OHR 0-6-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 130 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.21 (4-5)	Vert TL: 0.16 in	L / 999	(9-10)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.55 (7-9)	Vert LL: 0.09 in	L / 999	(9-10)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.42 (4-6)	Horz TL: 0.03 in		6	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	1,199 lbs	.	.	-56 lbs	-56 lbs	124 lbs
6	1	5.5 in	1.50 in	1,064 lbs	.	.	-39 lbs	-39 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL #2 2 x 4: 1-9, 2-7, 4-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-7, 4-6

Loads

- This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

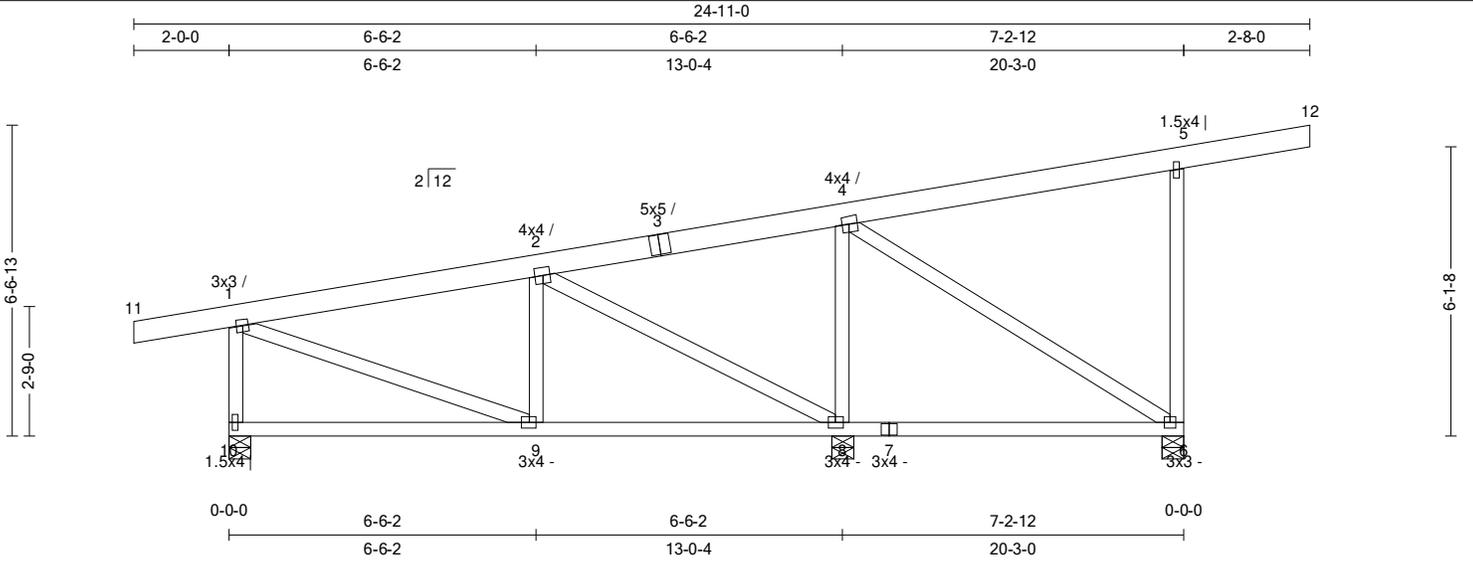
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9
TC	1-2	0.198	(-1,515 lbs)						
	2-4	0.206	(-1,174 lbs)						
BC	6-7	0.508	1,111 lbs	7-9	0.549	1,450 lbs			
Web	1-10	0.240	(-1,160 lbs)	2-9	0.104	(-379 lbs)	4-7	0.126	376 lbs
	1-9	0.293	1,526 lbs	2-7	0.148	(-523 lbs)	4-6	0.418	(-1,334 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

SPAN 20-3-0	PITCH 2/12	QTY 9	OHL 2-0-0	OHR 2-8-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 123 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 25	Bldg Code : IBC 2021/	TC : 0.22 (5-12)	Vert TL: 0.12 in	L / 676	(6-7)	L / 180
TCDL : 15	TPI 1-2014	BC : 0.41 (8-9)	Vert LL: 0.07 in	L / 999	(6-7)	L / 240
BCLL : 0	Rep Mbr : Yes	Web : 0.70 (2-8)	Horz TL: 0 in		8	
BCDL : 6	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	718 lbs	.	.	-68 lbs	-68 lbs	121 lbs
8	1	5.5 in	1.50 in	1,034 lbs
6	1	5.5 in	1.50 in	543 lbs	.	.	-110 lbs	-110 lbs	.

Material

TC: DFL SS 2 x 6
 BC: DFL #2 2 x 4
 Web: DFL Stud 2 x 4 except:
 DFL #2 2 x 4: 4-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9
TC	1-2	0.146	(-572 lbs)						
BC	8-9	0.407	526 lbs						
Web	1-10	0.142	(-684 lbs)	2-8	0.697	(-741 lbs)	5-6	0.315	(-512 lbs)
	1-9	0.188	562 lbs	4-8	0.241	(-630 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) Listed wind uplift reactions based on MWFRS & C&C loading.