Built-in cove

06 10 00-7-0:Care Center Roof Truss

Status	**Open** Submitted
Spec section	06 10 00 Rough Carpentry
Manager	**Joe Geise** (Walsh Construction Co.)
Responsible contractor	**Joe Geise** (Walsh Construction Co.)
Reviewers step 01	**Jill Krance** (In Site Architects) **Kyle Gysler** (AHBL Engineers)

Architect Review:

in site architects

- ☐ NO EXCEPTIONS TAKEN
- ☒ MAKE CORRECTIONS NOTED
- ☐ REVISE & RESUBMIT
- ☐ REJECTED
- ☐ REVIEWED FOR COORDINATION PURPOSES ONLY

BY Jill Krance DATE 12/10/2025

REVIEW IS ONLY FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE CONTRACT DOCUMENT. ANY ACTION SHOWN IS SUBJECT TO REQUIREMENTS OF THE PLANS AND SPECIFICATIONS. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO VERIFY AND CORRELATE ALL DIMENSIONS AND CONDITIONS AT THE JOBSITE, FABRICATION, METHODS, CONSTRUCTION TECHNIQUES AND CONFORMATION AND SATISFACTORY PERFORMANCE OF ALL CONTRACTORS AND SUB-CONTRACTORS.

Other Review:

- ☐ NO EXCEPTIONS TAKEN
- ☑ FURNISH AS CORRECTED
- ☐ REVISE AND RESUBMIT
- ☐ REJECTED SEE REMARKS
- ☐ SUBMIT SPECIFIED ITEM

AHBL www.ahbl.com

Corrections or comments made on the shop drawings during this review do not relieve Contractor from compliance with requirements of the drawings and specifications. This check is only for review of general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. The Contractor is responsible for confirming and correlating all quantities and dimensions, selecting fabrication processes and techniques of construction, coordinating the Work with that of all other trades, and performing the Work in a safe and satisfactory manner.

By: kgysler

Date: 11/25/2025 **Project #:** 2220236.20

Contractor's review of shop drawings, revisions, and other documents from Subcontractor/Supplier does not relieve Subcontractor/Supplier from any of its obligations under the Subcontract/Purchase Order or give rise to any cause of action in favor of Subcontractor/Supplier or third parties against Contractor or Owner.

City of Puyallup
Building
REVIEWED FOR COMPLIANCE
RayC
02/19/2026
11:46:42 AM

CITY OF PUYALLUP STATE OF WASHINGTON

Approval of submitted plans is not an approval of omissions or oversights by this office or non compliance with any applicable regulations of local government. The contractor is responsible for making sure that the building complies with all applicable codes and regulations of the local government.

The approved construction plans, documents, and all engineering must be posted on the job at all inspections in a visible and readily accessible location.

Full sized legible color plans are required to be provided by the permitee on site for inspection.

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

Contractor to note sheet 8 of 129 for notes; Truss Layout

MUSTANG TRUSS

2525 Hyacinth Street NE
Salem, OR 97301
Phone: 503-399-1432

To: EWS - Engineered Wood Solutions

ATT: Engineer of Record
Architect of Record

The contents of this submittal are provided for your review and approval as required by the contract documents. We request that you review the attached documents at your earliest convenience.

PROJECT NAME
Bradley Park Care Center

DATE:
10/29/2025

PROJECT NUMBER
2501272-R

ATTACHMENTS	DATE	PAGES	SPECIFICATIONS
PDF	10/29/2025	5	Roof Truss Take-Off
PDF	10/29/2025	1	Roof Truss Layout
PDF	10/29/2025	21	Roof Truss Sealed Component Drawings

SUBMITAL TYPE

- ☐ Shop Drawings
- ☑ Submittal
- ☐ Resubmittal
- ☐ Final Construction Documents
- ☐ For View Only
- ☐ Other

APPROVAL COMMENTS	Please acknowledge receipt of this submittal and provide your comments and approvals as per the project schedule.

AUTHORIZED SIGNATURE OF APPROVAL

Graham Orthmann

DATE
10/29/2025

Mustang Truss

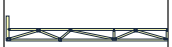
2525 Hyacinth St NE
Salem, OR 97301
Phone: (503) 399 - 1432

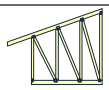
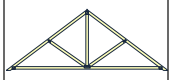
Quote #	**Q2501272-R**
Quote Date	**10/01/2025**
Sales Person	**Graham Orthmann**
Designer	**Anthony Wagner**
PO Number	

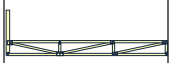
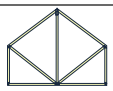
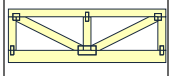
Truss Reaction

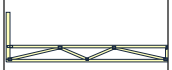
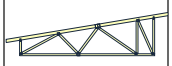
Customer:	**EWS - Engineered Wood Solutions**	Project:	**2501272**
Contact:	**Doug Erickson**	Prj Description:	**Bradley Park Phase 2**
Phone:	**(503) 710 - 7549**	Tkt Description:	**Bradley Park Phase 2**

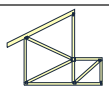
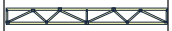
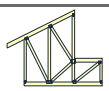
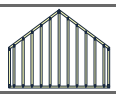
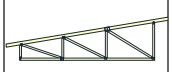
Roof Trusses: Loading: 25 - 7 - 0 - 10 - Building Code: IRC 2021 - Wind Speed: 98

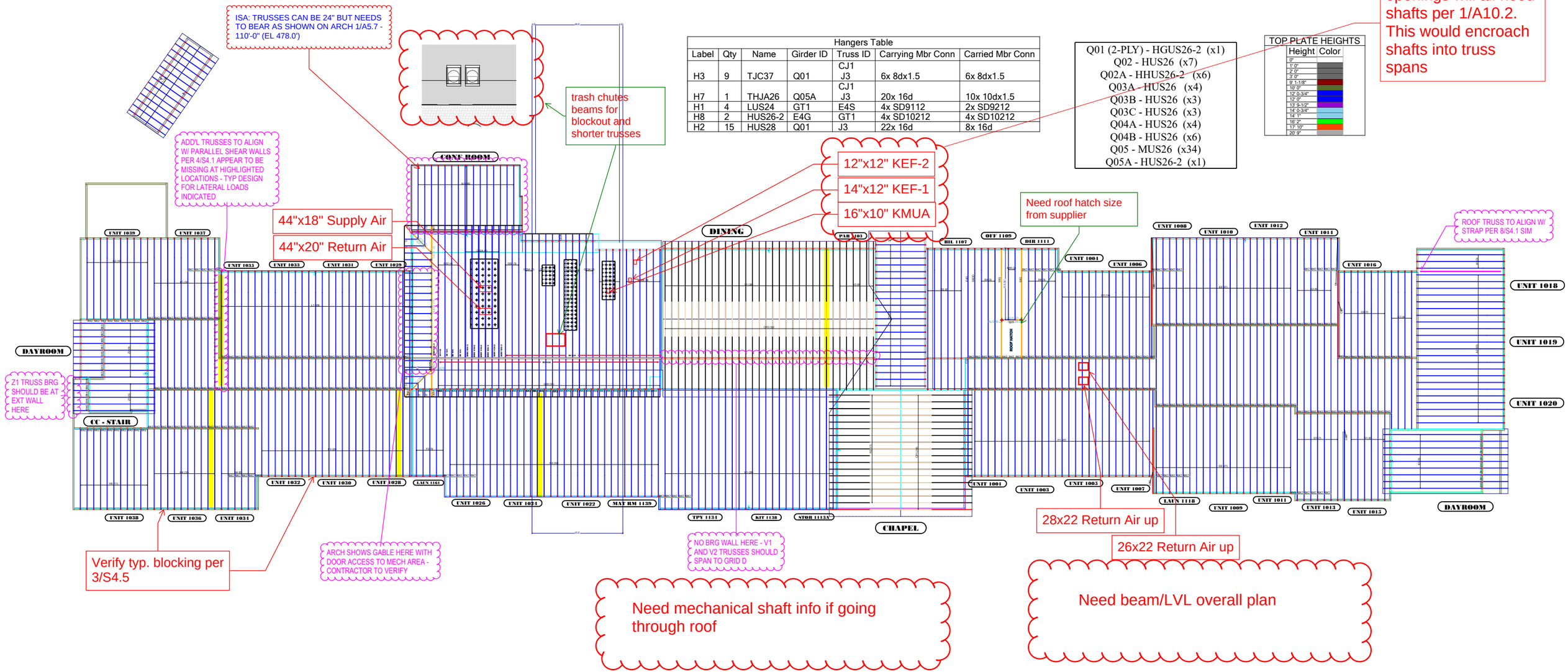
Name	Profile	QTY	Pitch	Unit WGT	Span Spacing	OH Left OH Right	HH Left HH Right	Bearing Locations	Bearing Widths	Bearing Reactions	Bearing Uplift	TC Length Left	TC Length Right
BK9		1		13	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-04-06, Y=00-00-00 X=01-10-08, Y=00-00-00	1.5 5.5 1.5	722 416 320	-673 -355 -304	1-10-8	1-10-8
A1		3		118	26-2-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-02-04, Y=00-00-00	5.5 5.5	1,100 1,100	-133 -133	26-2-4	26-2-4
A2		23		114	26-2-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-02-04, Y=00-00-00	5.5 5.5	1,096 1,104	-124 -157	26-2-4	26-2-4
B1		9	2	200	33-2-14 2-0-0	2-0-0 2-0-0	2-9-0 8-3-8	X=00-00-00, Y=00-00-00 X=27-06-02, Y=00-00-00	5.5 5.5	1,213 1,835	-52 -226	37-9-1	4-0-11
BK1		8		35	1-10-8 2-0-0	0-0-0 0-0-0	7-4-4 7-4-4	X=00-00-00, Y=00-00-00	22.5	2,540	-2,477	1-10-8	1-10-8
BK10		1		12	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-10-08, Y=00-00-00	5.5 1.5	590 585	-521 -527	1-10-8	1-10-8
BK11		1		11	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-10-08, Y=00-00-00	5.5 1.5	91 67	-50 -30	1-10-8	1-10-8
BK3		1		12	1-8-10 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00	20.6	646	-588	1-8-10	1-8-10
BK13		1		12	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-10-08, Y=00-00-00	1.5 9.8	639 639	-575 -575	1-10-8	1-10-8
BK2		257		11	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00	22.5	79	-37	1-10-8	1-10-8
BK4		5		27	1-10-8 2-0-0	0-0-0 0-0-0	5-8-7 5-8-7	X=00-00-00, Y=00-00-00	22.5	1,954	-1,891	1-10-8	1-10-8
BK5		8		24	1-10-8 2-0-0	0-0-0 0-0-0	4-11-4 4-11-4	X=00-00-00, Y=00-00-00	22.5	1,682	-1,619	1-10-8	1-10-8

BK6		1		10	1-3-6 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-03-06, Y=00-00-00	4.0 1.5	685 685	-642 -642	1-3-6	1-3-6
BK7		40		12	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-10-08, Y=00-00-00	1.5 1.5	639 639	-575 -575	1-10-8	1-10-8
BK12		1		12	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-10-08, Y=00-00-00	1.5 16.3	623 634	-591 -580	1-10-8	1-10-8
E5		6		254	42-3-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00 X=42-03-08, Y=00-00-00	5.5 5.5 5.5	1,174 2,737 84	-64 -282 -583	42-3-8	42-3-8
CJ1		2	4.243	100	8-4-5 2-0-5	2-9-15 0-0-0	6-0-0 8-11-8	X=00-00-00, Y=00-00-00 X=03-00-06, Y=07-00-14 X=05-10-05, Y=00-00-00 X=08-04-05, Y=00-00-00	7.0 2.2 2.2 1.5	318 94 258 55	-225 -68 -228 -47	11-10-7	0-0-0
C2		7		238	42-4-2 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=25-10-10, Y=00-00-00 X=42-04-02, Y=00-00-00	5.5 5.5 2.8	933 2,226 551	-57 -207 -34	42-4-2	42-4-2
BK8		1		12	1-10-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=01-10-08, Y=00-00-00	1.5 15.6	624 633	-590 -581	1-10-8	1-10-8
CP1		18	9	71	16-11-1 2-0-0	0-7-13 0-7-13	0-4-6 0-4-6	X=00-00-01, Y=00-00-00	203.0	676	-98	11-4-11	11-4-11
D1		13		120	26-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00	2.8 5.5	1,097 1,089	-176 -105	26-0-4	26-0-4
CP2		32	6	28	8-5-14 2-0-0	0-10-13 0-10-13	0-3-15 0-3-15	X=00-00-00, Y=00-00-00	101.9	258	-57	5-9-1	5-9-1
D1D		1		122	26-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00	2.8 5.5	1,097 1,089	-176 -105	26-0-4	26-0-4
D2		21		117	26-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00	2.8 5.5	1,097 1,089	-176 -105	26-0-4	26-0-4
D3		14		112	26-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00	5.5 2.8	1,089 1,097	-124 -157	26-0-4	26-0-4
D2D		1		173	30-10-0 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=30-10-00, Y=00-00-00 X=00-00-00, Y=00-00-00	1.5 235.0	629 1,106	-161 -122	30-10-0	30-10-0
E1		1		299	52-2-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00 X=52-02-12, Y=00-00-00	5.5 5.5 2.8	929 2,655 943	0 0 0	52-2-12	52-2-12

E2		6		299	52-2-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00 X=52-02-12, Y=00-00-00	5.5 5.5 2.8	929 2,655 943	0 0 0	52-2-12	52-2-12
E4DD		1		145	32-9-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00	5.5 2.8	1,373 1,380	-111 -169	32-9-4	32-9-4
E3		21		289	50-1-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00 X=50-01-12, Y=00-00-00	5.5 5.5 2.8	936 2,546 865	0 0 0	50-1-12	50-1-12
E4G		2		180	32-9-4 3-11-10	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00	5.5 2.8	15,096 22,616	-160 -912	32-9-4	32-9-4
V2		6	6	243	42-3-8 2-0-0	2-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00	5.5 5.5	1,398 2,282	-139 -382	20-1-8	17-10-11
E4S		2		99	21-3-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=21-03-08, Y=00-00-00	5.5 1.5	890 898	-94 -179	21-3-8	21-3-8
C1		4		266	47-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=25-10-10, Y=00-00-00 X=47-00-04, Y=00-00-00	5.5 5.5 1.5	938 2,391 750	0 0 0	47-0-4	47-0-4
E4D		1		145	32-9-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00	5.5 2.8	1,373 1,380	-111 -169	32-9-4	32-9-4
T2		22	9	95	15-0-0 2-0-0	0-0-0 0-0-0	6-0-0 6-0-0	X=00-00-00, Y=00-00-00 X=15-00-00, Y=00-00-00	5.5 5.5	630 630	-69 -69	9-4-8	9-4-8
E6D		1		156	35-9-8 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=35-09-08, Y=00-00-00 X=00-00-00, Y=00-00-00	2.0 315.0	627 1,441	-89 -166	35-9-8	35-9-8
F3		22		112	26-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00	2.8 5.5	1,097 1,089	-157 -124	26-0-4	26-0-4
F1D		1		153	35-3-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=09-03-08, Y=00-00-00 X=35-03-12, Y=00-00-00	2.8 5.5 5.5	1,175 2,118 930	-1,238 -207 -72	35-3-12	35-3-12
F3D		1		113	26-0-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00	2.8 5.5	1,097 1,089	-157 -124	26-0-4	26-0-4
GT1		1		32	5-10-8 1-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=05-10-08, Y=00-00-00	1.5 1.5	8,176 8,176	0 0	5-10-8	5-10-8
F2		5		110	26-2-6 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-02-06, Y=00-00-00	5.5 5.5	1,100 1,100	-147 -147	26-2-6	26-2-6

Name	Profile	QTY	Pitch	Unit WGT	Span Spacing	OH Left OH Right	HH Left HH Right	Bearing Locations	Bearing Widths	Bearing Reactions	Bearing Uplift	TC Length Left	TC Length Right
K1		10		256	45-1-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=24-03-10, Y=00-00-00 X=45-01-12, Y=00-00-00	5.5 5.5 2.8	883 2,283 746	-51 -212 -57	45-1-12	45-1-12
H4		16		157	35-9-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=11-07-00, Y=00-00-00 X=35-09-04, Y=00-00-00	2.8 5.5 5.5	344 1,964 875	-99 -217 -42	35-9-4	35-9-4
H1		26		184	35-9-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=35-09-04, Y=00-00-00	2.8 5.5	1,506 1,498	-165 -112	35-9-4	35-9-4
H5		11		110	24-5-0 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=24-05-00, Y=00-00-00	5.5 5.5	1,029 1,022	-177 -102	24-5-0	24-5-0
H2		32		167	32-3-0 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-03-00, Y=00-00-00	5.5 5.5	1,351 1,358	-112 -169	32-3-0	32-3-0
J1		4	6	26	1-10-15 2-0-0	2-0-0 0-0-0	4-0-0 4-11-8	X=00-00-00, Y=00-00-00 X=01-10-15, Y=00-00-00	5.5 1.5	343 73	-57 -194	4-4-8	0-0-0
J2		4	6	31	3-10-15 2-0-0	2-0-0 0-0-0	4-0-0 5-11-8	X=00-00-00, Y=00-00-00 X=03-10-15, Y=00-00-00	5.5 1.5	365 151	-113 -193	6-7-5	0-0-0
J3		17	6	53	6-0-0 2-0-0	2-0-0 0-0-0	6-0-0 9-0-0	X=00-00-00, Y=00-00-00 X=06-00-00, Y=00-00-00	5.5 1.5	443 231	-82 -211	8-11-5	0-0-0
K2		10		106	24-6-6 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=24-06-06, Y=00-00-00	5.5 5.5	1,026 1,034	-122 -157	24-6-6	24-6-6
L1		29		152	35-3-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=26-00-04, Y=00-00-00 X=35-03-12, Y=00-00-00	5.5 5.5 2.8	928 2,126 189	-67 -223 -308	35-3-12	35-3-12
N1		16		89	19-7-1 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=19-07-01, Y=00-00-00	3.5 4.0	819 827	-89 -181	19-7-1	19-7-1
E4		8		145	32-9-4 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00	5.5 2.8	1,373 1,380	-111 -169	32-9-4	32-9-4
Q02A		6	6	273	37-3-0 2-0-0	2-0-0 0-0-0	6-0-0 3-3-0	X=00-00-00, Y=00-00-00 X=37-03-00, Y=00-00-00	5.5 1.5	31,792 28,352	-2,712 -1,800	11-2-3	31-3-0
Z1		5	2	127	20-3-0 2-0-0	2-0-0 2-0-0	2-9-0 6-1-8	X=00-00-00, Y=00-00-00 X=17-07-06, Y=00-00-00	5.5 5.5	839 1,118	-111 -235	24-7-0	4-0-11
Q02		7	6	246	37-3-0 2-0-0	2-0-0 0-0-0	6-0-0 3-3-0	X=00-00-00, Y=00-00-00 X=37-03-00, Y=00-00-00	5.5 1.5	1,801 1,584	-338 -225	11-2-3	31-3-0
P1		14	6	46	9-2-12 2-0-0	2-0-0 2-0-0	1-1-8 1-1-8	X=00-00-00, Y=00-00-00 X=09-02-12, Y=00-00-00	5.5 5.5	545 545	-184 -184	7-4-12	7-4-12

F1		27		152	35-3-12 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00 X=09-03-08, Y=00-00-00 X=35-03-12, Y=00-00-00	2.8 5.5 5.5	189 2,126 928	-308 -208 -71	35-3-12	35-3-12
Q03B		3	6	234	34-9-8 2-0-0	2-0-0 0-0-0	7-2-12 3-3-0	X=00-00-00, Y=00-00-00 X=34-09-08, Y=00-00-00	5.5 1.5	2,116 1,662	-358 -220	8-5-3	31-3-0
Q03A		5	6	233	34-9-8 2-0-0	2-0-0 0-0-0	7-2-12 3-3-0	X=00-00-00, Y=00-00-00 X=34-09-08, Y=00-00-00	5.5 1.5	1,705 1,473	-358 -220	8-5-3	31-3-0
Q05		34	6	76	9-3-8 2-0-0	2-0-0 0-0-0	6-0-0 3-3-0	X=00-00-00, Y=00-00-00 X=09-03-08, Y=00-00-00	5.5 1.5	566 429	-132 -175	8-11-5	3-3-8
Q04A		4		189	32-2-4 2-0-0	0-0-0 0-0-0	3-3-0 3-3-0	X=00-00-00, Y=00-00-00 X=32-02-04, Y=00-00-00	5.5 1.5	2,219 1,685	-147 -147	32-2-4	32-2-4
Q03C		3	6	263	34-9-8 2-0-0	2-0-0 0-0-0	7-2-12 3-3-0	X=00-00-00, Y=00-00-00 X=34-09-08, Y=00-00-00	5.5 1.5	3,225 2,754	-358 -219	8-5-3	31-3-0
Q05A		1	6	96	9-3-8 2-0-0	2-0-0 0-0-0	6-0-0 3-3-0	X=00-00-00, Y=00-00-00 X=09-03-08, Y=00-00-00	5.5 1.5	5,232 6,216	-680 -2,280	8-11-5	3-3-8
R2		17	9	297	38-5-6 2-0-0	2-0-0 2-0-0	4-2-0 4-2-0	X=00-00-00, Y=00-00-00 X=38-05-06, Y=00-00-00	5.5 5.5	1,743 1,743	-124 -124	15-1-11	15-1-11
Q04B		6		185	32-2-4 2-0-0	0-0-0 0-0-0	3-3-0 3-3-0	X=00-00-00, Y=00-00-00 X=32-02-04, Y=00-00-00	5.5 1.5	1,352 1,352	-147 -147	32-2-4	32-2-4
T1		1	9	175	15-0-0 2-0-0	0-0-0 0-0-0	6-0-0 6-0-0	X=00-00-00, Y=00-00-00	180.0	286	-288	9-4-8	9-4-8
R1		1	9	357	38-5-6 2-0-0	2-0-0 2-0-0	4-2-0 4-2-0	X=00-00-00, Y=00-00-00	461.4	226	-38	15-1-11	15-1-11
V1		26	6	243	42-0-12 2-0-0	2-0-0 0-0-0	2-0-0 2-1-6	X=00-00-00, Y=00-00-00 X=32-09-04, Y=00-00-00	5.5 5.5	1,404 2,257	-141 -376	20-1-8	17-7-9
C3D		1		116	26-2-0 2-0-0	0-0-0 0-0-0	2-0-0 2-0-0	X=00-00-00, Y=00-00-00	314.0	2,980	-2,900	26-2-0	26-2-0
Q01		1	6	295	37-3-0 2-0-0	2-0-0 0-0-0	6-0-0 3-3-0	X=00-00-00, Y=00-00-00 X=37-03-00, Y=00-00-00	5.5 1.5	21,840 22,768	-4,264 -6,104	11-2-3	31-3-0
Z2		9	2	122	20-3-0 2-0-0	2-0-0 2-0-0	2-9-0 6-1-8	X=00-00-00, Y=00-00-00 X=13-00-04, Y=00-00-00 X=20-03-00, Y=00-00-00	5.5 5.5 5.5	633 972 405	-119 -51 -125	24-7-0	4-0-11



Hangers Table

Label	Qty	Name	Girder ID	Truss ID	Carrying Mbr Conn	Carried Mbr Conn
H3	9	TJC37	Q01	CJ1 J3	6x 8dx1.5	6x 8dx1.5
H7	1	THJA26	Q05A	CJ1 J3	20x 16d	10x 10dx1.5
H1	4	LUS24	GT1	E4S	4x SD9112	2x SD9212
H8	2	HUS26-2	E4G	GT1	4x SD10212	4x SD10212
H2	15	HUS28	Q01	J3	22x 16d	8x 16d

Builder: EWS - Engineered Wood Solutions

Job: 2501272
707 39th Avenue SE
Puyallup, WA

685580

Designer: Anthony Wagner

10/28/2025

LOADING

TCLL: 25 psf
TCDL: 7 psf
BCLL: 0 psf
BCDL: 10 psf

Total Loads: 42 psf

Snow GSL: 25 psf

Mustang Truss
2525 Hyacinth St. NE
Salem, OR 97301

Office: 503-399-1432
Fax: 503-399-1435
Toll Free: 866-391-8787

MUSTANG TRUSS
Creating the Greatest Value

EAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Mustang Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **2501272** - 1250352
A1, A2, B1, BK1, BK10, BK11, BK12, BK13, BK2, BK3, BK4, BK5, BK6, BK7, BK8, BK9, C1, C2, C3D, CJ1, CP1, CP2, D1, D1D, D2, D2D, D3, E1, E2, E3, E4, E4D, E4DD, E4G, E4S, E5, E6D, F1, F1D, F2, F3, F3D, GT1, H1, H2, H4, H5, J1, J2, J3, K1, K2, L1, N1, P1, Q01, Q02, Q02A, Q03A, Q03B, Q03C, Q04A, Q04B, Q05, Q05A, R1, R2, T1, T2, V1, V2, Z1, Z2

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

Anish Kekre (WA, 24036081)
My license expiration date for the state of WA is 12/27/2027.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

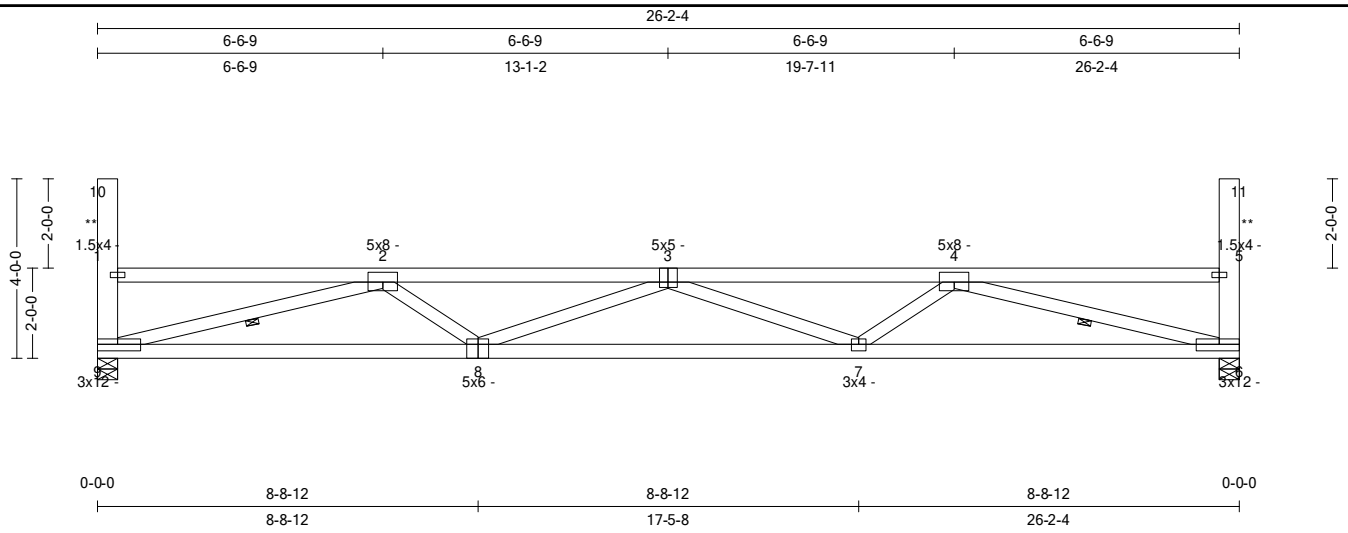
REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: A1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:24
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-4	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.56 (1-2)	Vert TL:	0.93 in	L / 325	(7-8)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.76 (7-8)	Vert LL:	0.38 in	L / 798	(7-8)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.73 (4-6)	Horz TL:	0.16 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,100 lbs	·	·	-133 lbs	-133 lbs	107 lbs
6	1	5.5 in	1.50 in	1,100 lbs	·	·	-133 lbs	-133 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-9, 11-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to members 10-1 & 11-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.520	564 lbs	(-3,650 lbs)	3-4	0.520	564 lbs	(-3,650 lbs)				
BC	6-7	0.675	3,152 lbs	(-488 lbs)	7-8	0.757	4,176 lbs	(-606 lbs)	8-9	0.675	3,152 lbs	(-488 lbs)
Web	2-9	0.727	459 lbs	(-3,265 lbs)	3-7	0.255		(-604 lbs)				
	2-8	0.303	685 lbs		4-7	0.303	685 lbs					
	3-8	0.255		(-604 lbs)	4-6	0.727	459 lbs	(-3,265 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: A1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:25
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-4	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs

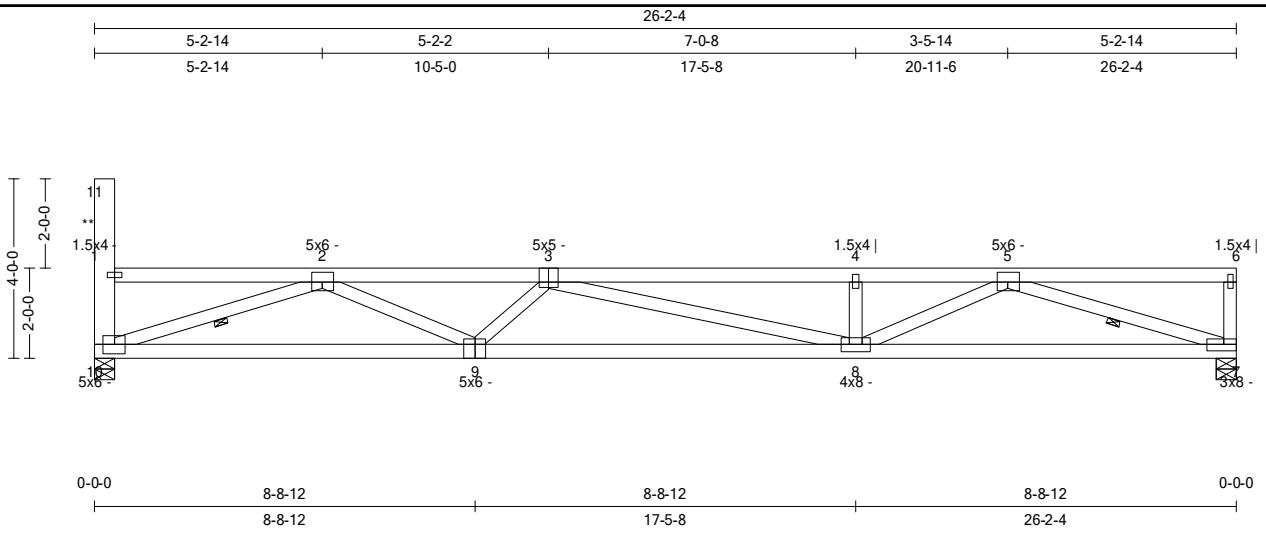
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.16 in, 2L/320 (10-1), Allowable 2L/120.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: A2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:26
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-4	0/12	23	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	114 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.82 (3-4)	Vert TL:	0.85 in	L / 356	(8-9)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.77 (8-9)	Vert LL:	0.34 in	L / 882	(8-9)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.62 (3-8)	Horz TL:	0.14 in		7	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	1,096 lbs	·	·	-124 lbs	-124 lbs	96 lbs
7	1	5.5 in	1.50 in	1,104 lbs	·	·	-157 lbs	-157 lbs	·

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 11-10

Bracing

TC: Sheathed or Purlins at 2-1-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.
Web: One Midpoint Row: 2-10, 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 11-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.463	529 lbs	(-3,690 lbs)	4-5	0.416	516 lbs	(-3,818 lbs)				
	3-4	0.820	516 lbs	(-3,818 lbs)								
BC	7-8	0.640	2,594 lbs	(-395 lbs)	8-9	0.765	4,046 lbs	(-602 lbs)	9-10	0.632	2,577 lbs	(-494 lbs)
Web	2-10	0.461	373 lbs	(-2,723 lbs)	3-8	0.620	373 lbs	(-655 lbs)	5-7	0.478	437 lbs	(-2,736 lbs)
	2-9	0.547	1,239 lbs		4-8	0.046		(-358 lbs)				
	3-9	0.077		(-507 lbs)	5-8	0.602	1,363 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: A2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:26
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-4	0/12	23	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	114 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.16 in, 2L/313 (11-1), Allowable 2L/120.

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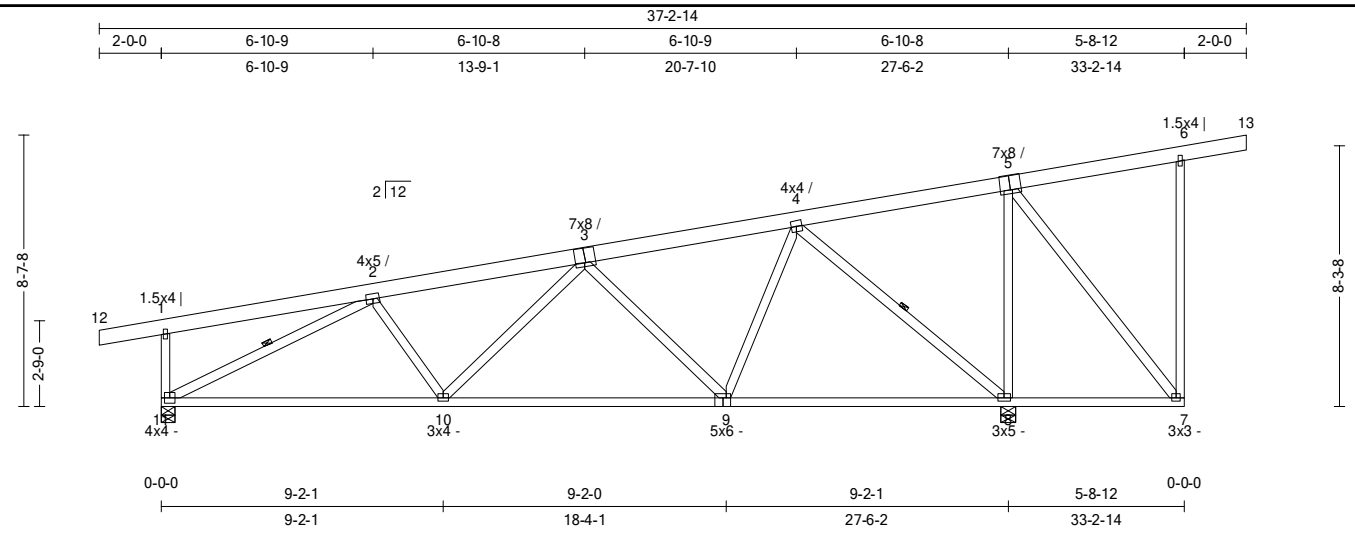
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: B1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:28
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
33-2-14	2/12	9	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	200 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.17 (4-5)	Vert TL:	0.45 in	L / 714	(10-11)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.88 (10-11)	Vert LL:	0.21 in	L / 999	(10-11)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.67 (6-7)	Cant / OH TL:	0.04 in UP	2L / 999	7	2L / 480
BCDL:	10	Lumber D.O.L.:	115 %			Cant / OH LL:	0.04 in UP	2L / 999	7	2L / 480
						Horz TL:	0.07 in		8	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,213 lbs	·	·	-52 lbs	-52 lbs	154 lbs
8	1	5.5 in	1.96 in	1,835 lbs	·	·	-226 lbs	-226 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 4-8, 5-7

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 4-8

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
240360B1
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.127	(-1,785 lbs)	4-5	0.174	311 lbs					
	3-4	0.128	(-1,268 lbs)								
BC	8-9	0.774	965 lbs	9-10	0.880	1,636 lbs		10-11	0.880	1,650 lbs	
Web	1-11	0.108	(-343 lbs)	4-9	0.317	718 lbs		5-7	0.218	426 lbs	
	2-11	0.527	(-1,863 lbs)	4-8	0.450		(-1,352 lbs)				
	3-9	0.510	(-672 lbs)	5-8	0.661		(-796 lbs)				

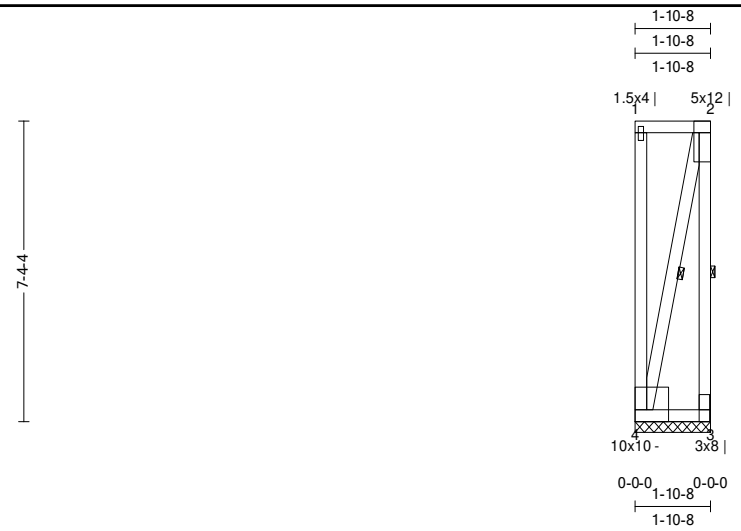
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: BK1 Job: 2501272 Designer:Anthony Date: 10/28/25 13:51:29 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	8	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	35 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL :	0	Rep Mbr :	Yes	Web :	0.82 (2-4)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

10/28/2025

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	2,540 lbs	-2,477 lbs	-259 lbs	-37 lbs	-2,477 lbs	-102 lbs
1	22.5 in	N/A	2,540 lbs	-2,477 lbs	-259 lbs	-37 lbs	-2,477 lbs	563 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL #2 2 x 4 except:
DFL Standard 2 x 4: 2-4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-4, 2-3

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.065	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.816	2,571 lbs	(-2,571 lbs)	2-3	0.524	2,495 lbs	(-2,522 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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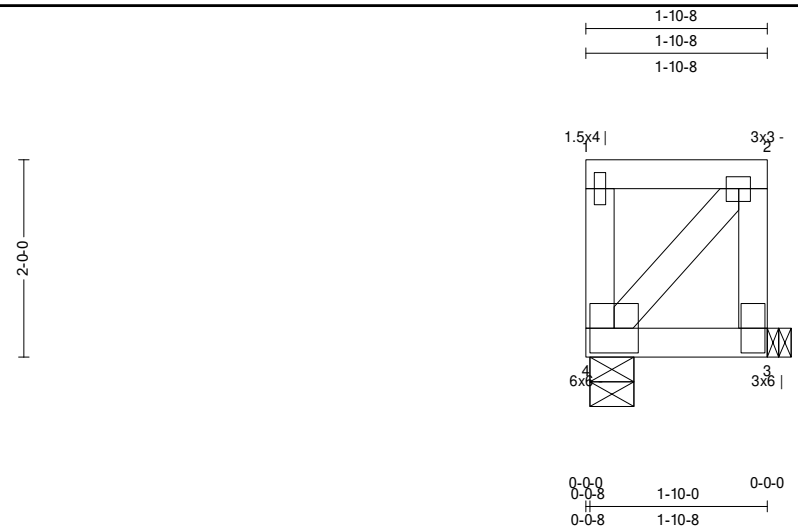
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK10
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:30
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-8	0-0-0	1	24 in	12 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL:	7			BC:	0.15 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.26 (2-4)	Cant / OH TL:	0 in UP	2L / 999	4	2L / 480
BCDL:	10	Lumber D.O.L.:	115 %			Cant / OH LL:	0 in UP	2L / 999	4	2L / 480
						Horz TL:	0 in		3	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	590 lbs	-521 lbs	-11 lbs	-41 lbs	-521 lbs	563 lbs
3	1	1.5 in	1.50 in	585 lbs	-527 lbs	-11 lbs	-33 lbs	-527 lbs	·

10/28/2025

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.072	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.263	827 lbs	(-827 lbs)	2-3	0.189	594 lbs	(-620 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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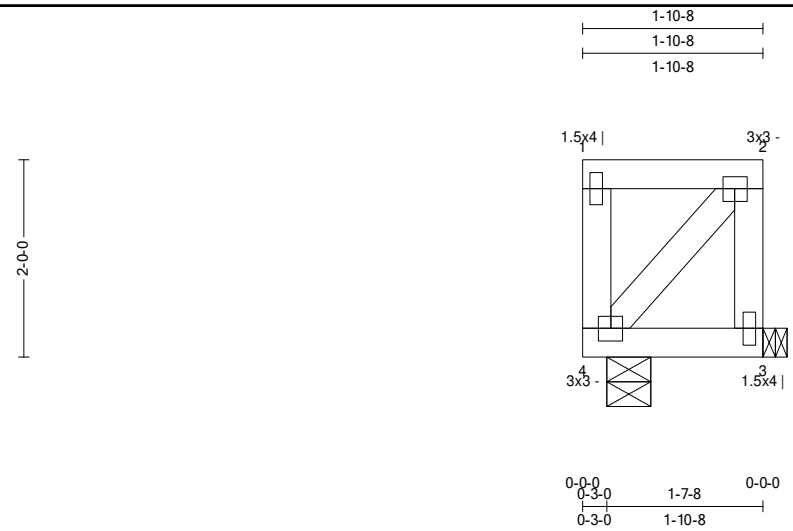
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK11
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:31
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-3-0	0-0-0	1	24 in	11 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.05 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.07 (1-4)	Cant / OH TL:	0 in UP	2L / 999	4	2L / 480
BCDL:	10	Lumber D.O.L.:	115 %			Cant / OH LL:	0 in UP	2L / 999	(4-4)	2L / 480
						Horz TL:	0 in		3	

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	91 lbs	·	-13 lbs	-50 lbs	-50 lbs	-32 lbs
3	1	1.5 in	1.50 in	67 lbs	·	-12 lbs	-30 lbs	-30 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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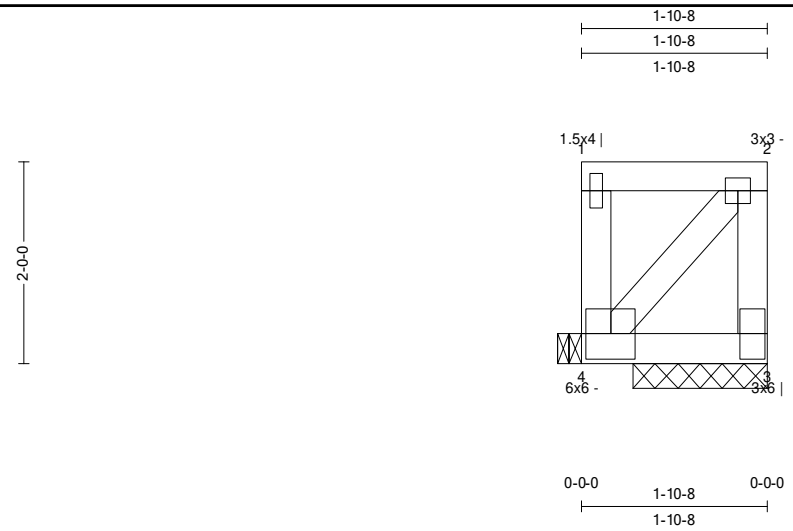
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK12
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:32
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	12 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-3)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.01 (3-4)	Vert LL:	0 in	L / 999	3	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.26 (2-4)	Horz TL:	0 in		3	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	623 lbs	-591 lbs	-22 lbs	-46 lbs	-591 lbs	563 lbs
3	1	16.25 in	N/A	634 lbs	-580 lbs	-15 lbs	-40 lbs	-580 lbs	·
3	1	16.25 in	N/A	43 lbs	·	·	·	·	·

10/28/2025

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.072	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.263	827 lbs	(-827 lbs)	2-3	0.189	594 lbs	(-620 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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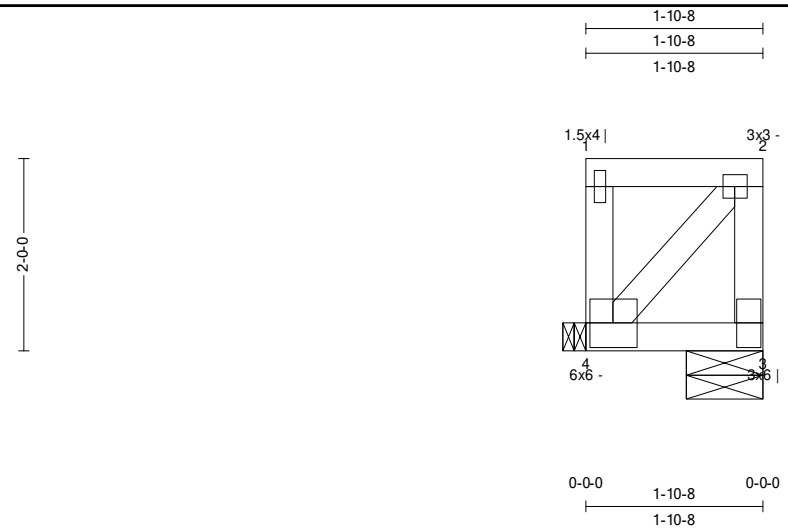
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: BK13 Job: 2501272 Designer:Anthony Date: 10/28/25 13:51:33 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	12 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL :	0	Rep Mbr :	No	Web :	0.26 (2-4)	Horz TL:	0 in		3	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	639 lbs	-575 lbs	-12 lbs	-37 lbs	-575 lbs	563 lbs
3	1	9.75 in	1.50 in	639 lbs	-575 lbs	-12 lbs	-37 lbs	-575 lbs	·

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.072	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.263	827 lbs	(-827 lbs)	2-3	0.189	594 lbs	(-620 lbs)

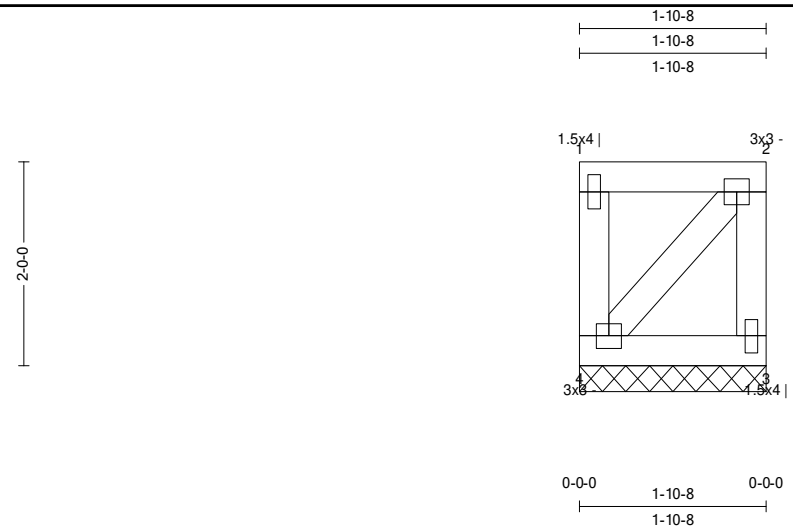
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.11
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: BK2 Job: 2501272 Designer:Anthony Date: 10/28/25 13:51:35 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	257	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	11 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.04 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL :	0	Rep Mbr :	Yes	Web :	0.07 (1-4)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	79 lbs	·	-12 lbs	-37 lbs	-37 lbs	-28 lbs
1	22.5 in	N/A	79 lbs	·	-12 lbs	-37 lbs	-37 lbs	28 lbs

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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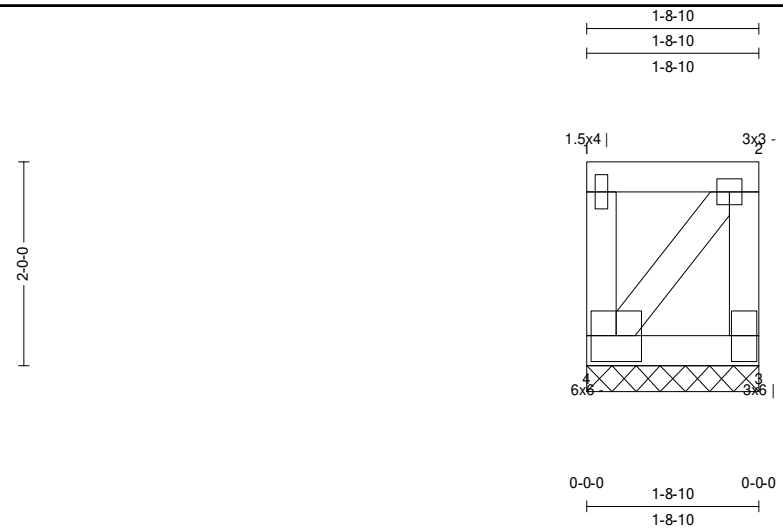
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:36
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-8-10	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	12 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.02 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.26 (2-4)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	20.625 in	N/A	646 lbs	-588 lbs	-14 lbs	-34 lbs	-588 lbs	-28 lbs
1	20.625 in	N/A	646 lbs	-588 lbs	-14 lbs	-34 lbs	-588 lbs	516 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 516 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.065	472 lbs	(-472 lbs)				
BC								
Web	2-4	0.255	804 lbs	(-804 lbs)	2-3	0.192	605 lbs	(-629 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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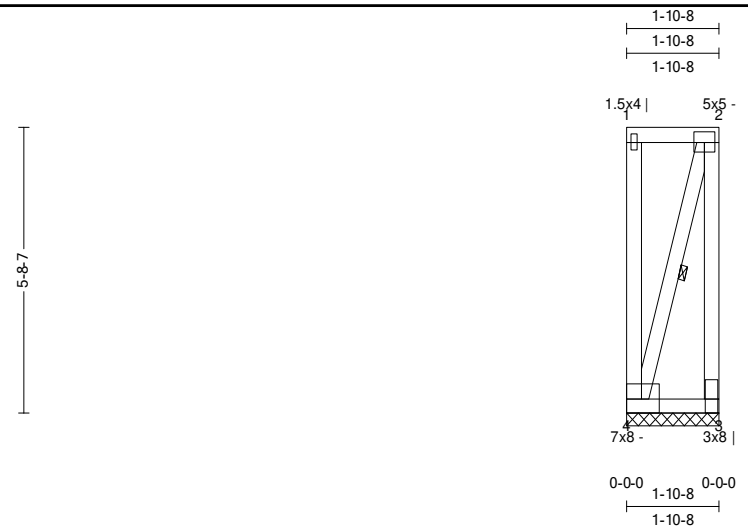
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK4
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:37
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	5	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	27 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL: 7		TPI 1-2014	BC:	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL: 0	Rep Mbr:	Yes	Web:	0.98 (2-3)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	1,954 lbs	-1,891 lbs	-152 lbs	-37 lbs	-1,891 lbs	-79 lbs
1	22.5 in	N/A	1,954 lbs	-1,891 lbs	-152 lbs	-37 lbs	-1,891 lbs	563 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-4

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.065	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.636	2,003 lbs	(-2,003 lbs)	2-3	0.981	1,909 lbs	(-1,936 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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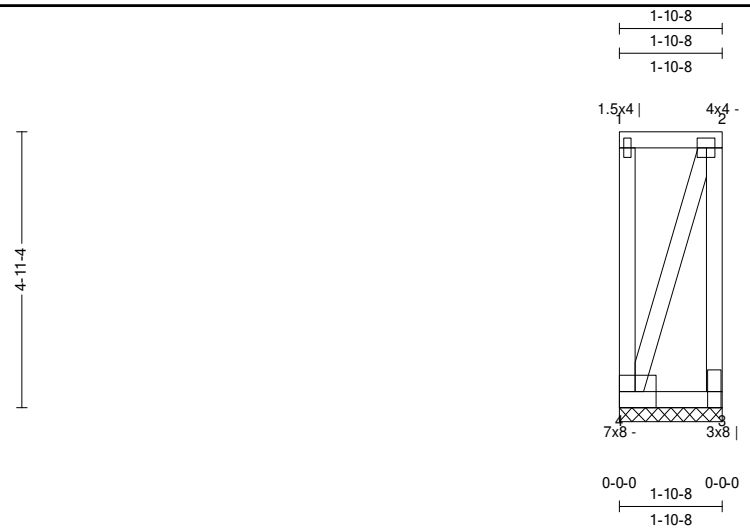
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK5
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:38
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	8	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	24 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.70 (2-4)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	22.5 in	N/A	1,682 lbs	-1,619 lbs	-112 lbs	-37 lbs	-1,619 lbs	-69 lbs
1	22.5 in	N/A	1,682 lbs	-1,619 lbs	-112 lbs	-37 lbs	-1,619 lbs	563 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.065	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.704	1,744 lbs	(-1,744 lbs)	2-3	0.620	1,637 lbs	(-1,664 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 4 may need to be considered.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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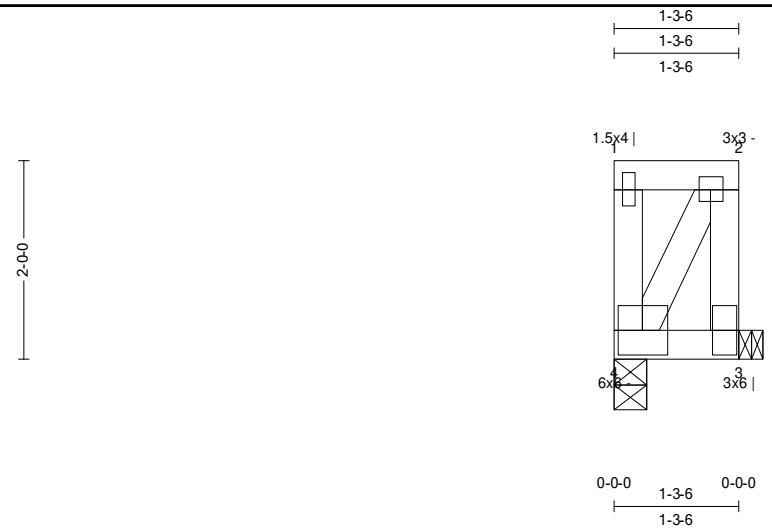
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK6
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:39
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-3-6	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	10 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.05 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.01 (3-4)	Vert LL:	0 in	L / 999	3	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.24 (2-4)	Horz TL:	0 in		3	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	4 in	1.50 in	685 lbs	-642 lbs	-24 lbs	-25 lbs	-642 lbs	384 lbs
3	1	1.5 in	1.50 in	685 lbs	-642 lbs	-24 lbs	-25 lbs	-642 lbs	·

10/28/2025

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 384 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.047	341 lbs	(-341 lbs)				
BC								
Web	2-4	0.243	767 lbs	(-767 lbs)	2-3	0.208	655 lbs	(-673 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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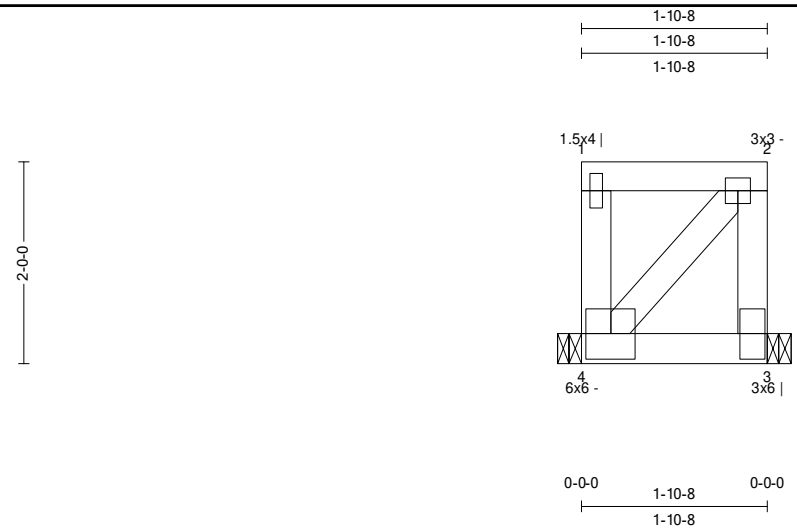
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: BK7
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:40
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	40	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	12 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.26 (2-4)	Horz TL:	0 in		3	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	639 lbs	-575 lbs	-12 lbs	-37 lbs	-575 lbs	563 lbs
3	1	1.5 in	1.50 in	639 lbs	-575 lbs	-12 lbs	-37 lbs	-575 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.065	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.263	827 lbs	(-827 lbs)	2-3	0.189	594 lbs	(-620 lbs)

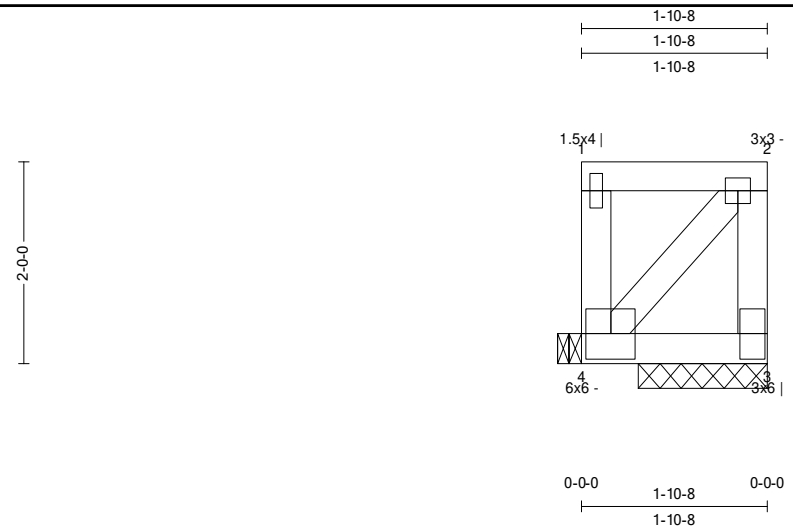
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.11
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: BK8 Job: 2501272 Designer:Anthony Date: 10/28/25 13:51:41 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	12 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.07 (1-2)	Vert TL:	0 in	L / 999	(3-3)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.01 (3-4)	Vert LL:	0 in	L / 999	3	L / 240
BCLL :	0	Rep Mbr :	No	Web :	0.26 (2-4)	Horz TL:	0 in		3	
BCDL :	10	Lumber D.O.L. :	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	624 lbs	-590 lbs	-21 lbs	-45 lbs	-590 lbs	563 lbs
3	1	15.625 in	N/A	633 lbs	-581 lbs	-15 lbs	-40 lbs	-581 lbs	·
3	1	15.625 in	N/A	41 lbs	·	·	·	·	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.072	519 lbs	(-519 lbs)				
BC								
Web	2-4	0.263	827 lbs	(-827 lbs)	2-3	0.189	594 lbs	(-620 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 4, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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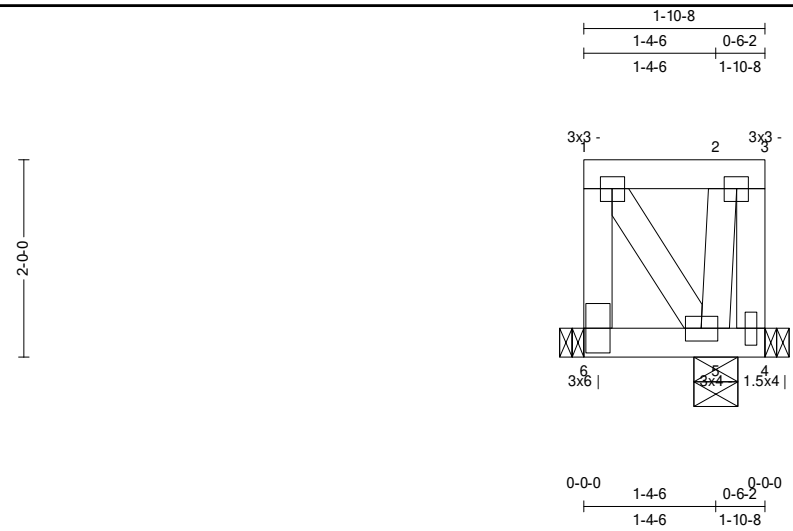
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: BK9 Job: 2501272 Designer:Anthony Date: 10/28/25 13:51:43 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.06 (1-3)	Vert TL:	0 in	L / 999	(5-6)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.08 (5-6)	Vert LL:	0 in	L / 999	(5-6)	L / 240
BCLL :	0	Rep Mbr :	No	Web :	0.27 (1-5)	Horz TL:	0 in		4	
BCDL :	10	Lumber D.O.L. :	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	1.50 in	722 lbs	-673 lbs	-17 lbs	-31 lbs	-673 lbs	-563 lbs
5	1	5.5 in	1.50 in	416 lbs	-355 lbs	-3 lbs	-24 lbs	-355 lbs	·
4	1	1.5 in	1.50 in	320 lbs	-304 lbs	-10 lbs	-18 lbs	-304 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-3-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 563 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-3	0.063	452 lbs	(-449 lbs)											
BC	5-6	0.083	563 lbs	(-563 lbs)											
Web	1-6	0.218	687 lbs	(-708 lbs)	1-5	0.271	852 lbs	(-858 lbs)	3-5	0.099	312 lbs	(-327 lbs)	3-4	0.097	306 lbs	(-318 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 6, 5, 4 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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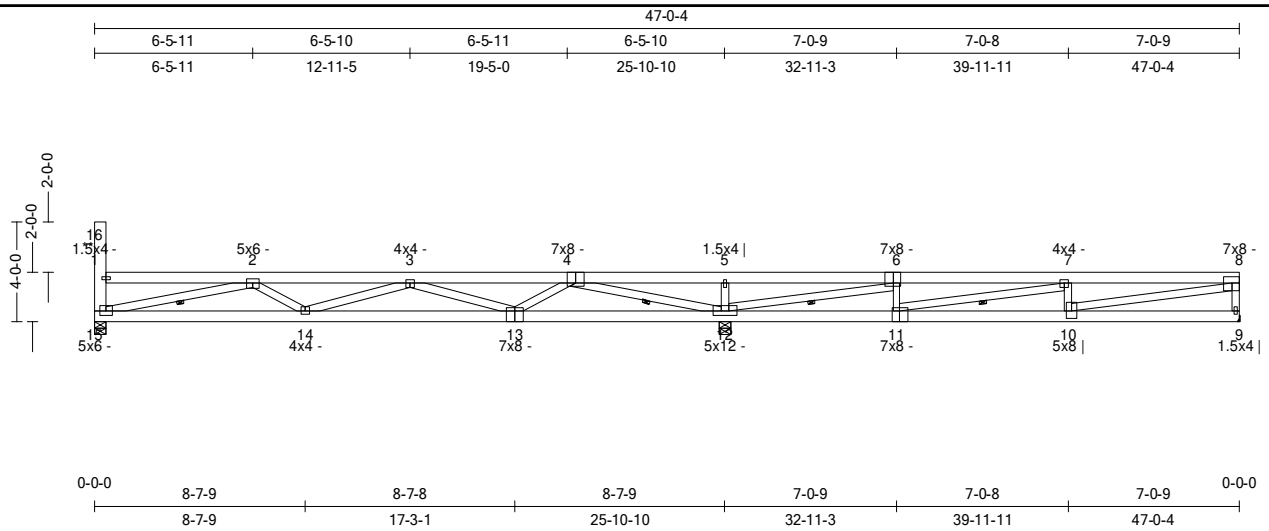
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: C1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:45
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
47-0-4	0/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	266 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.58 (4-5)	Vert TL:	0.55 in	L / 554	(13-14)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.36 (13-14)	Vert LL:	0.24 in	L / 999	(13-14)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.95 (8-10)	Horz TL:	0.07 in		9	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
240360B1
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	938 lbs	·	·	·	·	96 lbs
12	1	5.5 in	2.55 in	2,391 lbs	·	·	·	·	·
9	1	1.5 in	---	750 lbs	·	·	·	·	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 16-15

Bracing

TC: Sheathed or Purlins at 4-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-15, 4-12, 6-12, 7-11

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 16-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.165		(-3,049 lbs)	4-5	0.576	2,494 lbs		6-7	0.162	552 lbs	(-1,404 lbs)
	3-4	0.173		(-2,180 lbs)	5-6	0.576	2,494 lbs		7-8	0.192		(-2,096 lbs)
BC	10-11	0.263	2,096 lbs		12-13	0.321	1,357 lbs		14-15	0.301	2,690 lbs	
	11-12	0.300	1,359 lbs	(-606 lbs)	13-14	0.361	3,265 lbs					
Web	2-15	0.597		(-2,771 lbs)	4-12	0.746		(-3,470 lbs)	7-11	0.288		(-1,139 lbs)
	2-14	0.235	532 lbs		5-12	0.074		(-599 lbs)	7-10	0.041		(-333 lbs)
	3-13	0.517		(-1,297 lbs)	6-12	0.788		(-3,119 lbs)	8-10	0.948	2,147 lbs	
	4-13	0.458	1,037 lbs		6-11	0.171	388 lbs		8-9	0.083		(-672 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: C1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:45
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
47-0-4	0/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	266 lbs

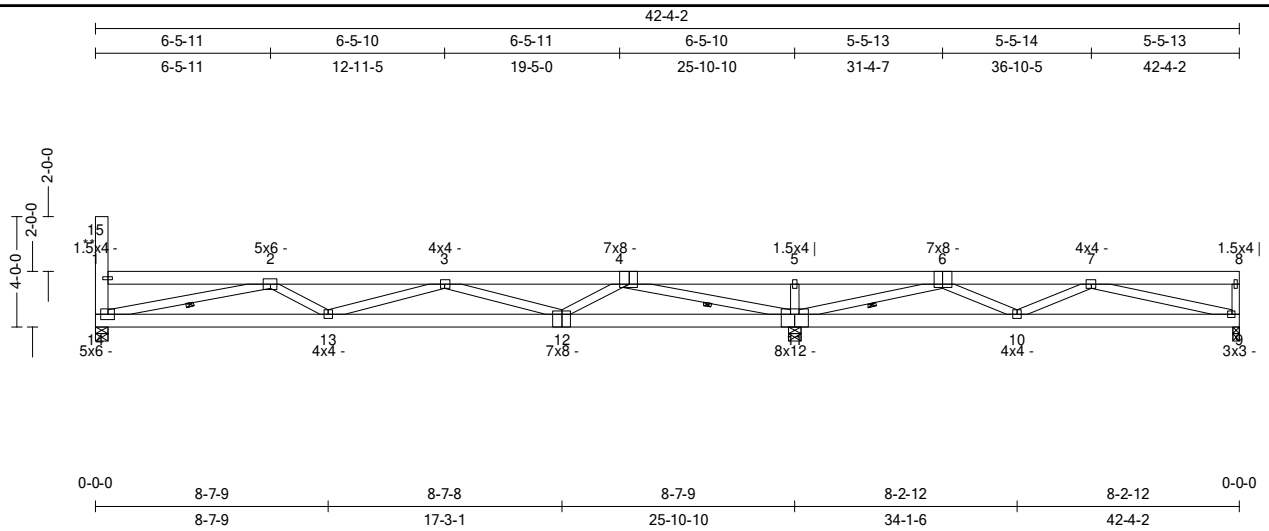
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
8) A creep factor of 2.00 has been applied for this truss analysis.
9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.09 in, 2L/626 (16-1), Allowable 2L/120.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: C2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:47
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-4-2	0/12	7	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	238 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.51 (4-5)	Vert TL:	0.55 in	L / 554	(12-13)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.36 (12-13)	Vert LL:	0.23 in	L / 999	(12-13)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.75 (4-11)	Horz TL:	0.06 in		11	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.50 in	933 lbs	·	·	-57 lbs	-57 lbs	96 lbs
11	1	5.5 in	2.37 in	2,226 lbs	·	·	-207 lbs	-207 lbs	·
9	1	2.75 in	1.50 in	551 lbs	·	·	-34 lbs	-34 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 15-14

Bracing

TC: Sheathed or Purlins at 4-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-1-0, Purlin design by Others.
Web: One Midpoint Row: 2-14, 4-11, 6-11

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 15-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.163		(-3,019 lbs)	4-5	0.514	2,371 lbs		6-7	0.092	520 lbs	(-961 lbs)
	3-4	0.179		(-2,125 lbs)	5-6	0.514	2,371 lbs					
BC	9-10	0.174	1,090 lbs		11-12	0.284	1,286 lbs		13-14	0.299	2,668 lbs	(-374 lbs)
	10-11	0.192	585 lbs	(-1,095 lbs)	12-13	0.355	3,223 lbs	(-371 lbs)				
Web	2-14	0.592		(-2,749 lbs)	4-11	0.754	416 lbs	(-3,508 lbs)	7-10	0.091		(-444 lbs)
	2-13	0.241	547 lbs		5-11	0.069		(-559 lbs)	7-9	0.626		(-1,135 lbs)
	3-12	0.495		(-1,241 lbs)	6-11	0.393		(-2,111 lbs)				
	4-12	0.454	1,028 lbs		6-10	0.311	705 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: C2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:47
Page: 2 of 2

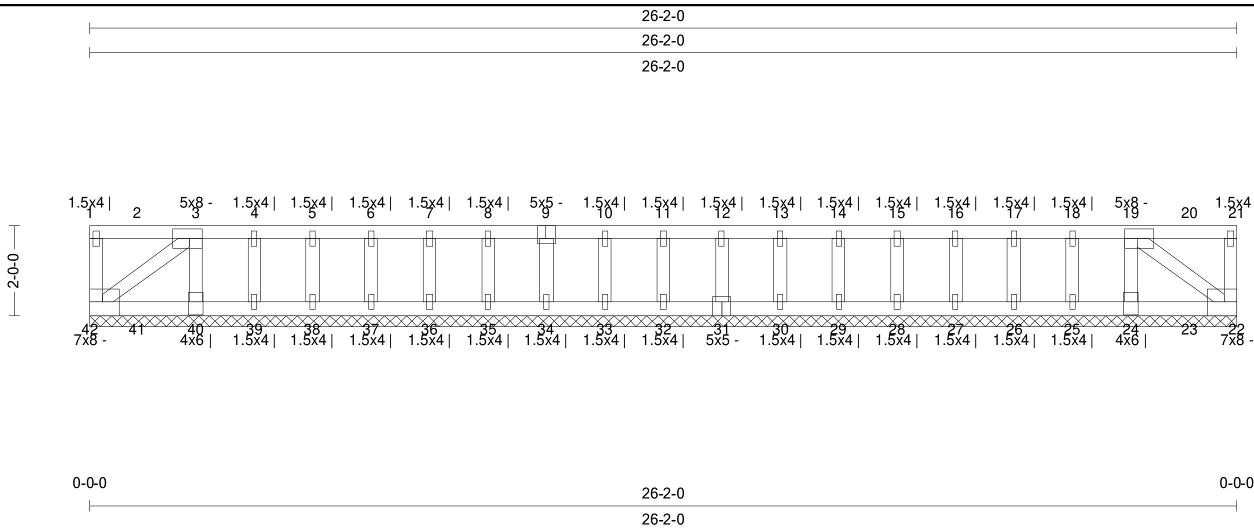
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-4-2	0/12	7	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	238 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.09 in, 2L/624 (15-1), Allowable 2L/120.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: C3D Job: 2501272 Designer:Anthony Date: 10/28/25 13:51:51 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	116 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	**General**		**CSI**		**Deflection**		**L/**	**(loc)**	**Allowed**
TCLL : 25	Bldg Code :	IRC 2021/	TC :	0.48 (3-4)	Vert TL:	0 in UP	L / 999	22	L / 180
TCDL : 7		TPI 1-2014	BC :	0.04 (22-24)	Vert LL:	0 in	L / 999	22	L / 240
BCLL : 0	Rep Mbr :	No	Web :	0.91 (3-40)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. :	115 %							

10/28/2025

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	314 in	N/A	2,980 lbs	-2,900 lbs	-0 lbs	-12 lbs	-2,900 lbs	-3,928 lbs
1	314 in	N/A	2,967 lbs	-2,823 lbs	·	-31 lbs	-2,823 lbs	·
1	314 in	N/A	109 lbs	-35 lbs	·	-14 lbs	-35 lbs	·
1	314 in	N/A	117 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	115 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	117 lbs	·	·	-14 lbs	-14 lbs	·
1	314 in	N/A	109 lbs	-35 lbs	·	-14 lbs	-35 lbs	·
1	314 in	N/A	2,967 lbs	-2,823 lbs	·	-31 lbs	-2,823 lbs	·
1	314 in	N/A	2,980 lbs	-2,900 lbs	-0 lbs	-12 lbs	-2,900 lbs	3,928 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 3-42, 19-22

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to a 7,850 lbs (300 plf) drag load distributed along the TC rake from each direction.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: C3D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:52
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	116 lbs

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-3	0.157	697 lbs	(-701 lbs)	7-8	0.224	1,597 lbs	(-1,603 lbs)	12-13	0.114	797 lbs	(-803 lbs)	17-18	0.447	2,797 lbs	(-2,803 lbs)
	3-4	0.483	3,197 lbs	(-3,203 lbs)	8-9	0.169	1,197 lbs	(-1,203 lbs)	13-14	0.169	1,197 lbs	(-1,203 lbs)	18-19	0.483	3,197 lbs	(-3,203 lbs)
	4-5	0.447	2,797 lbs	(-2,803 lbs)	9-10	0.114	797 lbs	(-803 lbs)	14-15	0.224	1,597 lbs	(-1,603 lbs)	19-21	0.157	697 lbs	(-701 lbs)
	5-6	0.333	2,397 lbs	(-2,403 lbs)	10-11	0.058	397 lbs	(-403 lbs)	15-16	0.280	1,997 lbs	(-2,003 lbs)				
	6-7	0.280	1,997 lbs	(-2,003 lbs)	11-12	0.058	397 lbs	(-403 lbs)	16-17	0.333	2,397 lbs	(-2,403 lbs)				
BC																
Web	3-42	0.673	4,878 lbs	(-4,881 lbs)												
	3-40	0.910	2,868 lbs	(-2,922 lbs)												
	19-24	0.910	2,868 lbs	(-2,922 lbs)												
	19-22	0.673	4,878 lbs	(-4,881 lbs)												

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) Provide adequate drainage to prevent ponding.
8) A creep factor of 2.00 has been applied for this truss analysis.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 22, 24, 25, 39, 40, 42 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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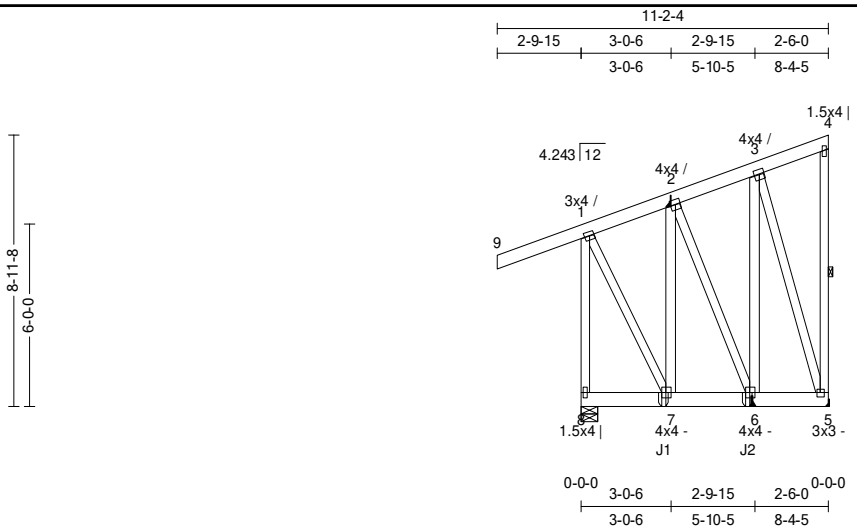
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: CJ1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:25
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
8-4-5	4.243 / 12	2	2-9-15	0-0-0	0-0-0	0-0-0	1	24.28 in	100 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)		Bldg Code :	IRC 2021/ TPI 1-2014	TC :	0.19 (9-1)	Vert TL:	0 in UP	L / 999	(6-7)	L / 180
TCLL :	25			BC :	0.01 (6-7)	Vert LL:	0 in UP	L / 999	(6-7)	L / 240
TCDL :	7	Rep Mbr :	No	Web :	0.53 (1-8)	Horz TL:	0 in		2	
BCLL :	0	Lumber D.O.L. :	115 %							
BCDL :	10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	7.028 in	1.50 in	318 lbs	·	-38 lbs	-225 lbs	-225 lbs	135 lbs
2	1	2.184 in	---	94 lbs	·	-68 lbs	·	-68 lbs	·
6	1	2.184 in	---	258 lbs	·	-228 lbs	·	-228 lbs	·
5	1	1.5 in	---	55 lbs	·	-47 lbs	·	-47 lbs	·

10/28/2025
ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 4-5

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-5

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-9-15	-1-5-1	Down	Proj	0 plf	35.91 plf	
Top	-1-5-1	2-10-4	Down	Proj	35.91 plf	0 plf	
Top	-2-9-15	-1-5-1	Down	Proj	0 plf	35.91 plf	
Top	-1-5-1	2-10-4	Down	Proj	35.91 plf	0 plf	
Top	6-7-7	8-4-5	Down	Proj	25.85 plf	4.1 plf	
Top	6-7-7	8-4-5	Down	Proj	24.74 plf	2.99 plf	

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: CJ1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:26
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
8-4-5	4.243 /12	2	2-9-15	0-0-0	0-0-0	0-0-0	1	24.28 in	100 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-9-15	-1-5-1	Down	Proj	0 plf	10.05 plf	
Top	-1-5-1	2-10-4	Down	Proj	10.05 plf	0 plf	
Top	-2-9-15	-1-5-1	Down	Proj	0 plf	10.05 plf	
Top	-1-5-1	2-10-4	Down	Proj	10.05 plf	0 plf	
Top	6-7-7	8-4-5	Down	Proj	7.24 plf	1.15 plf	
Top	6-7-7	8-4-5	Down	Proj	6.93 plf	0.84 plf	
Bot	6-7-7	8-4-5	Down	Proj	10.34 plf	1.64 plf	
Bot	6-7-7	8-4-5	Down	Proj	9.9 plf	1.2 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web	1-8	0.530	(-315 lbs)

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
J1	BC	2-9-8
J1	BC	2-9-8
J2	BC	5-7-7
J2	BC	5-7-7

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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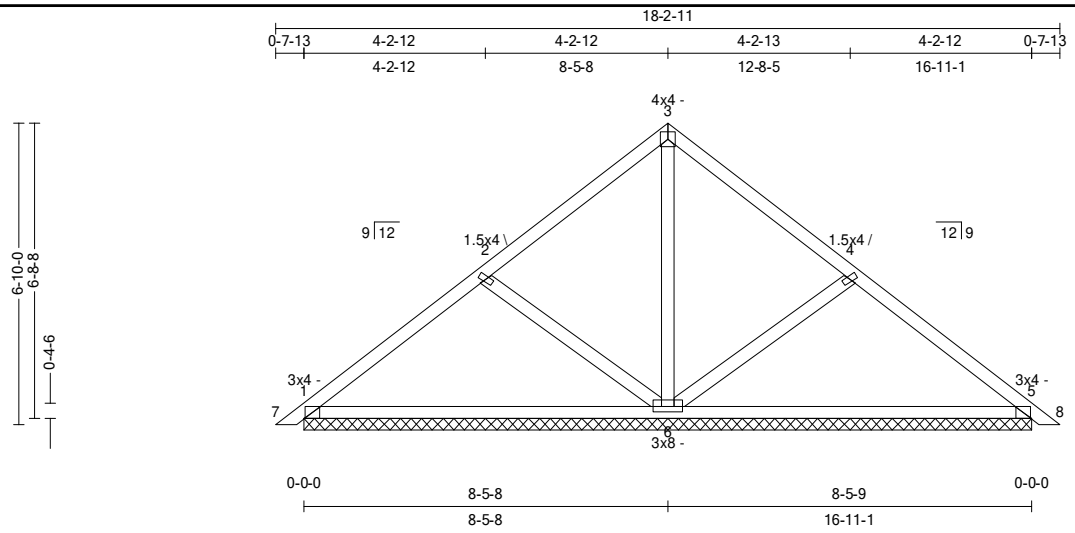
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: CP1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:53
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
16-11-1	9/12	18	0-7-13	0-7-13	0-0-0	0-0-0	1	24 in	71 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.23 (3-4)	Vert TL:	0.1 in	L / 999	(5-6)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.49 (5-6)	Vert LL:	0.05 in	L / 999	(5-6)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.18 (3-6)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	203.042 in	N/A	676 lbs	·	·	-98 lbs	-98 lbs	163 lbs
1	203.042 in	N/A	0 lbs	-685 lbs	-202 lbs	-140 lbs	-685 lbs	619 lbs
1	203.042 in	N/A	0 lbs	-685 lbs	-202 lbs	-140 lbs	-685 lbs	-619 lbs
1	203.042 in	N/A	941 lbs	·	·	·	·	681 lbs
1	203.042 in	N/A	941 lbs	·	·	·	·	-681 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.209	863 lbs	(-357 lbs)
	4-5	0.209	863 lbs	(-357 lbs)
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 5, 1 may need to be considered.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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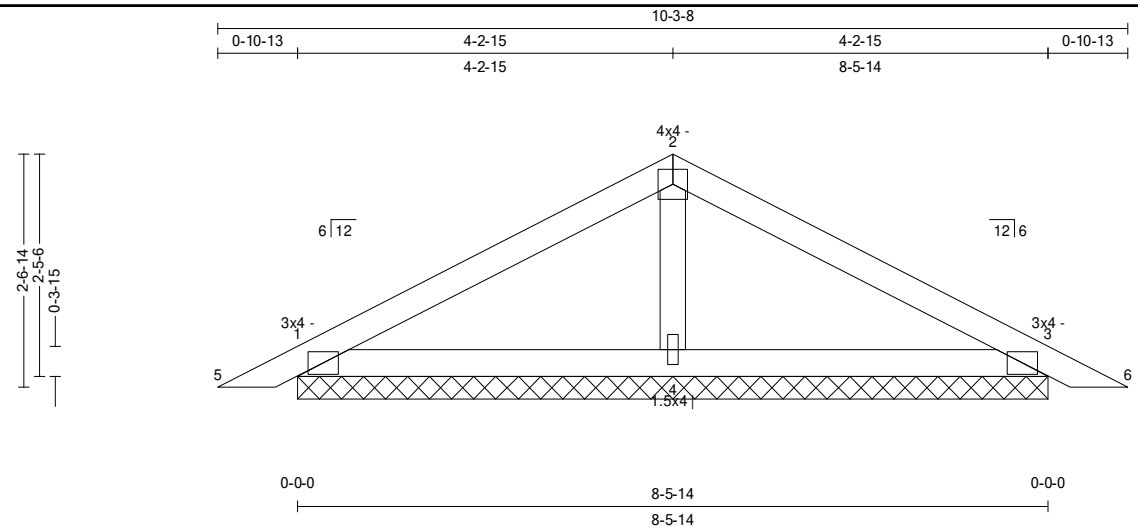
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: CP2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:54
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
8-5-14	6/12	32	0-10-13	0-10-13	0-0-0	0-0-0	1	24 in	28 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.22 (2-3)	Vert TL:	0.01 in	L / 999	(3-4)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.10 (4-1)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.03 (2-4)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	465 lbs	162 plf	-105 lbs	-39 lbs	-104 lbs	-105 lbs	-391 lbs

10/28/2025

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.225	378 lbs	2-3	0.225	378 lbs
BC						
Web						

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 1 may need to be considered.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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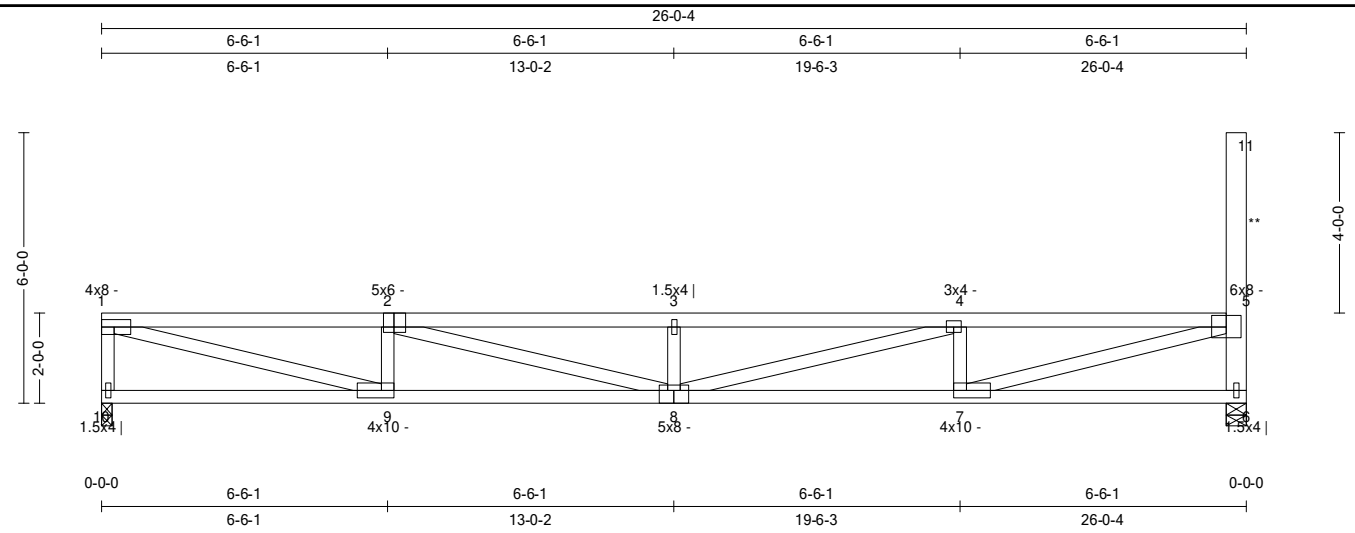
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:55
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	13	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	120 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.77 (1-2)	Vert TL:	0.95 in	L / 321	(8-9)	L / 180
TCDL:	7	Rep Mbr:	Yes	BC:	0.73 (8-9)	Vert LL:	0.4 in	L / 760	8	L / 240
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.63 (1-9)	Horz TL:	0.08 in		6	
BCDL:	10									

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	2.75 in	1.50 in	1,097 lbs	·	·	-176 lbs	-176 lbs	-197 lbs
6	1	5.5 in	1.50 in	1,089 lbs	·	·	-105 lbs	-105 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 1-9, 5-7
DFL SS 2 x 6: 11-6

Bracing

TC: Sheathed or Purlins at 2-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 11-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.775	531 lbs	(-3,182 lbs)	2-3	0.572	764 lbs	(-4,165 lbs)	3-4	0.610	764 lbs	(-4,165 lbs)	4-5	0.652	758 lbs	(-3,174 lbs)
BC	7-8	0.720	3,174 lbs	(-522 lbs)	8-9	0.733	3,243 lbs	(-314 lbs)								
Web	1-10	0.133		(-1,037 lbs)	2-8	0.473	1,072 lbs		4-7	0.093		(-725 lbs)				
	1-9	0.633	3,295 lbs	(-530 lbs)	3-8	0.049		(-387 lbs)	5-7	0.632	3,289 lbs	(-304 lbs)				
	2-9	0.092		(-718 lbs)	4-8	0.472	1,068 lbs		5-6	0.223		(-1,029 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) A creep factor of 2.00 has been applied for this truss analysis.

TrueBuild®Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:56
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	13	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	120 lbs

7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.33 in, 2L/301 (11-5), Allowable 2L/120.

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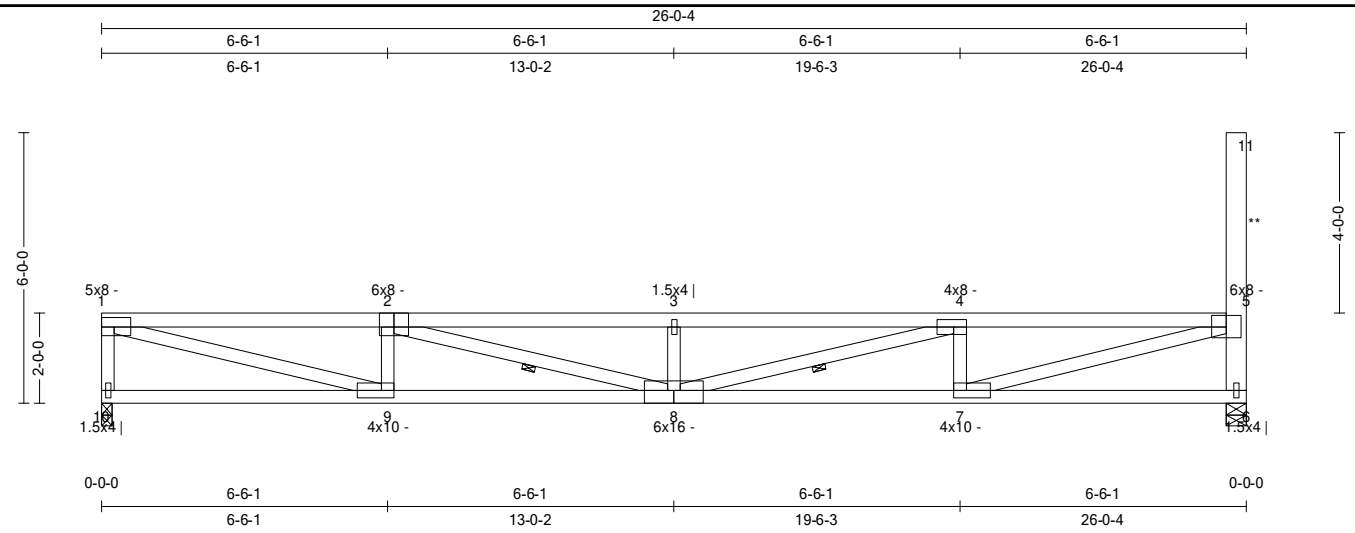
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D1D
Job: 2501272
Designer:Anthony
Date: 10/28/25 13:51:57
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	122 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 25	Bldg Code :	IRC 2021/	TC :	0.91 (1-2)	Vert TL:	0.93 in	L / 328	(8-9)	L / 180
TCDL : 7		TPI 1-2014	BC :	0.89 (9-10)	Vert LL:	0.39 in	L / 775	8	L / 240
BCLL : 0	Rep Mbr :	No	Web :	0.79 (4-8)	Horz TL:	0.15 in		6	
BCDL : 10	Lumber D.O.L. :	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	2.75 in	1.50 in	1,097 lbs	-77 lbs	·	-176 lbs	-176 lbs	7,806 lbs
6	1	5.5 in	1.50 in	1,089 lbs	-78 lbs	·	-105 lbs	-105 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 1-9, 5-7
DFL SS 2 x 6: 11-6

Bracing

TC: Sheathed or Purlins at 2-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 2-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-8, 4-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 7,806 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
7) ** - Indicates parapet wind loading has been applied to member 11-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.908	640 lbs	(-3,215 lbs)	2-3	0.672	763 lbs	(-4,161 lbs)	3-4	0.729	763 lbs	(-4,161 lbs)	4-5	0.764	758 lbs	(-3,174 lbs)
BC	7-8	0.590	3,174 lbs	(-611 lbs)	8-9	0.789	7,095 lbs	(-4,486 lbs)	9-10	0.888	7,806 lbs	(-7,806 lbs)				
Web	1-10	0.133		(-1,037 lbs)	2-8	0.744	2,345 lbs	(-1,568 lbs)	4-7	0.100	316 lbs	(-727 lbs)				
	1-9	0.633	3,375 lbs	(-708 lbs)	3-8	0.049		(-385 lbs)	5-7	0.632	3,289 lbs	(-633 lbs)				
	2-9	0.101	317 lbs	(-723 lbs)	4-8	0.791	2,491 lbs	(-1,665 lbs)	5-6	0.223		(-1,028 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D1D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:57
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	122 lbs

8) ✉ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 10, 6 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.32 in, 2L/311 (11-5), Allowable 2L/120.

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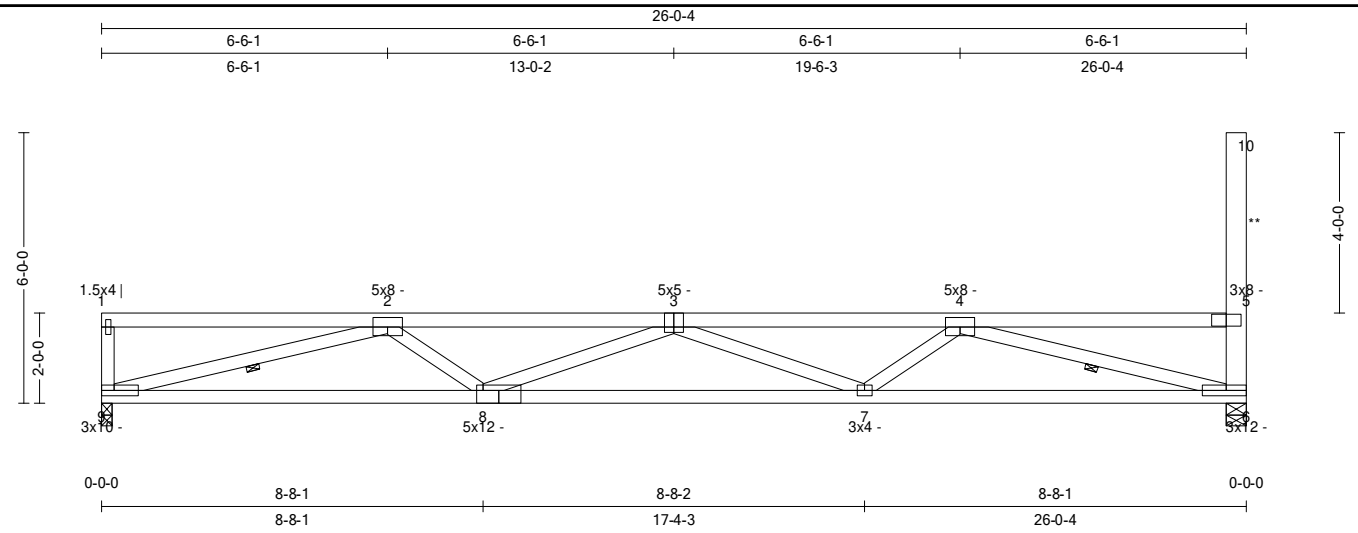
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:59
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	21	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	117 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.56 (1-2)	Vert TL:	0.88 in	L / 346	(7-8)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.77 (7-8)	Vert LL:	0.37 in	L / 817	(7-8)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.74 (2-9)	Horz TL:	0.16 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	2.75 in	1.50 in	1,097 lbs	·	·	-176 lbs	-176 lbs	-197 lbs
6	1	5.5 in	1.50 in	1,089 lbs	·	·	-105 lbs	-105 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-11-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.512	564 lbs	(-3,676 lbs)	3-4	0.507	707 lbs	(-3,591 lbs)	4-5	0.550	465 lbs	
BC	6-7	0.639	3,100 lbs	(-564 lbs)	7-8	0.769	4,128 lbs	(-564 lbs)	8-9	0.683	3,118 lbs	(-349 lbs)
Web	2-9	0.739	586 lbs	(-3,228 lbs)	3-7	0.251		(-602 lbs)				
	2-8	0.323	732 lbs		4-7	0.299	678 lbs					
	3-8	0.209		(-550 lbs)	4-6	0.709	348 lbs	(-3,212 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:51:59
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	21	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	117 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.31 in, 2L/325 (10-5), Allowable 2L/120.

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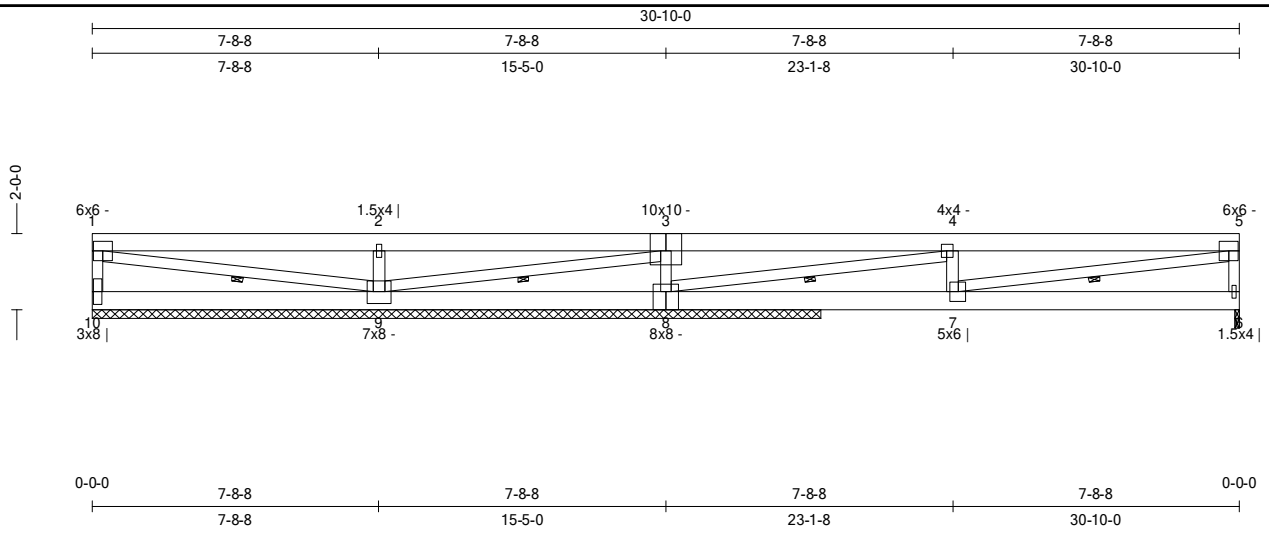
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D2D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:01
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
30-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	173 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.32 (3-4)	Vert TL:	0.2 in	L / 664	(6-7)	L / 180
TCDL:	7			BC:	0.27 (7-8)	Vert LL:	0.08 in	L / 999	(6-7)	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.91 (3-9)	Horz TL:	0.01 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
240360081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	1.50 in	629 lbs	-161 lbs	·	-67 lbs	-161 lbs	·
8	1	234.999 in	N/A	1,106 lbs	·	·	-122 lbs	-122 lbs	4,088 lbs
9	1	234.999 in	N/A	726 lbs	·	·	-77 lbs	-77 lbs	-5,437 lbs
10	1	234.999 in	N/A	621 lbs	-423 lbs	·	-29 lbs	-423 lbs	18 lbs

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 5-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 7-3-0, Purlin design by Others.
Web: One Midpoint Row: 1-9, 3-9, 4-8, 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 9,250 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.291	2,576 lbs	(-2,475 lbs)	2-3	0.300	2,106 lbs	(-2,005 lbs)	3-4	0.318	1,775 lbs	(-1,500 lbs)	4-5	0.260	1,310 lbs	(-2,369 lbs)
BC	7-8	0.266	2,413 lbs	(-1,354 lbs)												
Web	1-10	0.155	488 lbs	(-558 lbs)	3-9	0.912	2,874 lbs	(-2,696 lbs)	4-7	0.139	437 lbs	(-360 lbs)				
	1-9	0.816	2,570 lbs	(-2,673 lbs)	3-8	0.132	416 lbs	(-704 lbs)	5-7	0.782	2,462 lbs	(-1,381 lbs)				
	2-9	0.070		(-566 lbs)	4-8	0.783	1,428 lbs	(-2,788 lbs)	5-6	0.072		(-551 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D2D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:01

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
30-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	173 lbs

8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 6, 10 may need to be considered.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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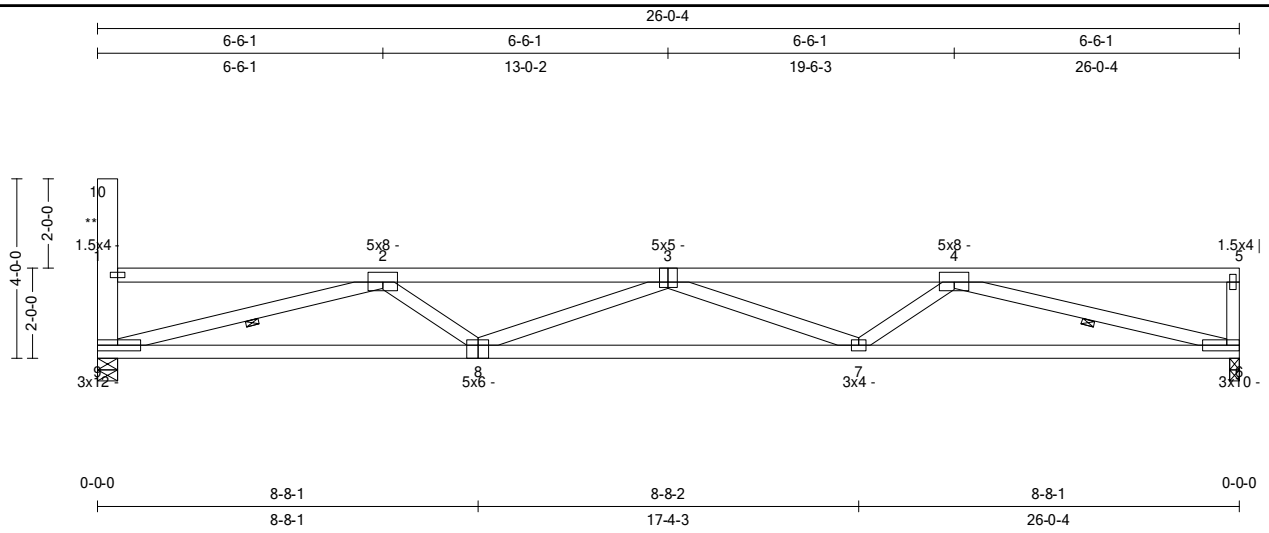
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:02
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	14	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	112 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.56 (4-5)	Vert TL:	0.92 in	L / 332	(7-8)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.76 (7-8)	Vert LL:	0.37 in	L / 815	(7-8)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.74 (4-6)	Horz TL:	0.16 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,089 lbs	·	·	-124 lbs	-124 lbs	96 lbs
6	1	2.75 in	1.50 in	1,097 lbs	·	·	-157 lbs	-157 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-9

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-8-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 10-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.504	513 lbs	(-3,596 lbs)	3-4	0.509	461 lbs	(-3,616 lbs)				
BC	6-7	0.675	3,131 lbs	(-467 lbs)	7-8	0.755	4,120 lbs	(-623 lbs)	8-9	0.665	3,102 lbs	(-545 lbs)
Web	2-9	0.709	419 lbs	(-3,215 lbs)	3-7	0.252		(-605 lbs)				
	2-8	0.301	681 lbs		4-7	0.299	677 lbs					
	3-8	0.245		(-588 lbs)	4-6	0.742	504 lbs	(-3,242 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: D3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:03
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	14	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	112 lbs

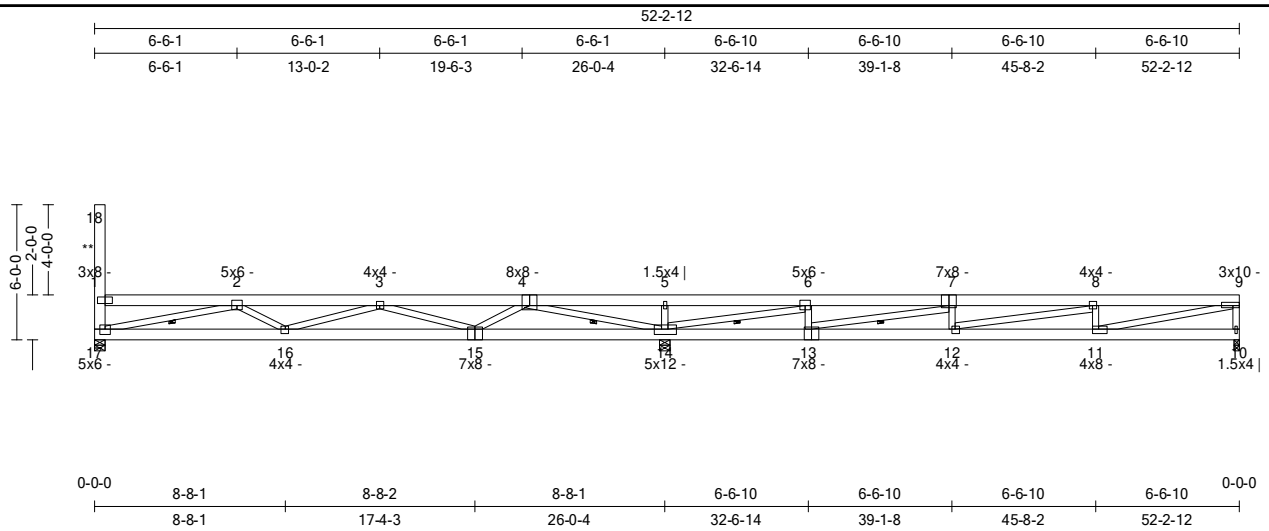
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.16 in, 2L/324 (10-1), Allowable 2L/120.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:05
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
52-2-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	299 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.75 (5-6)	Vert TL:	0.58 in	L / 532	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.40 (15-16)	Vert LL:	0.28 in	L / 999	12	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.86 (6-14)	Horz TL:	0.09 in		10	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.50 in	929 lbs	·	·	·	·	197 lbs
14	1	5.5 in	2.83 in	2,655 lbs	·	·	·	·	·
10	1	2.75 in	1.50 in	943 lbs	·	·	·	·	·

10/28/2025

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 9-11
DFL SS 2 x 6: 18-17

Bracing

TC: Sheathed or Purlins at 4-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-3-0, Purlin design by Others.
Web: One Midpoint Row: 2-17, 4-14, 6-14, 7-13

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 18-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.194	500 lbs		3-4	0.196		(-2,041 lbs)	5-6	0.745	3,202 lbs		7-8	0.205	(-3,253 lbs)
	2-3	0.179		(-3,004 lbs)	4-5	0.745	3,202 lbs		6-7	0.171	645 lbs	(-1,441 lbs)	8-9	0.211	(-2,807 lbs)
BC	11-12	0.343	2,807 lbs		13-14	0.379	1,394 lbs	(-679 lbs)	15-16	0.396	3,181 lbs				
	12-13	0.389	3,250 lbs		14-15	0.357	1,187 lbs	(-820 lbs)	16-17	0.334	2,662 lbs				
Web	2-17	0.595		(-2,741 lbs)	4-14	0.778		(-3,593 lbs)	8-12	0.202	458 lbs				
	2-16	0.216	490 lbs		5-14	0.076		(-614 lbs)	8-11	0.068		(-554 lbs)			
	3-16	0.150	340 lbs		6-14	0.863		(-3,823 lbs)	9-11	0.554	2,887 lbs				
	3-15	0.581		(-1,446 lbs)	6-13	0.250	567 lbs		9-10	0.107		(-868 lbs)			
	4-15	0.496	1,123 lbs		7-13	0.493		(-2,186 lbs)							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:05
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
52-2-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	299 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.16 in, 2L/642 (18-1), Allowable 2L/120.

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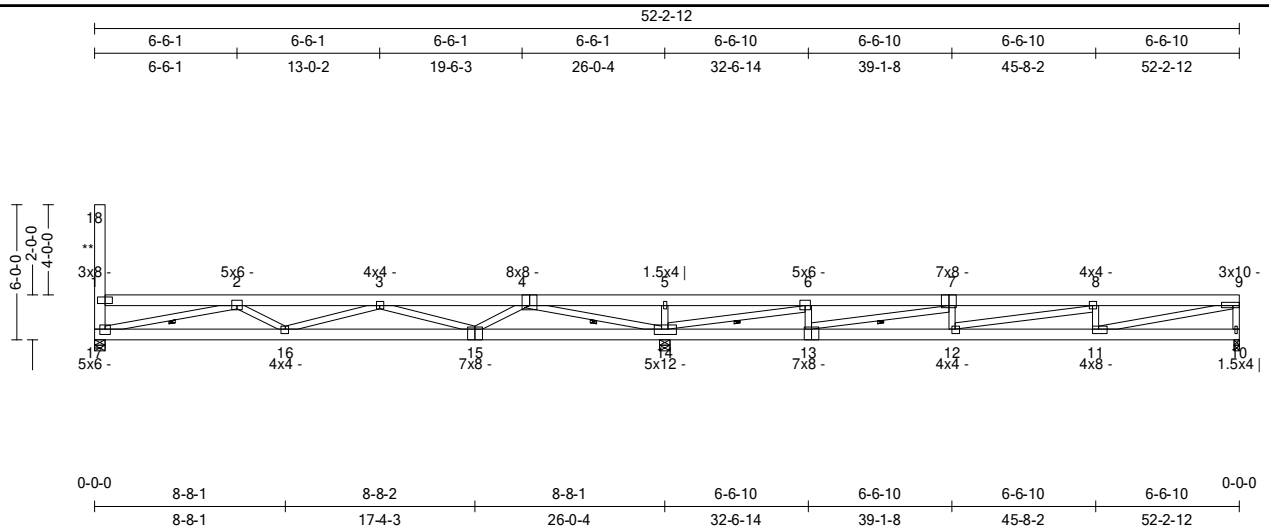
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:07
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
52-2-12	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	299 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.66 (5-6)	Vert TL:	0.58 in	L / 532	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.35 (15-16)	Vert LL:	0.28 in	L / 999	12	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.86 (6-14)	Horz TL:	0.09 in		10	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.50 in	929 lbs	·	·	·	·	197 lbs
14	1	5.5 in	2.83 in	2,655 lbs	·	·	·	·	·
10	1	2.75 in	1.50 in	943 lbs	·	·	·	·	·

10/28/2025
ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 9-11
DFL SS 2 x 6: 18-17

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-10-0, Purlin design by Others.
Web: One Midpoint Row: 2-17, 4-14, 6-14, 7-13

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 18-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.169	500 lbs		3-4	0.169		(-2,041 lbs)	5-6	0.658	3,202 lbs		7-8	0.177	(-3,253 lbs)
	2-3	0.162		(-3,004 lbs)	4-5	0.658	3,202 lbs		6-7	0.148	645 lbs	(-1,441 lbs)	8-9	0.181	(-2,807 lbs)
BC	11-12	0.307	2,807 lbs		13-14	0.334	1,394 lbs	(-679 lbs)	15-16	0.355	3,181 lbs				
	12-13	0.349	3,250 lbs		14-15	0.312	1,187 lbs	(-820 lbs)	16-17	0.299	2,662 lbs				
Web	2-17	0.595		(-2,741 lbs)	4-14	0.778		(-3,593 lbs)	8-12	0.202	458 lbs				
	2-16	0.216	490 lbs		5-14	0.076		(-614 lbs)	8-11	0.068		(-554 lbs)			
	3-16	0.150	340 lbs		6-14	0.863		(-3,823 lbs)	9-11	0.554	2,887 lbs				
	3-15	0.581		(-1,446 lbs)	6-13	0.250	567 lbs		9-10	0.107		(-868 lbs)			
	4-15	0.496	1,123 lbs		7-13	0.493		(-2,186 lbs)							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild®Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:08
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
52-2-12	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	299 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.16 in, 2L/642 (18-1), Allowable 2L/120.

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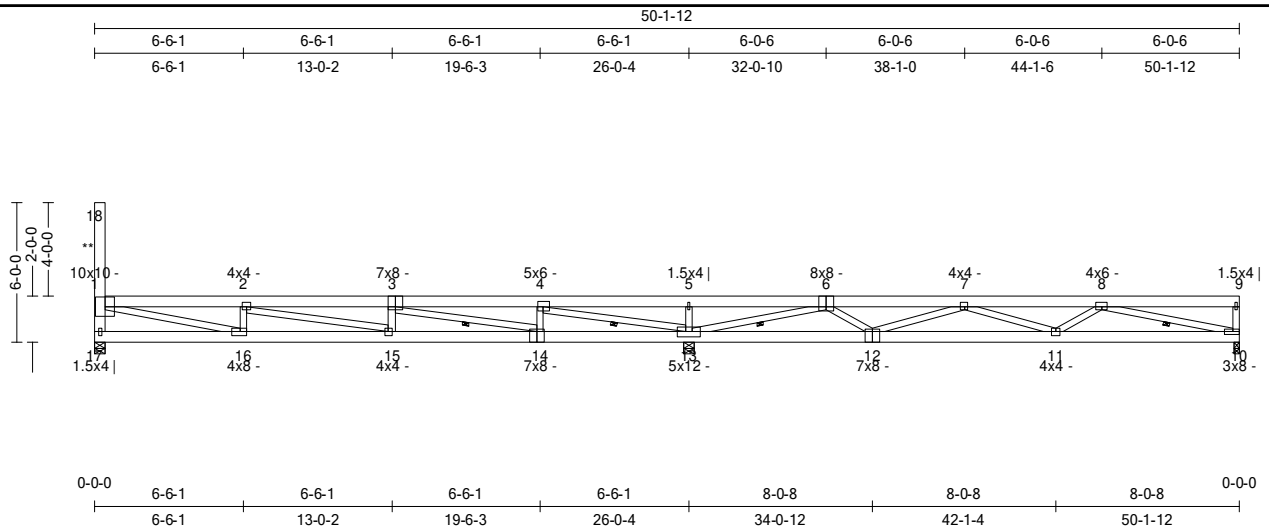
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:10
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
50-1-12	0/12	21	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	289 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.62 (5-6)	Vert TL:	0.57 in	L / 537	(15-16)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.35 (14-15)	Vert LL:	0.27 in	L / 999	15	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.83 (4-13)	Horz TL:	0.08 in		10	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036031
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.50 in	936 lbs	·	·	·	·	197 lbs
13	1	5.5 in	2.72 in	2,546 lbs	·	·	·	·	·
10	1	2.75 in	1.50 in	865 lbs	·	·	·	·	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 1-16
DFL SS 2 x 6: 18-17

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-9-0, Purlin design by Others.
Web: One Midpoint Row: 3-14, 4-13, 6-13, 8-10

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 18-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.173		(-2,769 lbs)	3-4	0.147	409 lbs	(-1,490 lbs)	5-6	0.617	2,894 lbs		7-8	0.138	(-2,578 lbs)
	2-3	0.176		(-3,247 lbs)	4-5	0.617	2,894 lbs		6-7	0.132	375 lbs	(-1,728 lbs)			
BC	10-11	0.258	2,294 lbs		12-13	0.293	1,101 lbs	(-835 lbs)	14-15	0.348	3,245 lbs		16-17	0.097	(-466 lbs)
	11-12	0.306	2,702 lbs		13-14	0.330	1,444 lbs	(-441 lbs)	15-16	0.303	2,769 lbs				
Web	1-17	0.230		(-860 lbs)	4-14	0.241	545 lbs		7-12	0.457		(-1,290 lbs)			
	1-16	0.548	2,852 lbs		4-13	0.827		(-3,706 lbs)	7-11	0.154	349 lbs				
	2-16	0.069		(-560 lbs)	5-13	0.075		(-607 lbs)	8-11	0.185	419 lbs				
	2-15	0.217	491 lbs		6-13	0.673		(-3,212 lbs)	8-10	0.481		(-2,371 lbs)			
	3-14	0.462		(-2,071 lbs)	6-12	0.399	904 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:10
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
50-1-12	0/12	21	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	289 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.18 in, 2L/569 (18-1), Allowable 2L/120.

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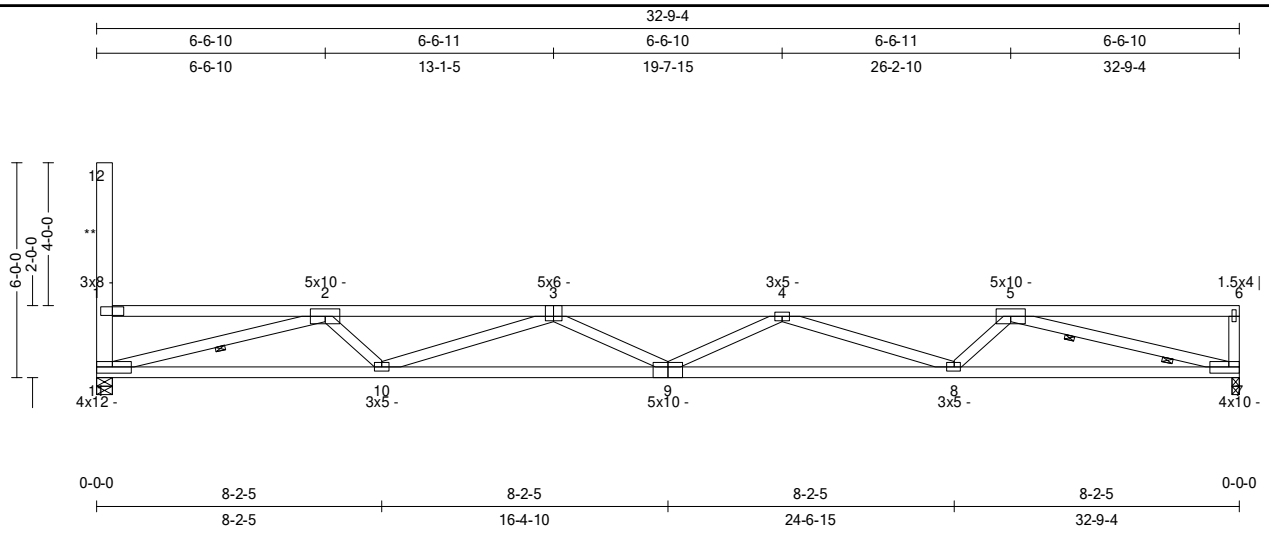
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:12
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	8	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	145 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.81 (3-4)	Vert TL:	1.86 in	L / 206	(8-9)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.97 (8-9)	Vert LL:	0.79 in	L / 488	9	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.96 (2-11)	Horz TL:	0.3 in		7	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,373 lbs	·	·	-111 lbs	-111 lbs	197 lbs
7	1	2.75 in	1.50 in	1,380 lbs	·	·	-169 lbs	-169 lbs	·

Material

TC: DFL #1B 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed
BC: Sheathed or Purlins at 7-1-0, Purlin design by Others.
Web: One Midpoint Row: 2-11
Two Third Point Rows: 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.373	463 lbs		3-4	0.814	842 lbs	(-6,405 lbs)							
	2-3	0.498	792 lbs	(-4,816 lbs)	4-5	0.497	572 lbs	(-4,852 lbs)							
BC	7-8	0.737	4,213 lbs	(-538 lbs)	8-9	0.967	6,262 lbs	(-850 lbs)	9-10	0.966	6,249 lbs	(-938 lbs)	10-11	0.727	4,171 lbs	(-802 lbs)
Web	2-11	0.964	388 lbs	(-4,321 lbs)	3-9	0.247	560 lbs		5-8	0.407	922 lbs					
	2-10	0.412	932 lbs		4-9	0.242	549 lbs		5-7	0.821	576 lbs	(-4,360 lbs)				
	3-10	0.781		(-1,517 lbs)	4-8	0.769	333 lbs	(-1,493 lbs)							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:12
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	8	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	145 lbs

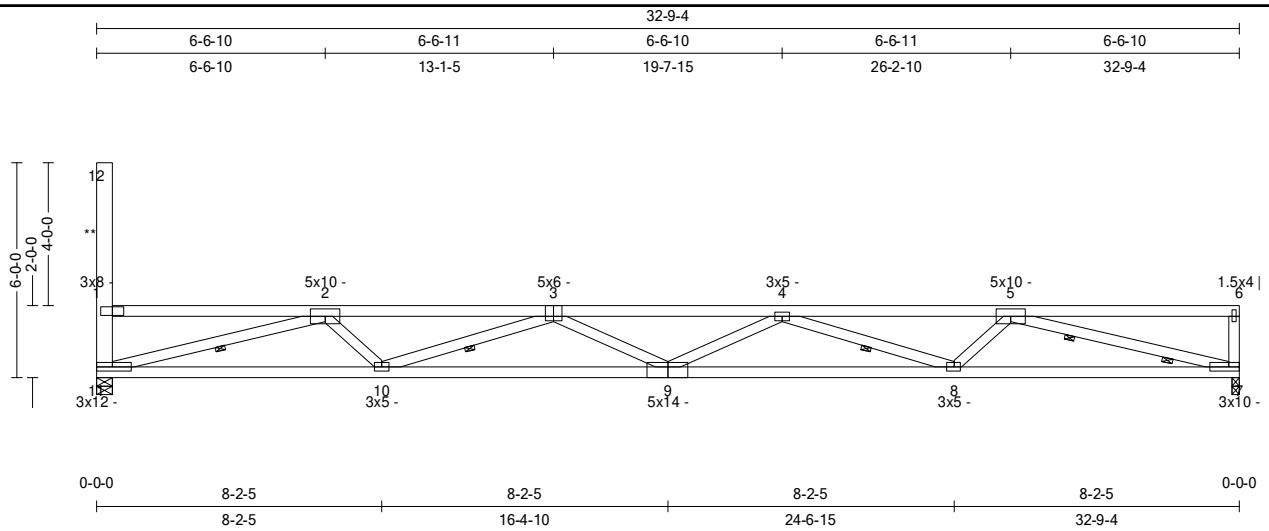
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.6 in, 2L/167 (12-1), Allowable 2L/120.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:13
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	145 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.97 (3-4)	Vert TL:	1.77 in	L / 217	(8-9)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.72 (10-11)	Vert LL:	0.75 in	L / 514	9	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.96 (2-11)	Horz TL:	0.27 in		7	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,373 lbs	·	·	-111 lbs	-111 lbs	-9,832 lbs
7	1	2.75 in	1.50 in	1,380 lbs	·	·	-169 lbs	-169 lbs	·

Material

TC: DFL #1B 2 x 4
BC: DFL 2400/2.0 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed
BC: Sheathed or Purlins at 3-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 3-10, 4-8
Two Third Point Rows: 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 9,832 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
7) ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.427	1,911 lbs	(-1,911 lbs)	3-4	0.970	842 lbs	(-6,406 lbs)	5-6	0.440	1,980 lbs	(-1,980 lbs)						
	2-3	0.585	792 lbs	(-4,815 lbs)	4-5	0.585	572 lbs	(-4,851 lbs)										
BC	7-8	0.559	4,213 lbs	(-538 lbs)	8-9	0.700	6,400 lbs	(-1,422 lbs)	9-10	0.699	8,374 lbs	(-3,408 lbs)	10-11	0.719	9,522 lbs	(-6,211 lbs)		
Web	2-11	0.963	388 lbs	(-4,320 lbs)	3-9	0.408	1,285 lbs	(-943 lbs)	5-8	0.407	1,197 lbs							
	2-10	0.411	1,208 lbs		4-9	0.408	1,284 lbs	(-954 lbs)	5-7	0.821	576 lbs	(-4,360 lbs)						
	3-10	0.331	1,042 lbs	(-2,082 lbs)	4-8	0.338	1,064 lbs	(-2,085 lbs)										

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:14
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	145 lbs

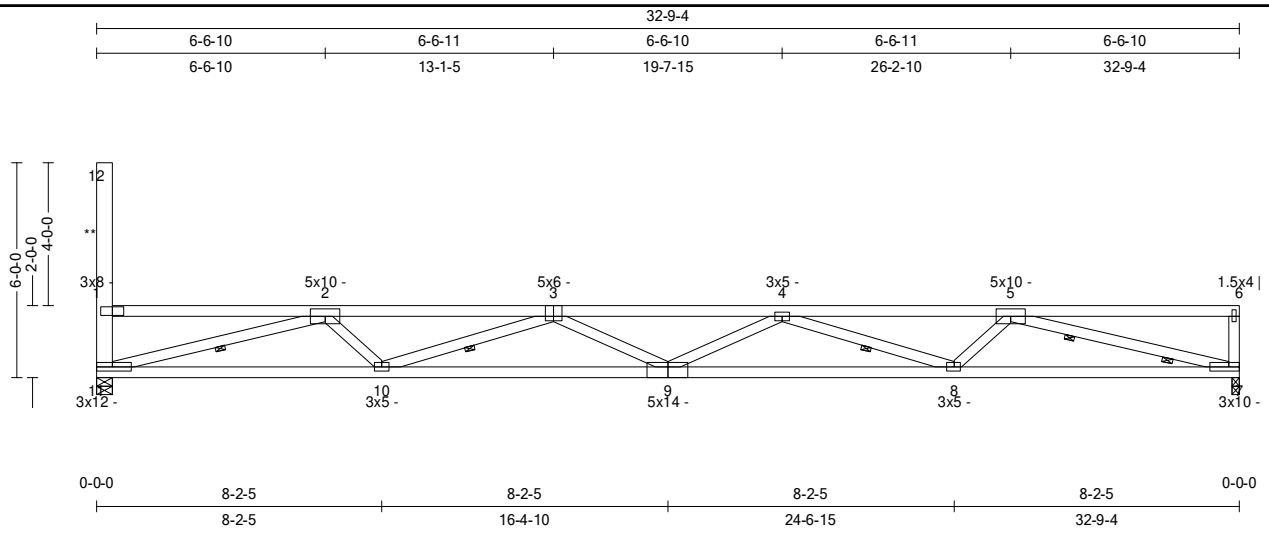
8) ✉ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.56 in, 2L/178 (12-1), Allowable 2L/120.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4DD
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:15
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	145 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.97 (3-4)	Vert TL:	1.77 in	L / 217	(8-9)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.72 (10-11)	Vert LL:	0.75 in	L / 514	9	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.96 (2-11)	Horz TL:	0.27 in		7	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,373 lbs	·	·	-111 lbs	-111 lbs	-9,832 lbs
7	1	2.75 in	1.50 in	1,380 lbs	·	·	-169 lbs	-169 lbs	·

Material

TC: DFL #1B 2 x 4
BC: DFL 2400/2.0 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed
BC: Sheathed or Purlins at 3-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 3-10, 4-8
Two Third Point Rows: 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 9,832 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
7) ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.427	1,911 lbs	(-1,911 lbs)	3-4	0.970	842 lbs	(-6,406 lbs)	5-6	0.440	1,980 lbs	(-1,980 lbs)						
	2-3	0.585	792 lbs	(-4,815 lbs)	4-5	0.585	572 lbs	(-4,851 lbs)										
BC	7-8	0.559	4,213 lbs	(-538 lbs)	8-9	0.700	6,400 lbs	(-1,422 lbs)	9-10	0.699	8,374 lbs	(-3,408 lbs)	10-11	0.719	9,522 lbs	(-6,211 lbs)		
Web	2-11	0.963	388 lbs	(-4,320 lbs)	3-9	0.408	1,285 lbs	(-943 lbs)	5-8	0.407	1,197 lbs							
	2-10	0.411	1,208 lbs		4-9	0.408	1,284 lbs	(-954 lbs)	5-7	0.821	576 lbs	(-4,360 lbs)						
	3-10	0.331	1,042 lbs	(-2,082 lbs)	4-8	0.338	1,064 lbs	(-2,085 lbs)										

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
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Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4DD
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:16
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	145 lbs

8) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.56 in, 2L/178 (12-1), Allowable 2L/120.

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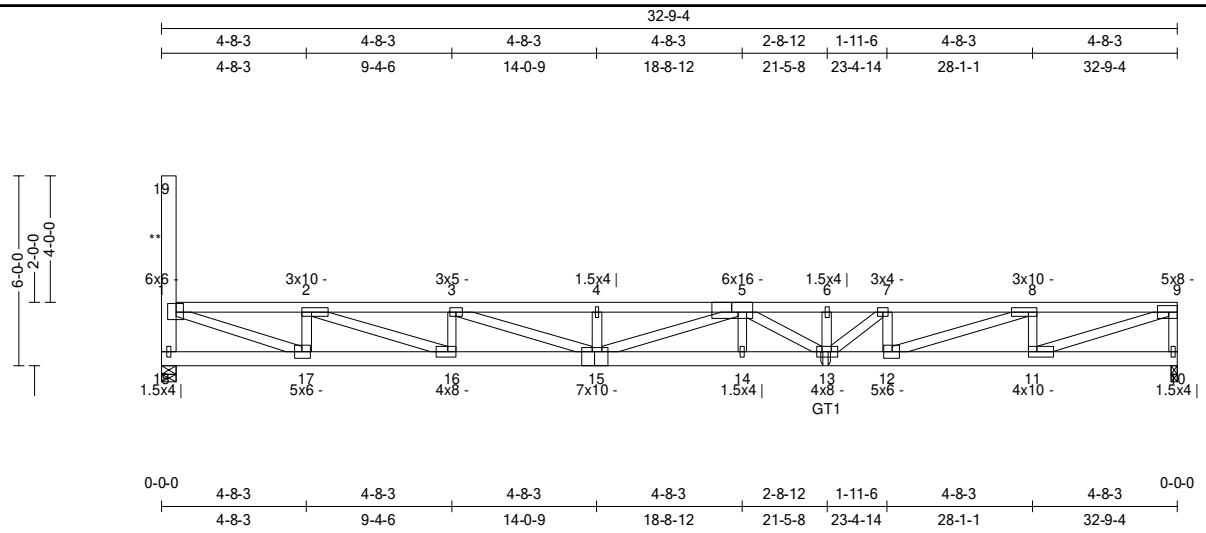
Truss: E4G
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:29
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	47.63 in	180 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IRC 2021/ TPI 1-2014	TC :	0.70 (7-8)	Vert TL:	1.52 in	L / 252	(14-15)	L / 180
TCLL : 25			BC :	0.73 (13-14)	Vert LL:	0.65 in	L / 595	(14-15)	L / 240
TCDL : 7	Rep Mbr :	No	Web :	0.86 (2-16)	Horz TL:	0.11 in		10	
BCLL : 0	Lumber D.O.L. :	115 %							
BCDL : 10									

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	5.5 in	1.50 in	1,887 lbs	·	·	-20 lbs	-20 lbs	392 lbs
10	1	2.75 in	1.51 in	2,827 lbs	·	·	-114 lbs	-114 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 1-17, 8-12, 9-11
DFL SS 2 x 6: 19-18

Bracing

TC: Sheathed or Purlins at 3-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 19-1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	28-8-2	Down	Proj	24.22 plf	24.22 plf	
Top	28-8-2	32-9-4	Down	Proj	24.22 plf	24.22 plf	
Top	0-0-0	21-5-0	Down	Proj	25.78 plf	25.78 plf	
Top	21-5-0	32-9-4	Down	Proj	75 plf	75 plf	

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Truss: E4G
Job: 2501272
Designer:Anthony
Date: 10/28/25 13:53:30
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-9-4	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	47.63 in	180 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	28-8-2	Down	Proj	6.78 plf	6.78 plf	
Top	28-8-2	32-9-4	Down	Proj	6.78 plf	6.78 plf	
Top	0-0-0	21-5-0	Down	Proj	7.22 plf	7.22 plf	
Top	21-5-0	32-9-4	Down	Proj	21 plf	21 plf	
Bot	0-0-0	28-8-2	Down	Proj	9.69 plf	9.69 plf	
Bot	28-8-2	32-9-4	Down	Proj	9.69 plf	9.69 plf	
Bot	0-0-0	21-5-0	Down	Proj	10.31 plf	10.31 plf	
Bot	21-5-0	32-9-4	Down	Proj	30 plf	30 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.270	475 lbs	(-2,319 lbs)	3-4	0.608	324 lbs	(-5,340 lbs)	5-6	0.678		(-6,193 lbs)	7-8	0.700	(-5,618 lbs)
	2-3	0.472	431 lbs	(-4,148 lbs)	4-5	0.603	326 lbs	(-5,347 lbs)	6-7	0.667		(-6,193 lbs)	8-9	0.431	(-3,406 lbs)
BC	11-12	0.459	3,406 lbs		13-14	0.731	6,201 lbs		15-16	0.515	4,148 lbs	(-395 lbs)	17-18	0.042	(-436 lbs)
	12-13	0.684	5,618 lbs		14-15	0.679	6,170 lbs		16-17	0.275	2,319 lbs	(-439 lbs)			
Web	1-18	0.219		(-907 lbs)	3-15	0.558	1,264 lbs		8-11	0.133		(-1,110 lbs)			
	1-17	0.474	2,469 lbs		5-15	0.137		(-877 lbs)	9-11	0.695	3,618 lbs				
	2-17	0.096		(-799 lbs)	7-13	0.331	749 lbs		9-10	0.161		(-1,347 lbs)			
	2-16	0.855	1,936 lbs		7-12	0.085		(-706 lbs)							
	3-16	0.069		(-573 lbs)	8-12	0.450	2,342 lbs								

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
GT1	BC	21-5-0

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc , Webs - 1 row @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental Head Side - FastenMaster FlatLOK (2 - Ply) Screws within 12" along the chord or into converging webs at point load:
BC: 21-5-0,(3)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
9) Lateral bracing shall be attached to each ply.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.49 in, 2L/201 (19-1), Allowable 2L/120.

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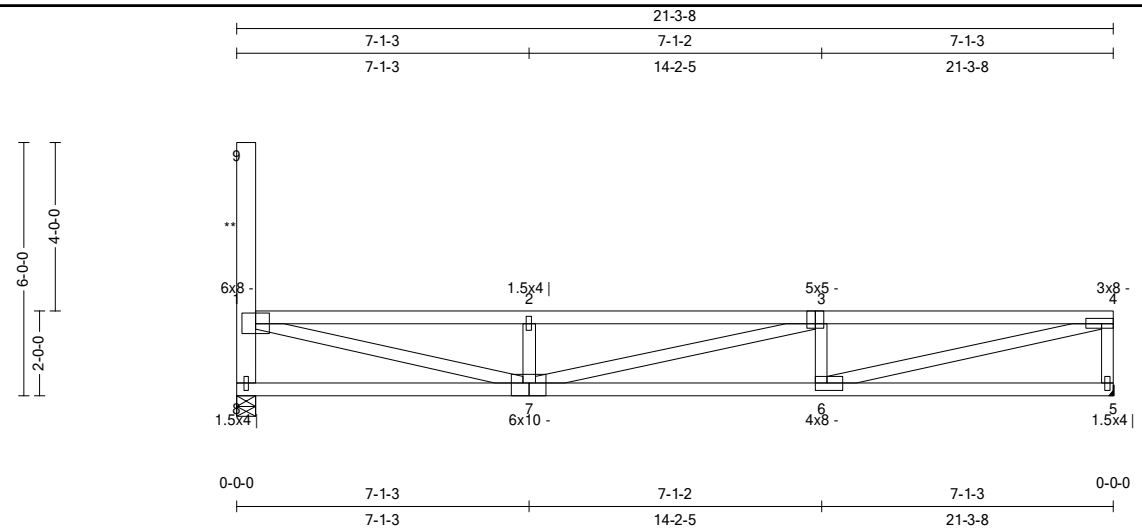
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4S
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:20
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
21-3-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	99 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.79 (3-4)	Vert TL:	0.52 in	L / 477	(6-7)	L / 180
TCDL: 7		TPI 1-2014	BC:	0.76 (6-7)	Vert LL:	0.21 in	L / 999	(6-7)	L / 240
BCLL: 0	Rep Mbr:	No	Web:	0.51 (1-7)	Horz TL:	0.04 in		5	
BCDL: 10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036031
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	5.5 in	1.50 in	890 lbs	·	·	-94 lbs	-94 lbs	197 lbs
5	1	1.5 in	---	898 lbs	·	·	-179 lbs	-179 lbs	·

Material

TC: DFL #1B 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 1-7, 4-6
DFL SS 2 x 6: 9-8

Bracing

TC: Sheathed or Purlins at 2-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-7-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 9-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.710	699 lbs	(-2,581 lbs)	2-3	0.464	699 lbs	(-2,581 lbs)	3-4	0.794	545 lbs	(-2,582 lbs)		
BC	6-7	0.755	2,624 lbs	(-524 lbs)	7-8	0.429		(-425 lbs)						
Web	1-8	0.223		(-826 lbs)	2-7	0.062		(-483 lbs)	3-6	0.071		(-487 lbs)	4-5	0.107		(-834 lbs)
	1-7	0.511	2,660 lbs		3-7	0.464	446 lbs	(-503 lbs)	4-6	0.510	2,658 lbs	(-541 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) A creep factor of 2.00 has been applied for this truss analysis.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E4S
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:21
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
21-3-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	99 lbs

8) Listed wind uplift reactions based on MWFRS & C&C loading.
9) Parapet TL: 0.18 in, 2L/545 (9-1), Allowable 2L/120.

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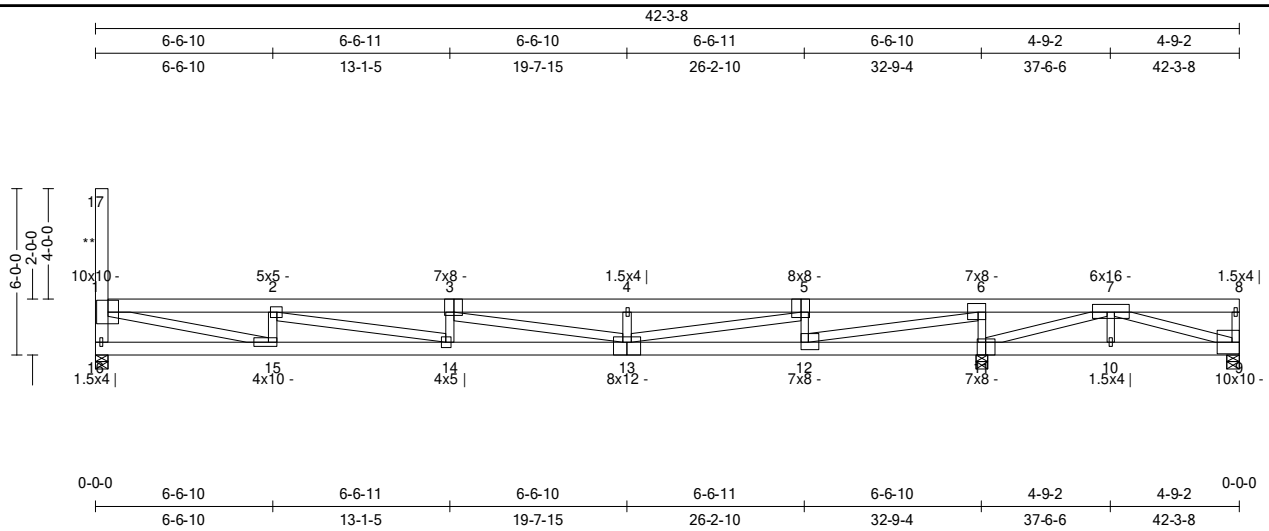
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E5
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:17
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-3-8	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	254 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.66 (6-7)	Vert TL:	1.18 in	L / 327	(13-14)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.51 (13-14)	Vert LL:	0.5 in	L / 772	(13-14)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.97 (7-11)	Horz TL:	0.05 in		11	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.50 in	1,174 lbs	·	·	-64 lbs	-64 lbs	197 lbs
11	1	5.5 in	2.92 in	2,737 lbs	·	·	-282 lbs	-282 lbs	·
9	1	5.5 in	1.50 in	84 lbs	-583 lbs	-65 lbs	·	-583 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 1-15, 5-13, 6-12
DFL SS 2 x 6: 17-16

Bracing

TC: Sheathed or Purlins at 3-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 4-2-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 17-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.207	697 lbs	(-3,745 lbs)	3-4	0.238	525 lbs	(-4,404 lbs)	5-6	0.295		(-1,204 lbs)			
	2-3	0.280	703 lbs	(-5,155 lbs)	4-5	0.238	525 lbs	(-4,404 lbs)	6-7	0.660	3,826 lbs	(-409 lbs)			
BC	9-10	0.106		(-1,990 lbs)	11-12	0.273	420 lbs	(-3,666 lbs)	13-14	0.506	5,175 lbs	(-676 lbs)	15-16	0.101	(-466 lbs)
	10-11	0.244		(-1,990 lbs)	12-13	0.211	1,307 lbs		14-15	0.400	3,745 lbs	(-662 lbs)			
Web	1-16	0.229		(-1,096 lbs)	3-13	0.825		(-1,075 lbs)	6-12	0.958	4,988 lbs	(-526 lbs)			
	1-15	0.740	3,855 lbs		4-13	0.052		(-418 lbs)	6-11	0.210		(-1,701 lbs)			
	2-15	0.097		(-785 lbs)	5-13	0.616	3,206 lbs	(-389 lbs)	7-11	0.971		(-2,295 lbs)			
	2-14	0.639	1,448 lbs		5-12	0.136		(-1,104 lbs)	7-9	0.927	2,098 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E5
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:18
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-3-8	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	254 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 9 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.
9) Parapet TL: 0.34 in, 2L/297 (17-1), Allowable 2L/120.

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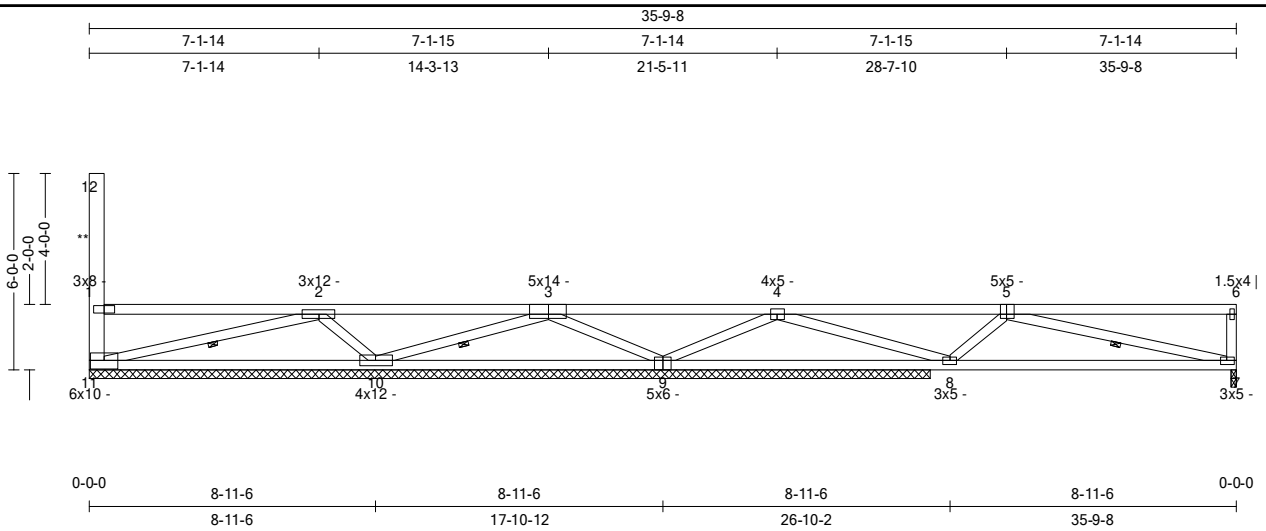
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E6D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:19
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-9-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	156 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.96 (3-4)	Vert TL:	0.46 in	L / 246	(7-8)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.96 (7-8)	Vert LL:	0.18 in	L / 622	(7-8)	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.89 (3-10)	Horz TL:	0.06 in		7	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	2 in	1.50 in	627 lbs	-89 lbs	·	-62 lbs	-89 lbs	·
9	1	315 in	N/A	1,441 lbs	·	·	-166 lbs	-166 lbs	5,503 lbs
10	1	315 in	N/A	746 lbs	·	·	-81 lbs	-81 lbs	-3,372 lbs
11	1	315 in	N/A	680 lbs	-460 lbs	-11 lbs	·	-460 lbs	-2,320 lbs

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 12-11

Bracing

TC: Sheathed or Purlins at 4-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 3-10, 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 10,738 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
7) ** - Indicates parapet wind loading has been applied to member 12-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.846	2,092 lbs	(-2,092 lbs)	3-4	0.962	3,432 lbs	(-2,421 lbs)	5-6	0.776	2,161 lbs	(-2,161 lbs)
	2-3	0.906	1,468 lbs	(-1,220 lbs)	4-5	0.617	1,980 lbs	(-3,164 lbs)				
BC	7-8	0.956	1,957 lbs	(-834 lbs)	8-9	0.802	2,889 lbs	(-2,766 lbs)				
Web	2-11	0.759	2,390 lbs	(-2,377 lbs)	3-9	0.657	1,596 lbs	(-2,209 lbs)	5-8	0.169	533 lbs	(-449 lbs)
	2-10	0.209	660 lbs	(-986 lbs)	4-9	0.571	305 lbs	(-1,846 lbs)	5-7	0.507	859 lbs	(-2,014 lbs)
	3-10	0.886	2,790 lbs	(-2,570 lbs)	4-8	0.535	1,686 lbs	(-573 lbs)				

DRAG LOAD TO MATCH SHEAR WALL CAPACITY BELOW (I.E. 460 PLF ASD FOR TYPE W/4) - TYP

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: E6D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:20
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-9-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	156 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 11 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.16 in, 2L/639 (12-1), Allowable 2L/120.

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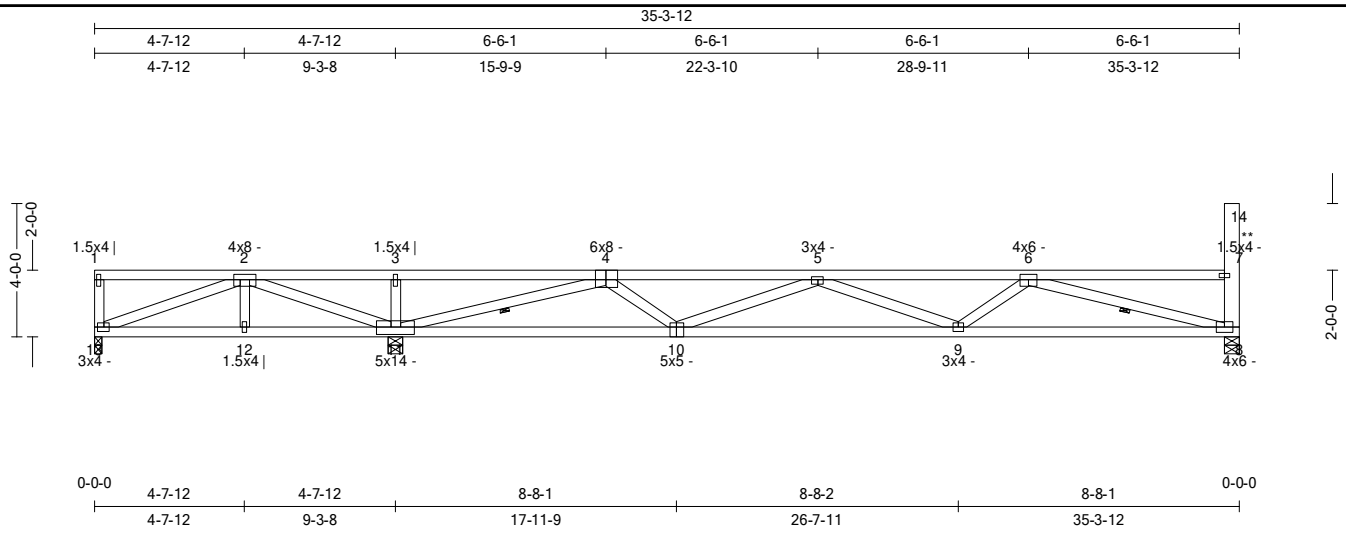
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:21
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-3-12	0/12	27	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.99 (2-3)	Vert TL:	0.78 in	L / 389	(9-10)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.92 (9-10)	Vert LL:	0.31 in	L / 996	(9-10)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.88 (4-11)	Horz TL:	0.1 in		8	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.50 in	189 lbs	-308 lbs	-36 lbs	·	-308 lbs	-96 lbs
11	1	5.5 in	2.27 in	2,126 lbs	·	·	-208 lbs	-208 lbs	·
8	1	5.5 in	1.50 in	928 lbs	·	·	-71 lbs	-71 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-11, 6-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.995	2,322 lbs		4-5	0.405		(-1,989 lbs)			
	3-4	0.995	2,322 lbs		5-6	0.459	332 lbs	(-2,819 lbs)			
BC	8-9	0.877	2,523 lbs		10-11	0.640	1,315 lbs		12-13	0.170	(-1,019 lbs)
	9-10	0.920	2,932 lbs	(-301 lbs)	11-12	0.397		(-1,019 lbs)			
Web	2-13	0.482	1,090 lbs		4-10	0.413	936 lbs				
	2-11	0.732		(-1,744 lbs)	5-10	0.473		(-1,133 lbs)			
	3-11	0.052		(-408 lbs)	6-9	0.228	516 lbs				
	4-11	0.880	447 lbs	(-3,720 lbs)	6-8	0.577		(-2,615 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:22
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-3-12	0/12	27	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.13 in, 2L/395 (14-7), Allowable 2L/120.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
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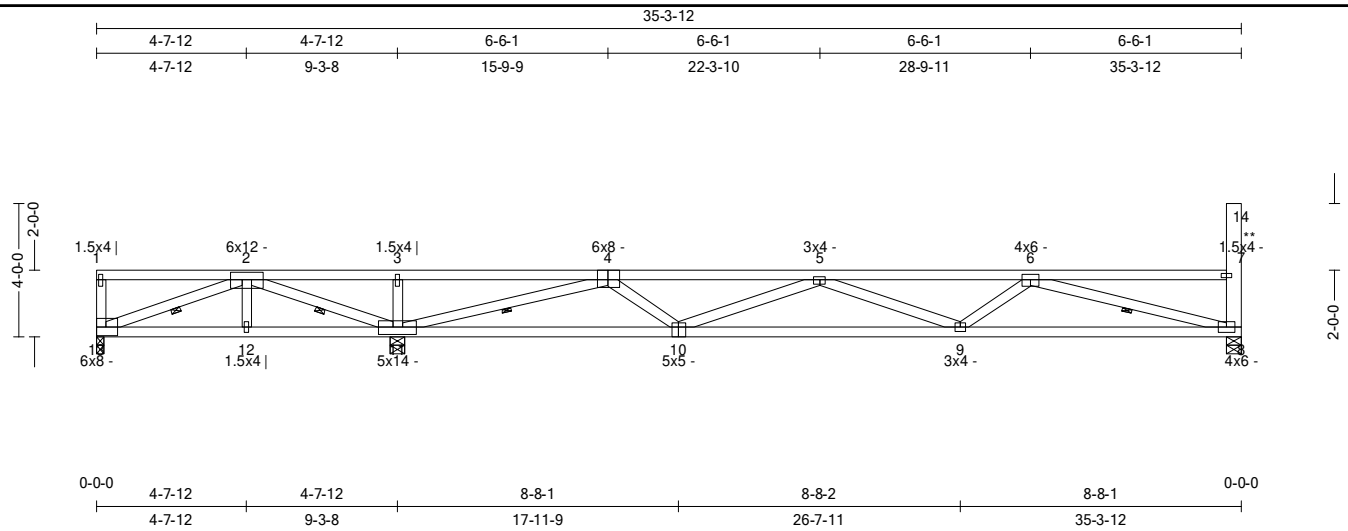
Truss: F1D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:23
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-3-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	153 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.84 (3-4)	Vert TL:	0.72 in	L / 421	(9-10)	L / 180
TCDL: 7		TPI 1-2014	BC:	0.96 (11-12)	Vert LL:	0.12 in	L / 913	(12-13)	L / 240
BCLL: 0	Rep Mbr:	No	Web:	0.88 (4-11)	Horz TL:	0.18 in		8	
BCDL: 10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.50 in	1,175 lbs	-1,238 lbs	-35 lbs	·	-1,238 lbs	-10,594 lbs
11	1	5.5 in	2.26 in	2,118 lbs	-77 lbs	·	-207 lbs	-207 lbs	·
8	1	5.5 in	1.50 in	930 lbs	·	·	-72 lbs	-72 lbs	·

10/28/2025

Material

TC: DFL #1B 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-13
DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.
BC: Sheathed or Purlins at 2-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-13, 2-11, 4-11, 6-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 10,594 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
7) ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.315	1,403 lbs	(-1,403 lbs)	3-4	0.835	4,476 lbs	(-2,654 lbs)	5-6	0.397	1,319 lbs	(-3,697 lbs)
	2-3	0.835	5,878 lbs	(-4,057 lbs)	4-5	0.353	2,265 lbs	(-3,946 lbs)	6-7	0.426	1,895 lbs	(-1,895 lbs)
BC	8-9	0.737	2,528 lbs		10-11	0.612	3,554 lbs	(-2,593 lbs)	12-13	0.836	7,011 lbs	(-7,563 lbs)
	9-10	0.771	3,177 lbs	(-907 lbs)	11-12	0.955	7,011 lbs	(-7,563 lbs)				
Web	2-13	0.529	3,832 lbs	(-3,241 lbs)	4-10	0.412	934 lbs		6-8	0.578		(-2,620 lbs)
	2-11	0.822	2,589 lbs	(-3,941 lbs)	5-10	0.472	411 lbs	(-1,131 lbs)				
	3-11	0.053		(-412 lbs)	5-9	0.278	801 lbs	(-684 lbs)				
	4-11	0.876	445 lbs	(-3,703 lbs)	6-9	0.229	718 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F1D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:24
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-3-12	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	153 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 13, 11 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.12 in, 2L/439 (14-7), Allowable 2L/120.

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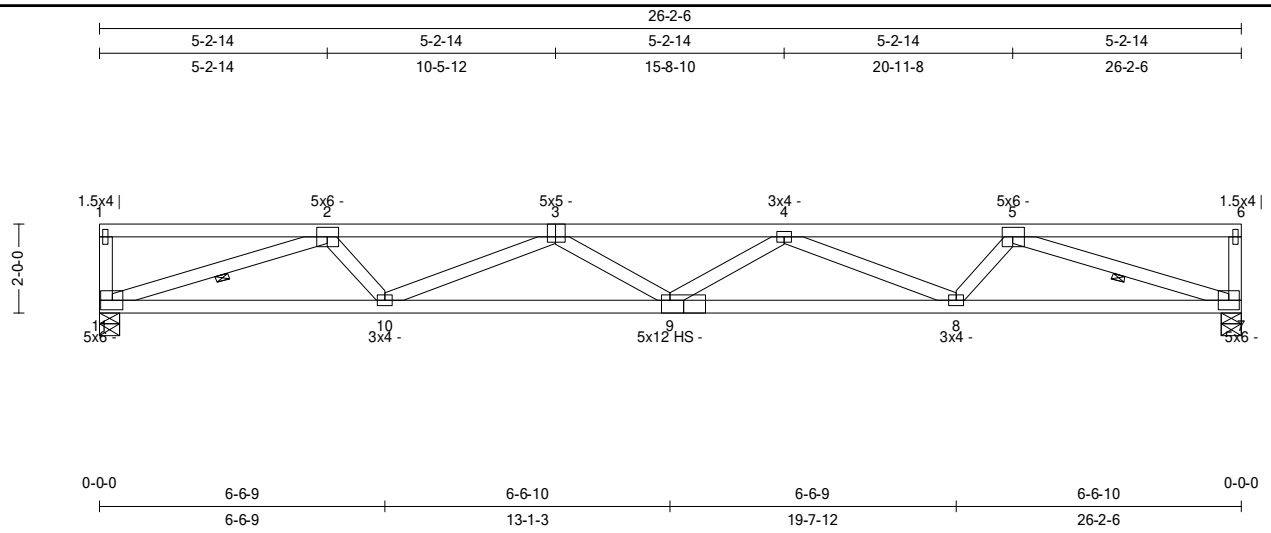
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:25
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-2-6	0/12	5	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	110 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.46 (3-4)	Vert TL:	0.86 in	L / 351	(9-10)	L / 180
TCDL:	7	Rep Mbr:	Yes	BC:	0.86 (9-10)	Vert LL:	0.36 in	L / 850	9	L / 240
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.49 (2-11)	Horz TL:	0.17 in		7	
BCDL:	10									

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
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PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,100 lbs	·	·	-147 lbs	-147 lbs	32 lbs
7	1	5.5 in	1.50 in	1,100 lbs	·	·	-147 lbs	-147 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 2-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.325	374 lbs	(-3,079 lbs)	4-5	0.325	378 lbs	(-3,073 lbs)							
	3-4	0.464	499 lbs	(-4,073 lbs)											
BC	7-8	0.645	2,666 lbs	(-347 lbs)	8-9	0.832	3,984 lbs	(-514 lbs)	9-10	0.864	3,973 lbs	(-520 lbs)	10-11	0.645	2,670 lbs	(-345 lbs)
Web	2-11	0.492	384 lbs	(-2,816 lbs)	3-9	0.176	399 lbs		5-8	0.296	670 lbs					
	2-10	0.297	672 lbs		4-9	0.155	351 lbs		5-7	0.491	386 lbs	(-2,812 lbs)				
	3-10	0.347		(-975 lbs)	4-8	0.354		(-993 lbs)								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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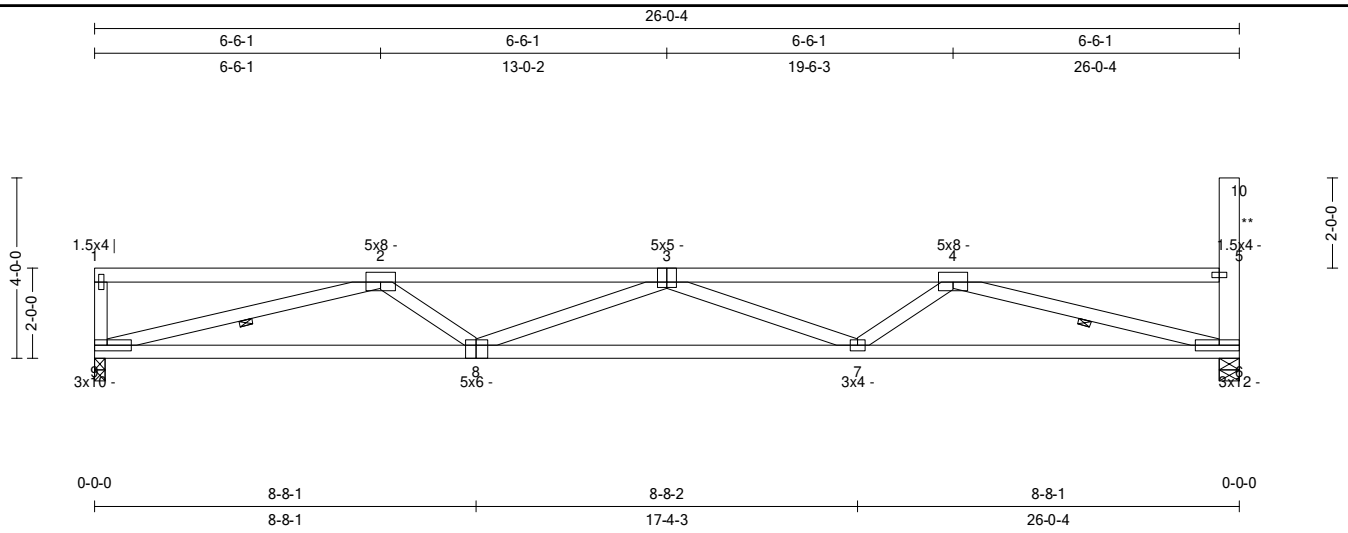
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:27
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	22	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	112 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.56 (1-2)	Vert TL:	0.92 in	L / 332	(7-8)	L / 180
TCDL: 7		TPI 1-2014	BC:	0.76 (7-8)	Vert LL:	0.37 in	L / 816	(7-8)	L / 240
BCLL: 0	Rep Mbr:	Yes	Web:	0.74 (2-9)	Horz TL:	0.16 in		6	
BCDL: 10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
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PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	2.75 in	1.50 in	1,097 lbs	·	·	-157 lbs	-157 lbs	-96 lbs
6	1	5.5 in	1.50 in	1,089 lbs	·	·	-124 lbs	-124 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-6-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.509	461 lbs	(-3,616 lbs)	3-4	0.504	514 lbs	(-3,596 lbs)				
BC	6-7	0.665	3,102 lbs	(-450 lbs)	7-8	0.755	4,120 lbs	(-528 lbs)	8-9	0.675	3,131 lbs	(-372 lbs)
Web	2-9	0.742	504 lbs	(-3,242 lbs)	3-7	0.245		(-588 lbs)				
	2-8	0.299	677 lbs		4-7	0.301	681 lbs					
	3-8	0.252		(-605 lbs)	4-6	0.709	420 lbs	(-3,215 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F3
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:27
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	22	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	112 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.16 in, 2L/324 (10-5), Allowable 2L/120.

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Mustang Truss
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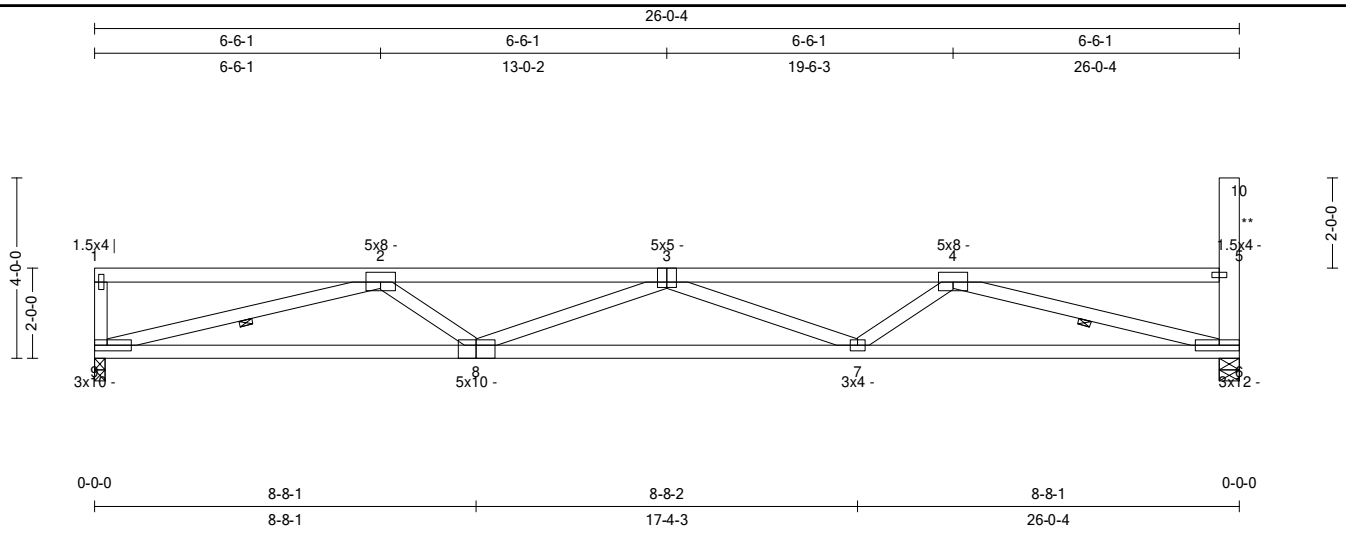
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: F3D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:28
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	113 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.64 (1-2)	Vert TL:	0.92 in	L / 332	(7-8)	L / 180
TCDL: 7		TPI 1-2014	BC:	0.89 (8-9)	Vert LL:	0.37 in	L / 816	(7-8)	L / 240
BCLL: 0	Rep Mbr:	No	Web:	0.74 (2-9)	Horz TL:	0.17 in		6	
BCDL: 10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	2.75 in	1.50 in	1,097 lbs	-77 lbs	·	-157 lbs	-157 lbs	-7,806 lbs
6	1	5.5 in	1.50 in	1,089 lbs	-78 lbs	·	-124 lbs	-124 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects due to a 7,806 lbs (300 plf) drag load distributed along the TC rake from each direction.
3) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
4) This truss has not been designed for the effects of unbalanced snow loads.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
7) ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.642	1,969 lbs	(-1,969 lbs)	2-3	0.602	461 lbs	(-3,616 lbs)	3-4	0.603	514 lbs	(-3,596 lbs)	4-5	0.631	1,900 lbs	(-1,900 lbs)
BC	6-7	0.752	3,107 lbs	(-683 lbs)	7-8	0.853	5,493 lbs	(-2,240 lbs)	8-9	0.888	7,061 lbs	(-4,615 lbs)				
Web	2-9	0.742	771 lbs	(-3,305 lbs)	3-7	0.627	1,285 lbs	(-1,541 lbs)								
	2-8	0.386	1,216 lbs	(-470 lbs)	4-7	0.385	1,213 lbs	(-462 lbs)								
	3-8	0.621	1,288 lbs	(-1,525 lbs)	4-6	0.709	707 lbs	(-3,220 lbs)								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

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Salem, OR 97301
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Truss: F3D
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:29
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-4	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	113 lbs

9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 9, 6 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.16 in, 2L/324 (10-5), Allowable 2L/120.

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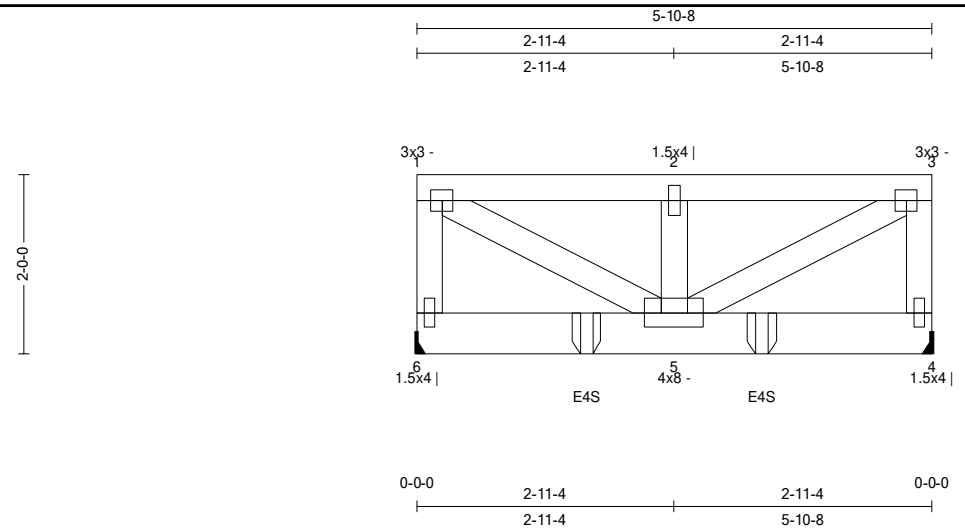
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: GT1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:27
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
5-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	12 in	32 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IRC 2021/	TC :	0.06 (1-2)	Vert TL:	0.02 in	L / 999	(4-5)	L / 180
TCLL : 25		TPI 1-2014	BC :	0.11 (4-5)	Vert LL:	0.01 in	L / 999	(4-5)	L / 240
TCDL : 7	Rep Mbr :	No	Web :	0.30 (1-5)	Horz TL:	0 in		4	
BCLL : 0	Lumber D.O.L. :	115 %							
BCDL : 10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	1,022 lbs	·	·	·	·	-16 lbs
4	1	1.5 in	---	1,022 lbs	·	·	·	·	·

10/28/2025
ANISH KEKRE
STATE OF WASHINGTON
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Material

TC: DFL #2 2 x 4
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.065		(-588 lbs)	2-3	0.065		(-588 lbs)			
BC											
Web	1-6	0.044		(-369 lbs)	3-5	0.300	680 lbs				
	1-5	0.300	680 lbs		3-4	0.044		(-369 lbs)			

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
E4S	BC	1-11-4
E4S	BC	3-11-4

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
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Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: GT1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:27
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
5-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	12 in	32 lbs

7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 1 row @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc , Webs - 1 row @ 2-0-0 oc.
8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
9) Lateral bracing shall be attached to each ply.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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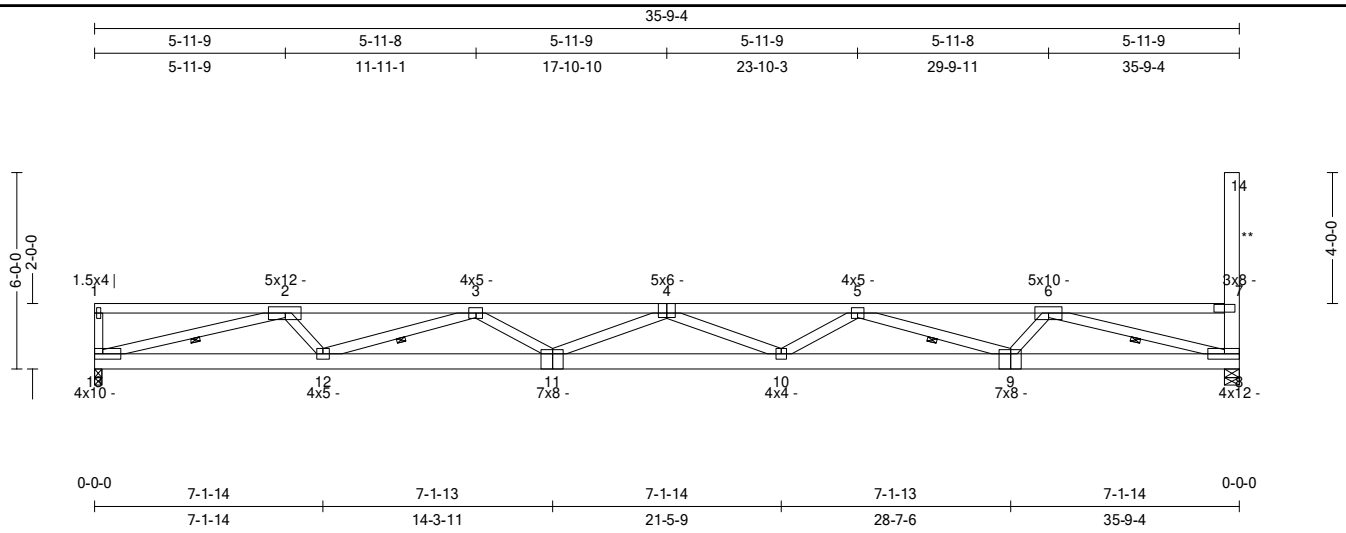
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:30
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-9-4	0/12	26	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	184 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.88 (3-4)	Vert TL:	2.15 in	L / 196	(10-11)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.77 (10-11)	Vert LL:	0.91 in	L / 464	(10-11)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.93 (2-13)	Horz TL:	0.24 in		8	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
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REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.61 in	1,506 lbs	·	·	-165 lbs	-165 lbs	-197 lbs
8	1	5.5 in	1.60 in	1,498 lbs	·	·	-112 lbs	-112 lbs	·

Material

TC: DFL #1B 2 x 4
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed
BC: Sheathed or Purlins at 9-10-0, Purlin design by Others.
Web: One Midpoint Row: 2-13, 3-12, 5-9, 6-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.484	549 lbs	(-5,076 lbs)	4-5	0.875	961 lbs	(-7,612 lbs)	6-7	0.308	476 lbs	
	3-4	0.878	871 lbs	(-7,627 lbs)	5-6	0.480	817 lbs	(-5,030 lbs)				
BC	8-9	0.442	4,415 lbs	(-605 lbs)	10-11	0.767	8,046 lbs	(-816 lbs)	12-13	0.449	4,466 lbs	(-307 lbs)
	9-10	0.689	7,119 lbs	(-794 lbs)	11-12	0.692	7,143 lbs	(-644 lbs)				
Web	2-13	0.933	542 lbs	(-4,637 lbs)	4-11	0.202		(-670 lbs)	6-9	0.460	1,040 lbs	
	2-12	0.456	1,032 lbs		4-10	0.197		(-655 lbs)	6-8	0.891	376 lbs	(-4,589 lbs)
	3-12	0.366	346 lbs	(-2,184 lbs)	5-10	0.323	732 lbs					
	3-11	0.328	743 lbs		5-9	0.369		(-2,206 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:31
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-9-4	0/12	26	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	184 lbs

8) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.65 in, 2L/153 (14-7), Allowable 2L/120.

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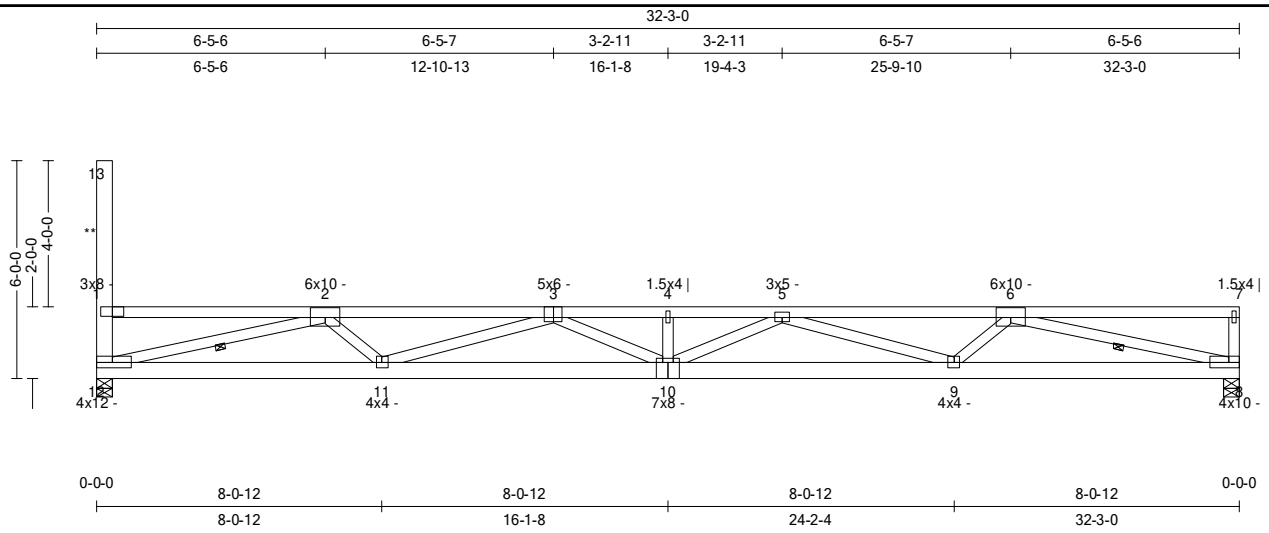
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:32
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-3-0	0/12	32	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	167 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.65 (2-3)	Vert TL:	1.6 in	L / 235	(9-10)	L / 180
TCDL:	7	Rep Mbr:	Yes	BC:	0.63 (9-10)	Vert LL:	0.68 in	L / 554	10	L / 240
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.98 (6-8)	Horz TL:	0.18 in		8	
BCDL:	10									

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
240360B1
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	5.5 in	1.50 in	1,351 lbs	·	·	-112 lbs	-112 lbs	197 lbs
8	1	5.5 in	1.50 in	1,358 lbs	·	·	-169 lbs	-169 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 13-12

Bracing

TC: Sheathed or Purlins at 2-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-12, 6-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 13-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.502	476 lbs		3-4	0.648	882 lbs	(-6,492 lbs)	5-6	0.654	578 lbs	(-4,833 lbs)					
	2-3	0.654	802 lbs	(-4,798 lbs)	4-5	0.648	882 lbs	(-6,492 lbs)									
BC	8-9	0.442	4,213 lbs	(-550 lbs)	9-10	0.627	6,255 lbs	(-854 lbs)	10-11	0.625	6,242 lbs	(-944 lbs)	11-12	0.436	4,171 lbs	(-819 lbs)	
Web	2-12	0.932	392 lbs	(-4,311 lbs)	3-10	0.267	605 lbs		6-9	0.389	880 lbs						
	2-11	0.393	890 lbs		5-10	0.273	618 lbs		6-8	0.976	586 lbs	(-4,351 lbs)					
	3-11	0.751		(-1,524 lbs)	5-9	0.740	329 lbs	(-1,500 lbs)									

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:33
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-3-0	0/12	32	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	167 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.5 in, 2L/200 (13-1), Allowable 2L/120.

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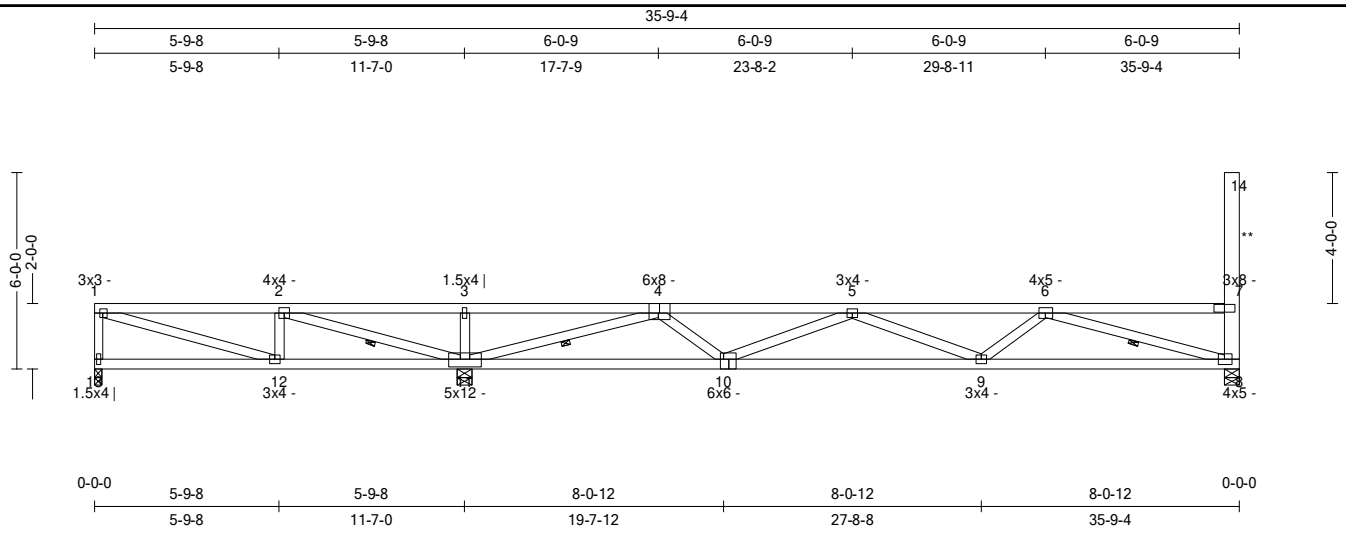
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H4
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:34
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-9-4	0/12	16	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	157 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.90 (2-3)	Vert TL:	0.58 in	L / 483	(9-10)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.79 (9-10)	Vert LL:	0.24 in	L / 999	(9-10)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.68 (4-11)	Horz TL:	0.09 in		8	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	2.75 in	1.50 in	344 lbs	-99 lbs	-8 lbs	-19 lbs	-99 lbs	-197 lbs
11	1	5.5 in	2.10 in	1,964 lbs	·	·	-217 lbs	-217 lbs	·
8	1	5.5 in	1.50 in	875 lbs	·	·	-42 lbs	-42 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 14-8

Bracing

TC: Sheathed or Purlins at 3-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 7-10-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 4-11, 6-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 14-7

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.427	625 lbs	(-451 lbs)	3-4	0.903	1,888 lbs		5-6	0.388	490 lbs	(-2,484 lbs)
	2-3	0.903	1,888 lbs		4-5	0.379		(-1,870 lbs)	6-7	0.429	463 lbs	
BC	8-9	0.745	2,214 lbs	(-366 lbs)	10-11	0.598	1,265 lbs					
	9-10	0.786	2,615 lbs		11-12	0.371	451 lbs	(-625 lbs)				
Web	1-12	0.390	471 lbs	(-653 lbs)	4-10	0.369	836 lbs					
	2-11	0.361		(-1,846 lbs)	5-10	0.331		(-910 lbs)				
	3-11	0.053		(-417 lbs)	6-9	0.205	465 lbs					
	4-11	0.681	466 lbs	(-3,189 lbs)	6-8	0.460		(-2,307 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H4
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:35
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-9-4	0/12	16	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	157 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.21 in, 2L/481 (14-7), Allowable 2L/120.

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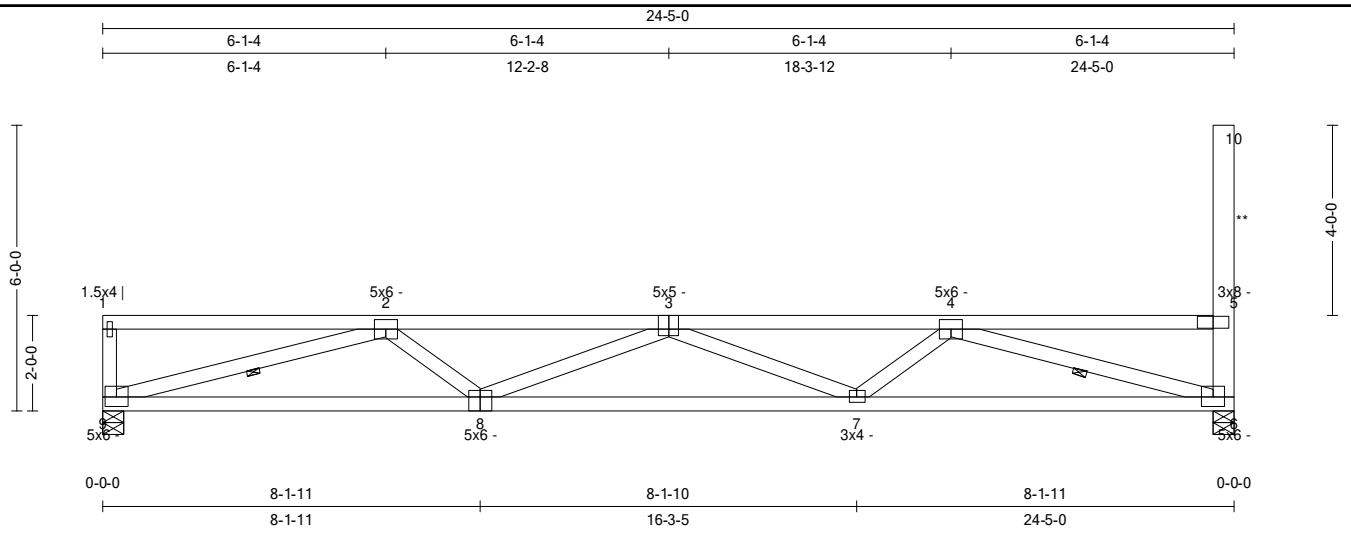
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H5
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:36
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-5-0	0/12	11	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	110 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.49 (1-2)	Vert TL:	0.75 in	L / 375	(7-8)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.91 (7-8)	Vert LL:	0.31 in	L / 924	(7-8)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.60 (2-9)	Horz TL:	0.15 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
240360 81
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,029 lbs	·	·	-177 lbs	-177 lbs	-197 lbs
6	1	5.5 in	1.50 in	1,022 lbs	·	·	-102 lbs	-102 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-6

Bracing

TC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-9-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 10-5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.430	530 lbs	(-3,179 lbs)	3-4	0.428	679 lbs	(-3,160 lbs)	4-5	0.477	465 lbs	
BC	6-7	0.794	2,725 lbs	(-528 lbs)	7-8	0.906	3,623 lbs	(-525 lbs)	8-9	0.807	2,752 lbs	(-306 lbs)
Web	2-9	0.599	544 lbs	(-2,863 lbs)	3-7	0.197		(-522 lbs)				
	2-8	0.270	611 lbs		4-7	0.272	615 lbs					
	3-8	0.202		(-538 lbs)	4-6	0.573	312 lbs	(-2,838 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: H5
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:36
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-5-0	0/12	11	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	110 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.27 in, 2L/365 (10-5), Allowable 2L/120.

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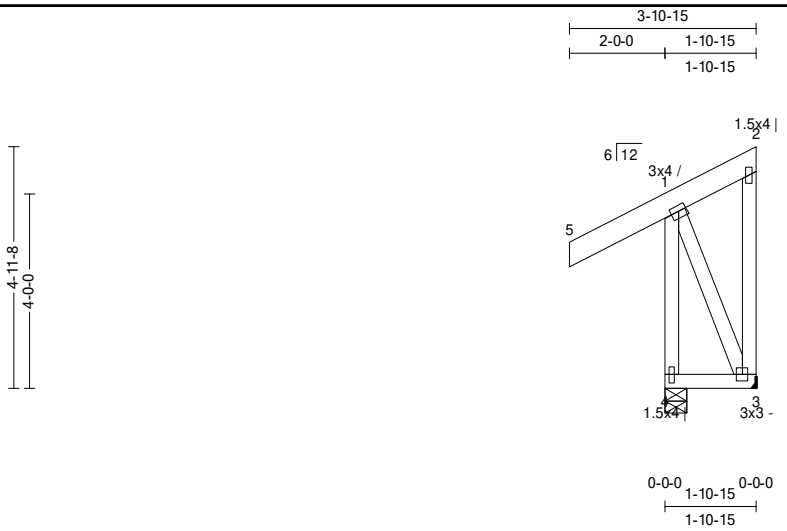
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: J1 Job: 2501272 Designer:Anthony Date: 10/28/25 13:53:21 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-15	6/12	4	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	26 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.13 (5-1)	Vert TL:	0 in	L / 999	(3-4)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.03 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 240
BCLL :	0	Rep Mbr :	Yes	Web :	0.34 (2-3)	Cant / OH TL:	0 in	2L / 999	4	2L / 480
BCDL :	10	Lumber D.O.L. :	115 %			Cant / OH LL:	0 in	2L / 999	4	2L / 480
						Horz TL:	0 in		3	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	343 lbs	·	-57 lbs	-49 lbs	-57 lbs	123 lbs
3	1	1.5 in	---	73 lbs	-96 lbs	-120 lbs	-194 lbs	-194 lbs	·

10/28/2025

Material

TC: DFL SS 2 x 6
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed to account for the effects of ice dams forming at the eaves.
5) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
6) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC											
BC											
Web	1-4	0.236		(-324 lbs)	1-3	0.107		337 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 3 may need to be considered.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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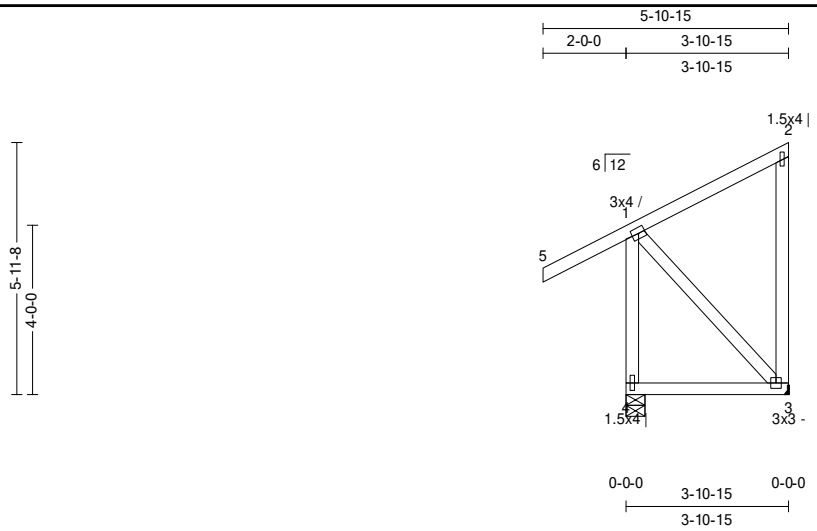
	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: J2 Job: 2501272 Designer:Anthony Date: 10/28/25 13:53:23 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-10-15	6/12	4	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	31 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.47 (5-1)	Vert TL:	0.02 in	L/999	(3-4)	L/180
TCDL: 7		TPI 1-2014	BC:	0.14 (3-4)	Vert LL:	0.01 in	L/999	(3-4)	L/240
BCLL: 0	Rep Mbr:	Yes	Web:	0.50 (2-3)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	365 lbs	·	-14 lbs	-113 lbs	-113 lbs	147 lbs
3	1	1.5 in	---	151 lbs	·	-64 lbs	-193 lbs	-193 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web	1-4	0.266	(-326 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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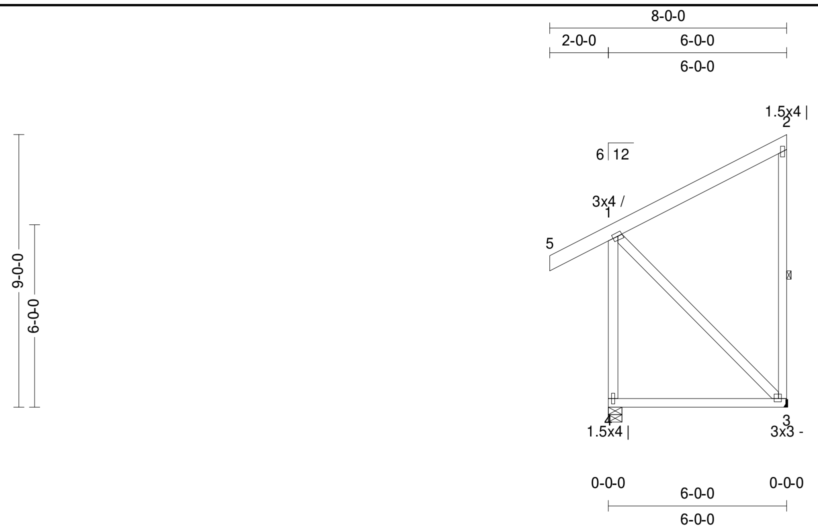
Eagle Metal Products

	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: J3 Job: 2501272 Designer:Anthony Date: 10/28/25 13:53:24 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
6-0-0	6/12	17	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	53 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 25	Bldg Code :	IRC 2021/	TC :	0.13 (5-1)	Vert TL:	0.11 in	L / 569	(3-4)	L / 180
TCDL : 7		TPI 1-2014	BC :	0.36 (3-4)	Vert LL:	0.06 in	L / 999	(3-4)	L / 240
BCLL : 0	Rep Mbr :	Yes	Web :	0.53 (1-4)	Horz TL:	0 in		3	
BCDL : 10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	443 lbs	·	-16 lbs	-82 lbs	-82 lbs	156 lbs
3	1	1.5 in	---	231 lbs	·	-89 lbs	-211 lbs	-211 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-3

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-3

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web	1-4 0.531 (-383 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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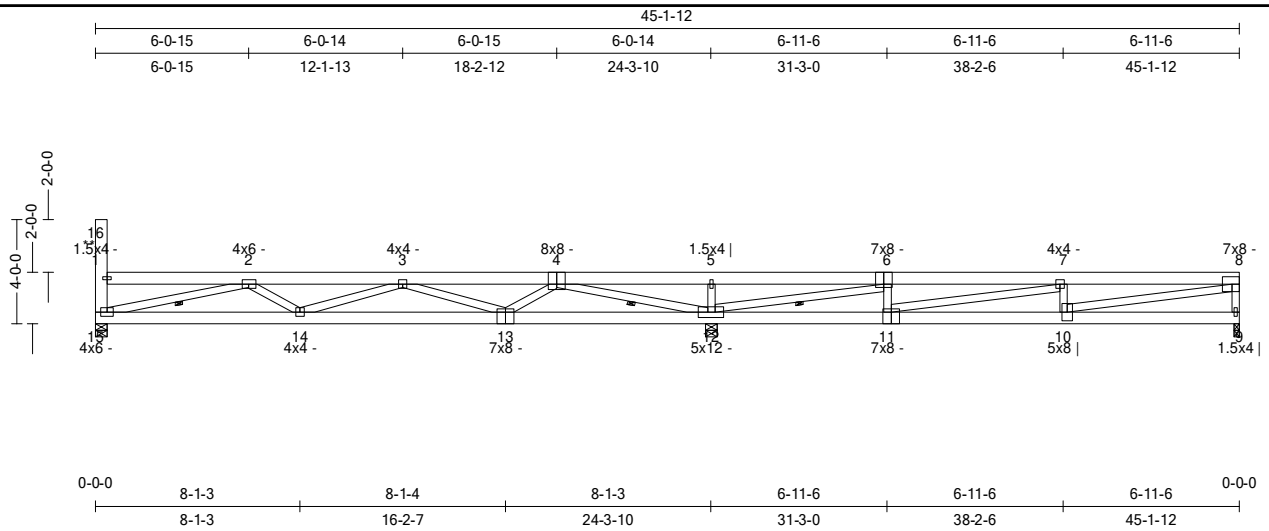
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: K1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:38
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-1-12	0/12	10	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	256 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.54 (5-6)	Vert TL:	0.44 in	L / 646	(13-14)	L / 180
TCDL: 7		TPI 1-2014	BC:	0.32 (13-14)	Vert LL:	0.2 in	L / 999	(13-14)	L / 240
BCLL: 0	Rep Mbr:	Yes	Web:	0.93 (8-10)	Horz TL:	0.07 in		9	
BCDL: 10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	883 lbs	·	·	-51 lbs	-51 lbs	96 lbs
12	1	5.5 in	2.44 in	2,283 lbs	·	·	-212 lbs	-212 lbs	·
9	1	2.75 in	1.50 in	746 lbs	·	·	-57 lbs	-57 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 16-15

Bracing

TC: Sheathed or Purlins at 5-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-15, 4-12, 6-12

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 16-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.144		(-2,691 lbs)	4-5	0.537	2,203 lbs		6-7	0.158	373 lbs	(-1,417 lbs)
	3-4	0.138		(-1,940 lbs)	5-6	0.537	2,203 lbs		7-8	0.186		(-2,063 lbs)
BC	10-11	0.258	2,063 lbs		12-13	0.305	1,328 lbs		14-15	0.264	2,375 lbs	(-338 lbs)
	11-12	0.297	1,374 lbs	(-424 lbs)	13-14	0.322	2,876 lbs	(-320 lbs)				
Web	2-15	0.485		(-2,456 lbs)	4-12	0.664	362 lbs	(-3,141 lbs)	7-11	0.886		(-1,029 lbs)
	2-14	0.211	477 lbs		5-12	0.073		(-592 lbs)	7-10	0.041		(-334 lbs)
	3-13	0.412		(-1,148 lbs)	6-12	0.733		(-2,966 lbs)	8-10	0.934	2,115 lbs	
	4-13	0.368	834 lbs		6-11	0.165	374 lbs		8-9	0.082		(-668 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: K1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:39
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-1-12	0/12	10	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	256 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.07 in, 2L/736 (16-1), Allowable 2L/120.

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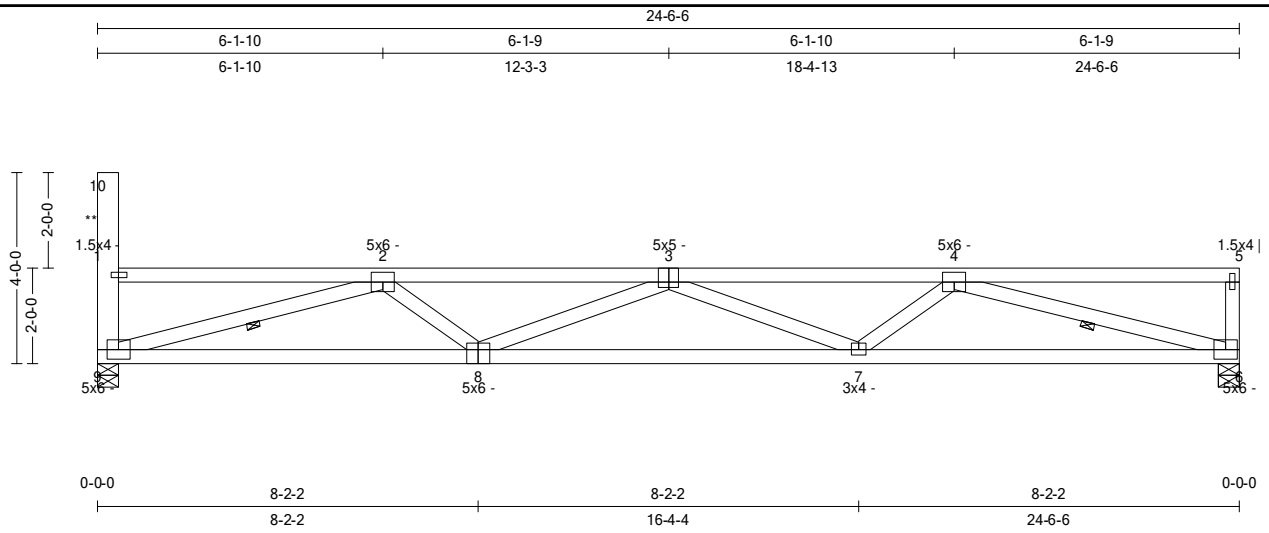
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: K2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:40
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-6	0/12	10	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	106 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.49 (4-5)	Vert TL:	0.77 in	L / 370	(7-8)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.91 (7-8)	Vert LL:	0.31 in	L / 912	(7-8)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.61 (4-6)	Horz TL:	0.15 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	1,026 lbs	·	·	-122 lbs	-122 lbs	96 lbs
6	1	5.5 in	1.50 in	1,034 lbs	·	·	-157 lbs	-157 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 10-9

Bracing

TC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-6-0, Purlin design by Others.
Web: One Midpoint Row: 2-9, 4-6

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 10-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.433	489 lbs	(-3,190 lbs)	3-4	0.435	437 lbs	(-3,209 lbs)				
BC	6-7	0.815	2,778 lbs	(-434 lbs)	7-8	0.914	3,657 lbs	(-582 lbs)	8-9	0.802	2,751 lbs	(-512 lbs)
Web	2-9	0.582	388 lbs	(-2,864 lbs)	3-7	0.206		(-542 lbs)				
	2-8	0.274	620 lbs		4-7	0.272	616 lbs					
	3-8	0.200		(-527 lbs)	4-6	0.609	472 lbs	(-2,889 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: K2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:40

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-6	0/12	10	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	106 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.14 in, 2L/360 (10-1), Allowable 2L/120.

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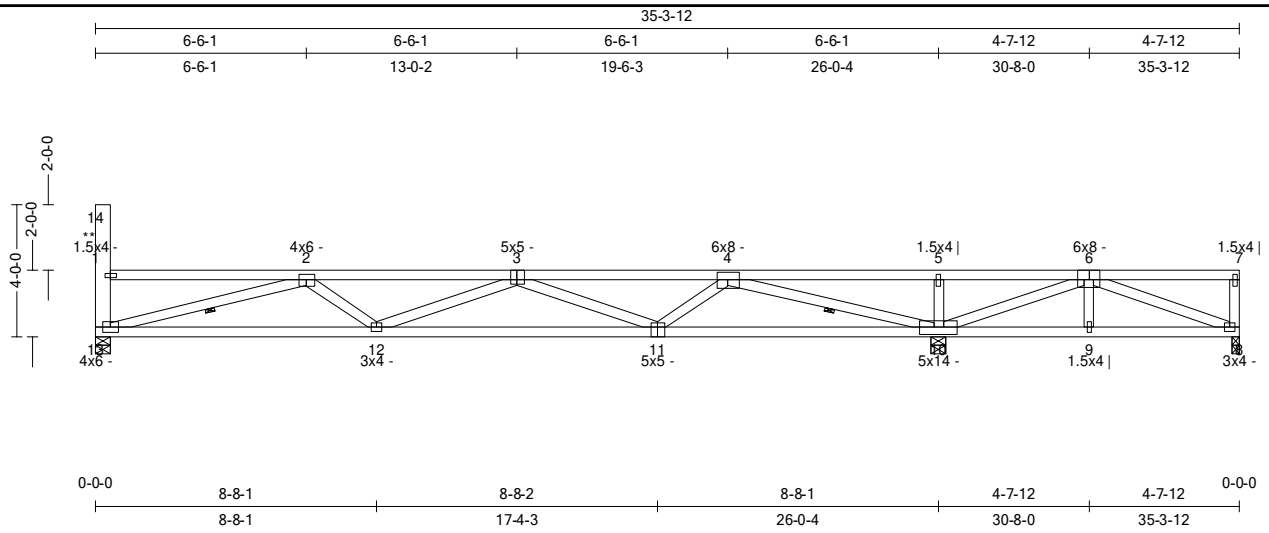
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: L1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:42
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-3-12	0/12	29	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.99 (5-6)	Vert TL:	0.78 in	L / 389	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.92 (11-12)	Vert LL:	0.31 in	L / 996	(11-12)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.88 (4-10)	Horz TL:	0.11 in		10	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	5.5 in	1.50 in	928 lbs	·	·	-67 lbs	-67 lbs	96 lbs
10	1	5.5 in	2.27 in	2,126 lbs	·	·	-223 lbs	-223 lbs	·
8	1	2.75 in	1.50 in	189 lbs	-308 lbs	-29 lbs	·	-308 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 14-13

Bracing

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-13, 4-10

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 14-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.459	313 lbs	(-2,819 lbs)	4-5	0.995	2,322 lbs					
	3-4	0.405		(-1,989 lbs)	5-6	0.995	2,322 lbs					
BC	8-9	0.170		(-1,019 lbs)	10-11	0.640	1,315 lbs		12-13	0.877	2,523 lbs	(-379 lbs)
	9-10	0.397		(-1,019 lbs)	11-12	0.920	2,932 lbs	(-368 lbs)				
Web	2-13	0.577		(-2,615 lbs)	4-10	0.880	461 lbs	(-3,720 lbs)				
	2-12	0.228	516 lbs		5-10	0.052		(-408 lbs)				
	3-11	0.473		(-1,133 lbs)	6-10	0.732		(-1,744 lbs)				
	4-11	0.413	936 lbs		6-8	0.482	1,090 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: L1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:42
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
35-3-12	0/12	29	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 8 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.
11) Parapet TL: 0.13 in, 2L/395 (14-1), Allowable 2L/120.

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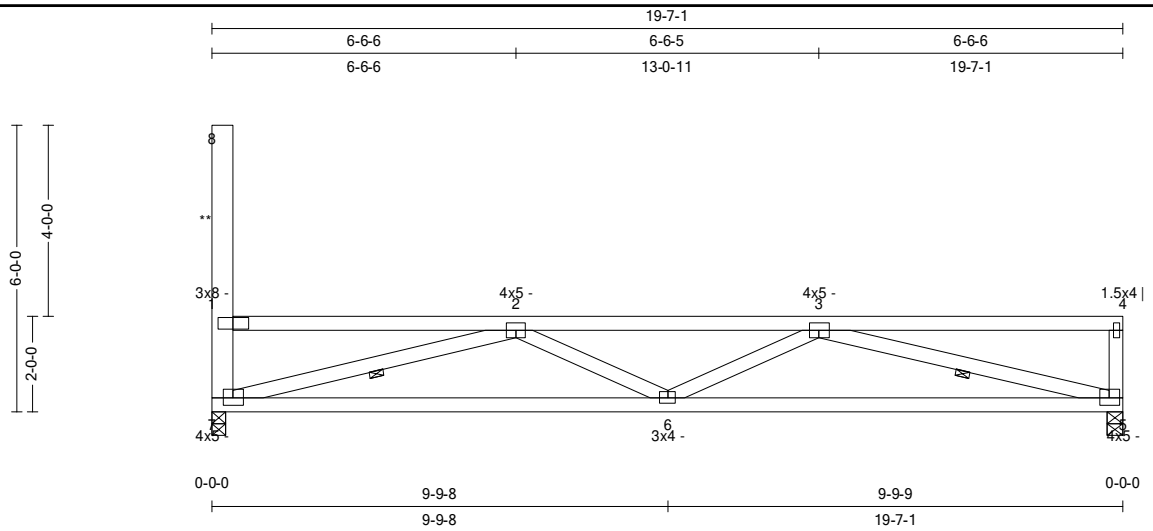
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: N1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:44
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-7-1	0/12	16	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	89 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.52 (3-4)	Vert TL:	0.58 in	L / 393	(5-6)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.86 (5-6)	Vert LL:	0.21 in	L / 999	(5-6)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.50 (3-5)	Horz TL:	0.07 in		5	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	819 lbs	·	·	-89 lbs	-89 lbs	197 lbs
5	1	4 in	1.50 in	827 lbs	·	·	-181 lbs	-181 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #1B 2 x 4
Web: DFL Standard 2 x 4 except:
DFL SS 2 x 6: 8-7

Bracing

TC: Sheathed or Purlins at 3-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-2-0, Purlin design by Others.
Web: One Midpoint Row: 2-7, 3-5

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) ** - Indicates parapet wind loading has been applied to member 8-1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.508	466 lbs		2-3	0.451	498 lbs	(-2,332 lbs)
BC	5-6	0.865	2,119 lbs	(-521 lbs)	6-7	0.864	2,104 lbs	(-671 lbs)
Web	2-7	0.483		(-2,180 lbs)	3-6	0.224	507 lbs	
	2-6	0.227	515 lbs		3-5	0.505	561 lbs	(-2,194 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: N1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:44

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-7-1	0/12	16	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	89 lbs

9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.18 in, 2L/560 (8-1), Allowable 2L/120.

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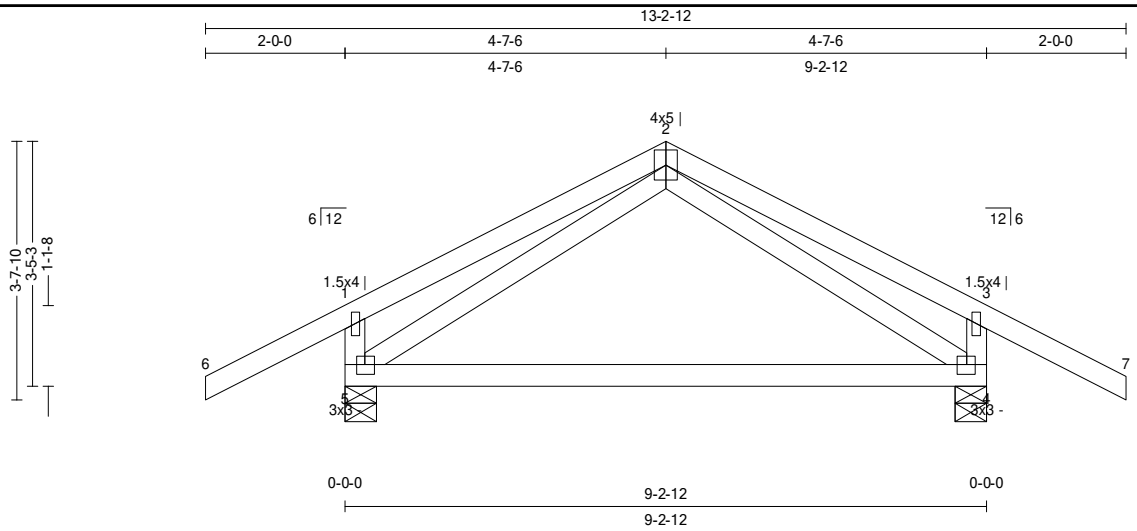
	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: P1 Job: 2501272 Designer:Anthony Date: 10/28/25 13:52:45 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
9-2-12	6/12	14	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	46 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	**General**		**CSI**		**Deflection**		**L/**	**(loc)**	**Allowed**
TCLL : 25	Bldg Code :	IRC 2021/	TC :	0.47 (6-1)	Vert TL:	0.54 in	L / 184	(4-5)	L / 180
TCDL : 7		TPI 1-2014	BC :	0.50 (4-5)	Vert LL:	0.27 in	L / 368	(4-5)	L / 240
BCLL : 0	Rep Mbr :	Yes	Web :	0.11 (2-4)	Horz TL:	0 in		4	
BCDL : 10	Lumber D.O.L. :	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	5.5 in	1.50 in	545 lbs	·	·	-184 lbs	-184 lbs	-20 lbs
4	1	5.5 in	1.50 in	545 lbs	·	·	-184 lbs	-184 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL 2400/2.0 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC											
BC											
Web	1-5	0.091	(-330 lbs)	3-4	0.091	(-330 lbs)					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

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Mustang Truss
2525 Hyacinth Street NE
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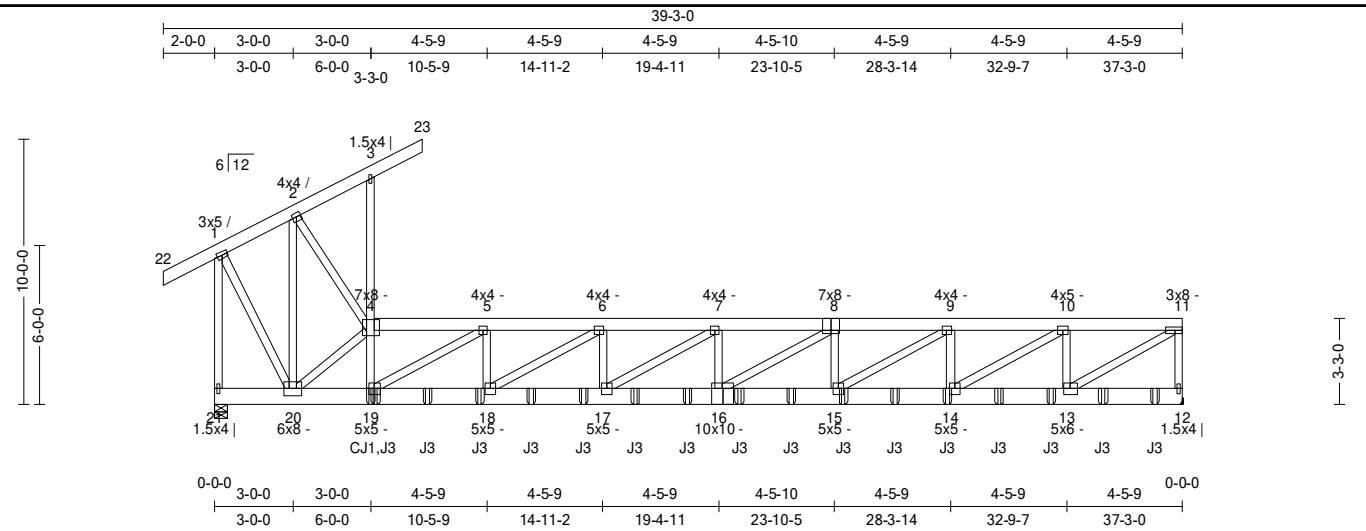
Truss: Q01
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:33
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
37-3-0	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	295 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IRC 2021/	TC:	0.27 (7-8)	Vert TL:	0.64 in	L/687	(16-17)	L/180
TCLL: 25		TPI 1-2014	BC:	0.41 (16-17)	Vert LL:	0.26 in	L/999	(16-17)	L/240
TCDL: 7	Rep Mbr:	No	Web:	0.76 (10-14)	Horz TL:	0.09 in		12	
BCLL: 0	Lumber D.O.L.:	115 %							
BCDL: 10									

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
21	1	5.5 in	1.50 in	2,730 lbs	·	-533 lbs	·	-533 lbs	197 lbs
12	1	1.5 in	---	2,846 lbs	·	-763 lbs	·	-763 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 8
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 11-13

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	4-0-12	Down	Proj	25.78 plf	25.78 plf	
Top	4-0-12	5-2-4	Down	Proj	25.78 plf	11.72 plf	
Top	-2-0-0	37-3-0	Down	Proj	24.22 plf	24.22 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	4-0-12	Down	Proj	7.22 plf	7.22 plf	
Top	4-0-12	5-2-4	Down	Proj	7.22 plf	3.28 plf	
Top	-2-0-0	37-3-0	Down	Proj	6.78 plf	6.78 plf	
Bot	0-0-0	4-0-12	Down	Proj	10.31 plf	10.31 plf	
Bot	4-0-12	5-2-4	Down	Proj	10.31 plf	4.69 plf	
Bot	0-0-0	37-3-0	Down	Proj	9.69 plf	9.69 plf	

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
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Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q01
Job: 2501272
Designer:Anthony
Date: 10/28/25 13:53:33
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
37-3-0	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	295 lbs

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.059		(-565 lbs)	6-7	0.263	1,208 lbs	(-4,555 lbs)	9-10	0.200	919 lbs	(-3,461 lbs)
	4-5	0.145	683 lbs	(-2,487 lbs)	7-8	0.273	1,256 lbs	(-4,743 lbs)	10-11	0.117	530 lbs	(-2,000 lbs)
	5-6	0.220	1,019 lbs	(-3,807 lbs)	8-9	0.253	1,161 lbs	(-4,378 lbs)				
BC	13-14	0.197	2,000 lbs	(-542 lbs)	16-17	0.410	4,741 lbs	(-1,266 lbs)	19-20	0.227	2,478 lbs	(-692 lbs)
	14-15	0.316	3,461 lbs	(-931 lbs)	17-18	0.387	4,555 lbs	(-1,220 lbs)				
	15-16	0.379	4,393 lbs	(-1,177 lbs)	18-19	0.337	3,807 lbs	(-1,031 lbs)				
Web	1-21	0.219		(-1,300 lbs)	5-19	0.248	394 lbs	(-1,546 lbs)	9-14	0.077		(-624 lbs)
	1-20	0.463	1,048 lbs		5-18	0.328	742 lbs		10-14	0.755	1,710 lbs	(-456 lbs)
	2-20	0.369	834 lbs		6-18	0.141		(-876 lbs)	10-13	0.119		(-964 lbs)
	2-4	0.136		(-824 lbs)	6-17	0.171	387 lbs		11-13	0.454	2,363 lbs	(-633 lbs)
	4-20	0.364	730 lbs	(-2,695 lbs)	8-16	0.185	418 lbs		11-12	0.160	342 lbs	(-1,295 lbs)
	4-19	0.598	1,033 lbs		9-15	0.474	1,073 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
CJ1	BC	6-0-0
J3	BC	6-2-4
J3	BC	8-2-4
J3	BC	10-2-4
J3	BC	12-2-4
J3	BC	14-2-4
J3	BC	16-2-4
J3	BC	18-2-4
J3	BC	20-2-4
J3	BC	22-2-4
J3	BC	24-2-4
J3	BC	26-2-4
J3	BC	28-2-4
J3	BC	30-2-4
J3	BC	32-2-4
J3	BC	34-2-4
J3	BC	36-2-4

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc , Webs - 1 row @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental Head Side - FastenMaster FlatLOK (2 - Ply) Screws within 12" along the chord or into converging webs at point load:
BC: 6-0-0,(1)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

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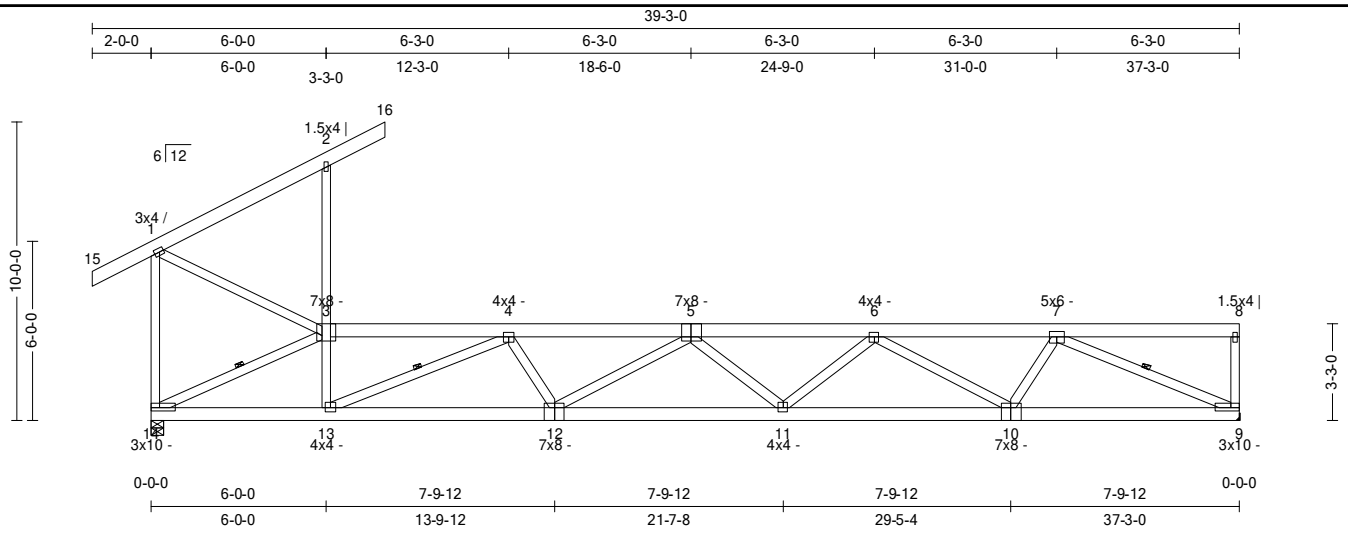
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q02
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:47
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
37-3-0	6/12	7	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	246 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.27 (5-6)	Vert TL:	0.8 in	L / 550	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.47 (11-12)	Vert LL:	0.33 in	L / 999	(11-12)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.78 (6-10)	Horz TL:	0.16 in		9	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.92 in	1,801 lbs	·	·	-338 lbs	-338 lbs	243 lbs
9	1	1.5 in	---	1,584 lbs	·	·	-225 lbs	-225 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-13

Bracing

TC: Sheathed or Purlins at 3-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-4-0, Purlin design by Others.
Web: One Midpoint Row: 3-14, 4-13, 7-9

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.172	1,122 lbs	(-2,967 lbs)	5-6	0.272	895 lbs	(-5,055 lbs)				
	4-5	0.265	1,119 lbs	(-4,928 lbs)	6-7	0.184	527 lbs	(-3,420 lbs)				
BC	9-10	0.292	2,913 lbs	(-429 lbs)	11-12	0.469	5,271 lbs	(-1,006 lbs)	13-14	0.264	2,956 lbs	(-1,061 lbs)
	10-11	0.417	4,666 lbs	(-773 lbs)	12-13	0.429	4,727 lbs	(-1,133 lbs)				
Web	1-14	0.319		(-333 lbs)	4-13	0.458		(-1,927 lbs)	6-11	0.285	646 lbs	
	3-14	0.698	964 lbs	(-3,275 lbs)	4-12	0.246	556 lbs		6-10	0.781	352 lbs	(-1,451 lbs)
	2-3	0.173		(-310 lbs)	5-12	0.345		(-642 lbs)	7-10	0.458	1,038 lbs	
	3-13	0.257	926 lbs		5-11	0.131		(-414 lbs)	7-9	0.738	503 lbs	(-3,203 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
8) A creep factor of 2.00 has been applied for this truss analysis.

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Eagle Metal Products

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Truss: Q02
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:47
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
37-3-0	6/12	7	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	246 lbs

9) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

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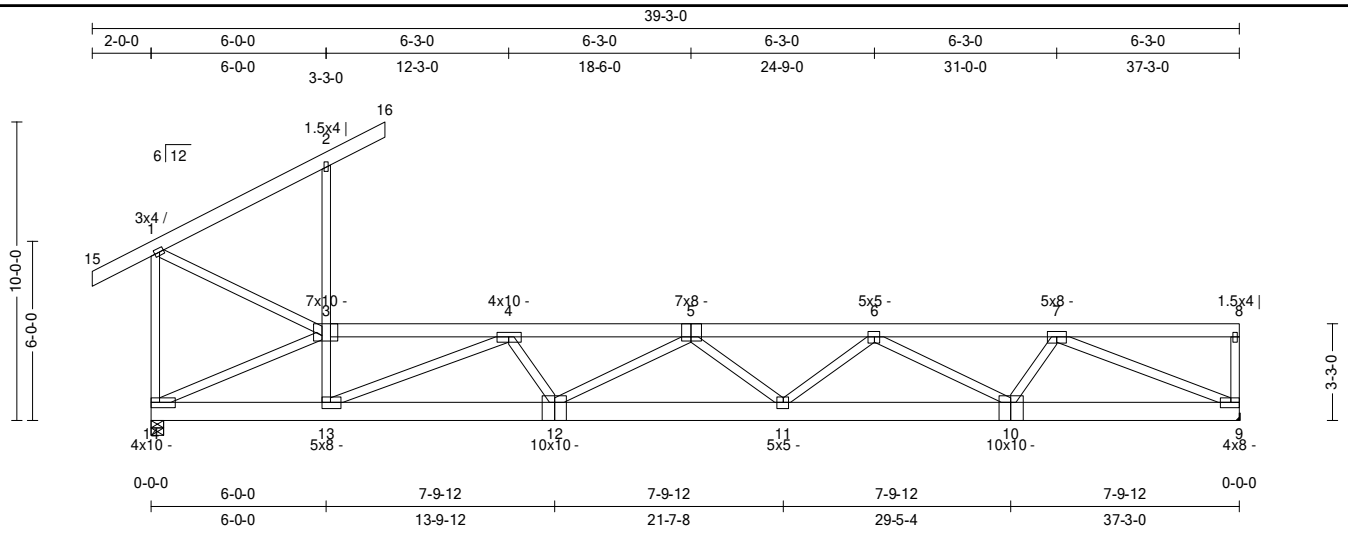
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q02A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:50
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
37-3-0	6/12	6	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	273 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.39 (5-6)	Vert TL:	1.3 in	L / 339	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.59 (11-12)	Vert LL:	0.16 in	L / 999	(11-12)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.91 (7-9)	Horz TL:	0.22 in		9	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	2.12 in	3,974 lbs	·	·	-339 lbs	-339 lbs	243 lbs
9	1	1.5 in	---	3,544 lbs	·	·	-225 lbs	-225 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 8
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 5-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

User-defined Load Case D2: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	37-3-0	Down	Proj	7 psf	7 psf	24 in
Bot	0-0-0	37-3-0	Down	Proj	10 psf	10 psf	24 in
Top	7-8-0	28-0-0	Down	Proj	100 psf	100 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000
7	D1 + S3	1.000
8	D2 + S1	1.000

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q02A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:50
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
37-3-0	6/12	6	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	273 lbs

9	D2 + S2	1.000
10	D2 + S3	1.000
11	0.60 D1 + 0.60 W1 [Uplift]	1.000
12	0.60 D1 + 0.60 W2 [Uplift]	1.000
13	0.60 D1 + 0.60 W4 [Uplift]	1.000
14	0.60 D1 + 0.60 W8 [Uplift]	1.000
15	0.60 D2 + 0.60 W1 [Uplift]	1.000
16	0.60 D2 + 0.60 W2 [Uplift]	1.000
17	0.60 D2 + 0.60 W4 [Uplift]	1.000
18	0.60 D2 + 0.60 W8 [Uplift]	1.000
19	D1 + L10*1	1.000
20	D2 + L10*1	1.000
21	D1 + I1	1.000
22	D2 + I1	1.000
23	D1 + UR1	1.000
24	D1 + UR2	1.000
25	D2 + UR1	1.000
26	D2 + UR2	1.000
27	D1 + AS10*1	1.000
28	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.293	565 lbs	(-3,841 lbs)	5-6	0.391	455 lbs	(-7,268 lbs)				
	4-5	0.382	570 lbs	(-7,099 lbs)	6-7	0.239		(-4,434 lbs)				
BC	9-10	0.281	3,650 lbs		11-12	0.591	7,773 lbs	(-513 lbs)	13-14	0.277	3,828 lbs	(-535 lbs)
	10-11	0.498	6,674 lbs	(-394 lbs)	12-13	0.518	6,842 lbs	(-577 lbs)				
Web	3-14	0.888	485 lbs	(-4,221 lbs)	5-12	0.158		(-900 lbs)	7-10	0.739	1,574 lbs	
	3-13	0.842	1,370 lbs		5-11	0.103		(-732 lbs)	7-9	0.909		(-3,995 lbs)
	4-13	0.769		(-3,273 lbs)	6-11	0.376	850 lbs					
	4-12	0.260	588 lbs		6-10	0.455		(-2,589 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc , Webs - 1 row @ 2-0-0 oc.
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

Mustang Truss
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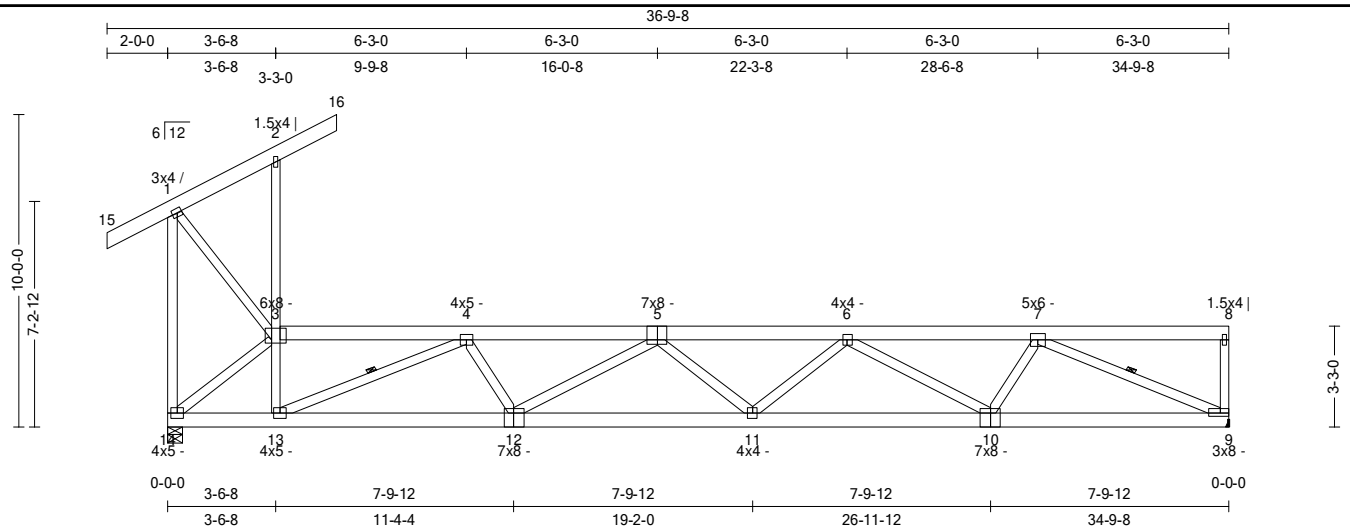
Truss: Q03A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:52
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
34-9-8	6/12	5	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	233 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.24 (5-6)	Vert TL:	0.61 in	L / 673	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.41 (11-12)	Vert LL:	0.25 in	L / 999	(11-12)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.71 (3-14)	Horz TL:	0.13 in		9	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.82 in	1,705 lbs	·	·	-358 lbs	-358 lbs	238 lbs
9	1	1.5 in	---	1,473 lbs	·	·	-220 lbs	-220 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-13

Bracing

TC: Sheathed or Purlins at 4-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.
Web: One Midpoint Row: 4-13, 7-9

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.148	950 lbs	(-1,746 lbs)	5-6	0.239	832 lbs	(-4,446 lbs)				
	4-5	0.216	1,004 lbs	(-4,008 lbs)	6-7	0.168	502 lbs	(-3,119 lbs)				
BC	9-10	0.273	2,674 lbs	(-410 lbs)	11-12	0.411	4,536 lbs	(-923 lbs)	13-14	0.169	1,741 lbs	(-888 lbs)
	10-11	0.377	4,181 lbs	(-726 lbs)	12-13	0.344	3,747 lbs	(-1,007 lbs)				
Web	1-3	0.113	356 lbs		4-12	0.272	616 lbs		6-10	0.665	328 lbs	(-1,236 lbs)
	3-14	0.707	926 lbs	(-2,253 lbs)	5-12	0.403		(-749 lbs)	7-10	0.403	911 lbs	
	3-13	0.234	1,030 lbs		5-11	0.096		(-304 lbs)	7-9	0.677	483 lbs	(-2,940 lbs)
	4-13	0.521		(-2,192 lbs)	6-11	0.237	537 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
8) A creep factor of 2.00 has been applied for this truss analysis.

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Eagle Metal Products

Mustang Truss
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Truss: Q03A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:52
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
34-9-8	6/12	5	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	233 lbs

9) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
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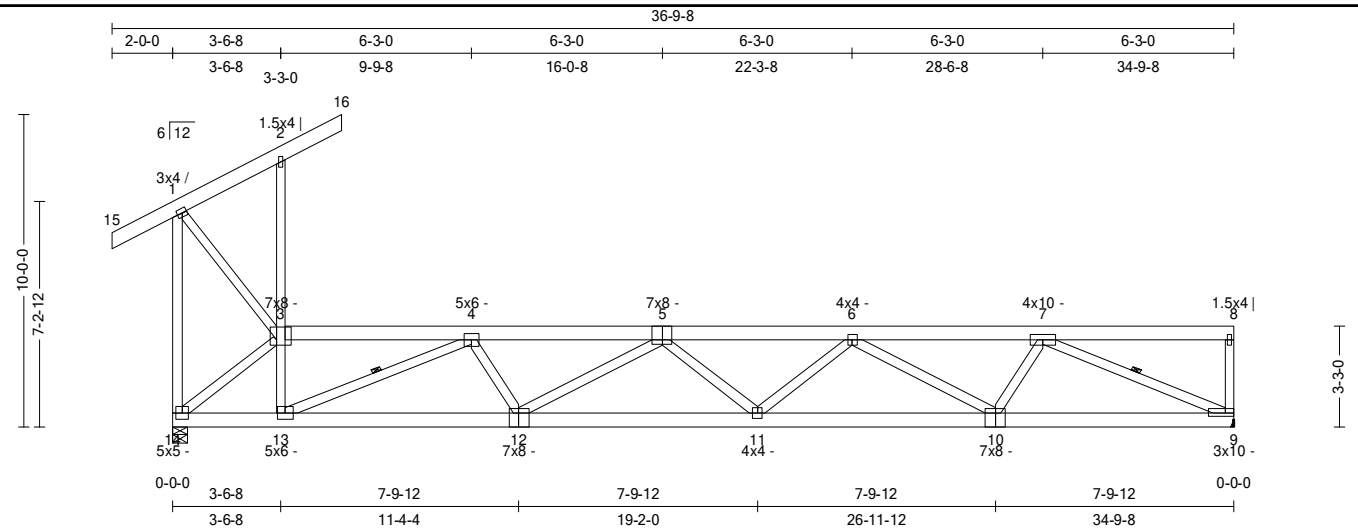
Truss: Q03B
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:54
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
34-9-8	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	234 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.36 (4-5)	Vert TL:	0.84 in	L / 487	(11-12)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.52 (11-12)	Vert LL:	0.25 in	L / 999	(11-12)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.91 (3-14)	Horz TL:	0.18 in		9	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	2.26 in	2,116 lbs	·	·	-358 lbs	-358 lbs	238 lbs
9	1	1.5 in	---	1,662 lbs	·	·	-220 lbs	-220 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-13

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.
Web: One Midpoint Row: 4-13, 7-9

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

User-defined Load Case D2: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	34-9-8	Down	Proj	7 psf	7 psf	24 in
Bot	0-0-0	34-9-8	Down	Proj	10 psf	10 psf	24 in
Top	8-0-0	14-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000

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Eagle Metal Products

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Truss: Q03B
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:55
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
34-9-8	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	234 lbs

7	D1 + S3	1.000
8	D2 + S1	1.000
9	D2 + S2	1.000
10	D2 + S3	1.000
11	0.60 D1 + 0.60 W1 [Uplift]	1.000
12	0.60 D1 + 0.60 W2 [Uplift]	1.000
13	0.60 D1 + 0.60 W4 [Uplift]	1.000
14	0.60 D1 + 0.60 W8 [Uplift]	1.000
15	0.60 D2 + 0.60 W1 [Uplift]	1.000
16	0.60 D2 + 0.60 W2 [Uplift]	1.000
17	0.60 D2 + 0.60 W4 [Uplift]	1.000
18	0.60 D2 + 0.60 W8 [Uplift]	1.000
19	D1 + L10*1	1.000
20	D2 + L10*1	1.000
21	D1 + I1	1.000
22	D2 + I1	1.000
23	D1 + UR1	1.000
24	D1 + UR2	1.000
25	D2 + UR1	1.000
26	D2 + UR2	1.000
27	D1 + AS10*1	1.000
28	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.183	950 lbs	(-2,249 lbs)	5-6	0.296	832 lbs	(-5,501 lbs)				
	4-5	0.357	1,004 lbs	(-5,385 lbs)	6-7	0.195	502 lbs	(-3,629 lbs)				
BC	9-10	0.306	3,083 lbs	(-410 lbs)	11-12	0.523	5,826 lbs	(-923 lbs)	13-14	0.210	2,242 lbs	(-888 lbs)
	10-11	0.443	4,998 lbs	(-726 lbs)	12-13	0.460	5,160 lbs	(-1,007 lbs)				
Web	1-3	0.113	356 lbs		4-12	0.272	616 lbs		6-10	0.857	328 lbs	(-1,592 lbs)
	3-14	0.911	926 lbs	(-2,902 lbs)	5-12	0.403		(-749 lbs)	7-10	0.494	1,119 lbs	
	3-13	0.325	1,445 lbs		5-11	0.195		(-618 lbs)	7-9	0.781	483 lbs	(-3,390 lbs)
	4-13	0.758		(-3,188 lbs)	6-11	0.378	857 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
8) A creep factor of 2.00 has been applied for this truss analysis.
9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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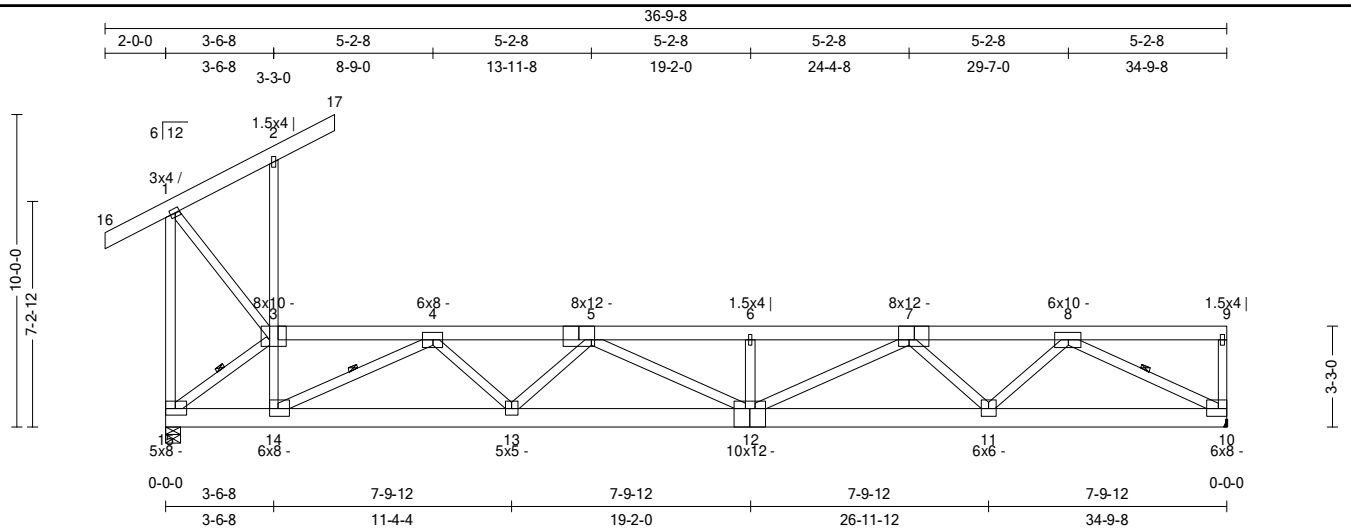
Truss: Q03C
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:57
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
34-9-8	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	263 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.56 (5-6)	Vert TL:	1.41 in	L / 290	(12-13)	L / 180
TCDL:	7			BC:	0.73 (12-13)	Vert LL:	0.23 in	L / 999	(12-13)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.96 (8-10)	Horz TL:	0.25 in		10	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	3.44 in	3,225 lbs	·	·	-358 lbs	-358 lbs	238 lbs
10	1	1.5 in	---	2,754 lbs	·	·	-219 lbs	-219 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 8
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-14, 8-11

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-11-0, Purlin design by Others.
Web: One Midpoint Row: 3-15, 4-14, 8-10

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

User-defined Load Case D2: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	34-9-8	Down	Proj	25 psf	25 psf	24 in
Top	6-0-0	27-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000
7	D1 + S3	1.000
8	D2 + S1	1.000

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q03C
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:57
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
34-9-8	6/12	3	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	263 lbs

9	D2 + S2	1.000
10	D2 + S3	1.000
11	0.60 D1 + 0.60 W1 [Uplift]	1.000
12	0.60 D1 + 0.60 W2 [Uplift]	1.000
13	0.60 D1 + 0.60 W4 [Uplift]	1.000
14	0.60 D1 + 0.60 W8 [Uplift]	1.000
15	0.60 D2 + 0.60 W1 [Uplift]	1.000
16	0.60 D2 + 0.60 W2 [Uplift]	1.000
17	0.60 D2 + 0.60 W4 [Uplift]	1.000
18	0.60 D2 + 0.60 W8 [Uplift]	1.000
19	D1 + L10*1	1.000
20	D2 + L10*1	1.000
21	D1 + I1	1.000
22	D2 + I1	1.000
23	D1 + UR1	1.000
24	D1 + UR2	1.000
25	D2 + UR1	1.000
26	D2 + UR2	1.000
27	D1 + AS10*1	1.000
28	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.269	957 lbs	(-3,500 lbs)	6-7	0.533	878 lbs	(-10,044 lbs)								
	4-5	0.462	1,016 lbs	(-8,777 lbs)	7-8	0.342	507 lbs	(-6,482 lbs)								
	5-6	0.562	877 lbs	(-10,036 lbs)												
BC	10-11	0.366	4,622 lbs	(-357 lbs)	12-13	0.733	9,758 lbs	(-976 lbs)	14-15	0.250	3,490 lbs	(-895 lbs)				
	11-12	0.641	8,232 lbs	(-630 lbs)	13-14	0.585	7,631 lbs	(-1,025 lbs)								
Web	1-15	0.546		(-402 lbs)	3-14	0.486	2,155 lbs		5-12	0.309	700 lbs		8-11	0.516	2,689 lbs	
	1-3	0.114	358 lbs		4-14	0.879		(-4,659 lbs)	6-12	0.173		(-1,139 lbs)	8-10	0.962	438 lbs	(-5,246 lbs)
	3-15	0.626	925 lbs	(-4,471 lbs)	4-13	0.732	1,657 lbs		7-12	0.905	2,048 lbs					
	2-3	0.200		(-359 lbs)	5-13	0.395		(-1,564 lbs)	7-11	0.663		(-2,622 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
8) A creep factor of 2.00 has been applied for this truss analysis.
9) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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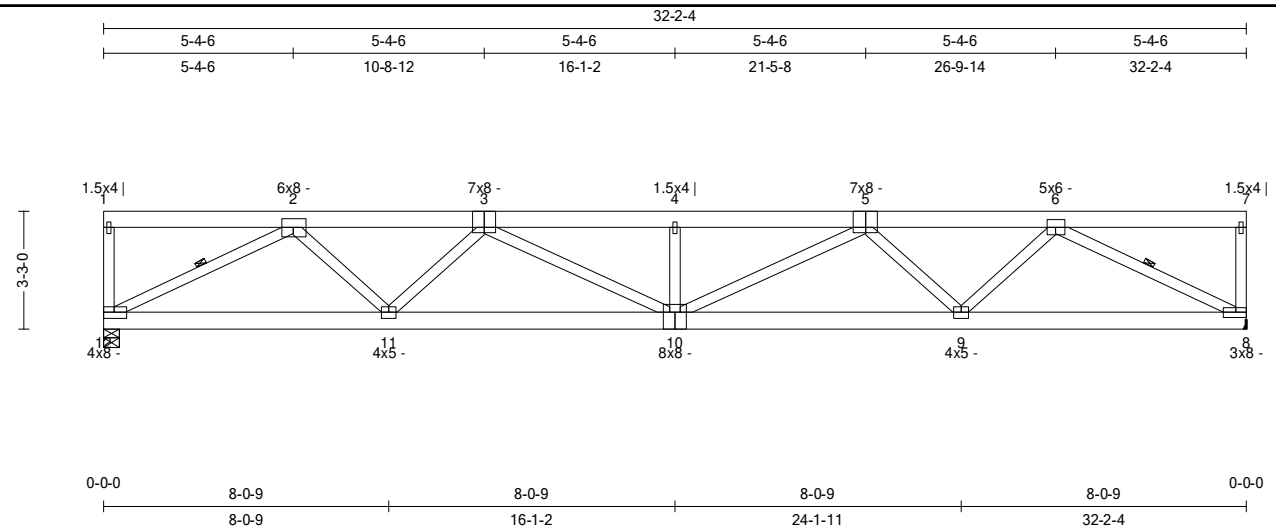
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q04A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:59
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-2-4	0/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	189 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/ TPI 1-2014	TC:	0.31 (3-4)	Vert TL:	0.75 in	L / 509	(10-11)	L / 180
TCDL:	7			BC:	0.51 (10-11)	Vert LL:	0.18 in	L / 999	10	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.80 (2-12)	Horz TL:	0.16 in		8	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	5.5 in	2.37 in	2,219 lbs	·	·	-147 lbs	-147 lbs	-52 lbs
8	1	1.5 in	---	1,685 lbs	·	·	-147 lbs	-147 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-12, 6-8

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

User-defined Load Case D2: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	32-2-4	Down	Proj	7 psf	7 psf	24 in
Bot	0-0-0	32-2-4	Down	Proj	10 psf	10 psf	24 in
Top	3-0-0	15-0-0	Down	Proj	50 psf	50 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D2 + S1	1.000
7	0.60 D1 + 0.60 W1 [Uplift]	1.000
8	0.60 D1 + 0.60 W2 [Uplift]	1.000

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q04A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:52:59
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-2-4	0/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	189 lbs

9	0.60 D1 + 0.60 W4 [Uplift]	1.000
10	0.60 D1 + 0.60 W8 [Uplift]	1.000
11	0.60 D2 + 0.60 W1 [Uplift]	1.000
12	0.60 D2 + 0.60 W2 [Uplift]	1.000
13	0.60 D2 + 0.60 W4 [Uplift]	1.000
14	0.60 D2 + 0.60 W8 [Uplift]	1.000
15	D1 + L10*1	1.000
16	D2 + L10*1	1.000
17	D1 + UR1	1.000
18	D1 + UR2	1.000
19	D2 + UR1	1.000
20	D2 + UR2	1.000
21	D1 + AS10*1	1.000
22	D2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.252		(-4,753 lbs)	4-5	0.307	428 lbs	(-5,778 lbs)						
	3-4	0.307	428 lbs	(-5,778 lbs)	5-6	0.200		(-3,770 lbs)						
BC	8-9	0.277	2,716 lbs		9-10	0.421	4,656 lbs	(-349 lbs)	10-11	0.510	5,689 lbs	(-349 lbs)	11-12	0.359	3,659 lbs
Web	2-12	0.795		(-4,150 lbs)	3-10	0.302	684 lbs	(-347 lbs)	5-9	0.347		(-1,279 lbs)			
	2-11	0.697	1,578 lbs		4-10	0.076		(-478 lbs)	6-9	0.672	1,521 lbs				
	3-11	0.366		(-1,351 lbs)	5-10	0.633	1,433 lbs		6-8	0.590		(-3,080 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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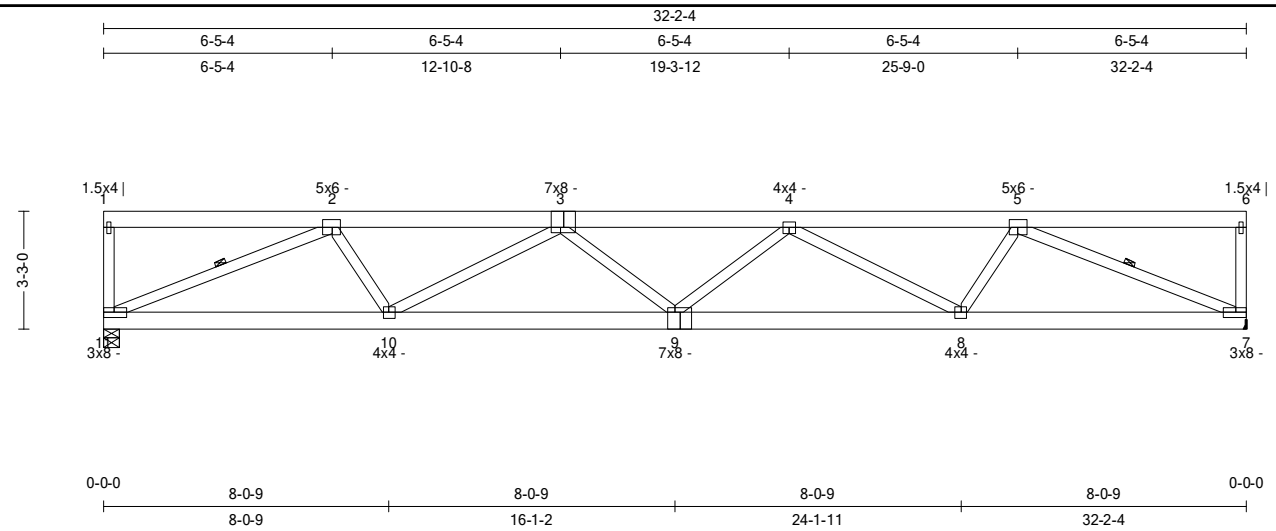
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q04B
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:01
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-2-4	0/12	6	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	185 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.20 (3-4)	Vert TL:	0.47 in	L / 808	(9-10)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.34 (9-10)	Vert LL:	0.2 in	L / 999	9	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.65 (2-11)	Horz TL:	0.11 in		7	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	1,352 lbs	·	·	-147 lbs	-147 lbs	-52 lbs
7	1	1.5 in	---	1,352 lbs	·	·	-147 lbs	-147 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 4-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-11, 5-7

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 25 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.154		(-2,850 lbs)	4-5	0.154		(-2,848 lbs)						
	3-4	0.204	397 lbs	(-3,781 lbs)										
BC	7-8	0.261	2,471 lbs		8-9	0.338	3,689 lbs	(-388 lbs)	9-10	0.343	3,696 lbs	(-391 lbs)	10-11	0.260	2,474 lbs
Web	2-11	0.649	310 lbs	(-2,707 lbs)	3-9	0.170	385 lbs		5-8	0.333	755 lbs			
	2-10	0.333	753 lbs		4-9	0.169	383 lbs		5-7	0.648	310 lbs	(-2,704 lbs)		
	3-10	0.547		(-973 lbs)	4-8	0.547		(-972 lbs)						

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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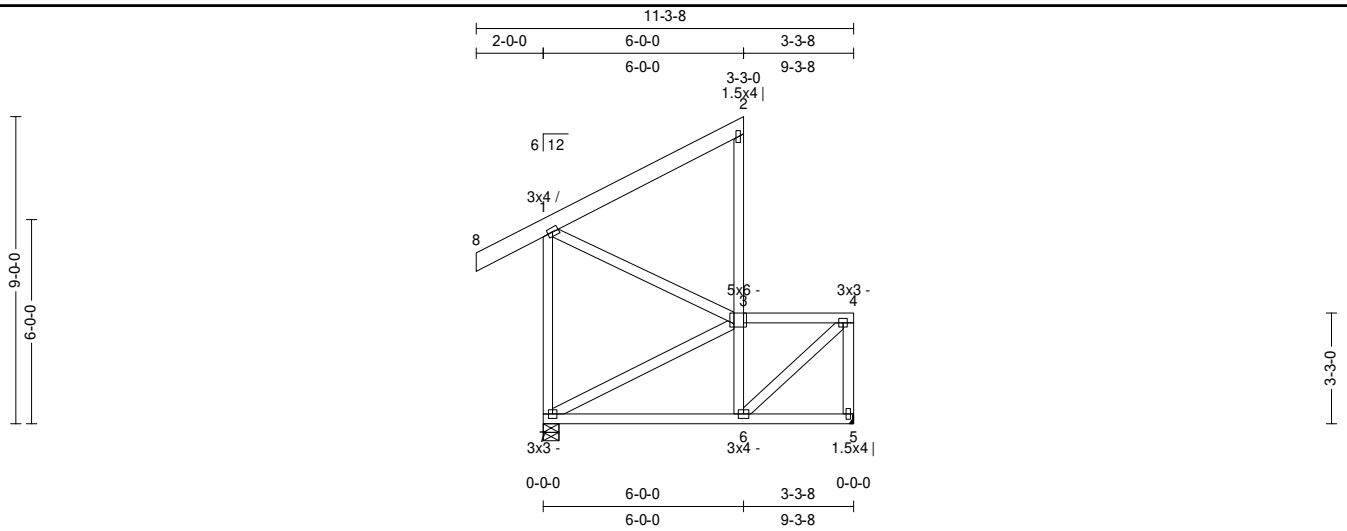
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q05
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:02
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
9-3-8	6/12	34	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	76 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.23 (3-4)	Vert TL:	0.08 in	L / 999	(6-7)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.29 (6-7)	Vert LL:	0.03 in	L / 999	(6-7)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.49 (1-7)	Horz TL:	0 in		5	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
240360831
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	5.5 in	1.50 in	566 lbs	·	·	-132 lbs	-132 lbs	156 lbs
5	1	1.5 in	---	429 lbs	·	-40 lbs	-175 lbs	-175 lbs	·

Material

TC: DFL #2 2 x 4 except:
DFL SS 2 x 6: 8-2
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 2-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.231		(-302 lbs)			
BC	6-7	0.291	301 lbs				
Web	1-7	0.489		(-417 lbs)	4-6	0.180	407 lbs
	3-7	0.239		(-339 lbs)	4-5	0.186	(-409 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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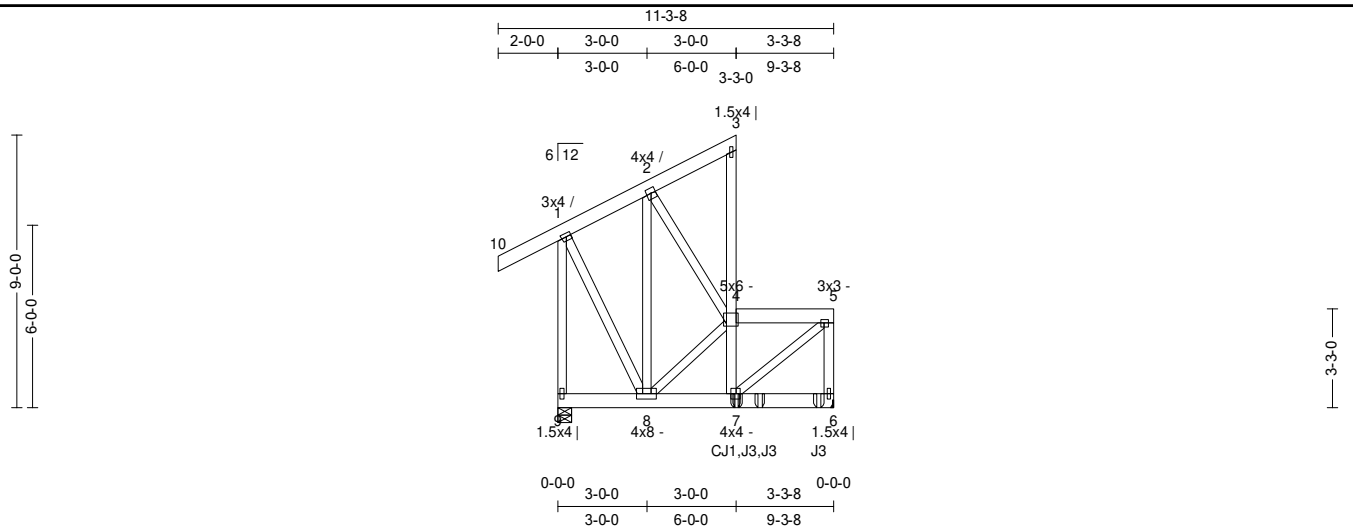
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q05A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:35
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
9-3-8	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	96 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)		Bldg Code :	IRC 2021/ TPI 1-2014	TC :	0.08 (10-1)	Vert TL:	0.01 in	L / 999	(6-7)	L / 180
TCLL :	25	Rep Mbr :	No	BC :	0.07 (6-7)	Vert LL:	0.01 in UP	L / 999	(6-7)	L / 240
TCDL :	7	Lumber D.O.L. :	115 %	Web :	0.24 (1-9)	Horz TL:	0 in		6	
BCLL :	0									
BCDL :	10									

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	654 lbs	·	·	-85 lbs	-85 lbs	153 lbs
6	1	1.5 in	---	777 lbs	·	-285 lbs	·	-285 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 3-7

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	9-3-8	Down	Proj	24.22 plf	24.22 plf	
Top	-2-0-0	4-0-12	Down	Proj	25.78 plf	25.78 plf	
Top	4-0-12	5-0-12	Down	Proj	25.78 plf	13.28 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-2-0-0	9-3-8	Down	Proj	6.78 plf	6.78 plf	
Top	-2-0-0	4-0-12	Down	Proj	7.22 plf	7.22 plf	
Top	4-0-12	5-0-12	Down	Proj	7.22 plf	3.72 plf	
Bot	0-0-0	9-3-8	Down	Proj	9.69 plf	9.69 plf	
Bot	0-0-0	4-0-12	Down	Proj	10.31 plf	10.31 plf	
Bot	4-0-12	5-0-12	Down	Proj	10.31 plf	5.31 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC

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Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Q05A
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:35
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
9-3-8	6/12	1	2-0-0	0-0-0	0-0-0	0-0-0	2	24 in	96 lbs

BC				
Web	1-9	0.238		(-307 lbs)
	5-7	0.154	349 lbs	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
CJ1	BC	6-0-0
J3	BC	6-0-12
J3	BC	6-9-12
J3	BC	8-9-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (2 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc , Webs - 1 row @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental Head Side - FastenMaster FlatLOK (2 - Ply) Screws within 12" along the chord or into converging webs at point load:
BC: 6-0-0,(1)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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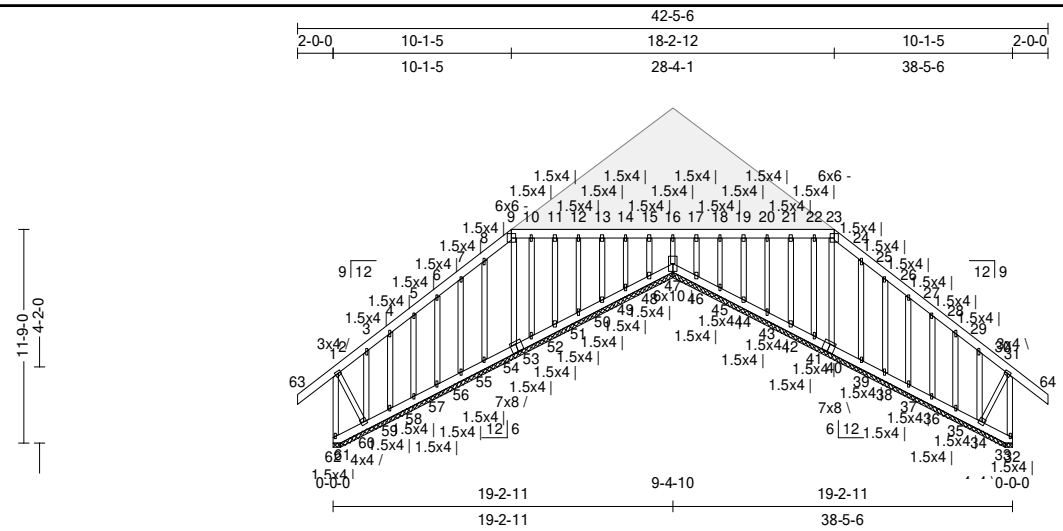
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: R1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:06
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
38-5-6	9/12	1	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	357 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.16 (63-1)	Vert TL:	0 in UP	L / 999	32	L / 180
TCDL:	7		TPI 1-2014	BC:	0.01 (60-62)	Vert LL:	0 in	L / 999	32	L / 240
BCLL:	0	Rep Mbr:	No	Web:	0.15 (1-62)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
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Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	226 lbs	98 plf	-	-74 lbs	-111 lbs	-111 lbs	98 lbs

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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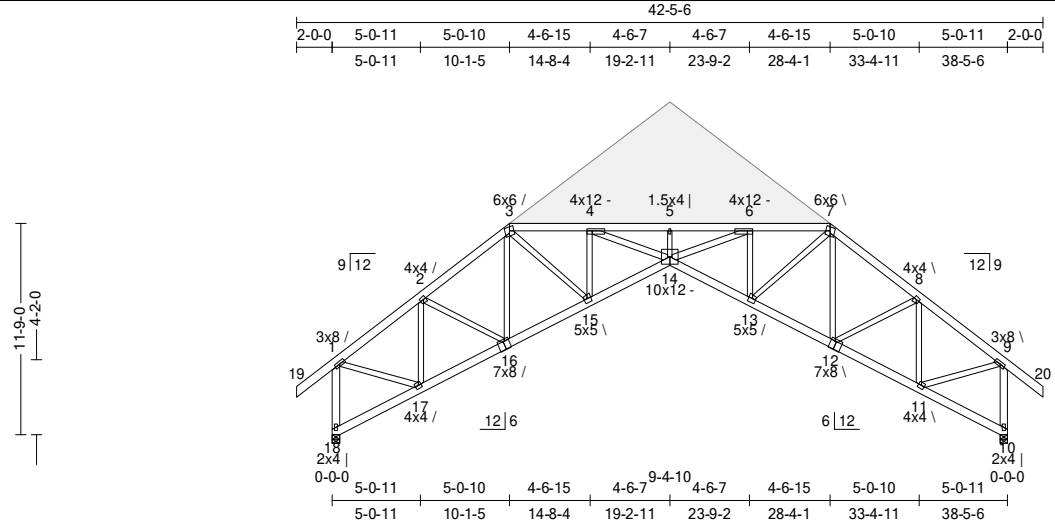
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: R2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:08
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
38-5-6	9/12	17	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	297 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.40 (5-6)	Vert TL:	1.07 in	L / 422	14	L / 180
TCDL:	7		TPI 1-2014	BC:	0.58 (13-14)	Vert LL:	0.45 in	L / 994	14	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.98 (7-13)	Horz TL:	1.16 in		10	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

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Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	5.5 in	1.59 in	1,743 lbs	·	·	-124 lbs	-124 lbs	-101 lbs
10	1	5.5 in	1.59 in	1,743 lbs	·	·	-124 lbs	-124 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL SS 2 x 6
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 4-14, 6-14
DFL SS 2 x 6: 1-18, 9-10

Bracing

TC: Sheathed or Purlins at 2-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.107		(-1,735 lbs)	4-5	0.398		(-7,524 lbs)	7-8	0.125		(-2,350 lbs)		
	2-3	0.125		(-2,350 lbs)	5-6	0.398		(-7,524 lbs)	8-9	0.107		(-1,735 lbs)		
	3-4	0.182		(-3,463 lbs)	6-7	0.182		(-3,463 lbs)						
BC	11-12	0.127	1,481 lbs		13-14	0.578	3,833 lbs		15-16	0.230	2,082 lbs			
	12-13	0.230	2,082 lbs		14-15	0.578	3,833 lbs		16-17	0.127	1,481 lbs			
Web	1-18	0.199		(-1,697 lbs)	3-15	0.979	2,217 lbs		7-13	0.979	2,217 lbs		9-10	0.199 (-1,697 lbs)
	1-17	0.605	1,369 lbs		4-15	0.590		(-2,069 lbs)	7-12	0.313		(-479 lbs)		
	2-17	0.399		(-953 lbs)	4-14	0.844	4,396 lbs		8-12	0.255	576 lbs			
	2-16	0.255	576 lbs		6-14	0.844	4,396 lbs		8-11	0.399		(-953 lbs)		
	3-16	0.313		(-479 lbs)	6-13	0.590		(-2,069 lbs)	9-11	0.605	1,369 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: R2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:08
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
38-5-6	9/12	17	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	297 lbs

5) Listed wind uplift reactions based on MWFRS & C&C loading.

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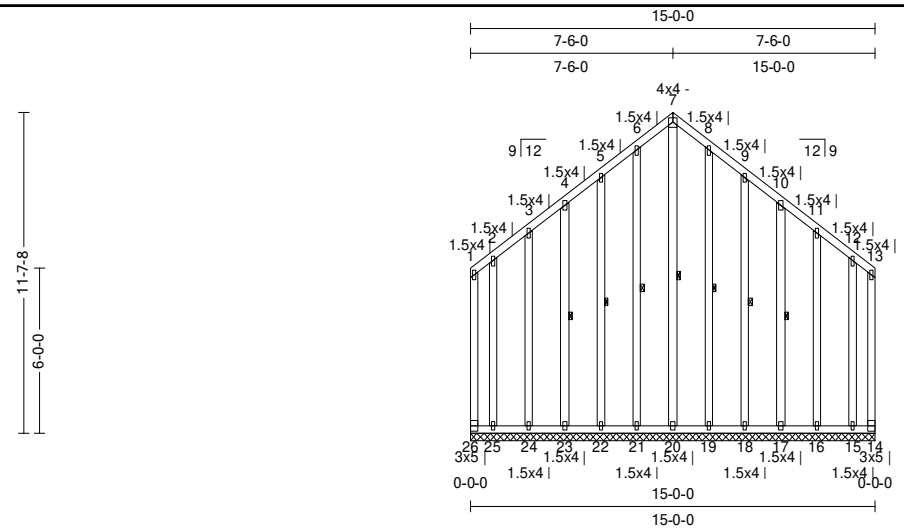
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: T1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:10
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
15-0-0	9/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	175 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 25	Bldg Code:	IRC 2021/	TC:	0.44 (1-2)	Vert TL:	0 in UP	L / 999	14	L / 180
TCDL: 7		TPI 1-2014	BC:	0.01 (23-24)	Vert LL:	0 in	L / 999	14	L / 240
BCLL: 0	Rep Mbr:	No	Web:	0.47 (13-14)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	286 lbs	163 plf	-	-288 lbs	-155 lbs	-288 lbs	-66 lbs

10/28/2025

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL #2 2 x 4 except:
DFL Standard 2 x 4: 2-25, 3-24, 11-16, 12-15

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-23, 5-22, 6-21, 7-20, 8-19, 9-18, 10-17

Loads

1) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	
BC	
Web	

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) Gable must be sheathed on one side or lateral bracing applied appropriately.
8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
9) A creep factor of 2.00 has been applied for this truss analysis.
10) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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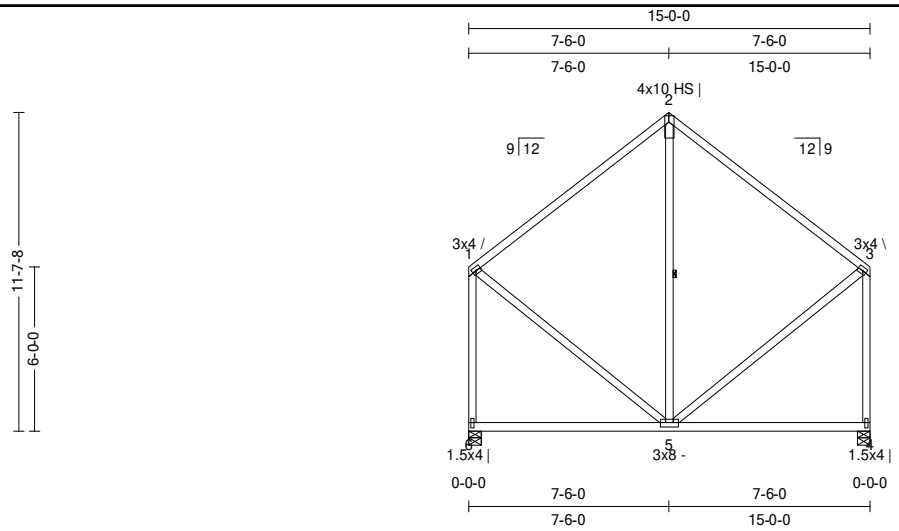
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: T2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:11
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
15-0-0	9/12	22	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	95 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.84 (2-3)	Vert TL:	0.16 in	L / 999	(4-5)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.65 (5-6)	Vert LL:	0.09 in	L / 999	(4-5)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.45 (1-6)	Horz TL:	0 in		4	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

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STATE OF WASHINGTON
240360 81
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Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	630 lbs	·	·	-69 lbs	-69 lbs	-127 lbs
4	1	5.5 in	1.50 in	630 lbs	·	·	-69 lbs	-69 lbs	·

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL #2 2 x 4 except:
DFL Standard 2 x 4: 1-6, 3-4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-5

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.840		(-420 lbs)	2-3	0.840		(-420 lbs)			
BC											
Web	1-6	0.446		(-571 lbs)	3-5	0.061	319 lbs				
	1-5	0.061	319 lbs		3-4	0.446		(-571 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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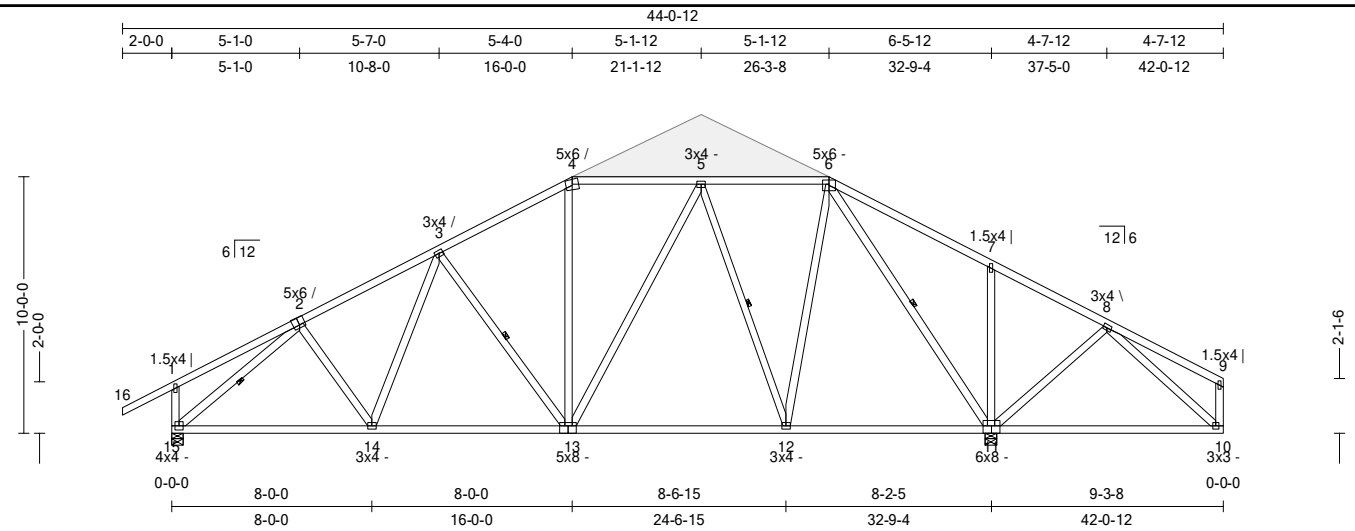
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: V1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:13
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-0-12	6/12	26	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	243 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.58 (6-7)	Vert TL:	0.31 in	L / 999	(12-13)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.86 (11-12)	Vert LL:	0.18 in	L / 999	(12-13)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.94 (6-11)	Cant / OH TL:	0.41 in	2L / 536	(10-11)	2L / 480
BCDL:	10	Lumber D.O.L.:	115 %			Cant / OH LL:	0.41 in	2L / 999	(10-11)	2L / 480
						Horz TL:	0.06 in		11	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	1,404 lbs	·	·	-141 lbs	-141 lbs	71 lbs
11	1	5.5 in	2.41 in	2,257 lbs	·	·	-376 lbs	-376 lbs	·

10/28/2025
ANISH KEKRE
STATE OF WASHINGTON
240360831
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Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 3-13, 4-13, 5-13, 5-12, 6-12, 6-11

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-15, 3-13, 5-12, 6-11

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.299		(-1,616 lbs)	5-6	0.366		(-762 lbs)			
	3-4	0.312		(-1,319 lbs)	6-7	0.576	659 lbs				
	4-5	0.312		(-1,106 lbs)	7-8	0.548	646 lbs				
BC	11-12	0.861	637 lbs		13-14	0.838	1,342 lbs				
	12-13	0.827	962 lbs		14-15	0.744	1,313 lbs				
Web	2-15	0.381		(-1,734 lbs)	5-12	0.290		(-668 lbs)	8-11	0.279	(-414 lbs)
	3-13	0.129		(-408 lbs)	6-12	0.154	801 lbs				
	4-13	0.061	317 lbs		6-11	0.940		(-1,723 lbs)			
	5-13	0.085	443 lbs		7-11	0.274		(-376 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: V1
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:14
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-0-12	6/12	26	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	243 lbs

7) Listed wind uplift reactions based on MWFRS & C&C loading.

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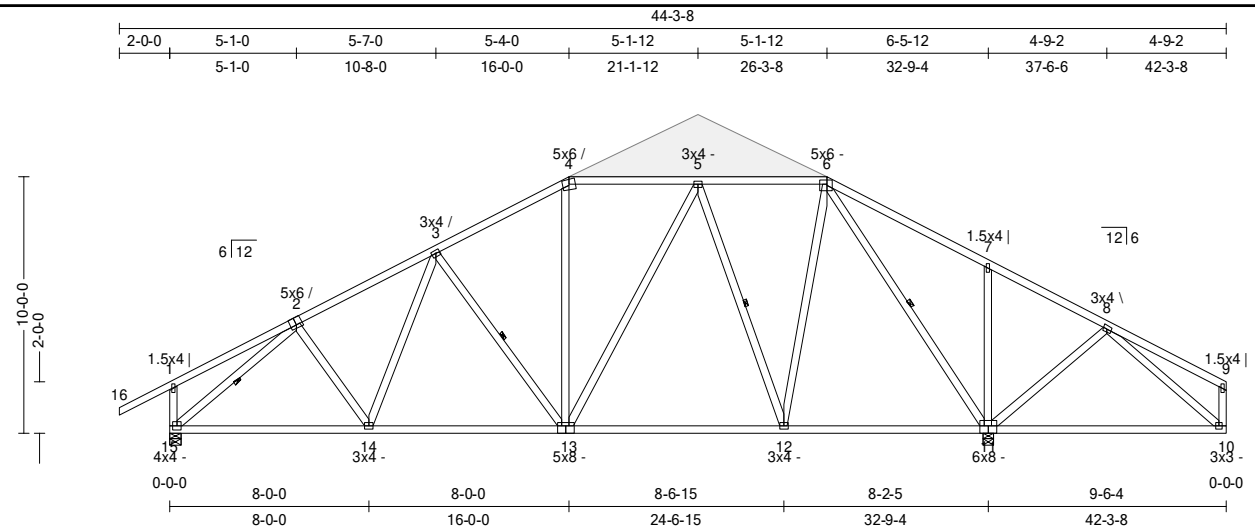
Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: V2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:16
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-3-8	6/12	6	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	243 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.58 (6-7)	Vert TL:	0.32 in	L / 999	(12-13)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.89 (11-12)	Vert LL:	0.19 in	L / 999	(12-13)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.95 (6-11)	Cant / OH TL:	0.45 in	2L / 494	(10-11)	2L / 480
BCDL:	10	Lumber D.O.L.:	115 %			Cant / OH LL:	0.45 in	2L / 999	(10-11)	2L / 480
						Horz TL:	0.06 in		11	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	1,398 lbs	·	·	-139 lbs	-139 lbs	69 lbs
11	1	5.5 in	2.43 in	2,282 lbs	·	·	-382 lbs	-382 lbs	·

10/28/2025

Material

TC: DFL #2 2 x 4
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4 except:
DFL #2 2 x 4: 3-13, 4-13, 5-13, 5-12, 6-12, 6-11

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-15, 3-13, 5-12, 6-11

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (19.2 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5
6) Truss designed as the base truss for a cap truss. Cap graphic is for reference only.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.298		(-1,612 lbs)	5-6	0.367		(-756 lbs)			
	3-4	0.312		(-1,315 lbs)	6-7	0.581	689 lbs				
	4-5	0.312		(-1,103 lbs)	7-8	0.557	677 lbs				
BC	11-12	0.888	631 lbs		13-14	0.839	1,338 lbs				
	12-13	0.819	957 lbs		14-15	0.743	1,311 lbs				
Web	2-15	0.380		(-1,730 lbs)	5-12	0.293		(-674 lbs)	8-11	0.291	(-424 lbs)
	3-13	0.129		(-407 lbs)	6-12	0.155	805 lbs		8-10	0.136	308 lbs
	4-13	0.060	314 lbs		6-11	0.952		(-1,744 lbs)			
	5-13	0.085	445 lbs		7-11	0.275		(-378 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

Mustang Truss
2525 Hyacinth Street NE
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: V2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:16
Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
42-3-8	6/12	6	2-0-0	0-0-0	0-0-0	0-0-0	1	24 in	243 lbs

7) Listed wind uplift reactions based on MWFRS & C&C loading.

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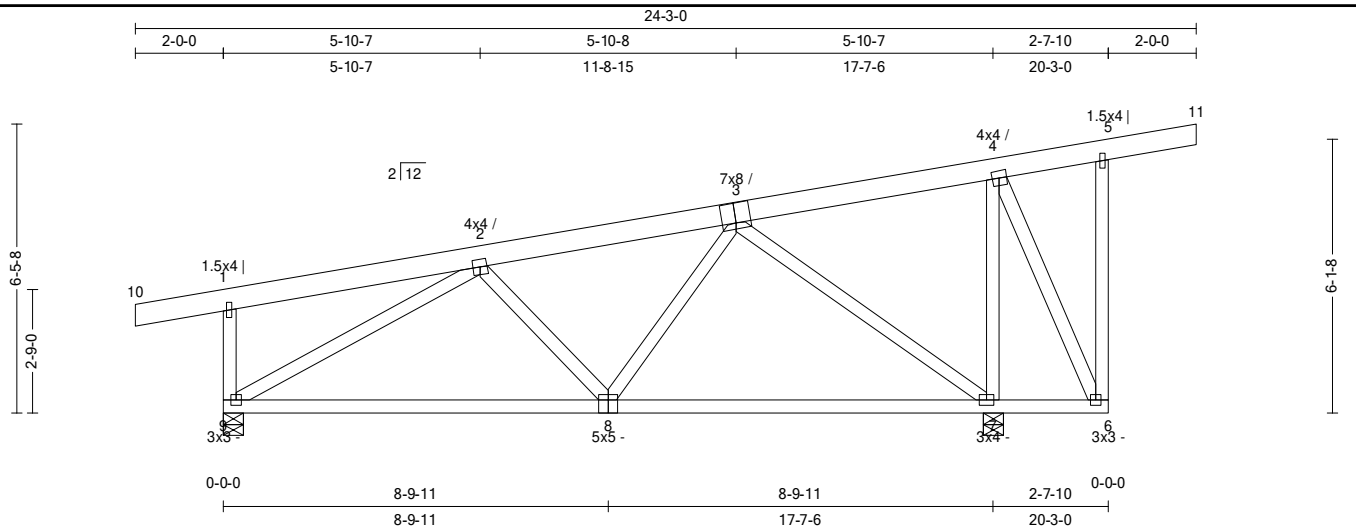
	Mustang Truss 2525 Hyacinth Street NE Salem, OR 97301 Ph: (503) 399-1432 Fax: (503) 399-1435	Truss: Z1 Job: 2501272 Designer:Anthony Date: 10/28/25 13:53:17 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-3-0	2/12	5	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	127 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	25	Bldg Code :	IRC 2021/	TC :	0.13 (10-1)	Vert TL:	0.32 in	L / 638	(8-9)	L / 180
TCDL :	7		TPI 1-2014	BC :	0.81 (8-9)	Vert LL:	0.15 in	L / 999	(8-9)	L / 240
BCLL :	0	Rep Mbr :	Yes	Web :	0.76 (3-7)	Cant / OH TL:	0.01 in UP	2L / 999	(6-7)	2L / 480
BCDL :	10	Lumber D.O.L. :	115 %			Cant / OH LL:	0.01 in UP	2L / 999	6	2L / 480
						Horz TL:	0.02 in		7	

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
9	1	5.5 in	1.50 in	839 lbs	·	·	-111 lbs	-111 lbs	120 lbs
7	1	5.5 in	1.50 in	1,118 lbs	·	·	-235 lbs	-235 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.088		(-858 lbs)								
BC	7-8	0.790	660 lbs		8-9	0.807	844 lbs					
Web	1-9	0.120		(-320 lbs)	3-8	0.157	356 lbs		4-7	0.239		(-496 lbs)
	2-9	0.754		(-979 lbs)	3-7	0.763		(-825 lbs)	4-6	0.133	301 lbs	

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

Mustang Truss
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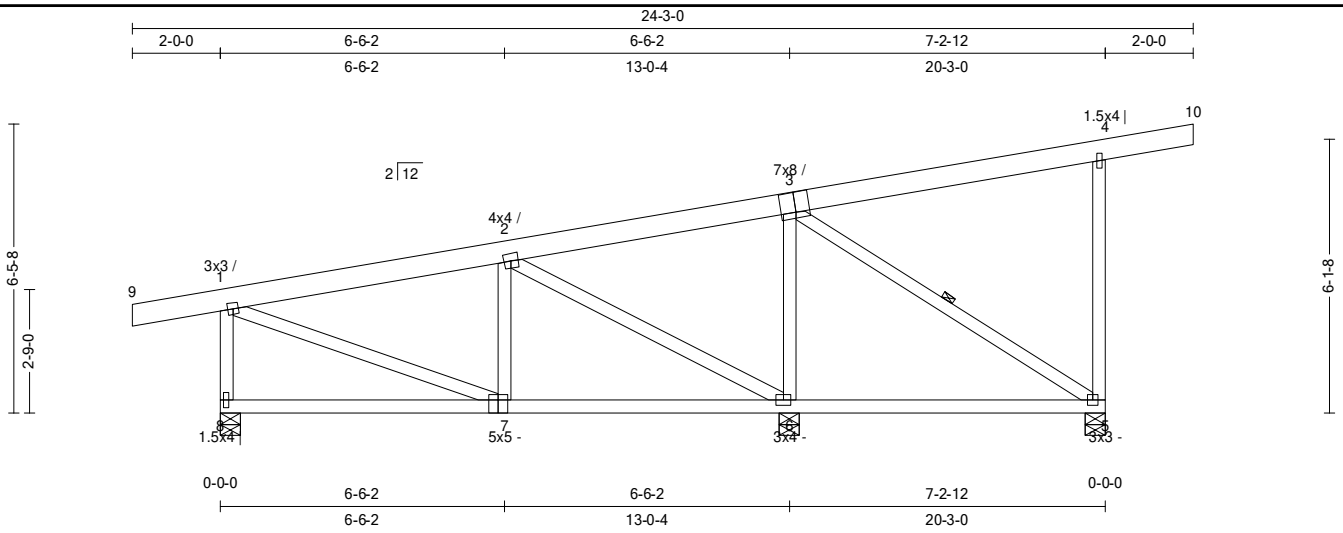
Salem, OR 97301
Ph: (503) 399-1432 Fax: (503) 399-1435

Truss: Z2
Job: 2501272
Designer: Anthony
Date: 10/28/25 13:53:19
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-3-0	2/12	9	2-0-0	2-0-0	0-0-0	0-0-0	1	24 in	122 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	25	Bldg Code:	IRC 2021/	TC:	0.19 (2-3)	Vert TL:	0.14 in	L / 553	(5-6)	L / 180
TCDL:	7		TPI 1-2014	BC:	0.49 (6-7)	Vert LL:	0.07 in	L / 999	(5-6)	L / 240
BCLL:	0	Rep Mbr:	Yes	Web:	0.63 (2-6)	Horz TL:	0.01 in		6	
BCDL:	10	Lumber D.O.L.:	115 %							

10/28/2025

ANISH KEKRE
STATE OF WASHINGTON
24036081
REGISTERED
PROFESSIONAL ENGINEER
EXPIRES 12/27/2027

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	5.5 in	1.50 in	633 lbs	·	·	-119 lbs	-119 lbs	120 lbs
6	1	5.5 in	1.50 in	972 lbs	·	·	-51 lbs	-51 lbs	·
5	1	5.5 in	1.50 in	405 lbs	·	-22 lbs	-125 lbs	-125 lbs	·

Material

TC: DFL SS 2 x 6
BC: DFL #2 2 x 4
Web: DFL Standard 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 3-5

Loads

1) This truss has been designed for the effects due to non-concurrent 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
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4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 98 mph (Factored), Exposure B, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.122		(-514 lbs)							
BC	6-7	0.492	480 lbs								
Web	1-8	0.121		(-580 lbs)	2-6	0.627	(-679 lbs)	4-5	0.431	(-345 lbs)	
	1-7	0.226	513 lbs		3-6	0.188	(-532 lbs)				

Notes

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3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
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7) Listed wind uplift reactions based on MWFRS & C&C loading.

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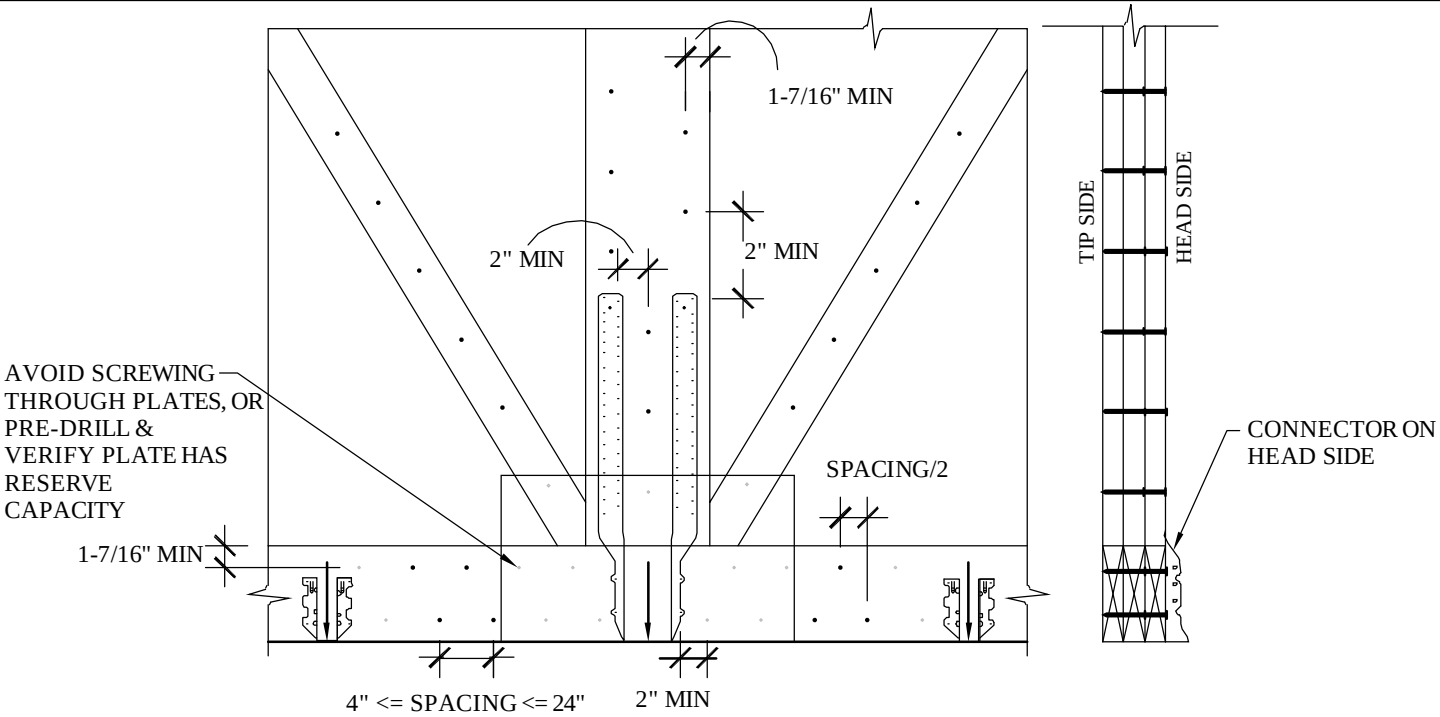

POINT LOAD	1/4"Ø SDS/SDW SCREWS INTO SP/DFL			1/4"Ø SDS/SDW SCREWS INTO SPF/HF		
	2-PLY (3" MIN)	3-PLY (4.5" MIN)	4-PLY (6" MIN)	2-PLY (3" MIN)	3-PLY (4.5" MIN)	4-PLY (6" MIN)
500	1	1	2	2	2	2
1,000	2	2	3	3	3	3
1,500	3	3	4	4	4	5
2,000	4	4	5	5	6	6
2,500	5	5	6	7	7	8
3,000	6	6	7	8	8	9
3,500	7	7	8	9	10	11
4,000	8	8	9	10	11	12
4,500	9	9	10	12	12	14
5,000	9	10	11	13	14	15
5,500	10	11	12	14	15	17
6,000	11	12	13	15	16	18
6,500	12	13	14	17	18	20
7,000	13	14	15	18	19	21
7,500	14	15	17	19	20	23
8,000	15	16	18	20	22	24
8,500	16	17	19	22	23	26
9,000	17	18	20	23	24	27
9,500	17	19	21	24	26	29
10,000	18	20	22	25	27	30
P =	P/280/2	2P/350/3	3P/350/4	P/200/2	2P/250/3	3P/250/4

GENERAL NOTES:

1. ALLOWABLE LOADS IN TABLES ARE SHOWN AT THE LOAD DURATION FACTOR OF CD = 1.00 AND SHALL BE MULTIPLIED BY ALL APPLICABLE ADJUSTMENT FACTORS PER THE NDS. LOADS MAY BE INCREASED FOR LOAD DURATION PER THE BUILDING CODE UP TO A CD OF 1.6.

2. MINIMUM FASTENER SPACING REQUIREMENTS: 6" END DISTANCE, 1-7/16" EDGE DISTANCE, 5/8" BETWEEN STAGGERED ROWS OF FASTENERS, 4" BETWEEN NON-STAGGERED ROWS OF FASTENERS AND 6" BETWEEN FASTENERS IN A ROW.

3. MAXIMUM FASTENER SPACING IS RECOMMENDED TO NOT EXCEED 24" ON-CENTER EXCEPT AS APPROVED BY A QUALIFIED DESIGNER.

4. FOR STRUCTURAL COMPOSITE LUMBER (SCL = LVL, PSL OR LSL) LOADS, ASSUME AN EQUIVALENT SPECIFIC GRAVITY OF 0.50 OR HIGHER.

5. RECOMMEND 1 ROW FOR 2x4, 2 ROWS FOR 2x6 & 2x8, 3 ROWS FOR 2x10 &2x12.

6. INSTALL PER MANUFACTURE RECOMMENDATIONS.

7. ALL APPLICATIONS ARE BASED ON FULI PENETRATION INTO THE MAIN MEMBER

EAGLE METAL

MULTI-PLY TRUSS GIRDER PLY CONNECTOR FOR ISOLATED POINT LOADS (SCREWS)

REV:	2.1
ENG:	MDV
CAD:	RC
DATE:	03/16/18

DRAWING NUMBER

DR-2