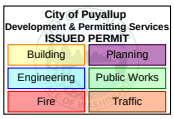
PRCTI20260575



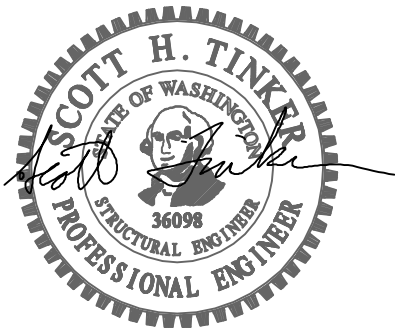

QUANTUM
CONSULTING ENGINEERS

April 22, 2026

**STRUCTURAL CALCULATIONS**
(Permit Submittal)

**CENTERIS DATA CENTER**
**LEVEL 2 DATA CENTER TI**
1023 39th Avenue SE
Puyallup, WA 98374

Quantum Job Number: 23444.01




*Prepared for:*
CENTERIS DATA CENTERS
18300 Cascade Avenue S
Seattle, WA 981188

*Prepared by:*
QUANTUM CONSULTING ENGINEERS
1511 Third Avenue, Suite 323
Seattle, WA 98101
TEL 206.957.3900
FAX 206.957.3901

# Structural Design Criteria

**Building Code:** 2021 International Building Code
**Building Department:** City of Puyallup

**Seismic Criteria**

| | | | |
|---|---|---|---|
| $S_s$: | 1.26 | | |
| $S_1$: | 0.43 | Seismic Soil Site Class: | D |
| $S_{ds}$: | 1.01 | Seismic Design Category: | D |
| $S_{d1}$: | 0.50 | | |

**Materials Criteria**

**Concrete (28 Day Strength):**

| | |
|---|---|
| Foundation/Slab on Grade | F'c= 3,000 PSI |
| Walls and Columns | F'c= 3,000 PSI |

**Reinforcing Steel:**

| | |
|---|---|
| Grade 60 | Fy= 60,000 PSI |

**Structural Steel:**

| | |
|---|---|
| Wide-Flange Sections: A-992 | Fy= 50,000 PSI |
| Miscellaneous Sections: A-36 | Fy= 36,000 PSI |
| Tube Sections: A-500 | Fy= 46,000 PSI |
| Pipe Sections: A-53 | Fy= 35,000 PSI |
| Welding | Fy= 70,000 PSI |

This is a beta release of the new ATC Hazards by Location website. Please contact us with feedback.

The ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

# Hazards by Location

## Search Information


| | |
|---|---|
| **Address:** | 1015 39th Ave SE Puyallup, WA 98374 |
| **Coordinates:** | 47.1590004, -122.2794422 |
| **Elevation:** | 489 ft |
| **Timestamp:** | 2023-12-01T15:14:56.409Z |
| **Hazard Type:** | Seismic |
| **Reference Document:** | ASCE7-16 |
| **Risk Category:** | III |
| **Site Class:** | D-default |

## Basic Parameters

| Name | Value | Description |
|---|---|---|
| $S_S$ | 1.257 | $MCE_R$ ground motion (period=0.2s) |
| $S_1$ | 0.433 | $MCE_R$ ground motion (period=1.0s) |
| $S_{MS}$ | 1.508 | Site-modified spectral acceleration value |
| $S_{M1}$ | * null | Site-modified spectral acceleration value |
| $S_{DS}$ | 1.005 | Numeric seismic design value at 0.2s SA |
| $S_{D1}$ | * null | Numeric seismic design value at 1.0s SA |

* See Section 11.4.8

## Additional Information

| Name | Value | Description |
|---|---|---|
| SDC | * null | Seismic design category |
| $F_a$ | 1.2 | Site amplification factor at 0.2s |
| $F_v$ | * null | Site amplification factor at 1.0s |
| $CR_S$ | 0.914 | Coefficient of risk (0.2s) |
| $CR_1$ | 0.898 | Coefficient of risk (1.0s) |
| PGA | 0.5 | $MCE_G$ peak ground acceleration |
| $F_{PGA}$ | 1.2 | Site amplification factor at PGA |
| $PGA_M$ | 0.6 | Site modified peak ground acceleration |
| $T_L$ | 6 | Long-period transition period (s) |
| SsRT | 1.257 | Probabilistic risk-targeted ground motion (0.2s) |
| SsUH | 1.375 | Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years) |
| SsD | 1.5 | Factored deterministic acceleration value (0.2s) |
| S1RT | 0.433 | Probabilistic risk-targeted ground motion (1.0s) |
| S1UH | 0.483 | Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years) |
| S1D | 0.6 | Factored deterministic acceleration value (1.0s) |
| PGAd | 0.5 | Factored deterministic acceleration value (PGA) |

* See Section 11.4.8

*The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.*

## CABINET ANCHORAGE DESIGN : 2021 IBC / ASCE 7-16

$$F_p = 0.4 \frac{a_p}{R_p} S_{DS} W_p I_p \left(1 + 2\frac{z}{h}\right)$$ Eqn 13.3-1

$\frac{a_p}{R_p} = \frac{1}{2.5} = 0.4$ Table 13.5-1

$S_{DS} = 1.01$ USGS

$W_p = 2{,}500$ lb (max.)

$I_p = 1.0$ 13.1.3

$z/h = 25/62 = 0.40$

$F_p = 0.4(0.4)\ 1.01\ W_p\ (1.0)(1 + 2(0.4))$

$= \boxed{0.29\ W_p}$ GOVERNS

$F_{p\,min} = 0.3 S_{DS} I_p W_p$

$= 0.25 W_p$ ~~governs~~

$F_{p\,A.S.D.} = 0.7\ (0.29)\ 2{,}500$ lb

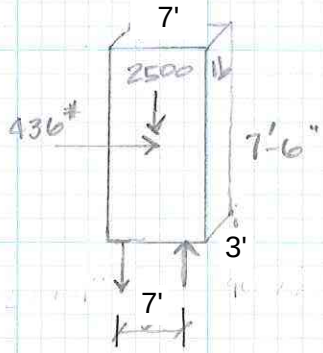
$= 510$ lb

AT R.A.F.

$M_{OTM} = F_p \cdot \frac{10.67'}{2} = 12721$ lb-ft

$M_R = W_p \cdot 7'/2 = 38700$ lb-ft

SF = 3.20 > 1.67 OK




Maximum force in threaded rod (conservatively neglecting the RAF)

$F = \frac{2{,}500\ lb}{4}$ + (3) (510 LB) (10.67'/2) / 9' / 2

$= 625 + 453$ = 1078 lb

1/2"ϕ THREADED ROD, h = 22"

$P_{CR} = \frac{\pi^2 E I}{k \ell^2} = \frac{\pi^2 (29{,}000)(\pi/4 \cdot 0.25^4)}{(1.0)(22)^2} = 1.81^k$ >> 1078 lb OK

CHECK: "SHORT COLUMN" $P_{allow} = 36\ ksi\ (0.6)\ .15\ in^2 = 3{,}240$ lb, ∴ buckling governs

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www.quantumce.com

| | | |
|---|---|---|
| CENTERIS - SDC | 4/17/2026 | 23444.01 |
| project | date | project no. |
| | TVM | |
| | designer | sheet |
| CENTERIS | | |
| client | checked by | |

## CABINET ANCHORAGE DESIGN (Cont.)

- (E) R.A.F. designed for 100 PSF LL, 25 PSF SEISMIC MASS

TYPICAL CABINET ROW:

2500 LB / (3'(7' + 4'))) = 76 PSF  SERVER IS 3'X7', AISLE IS 8' WIDE

∴ GRAVITY SUPPORT IS ADEQUATE, BUT SEISMIC BOLSTERING IS REQ'D w/ DIAGONAL RODS

3' CABINET DIRECTION

$F_p$ = (1.41) 510 lb / 2 braces = 360 lb, ½"ϕ ROD OK by insp

7' CABINET DIRECTION

$F_p$ = 510 lb tension or compression, ½"ϕ rod

$l_{rod} = \sqrt{2^2 + 7^2} = 7.3' = 88"$

$F_{rod}$ = 510 lb (7.3'/7') = 532 lb  $f_t$ = 0.9 ksi OK for tension only

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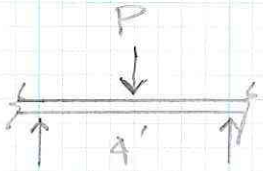
| | | |
|---|---|---|
| CENTERIS - SDC (project) | 4/17/2026 (date) | 23444.01 (project no.) |
| | TVM (designer) | (sheet) |
| CENTERIS (client) | (checked by) | |

# CHECK UNI-STRUT

- design for worst case
- conservatively neglect RAF support for seismic V.O.T.

$l$ = 4'

$P$ = 453 lb




P_all = 850 lb (0.67) (0.88)
P_all = 501 lb > 453 lb OK

1⅝" x 1⅝" UNISTRUT

See next sheet.

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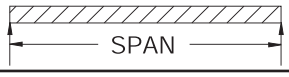
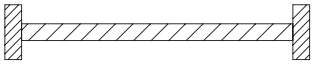
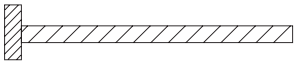
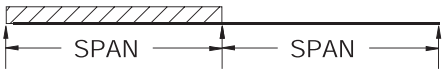
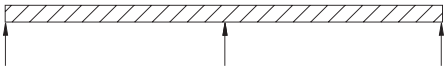
| | | |
|---|---|---|
| CENTERIS - SDC | 4/17/2026 | 23444.01 |
| project | date | project no. |
| | TVM | |
| | designer | sheet |
| CENTERIS | | |
| client | checked by | |

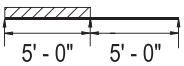
## Conversion Factors For Beams With Various Static Loading Conditions

All Beam Load tables are for single-span (simple) beams supported at the ends.These can be used in the majority of the cases. However, there are times when it is necessary to know what happens with other loading and support conditions. Some common arrangements are shown below. Simply multiply the values from the Beam Load tables by factors given below

| Load and Support Condition | Load Factor | Deflection Factor |
|---|---|---|
| 1. **Simple Beam, Uniform Load**   | **1.00** | **1.00** |
| 2. **Simple Beam, Concentrated Load at Center**  | **.50** | **.80** |
| 3. **Simple Beam, Two Equal Concentrated Loadcs at 1/4 pts**  | **1.00** | **1.10** |
| 4. **Beam Fixed at Both Ends, Uniform Load**  | **1.50** | **.30** |
| 5. **Beam Fixed at Both Ends, Concentrated Load at Center**  | **1.00** | **.40** |
| 6. **Cantilever Beam, Uniform Load**  | **.25** | **2.40** |
| 7. **Cantilever Beam, Concentrated Load at End**  | **.12** | **3.20** |
| 8. **Continuous Beam, Two Equal Spans, Uniform Load on One Span**   | **1.30** | **.92** |
| 9. **Continuous Beam, Two Equal Spans, Uniform Load on Both Ends**  | **1.00** | **.42** |
| 10. **Continuous Beam, Two Equal Spans, Concentrated Load at Center of One Span**  | **.62** | **.71** |
| 11. **Continuous Beam, Two Equal Spans, Concentrated Load at Center of Each Span**  | **.67** | **.48** |

**EXAMPLE I:**

Determine load and deflection of a P 1000 beam continuous over one support and loaded uniformly on one span.

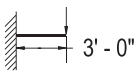



**SOLUTION:**

A. From load table for P1000 on page 25 load for a 5'-0" span is 680# and deflection is .35".
B. Multiply by factors from Table above.
Load = 680# x 1.30 = 884#
Deflection = .35" x .92 = .32"

**EXAMPLE II**

Determine load and deflection of a P 5500 cantilever beam with a concentrated load on the end.




**SOLUTION:**

A. From load table P5500 on page 58 load for a 3'-0" span is 2180# and deflection is .09".
B. Multiply by factors from Table above.
Load = 2180# x .12 = 262#
Deflection = .09" x 3.20 = .29"

## P1000 - Beam Loading

| Span In | Max. Allowable Uniform Load Lbs | Defl. at Uniform Load In | Uniform Loading at Deflection Span/180 Lbs | Span/240 Lbs | Span/360 Lbs |
|---|---|---|---|---|---|
| 24 | 1,690 | 0.06 | 1,690 | 1,690 | 1,690 |
| 36 | 1,130 | 0.13 | 1,130 | 1,130 | 900 |
| 48 | 850 | 0.22 | 850 | 760 | 500 |
| 60 | 680 | 0.35 | 650 | 480 | 320 |
| 72 | 560 | 0.50 | 450 | 340 | 220 |
| 84 | 480 | 0.68 | 330 | 250 | 160 |
| 96 | 420 | 0.89 | 250 | 190 | 130 |
| 108 | 380 | 1.14 | 200 | 150 | 100 |
| 120 | 340 | 1.40 | 160 | 120 | 80 |
| 144 | 280 | 2.00 | 110 | 80 | 60 |
| 168 | 240 | 2.72 | 80 | 60 | 40 |
| 192 | 210 | 3.55 | 60 | 50 | NR |
| 216 | 190 | 4.58 | 50 | 40 | NR |
| 240 | 170 | 5.62 | 40 | NR | NR |

## P1001 - Beam Loading

| Span In | Max. Allowable Uniform Load Lbs | Defl. at Uniform Load In | Uniform Loading at Deflection Span/180 Lbs | Span/240 Lbs | Span/360 Lbs |
|---|---|---|---|---|---|
| 24 | 3,500* | 0.02 | 3,500* | 3,500* | 3,500* |
| 36 | 3,190 | 0.07 | 3,190 | 3,190 | 3,190 |
| 48 | 2,390 | 0.13 | 2,390 | 2,390 | 2,390 |
| 60 | 1,910 | 0.20 | 1,910 | 1,910 | 1,620 |
| 72 | 1,600 | 0.28 | 1,600 | 1,600 | 1,130 |
| 84 | 1,370 | 0.39 | 1,370 | 1,240 | 830 |
| 96 | 1,200 | 0.51 | 1,200 | 950 | 630 |
| 108 | 1,060 | 0.64 | 1,000 | 750 | 500 |
| 120 | 960 | 0.79 | 810 | 610 | 410 |
| 144 | 800 | 1.14 | 560 | 420 | 280 |
| 168 | 680 | 1.53 | 410 | 310 | 210 |
| 192 | 600 | 2.02 | 320 | 240 | 160 |
| 216 | 530 | 2.54 | 250 | 190 | 130 |
| 240 | 480 | 3.16 | 200 | 150 | 100 |

## P1000 - Column Loading

| Unbraced Height In | Max. Allowable Load at Slot Face Lbs | Maximum Column Load Applied at C.G. K = 0.65 Lbs | K = 0.80 Lbs | K =1.0 Lbs | K = 1.2 Lbs |
|---|---|---|---|---|---|
| 24 | 3,550 | 10,740 | 9,890 | 8,770 | 7,740 |
| 36 | 3,190 | 8,910 | 7,740 | 6,390 | 5,310 |
| 48 | 2,770 | 7,260 | 6,010 | 4,690 | 3,800 |
| 60 | 2,380 | 5,910 | 4,690 | 3,630 | 2,960 |
| 72 | 2,080 | 4,840 | 3,800 | 2,960 | 2,400 |
| 84 | 1,860 | 4,040 | 3,200 | 2,480 | 1,980 |
| 96 | 1,670 | 3,480 | 2,750 | 2,110 | 1,660 |
| 108 | 1,510 | 3,050 | 2,400 | 1,810 | ** |
| 120 | 1,380 | 2,700 | 2,110 | ** | ** |
| 144 | 1,150 | 2,180 | 1,660 | ** | ** |

## P1001 - Column Loading

| Unbraced Height In | Max. Allowable Load at Slot Face Lbs | Maximum Column Load Applied at C.G. K = 0.65 Lbs | K = 0.80 Lbs | K =1.0 Lbs | K = 1.2 Lbs |
|---|---|---|---|---|---|
| 24 | 6,430 | 24,280 | 23,610 | 22,700 | 21,820 |
| 36 | 6,290 | 22,810 | 21,820 | 20,650 | 19,670 |
| 48 | 6,160 | 21,410 | 20,300 | 18,670 | 16,160 |
| 60 | 6,000 | 20,210 | 18,670 | 15,520 | 12,390 |
| 72 | 5,620 | 18,970 | 16,160 | 12,390 | 8,950 |
| 84 | 5,170 | 16,950 | 13,630 | 9,470 | 6,580 |
| 96 | 4,690 | 14,890 | 11,190 | 7,250 | 5,040 |
| 108 | 4,170 | 12,850 | 8,950 | 5,730 | 3,980 |
| 120 | 3,690 | 10,900 | 7,250 | 4,640 | ** |
| 144 | 2,930 | 7,630 | 5,040 | ** | ** |

## P1000/P1001 - Elements of Section

| Parameter | P1000 | | P1001 | |
|---|---|---|---|---|
| Area of Section | 0.555 | $In^2$ | 1.111 | $In^2$ |
| Axis 1-1 | | | | |
| Moment of Inertia (I) | 0.185 | $In^4$ | 0.928 | $In^4$ |
| Section Modulus (S) | 0.202 | $In^3$ | 0.571 | $In^3$ |
| Radius of Gyration (r) | 0.577 | In | 0.914 | In |
| Axis 2-2 | | | | |
| Moment of Inertia (I) | 0.236 | $In^4$ | 0.471 | $In^4$ |
| Section Modulus (S) | 0.290 | $In^3$ | 0.580 | $In^3$ |
| Radius of Gyration (r) | 0.651 | In | 0.651 | In |

Notes:

* Load limited by spot weld shear.

** KL/r > 200

NR = Not Recommended.

1. Beam loads are given in *total* uniform load (W Lbs) not uniform load (w lbs/ft or w lbs/in).
2. Beam loads are based on a simple span and assumed to be adequately laterally braced. Unbraced spans can reduce beam load carrying capacity. Refer to Page 62 for reduction factors for unbraced lengths.
3. For pierced channel, multiply beam loads by the following factor:

**"KO" Series.....95%**
**"HS" Series.....90%**
**"H3" Series.....90%**
**"WT" Series.....85%**
**"T" Series.....85%**
**"SL" Series.....85%**
**"DS" Series.....70%**

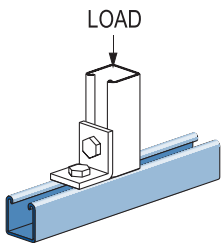
4. Deduct channel weight from the beam loads.
5. For concentrated midspan point loads, multiply beam loads by 50% and the corresponding deflection by 80%. For other load conditions refer to page 18.
6. All beam loads are for bending about Axis 1-1.

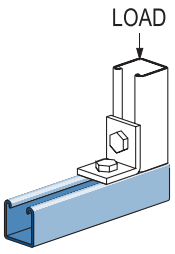
## Lateral Bracing Load Reduction Charts

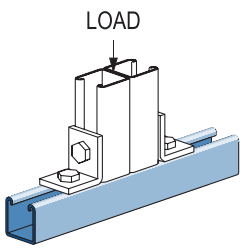
| Span | | Single Channel | | | | | | | | | | | Double Channel | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| Ft. *(m)* | In. *(cm)* | P1000 | P1100 | P2000 | P3000 | P3300 | P4000 | P4100 | P4400 | P4520 | P5000 | P5500 | P1001 | P1101 | P2001 | P3001 | P3301 | P4001 | P4101 | P4401 | P4521 | P5001 | P5501 |
| 2 *(0.61)* | 24 *(61)* | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.98 | 0.99 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| 3 *(0.91)* | 36 *(91)* | 0.94 | 0.89 | 0.88 | 0.96 | 1.00 | 0.94 | 0.98 | 1.00 | 1.00 | 0.85 | 0.89 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| 4 *(1.22)* | 48 *(122)* | 0.88 | 0.78 | 0.75 | 0.91 | 1.00 | 0.88 | 0.94 | 0.98 | 1.00 | 0.70 | 0.77 | 1.00 | 0.98 | 0.98 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 1.00 | 0.97 | 0.98 |
| 5 *(1.52)* | 60 *(152)* | 0.82 | 0.68 | 0.61 | 0.88 | 0.98 | 0.83 | 0.91 | 0.96 | 1.00 | 0.55 | 0.67 | 0.97 | 0.93 | 0.92 | 0.98 | 1.00 | 0.93 | 0.96 | 1.00 | 1.00 | 0.90 | 0.93 |
| 6 *(1.83)* | 72 *(183)* | 0.78 | 0.59 | 0.48 | 0.84 | 0.97 | 0.79 | 0.89 | 0.94 | 0.98 | 0.44 | 0.58 | 0.93 | 0.87 | 0.85 | 0.95 | 0.97 | 0.88 | 0.92 | 0.97 | 0.97 | 0.83 | 0.87 |
| 7 *(2.13)* | 84 *(213)* | 0.75 | 0.52 | 0.41 | 0.82 | 0.96 | 0.75 | 0.86 | 0.92 | 0.97 | 0.38 | 0.51 | 0.89 | 0.82 | 0.78 | 0.92 | 0.95 | 0.83 | 0.89 | 0.95 | 0.95 | 0.76 | 0.81 |
| 8 *(2.44)* | 96 *(244)* | 0.71 | 0.47 | 0.35 | 0.79 | 0.94 | 0.72 | 0.84 | 0.91 | 0.96 | 0.33 | 0.46 | 0.85 | 0.76 | 0.71 | 0.88 | 0.92 | 0.79 | 0.85 | 0.92 | 0.92 | 0.68 | 0.76 |
| 9 *(2.74)* | 108 *(274)* | 0.69 | 0.43 | 0.32 | 0.77 | 0.93 | 0.69 | 0.82 | 0.89 | 0.95 | 0.30 | 0.42 | 0.81 | 0.70 | 0.64 | 0.85 | 0.90 | 0.74 | 0.81 | 0.90 | 0.90 | 0.61 | 0.70 |
| 10 *(3.05)* | 120 *(305)* | 0.66 | 0.40 | 0.29 | 0.75 | 0.92 | 0.66 | 0.80 | 0.87 | 0.94 | 0.28 | 0.40 | 0.78 | 0.65 | 0.57 | 0.82 | 0.87 | 0.69 | 0.78 | 0.87 | 0.87 | 0.54 | 0.64 |
| 12 *(3.66)* | 144 *(366)* | 0.61 | 0.36 | 0.25 | 0.70 | 0.89 | 0.60 | 0.76 | 0.84 | 0.91 | 0.24 | 0.36 | 0.70 | 0.54 | 0.45 | 0.76 | 0.82 | 0.60 | 0.71 | 0.82 | 0.83 | 0.43 | 0.53 |
| 14 *(4.27)* | 168 *(427)* | 0.55 | 0.32 | 0.23 | 0.66 | 0.86 | 0.55 | 0.73 | 0.81 | 0.89 | 0.22 | 0.32 | 0.63 | 0.45 | 0.38 | 0.70 | 0.78 | 0.51 | 0.64 | 0.77 | 0.78 | 0.35 | 0.45 |
| 16 *(4.88)* | 192 *(488)* | 0.51 | 0.30 | 0.21 | 0.62 | 0.84 | 0.50 | 0.69 | 0.78 | 0.87 | 0.21 | 0.30 | 0.56 | 0.39 | 0.32 | 0.64 | 0.73 | 0.44 | 0.57 | 0.72 | 0.73 | 0.30 | 0.39 |
| 18 *(5.49)* | 216 *(549)* | 0.47 | 0.28 | 0.19 | 0.58 | 0.81 | 0.47 | 0.65 | 0.75 | 0.84 | 0.19 | 0.28 | 0.49 | 0.34 | 0.28 | 0.58 | 0.68 | 0.39 | 0.50 | 0.67 | 0.68 | 0.27 | 0.34 |
| 20 *(6.10)* | 240 *(610)* | 0.44 | 0.26 | 0.18 | 0.54 | 0.78 | 0.43 | 0.61 | 0.72 | 0.82 | 0.18 | 0.26 | 0.44 | 0.31 | 0.25 | 0.52 | 0.63 | 0.35 | 0.45 | 0.62 | 0.63 | 0.24 | 0.30 |

## Bearing Loads on Unistrut Channel

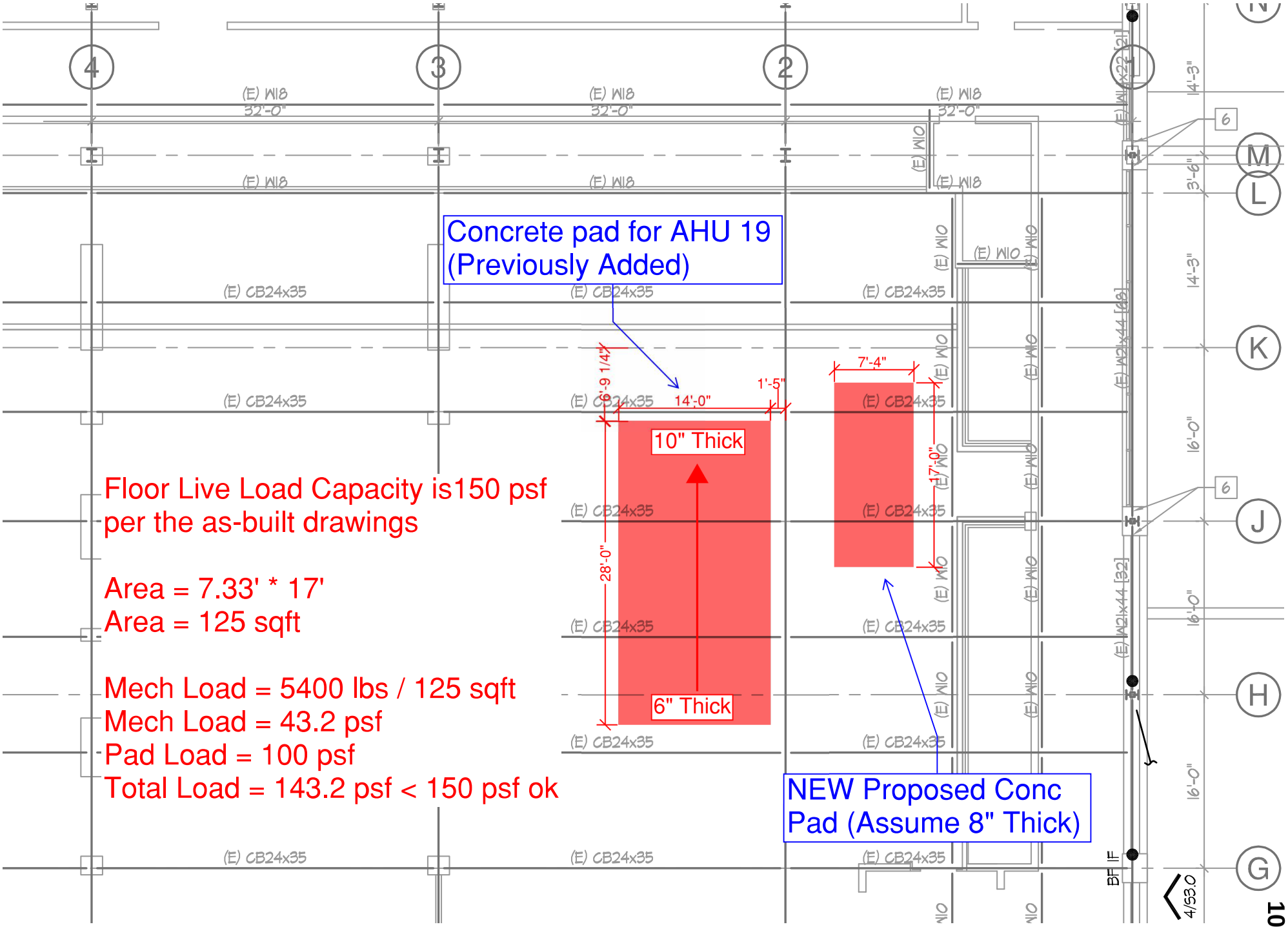
Loads are calculated based on 2007 Specification For The Design Of Cold Formed Steel Structural Members published by AISI




| Channel | Bearing Length 1⅝" (41 mm) Maximum Allowable Loads Lbs *(kN)* | Bearing Length 1⅝" (41 mm) Maximum Allowable Loads Lbs *(kN)* | Bearing Length 3¼" (82 mm) Maximum Allowable Loads Lbs *(kN)* |
|---|---|---|---|
| P1000 | 6,700<br>*29.80* | 3,100<br>*13.79* | 7,700<br>*34.25* |
| P1100 | 3,500<br>*15.57* | 1,700<br>*7.56* | 4,000<br>*17.79* |
| P2000 | 2,500<br>*11.12* | 1,200<br>*5.34* | 3,000<br>*13.34* |
| P3000 | 6,700<br>*29.80* | 3,200<br>*14.23* | 7,700<br>*34.25* |
| P3300 | 6,800<br>*30.25* | 3,200<br>*14.23* | 7,800<br>*34.70* |
| P4000 | 2,600<br>*11.57* | 1,200<br>*5.34* | 3,000<br>*13.34* |
| P4100 | 3,500<br>*15.57* | 1,800<br>*8.01* | 4,100<br>*18.24* |
| P4400 | 7,300<br>*32.47* | 3,400<br>*15.12* | 8,400<br>*37.37* |
| P4520 | 7,300<br>*32.47* | 3,400<br>*15.12* | 8,400<br>*37.37* |
| P5000 | 6,500<br>*28.91* | 3,000<br>*13.34* | 7,500<br>*33.36* |
| P5500 | 6,600<br>*29.36* | 3,100<br>*13.79* | 7,600<br>*33.81* |


Concrete pad for AHU 19
(Previously Added)
Floor Live Load Capacity is150 psf
per the as-built drawings
Area = 7.33' * 17'
Area = 125 sqft
Mech Load = 5400 lbs / 125 sqft
Mech Load = 43.2 psf
Pad Load = 100 psf
Total Load = 143.2 psf < 150 psf ok
10" Thick
6" Thick
6'-9 1/4"
28'-0"
14'-0"
1'-5"
7'-4"
17'-0"
NEW Proposed Conc
Pad (Assume 8" Thick)
(E) CB24x35
(E) W18
32'-0"
(E) W10
(E) CM22x44 [21]
(E) CM22x44 [69]
(E) CM22x44 [32]
4
3
2
1
M
L
K
J
H
G
14'-3"
3'-9"
16'-0"
BF IF
4/S3.0
10