



A&A ENGINEERING
CIVIL • STRUCTURAL

6036 Renaissance Place
Toledo, Ohio 43623

Tel: 419-292-1983
Fax: 419-292-0955

PROJECT: MILAM AUTO		PROJECT NO.: 233-21-3409	SHEET NO.: 1/7
CLIENT: AMERICAN CARPORTS	LOCATION: PUYALLUP, WA 98371		
CALCULATED BY: A.F.	CHECKED BY: O.A.	APPROVED BY:	SCALE:
DATE: 11/6/2021	DATE: 11/6/2021	DATE:	

Entity Color Legend

TS252514GA

TS252514GAD14

TS252514GAQ14

TS222214GA

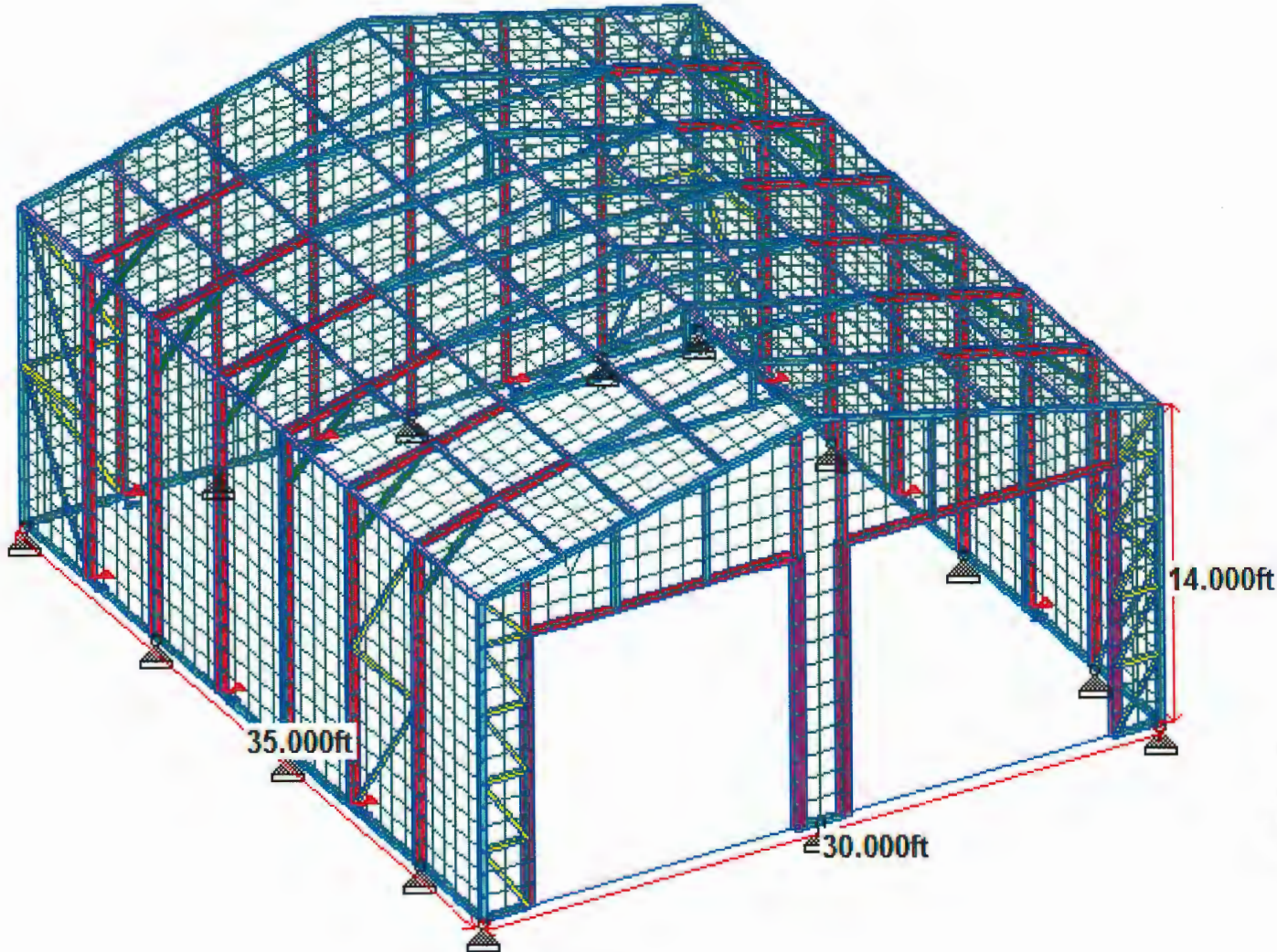
C401014GAD

C351514GA

DUMMY

Default Plate Color

Default Solid Color



DATE EXPIRES: 01/03/2024
DATE SIGNED: 04/05/2022



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PROJECT: MILAM AUTO		PROJECT NO: 233-21-3409	SHEET NO: 2/7
CLIENT: AMERICAN CARPORTS	LOCATION: PUYALLUP, WA 98371		SCALE:
CALCULATED BY: A.F.	CHECKED BY: O.A.	APPROVED BY:	
DATE: 11/6/2021	DATE: 11/6/2021	DATE:	

INDEX TO CONTENTS

I. SUMMARY: 3

1. BUILDING SIZE: 3

2. LOADS: 3

II. CODE INFORMATION: 3

1. CODE: 3

2. OCCUPANCY: 3

III. LOAD CALCULATIONS: 3

1. DEAD LOAD (D): 3

2. ROOF LIVE LOAD (L_R): 3

3. SNOW LOAD(S): 3

4. WIND LOAD (W): 3

5. EARTHQUAKE LOAD (E): 3

IV. LOAD COMBINATIONS: 4

A. STRENGTH 4

V. MATERIAL PROPERTIES: 4

1. STEEL: 4

VI. SECTION PROPERTIES: 4

VII. MAXIMUM DEFLECTIONS : 5

VIII. CONNECTION CALCULATIONS: 5

1. FRAME COLUMN – BASE CONNECTION: 5

2. HAT-CHANNEL - KNEE BRACE CONNECTION: 6

3. DIAGONAL BRACING COMPONENT CHECKS: 6

IX. FOUNDATION CALCULATIONS: 7

1. ANCHOR REACTIONS: 7

2. HOME PRIDE HP 9 (BARBED ANCHOR): 7

3. SOIL BEARING CHECK 7

X. APPENDICES:

1. WIND LOAD CALCULATIONS: W 1-04

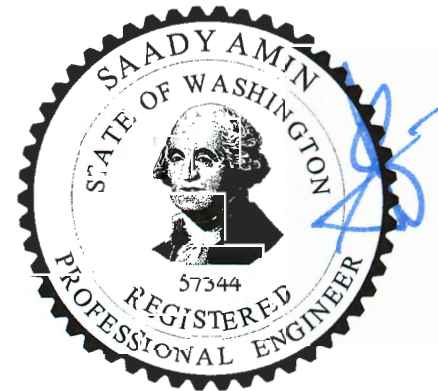
2. ANALYSIS (STAAD.PRO) INPUT FILE: SI 01-04

3. ANALYSIS (STAAD.PRO) OUTPUT FILE: SO 01-25


4. METAL SHEATHING SPECIFICATIONS: MS 01

5. ANCHOR SPECIFICATIONS: ST 1-3

6. ASCE 7 HAZARD REPORT: H 1-3



DATE EXPIRES: 01/03/2024
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	CLIENT: AMERICAN CARPORTS	LOCATION: PUYALLUP, WA 98371		
	CALCULATED BY: A.F.	CHECKED BY: O.A.	APPROVED BY:	
	DATE: 11/6/2021	DATE: 11/6/2021	DATE:	

I. Summary:

1. Building Size:

Length:	35 ft
Width:	30 ft
Eave height:	14 ft
Roof pitch:	3 / 12
Total Building square footage:	1050 Sq ft

2. Loads:

Roof live load:	20 psf
Ground snow load:	30 psf
Wind speed:	110 mph
Exposure:	C
Seismic S_s spectral response accel.:	1.277
Seismic Design Category:	D

II. Code Information:

1. Code:

WSBC (IBC 2018)

2. Occupancy:

Occupancy Group:	U
Construction Type:	II-B
Risk Category:	I

III. Load Calculations:

1. Dead Load (D):

Dead load (super imposed): $D = 2$ psf

2. Roof Live Load (L_r):

Roof live load (specified): $L_r = 20$ psf

3. Snow Load(S):

Peak ground snow load:	$p_g = 30$ psf
Exposure factor:	$C_e = 1.00$
Thermal factor:	$C_t = 1.20$
Importance factor:	$I_s = 0.80$
Slope factor:	$C_s = 1.0$
Flat roof load:	$\rightarrow p_f = 20.00$ psf
Sloped roof load:	$\rightarrow p_s = 20.00$ psf
*Roof live load controls	

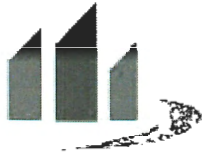
4. Wind Load (W):

*Please see Appendix W1-W4 for wind load calculations

Base Shear due to Wind Loads:	V
V_W in X dir:	12.031 Kips
V_W in Z dir:	12.135 Kips
(Refer Statics Check Results on sheet SO 10, 11 / 25)	

5. Earthquake Load (E):

Site Class:	D (Default)
Seismic Design category:	D
Spectral response acceleration at short periods:	$S_S = 1.277$ g
Site Coefficient:	$F_a = 1.200$
Adjusted spectral response acceleration:	$S_{MS} = F_a S_S$
$\rightarrow S_{MS} = 1.200 \times 1.277 = 1.532$ % g	
Design spectral response acceleration:	$S_{DS} = \frac{2}{3} S_{MS}$
$\rightarrow S_{DS} = (2 / 3) \times 1.532 = 1.022$ % g	
Lateral Force resisting system :	
"steel ordinary concentrically braced frames"	
Response modification factor:	R = 3.25
(ASCE Table 12.2-1)	
Over strength Factor (Applied in load combinations):	$\Omega = 2$
(ASCE 7 Table 12.2-1)	
Occupancy importance factor:	I = 1.00
Weight of structure:	W = 2100.00 lbs
20% of Roof Live Load (20 psf):	0.20 $L_r = 0.00$ lbs
Effective Seismic weight of structure:	$W_s = D + 0.20 L_r$



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CLIENT: AMERICAN CARPORTS	LOCATION: PUYALLUP, WA 98371		SCALE:
CALCULATED BY: A.F.	CHECKED BY: O.A.	APPROVED BY:	
DATE: 11/6/2021	DATE: 11/6/2021	DATE:	

$$W_s = 2100.00 \text{ lbs}$$

Seismic response coefficient:

$$\rightarrow C_s = 1.022 / (3.25 / 1.00) = 0.314$$

Seismic base shear:

$$\rightarrow V_s = 0.314 \times 2100.00 = 660.11 \text{ lbs}$$

Min. no. of resisting frames in X dir:

$$n_x = 4$$

Seismic Force per frame in X dir:

$$E_x = V_s / n_x$$

$$\rightarrow E_x = 660.11 / 4 = 165.03 \text{ lbs}$$

Min. no. of resisting frames in Z dir:

$$n_z = 4$$

Seismic Force per frame in Z dir:

$$E_z = V_s / n_z$$

$$\rightarrow E_z = 660.11 / 4 = 165.03 \text{ lbs}$$

Minimum Base Shear due to Wind Loads:

(Refer Statics Check Results on sheet SO 10, 11 / 25)

V_w in X dir:

$$12.031 \text{ Kips}$$

$$V_w = 12031.00 \text{ lbs} > V_s = 660.11 \text{ lbs}$$

Thus, Wind load controls Base Shear in X direction.

V_w in Z dir:

$$12.135 \text{ Kips}$$

$$V_w = 12135.00 \text{ lbs} > V_s = 660.11 \text{ lbs}$$

Thus, Wind load controls Base Shear in Z direction.

IV. Load Combinations:

a. Strength

1. D + (L_r or S)
2. D + (0.6W or 0.7 E × Ω)
3. D + 0.75 (L_r or S) + 0.75 (0.6W)
4. 0.6 D + (0.6W or 0.7 E × Ω)

Over strength Factor:

$$\Omega = 2$$

(Based on ASCE 7 Table 12.2-1)

V. Material Properties:

1. Steel:

ASTM A572 Grade 50

$$F_y = 50 \text{ ksi}$$


$$F_u = 65 \text{ ksi}$$

$$E = 29,000 \text{ ksi}$$

$$\gamma = 490 \text{ lbs/ft}^3$$

VI. Section Properties:

Section	A	I _x	I _y	Gauge
TS252514GA	0.802	0.782	0.782	14
TS252514GAD14	1.605	3.596	1.564	14
TS252514GAQ14	1.632	7.586	7.586	14
TS222214GA	0.719	0.564	0.564	14
C351514GA	0.32	0.117	0.317	14
C401014GAD	1.188	1.929	4.098	14

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	CLIENT: AMERICAN CARPORTS		LOCATION: PUYALLUP, WA 98371		
	CALCULATED BY: A.F.		CHECKED BY: O.A.	APPROVED BY:	SCALE:
	DATE: 11/6/2021		DATE: 11/6/2021	DATE:	

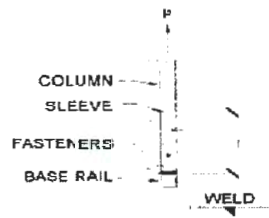
VII. Maximum Deflections :

Xmax	$\frac{I_y}{120}$	Ymax	$\frac{I_x}{180}$	Zmax	$\frac{I_y}{120}$
0.41	1.78 OK	0.73	2.00 OK	0.56	1.78 OK

VIII. Connection Calculations:

Note: See ESR2196 for Hilti Kwik-Pro (equivalent to ITW BuildexTEKS screws) specifications

1. Frame Column – Base Connection:



A. Forces:

Uplift force in column: $P = 1.53$ Kips
 Max. Shear force $V = 1.25$ Kips
 Combined Uplift + Shear: $F = (P^2 + V^2)^{1/2} = 1.97$ Kips

B. Dimensions & Properties:

a. Column:

Tube side: $s_c = 2.5$ in
 Tube thickness: $t_c = 0.083$ in

b. Base rail:

Tube side: $s_b = 2.5$ in
 Tube thickness: $t_b = 0.083$ in

c. Connector Sleeve:

Tube side: $s_s = 2$ in
 Tube thickness: $t_s = 0.083$ in
 Length: $l_s = 6$ in

d. Fasteners:

Fastener type: **HILTI Kwik Pro**
 Nominal diameter: **#12**
 Min. number of fasteners: $n_f = 4$

e. Weld:

Weld size: $t_w = 0.125$ in
 Weld tensile strength: $F_{uw} = 60$ ksi

C. Component checks:

a. Weld:

Total length of weld:
 $\rightarrow l_w = 2 \times 2.5 = 5$ in
 Nominal shear strength: $P_{nw} = t l_w F_{uw}$ (AISI E2.4-3)
 $\rightarrow P_{nw} = 0.125 \times 5 \times 60 = 37.5$ Kips
 Allowable shear strength: $P_{aw} = P_{nw} / \Omega$
 $\rightarrow P_{aw} = 37.5 / 2.35 = 15.96$ Kips $> P = 1.53$ Kips **OK**
 $\rightarrow P_{aw} = 37.5 / 2.35 = 15.96$ Kips $> V = 1.25$ Kips **OK**
 $\rightarrow P_{aw} = 37.5 / 2.35 = 15.96$ Kips $> F = 1.97$ Kips **OK**

b. Fasteners:

Allowable shear strength: $V_{af} = 0.76$ Kips (ESR-2196)
 Total allowable shear strength: $V_{aft} = n_f V_{af}$
 $\rightarrow V_{aft} = 4 \times 0.76 = 3.04$ Kips $> P = 1.97$ Kips **OK**



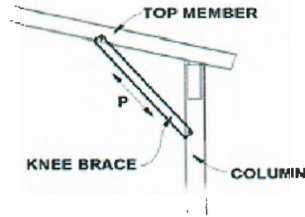
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PROJECT: MILAM AUTO		PROJECT NO.: 233-21-3409	SHEET NO.: 6/7
CLIENT: AMERICAN CARPORTS	LOCATION: PUYALLUP, WA 98371		SCALE:
CALCULATED BY: A.F.	CHECKED BY: O.A.	APPROVED BY:	
DATE: 11/6/2021	DATE: 11/6/2021	DATE:	

2. Hat-Channel - Knee Brace Connection:



A. Forces:

Max. force in double knee brace: $2P = 1.102$ Kips
 Max. force in single knee brace: $P = 0.551$ Kips

B. Dimensions & Properties:

a. Knee brace:

Channel flanges: $s_c = 4.0$ in
 Channel wall thickness: $t_c = 0.083$ in

b. Top member:

Tube side: $s_b = 2.5$ in
 Tube thickness: $t_b = 0.083$ in

c. Fasteners:

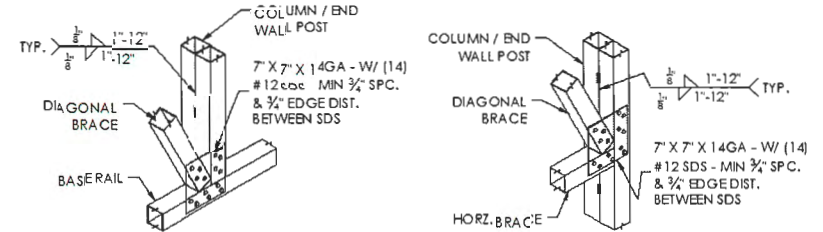
Fastener type: **HILTI Kwik Pro**
 Nominal diameter: **#12**
 Min. number of fasteners on end: $n_f = 3$

C. Component checks:

a. Fasteners in shear:

Allowable shear strength: $V_{af} = 0.76$ Kips (ESR-2196)
 Total allowable shear strength: $V_{aft} = n_f V_{af}$
 $\rightarrow V_{aft} = 3 \times 0.76 = 2.28$ Kips > 0.551 Kips **OK**

3. Diagonal Bracing Component Checks:



A. Design Forces:

Max. Axial force in diagonal brace: $P = 1.05$ Kips

B. Dimensions & Properties:

a. Brace member:

Tube side: $s_b = 2.25$ in
 Tube thickness: $t_b = 0.083$ in

b. Plate:

Clip thickness: **14GA**
 Min. Plate dimension: $l_{ps} = 7$ in


c. Fasteners:

Fastener type: **HILTI Kwik Pro**
 Nominal diameter: **#12**
 Min. number of fasteners at connection: $n_{ft} = 4$

C. Component checks:

a. Fasteners :

Allowable shear strength: $V_{af} = 0.76$ Kips (ESR-2196)
 Total allowable shear strength: $V_{aft} = n_f V_{af}$
 $\rightarrow V_{aft} = 4 \times 0.76 = 3.04$ Kips > 1.05 Kips **OK**

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	CLIENT: AMERICAN CARPORTS		LOCATION: PUYALLUP, WA 98371		
	CALCULATED BY: A.F.		CHECKED BY: O.A.	APPROVED BY:	SCALE:
	DATE: 11/6/2021		DATE: 11/6/2021	DATE:	

IX. Foundation Calculations:

1. Anchor Reactions:

Max. Uplift reaction:	$F_{up} = 1.53$ Kips
Total uplift @ side wall:	$F_{up,1} = 5.881$ Kips
Total uplift @ end wall:	$F_{up,2} = 3.072$ Kips
Max downwards reaction:	$F_{dn} = 3.36$ Kips
Max. lateral reaction:	$F_{lt} = 1.25$ Kips
Total lateral @ Side -wall:	$F_{lt,1} = 4.229$ Kips
Total lateral @ End -wall:	$F_{lt,2} = 3.243$ Kips

2. Home Pride HP 9 (Barbed Anchor):

Note: See appendices for specs.

Anchor Model:

Anchor dimensions:

HP 9
(4) 2" × 3/4" × 1/8" tabs
3/4 in diameter
22 in long shaft

Anchor testing load (See Report No: LO28906): **4725 lbf**

Allowable uplift capacity: $Q_u = S.F. \times n \times 4.725$ Kips

Total no. of Anchors at side wall: $n = 5$

Allowable uplift check:

$\rightarrow Q_u = 5 \times 2/3 \times 4.725 = 15.75$ Kips $> F_{up,1} = 5.881$ Kips **OK**

Total no. of Anchors at end wall: $n = 5$

Allowable uplift check:

$Q_u = 5 \times 2/3 \times 4.725 = 15.75$ Kips $> F_{up,2} = 3.072$ Kips **OK**

3. Soil Bearing Check

Dead Load total (DL) Statics Check sheet SO 10, 11 / 25: **2.1** Kips

Live Load total (LL) Statics Check sheet SO 10, 11 / 25: **21.0** Kips

Total Load applied at base to soil (DL + LL): **23.1** Kips

Length of base rail each side: **35 ft**

Total base rail under bearing: 2×35 ft = **70 ft**

Effective base rail width:	3×2.5 in = 7.5 in
Effective base rail surface area:	70 ft \times 7.5 in = 43.75 sq.ft
Pressure applied to soil at base rail:	23.1 Kips / 43.75 sq.ft = 528.00 psf
Allowed Soil bearing capacity:	1500 psf
Soil Bearing Check:	1500 psf > 528.00 psf OK



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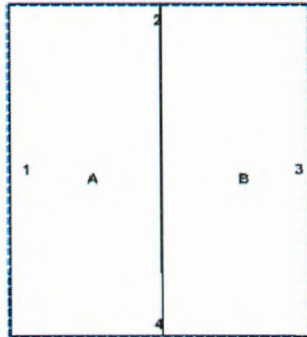
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PROJECT:		PROJECT NO.:	SHEET NO.:
CLIENT:	LOCATION:		
CALCULATED BY:	CHECKED BY:	APPROVED BY:	SCALE:
DATE:	DATE:	DATE:	

APPENDICES

Project Number: 233-21-3409
 Customer Name: AMERICAN CARPORTS, INC
 Address: 608 RIVER RD, PUYALLUP, WA, 98371
 Prepared By: AF
 Project Comments: 30' X 35' X 14'



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These calculations are based on the ASCE 7-98, ASCE 7-02, ASCE 7-05, ASCE 7-10, and ASCE7-16 Standard Practices for determining the minimum wind loads, and intended performance goals, for buildings, other structures and their nonstructural components that are subject to building code requirements. While the information presented by this program is believed to be correct, SDG, Inc. neither intends for this program to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such a professional in interpreting and applying the results of the report provided by the WLS 2019 program.

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Signature / Stamp

Input

Site Information

Wind Dir.	Exposure
1	C
2	C
3	C
4	C

Basic Wind Speed: 110 mph
 Topography: Flat

Optional Factors

This project uses load combinations from ASCE 7.

Structure Information

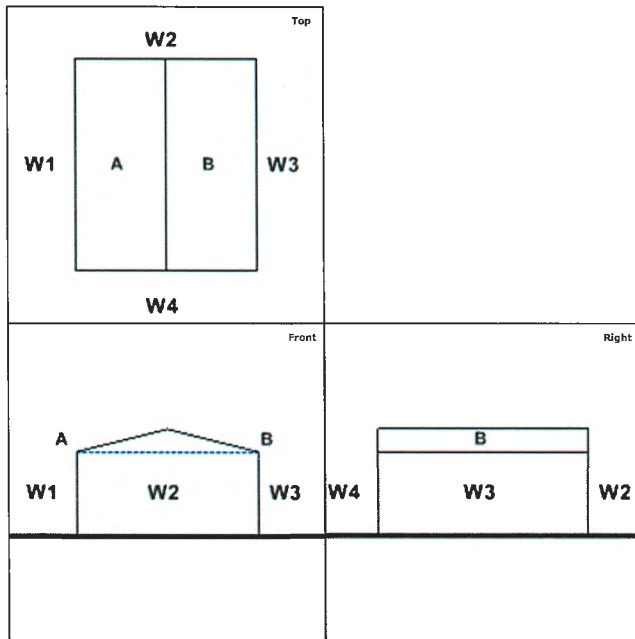
Structure Type: Building
 Structure Category: 1
 Enclosure Classification: Enclosed

Main Section

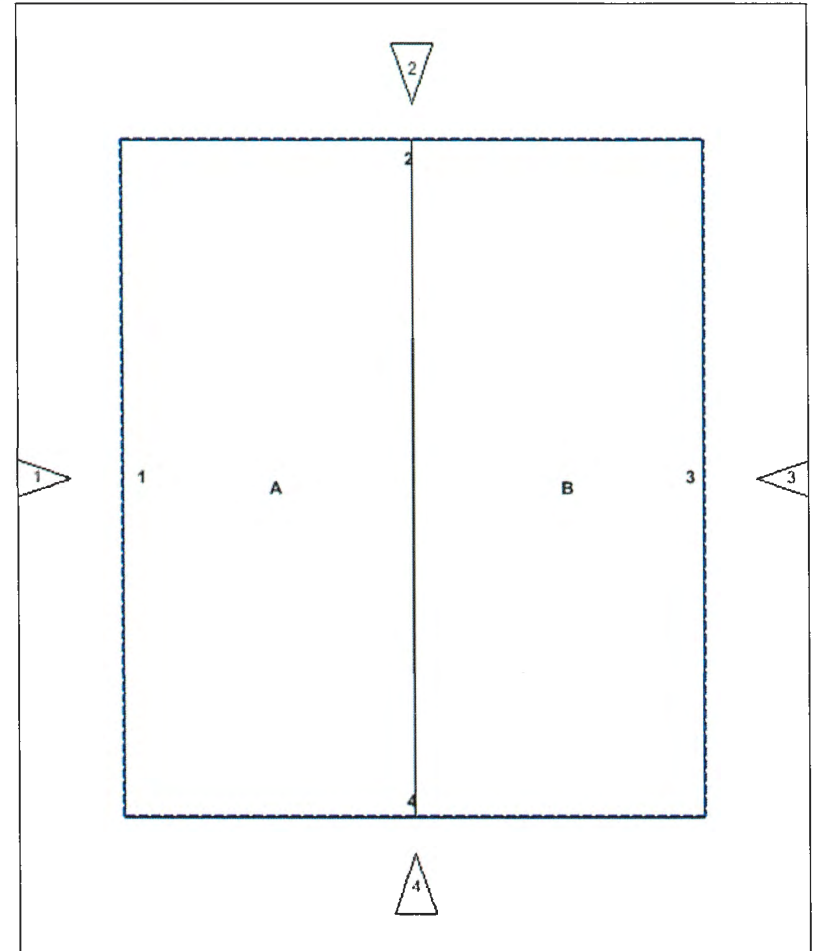
Wall	Length (ft)	Overhang (ft)
1	35.0	0.00
2	30.0	0.00
3	35.0	0.00
4	30.0	0.00

Eave Height: 14.0 ft
 Parapet Height: 0.00 ft
 Parapet Enclosure: Solid
 Roof Shape: Gabled

Roof	Slope (X:12)
A & B	3.00



Composite Drawing



MILAM AUTO

ASCE 7-16 Wind Load Report - Wind Direction 1

November 06, 2021

This data was calculated using the building of all heights method.

#	Surface	z (ft)	q (psf)	G	Cp	GCpl	Ext Pres (psf)	Net w/ +GCpl (psf)	Net w/ -GCpl (psf)
1	Windward Wall	14.0	22.3	0.886	0.800	0.180	15.8	11.8	19.9
2	Side Wall	15.9	22.6	0.886	-0.700	0.180	-14.0	-18.1	-9.96
3	Leeward Wall	15.9	22.6	0.886	-0.500	0.180	-10.0	-14.1	-5.95
4	Side Wall	15.9	22.6	0.886	-0.700	0.180	-14.0	-18.1	-9.96
A	Windward Roof	15.9	22.6	0.886	-0.180	0.180	-3.61	-7.68	0.462
		15.9	22.6		-0.755		-15.1	-19.2	-11.1
B	Leeward Roof	15.9	22.6	0.886	-0.507	0.180	-10.2	-14.2	-6.09

This is load case 1 in ASCE 7-16 Figure 27.3-8. See Figure 27.3-8 for other cases.

MILAM AUTO

ASCE 7-16 Wind Load Report - Wind Direction 2

November 06, 2021

This data was calculated using the building of all heights method.

#	Surface	z (ft)	q (psf)	G	Cp	GCpl	Ext Pres (psf)	Net w/ +GCpl (psf)	Net w/ -GCpl (psf)
1	Side Wall	15.9	22.6	0.889	-0.700	0.180	-14.1	-18.1	-9.99
2	Windward Wall	15.0	22.3	0.889	0.800	0.180	15.9	11.8	19.9
		15.9	22.6				16.1	12.0	20.1
		17.7	23.1				16.5	12.4	20.5
3	Side Wall	15.9	22.6	0.889	-0.700	0.180	-14.1	-18.1	-9.99
4	Leeward Wall	15.9	22.6	0.889	-0.467	0.180	-9.37	-13.4	-5.31
A&B	Roof	0.00 - 7.94*	22.6	0.889	-0.900	0.180	-18.1	-22.1	-14.0
		7.94 - 15.9*	22.6				-18.1	-22.1	-14.0
		15.9 - 31.7*	22.6		-0.500		-10.0	-14.1	-5.97
		31.7 - 35.0*	22.6		-0.300		-6.03	-10.1	-1.96
		0.00 - 35.0*	22.6		-0.180		-3.62	-7.68	0.453

This is load case 1 in ASCE 7-16 Figure 27.3-8. See Figure 27.3-8 for other cases.
 * Distance from windward edge.

MILAM AUTO

ASCE 7-16 Wind Load Report - Wind Direction 3

November 06, 2021

This data was calculated using the building of all heights method.

#	Surface	z (ft)	q (psf)	G	Cp	GCpi	Ext Pres (psf)	Net w/ +GCpi (psf)	Net w/ -GCpi (psf)
1	Leeward Wall	15.9	22.6	0.886	-0.500	0.180	-10.0	-14.1	-5.95
2	Side Wall	15.9	22.6	0.886	-0.700	0.180	-14.0	-18.1	-9.96
3	Windward Wall	14.0	22.3	0.886	0.800	0.180	15.8	11.8	19.9
4	Side Wall	15.9	22.6	0.886	-0.700	0.180	-14.0	-18.1	-9.96
B	Windward Roof	15.9	22.6	0.886	-0.180	0.180	-3.61	-7.68	0.462
		15.9	22.6		-0.755		-15.1	-19.2	-11.1
A	Leeward Roof	15.9	22.6	0.886	-0.507	0.180	-10.2	-14.2	-6.09

This is load case 1 in ASCE 7-16 Figure 27.3-8. See Figure 27.3-8 for other cases.

MILAM AUTO

ASCE 7-16 Wind Load Report - Wind Direction 4

November 06, 2021

This data was calculated using the building of all heights method.

#	Surface	z (ft)	q (psf)	G	Cp	GCpi	Ext Pres (psf)	Net w/ +GCpi (psf)	Net w/ -GCpi (psf)
1	Side Wall	15.9	22.6	0.889	-0.700	0.180	-14.1	-18.1	-9.99
2	Leeward Wall	15.9	22.6	0.889	-0.467	0.180	-9.37	-13.4	-5.31
3	Side Wall	15.9	22.6	0.889	-0.700	0.180	-14.1	-18.1	-9.99
4	Windward Wall	15.0	22.3	0.889	0.800	0.180	15.9	11.8	19.9
		15.9	22.6				16.1	12.0	20.1
		17.7	23.1				16.5	12.4	20.5
A&B	Roof	0.00 - 7.94*	22.6	0.889	-0.900	0.180	-18.1	-22.1	-14.0
		7.94 - 15.9*	22.6				-18.1	-22.1	-14.0
		15.9 - 31.7*	22.6		-0.500		-10.0	-14.1	-5.97
		31.7 - 35.0*	22.6		-0.300		-6.03	-10.1	-1.96
		0.00 - 35.0*	22.6		-0.180		-3.62	-7.68	0.453

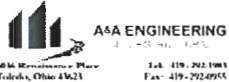
This is load case 1 in ASCE 7-16 Figure 27.3-8. See Figure 27.3-8 for other cases.
 * Distance from windward edge.

AAD SPACE
 ART JOB INFORMATION
 ENGINEER DATE 06-Nov-21
 B NAME MILAM AUTO
 B CLIENT AMERICAN CARPORTS, INC
 B NO 233-21-3409
 ENGINEER NAME AF
 CHECKER NAME OA
 CHECKER DATE 06-Nov-21
 D JOB INFORMATION
 PUT WIDTH 79
 IT INCHES KIP
 T FLOOR LOAD TOLERANCE 0.01
 INT COORDINATES
 0 0 -420; 2 0 0 -300; 3 0 0 -180; 4 0 0 -60; 5 0 0 0; 6 102.9 0 -420;
 180 0 0; 8 205.7 0 -420; 9 308.5 0 -420; 10 360 0 -420; 11 360 0 -300;
 360 0 -180; 13 360 0 -60; 14 360 0 0; 15 0 0 -360; 16 0 0 -240; 17 0 0 -120;
 MEMBER INCIDENCES
 100 1; 2 101 100; 3 102 101; 4 103 102; 5 15 103; 6 105 15; 7 106 105;
 107 106; 9 108 107; 10 2 108; 11 109 2; 12 110 109; 13 111 110; 14 112 111;
 16 112; 16 114 16; 17 115 114; 18 116 115; 19 117 116; 20 3 117; 21 118 3;
 ELEMENT INCIDENCES SHELL
 50 1 100 132 131; 2651 100 101 133 132; 2652 101 102 134 133;
 53 102 103 135 134; 2654 103 15 136 135; 2655 15 105 137 136;
 56 105 106 138 137; 2657 106 107 139 138; 2658 107 108 140 139;
 ART USER TABLE
 BLE 1
 IT INCHES KIP
 BE
 252512GA
 04248 2.5 2.5 0.109 0.995349 0.995349 1.48993 0.545 0.545
 252514GA
 802444 2.5 2.5 0.083 0.782219 0.782219 1.17195 0.415 0.415
 222212GA
 933476 2.25 2.25 0.109 0.715006 0.715006 1.06974 0.4905 0.4905
 222214GA
 719444 2.25 2.25 0.083 0.563898 0.563898 0.844607 0.3735 0.3735
 252512GAD12
 08496 5 2.5 0.109 5.24844 1.9907 2 1.09 1.09
 252514GAD14
 60489 5 2.5 0.083 3.59562 1.56447 2 0.83 0.83
 222212GAD12
 86695 4.5 2.25 0.109 3.79287 1.43001 1.06974 0.933476 0.933476
 222214GAD14
 43889 4.5 2.25 0.083 1.47444 1.1278 1 0.719444 0.719444
 222212GAD14
 65292 4.25 2.25 0.083 3.3358 1.279 1.06974 0.82646 0.82646
 252514GAI14
 366 2.5 2.5 0.166 1.501 1.501 2.33454 0.683 0.683
 252514GAI12
 715 2.5 2.5 0.192 1.715 1.715 2.33454 0.857 0.857
 252512GAI14
 606 2.5 2.5 0.192 1.714 1.714 2.33454 0.803 0.803
 252512GAI12
 97596 2.5 2.5 0.218 1.71036 1.71036 2.55967 1.0355 1.0355
 303014GA
 968445 3 3 0.083 1.37451 1.37451 2.0601 0.498 0.498
 202014GA
 636444 2 2 0.083 0.390541 0.390541 0.584715 0.332 0.332
 222212GAI12
 758 2.25 2.25 0.218 1.28 1.28 1.82906 0.981 0.981
 512IN2514
 73438 2.5 2.5 0.1875 1.55597 1.55597 2.31871 0.9375 0.9375
 222212GAD12I
 21378 4.5 2.25 0.166 5.90277 2.32002 5.09474 1.494 0.83
 252514GII2GAD
 73254 5 2.5 0.192 8.69836 2.86597 6.64501 1.92 0.96
 252514GAT1414
 40733 7.5 2.5 0.083 12.3772 2.34666 5.42485 1.245 0.276667
 252514GAT
 40733 7.5 2.5 0.083 12.3772 2.34666 3.51585 1.245 1.245
 MMY
 019796 0.2 0.1 0.049 6.64898e-005 1.66666e-005 2.87719e-005 0.0196 0.0098
 252514GAQ14
 63244 5 5 0.083 7.586 7.586 9.86685 0.83 0.83
 222212GAQ12
 63244 4.5 4.5 0.109 7.586 7.586 15.2 1.962 1.962
 BLE 2
 ANNEL
 252514GA
 608722 2.5 0.083 2.5 0.083 0.694277 0.409143 0.00139783 0.865402 -

0.2075 0.276667
 CS25251HGA
 0.381675 2.5 0.0516 2.5 0.0516 0.445919 0.259691 0.000338744 0.85332 -
 0.129 0.172
 PROFILE POINTS
 -1 0.0435 -1 -0.0435 1 -0.0435 1 0.0435
 STRESS LOCATIONS
 -1 0.0435 -1 -0.0435 1 -0.0435 1 0.0435
 CS5025GA14
 12.5 5 0.083 2.5 0.083 19.7476 6.51042 26.0417 1.25 0.415 8.33333
 PROFILE POINTS
 -1 0.0435 -1 -0.0435 1 -0.0435 1 0.0435
 STRESS LOCATIONS
 -1 0.0435 -1 -0.0435 1 -0.0435 1 0.0435
 TABLE 3
 GENERAL
 CH501014GA
 0.4353 1.0747 0.0747 5 0.0747 0.0706801 0.92233 0.00765685 0.113423 -
 0.368931 0.108825 0.326475 0.559575 0.160151 0.0174336 0
 PROFILE POINTS
 -2.5 -0.5484 -2.5 -0.6231 -1.9845 -0.6231 -0.9845 0.3769 0.9846 0.3769 1.9846 -0.6231 2.5 -0.6231 2.5 -0.5484 -
 0.155 -0.5484 1.0155 0.4516 -1.0154 0.4516 -2.0154 -0.5484
 STRESS LOCATIONS
 -2.5 -0.6231 2.5 -0.6231 1.0155 0.4516 -1.0154 0.4516
 CH501014GAD
 0.87073 1.6494 0.083 5 0.083 2.66206 1.84467 1 1.1451 0.737868 0.43515 -
 0.43515 0 0 0
 PROFILE POINTS
 -2.50001 2.32465 -2.50001 2.24995 -2.01541 2.24995 -1.01541 1.24996 -7e-006 1.24996 -7e-006 -1.25004 -1.01541 -
 2.50004 -2.01541 -2.25005 -2.50001 -2.25005 -2.50001 -2.32475 -1.98451 -2.32475 -0.984507 -1.32475 0.984593 -1.3-
 75 1.98459 -2.32475 2.49999 -2.32475 2.49999 -2.25005 2.49999 -2.25005 2.01549 -2.25005 1.01549 -1.25004 -7e-006 -1.25004 9.3e-07
 1.24996 1.01549 1.24996 2.01549 2.24995 2.49999 2.24995 2.49999 2.32465 1.98459 2.32465 0.984593 1.32465 -0.98-
 07 1.32465 -1.98451 2.32465
 STRESS LOCATIONS
 -2.50001 2.32465 -2.50001 -2.32475 2.49999 -2.32475 2.49999 2.32465
 C401014GA
 0.593849 4 0.087 1 0.087 1.40526 0.122165 0.00141953 0.702629 0.106602 -
 0.138396 0.301615 0.829011 0.22851 0.319817 0
 C351514GA
 0.319991 1.5 0.087 3.5 0.087 0.116858 0.316509 0.000253754 0.146933 -
 0.18086 0.116754 0.0799434 0.294492 0.178795 0.053675 1.5
 PROFILE POINTS
 -1.75 -0.795309 -0.950005 -0.795309 -0.950005 0.654692 0.949996 0.654692 0.949996 -0.795309 1.75 -0.795309 1.7-
 -0.745309 0.999996 -0.745309 0.999996 0.704691 -1 0.704691 -1 -0.745309 -1.75 -0.745309 -1.75 -0.795309
 C401014GAD
 1.1876 1.834 0.083 4 0.083 1.92899 4.09819 2 2.10359 2.04909 1.1876 -
 1.1876 4.90669 3.36169 4 1.834
 PROFILE POINTS
 0 -5.5374 0 -5.6204 0.7183 -5.6204 0.9683 -6.5374 3.0317 -6.5374 3.2817 -5.6204 4 -5.6204 4 -5.4473 3.2817 -5.4-
 73 3.0317 -4.5303 0.9683 -4.5303 0.7183 -5.4473 0 -5.4473 0 -5.5303 0.7817 -5.5303 1.0317 -4.6133 2.9683 -4.613-
 3.2183 -5.5303 3.2183 -5.5374 2.9683 -6.4544 1.0317 -6.4544 0.7817 -5.5374
 END
 START GROUP DEFINITION
 FLOOR
 LROOF 65 66 69 70 73 74 76 77 79 TO 81 83 84 86 87 92 94 95 97 TO 104 106 -
 107 TO 114 116 TO 123 130 TO 137 145 TO 152 154 156 TO 163 165 TO 172 175 -
 176 TO 182 654 TO 665 669 TO 674 676 TO 681 683 TO 688 705 TO 710 712 TO 717 -
 719 TO 724 1850 TO 2024
 LWALL 1 TO 63 570 TO 653 1850 TO 1884
 RWALL 309 TO 371 888 TO 971 2130 TO 2164
 FWALL 37 39 41 43 45 47 49 51 53 55 57 59 61 63 66 67 70 71 74 76 80 84 86 -
 88 TO 92 94 103 113 123 125 TO 129 137 152 162 172 174 182 190 198 207 216 -
 227 242 TO 247 256 265 275 278 280 TO 284 286 289 292 296 298 301 302 305 -
 307 345 347 349 351 353 355 357 359 361 363 365 367 369 371 666 TO 668 675 -
 682 704 711 718 725 726 743 TO 746 764 TO 777 795 TO 798 815 816 823 830 -
 837 859 866 873 TO 875 1170 TO 1217 2350 2351
 BWALL 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 65 68 69 72 73 75 -
 77 TO 79 81 TO 83 85 87 93 95 96 104 TO 106 114 TO 116 124 130 138 145 153 -
 154 TO 155 163 TO 165 173 175 183 191 199 200 208 209 217 TO 220 234 235 248 -
 249 257 258 266 TO 268 276 277 279 285 287 288 290 291 293 TO 295 297 299 -
 300 303 304 306 308 344 346 348 350 352 354 356 358 360 362 364 366 368 370 -
 689 TO 703 727 TO 742 747 TO 763 778 TO 794 799 TO 814 838 TO 852
 RROOF 191 TO 198 200 TO 207 209 TO 216 218 220 TO 227 235 TO 242 249 TO 256 -
 258 TO 266 268 TO 275 277 278 280 285 286 288 289 291 TO 293 295 296 298 -
 299 302 303 306 307 817 TO 822 824 TO 829 831 TO 836 853 TO 858 860 TO 865 -
 867 TO 872 876 TO 887 1990 TO 2164
 JOINT
 END GROUP DEFINITION
 ELEMENT PROPERTY
 2650 TO 5545 THICKNESS 0.0135
 DEFINE MATERIAL START

TO 371 UPTABLE 1 TS252514GA
 0 TO 971 UPTABLE 1 TS252514GAD14
 70 TO 1217 UPTABLE 1 TS252514GAQ14
 00 TO 1435 UPTABLE 1 TS222214GA
 50 TO 1661 UPTABLE 3 C401014GAD
 50 TO 2164 UPTABLE 3 C351514GA
 50 2351 UPTABLE 1 DUMMY
 INSTANTS
 TA 270 MEMB 1650 TO 1661
 TA 346 MEMB 1850 TO 1989
 TA 14 MEMB 2025 TO 2164
 TA 90 MEMB 666 TO 668 675 682 689 TO 704 711 718 725 TO 816 823 830 837 -
 8 TO 852 859 866 873 TO 875
 TERIAL STEEL ALL
 PPORTS
 TO 14 PINNED
 TO 23 FIXED BUT FY MX MY MZ
 MBER RELEASE
 24 START MX MY MZ
 IT FEET POUND
 AD 1 LOADTYPE Dead TITLE DEAD
 OOR LOAD
 ROOF FLOAD -1.94 GY INCLINED
 ROOF FLOAD -1.94 GY INCLINED
 AD 2 LOADTYPE Roof Live TITLE ROOFLIVE
 OOR LOAD
 ROOF FLOAD -19.4 GY INCLINED
 ROOF FLOAD -19.4 GY INCLINED
 AD 3 LOADTYPE Wind TITLE WINDXP
 OOR LOAD
 WALL FLOAD 11.8 GX
 WALL FLOAD 14.1 GX
 WALL FLOAD 18.1 GZ
 WALL FLOAD -18.1 GZ
 ROOF FLOAD 18.63 GY INCLINED
 ROOF FLOAD -4.66 GX INCLINED
 ROOF FLOAD 13.78 GY INCLINED
 ROOF FLOAD 3.44 GX INCLINED
 AD 4 LOADTYPE Wind TITLE WINDXN
 OOR LOAD
 WALL FLOAD 19.9 GX
 WALL FLOAD 5.95 GX
 WALL FLOAD 9.96 GZ
 WALL FLOAD -9.96 GZ
 ROOF FLOAD 10.77 GY INCLINED
 ROOF FLOAD -2.69 GX INCLINED
 ROOF FLOAD 5.91 GY INCLINED
 ROOF FLOAD 1.48 GX INCLINED
 AD 5 LOADTYPE Wind TITLE WINDZP
 OOR LOAD
 WALL FLOAD 12.07 GZ
 WALL FLOAD 13.4 GZ
 WALL FLOAD -18.1 GX
 WALL FLOAD 18.1 GX
 ROOF FLOAD 16.56 GY INCLINED
 ROOF FLOAD 16.56 GY INCLINED
 ROOF FLOAD -4.14 GX INCLINED
 ROOF FLOAD 4.14 GX INCLINED
 AD 6 LOADTYPE Wind TITLE WINDZN
 OOR LOAD
 WALL FLOAD 20.17 GZ
 WALL FLOAD 5.31 GZ
 WALL FLOAD -9.99 GX
 WALL FLOAD 9.99 GX
 ROOF FLOAD 8.69 GY INCLINED
 ROOF FLOAD 8.69 GY INCLINED
 ROOF FLOAD -2.17 GX INCLINED
 ROOF FLOAD 2.17 GX INCLINED
 AD 7 LOADTYPE Seismic TITLE EXP
 INT LOAD
 9 634 766 890 2282 2406 3022 3057 FX 82.51
 AD 8 LOADTYPE Seismic TITLE EXN
 INT LOAD
 9 634 766 890 2282 2406 3022 3057 FX -82.51
 AD 9 LOADTYPE Seismic TITLE EZP
 INT LOAD
 9 604 624 629 3022 3027 3052 3057 FZ 82.51
 AD 10 LOADTYPE Seismic TITLE EZN
 INT LOAD
 9 604 624 629 3022 3027 3052 3057 FZ -82.51
 TRESS COMBINATIONS

LOAD COMB 11 D+LR
 1 1.0 2 1.0
 LOAD COMB 12 D+0.6WXP
 1 1.0 3 0.6
 LOAD COMB 13 D+0.6WXN
 1 1.0 4 0.6
 LOAD COMB 14 D+0.6WZP
 1 1.0 5 0.6
 LOAD COMB 15 D+0.6WZN
 1 1.0 6 0.6
 LOAD COMB 16 D+0.75LR+0.45WXP
 1 1.0 2 0.75 3 0.45
 LOAD COMB 17 D+0.75LR+0.45WXN
 1 1.0 2 0.75 4 0.45
 LOAD COMB 18 D+0.75LR+0.45WZP
 1 1.0 2 0.75 5 0.45
 LOAD COMB 19 D+0.75LR+0.45WZN
 1 1.0 2 0.75 6 0.45
 LOAD COMB 20 0.6D+0.6WXP
 1 0.6 3 0.6
 LOAD COMB 21 0.6D+0.6WXN
 1 0.6 4 0.6
 LOAD COMB 22 0.6D+0.6WZP
 1 0.6 5 0.6
 LOAD COMB 23 0.6D+0.6WZN
 1 0.6 6 0.6
 LOAD COMB 24 D+0.7EXP *(OS FACTOR=2)
 1 1.0 7 1.4
 LOAD COMB 25 D+0.7EXN *(OS FACTOR=2)
 1 1.0 8 1.4
 LOAD COMB 26 D+0.7EZP *(OS FACTOR=2)
 1 1.0 9 1.4
 LOAD COMB 27 D+0.7EZN *(OS FACTOR=2)
 1 1.0 10 1.4
 LOAD COMB 28 D+0.75LR+0.53EXP
 1 1.0 2 0.75 7 0.53
 LOAD COMB 29 D+0.75LR+0.53EXN
 1 1.0 2 0.75 8 0.53
 LOAD COMB 30 D+0.75LR+0.53EZP
 1 1.0 2 0.75 9 0.53
 LOAD COMB 31 D+0.75LR+0.53EZN
 1 1.0 2 0.75 10 0.53
 LOAD COMB 32 0.6D+0.7EXP *(OS FACTOR=2)
 1 0.6 7 1.4
 LOAD COMB 33 0.6D+0.7EXN *(OS FACTOR=2)
 1 0.6 8 1.4
 LOAD COMB 34 0.6D+0.7EZP *(OS FACTOR=2)
 1 0.6 9 1.4
 LOAD COMB 35 0.6D+0.7EZN *(OS FACTOR=2)
 1 0.6 10 1.4
 PERFORM ANALYSIS
 DEFINE ENVELOPE
 11 TO 35 ENVELOPE 1 TYPE STRESS
 END DEFINE ENVELOPE
 LOAD LIST 11 TO 35
 UNIT INCHES KIP
 PARAMETER 1
 CODE AISC UNIFIED 2010
 METHOD ASD
 FU 65 ALL
 FYLD 50 ALL
 CHECK CODE ALL
 FINISH



Software licensed to

Job Title **MILAM AUTO**

Client **AMERICAN CARPORTS, INC**

Job No
233-21-3409

Sheet No
SO 1 /25

Rev

Part

Ref

By **AF** Date **06-Nov-21** Chd **OA**

File **233-21-3409.std**

Date/Time **06-Nov-2021 13:08**

Job Information

	Engineer	Checked	Approved
Name:	AF	OA	
Date:	06-Nov-21	06-Nov-21	

Project ID	
Project Name	

Structure Type **SPACE FRAME**

Number of Nodes	2972	Highest Node	3057
Number of Elements	1186	Highest Beam	2351
Number of Plates	2896	Highest Plate	5545

Number of Basic Load Cases	-2
Number of Combination Load Cases	25

Included in this printout are data for:

View	MAIN FRAME
------	-------------------

Included in this printout are results for load cases:

Type	L/C	Name
Combination	11	D+LR
Combination	12	D+0.6WXP
Combination	13	D+0.6WXN
Combination	14	D+0.6WZP
Combination	15	D+0.6WZN
Combination	16	D+0.75LR+0.45WXP
Combination	17	D+0.75LR+0.45WXN
Combination	18	D+0.75LR+0.45WZP
Combination	19	D+0.75LR+0.45WZN

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Job Title **MILAM AUTO**

Client **AMERICAN CARPORTS, INC**

Job No
233-21-3409

Sheet No
SO 2 / 25

Rev

Part

Ref

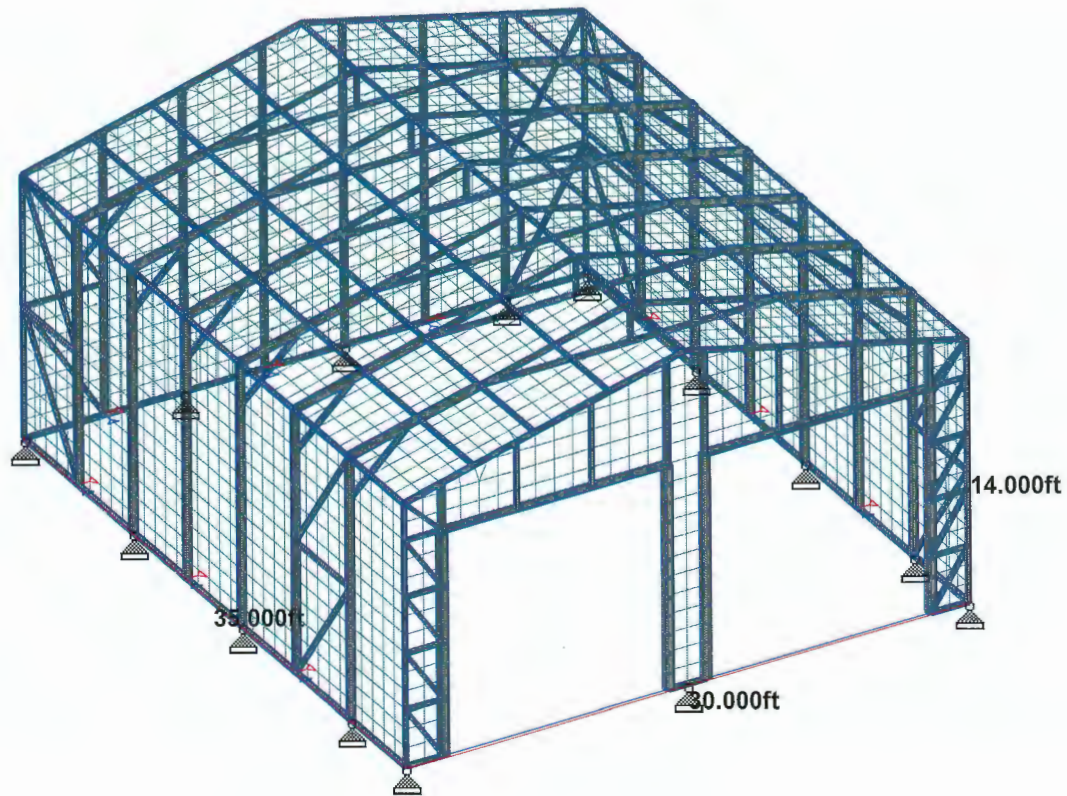
By **AF**

Date **06-Nov-21**

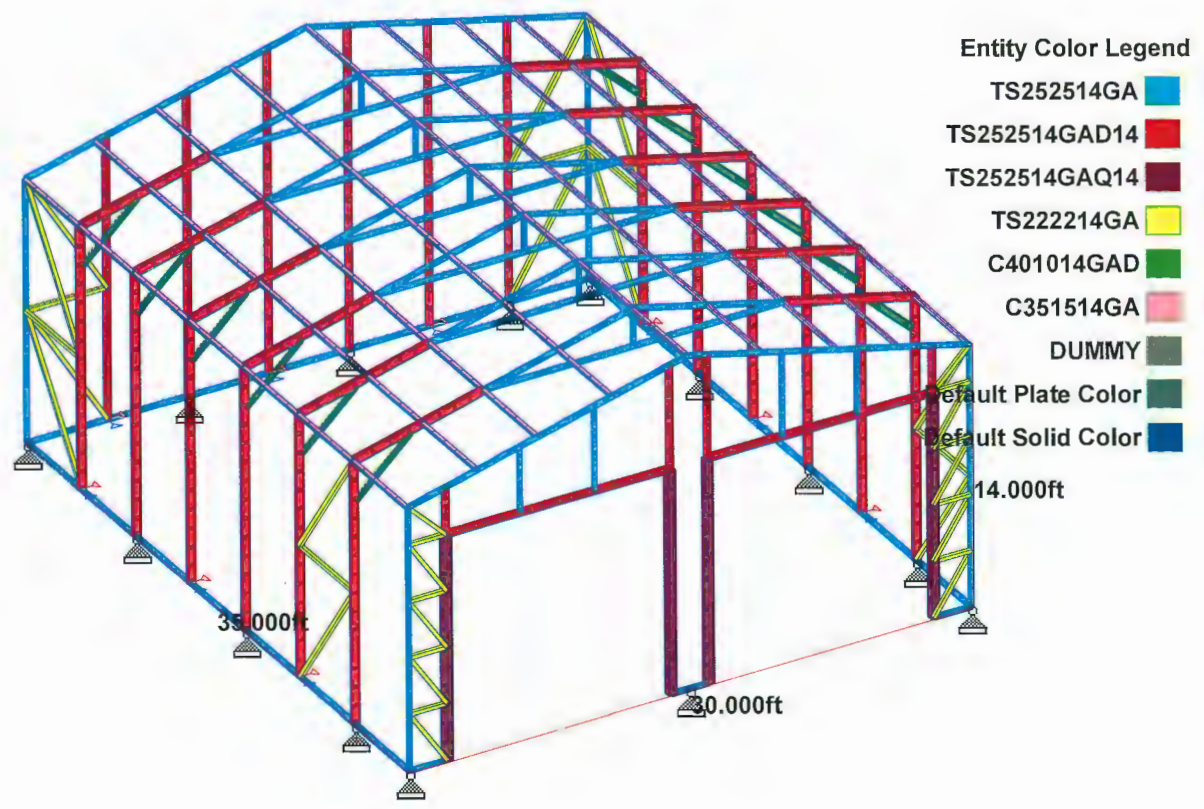
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WHOLE STRUCTURE



MAIN FRAME OVERVIEW

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Job Title **MILAM AUTO**

Part

Ref

By **AF**

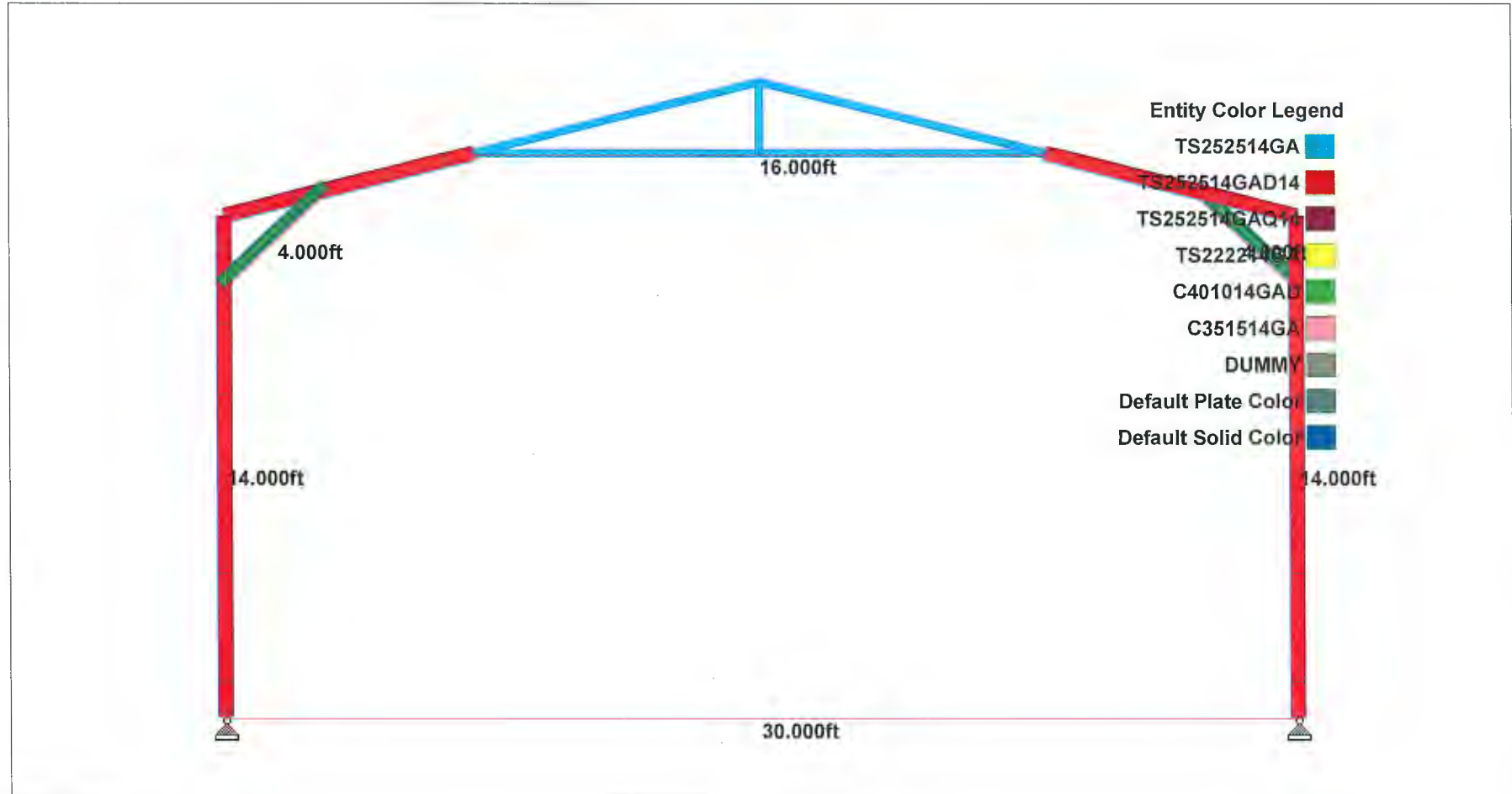
Date **06-Nov-21**

Chd **OA**

Client **AMERICAN CARPORTS, INC**

File **233-21-3409.std**

Date/Time **06-Nov-2021 13:08**



Entity Color Legend

- TS252514GA ■
- TS252514GAD14 ■
- TS252514GAQ14 ■
- TS22224000 ■
- C401014GAD ■
- C351514GA ■
- DUMMY ■
- Default Plate Color ■
- Default Solid Color ■

MAIN FRAME

Software licensed to

Job Title **MILAM AUTO**

Client **AMERICAN CARPORTS, INC**

Job No
233-21-3409

Sheet No
SO 5 /25

Rev

Part

Ref

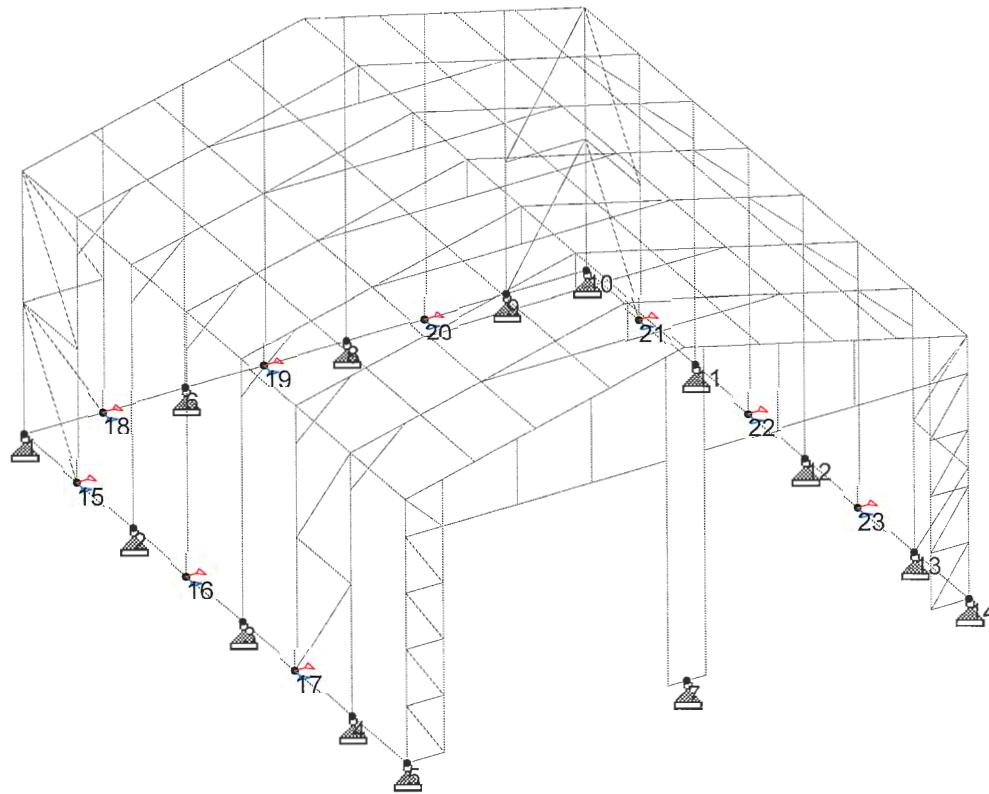
By **AF**

Date **06-Nov-21**

Chd **OA**

File **233-21-3409.std**

Date/Time **06-Nov-2021 13:08**



SUPPORTS WITH NUMBERS

 A+A ENGINEERING 1100 S. GARDNER ST. TULSA, OK 74106 Software licensed to:	Job No	Sheet No	Rev
	233-21-3409	SO 6 /25	
	Part		
	Ref		
Job Title MILAM AUTO	By AF	Date 06-Nov-21	Chd OA
Client AMERICAN CARPORTS, INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Section Properties

Prop	Section	Area (in ²)	I _{yy} (in ⁴)	I _{zz} (in ⁴)	J (in ⁶)	Material
2	TS252514GA	0.802	0.782	0.782	1.172	STEEL
3	TS252514GAD14	1.605	1.564	3.596	2.000	STEEL
4	TS252514GAQ14	1.632	7.586	7.586	9.867	STEEL
5	TS222214GA	0.719	0.564	0.564	0.845	STEEL
6	C401014GAD	1.188	4.098	1.929	2.000	STEEL
7	C351514GA	0.320	0.317	0.117	0.000	STEEL
8	DUMMY	0.020	0.000	0.000	0.000	STEEL

Plate Thickness


Prop	Node A (in)	Node B (in)	Node C (in)	Node D (in)	Material
1	0.014	0.014	0.014	0.014	STEEL

Materials

Mat	Name	E (kip/in ²)	ν	Density (kip/in ³)	α (/°F)
1	STEEL	29E+3	0.300	0.000	6.5E-6
2	STAINLESSSTEEL	28E+3	0.300	0.000	10E-6
3	ALUMINUM	10E+3	0.330	0.000	13E-6
4	CONCRETE	3.15E+3	0.170	0.000	5E-6


Primary Load Cases

Number	Name	Type
1	DEAD	Dead
2	ROOFLIVE	Roof Live
3	WINDXP	Wind
4	WINDXN	Wind
5	WINDZP	Wind
6	WINDZN	Wind
7	EXP	Seismic
8	EXN	Seismic
9	EZP	Seismic
10	EZN	Seismic

 A+A ENGINEERING 1100 S. GARDNER ST. TULSA, OK 74106 Software licensed to:	Job No	Sheet No	Rev
	233-21-3409	SO 7 /25	
	Part		
	Ref		
Job Title MILAM AUTO	By AF	Date 06-Nov-21	Chd OA
Client AMERICAN CARPORTS, INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Combination Load Cases

Comb.	Combination L/C Name	Primary	Primary L/C Name	Factor
11	D+LR	1	DEAD	1.00
		2	ROOFLIVE	1.00
12	D+0.6WXP	1	DEAD	1.00
		3	WINDXP	0.60
13	D+0.6WYN	1	DEAD	1.00
		4	WINDXN	0.60
14	D+0.6WZP	1	DEAD	1.00
		5	WINDZP	0.60
15	D+0.6WZN	1	DEAD	1.00
		6	WINDZN	0.60
16	D+0.75LR+0.45WXP	1	DEAD	1.00
		2	ROOFLIVE	0.75
		3	WINDXP	0.45
17	D+0.75LR+0.45WYN	1	DEAD	1.00
		2	ROOFLIVE	0.75
		4	WINDXN	0.45
18	D+0.75LR+0.45WZP	1	DEAD	1.00
		2	ROOFLIVE	0.75
		5	WINDZP	0.45
19	D+0.75LR+0.45WZN	1	DEAD	1.00
		2	ROOFLIVE	0.75
		6	WINDZN	0.45
20	0.6D+0.6WXP	1	DEAD	0.60
		3	WINDXP	0.60
21	0.6D+0.6WYN	1	DEAD	0.60
		4	WINDXN	0.60
22	0.6D+0.6WZP	1	DEAD	0.60
		5	WINDZP	0.60
23	0.6D+0.6WZN	1	DEAD	0.60
		6	WINDZN	0.60
24	D+0.7EXP *(OS FACTOR=2)	1	DEAD	1.00
		7	EXP	1.40
25	D+0.7EXN *(OS FACTOR=2)	1	DEAD	1.00
		8	EXN	1.40
26	D+0.7EZP *(OS FACTOR=2)	1	DEAD	1.00
		9	EZP	1.40
27	D+0.7EZN *(OS FACTOR=2)	1	DEAD	1.00
		10	EZN	1.40
28	D+0.75LR+0.53EXP	1	DEAD	1.00
		2	ROOFLIVE	0.75
		7	EXP	0.53
29	D+0.75LR+0.53EXN	1	DEAD	1.00
		2	ROOFLIVE	0.75
		8	EXN	0.53
30	D+0.75LR+0.53EZP	1	DEAD	1.00


 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 8 /25	Rev
	Part		
	Ref		
	By AF	Date:06-Nov-21	Chd OA
Client AMERICAN CARPORTS,INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Combination Load Cases Cont...

Comb.	Combination L/C Name	Primary	Primary L/C Name	Factor
		2	ROOFLIVE	0.75
		9	EZP	0.53
31	D+0.75LR+0.53EZN	1	DEAD	1.00
		2	ROOFLIVE	0.75
		10	EZN	0.53
32	0.6D+0.7EXP *(OS FACTOR=2)	1	DEAD	0.60
		7	EXP	1.40
33	0.6D+0.7EXN *(OS FACTOR=2)	1	DEAD	0.60
		8	EXN	1.40
34	0.6D+0.7EZP *(OS FACTOR=2)	1	DEAD	0.60
		9	EZP	1.40
35	0.6D+0.7EZN *(OS FACTOR=2)	1	DEAD	0.60
		10	EZN	1.40

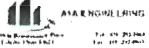
Node Displacement Summary

	Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
Max X	367	21:0.6D+0.6W	0.406	-0.001	0.002	0.406	0.000	0.000	-0.000
Min X	434	11:D+LR	-0.379	0.001	-0.003	0.379	-0.000	-0.000	-0.001
Max Y	1188	20:0.6D+0.6W	-0.041	0.466	0.001	0.467	-0.000	0.000	0.000
Min Y	1856	11:D+LR	-0.058	-0.734	-0.001	0.736	-0.002	0.000	-0.002
Max Z	1636	12:D+0.6WXP	0.031	0.001	0.563	0.564	0.000	-0.002	-0.000
Min Z	1748	12:D+0.6WXP	0.006	0.000	-0.473	0.474	-0.001	-0.001	-0.000
Max rX	1839	11:D+LR	-0.000	-0.094	0.019	0.096	0.009	-0.002	-0.000
Min rX	1894	12:D+0.6WXP	0.035	0.005	0.178	0.182	-0.010	0.002	0.000
Max rY	2391	20:0.6D+0.6W	0.000	0.010	0.249	0.249	-0.000	0.012	-0.000
Min rY	751	12:D+0.6WXP	0.000	-0.005	0.291	0.291	0.000	-0.014	-0.000
Max rZ	2306	11:D+LR	0.050	-0.267	0.002	0.291	-0.000	-0.000	0.007
Min rZ	16	21:0.6D+0.6W	0.000	-0.001	0.000	0.001	0.000	0.000	-0.008
Max Rst	1856	11:D+LR	-0.058	-0.734	-0.001	0.736	-0.002	0.000	-0.002

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 9 /25	Rev
	Part		
	Ref		
	By AF	Date:06-Nov-21	Chd OA
Client AMERICAN CARPORTS,INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Beam Maximum Forces by Section Property

Section		Axial	Shear		Torsion	Bending	
		Max Fx (kip)	Max Fy (kip)	Max Fz (kip)	Max Mx (kip'in)	Max My (kip'in)	Max Mz (kip'in)
TS252514GA	Max +ve	3.236	0.597	0.764	4.625	10.757	8.828
	Max -ve	-4.445	-0.594	-0.772	-2.460	-11.030	-5.572
TS252514GAD14	Max +ve	1.715	0.487	0.390	3.216	3.209	18.695
	Max -ve	-2.760	-0.487	-0.397	-3.411	-3.131	-16.151
TS252514GAQ14	Max +ve	1.591	0.592	0.102	5.726	0.965	4.601
	Max -ve	-1.665	-0.602	-0.368	-7.922	-23.358	-4.384
TS222214GA	Max +ve	0.646	0.015	0.200	1.230	5.035	0.210
	Max -ve	-1.050	-0.018	-0.114	-0.985	-2.902	-0.330
C401014GAD	Max +ve	0.989	0.057	0.157	0.395	3.941	1.750
	Max -ve	-1.102	-0.053	-0.145	-0.374	-6.327	-1.759
C351514GA	Max +ve	3.355	0.234	0.129	0.000	0.948	2.590
	Max -ve	-1.738	-0.260	-0.122	-0.000	-1.281	-3.168


 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 10 /25	Rev
	Part		
Ref			
By AF Date:06-Nov-21 Chd OA			
Client AMERICAN CARPORTS,INC	File 233-21-3409...std	Date/Time 06-Nov-2021 13:10	

Reaction Summary

	Node	L/C	Horizontal FX (kip)	Vertical FY (kip)	Horizontal FZ (kip)	Moment MX (kip'in) MY (kip'in) MZ (kip'in)		
Max FX	17	18:D+0.75LR+	0.479	0.000	-0.186	0.000	0.000	0.000
Min FX	4	20:0.6D+0.6W	-0.775	-0.955	0.054	0.000	0.000	0.000
Max FY	12	11:D+LR	-0.464	3.365	-0.002	0.000	0.000	0.000
Min FY	3	20:0.6D+0.6W	-0.538	-1.525	0.013	0.000	0.000	0.000
Max FZ	19	12:D+0.6WXP	-0.116	0.000	0.389	0.000	0.000	0.000
Min FZ	7	12:D+0.6WXP	-0.690	-0.502	-1.246	0.000	0.000	0.000
Max MX	1	11:D+LR	0.008	0.664	0.046	0.000	0.000	0.000
Min MX	1	11:D+LR	0.008	0.664	0.046	0.000	0.000	0.000
Max MY	1	11:D+LR	0.008	0.664	0.046	0.000	0.000	0.000
Min MY	1	11:D+LR	0.008	0.664	0.046	0.000	0.000	0.000
Max MZ	1	11:D+LR	0.008	0.664	0.046	0.000	0.000	0.000
Min MZ	1	11:D+LR	0.008	0.664	0.046	0.000	0.000	0.000

Statics Check Results

L/C		FX (kip)	FY (kip)	FZ (kip)	MX (kip'in)	MY (kip'in)	MZ (kip'in)
1:DEAD	Loads	0.000	-2.100	0.000	-440.935	-0.000	-377.944
1:DEAD	Reactions	-0.000	2.100	0.000	440.935	0.000	377.944
	Difference	-0.000	-0.000	0.000	-0.000	0.000	0.000
2:ROOFLIVE	Loads	-0.000	-20.997	0.000	-4.41E+3	0.000	-3.78E+3
2:ROOFLIVE	Reactions	0.000	20.997	0.000	4.41E+3	-0.000	3.78E+3
	Difference	-0.000	-0.000	0.000	-0.000	0.000	0.000
3:WINDXP	Loads	12.031	17.539	-0.000	3.69E+3	-2.53E+3	1.98E+3
3:WINDXP	Reactions	-12.031	-17.539	0.000	-3.69E+3	2.53E+3	-1.98E+3
	Difference	-0.000	0.000	-0.000	-0.000	-0.000	-0.000
4:WINDXN	Loads	12.012	9.027	-0.000	1.9E+3	-2.52E+3	448.822
4:WINDXN	Reactions	-12.012	-9.027	0.000	-1.9E+3	2.52E+3	-448.822
	Difference	-0.000	0.000	-0.000	-0.000	0.000	-0.000
5:WINDZP	Loads	-0.000	17.923	12.130	4.92E+3	-2.18E+3	3.23E+3
5:WINDZP	Reactions	0.000	-17.923	-12.130	-4.92E+3	2.18E+3	-3.23E+3
	Difference	0.000	0.000	0.000	-0.000	-0.000	-0.000
6:WINDZN	Loads	-0.000	9.405	12.135	3.13E+3	-2.18E+3	1.69E+3
6:WINDZN	Reactions	0.000	-9.405	-12.135	-3.13E+3	2.18E+3	-1.69E+3
	Difference	0.000	0.000	0.000	-0.000	-0.000	-0.000
7:EXP	Loads	0.660	0.000	0.000	0.000	-138.617	-114.008
7:EXP	Reactions	-0.660	0.000	-0.000	0.000	138.617	114.008
	Difference	-0.000	0.000	-0.000	0.000	-0.000	-0.000
8:EXN	Loads	-0.660	0.000	0.000	0.000	138.617	114.008
8:EXN	Reactions	0.660	-0.000	0.000	-0.000	-138.617	-114.008
	Difference	0.000	-0.000	0.000	-0.000	0.000	0.000


 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 11 /25	Rev
	Part		
Ref			
By AF Date:06-Nov-21 Chd OA			
Client AMERICAN CARPORTS,INC	File 233-21-3409...std	Date/Time 06-Nov-2021 13:10	

Statics Check Results Cont...

L/C		FX (kip)	FY (kip)	FZ (kip)	MX (kip'in)	MY (kip'in)	MZ (kip'in)
9:EZP	Reactions	0.000	0.000	-0.660	-110.893	118.814	0.000
	Difference	0.000	0.000	0.000	0.000	-0.000	0.000
10:EZN	Loads	0.000	0.000	-0.660	-110.893	118.814	0.000
10:EZN	Reactions	-0.000	-0.000	0.660	110.893	-118.814	-0.000
	Difference	-0.000	-0.000	-0.000	-0.000	0.000	-0.000

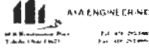
Reaction Envelope

Node	Env	Horizontal FX (kip)	Vertical FY (kip)	Horizontal FZ (kip)	MX (kip'in)	MY (kip'in)	MZ (kip'in)
1	+ve	0.168	0.664	0.141	0.000	0.000	0.000
1	+ve	Load: 14	Load: 11	Load: 12	-	-	-
1	-ve	-0.291	-0.966	-0.270	0.000	0.000	0.000
1	-ve	Load: 21	Load: 22	Load: 23	-	-	-
2	+ve	0.475	3.116	0.065	0.000	0.000	0.000
2	+ve	Load: 18	Load: 11	Load: 27	-	-	-
2	-ve	-0.586	-1.488	-0.251	0.000	0.000	0.000
2	-ve	Load: 21	Load: 20	Load: 23	-	-	-
3	+ve	0.473	3.354	0.061	0.000	0.000	0.000
3	+ve	Load: 18	Load: 11	Load: 35	-	-	-
3	-ve	-0.600	-1.525	-0.244	0.000	0.000	0.000
3	-ve	Load: 21	Load: 20	Load: 14	-	-	-
4	+ve	0.315	2.165	0.059	0.000	0.000	0.000
4	+ve	Load: 11	Load: 11	Load: 35	-	-	-
4	-ve	-0.775	-1.157	-0.249	0.000	0.000	0.000
4	-ve	Load: 20	Load: 22	Load: 19	-	-	-
5	+ve	0.403	0.836	0.037	0.000	0.000	0.000
5	+ve	Load: 14	Load: 19	Load: 35	-	-	-
5	-ve	-0.389	-1.008	-0.623	0.000	0.000	0.000
5	-ve	Load: 21	Load: 20	Load: 12	-	-	-
6	+ve	0.040	0.868	0.355	0.000	0.000	0.000
6	+ve	Load: 25	Load: 11	Load: 12	-	-	-
6	-ve	-0.144	-0.453	-0.434	0.000	0.000	0.000
6	-ve	Load: 21	Load: 22	Load: 23	-	-	-
7	+ve	0.197	1.402	0.008	0.000	0.000	0.000
7	+ve	Load: 33	Load: 11	Load: 35	-	-	-
7	-ve	-0.690	-0.553	-1.246	0.000	0.000	0.000
7	-ve	Load: 12	Load: 20	Load: 12	-	-	-
8	+ve	0.032	1.022	0.386	0.000	0.000	0.000
8	+ve	Load: 33	Load: 11	Load: 12	-	-	-
8	-ve	-0.117	-0.422	-0.460	0.000	0.000	0.000
8	-ve	Load: 12	Load: 20	Load: 23	-	-	-
9	+ve	0.045	0.555	0.333	0.000	0.000	0.000

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 12 /25	Rev
	Part		
Client AMERICAN CARPORTS,INC	Ref		
	By AF	Date 06-Nov-21	Chd OA
	File 233-21-3409...std	Date/Time 06-Nov-2021 13:10	

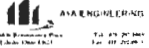
Reaction Envelope Cont...

Node	Env	Horizontal		Vertical	Moment		
		FX (kip)	FY (kip)		FZ (kip)	MX (kip'in)	MY (kip'in)
9	+ve	0.045	0.555	0.333	0.000	0.000	0.000
9	+ve	Load: 33	Load: 11	Load: 12	-	-	-
9	-ve	-0.167	-0.422	-0.458	0.000	0.000	0.000
9	-ve	Load: 13	Load: 20	Load: 23	-	-	-
10	+ve	0.028	0.870	0.143	0.000	0.000	0.000
10	+ve	Load: 25	Load: 17	Load: 12	-	-	-
10	-ve	-0.251	-0.854	-0.270	0.000	0.000	0.000
10	-ve	Load: 20	Load: 22	Load: 23	-	-	-
11	+ve	0.000	3.113	0.065	0.000	0.000	0.000
11	+ve	-	Load: 11	Load: 27	-	-	-
11	-ve	-0.487	-1.270	-0.251	0.000	0.000	0.000
11	-ve	Load: 16	Load: 22	Load: 23	-	-	-
12	+ve	0.000	3.355	0.061	0.000	0.000	0.000
12	+ve	-	Load: 11	Load: 35	-	-	-
12	-ve	-0.475	-1.381	-0.244	0.000	0.000	0.000
12	-ve	Load: 16	Load: 22	Load: 14	-	-	-
13	+ve	0.043	2.165	0.059	0.000	0.000	0.000
13	+ve	Load: 22	Load: 11	Load: 35	-	-	-
13	-ve	-0.315	-1.157	-0.249	0.000	0.000	0.000
13	-ve	Load: 11	Load: 22	Load: 19	-	-	-
14	+ve	0.076	0.836	0.037	0.000	0.000	0.000
14	+ve	Load: 33	Load: 19	Load: 35	-	-	-
14	-ve	-0.661	-0.177	-0.637	0.000	0.000	0.000
14	-ve	Load: 12	Load: 35	Load: 12	-	-	-
15	+ve	0.386	0.000	0.065	0.000	0.000	0.000
15	+ve	Load: 18	-	Load: 27	-	-	-
15	-ve	-0.568	0.000	-0.248	0.000	0.000	0.000
15	-ve	Load: 21	-	Load: 23	-	-	-
16	+ve	0.443	0.000	0.061	0.000	0.000	0.000
16	+ve	Load: 18	-	Load: 35	-	-	-
16	-ve	-0.564	0.000	-0.243	0.000	0.000	0.000
16	-ve	Load: 21	-	Load: 15	-	-	-
17	+ve	0.479	0.000	0.062	0.000	0.000	0.000
17	+ve	Load: 18	-	Load: 35	-	-	-
17	-ve	-0.501	0.000	-0.245	0.000	0.000	0.000
17	-ve	Load: 21	-	Load: 14	-	-	-
18	+ve	0.047	0.000	0.359	0.000	0.000	0.000
18	+ve	Load: 33	-	Load: 12	-	-	-
18	-ve	-0.171	0.000	-0.457	0.000	0.000	0.000
18	-ve	Load: 13	-	Load: 23	-	-	-
19	+ve	0.032	0.000	0.389	0.000	0.000	0.000
19	+ve	Load: 25	-	Load: 12	-	-	-
19	-ve	-0.116	0.000	-0.460	0.000	0.000	0.000
19	-ve	Load: 21	-	Load: 23	-	-	-

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 13 /25	Rev
	Part		
Client AMERICAN CARPORTS,INC	Ref		
	By AF	Date 06-Nov-21	Chd OA
	File 233-21-3409...std	Date/Time 06-Nov-2021 13:10	

Reaction Envelope Cont...

Node	Env	Horizontal		Vertical	Moment		
		FX (kip)	FY (kip)		FZ (kip)	MX (kip'in)	MY (kip'in)
20	+ve	0.038	0.000	0.352	0.000	0.000	0.000
20	+ve	Load: 25	-	Load: 12	-	-	-
20	-ve	-0.140	0.000	-0.434	0.000	0.000	0.000
20	-ve	Load: 20	-	Load: 23	-	-	-
21	+ve	0.020	0.000	0.065	0.000	0.000	0.000
21	+ve	Load: 33	-	Load: 27	-	-	-
21	-ve	-0.412	0.000	-0.249	0.000	0.000	0.000
21	-ve	Load: 16	-	Load: 22	-	-	-
22	+ve	0.001	0.000	0.061	0.000	0.000	0.000
22	+ve	Load: 33	-	Load: 35	-	-	-
22	-ve	-0.443	0.000	-0.243	0.000	0.000	0.000
22	-ve	Load: 16	-	Load: 15	-	-	-
23	+ve	0.000	0.000	0.062	0.000	0.000	0.000
23	+ve	-	-	Load: 35	-	-	-
23	-ve	-0.484	0.000	-0.245	0.000	0.000	0.000
23	-ve	Load: 16	-	Load: 14	-	-	-

 Software licensed to Job Title MILAM AUTO	Job No	233-21-3409	Sheet No	SO 14 /25	Rev	
	Part					
	Ref					
	By	AF	Date	06-Nov-21	Chd	OA
	Client	AMERICAN CARPORTS,INC	File	233-21-3409.std	Date/Time	06-Nov-2021 13:08

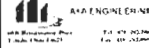
Utilization Ratio

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
1	TS252514G/	TS252514G/	0.063	1.000	0.063	Eq. H3-1	21	0.802	0.782	0.782	1.172
2	TS252514G/	TS252514G/	0.063	1.000	0.063	Eq. H3-1	21	0.802	0.782	0.782	1.172
3	TS252514G/	TS252514G/	0.063	1.000	0.063	Eq. H3-1	21	0.802	0.782	0.782	1.172
4	TS252514G/	TS252514G/	0.063	1.000	0.063	Eq. H3-1	21	0.802	0.782	0.782	1.172
5	TS252514G/	TS252514G/	0.071	1.000	0.071	Eq. H1-1b	21	0.802	0.782	0.782	1.172
6	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H1-1b	11	0.802	0.782	0.782	1.172
7	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	21	0.802	0.782	0.782	1.172
8	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	21	0.802	0.782	0.782	1.172
9	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	21	0.802	0.782	0.782	1.172
10	TS252514G/	TS252514G/	0.032	1.000	0.032	Eq. H1-1b	21	0.802	0.782	0.782	1.172
11	TS252514G/	TS252514G/	0.039	1.000	0.039	Eq. H1-1b	14	0.802	0.782	0.782	1.172
12	TS252514G/	TS252514G/	0.011	1.000	0.011	Eq. H1-1b	22	0.802	0.782	0.782	1.172
13	TS252514G/	TS252514G/	0.011	1.000	0.011	Eq. H1-1b	13	0.802	0.782	0.782	1.172
14	TS252514G/	TS252514G/	0.011	1.000	0.011	Eq. H1-1b	13	0.802	0.782	0.782	1.172
15	TS252514G/	TS252514G/	0.029	1.000	0.029	Eq. H1-1b	22	0.802	0.782	0.782	1.172
16	TS252514G/	TS252514G/	0.025	1.000	0.025	Eq. H1-1b	13	0.802	0.782	0.782	1.172
17	TS252514G/	TS252514G/	0.013	1.000	0.013	Eq. H1-1b	14	0.802	0.782	0.782	1.172
18	TS252514G/	TS252514G/	0.014	1.000	0.014	Eq. H1-1b	14	0.802	0.782	0.782	1.172
19	TS252514G/	TS252514G/	0.014	1.000	0.014	Eq. H1-1b	14	0.802	0.782	0.782	1.172
20	TS252514G/	TS252514G/	0.043	1.000	0.043	Eq. H1-1b	21	0.802	0.782	0.782	1.172
21	TS252514G/	TS252514G/	0.040	1.000	0.040	Eq. H1-1b	18	0.802	0.782	0.782	1.172
22	TS252514G/	TS252514G/	0.017	1.000	0.017	Eq. H1-1b	21	0.802	0.782	0.782	1.172
23	TS252514G/	TS252514G/	0.024	1.000	0.024	Eq. H1-1b	20	0.802	0.782	0.782	1.172
24	TS252514G/	TS252514G/	0.026	1.000	0.026	Eq. H1-1b	20	0.802	0.782	0.782	1.172
25	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H1-1b	22	0.802	0.782	0.782	1.172
26	TS252514G/	TS252514G/	0.046	1.000	0.046	Eq. H1-1b	14	0.802	0.782	0.782	1.172
27	TS252514G/	TS252514G/	0.031	1.000	0.031	Eq. H1-1b	22	0.802	0.782	0.782	1.172
28	TS252514G/	TS252514G/	0.056	1.000	0.056	Eq. H1-1b	12	0.802	0.782	0.782	1.172
29	TS252514G/	TS252514G/	0.095	1.000	0.095	Eq. H1-1b	12	0.802	0.782	0.782	1.172
30	TS252514G/	TS252514G/	0.140	1.000	0.140	Eq. H1-1b	12	0.802	0.782	0.782	1.172
31	TS252514G/	TS252514G/	0.171	1.000	0.171	Eq. H1-1b	20	0.802	0.782	0.782	1.172
32	TS252514G/	TS252514G/	0.128	1.000	0.128	Eq. H1-1b	12	0.802	0.782	0.782	1.172
33	TS252514G/	TS252514G/	0.261	1.000	0.261	Eq. H1-1b	12	0.802	0.782	0.782	1.172
34	TS252514G/	TS252514G/	0.390	1.000	0.390	Eq. H1-1b	20	0.802	0.782	0.782	1.172
35	TS252514G/	TS252514G/	0.511	1.000	0.511	Eq. H1-1b	12	0.802	0.782	0.782	1.172
36	TS252514G/	TS252514G/	0.136	1.000	0.136	Sec. E1	11	0.802	0.782	0.782	1.172
37	TS252514G/	TS252514G/	0.123	1.000	0.123	Sec. E1	11	0.802	0.782	0.782	1.172
38	TS252514G/	TS252514G/	0.116	1.000	0.116	Sec. E1	11	0.802	0.782	0.782	1.172
39	TS252514G/	TS252514G/	0.108	1.000	0.108	Sec. E1	11	0.802	0.782	0.782	1.172
40	TS252514G/	TS252514G/	0.102	1.000	0.102	Sec. E1	11	0.802	0.782	0.782	1.172
41	TS252514G/	TS252514G/	0.098	1.000	0.098	Sec. E1	11	0.802	0.782	0.782	1.172
42	TS252514G/	TS252514G/	0.093	1.000	0.093	Sec. E1	11	0.802	0.782	0.782	1.172
43	TS252514G/	TS252514G/	0.091	1.000	0.091	Sec. E1	11	0.802	0.782	0.782	1.172
44	TS252514G/	TS252514G/	0.085	1.000	0.085	Sec. E1	11	0.802	0.782	0.782	1.172
45	TS252514G/	TS252514G/	0.085	1.000	0.085	Sec. E1	11	0.802	0.782	0.782	1.172

Print Time/Date: 09/11/2021 13:14


STAAD.Pro V8i (SELECTSeries 6) 20.07.11.90

Print Run 1 of 27

 Software licensed to Job Title MILAM AUTO	Job No	233-21-3409	Sheet No	SO 15 /25	Rev	
	Part					
	Ref					
	By	AF	Date	06-Nov-21	Chd	OA
	Client	AMERICAN CARPORTS,INC	File	233-21-3409.std	Date/Time	06-Nov-2021 13:08


Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
46	TS252514G/	TS252514G/	0.080	1.000	0.080	Sec. E1	11	0.802	0.782	0.782	1.172
47	TS252514G/	TS252514G/	0.080	1.000	0.080	Sec. E1	11	0.802	0.782	0.782	1.172
48	TS252514G/	TS252514G/	0.075	1.000	0.075	Sec. E1	11	0.802	0.782	0.782	1.172
49	TS252514G/	TS252514G/	0.075	1.000	0.075	Sec. E1	11	0.802	0.782	0.782	1.172
50	TS252514G/	TS252514G/	0.078	1.000	0.078	Sec. E1	11	0.802	0.782	0.782	1.172
51	TS252514G/	TS252514G/	0.070	1.000	0.070	Sec. E1	11	0.802	0.782	0.782	1.172
52	TS252514G/	TS252514G/	0.074	1.000	0.074	Sec. E1	11	0.802	0.782	0.782	1.172
53	TS252514G/	TS252514G/	0.065	1.000	0.065	Sec. E1	11	0.802	0.782	0.782	1.172
54	TS252514G/	TS252514G/	0.070	1.000	0.070	Sec. E1	11	0.802	0.782	0.782	1.172
55	TS252514G/	TS252514G/	0.093	1.000	0.093	Eq. H3-1	12	0.802	0.782	0.782	1.172
56	TS252514G/	TS252514G/	0.065	1.000	0.065	Sec. E1	11	0.802	0.782	0.782	1.172
57	TS252514G/	TS252514G/	0.093	1.000	0.093	Eq. H3-1	12	0.802	0.782	0.782	1.172
58	TS252514G/	TS252514G/	0.058	1.000	0.058	Sec. E1	11	0.802	0.782	0.782	1.172
59	TS252514G/	TS252514G/	0.093	1.000	0.093	Eq. H3-1	12	0.802	0.782	0.782	1.172
60	TS252514G/	TS252514G/	0.047	1.000	0.047	Sec. E1	11	0.802	0.782	0.782	1.172
61	TS252514G/	TS252514G/	0.134	1.000	0.134	Eq. H3-1	12	0.802	0.782	0.782	1.172
62	TS252514G/	TS252514G/	0.085	1.000	0.085	Eq. H1-1b	20	0.802	0.782	0.782	1.172
63	TS252514G/	TS252514G/	0.138	1.000	0.138	Eq. H1-1b	12	0.802	0.782	0.782	1.172
64	TS252514G/	TS252514G/	0.060	1.000	0.060	Eq. H1-1b	21	0.802	0.782	0.782	1.172
65	TS252514G/	TS252514G/	0.083	1.000	0.083	Eq. H1-1b	20	0.802	0.782	0.782	1.172
66	TS252514G/	TS252514G/	0.139	1.000	0.139	Eq. H1-1b	12	0.802	0.782	0.782	1.172
67	TS252514G/	TS252514G/	0.492	1.000	0.492	Eq. H1-1b	20	0.802	0.782	0.782	1.172
68	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
69	TS252514G/	TS252514G/	0.047	1.000	0.047	Eq. H3-1	23	0.802	0.782	0.782	1.172
70	TS252514G/	TS252514G/	0.111	1.000	0.111	Eq. H3-1	12	0.802	0.782	0.782	1.172
71	TS252514G/	TS252514G/	0.229	1.000	0.229	Eq. H1-1b	12	0.802	0.782	0.782	1.172
72	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
73	TS252514G/	TS252514G/	0.047	1.000	0.047	Eq. H3-1	23	0.802	0.782	0.782	1.172
74	TS252514G/	TS252514G/	0.129	1.000	0.129	Eq. H1-1b	20	0.802	0.782	0.782	1.172
75	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
76	TS252514G/	TS252514G/	0.083	1.000	0.083	Eq. H3-1	11	0.802	0.782	0.782	1.172
77	TS252514G/	TS252514G/	0.047	1.000	0.047	Eq. H3-1	23	0.802	0.782	0.782	1.172
78	TS252514G/	TS252514G/	0.096	1.000	0.096	Eq. H1-1b	23	0.802	0.782	0.782	1.172
79	TS252514G/	TS252514G/	0.131	1.000	0.131	Eq. H3-1	20	0.802	0.782	0.782	1.172
80	TS252514G/	TS252514G/	0.108	1.000	0.108	Eq. H3-1	20	0.802	0.782	0.782	1.172
81	TS252514G/	TS252514G/	0.087	1.000	0.087	Eq. H1-1b	11	0.802	0.782	0.782	1.172
82	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
83	TS252514G/	TS252514G/	0.063	1.000	0.063	Eq. H1-1b	11	0.802	0.782	0.782	1.172
84	TS252514G/	TS252514G/	0.108	1.000	0.108	Eq. H3-1	20	0.802	0.782	0.782	1.172
85	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
86	TS252514G/	TS252514G/	0.108	1.000	0.108	Eq. H3-1	20	0.802	0.782	0.782	1.172
87	TS252514G/	TS252514G/	0.068	1.000	0.068	Eq. Sec. D2	11</				

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 16 /25	Rev
	Part		
Ref			
By AF Date:06-Nov-21 Chd QA			
Client AMERICAN CARPORTS,INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	


Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
91	TS252514G/	TS252514G/	0.216	1.000	0.216	Eq. H1-1b	20	0.802	0.782	0.782	1.172
92	TS252514G/	TS252514G/	0.128	1.000	0.128	Eq. H3-1	20	0.802	0.782	0.782	1.172
93	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
94	TS252514G/	TS252514G/	0.128	1.000	0.128	Eq. H3-1	20	0.802	0.782	0.782	1.172
95	TS252514G/	TS252514G/	0.082	1.000	0.082	Eq. H1-1b	11	0.802	0.782	0.782	1.172
96	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
97	TS252514G/	TS252514G/	0.149	1.000	0.149	Eq. H1-1b	11	0.802	0.782	0.782	1.172
98	TS252514G/	TS252514G/	0.131	1.000	0.131	Eq. H1-1b	17	0.802	0.782	0.782	1.172
99	TS252514G/	TS252514G/	0.120	1.000	0.120	Eq. H1-1b	20	0.802	0.782	0.782	1.172
100	TS252514G/	TS252514G/	0.117	1.000	0.117	Eq. H1-1b	20	0.802	0.782	0.782	1.172
101	TS252514G/	TS252514G/	0.131	1.000	0.131	Eq. H1-1b	17	0.802	0.782	0.782	1.172
102	TS252514G/	TS252514G/	0.147	1.000	0.147	Eq. H1-1b	11	0.802	0.782	0.782	1.172
103	TS252514G/	TS252514G/	0.071	1.000	0.071	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
104	TS252514G/	TS252514G/	0.118	1.000	0.118	Eq. H3-1	20	0.802	0.782	0.782	1.172
105	TS252514G/	TS252514G/	0.070	1.000	0.070	Eq. H1-1b	23	0.802	0.782	0.782	1.172
106	TS252514G/	TS252514G/	0.118	1.000	0.118	Eq. H3-1	20	0.802	0.782	0.782	1.172
107	TS252514G/	TS252514G/	0.132	1.000	0.132	Eq. H1-1b	11	0.802	0.782	0.782	1.172
108	TS252514G/	TS252514G/	0.165	1.000	0.165	Eq. H1-1b	11	0.802	0.782	0.782	1.172
109	TS252514G/	TS252514G/	0.179	1.000	0.179	Eq. H1-1b	11	0.802	0.782	0.782	1.172
110	TS252514G/	TS252514G/	0.179	1.000	0.179	Eq. H1-1b	11	0.802	0.782	0.782	1.172
111	TS252514G/	TS252514G/	0.166	1.000	0.166	Eq. H1-1b	11	0.802	0.782	0.782	1.172
112	TS252514G/	TS252514G/	0.133	1.000	0.133	Eq. H1-1b	11	0.802	0.782	0.782	1.172
113	TS252514G/	TS252514G/	0.090	1.000	0.090	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
114	TS252514G/	TS252514G/	0.137	1.000	0.137	Eq. H1-1b	11	0.802	0.782	0.782	1.172
115	TS252514G/	TS252514G/	0.075	1.000	0.075	Eq. H1-1b	20	0.802	0.782	0.782	1.172
116	TS252514G/	TS252514G/	0.127	1.000	0.127	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
117	TS252514G/	TS252514G/	0.195	1.000	0.195	Eq. H1-1b	11	0.802	0.782	0.782	1.172
118	TS252514G/	TS252514G/	0.230	1.000	0.230	Eq. H1-1b	11	0.802	0.782	0.782	1.172
119	TS252514G/	TS252514G/	0.236	1.000	0.236	Eq. H1-1b	11	0.802	0.782	0.782	1.172
120	TS252514G/	TS252514G/	0.235	1.000	0.235	Eq. H1-1b	11	0.802	0.782	0.782	1.172
121	TS252514G/	TS252514G/	0.231	1.000	0.231	Eq. H1-1b	11	0.802	0.782	0.782	1.172
122	TS252514G/	TS252514G/	0.196	1.000	0.196	Eq. H1-1b	11	0.802	0.782	0.782	1.172
123	TS252514G/	TS252514G/	0.113	1.000	0.113	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
124	TS252514G/	TS252514G/	0.027	1.000	0.027	Eq. H1-1b	20	0.802	0.782	0.782	1.172
125	TS252514G/	TS252514G/	0.106	1.000	0.106	Eq. H1-1b	20	0.802	0.782	0.782	1.172
126	TS252514G/	TS252514G/	0.072	1.000	0.072	Eq. H1-1b	12	0.802	0.782	0.782	1.172
127	TS252514G/	TS252514G/	0.071	1.000	0.071	Eq. H1-1b	11	0.802	0.782	0.782	1.172
128	TS252514G/	TS252514G/	0.121	1.000	0.121	Eq. H1-1b	20	0.802	0.782	0.782	1.172
129	TS252514G/	TS252514G/	0.175	1.000	0.175	Eq. H1-1b	20	0.802	0.782	0.782	1.172
130	TS252514G/	TS252514G/	0.142	1.000	0.142	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
131	TS252514G/	TS252514G/	0.259	1.000	0.259	Eq. H1-1b	11	0.802	0.782	0.782	1.172
132	TS252514G/	TS252514G/	0.283	1.000	0.283	Eq. H1-1b	11	0.802	0.782	0.782	1.172
133	TS252514G/	TS252514G/	0.272	1.000	0.272	Eq. H1-1b	11	0.802	0.782	0.782	1.172
134	TS252514G/	TS252514G/	0.271	1.000	0.271	Eq. H1-1b	11	0.802	0.782	0.782	1.172
135	TS252514G/	TS252514G/	0.284	1.000	0.284	Eq. H1-1b	11	0.802	0.782	0.782	1.172

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 17 /25	Rev
	Part		
Ref			
By AF Date:06-Nov-21 Chd QA			
Client AMERICAN CARPORTS,INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
136	TS252514G/	TS252514G/	0.260	1.000	0.260	Eq. H1-1b	11	0.802	0.782	0.782	1.172
137	TS252514G/	TS252514G/	0.132	1.000	0.132	Eq. H3-1	20	0.802	0.782	0.782	1.172
138	TS252514G/	TS252514G/	0.029	1.000	0.029	Eq. H1-1b	23	0.802	0.782	0.782	1.172
139	TS252514G/	TS252514G/	0.242	1.000	0.242	Eq. H1-1b	11	0.802	0.782	0.782	1.172
140	TS252514G/	TS252514G/	0.291	1.000	0.291	Eq. H1-1b	11	0.802	0.782	0.782	1.172
141	TS252514G/	TS252514G/	0.305	1.000	0.305	Eq. H1-1b	11	0.802	0.782	0.782	1.172
142	TS252514G/	TS252514G/	0.304	1.000	0.304	Eq. H1-1b	11	0.802	0.782	0.782	1.172
143	TS252514G/	TS252514G/	0.292	1.000	0.292	Eq. H1-1b	11	0.802	0.782	0.782	1.172
144	TS252514G/	TS252514G/	0.250	1.000	0.250	Eq. H1-1b	11	0.802	0.782	0.782	1.172
145	TS252514G/	TS252514G/	0.162	1.000	0.162	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
146	TS252514G/	TS252514G/	0.238	1.000	0.238	Eq. H1-1b	11	0.802	0.782	0.782	1.172
147	TS252514G/	TS252514G/	0.268	1.000	0.268	Eq. H1-1b	11	0.802	0.782	0.782	1.172
148	TS252514G/	TS252514G/	0.267	1.000	0.267	Eq. H1-1b	11	0.802	0.782	0.782	1.172
149	TS252514G/	TS252514G/	0.267	1.000	0.267	Eq. H1-1b	11	0.802	0.782	0.782	1.172
150	TS252514G/	TS252514G/	0.269	1.000	0.269	Eq. H1-1b	11	0.802	0.782	0.782	1.172
151	TS252514G/	TS252514G/	0.239	1.000	0.239	Eq. H1-1b	11	0.802	0.782	0.782	1.172
152	TS252514G/	TS252514G/	0.147	1.000	0.147	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
153	TS252514G/	TS252514G/	0.024	1.000	0.024	Eq. H1-1b	20	0.802	0.782	0.782	1.172
154	TS252514G/	TS252514G/	0.180	1.000	0.180	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
155	TS252514G/	TS252514G/	0.068	1.000	0.068	Eq. H1-1b	20	0.802	0.782	0.782	1.172
156	TS252514G/	TS252514G/	0.132	1.000	0.132	Eq. H1-1b	11	0.802	0.782	0.782	1.172
157	TS252514G/	TS252514G/	0.148	1.000	0.148	Eq. H1-1b	11	0.802	0.782	0.782	1.172
158	TS252514G/	TS252514G/	0.147	1.000	0.147	Eq. H1-1b	11	0.802	0.782	0.782	1.172
159	TS252514G/	TS252514G/	0.147	1.000	0.147	Eq. H1-1b	11	0.802	0.782	0.782	1.172
160	TS252514G/	TS252514G/	0.149	1.000	0.149	Eq. H1-1b	11	0.802	0.782	0.782	1.172
161	TS252514G/	TS252514G/	0.131	1.000	0.131	Eq. H1-1b	11	0.802	0.782	0.782	1.172
162	TS252514G/	TS252514G/	0.163	1.000	0.163	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
163	TS252514G/	TS252514G/	0.190	1.000	0.190	Eq. H1-1b	11	0.802	0.782	0.782	1.172
164	TS252514G/	TS252514G/	0.055	1.000	0.055	Eq. H1-1b	12	0.802	0.782	0.782	1.172
165	TS252514G/	TS252514G/	0.174	1.000	0.174	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
166	TS252514G/	TS252514G/	0.184	1.000	0.184	Eq. H1-1b	11	0.802	0.782	0.782	1.172
167	TS252514G/	TS252514G/	0.206	1.000	0.206	Eq. H1-1b	11	0.802	0.782	0.782	1.172
168	TS252514G/	TS252514G/	0.208	1.000	0.208	Eq. H1-1b	11	0.802	0.782	0.782	1.172
169	TS252514G/	TS252514G/	0.208	1.000	0.208	Eq. H1-1b	11	0.802	0.782	0.782	1.172
170	TS252514G/	TS252514G/	0.206	1.000	0.206	Eq. H1-1b	11	0.802	0.782	0.782	1.172
171	TS252514G/	TS252514G/	0.186	1.000	0.186	Eq. H1-1b	11	0.802	0.782	0.782	1.172
172	TS252514G/	TS252514G/	0.182	1.000	0.182	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
173	TS252514G/	TS252514G/	0.033	1.000	0.033	Eq. H1-1b	23	0.802	0.782	0.782	1.172
174	TS252514G/	TS252514G/	0.566	1.000	0.566	Eq. H1-1b	20	0.802	0.782	0.782	1.172
175	TS252514G/	TS252514G/	0.252	1.000	0.252	Eq. H1-1b	19	0.802	0.782	0.782	1.172
176	TS252514G/	TS252514G/	0.388	1.000	0.388	Eq. H1-1b	11	0.802	0.782	0.782	1.172
177	TS252514										

 Software licensed to Job Title MILAM AUTO	Job No	Sheet No	Rev
	233-21-3409	SO 18 /25	
	Part	Ref	
	By AF	Date 06-Nov-21	Chd OA
Client	AMERICAN CARPORTS,INC	File	233-21-3409.std
		Date/Time	06-Nov-2021 13:08

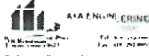
Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
181	TS252514G	TS252514G	0.389	1.000	0.389	Eq. H1-1b	11	0.802	0.782	0.782	1.172
182	TS252514G	TS252514G	0.445	1.000	0.445	Eq. H1-1b	20	0.802	0.782	0.782	1.172
183	TS252514G	TS252514G	0.038	1.000	0.038	Eq. H1-1b	15	0.802	0.782	0.782	1.172
184	TS252514G	TS252514G	0.087	1.000	0.087	Eq. H1-1b	12	0.802	0.782	0.782	1.172
185	TS252514G	TS252514G	0.124	1.000	0.124	Eq. H1-1b	20	0.802	0.782	0.782	1.172
186	TS252514G	TS252514G	0.126	1.000	0.126	Eq. H1-1b	20	0.802	0.782	0.782	1.172
187	TS252514G	TS252514G	0.124	1.000	0.124	Eq. H1-1b	20	0.802	0.782	0.782	1.172
188	TS252514G	TS252514G	0.124	1.000	0.124	Eq. H1-1b	20	0.802	0.782	0.782	1.172
189	TS252514G	TS252514G	0.091	1.000	0.091	Eq. H1-1b	12	0.802	0.782	0.782	1.172
190	TS252514G	TS252514G	0.573	1.000	0.573	Eq. H1-1b	20	0.802	0.782	0.782	1.172
191	TS252514G	TS252514G	0.252	1.000	0.252	Eq. H1-1b	19	0.802	0.782	0.782	1.172
192	TS252514G	TS252514G	0.388	1.000	0.388	Eq. H1-1b	11	0.802	0.782	0.782	1.172
193	TS252514G	TS252514G	0.416	1.000	0.416	Eq. H1-1b	11	0.802	0.782	0.782	1.172
194	TS252514G	TS252514G	0.424	1.000	0.424	Eq. H1-1b	11	0.802	0.782	0.782	1.172
195	TS252514G	TS252514G	0.425	1.000	0.425	Eq. H1-1b	11	0.802	0.782	0.782	1.172
196	TS252514G	TS252514G	0.416	1.000	0.416	Eq. H1-1b	11	0.802	0.782	0.782	1.172
197	TS252514G	TS252514G	0.387	1.000	0.387	Eq. H1-1b	11	0.802	0.782	0.782	1.172
198	TS252514G	TS252514G	0.451	1.000	0.451	Eq. H1-1b	20	0.802	0.782	0.782	1.172
199	TS252514G	TS252514G	0.033	1.000	0.033	Eq. H1-1b	23	0.802	0.782	0.782	1.172
200	TS252514G	TS252514G	0.174	1.000	0.174	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
201	TS252514G	TS252514G	0.184	1.000	0.184	Eq. H1-1b	11	0.802	0.782	0.782	1.172
202	TS252514G	TS252514G	0.206	1.000	0.206	Eq. H1-1b	11	0.802	0.782	0.782	1.172
203	TS252514G	TS252514G	0.208	1.000	0.208	Eq. H1-1b	11	0.802	0.782	0.782	1.172
204	TS252514G	TS252514G	0.208	1.000	0.208	Eq. H1-1b	11	0.802	0.782	0.782	1.172
205	TS252514G	TS252514G	0.206	1.000	0.206	Eq. H1-1b	11	0.802	0.782	0.782	1.172
206	TS252514G	TS252514G	0.184	1.000	0.184	Eq. H1-1b	11	0.802	0.782	0.782	1.172
207	TS252514G	TS252514G	0.183	1.000	0.183	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
208	TS252514G	TS252514G	0.054	1.000	0.054	Eq. H1-1b	12	0.802	0.782	0.782	1.172
209	TS252514G	TS252514G	0.190	1.000	0.190	Eq. H1-1b	11	0.802	0.782	0.782	1.172
210	TS252514G	TS252514G	0.132	1.000	0.132	Eq. H1-1b	11	0.802	0.782	0.782	1.172
211	TS252514G	TS252514G	0.148	1.000	0.148	Eq. H1-1b	11	0.802	0.782	0.782	1.172
212	TS252514G	TS252514G	0.147	1.000	0.147	Eq. H1-1b	11	0.802	0.782	0.782	1.172
213	TS252514G	TS252514G	0.148	1.000	0.148	Eq. H1-1b	11	0.802	0.782	0.782	1.172
214	TS252514G	TS252514G	0.147	1.000	0.147	Eq. H1-1b	11	0.802	0.782	0.782	1.172
215	TS252514G	TS252514G	0.132	1.000	0.132	Eq. H1-1b	11	0.802	0.782	0.782	1.172
216	TS252514G	TS252514G	0.163	1.000	0.163	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
217	TS252514G	TS252514G	0.067	1.000	0.067	Eq. H1-1b	20	0.802	0.782	0.782	1.172
218	TS252514G	TS252514G	0.180	1.000	0.180	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
219	TS252514G	TS252514G	0.024	1.000	0.024	Eq. H1-1b	23	0.802	0.782	0.782	1.172
220	TS252514G	TS252514G	0.162	1.000	0.162	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
221	TS252514G	TS252514G	0.239	1.000	0.239	Eq. H1-1b	11	0.802	0.782	0.782	1.172
222	TS252514G	TS252514G	0.268	1.000	0.268	Eq. H1-1b	11	0.802	0.782	0.782	1.172
223	TS252514G	TS252514G	0.268	1.000	0.268	Eq. H1-1b	11	0.802	0.782	0.782	1.172
224	TS252514G	TS252514G	0.268	1.000	0.268	Eq. H1-1b	11	0.802	0.782	0.782	1.172
225	TS252514G	TS252514G	0.268	1.000	0.268	Eq. H1-1b	11	0.802	0.782	0.782	1.172

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Print Run 5 of 27

 Software licensed to Job Title MILAM AUTO	Job No	Sheet No	Rev
	233-21-3409	SO 19 /25	
	Part	Ref	
	By AF	Date 06-Nov-21	Chd OA
Client	AMERICAN CARPORTS,INC	File	233-21-3409.std
		Date/Time	06-Nov-2021 13:08

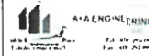
Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
226	TS252514G	TS252514G	0.238	1.000	0.238	Eq. H1-1b	11	0.802	0.782	0.782	1.172
227	TS252514G	TS252514G	0.147	1.000	0.147	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
228	TS252514G	TS252514G	0.242	1.000	0.242	Eq. H1-1b	11	0.802	0.782	0.782	1.172
229	TS252514G	TS252514G	0.291	1.000	0.291	Eq. H1-1b	11	0.802	0.782	0.782	1.172
230	TS252514G	TS252514G	0.306	1.000	0.306	Eq. H1-1b	11	0.802	0.782	0.782	1.172
231	TS252514G	TS252514G	0.307	1.000	0.307	Eq. H1-1b	11	0.802	0.782	0.782	1.172
232	TS252514G	TS252514G	0.291	1.000	0.291	Eq. H1-1b	11	0.802	0.782	0.782	1.172
233	TS252514G	TS252514G	0.239	1.000	0.239	Eq. H1-1b	11	0.802	0.782	0.782	1.172
234	TS252514G	TS252514G	0.029	1.000	0.029	Eq. H1-1b	23	0.802	0.782	0.782	1.172
235	TS252514G	TS252514G	0.142	1.000	0.142	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
236	TS252514G	TS252514G	0.260	1.000	0.260	Eq. H1-1b	11	0.802	0.782	0.782	1.172
237	TS252514G	TS252514G	0.283	1.000	0.283	Eq. H1-1b	11	0.802	0.782	0.782	1.172
238	TS252514G	TS252514G	0.272	1.000	0.272	Eq. H1-1b	11	0.802	0.782	0.782	1.172
239	TS252514G	TS252514G	0.272	1.000	0.272	Eq. H1-1b	11	0.802	0.782	0.782	1.172
240	TS252514G	TS252514G	0.282	1.000	0.282	Eq. H1-1b	11	0.802	0.782	0.782	1.172
241	TS252514G	TS252514G	0.259	1.000	0.259	Eq. H1-1b	11	0.802	0.782	0.782	1.172
242	TS252514G	TS252514G	0.127	1.000	0.127	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
243	TS252514G	TS252514G	0.103	1.000	0.103	Eq. H1-1b	20	0.802	0.782	0.782	1.172
244	TS252514G	TS252514G	0.078	1.000	0.078	Eq. H1-1b	12	0.802	0.782	0.782	1.172
245	TS252514G	TS252514G	0.070	1.000	0.070	Eq. H1-1b	11	0.802	0.782	0.782	1.172
246	TS252514G	TS252514G	0.092	1.000	0.092	Eq. H1-1b	11	0.802	0.782	0.782	1.172
247	TS252514G	TS252514G	0.130	1.000	0.130	Eq. H1-1b	11	0.802	0.782	0.782	1.172
248	TS252514G	TS252514G	0.025	1.000	0.025	Eq. H1-1b	23	0.802	0.782	0.782	1.172
249	TS252514G	TS252514G	0.126	1.000	0.126	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
250	TS252514G	TS252514G	0.195	1.000	0.195	Eq. H1-1b	11	0.802	0.782	0.782	1.172
251	TS252514G	TS252514G	0.230	1.000	0.230	Eq. H1-1b	11	0.802	0.782	0.782	1.172
252	TS252514G	TS252514G	0.236	1.000	0.236	Eq. H1-1b	11	0.802	0.782	0.782	1.172
253	TS252514G	TS252514G	0.237	1.000	0.237	Eq. H1-1b	11	0.802	0.782	0.782	1.172
254	TS252514G	TS252514G	0.230	1.000	0.230	Eq. H1-1b	11	0.802	0.782	0.782	1.172
255	TS252514G	TS252514G	0.192	1.000	0.192	Eq. H1-1b	11	0.802	0.782	0.782	1.172
256	TS252514G	TS252514G	0.114	1.000	0.114	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
257	TS252514G	TS252514G	0.059	1.000	0.059	Eq. H1-1b	20	0.802	0.782	0.782	1.172
258	TS252514G	TS252514G	0.135	1.000	0.135	Eq. H1-1b	11	0.802	0.782	0.782	1.172
259	TS252514G	TS252514G	0.132	1.000	0.132	Eq. H1-1b	11	0.802	0.782	0.782	1.172
260	TS252514G	TS252514G	0.165	1.000	0.165	Eq. H1-1b	11	0.802	0.782	0.782	1.172
261	TS252514G	TS252514G	0.180	1.000	0.180	Eq. H1-1b	11	0.802	0.782	0.782	1.172
262	TS252514G	TS252514G	0.180	1.000	0.180	Eq. H1-1b	11	0.802	0.782	0.782	1.172
263	TS252514G	TS252514G	0.166	1.000	0.166	Eq. H1-1b	11	0.802	0.782	0.782	1.172
264	TS252514G	TS252514G	0.126	1.000	0.126	Eq. H1-1b	11	0.802	0.782	0.782	1.172
265	TS252514G	TS252514G	0.091	1.000	0.091	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
266	TS252514G	TS252514G	0.116	1.000	0.116	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
267	TS252514G	TS252514G	0.067	1.000	0.067	Eq. H1-1b	12	0.802	0		

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 20 /25	Rev
	Part		
Ref			
By AF Date:06-Nov-21 Chd OA			
Client AMERICAN CARPORTS,INC	File 233-21-3409.std		Date/Time 06-Nov-2021 13:08


Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
271	TS252514G/	TS252514G/	0.131	1.000	0.131	Eq. H1-1b	20	0.802	0.782	0.782	1.172
272	TS252514G/	TS252514G/	0.133	1.000	0.133	Eq. H1-1b	20	0.802	0.782	0.782	1.172
273	TS252514G/	TS252514G/	0.141	1.000	0.141	Eq. H1-1b	20	0.802	0.782	0.782	1.172
274	TS252514G/	TS252514G/	0.160	1.000	0.160	Eq. H1-1b	11	0.802	0.782	0.782	1.172
275	TS252514G/	TS252514G/	0.072	1.000	0.072	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
276	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
277	TS252514G/	TS252514G/	0.083	1.000	0.083	Eq. H1-1b	11	0.802	0.782	0.782	1.172
278	TS252514G/	TS252514G/	0.091	1.000	0.091	Eq. H3-1	11	0.802	0.782	0.782	1.172
279	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
280	TS252514G/	TS252514G/	0.091	1.000	0.091	Eq. H3-1	11	0.802	0.782	0.782	1.172
281	TS252514G/	TS252514G/	0.107	1.000	0.107	Eq. H1-1b	20	0.802	0.782	0.782	1.172
282	TS252514G/	TS252514G/	0.072	1.000	0.072	Eq. H1-1b	16	0.802	0.782	0.782	1.172
283	TS252514G/	TS252514G/	0.092	1.000	0.092	Eq. H1-1b	11	0.802	0.782	0.782	1.172
284	TS252514G/	TS252514G/	0.138	1.000	0.138	Eq. H1-1b	11	0.802	0.782	0.782	1.172
285	TS252514G/	TS252514G/	0.069	1.000	0.069	Eq. Sec. D2	11	0.802	0.782	0.782	1.172
286	TS252514G/	TS252514G/	0.094	1.000	0.094	Eq. H3-1	20	0.802	0.782	0.782	1.172
287	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
288	TS252514G/	TS252514G/	0.067	1.000	0.067	Eq. H1-1b	11	0.802	0.782	0.782	1.172
289	TS252514G/	TS252514G/	0.094	1.000	0.094	Eq. H3-1	20	0.802	0.782	0.782	1.172
290	TS252514G/	TS252514G/	0.050	1.000	0.050	Eq. H3-1	23	0.802	0.782	0.782	1.172
291	TS252514G/	TS252514G/	0.094	1.000	0.094	Eq. H1-1b	11	0.802	0.782	0.782	1.172
292	TS252514G/	TS252514G/	0.094	1.000	0.094	Eq. H3-1	20	0.802	0.782	0.782	1.172
293	TS252514G/	TS252514G/	0.113	1.000	0.113	Eq. H3-1	19	0.802	0.782	0.782	1.172
294	TS252514G/	TS252514G/	0.103	1.000	0.103	Eq. H1-1b	23	0.802	0.782	0.782	1.172
295	TS252514G/	TS252514G/	0.047	1.000	0.047	Eq. H3-1	23	0.802	0.782	0.782	1.172
296	TS252514G/	TS252514G/	0.081	1.000	0.081	Eq. H3-1	11	0.802	0.782	0.782	1.172
297	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
298	TS252514G/	TS252514G/	0.118	1.000	0.118	Eq. H1-1b	16	0.802	0.782	0.782	1.172
299	TS252514G/	TS252514G/	0.047	1.000	0.047	Eq. H3-1	23	0.802	0.782	0.782	1.172
300	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
301	TS252514G/	TS252514G/	0.211	1.000	0.211	Eq. H1-1b	20	0.802	0.782	0.782	1.172
302	TS252514G/	TS252514G/	0.102	1.000	0.102	Eq. H3-1	12	0.802	0.782	0.782	1.172
303	TS252514G/	TS252514G/	0.047	1.000	0.047	Eq. H3-1	23	0.802	0.782	0.782	1.172
304	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
305	TS252514G/	TS252514G/	0.415	1.000	0.415	Eq. H1-1b	12	0.802	0.782	0.782	1.172
306	TS252514G/	TS252514G/	0.062	1.000	0.062	Eq. H1-1b	19	0.802	0.782	0.782	1.172
307	TS252514G/	TS252514G/	0.131	1.000	0.131	Eq. H1-1b	20	0.802	0.782	0.782	1.172
308	TS252514G/	TS252514G/	0.053	1.000	0.053	Eq. H3-1	23	0.802	0.782	0.782	1.172
309	TS252514G/	TS252514G/	0.046	1.000	0.046	Eq. H3-1	18	0.802	0.782	0.782	1.172
310	TS252514G/	TS252514G/	0.046	1.000	0.046	Eq. H3-1	18	0.802	0.782	0.782	1.172
311	TS252514G/	TS252514G/	0.046	1.000	0.046	Eq. H3-1	18	0.802	0.782	0.782	1.172
312	TS252514G/	TS252514G/	0.046	1.000	0.046	Eq. H3-1	18	0.802	0.782	0.782	1.172
313	TS252514G/	TS252514G/	0.052	1.000	0.052	Eq. H1-1b	18	0.802	0.782	0.782	1.172
314	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	16	0.802	0.782	0.782	1.172
315	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	16	0.802	0.782	0.782	1.172

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 21 /25	Rev
	Part		
Ref			
By AF Date:06-Nov-21 Chd OA			
Client AMERICAN CARPORTS,INC	File 233-21-3409.std		Date/Time 06-Nov-2021 13:08


Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
316	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	16	0.802	0.782	0.782	1.172
317	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H3-1	16	0.802	0.782	0.782	1.172
318	TS252514G/	TS252514G/	0.028	1.000	0.028	Eq. H1-1b	18	0.802	0.782	0.782	1.172
319	TS252514G/	TS252514G/	0.037	1.000	0.037	Eq. H1-1b	14	0.802	0.782	0.782	1.172
320	TS252514G/	TS252514G/	0.011	1.000	0.011	Eq. H1-1b	22	0.802	0.782	0.782	1.172
321	TS252514G/	TS252514G/	0.011	1.000	0.011	Eq. H1-1b	22	0.802	0.782	0.782	1.172
322	TS252514G/	TS252514G/	0.011	1.000	0.011	Eq. H1-1b	22	0.802	0.782	0.782	1.172
323	TS252514G/	TS252514G/	0.029	1.000	0.029	Eq. H1-1b	22	0.802	0.782	0.782	1.172
324	TS252514G/	TS252514G/	0.024	1.000	0.024	Eq. H1-1b	22	0.802	0.782	0.782	1.172
325	TS252514G/	TS252514G/	0.013	1.000	0.013	Eq. H1-1b	14	0.802	0.782	0.782	1.172
326	TS252514G/	TS252514G/	0.015	1.000	0.015	Eq. H1-1b	14	0.802	0.782	0.782	1.172
327	TS252514G/	TS252514G/	0.014	1.000	0.014	Eq. H1-1b	14	0.802	0.782	0.782	1.172
328	TS252514G/	TS252514G/	0.023	1.000	0.023	Eq. H1-1b	18	0.802	0.782	0.782	1.172
329	TS252514G/	TS252514G/	0.034	1.000	0.034	Eq. H1-1b	18	0.802	0.782	0.782	1.172
330	TS252514G/	TS252514G/	0.010	1.000	0.010	Eq. H3-1	11	0.802	0.782	0.782	1.172
331	TS252514G/	TS252514G/	0.010	1.000	0.010	Eq. H3-1	11	0.802	0.782	0.782	1.172
332	TS252514G/	TS252514G/	0.025	1.000	0.025	Eq. H1-1b	12	0.802	0.782	0.782	1.172
333	TS252514G/	TS252514G/	0.051	1.000	0.051	Eq. H1-1b	12	0.802	0.782	0.782	1.172
334	TS252514G/	TS252514G/	0.054	1.000	0.054	Eq. H1-1b	12	0.802	0.782	0.782	1.172
335	TS252514G/	TS252514G/	0.037	1.000	0.037	Eq. H1-1b	12	0.802	0.782	0.782	1.172
336	TS252514G/	TS252514G/	0.067	1.000	0.067	Eq. H1-1b	12	0.802	0.782	0.782	1.172
337	TS252514G/	TS252514G/	0.089	1.000	0.089	Eq. H1-1b	12	0.802	0.782	0.782	1.172
338	TS252514G/	TS252514G/	0.117	1.000	0.117	Eq. H1-1b	12	0.802	0.782	0.782	1.172
339	TS252514G/	TS252514G/	0.102	1.000	0.102	Eq. H1-1b	20	0.802	0.782	0.782	1.172
340	TS252514G/	TS252514G/	0.101	1.000	0.101	Eq. H1-1b	12	0.802	0.782	0.782	1.172
341	TS252514G/	TS252514G/	0.208	1.000	0.208	Eq. H1-1b	20	0.802	0.782	0.782	1.172
342	TS252514G/	TS252514G/	0.326	1.000	0.326	Eq. H1-1b	20	0.802	0.782	0.782	1.172
343	TS252514G/	TS252514G/	0.457	1.000	0.457	Eq. H1-1b	20	0.802	0.782	0.782	1.172
344	TS252514G/	TS252514G/	0.102	1.000	0.102	Sec. E1	11	0.802	0.782	0.782	1.172
345	TS252514G/	TS252514G/	0.123	1.000	0.123	Sec. E1	11	0.802	0.782	0.782	1.172
346	TS252514G/	TS252514G/	0.088	1.000	0.088	Sec. E1	11	0.802	0.782	0.782	1.172
347	TS252514G/	TS252514G/	0.107	1.000	0.107	Sec. E1	11	0.802	0.782	0.782	1.172
348	TS252514G/	TS252514G/	0.078	1.000	0.078	Sec. E1	11	0.802	0.782	0.782	1.172
349	TS252514G/	TS252514G/	0.096	1.000	0.096	Sec. E1	11	0.802	0.782	0.782	1.172
350	TS252514G/	TS252514G/	0.072	1.000	0.072	Sec. E1	11	0.802	0.782	0.782	1.172
351	TS252514G/	TS252514G/	0.089	1.000	0.089	Sec. E1	11	0.802	0.782	0.782	1.172
352	TS252514G/	TS252514G/	0.067	1.000	0.067	Sec. E1	11	0.802	0.782	0.782	1.172
353	TS252514G/	TS252514G/	0.082	1.000	0.082	Sec. E1	11	0.802	0.782	0.782	1.172
354	TS252514G/	TS252514G/	0.063	1.000	0.063	Sec. E1	11	0.802	0.782	0.782	1.172
355	TS252514G/	TS252514G/	0.076	1.000	0.076	Sec. E1	11	0.802	0.782	0.782	1.172
356	TS252514G/	TS252514G/	0.059	1.000	0.059	Sec. E1	11	0.802	0.782	0.782	1.172
357	TS252514G/	TS252514G/	0.266	1.000	0.26						

 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 24 /25	Rev
	Part		
Ref			
By AF Date 06-Nov-21 Chd OA			
Client AMERICAN CARPORTS,INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in ²)	Iz (in ⁴)	Iy (in ⁴)	Ix (in ⁴)
649	TS252514G	TS252514G	0.065	1.000	0.065	Eq. H1-1b	11	1.605	3.596	1.564	2.000
650	TS252514G	TS252514G	0.069	1.000	0.069	Eq. H1-1b	11	1.605	3.596	1.564	2.000
651	TS252514G	TS252514G	0.069	1.000	0.069	Eq. H1-1b	11	1.605	3.596	1.564	2.000
652	TS252514G	TS252514G	0.067	1.000	0.067	Eq. H1-1b	11	1.605	3.596	1.564	2.000
653	TS252514G	TS252514G	0.056	1.000	0.056	Eq. H1-1b	11	1.605	3.596	1.564	2.000
654	TS252514G	TS252514G	0.045	1.000	0.045	Eq. H1-1b	11	1.605	3.596	1.564	2.000
655	TS252514G	TS252514G	0.041	1.000	0.041	Eq. H1-1b	11	1.605	3.596	1.564	2.000
656	TS252514G	TS252514G	0.044	1.000	0.044	Eq. H1-1b	11	1.605	3.596	1.564	2.000
657	TS252514G	TS252514G	0.041	1.000	0.041	Eq. H1-1b	22	1.605	3.596	1.564	2.000
658	TS252514G	TS252514G	0.041	1.000	0.041	Eq. H1-1b	22	1.605	3.596	1.564	2.000
659	TS252514G	TS252514G	0.039	1.000	0.039	Eq. H1-1b	22	1.605	3.596	1.564	2.000
660	TS252514G	TS252514G	0.045	1.000	0.045	Eq. H1-1b	22	1.605	3.596	1.564	2.000
661	TS252514G	TS252514G	0.050	1.000	0.050	Eq. H1-1b	22	1.605	3.596	1.564	2.000
662	TS252514G	TS252514G	0.051	1.000	0.051	Eq. H1-1b	22	1.605	3.596	1.564	2.000
663	TS252514G	TS252514G	0.051	1.000	0.051	Eq. H1-1b	22	1.605	3.596	1.564	2.000
664	TS252514G	TS252514G	0.049	1.000	0.049	Eq. H1-1b	22	1.605	3.596	1.564	2.000
665	TS252514G	TS252514G	0.047	1.000	0.047	Eq. H1-1b	21	1.605	3.596	1.564	2.000
666	TS252514G	TS252514G	0.153	1.000	0.153	Eq. H1-1b	12	1.605	3.596	1.564	2.000
667	TS252514G	TS252514G	0.097	1.000	0.097	Eq. H3-1	12	1.605	3.596	1.564	2.000
668	TS252514G	TS252514G	0.097	1.000	0.097	Eq. H3-1	12	1.605	3.596	1.564	2.000
669	TS252514G	TS252514G	0.090	1.000	0.090	Eq. H1-1b	22	1.605	3.596	1.564	2.000
670	TS252514G	TS252514G	0.097	1.000	0.097	Eq. H1-1b	22	1.605	3.596	1.564	2.000
671	TS252514G	TS252514G	0.099	1.000	0.099	Eq. H1-1b	22	1.605	3.596	1.564	2.000
672	TS252514G	TS252514G	0.099	1.000	0.099	Eq. H1-1b	22	1.605	3.596	1.564	2.000
673	TS252514G	TS252514G	0.096	1.000	0.096	Eq. H1-1b	22	1.605	3.596	1.564	2.000
674	TS252514G	TS252514G	0.093	1.000	0.093	Eq. H1-1b	22	1.605	3.596	1.564	2.000
675	TS252514G	TS252514G	0.084	1.000	0.084	Eq. H1-1b	11	1.605	3.596	1.564	2.000
676	TS252514G	TS252514G	0.133	1.000	0.133	Eq. H1-1b	22	1.605	3.596	1.564	2.000
677	TS252514G	TS252514G	0.145	1.000	0.145	Eq. H1-1b	22	1.605	3.596	1.564	2.000
678	TS252514G	TS252514G	0.147	1.000	0.147	Eq. H1-1b	22	1.605	3.596	1.564	2.000
679	TS252514G	TS252514G	0.147	1.000	0.147	Eq. H1-1b	22	1.605	3.596	1.564	2.000
680	TS252514G	TS252514G	0.142	1.000	0.142	Eq. H1-1b	22	1.605	3.596	1.564	2.000
681	TS252514G	TS252514G	0.153	1.000	0.153	Eq. H1-1b	21	1.605	3.596	1.564	2.000
682	TS252514G	TS252514G	0.074	1.000	0.074	Eq. H1-1b	12	1.605	3.596	1.564	2.000
683	TS252514G	TS252514G	0.119	1.000	0.119	Eq. H1-1b	21	1.605	3.596	1.564	2.000
684	TS252514G	TS252514G	0.143	1.000	0.143	Eq. H1-1b	21	1.605	3.596	1.564	2.000
685	TS252514G	TS252514G	0.138	1.000	0.138	Eq. H1-1b	21	1.605	3.596	1.564	2.000
686	TS252514G	TS252514G	0.135	1.000	0.135	Eq. H1-1b	21	1.605	3.596	1.564	2.000
687	TS252514G	TS252514G	0.131	1.000	0.131	Eq. H1-1b	21	1.605	3.596	1.564	2.000
688	TS252514G	TS252514G	0.141	1.000	0.141	Eq. H1-1b	21	1.605	3.596	1.564	2.000
689	TS252514G	TS252514G	0.027	1.000	0.027	Eq. H3-1	23	1.605	3.596	1.564	2.000
690	TS252514G	TS252514G	0.041	1.000	0.041	Eq. H1-1b	20	1.605	3.596	1.564	2.000
691	TS252514G	TS252514G	0.067	1.000	0.067	Eq. H1-1b	20	1.605	3.596	1.564	2.000
692	TS252514G	TS252514G	0.089	1.000	0.089	Eq. H1-1b	15	1.605	3.596	1.564	2.000
693	TS252514G	TS252514G	0.106	1.000	0.106	Eq. H1-1b	15	1.605	3.596	1.564	2.000

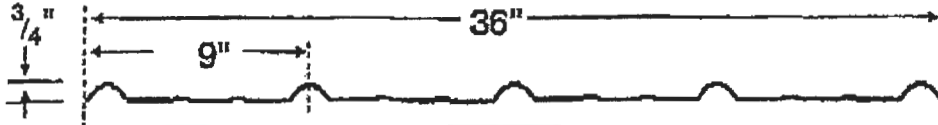
 Software licensed to Job Title MILAM AUTO	Job No 233-21-3409	Sheet No SO 25 /25	Rev
	Part		
Ref			
By AF Date 06-Nov-21 Chd OA			
Client AMERICAN CARPORTS,INC	File 233-21-3409.std	Date/Time 06-Nov-2021 13:08	

Failed Members

There is no data of this type.

THRU FASTENER INFORMATION

IMPERIAL RIB® 36" Coverage



SECTION PROPERTIES								
			TOP IN COMPRESSION			BOTTOM IN COMPRESSION		
PANEL THICKNESS	WEIGHT (PSF)	FY (KSI)	I _x (IN ⁴ /ft.)	S _e (IN ³ /ft.)	M _a (Kip in.)	I _x (IN ⁴ /ft.)	S _e (IN ³ /ft.)	M _a (Kip in.)
0.015	0.70	80	0.0103	0.0163	0.5852	0.0089	0.0165	0.5936
0.019	0.88	80	0.0135	0.0218	0.7829	0.0079	0.0189	0.6780

- NOTES:**
1. Section properties and allowable stresses are calculated in accordance with the 1986 AISI specifications for light gauge structural members.
 2. I_x is for deflection determination.
 3. S_e is for bending.
 4. M_a is allowable bending moment.
 5. All values are for one foot of panel width.

29 GAUGE		ALLOWABLE UNIFORM LOADS (PSF) IN POUNDS PER SQUARE FOOT															
Span Type	Load Type	1.00	1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50
Single	Positive Wind	520	231	130	83	58	42	33	26	21	17	14	12	11	9	8	7
	Negative Wind	528	235	132	84	59	43	33	26	21	17	15	12	11	9	8	7
	Live	390	173	98	62	43	32	24	19	16	13	11	9	8	7	6	5
	Deflection (L/180)	800	267	113	68	33	21	14	10	7	5	4	3	3	2	2	1
	Deflection (L/240)	876	200	84	43	25	18	11	7	5	4	3	2	2	2	1	1
2 Span	Positive Wind	528	236	132	84	59	43	33	28	21	17	15	12	11	9	8	7
	Negative Wind	520	231	130	83	58	42	33	26	21	17	14	12	11	9	8	7
	Live	396	178	99	63	44	32	25	20	16	13	11	9	8	7	6	5
	Deflection (L/180)	1874	555	234	120	69	44	29	21	15	11	9	7	6	4	4	3
	Deflection (L/240)	1406	416	178	90	52	33	22	15	11	8	7	5	4	3	3	2
3 Span	Positive Wind	660	293	165	106	73	54	41	33	26	22	18	13	12	10	9	8
	Negative Wind	650	289	163	104	72	53	41	32	26	21	18	15	13	12	10	9
	Live	495	220	124	79	55	40	31	24	20	16	14	12	10	9	8	7
	Deflection (L/180)	1468	435	184	84	54	34	23	16	12	9	7	5	4	3	3	2
	Deflection (L/240)	1101	326	138	70	41	26	17	12	9	7	5	4	3	3	2	2

26 GAUGE		ALLOWABLE UNIFORM LOADS (PSF) IN POUNDS PER SQUARE FOOT															
Span Type	Load Type	1.00	1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50
Single	Positive Wind	696	309	174	111	77	57	43	34	28	23	19	16	14	12	11	10
	Negative Wind	803	288	151	96	67	49	38	30	24	20	17	14	12	11	9	8
	Live	522	232	130	84	58	43	33	26	21	17	14	12	11	9	8	7
	Deflection (L/180)	1180	350	148	76	44	28	18	13	9	7	5	4	3	3	2	2
	Deflection (L/240)	885	262	111	57	33	21	14	10	7	5	4	3	3	2	2	1
2 Span	Positive Wind	603	268	151	96	67	49	38	30	24	20	17	14	12	11	9	8
	Negative Wind	696	309	174	111	77	57	43	34	28	23	19	16	14	12	11	10
	Live	452	201	113	72	50	37	28	22	18	15	13	11	9	8	7	6
	Deflection (L/180)	1663	493	208	106	62	39	26	18	13	10	8	6	5	4	3	3
	Deflection (L/240)	1248	370	158	80	46	29	19	14	10	7	6	5	4	3	2	2
3 Span	Positive Wind	753	335	188	121	84	61	47	37	30	25	21	18	15	13	12	10
	Negative Wind	870	387	217	139	97	71	54	43	35	29	24	21	18	15	14	12
	Live	565	251	141	90	63	48	35	28	23	19	18	13	12	10	9	8
	Deflection (L/180)	1303	386	163	83	48	30	20	14	10	8	6	5	4	3	3	2
	Deflection (L/240)	977	290	122	63	35	23	15	11	8	6	5	4	3	2	2	2

- NOTES:**
1. Section properties and allowable stresses are calculated in accordance with the 1986 AISI specifications for light gauge structural members.
 2. Steel strength of thru fastener panels is 80 KSI minimum yield. Steel strength of concealed panels is 50 KSI minimum yield. These conform to ASTM A635 (galvanized) and A792-94 (Galvalume®).
 3. Values shown as allowable loads are based on panel covering three equal spans. Multiply by 0.8 for two span allowable loads.
 4. Allowable loads for wind have been increased by 33%. Panel weight has not been deducted. Minimum bearing length must be checked.
 5. For agricultural structures, the UBC and SBCCI building codes require a minimum of 10 PSF roof live loads.
 6. Deflection loads are limited by a maximum deflection ratio of L/180 of span.



TRI-STATE

TESTING SERVICES, INC.

September 17, 2003

Report No.: LO28906

Home Pride, Inc.
P.O. Box 160387
Nashville, TN 37216
Telephone: (615) 226-6453
Fax: (615) 226-1301

SUBJECT: Performance Test Report -HP9

ITEM DESCRIPTION: Model HP9 (Barbed Anchor)

TEST DATE: September 12, 2003

TEST METHODOLOGY: Model HP9 was installed per installation instructions.

Mr. Oliphant - We have concluded testing on the above referenced item.

A Performance test was performed on model HP9. Model HP9 was installed by drilling a 1 1/2" hole 15 inches deep. Model HP9 was then driven into the hole to a depth of 13".

The performance test consists of three-load test. Each anchor was pre-bent to a forty-five degree angle and preloaded prior to the start of each test.

RESULTS: Model HP9 was found to be acceptable based on previously mentioned acceptance criteria (see attached data log). There was no disfiguring or failure to Model HP9, all anchors were loaded to any ultimate load of 4,725 lbs with no movement in excess of three inches in a horizontal direction and two inches in a vertical direction.

If we can answer any questions concerning this report please let me know.

Respectfully Submitted,
TRI-STATE TESTING SERVICES, INC.


Will Jackson
Assistant Manager



Tri-State Testing Services

45 degree anchor test

Customer: Home Pride, Inc.
Anchor mdl: HP9
Date: September 12, 2003

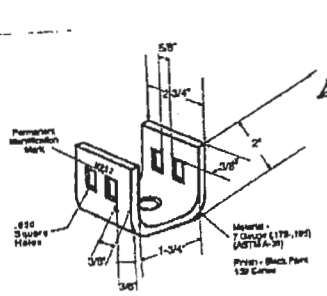
Test No. 1			Test No. 2			Test No. 3		
load	lift	deflection	load	lift	deflection	load	lift	deflection
1000	0	0	1000	0	0	1000	0	0
1500	0	0	1500	0	0	1500	0	0
2000	0	0	2000	0	0	2000	0	0
2500	0	0	2500	0	0	2500	0	0
3000	0	0	3000	0	0	3000	0	0
3500	0	0	3500	0	0	3500	0	0
4000	0	0	4000	0	0	4000	0	0
4500	0	0	4500	0	0	4500	0	0
4725	0	0	4725	0	0	4725	0	0

Total Load 4725
Total Lift 0
Total Defl. 0
Results Acceptable

Total Load 4725
Total Lift 0
Total Defl. 0
Results Acceptable

Total Load 4725
Total Lift 0
Total Defl. 0
Results Acceptable

HP9 Barbed Anchor



2" Swivel Head

22" x 3/4"
A529 Grade 50

2" x 3/4" x .125"
A36

4 Tabs
Must be Angled
To the Right.

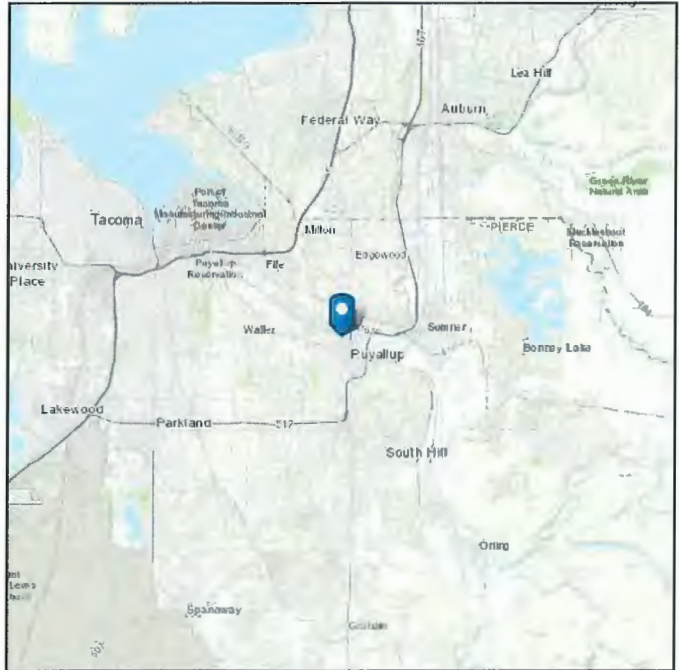
HP9
22" x 3/4"
Barbed Anchor

ASCE 7 Hazards Report

Address:
608 River Rd
Puyallup, Washington
98371

Standard: ASCE/SEI 7-16
Risk Category: I
Soil Class: D - Default (see Section 11.4.3)

Elevation: 41.39 ft (NAVD 88)
Latitude: 47.200016
Longitude: -122.30103



Wind

Results:

Wind Speed:	92 Vmph
10-year MRI	67 Vmph
25-year MRI	73 Vmph
50-year MRI	78 Vmph
100-year MRI	83 Vmph

Data Source: ASCE/SEI 7-16, Fig. 26.5-1A and Figs. CC.2-1–CC.2-4, and Section 26.5.2
Date Accessed: Sat Nov 06 2021

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 15% probability of exceedance in 50 years (annual exceedance probability = 0.00333, MRI = 300 years).

Site is not in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2.

Site Soil Class: D - Default (see Section 11.4.3)

Results:

S_S :	1.277	S_{D1} :	N/A
S_1 :	0.439	T_L :	6
F_a :	1.2	PGA :	0.5
F_v :	N/A	PGA _M :	0.6
S_{MS} :	1.533	F_{PGA} :	1.2
S_{M1} :	N/A	I_e :	1
S_{DS} :	1.022	C_v :	1.355

Ground motion hazard analysis may be required. See ASCE/SEI 7-16 Section 11.4.8.

Data Accessed: Sat Nov 06 2021

Date Source: [USGS Seismic Design Maps](#)



Snow

Results:

Ground Snow Load, p_g : 18 lb/ft²

Elevation: 41.4 ft

Data Source:

Date Accessed: Sat Nov 06 2021

Statutory requirements of the Authority Having Jurisdiction are not included.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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