GENERAL STRUCTURAL NOTES

These notes shall govern unless otherwise noted in the drawings. Refer to written specifications for further requirements and amplification of these notes.

Codes
International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction.
Project Jurisdiction: City of Puyallup, State of Washington.

Design Loads
Reference code: ASCE/SEI 7-16, Minimum Design Loads for Buildings and Other Structures.

Dead Loads
Refer to Load Maps & Load Diagrams (S-501 and S-502)

Floor	
Typical Floor	68 psf
Mechanical Floor	118 psf (or Typical Floor wt + actual wt of equip.)
Roof	
Typical Metal Deck Roof	31 psf (Includes 8 psf PV panels, +5 psf above corridor)
Typical Composite Deck Roof	84 psf (Includes 8 psf PV panels)
Roof Extension above Front Entry	13 psf (PV not allowed)

Mechanical Equipment
- Actual operating weight is by manufacturer.
- Weight indicated on drawings is the maximum design load.
- Notify Architect/Engineer if the manufacturer's operating weight of the equipment provided exceeds the maximum design load, prior to installation of the equipment.

Live Loads
(live load reduction is applied as allowed by the reference code)

Floor	
Typical Floor	100 psf
First Floor Corridors	100 psf
Upper Level Library Corridors	100 psf; 1000 lb on any 2.5 ft sq
Upper Level Office Corridors	100 psf; 2000 lb on any 2.5 ft sq
Upper Level School Corridors	100 psf
Other Upper Floor Corridors	same as occupancies served
Stairs and exit ways	100 psf
Storage (light)	125 psf or max load expected
Storage (heavy) (Rock Storage)	250 psf or max load expected
Roof	
Typical Roof	20 psf
Handrails/Guardrails	
Assembly	50 psf
Intermediate rails/panels	50# on any 1.0 ft sq, in any direction
Grab Bar	250# at any point, in any direction
Snow	
Ground snow load	Pg = 18 psf
Calculated flat roof snow load	Pf = 22 psf
Minimum flat roof snow load	Pf = 25 psf
Design uniform snow load	30 psf
Importance factor (snow)	Is = 1.1
Snow exposure factor	Ce = 1.0
Thermal factor	Ct = 1.0
Other Surcharges	per reference code
Unbalanced loads	per reference code
Wind	
Basic wind speed (3 second gust)	V = 104 mph
Exposure Category	B
Importance factor (wind)	Iw = 1.15
Topographical Factor	Kzt = 1.0
Directional Coefficient	Kd = 0.85
Gust Effect Factor	G = 0.85
Internal pressure coefficient	Gcpi = 0.18
Minimum Net Uplift	16 psf
Seismic	
Analysis Procedure	Modal Response Spectrum Analysis
Occupancy category	III
Importance factor - seismic	Ie = 1.25
Long-period transition period	Tl = 6.0 sec
Building period coefficient	Ct = 0.030
Site Class	C
Spectral acceleration parameters	Ss = 1.253
Spectral acceleration parameters	S1 = 0.432
Site coefficients	Fa = 1.2
Site coefficients	Fv = 1.0
Spectral response coefficient	Sds = 1.003
Spectral response coefficient	Sd1 = 0.432
Seismic design category	D
Lateral system	Steel buckling-restrained braced frames
Response modification coefficient	R = 8.0
Seismic response coefficient	Cs = 0.116
Structural wall out-of-plane	Fp = 0.501 x Wp
Non-structural components(rigid)	ap = 1.0, Fp = 0.481 x Wp
Non-structural components(flexible)	ap = 2.5, Fp = 1.203 x Wp
Earth	
Loading based on geotechnical report by: GeoEngineers dated: January 21, 2021	
Active pressure (drained)	35 pcf + 13*H seismic surcharge
Active pressure (undrained)	80 pcf + 13*H seismic surcharge (includes hydrostatic pressures)
At-rest pressure (drained)	55 pcf + 13*H seismic surcharge
At-rest pressure (undrained)	90 pcf + 13*H seismic surcharge (includes hydrostatic pressures)
An additional 2 feet of fill shall be added to represent a traffic surcharge of 250 psf if vehicles are within .5*H of the wall	
An additional 1 foot of fill shall be added to represent adjacent 100 psf occupant live load at top of wall condition	
Passive resistance	290 pcf
Coefficient of friction	0.40

Refer to load maps sheets and drawings for additional loads

CONSTRUCTION LOADS

1. Reference code: ASCE/SEI 37-14, Design Loads on Structures During Construction
2. The structure represented in these drawings has been designed to resist vertical and lateral loads prescribed by the reference code in its final constructed condition. The sequence of construction is the responsibility of the general contractor. All loads experienced by the structure due to the incomplete nature of the structure are the sole responsibility of the general contractor. The general contractor must design and provide temporary shoring and bracing until the final built condition, as shown in these drawings, is achieved.
3. The structure represented in these drawings has not been designed to resist vertical and lateral loads imparted by construction equipment. All loads experienced by the structure while supporting construction equipment are the sole responsibility of the general contractor. The general contractor must design and provide temporary shoring and bracing where required to support construction equipment.
4. Where temporary shoring or bracing is required, retain the services of a structural engineer registered in the project jurisdiction to design and detail the bracing of that equipment for the gravity and lateral forces prescribed by the reference code. Submit the stamped and signed design documents to the project jurisdiction as a deferred submittal for approval prior to performing the work.

GENERAL CONDITIONS

TYPICAL CONDITIONS
1. Typical details are not referenced at all locations for which they apply and may not be referenced at all. Details located on typical detail sheets represent the method of construction to be used at all locations, unless otherwise indicated in the drawings.

COORDINATION
1. The written specifications and the drawings of the architectural, mechanical, electrical and civil/landscape disciplines are to be used in conjunction with the structural drawings for bidding and construction.
2. Dimensions for some secondary elements such as windows, doors, walls and floor edges are located only in the architectural drawings. Shop drawing production for structural elements will require dimensional information contained in both the architectural and structural drawings. All requests for dimensions in shop drawing submittals will be referred to the general contractor.
3. The contractor shall coordinate dimensions and conditions between the drawings (including the architectural, mechanical, electrical and civil/landscape disciplines), the specifications, and the site conditions prior to fabrication and construction. Notify Architect/Engineer in writing of any discrepancies in dimensions or conditions found prior to fabricating and executing work in the area of the discrepancy. Architect/Engineer will respond in writing according to the provisions of the general conditions found in the specifications. Any related work performed by the contractor between the discovery of the discrepancy and receipt of the Architect's/Engineer's written response will be done at the contractor's risk.
4. Where the bracing of mechanical, plumbing, fire-suppression and/or electrical equipment is not specifically detailed in the mechanical, plumbing, fire-protection and/or electrical drawings or specifications, retain the services of a structural engineer registered in the jurisdiction of this project to design and detail the bracing of that equipment for the gravity and lateral forces prescribed by the governing building code. Submit the stamped and signed design documents to the project jurisdiction as a deferred submittal for approval prior to performing the work.
5. Where the bracing of ceilings and other architectural elements is not specifically detailed in the architectural drawings or specifications, retain the services of a structural engineer registered in the jurisdiction of this project to design and detail the bracing of those elements for the gravity and lateral forces prescribed by the governing building code. Submit the stamped and signed design documents to the project jurisdiction as a deferred submittal for approval prior to performing the work.
6. Provide coordination drawings showing all anticipated penetrations through the structural elements shown in these drawings. No penetrations through structural elements shall be allowed unless already indicated in the structural drawings or approved in writing by the structural engineer.

SUBMITTALS
1. Construction utilizing any given material shall not occur until the approved submittals for that material are received from the Architect/Engineer.

LATERAL FORCE AND SEISMIC LOAD RESISTING SYSTEMS

LATERAL FORCE RESISTING SYSTEM
1. Lateral-forces are transferred by the roof and floor diaphragms to the vertical lateral-force resisting elements (shearwalls, braced frames). Moments and shears resulting from these lateral-forces are transferred to the foundation system by the vertical lateral-force resisting elements. Lateral-forces are distributed through this load path in proportion to the stiffness of the transferring and resisting elements. Lateral-forces are resisted by the foundation system through passive pressure of the adjacent earth, sliding friction resistance at the soil interface, and overturning resistance of the structure's tributary dead load.
2. Any member designated '(C)' in the drawings is a chord/collector and is a component of the Lateral Force Resisting System.
3. Any weld designated 'DC' in the drawings is participating in the Lateral Force Resisting System and is 'Demand Critical'. See the Welding notes and the Special Inspection notes for requirements specific to Demand Critical welds.
4. Discontinuities created by fabrication or erection operations, such as tack welds, erection aids, air-arc gouging and thermal cutting are not allowed within the Protected Zone.
5. Welded shear studs and decking attachments that penetrate the beam flange shall not be placed on the beam flanges within the Protected Zone.
6. Welded, bolted, screwed or shot-in attachments for perimeter edge angles, exterior facades, partitions, duct work, piping or other construction shall not be placed within the Protected Zone. Refer to the drawings and these notes for definition of the Protected Zones.
7. Where violations of the Protected Zone occur, inform the Architect/Engineer in writing. Repair the discontinuity as required by the engineer of record's written response. All repairs in the Protected Zone will be subject to special inspection by the owner's agent.

DESIGNATED SEISMIC LOAD RESISTING SYSTEM
Buckling-Restrained Braced Frames (BRBF)
Buckling Restrained Braced Frames are composed of steel beams spanning between steel columns and buckling restrained braces (steel core encased in grout that is surrounded by a steel casing) spanning between the intersections of those beams and columns with gusset plate connections at the ends of those braces. See sheets S201 and S202 for brace frame elevations.

FOUNDATION

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 18

Strength
Capacities are based on geotechnical report and Addendum No. 1 by: GeoEngineers, project #: 21342-002-00, dated: January 21, 2021 & June 29, 2022

Allowable soil bearing pressure	3,500 psf
Passive equivalent fluid pressure	290 pcf
Coefficient of friction	0.40

Coordination
1. Coordinate grading and excavating requirements with civil/landscape drawings. Foundations are designed to bear on undisturbed native soils or compacted structural fill, as described in the Geotechnical Report, over undisturbed native soils.

Execution
1. Footing excavations should be cleaned of all loose soil, leveled, and protected from water and construction traffic.
2. Refer to geotechnical report and follow recommendations specific to wet weather earthwork.
3. Refer to geotechnical report and follow recommendations specific to temporary cut slopes.

Submittals
1. Suitability of soils for bearing is as described in the geotechnical report. Soil bearing surfaces must be observed and approved by the geotechnical engineer of record prior to pouring foundation concrete. Submit the written field report by the geotechnical engineer of record to the Architect/Engineer for approval.

REINFORCING

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction
- ACI 318-14, Building Code Requirements for Structural Concrete
- ACI 301-10, Specification for Structural Concrete
- TMS 402/602-16, Building Code Requirements and Specification for Masonry Structure
- AWS D1.4/D1.4M-18, Structural Welding Code - Steel Reinforcing Bars

Strength

Deformed Bars (new billet stock)	ASTM A615	Fy = 60 ksi
Weldable Deformed Bars	ASTM A706	Fy = 60 ksi
Epoxy Coated Deformed Bars	ASTM A775	Fy = 60 ksi
Plain Welded Wire Fabric (electrically welded)	ASTM A1064	Fy = 60 ksi

Fiber reinforced concrete shall conform to ACI Report 544.1R. Follow manufacturer's recommended mix quantity, but use no less than 1.5#/cu yd.

CONCRETE

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 19
- ACI 301-10, Specification for Structural Concrete
- ACI SP66 (04), ACI Detailing Manual
- ACI 318-14, Building Code Requirements for Structural Concrete

Strength
Provide concrete mix design in accordance with ACI 301, and meeting the requirements of the CONCRETE MIX DESIGN TABLE.

Coordination
1. Conduits embedded within slabs, walls or beams shall be placed between rebar mats where double mats occur. The outside diameter of the conduit shall be smaller than 1/4 x the member thickness and shall be spaced greater than 4 x the conduit outside diameter.
2. Coordinate reinforcing steel placement details with structural embeds and embeds specified in other disciplines. Utilize templates for placing steel in congested areas.
3. No concrete work shall be penetrated for piping or ducts, unless shown in the drawings or approved by the Engineer in writing.

Execution
1. Provide bar supports as required in the contract documents. Concrete dobies shall be minimum 4000 psi with cast-in double annealed 16 ga iron wires for tying. Wire chairs shall have Class 1 plastic tips.
2. Provide cover as shown in drawings, with a minimum cover as required by the CONCRETE REINFORCING COVER TABLE.
3. Provide rebar splice lengths as shown in the drawings, with a minimum splice as required by the CONCRETE REINFORCING SPLICE TABLE.
4. Camber concrete forms as shown plus deflection due to the weight of wet concrete.
5. One-way slabs shall be cambered 1/1000 of the span, unless noted otherwise on the drawings. Cambers of less than 1/8" may be neglected.
6. Air Content and Slump shall be measured at the truck discharge or at the end of the pump, wherever concrete is pumped. Slump shall be within +1"/-2" of slump specified in the approved mix design submittal.
7. Tie reinforcement into the correct positions using double annealed 16 ga iron wire. Use wire chairs at maximum 36 inch spacing in formed construction and concrete dobies at maximum 24 inch spacing at concrete cast against grade or rigid insulation to elevate the rebar into the designated positions and to maintain the required concrete cover.
8. Cold Bend bars as noted in drawings to radius specified in ACI 315. Bend bars one time only.
9. Trim bars may be omitted when the opening dimensions are less than the rebar spacing, for opening less than 12"x12". Relocate all interrupted rebar along one side of the opening and place an identical bar on the opposite side of the opening. Alternatively, where the interrupted bar is within 3" of opening edge, rebar may be sprung around the opening with no additional rebar required on the opposite side of the opening.
10. Provide 3/4" chamfer at all exposed concrete edges, unless noted otherwise.

Submittals
1. Mix Designs meeting or exceeding the requirements of the CONCRETE MIX DESIGN TABLE must be submitted to and approved by the Engineer prior to use. Provide mix designs which correspond to anticipated placement requirements and finish conditions. Deviation from the specified mix design must be demonstrated to be in accordance with ACI 318, Chapter 26 and must be submitted a minimum of two weeks prior to use for approval by the Engineer, with a written explanation of the reason for deviating from the specified mix design. Approval of deviation from the specified mix design is at the discretion of the Engineer.
2. Provide coordinated shop drawings with 1/4" scale elevations of all walls with all reinforcing, openings, structural embeds, and embedded items from other disciplines, all shown in conjunction and dimensioned relative to a common datum. Before submitting shop drawings for structural review, Mechanical and Electrical contractors must mark size and locations of all required penetrations and embeds on wall elevations.
3. Provide certification to show that all rebar welders hold a current WABO certification and are prequalified according to AWS D1.4 for all weld sizes and positions required.
4. Slab-on-Grade control joints shown in plan are schematic. Contractor shall provide control joint layout submittal to Engineer for approval showing all control joints that will be provided, conforming to the maximum joint spacing allowed.

THE APPROVED CONSTRUCTION PLANS, DOCUMENTS AND ALL ENGINEERING MUST BE POSTED ON THE JOB AT ALL INSPECTIONS IN A VISIBLE AND READILY ACCESSIBLE LOCATION.

FULL SIZED LEDGIBLE COLOR PLANS ARE REQUIRED TO BE PROVIDED BY THE PERMITEE ON SITE FOR INSPECTION

PROVIDE SPECIAL INSPECTION REPORT TO CITY OF PUYALLUP PRIOR TO CONCERT POUR FOR REVISED FOOTING DRAWINGS

MASONRY VENEER

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 21
- TMS 402/602-16, Building Code Requirements and Specification for Masonry Structures

Execution
1. Provide seismic veneer ties, spaced at 16" on center horizontal and 16" on center vertical per typical detail. This applies where support structure is concrete and/or metal studs.
2. Where veneer is not laid in running bond, joint reinforcement is required. See Masonry Veneer specification for requirements.
3. Provide hot dip galvanized steel angles over all openings in masonry veneer per typical detail.

METAL DECKING

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 22
- AISC 360-16, Specifications for Structural Steel Buildings
- AISC 341-16, Seismic Provisions for Structural Steel Buildings.
- AWS D1.3-08, Structural Welding Code - Sheet Steel

Strength
ASTM A653, Gr A, minimum yield 38 ksi.

Coordination
1. Deck gage shown in plan is the minimum gage required to conform to the structural design of the finished building. Provide a deck gage which meets this minimum requirement, as well as maximum span requirements, design for construction loads, and shoring requirements.
2. Provide acoustic deck where required by structural or architectural drawings. Provide acoustic deck with perforated vertical webs and acoustical insulation in the low flutes.

Execution
1. Layout deck to span continuously over a minimum of four supports, except where framing layout does not permit.
2. Design and provide closure plates at all deck ends, including at floor openings, floor edges, and around columns.
3. Fasten the deck to supporting framing with 3/4" diameter (5/8" effective diameter) puddle welds, one at each flute perpendicular to framing support and at spacing to match flute spacing parallel to framing support, unless noted otherwise. Welded shear studs may serve as substitute for puddle weld in composite deck systems.
4. Fasten sidelaps of untopped deck with interlocking punch at 8" maximum spacing, unless noted otherwise.
5. Fasten sidelaps of concrete topped deck with button punch at 24" maximum spacing, unless noted otherwise.
6. Provide G90 galvanized deck at exterior applications.
7. Provide G60 galvanized deck at interior applications.
8. Factory apply gray acrylic primer over the galvanized coating where deck is to receive paint - refer to architectural drawings for locations.

Submittals
1. Provide design calculations showing that the deck provided is sufficient to resist all construction loads and in-place loads specified in the contract documents. Calculations and shop drawings shall be stamped and signed by an engineer registered in the project jurisdiction. Calculations and shop drawings must be approved by the Engineer and the Building Official prior to installation of the deck.

STRUCTURAL STEEL

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 22
- AISC 360-16, Specifications for Structural Steel Buildings
- AISC 341-16, Seismic Provisions for Structural Steel Buildings.

Strength

W Shapes, WT Shapes	ASTM A992 or A572, Gr 50	Fy = 50 ksi
Angles, Channels	ASTM A36	Fy = 36 ksi
HSS Square Structural Tube	ASTM A500, Gr C	Fy = 50 ksi
HSS Rectangular Structural Tube	ASTM A500, Gr C	Fy = 50 ksi
HSS Round Structural Tube	ASTM A500, Gr C	Fy = 46 ksi
Steel Pipe	ASTM A53, Gr B	Fy = 35 ksi
All other Steel Shapes	ASTM A572 or A588	Fy = 50 ksi
All Steel Plate, UNO	ASTM A572 or A588	Fy = 50 ksi
Steel Plate Designated as 36 ksi	ASTM A36	Fy = 36 ksi

1. Plates 2" thick and thicker, that are part of the Lateral Force Resisting System, shall have a minimum Charpy V-notch toughness of 20 ft-lb at 70 degrees F, measured at any location permitted by ASTM A673.
2. Hot rolled shapes that are part of the Lateral Force Resisting System shall have a minimum Charpy V-notch toughness of 20 ft-lb at 70 degrees F, tested in the alternate core location as described in ASTM A6.

Coordination
1. Refer to architectural drawings for dimensions required for the location of steel elements including, but not limited to, framing around window and door openings, top of parapet and bottom of soffit elevations, wall locations, and edge of deck dimensions.

Execution
1. The corners of continuity plates and stiffeners placed in the webs of rolled shapes that are part of the Lateral Force Resisting System shall be clipped according to AISC 341 Commentary F3.5b.4, AWS D1.8 Commentary Section 4, and AISC 358 chapter 3.
2. Weld access holes shall comply with the requirements of AISC 360, Section J1.6.
3. Provide N-Bearing 3/4" diameter bolts through horizontal short slotted holes at beam shear connections, unless noted otherwise.
4. Where connection is not accessible to Twist-off Bolt Guns, provide A325 Bolts with load indicator washers.
5. All exterior exposed structural steel shall be hot-dip galvanized, unless noted otherwise.
6. Unless otherwise noted, beams are equally spaced between dimension points.
7. The Contractor shall be responsible for all erection aids and joint preparations.
8. The Contractor shall be responsible for compliance with all current OSHA requirements.
9. Modification to the structural steel, including holes and copes, shall not be made in the field without prior approval of the Engineer.

Submittals
1. Shop drawings shall include the following information relative to the Lateral Force Resisting System:
   a. Designate all members and connections that are part of the Lateral Force Resisting System.
   b. Designate all shop welds that are Demand Critical.
   c. Designate and dimension all protected zones.
   d. Include welding requirements as specified in AWS D1.8.

COLD-FORMED METAL FRAMING

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 22
- AISI S-100-16, North American Specification for the Design of Cold-Formed Steel Structural Members, including Supplement No. 1 Dated 2018.
- AISI S202-15, Code of Standard Practice for Cold-formed Steel Framing
- AWS D1.3-08, Structural Welding Code - Sheet Steel

Strength

Material thickness < 54 mil	ASTM A1003, Gr33, 33 ksi
Material thickness 54 mil & greater	ASTM A1003, Gr50, 50 ksi

Design
1. Where required by the drawings, design cold-formed steel members and connections to resist construction and building loads described in these documents.

Coordination
1. Structural walls are those shown in the structural drawings. Refer to architectural drawings for non-structural walls, partitions and soffits.

Execution
1. Installation of cold-formed metal stud walls shall conform to ASTM C1007.
2. The framing members shall have ends squarely cut by shearing or sawing, be installed plumb, square, true to line and securely fastened per the contract documents or according to the manufacturer's approved engineered connection design.
3. Structural "C" members are not permitted to have splices or cut-outs in the flanges, unless shown in the drawings.
4. Fastening of structural members shall be accomplished by screws, power actuated fasteners, welding, or a combination of methods. The type, size, and spacing shall be as required by the documents or according to the manufacturer's approved engineered connection design.
5. Provide members and connectors with minimum protective coating of G-60 galvanized finish.
6. Wherever protective coating is damaged or removed for the purpose of fastening, repair by painting with a zinc rich primer.

Submittals
1. All designs required to be provided by the manufacturer shall include drawings and calculations, stamped and signed by an engineer registered to work in the jurisdiction of this project.

PRCNC20221036
STRUCTURAL REVISION

City of Puyallup Building ACCEPTED
JMontgomery
02/01/2023
9:19:04 AM
CITY OF PUYALLUP STATE OF WASHINGTON

SHEET LIST

CONCRETE MIX DESIGN TABLE

Type	Use	28 Day Strength (psi)	Max Aggregate Size	Max W/C Ratio	Required Additives*	Exposure Categories & Classes** Freeze-Thaw (F)	Sulfate (S)	Contact w/ Water (W)	Corrosion (C)	% Air Range
A	Footings	4500	1"	0.50	-	F1	S0	W0	C1	2% max
B	Exterior Walls & Col's	4500	3/4"	0.45	-	F2	S0	W0	C1	6% ±1%
C	Interior Walls & Col's	4000	3/4"	0.45	-	F0	S0	W0	C0	2% max
D	Interior Slabs & Column Pour-backs	4000	3/4"	0.43	Superplasticizer (6"-8" Slump)	F0	S0	W0	C0	2% max
E	Exterior Slabs	5000	1"	0.40	Polyfiber Reinf	F2	S0	W1	C2	6% ±1%
F	Metal Deck Topping	4000	3/4"	0.43	Superplasticizer (6"-8" Slump) Polyfiber Reinf	F0	S0	W0	C0	2% max
G	Stair Pan Topping	5000	3/8"	0.43	Polyfiber Reinf	F0	S0	W0	C0	2% max
J	Retaining Walls	5000	3/4"	0.45	-	F2	S0	W0	C1	2% max
K	All Others	Consult with EOR on case-by-case basis								

Notes:
1. Concrete mixes A-K shall meet the requirements of ACI 318-14 Chapter 19 and Table 19.3.2.1 REQUIREMENTS FOR CONCRETE BY EXPOSURE CLASS
2. Air Entrainment shall be 2% max except where a higher range is required by it's exposure class
* All admixtures shall be chloride free otherwise approved by the Engineer
** Where the application applies to more than 1 type, the more stringent design parameters shall be used (e.g. Metal Deck topping would be 5000, 3/4", 6%, 0.40, F2, SO, W1, C2)

CONCRETE REINFORCING COVER TABLE

Location	Size	Cover
Cast against and permanently exposed to earth	All	3"
Exposed to earth or weather	#5 or smaller	1 1/2"
	#6 or larger	2"
Interior Wall Faces	#11 or smaller	3/4"
	#14 and #18	1 1/2"
Slab-on-Grade cast against vapor barrier	#11 or smaller	1" (see note 1)
	#14 and #18	1 1/2"
Elevated Slabs	#11 or smaller	1" (see note 1)
	#14 and #18	1 1/2"
Interior Frames - Beams and Columns (to Ties, Spirals, or stirrups)	All	1 1/2"
Exterior Frames - Beams and Columns (to Ties, Spirals, or stirrups)	All	2" (see note 2)

Notes:
1. Can be 3/4 for 1-hour fire - refer to code plan.
2. Can be 1 1/2 for less than 4-hour fire rating, unless exposed to earth or weather - refer to code plan.

CONCRETE REINFORCING SPLICE TABLE
f'c = 4000 psi

Reinforcing Size	Tension Development Length Ld (inches)	Tension Development Length Top Bars, Ldt (inches)	Tension Lap Splice Ls (inches)	Tension Lap Splice Top Bars, Lst (inches)
#3	15	20	20	26
#4	19	25	25	33
#5	24	32	32	42
#6	29	38	38	50
#7	42	55	55	72
#8	48	63	63	82
#9	54	71	71	93
#10	60	78	78	102
#11	66	86	86	112

1. Lengths shown are for reinforcing satisfying the following spacing and cover dimensions (multiply lengths shown by 1.5 if these requirements are satisfied):
   a. walls and slabs - clear cover > db & clear spacing > 2db
   b. beams and columns - clear cover > db & clear spacing > db
2. Top bars are horizontal beam reinforcing with more than 12" of concrete below.
3. Tension lap splice lengths shown are for Class B splices (1.3xLd).
4. Multiply lengths by 1.3 for lightweight concrete.
5. For splices of different bar sizes, use max of Ld of larger bar or Ls of smaller bar.
6. Lap sheets of 6x6 Welded Wire Fabric Reinforcing a minimum of 8" each direction.

IFC SET

integrus ARCHITECTURE
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TELEPHONE: (206)624-3173 FAX: (206)624-3174

THOMAS H. DANIEL — 38621 — REGISTERED PROFESSIONAL ENGINEER — STATE OF WASHINGTON

City of Puyallup
Development & Permitting Services
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PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

Revisions

Date	Description
1.30.23	IFC Revisions

STRUCTURAL GENERAL NOTES

S001

WELDS

Reference Codes
- AWS D1.1/D1.1M-15, Structural Welding Code - Steel
- AWS D1.3-08, Structural Welding Code - Sheet Steel
- AWS D1.4/D1.4M-18, Structural Welding Code - Steel Reinforcing Bars
- AWS D1.8-/D1.8M-16, Structural Welding Code - Seismic Supplement
- AISC 341-16, Seismic Provisions for Structural Steel Buildings

Strength

Electrodes: Minimum yield strength = 70 ksi for all weld processes

1. Where welds are designated as demand critical, they shall be made with a filler metal capable of providing a minimum Charpy V-notch toughness of 20 ft-lb at -20 degrees F as determined by the appropriate AWS classification test method or manufacturer certification, and a minimum Charpy V-notch toughness of 40 ft-lb at 70 degrees F per AWS D1.8.
2. All other weld used in members and connections in the Lateral Force Resisting System shall be made with a filler metal that can produce welds that have a minimum Charpy V-notch toughness of 20 ft-lb at 0 degrees F, as determined by the appropriate AWS A5 classification test method or manufacturer certification.

Execution
1. Welding shall be performed in accordance with a welding procedure specification (WPS) as required in AWS D1.1. The WPS variables shall be within the parameters established by the filler metal manufacturer.
2. Field welding symbols have not necessarily been indicated on the drawings. Where no field welding symbols are shown, it is the Contractor's responsibility to coordinate the use of shop and field welds.

Submittals
1. All welders shall be AWS or WABO certified. Provide certification that each welder is prequalified per AWS D1.1 for each position and weld type that the welder will perform.
2. Provide written Welding Procedure Specifications adhering to AWS D1.1 for all welds to be performed. Welding Procedure Specifications must be approved by the Owner's testing agency prior to fabrication or erection.
3. Wherever a weld is included in the Lateral Force Resisting System, provide a Welding Procedure Specification which includes all essential variables of AWS D1.1 as well as the following:
   a. Type of power source (constant current or constant voltage)
   b. Manufacturer and trade name of electrodes used in Demand Critical welds
   c. Manufacturer's certification of notch toughness requirements for electrodes used in Demand Critical welds.

BOLTS/RODS

Reference Codes
- AISC, Specification for Structural Joints Using ASTM A325 or A490 Bolts.

Strength

Erection Bolts in Steel	ASTM A307, Snug Tight
All other Bolts in Steel	ASTM F1852, Twist-off Tension Control Bolts
All Bolts in Wood	ASTM A307
Anchor Rods	ASTM F1554, Grade 55
Anchor Rods at BRB Columns	ASTM F1554, Grade 105
Threaded Rods	ASTM A36
Hardener Steel Washers	ASTM F436
Nuts	ASTM A563, Heavy Hex Nuts

Execution
1. Place hardened steel washers between nuts and slotted or oversized holes.
2. All high-strength bolts shall be installed, tightened, and inspected in accordance with the AISC Specification for Structural Joints using ASTM A325 Bolts. The criteria for slip critical connections shall apply to all connections, unless noted as snug tight.
3. Use galvanized bolts, rods, nuts and washers wherever used in exterior applications and wherever connecting galvanized steel elements.

Submittals
1. Include anchor rod setting templates with all base plate shop drawings.

SHEAR CONNECTOR STUDS

Reference Codes
- AWS D1.1/D1.1M-15, Structural Welding Code - Steel
- AISC 360-16, Specifications for Structural Steel Buildings

Execution
1. Shear connectors shall be installed per manufacturer's recommendation, using the manufacturer's specified equipment.
2. Provide shear studs at a maximum spacing of 12" on center at all beams supporting a steel deck with concrete fill or a cast-in-place concrete slab, even if no stud quantity is shown in plan for that beam.

Submittals
1. Provide manufacturer's certification of conformity with AWS D1.1.

FASTENERS

Execution
1. At repetitive fasteners, place end fasteners no more than 6" from the end of the member.
2. Locate fasteners according to the drawings and with the following minimum edge distance and spacing, unless noted otherwise:

Fastener	Typical size, UNO	Substrate	Min Edge Dist.	Min Spacing
Powder Driven	0.157Ø x 1 long	Concrete	3"	4"oc
Powder Driven	0.157Ø x 1/2 long	Steel	1 1/2"	1 1/2"oc
Self-Tapping Screw	1/4Ø x 1 3/4 long	Concrete	2 1/2"	3"oc
Self-Tapping Screw	1/4Ø x 1 3/4 long	Steel	1 1/2"	1 1/2"oc

DEFORMED BAR ANCHORS

Reference Codes
- AWS D1.1/D1.1M-15, Structural Welding Code - Steel
- AISC 360-16, Specifications for Structural Steel Buildings

Strength

ASTM A496 and requirements from Type C studs in accordance with AWS D1.1

Execution
1. Deformed Bar Anchors shall be installed per manufacturer's recommendation, using the manufacturer's specified equipment

Submittals
1. Provide manufacturer's certification of conformity with AWS D1.1.

WOOD STRUCTURAL PANELS

Reference Codes
- International Building Code (IBC), 2018 edition, as adopted and amended by the project jurisdiction, Chapter 23

Strength

Plywood sheathing shall be grade Structural I.
Exterior glue shall be in conformance with the building code, United States Voluntary Product Standards PS-1 or PS-2.

Execution
1. Stagger panel end joints.
2. Provide approved edge clips at 24 inches on center at unblocked roof and floor sheathing edges.
3. Provide 1/8 inches gap between all abutting panel edges.
4. Provide the following minimum nailing unless noted otherwise on plans:
   a. 8d at 6 inches on center at all supported panel edges.
   b. 8d at 12 inches on center field nailing.

CONSTRUCTION ADMIN

Deferred Submittals

The following items are defined as Deferred Submittal Structural Components:
- Concrete mix
- Interior metal stud systems: walls, soffits, and ceilings
- Metal deck
- Fall restraint system
- Temporary shoring/bracing
- MEP anchorage and bracing
- Buckling restrained braces and their gusset plate connections
- Precast concrete elements
- Roof curbs for MEP equipment
- Vibration isolation system for MEP equipment
- Seismic bracing for architectural ceilings
- Stairs where indicated on plan

1. Deferred Submittal components are intended to be vendor designed and have not been designed by the Architect/Engineer. Refer to the drawings, the specifications, and the material sections of these general notes for design and submittal requirements.
2. Deferred Submittal components have not been permitted under the base building application. The contractor shall submit the approved submittals for the Deferred Submittal components to the building official. Deferred Submittal components shall not be installed until the building official has permitted the Deferred Submittal.
3. When the manufacturer is registered and approved to perform work without special inspection, the Contractor is to submit to the Building Official certificates of compliance from the manufacturers at the completion of manufacture stating that the work was performed in accordance with the approved construction documents.

Structural Observations
1. Structural Observation by the Structural Engineer is for the purpose of confirming general conformance of the construction with the design intent of the structure.
2. Structural observations will occur as required to confirm general conformance of the construction with the design intent of the structure.
3. Structural Observation is not intended to review the Contractor's construction procedures or the Contractor's conformance with the contract.
4. Structural Observation is not intended to replace the required Special Inspection program.

Special Inspection

Special inspections shall be performed by an agency which:
1. Is employed by the owner;
2. Is qualified to perform special inspection activities in accordance with IBC Section 1704.2;
3. Has been approved by the building official to perform special inspections.

The special inspector shall provide written documentation of qualifications per IBC section 1704.2.1.

The special inspector shall furnish inspection reports and a final report in accordance with IBC Section 1704.2.4.

The construction or work for which special inspection is required shall remain accessible and exposed of special inspection purposes until completion of the required special inspections.

Each contractor responsible for the construction of a main wind- or seismic force-resisting system, designated seismic system or a wind- or seismic-resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on the system or component. The contractor's statement of responsibility shall contain acknowledgement of awareness of the special requirements contained in the statement of special inspection.

Fabricators

The special inspector shall verify fabrication and implementation procedures of the following items per IBC Section 1704.2.5 unless the fabricator is registered and approved by the building official to perform such work without special inspection. At the completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction documents.
1. Structural Steel - to include the requirements of AISC 360 Chapter N.
2. Ready Mix Concrete
3. Light-gage Steel Studs
4. Light-gage Metal Deck

SUBMITTALS TO SPECIAL INSPECTOR		CERTIFICATE
Welding	Structural Steel	AWS D1.1 Prequalification
	Light Gage Steel	AWS D1.3 Prequalification
	Reinforcing	AWS D1.4 Prequalification
	Welding	WABO Certified (all welders)
Bolting	High Strength Bolts	Manufacturer's Letter of Certification
CMU Walls	Block Units	Manufacturer's Letter of Certification
	Grout	Manufacturer's Letter of Certification
	Mortar	Manufacturer's Letter of Certification
Adhesive Anchor	Epoxy Adhesive	Manufacturer's Letter of Certification
Concrete	All Concrete Material	All trip tickets, including water/cement ratio in truck
Slab-on-Grade	Concrete	In place water/cement ratio test results
Metal Deck Topping	Concrete	In place water/cement ratio test results

Inspector to review submittals before installation begins. Inspector must inspect job from approved & stamped design & shop drawings, including all clarifications, both written & in drawing form.

STATEMENT OF SPECIAL INSPECTIONS

Seismic Requirements (IBC 1704.3.2)

Special Inspections for Seismic Resistance (IBC 1705.12)

Seismic Structural Steel and Steel Elements (IBC 1705.12.1, 1705.12.1.2 and AISC 341 Chapter J):

Special inspection for structural steel seismic force-resisting systems (moment frames, braced frames, steel plate shear walls, etc) and structural steel elements (struts, collectors, chords, foundation elements, etc) shall be in accordance with the quality assurance (QA) requirements of AISC 341 and as noted below;
1. The special inspector shall provide written documentation of practices and personnel as required by AISC 341 sections J3 and J4.
2. Inspection of welding shall be in accordance with AISC 341 Section J6 and Tables J6-1, J6-2, and J6-3, inspection tasks prior, during, and after welding.
3. Inspection of high-strength bolting shall be in accordance with AISC 341 Section J7 and Tables J7-1, J7-2, and J7-3, inspection tasks prior, during, and after bolting.
4. Inspection of protected zones and reduced beam section moment frames shall be in accordance with AISC 341 section J8 and table J8-1
5. Inspection of composite structures shall be in accordance with AISC 341 section J9 and table J9-1, J9-2, and J9-3, inspection tasks prior, during and after concrete placement.

Structural Wood Construction (IBC 1705.12.2):

Structural Wood seismic force-resisting systems (shear walls, diaphragms, collectors, drag struts, hold downs, etc) shall be inspected in accordance with IBC Section 1705.12.2.

Structural Cold-Formed Steel Light Frame Construction (IBC 1705.12.3):

Cold-formed steel light frame seismic force-resisting systems (shear walls, braces, diaphragms, collectors, drag struts, hold downs, etc) shall be inspected in accordance with IBC Section 1705.12.3.

Designated Seismic Systems (IBC 1705.12.4):

The special inspector shall examine architectural, mechanical, and electrical components, supports, and attachments indicated in Table 13.2-1 of ASCE 7 and verify that the label, anchorage or mounting conforms to the certificate of compliance required by Sections 13.2.1 and 13.2.2 of ASCE 7.

Architectural Components (IBC 1705.12.5):

Periodic special inspection is required during erection and fastening of exterior cladding, interior and exterior veneer, and interior and exterior non-bearing walls over 30 feet in height above grade or walking surface, and for anchorage of access floors. Inspection and limitations to be per IBC Section 1705.12.5.

Mechanical, Electrical and Plumbing Components (IBC 1705.12.6):

Periodic special inspection is required during the installation and anchorage of the following items:
1. Electrical equipment for emergency or standby power systems.
2. Piping systems designed to carry hazardous materials and their associated mechanical units.
3. Ductwork designed to carry hazardous materials.
4. Vibration isolation systems where a nominal clearance of 1/4 inch (6.4 mm) or less is required between the equipment support frame and restraint.
5. Minimum clearances of Mechanical and electrical equipment, including duct work, piping systems and their structural supports per IBC section 1705.12.6.6 and ASCE 7 Section 13.2.3

Storage Racks (1705.12.7):

Periodic special inspection is required during the anchorage of storage racks 8 feet (2438 mm) or greater in height per IBC Section 1705.12.7.

Seismic Isolation Systems (IBC 1705.12.8)

Periodic special inspection is required during the fabrication and installation of seismic isolation systems in accordance with IBC Section 1705.12.8.

Cold-Formed Steel Special Bolted Moment Frames (IBC 1705.12.9)

Periodic special inspection is required during the installation of cold-formed special bolted moment frames in accordance with IBC Section 1705.12.9.

Testing and Qualification for Seismic Resistance (IBC 1705.13)

Seismic Structural Steel and Steel Elements (IBC 1705.13.1 and AISC 341):

Nondestructive testing for welded joints within the structural steel seismic force-resisting system shall be in accordance with section J6.2 of AISC 341. This includes testing of k-area welds, CJP Groove welds, where base material is greater than 1.5" thick, welded splices, thermally cut surfaces or beam copes and access holes, reduced beam section repairs, and weld tab removals.

Concrete Reinforcement (IBC 1705.12 Item 1 and 1705.12.1):

Certified mill test reports indicating compliance with Section 20.2.2 of ACI 318 shall be provided for each shipment of reinforcement used in those elements designated in the drawings as shear walls, moment frames, and coupling beams.

Seismic Certification of Designated Seismic Systems (IBC 1705.13.3)

Manufacturer's certification of seismic qualification in accordance with Section 13.2.2 of ASCE 7 shall be provided for active mechanical and electrical equipment that must remain operable following the design earthquake ground motion and for components with hazardous substances.

Seismic Certification of Architectural, Mechanical and Electrical Components (IBC 1705.13.2):

Manufacturer's certification of seismic qualification by analysis, testing or experience data in accordance with ASCE 7 Section 13.2 as well as the applicable sections indicated in ASCE 7 Table 13.2-1 shall be provided for architectural, mechanical and electrical components, supports and attachments per IBC Section 1705.13.2.

Wind Requirements (IBC 1705.11)

Special inspections for wind resistance are not required.

STATEMENT OF SPECIAL INSPECTIONS (cont.)

Steel Construction (IBC 1705.2 and AISC 360 Chapter N)

Special inspection for structural steel shall be in accordance with the quality assurance (QA) requirements of AISC 360.

Inspection of welding shall be in accordance with;
1. AISC 360 sections N4.2 and N5.4
2. AISC 360 Table N5.4-1 - Inspection Tasks Prior to Welding
3. AISC 360 Table N5.4-2 - Inspection Tasks During Welding
4. AISC 360 Table N5.4-3 - Inspection Tasks After Welding
5. Nondestructive testing of welded joints shall be in accordance with AISC 360 sections N4.3 and N5.5

Inspection of high-strength bolting shall be in accordance with;
1. AISC 360 sections N5.6
2. AISC 360 Table N5.6-1 - Inspection Tasks Prior to Bolting
3. AISC 360 Table N5.6-2 - Inspection Tasks During Bolting
4. AISC 360 Table N5.6-3 - Inspection Tasks After Bolting

Other Inspection tasks, including inspection of anchor rods and embedments supporting structural steel shall be in accordance with AISC 360 N5.8.

Inspection of composite construction, including steel deck, headed stud anchors shall be in accordance with;
1. AISC 341 Section J9
2. AISC 341 Tables J9.1 - J9.3 - Inspection of Steel Elements of Composite Construction Prior to Concrete Placement

Quality assurance inspections, testing and report submittal shall be in accordance with AISC 360 sections N5.2 and N7.

Cold-Formed Steel Deck (IBC 1705.2.2)

Special inspections and qualification of welding special inspectors for cold-formed steel floor and roof deck shall be in accordance with the quality assurance inspection requirements of SDI QA/QC and in accordance with IBC Section 1705.2.2.

Concrete Construction (IBC 1705.3)

Special inspection for concrete construction shall be in accordance with IBC Table 1705.3 - Required Verification and Inspection of Concrete Construction.

Masonry Construction (IBC 1705.4)

Special inspections and tests for masonry construction shall be in accordance with the quality assurance requirements of TMS 402-16 section 1.3 and TMS 602-16 section 1.6.

(use for risk categories I, II, or III)
1. Inspection and testing of CMU shall be in accordance with TMS 602-16 Table 3 and Table 4 (Level 2 Quality Assurance)
2. Inspection of Masonry Veneer shall verify compliance with the approved submittals. (Level 1 Quality Assurance)

(use for risk category IV)
1. Inspection and testing of CMU shall be in accordance with TMS 602-16 Table 3 and Table 4 (Level 3 Quality Assurance)
2. Inspection and testing of Masonry Veneer shall be in accordance with TMS 602-16 Table 3 and Table 4 (Level 2 Quality Assurance)

Soils (IBC 1705.6)

Special inspection for existing site soil conditions, fill placement and load-bearing requirements shall be in accordance with;
1. IBC 1705.6
2. IBC Table 1705.6 - Required verification and Inspection of Soils

Fabricated Items (IBC 1705.10)

Special inspections of fabricated items shall be in accordance with IBC Section 1705.10.

Sprayed Fire-Resistant Materials (IBC 1705.14)

Special inspection for sprayed fire-resistant materials shall be in accordance with IBC Sections 1705.14.1 though 1705.14.6

Mastic and Intumescent Fire-Resisting Coatings (IBC 1705.15)

Special inspection for mastic and intumescent fire-resistant materials shall be in accordance with IBC Sections 1705.15 and AWCI 12-B

Fire-Resistant Penetrations and Joints (IBC 1705.17)

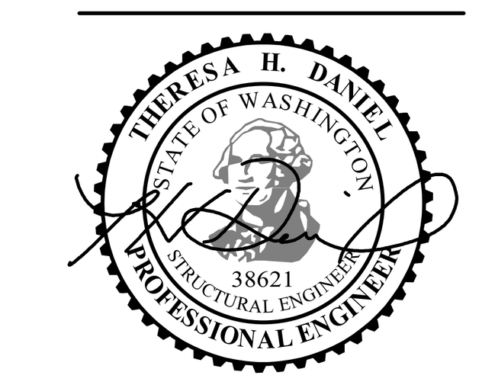
Special inspection for fire-resistant penetrations and joints shall be in accordance with IBC Sections 1705.17, 1705.17.1, and 1705.17.2

Smoke Control (IBC 1705.18)

~~Special inspection for smoke control systems shall be in accordance with IBC Sections 1705.18~~

PRCNC20221036
STRUCTURAL REVISION

integrus ARCHITECTURE



City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

Revisions

Date	Description
1.30.23	IFC Revisions

STRUCTURAL GENERAL NOTES

S002

IFC SET

ABBREVIATIONS

MISC

"	INCH, INCHES
#	NUMBER or POUND
%	PERCENT
&	AND
'	FOOT, FEET
=	EQUAL
@	AT
CL	CENTERLINE
FL	FLAT PLATE
L	ANGLE
PL	PLATE
SQ/FT	SQUARE FOOT
°	DEGREE
±	PLUS OR MINUS
Ø	DIAMETER
≤	LESS THAN OR EQUAL TO
≥	MORE THAN OR EQUAL TO

A

AB	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
ADD	ADDENDUM
ADDL	ADDITIONAL
ADH	ADHESIVE
ADJ	ADJACENT
AEC	ARCHITECTURAL, STRUCTURAL and ENGINEERING
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
AFF	ABOVE FINISH FLOOR
AHU	AIR HANDLING UNIT
AISC	AMERICAN INSTITUTE of STEEL CONSTRUCTION
ALT	ALTERNATE
ANCH	ANCHOR, ANCHORAGE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APPROX	APPROXIMATE
AR	ANCHOR ROD
ARCH	ARCHITECT, ARCHITECTURAL
ASCE	AMERICAN SOCIETY of CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
ATR	ALL THREADED REBAR
AWS	AMERICAN WELDING INSTITUTE

B

BAL	BALANCE
BCX	BOTTOM CHORD EXTENSION
BF	BRACED FRAME
BLDG	BUILDING
BLKG	BLOCKING
BM	BEAM
BOD	BOTTOM OF DECK
BOF	BOTTOM OF FOOTING, FOUNDATION
BOS	BOTTOM OF STEEL
BOT	BOTTOM
BP	BASE PLATE or BUTTON PUNCH
BRB	BUCKLING RESTRAINT BRACE
BRG	BEARING
BS	BOTH SIDES
BSMT	BASEMENT
BTWN	BETWEEN

C

(c)	COLLECTOR
C	CAMBER or CHANNEL
C-C	CENTER TO CENTER
C-GROUT	COURSE GROUT
CANT	CANTILEVER
CF	CUBIC FOOT
CFP	POLYFIBER REINFORCING (SEE SPECS)
CIP	CAST-IN-PLACE
CJ	CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLR	CLEAR, CLEARANCE
CLT	CROSS LAMINATED TIMBER
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
COMP	COMPOSITE or COMPRESSION
CONC	CONCRETE
CONFIG	CONFIGURATION
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
COORD	COORDINATE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
CTR	CENTER, CENTERED
CU	CUBIC
CW	CURTAIN WALL
CY	CUBIC YARD

D

d	PENNY (NAIL)
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DC	DEMAND CRITICAL
DEG	DEGREE
DEMO	DEMOLISH, DEMOLITION
DIA	DIAMETER
DIAG	DIAGONAL
DIAPH	DIAPHRAGM
DIM	DIMENSION
DISC	DISCONTINUE, DISCONTINUOUS
DL	DEAD LOAD
DN	DOWN
do.	DITTO
DP, D	DEEP, DEPTH
DTL	DETAIL
DWG, DWGS	DRAWING, DRAWINGS
DWL, DWLS	DOWEL, DOWELS

E

EA	EACH
EB	EXPANSION BOLT
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELECT	ELECTRICAL
ELEV	ELEVATOR
EMBED	EMBEDMENT
ENGR	ENGINEER
EOD	EDGE OF DECK
EOS	EDGE OF SLAB
EQ	EQUAL
EQP, EQUIP	EQUIPMENT
ES	EACH SIDE
EW	EACH WAY
EXIST, (E)	EXISTING
EXP	EXPANSION
EXT	EXTERIOR

ABBREVIATIONS

F

F-GROUT	FINE GROUT
FAB	FABRICATE
FB	FLAT BAR
FCJ	FLOOR CONTROL JOINT
FDN, FDTN	FOUNDATION
FFE	FINISH FLOOR ELEVATION
FIL	FILLET
FIN	FINISH
FLG	FLANGE
FLI	FERRULE LOOP INSERT
FLR	FLOOR
FOB	FACE OF BRICK
FOC	FACE OF CONCRETE
FOM	FACE OF MASONRY
FOS	FACE OF STUD
FOSW	FACE OF STEM WALL
FOV	FACE OF VENEER
FOW	FACE OF WALL
FRC	FLANGE BAR COUPLER
FRMG	FRAMING
FS	FAR SIDE
FT	FOOT, FEET
FTG	FOOTING

G

GA	GAUGE, GAGE
GALV	GALVANZIED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GLB	GLUE LAMINATED BEAM
GP	GUSSET PLATE
GR	GRADE, GRADING
GWB	GYPSUM WALL BOARD

H

HAS	HEADED ANCHOR STUD
HCP	HOLLOW CORE PLANK
HCS	HOLLOW CORE SLAB
HDG	HOT DIPPED GALVANIZED
HDR	HEADER
HGR	HANGER
HORIZ, H	HORIZONTAL
HP	HIGH POINT or BEARING PILE
HSB	HIGH STRENGTH BOLT
HSS	HOLLOW STRUCTURAL SECTION
HT	HEIGHT
HVAC	HEATING/ VENTILATION/ AIR CONDITIONING

I

IBC	INTERNATIONAL BUILDING CODE
ICBO	INTERNATIONAL CONFERENCE of BUILDING OFFICIALS
ICC-ES	ICC EVALUATION SERVICE
ICMU	INSULATED CONCRETE MASONRY UNIT
ID	INSIDE DIAMETER
IF	INSIDE FACE
IJ	ISOLATION JOINT
IN	INCH, INCHES
INCL	INCLUDED, INCLUDING
INFO	INFORMATION
INSUL	INSULATION
INT	INTERIOR
INV	INVERT

J

JST	JOIST
JT, JTS	JOINT, JOINTS

K

k, KIP	KILOPOUND
KSF	KIPS PER SQUARE FOOT
ksi	KIPS PER SQUARE INCH

L

L	ANGLE or LENGTH
LAT	LATERAL
LB	LAG BOLT OR POUND
LE	LEFT END
LF	LINEAR FOOT
LFRS	LATERAL FORCE RESISTING SYSTEM
LGT, LONG	LONGITUDINAL
LIB	LOAD INDICATOR BOLT
LIN	LINEAR
LIW	LOAD INDICATOR WASHER
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LNDG	LANDING
LNTL	LINTEL
LOC	LOCATION
LP	LOW POINT
LSH	LONG SLOTTED HOLE
LSL	LAMINATED STUD LUMBER
LVL	LAMINATED VENEER LUMBER

M

MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT (A-307)
MC	MISCELLANEOUS CHANNEL
MCR	MODIFIED CHLOROPRENE RUBBER
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, PLUMBING
MEZZ	MEZZANINE
MF	MOMENT FRAME
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
MLB	MICROLAM BEAM
MO	MASONRY OPENING
MPT	MAGNETIC-PARTICLE TEST
MT	STRUCTURAL TEE CUT FROM 'M' SECTION
MTL	METAL

N

N-GROUT	NON-SHRINK GROUT
NIC	NOT IN CONTRACT
NLB	NON-LOAD BEARING
NO	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE

O

OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OVERSIZED HOLE
OPNG	OPENING
OPP	OPPOSITE
OPP HD	OPPOSITE HAND
OPT	OPTIONAL
OSB	ORIENTED STRAND BOARD
OWJ	OPEN WEB JOIST

ABBREVIATIONS

P

PAF	POWER ACTUATED FASTENER
PC	PRECAST CONCRETE
PCF	POUNDS PER CUBIC FOOT
PDF	POWER DRIVEN FASTENERS
PEN	PENETRATION
PERIM	PERIMETER
PERP	PERPENDICULAR
PHW	PARTIAL HEIGHT WALL
PJP	PARTIAL JOINT PENETRATION
PL	PLATE
PLF	POUNDS PER LINEAL FOOT
PLYWD	PLYWOOD
PP	PARTIAL PENETRATION
PREFAB	PREFABRICATED
PRELIM	PRELIMINARY
PRSTR	PRESTRESSED
PSC	PRESTRESSED CONCRETE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSL	PARALLEL STRAND LUMBER
PT	POINT
PTC	POST-TENSION CONCRETE
PTFE	POLYTETRAFLUOROETHYLENE
PVC	POLYVINYL CHLORIDE

Q

QTY	QUANTITY

R

R	RADIUS
RC	REINFORCED CONCRETE
RE	REFER TO
REF	REFERENCE
REINF	REINFORCE(D), (ING), (MENT)
REQD	REQUIRED
RETG	RETAINING
REV	REVISION, REVISED
RFRE	RANDOM FIBER REINFORCED ELASTOMERIC
RJ	RUSTICATION JOINT
RO	ROUGH OPENING
RTU	ROOF TOP UNIT

S

S	AMERICAN STANDARD SHAPE
SC	SLIP CRITICAL
SCH, SCHED	SCHEDULE
SEC	SECURITY
SECT	SECTION
SF	SQUARE FOOT
SHT	SHEET
SHTG	SHEATHING
SIM	SIMILIAR
SOG	SLAB ON GRADE
SPA	SPACING
SPEC	SPECIFICATION
SQ	SQUARE
SS	STAINLESS STEEL
SSH	SHORT SLOTTED HOLE
SSPC	SOCIETY for PROTECTIVE COATINGS
ST	STRUCTURAL TEE CUT FROM 'S' SECTION
STD	STANDARD
STGD	STAGGERED
STIFF	STIFFENER
STL	STEEL
STR	STAIR
STRUCT	STRUCTURAL
SW	SHEAR WALL
SYM	SYMMETRICAL

T

T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
TCX	TOP CHORD EXTENSION
TEMP	TEMPERATURE, TEMPERED, TEMPORARY
TENS	TENSION
THK	THICK, THICKNESS
THRD	THREAD, THREADED
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOCW	TOP OF CONCRETE WALL
TOF	TOP OF FOOTING
TOP	TOP OF PARAPET
TOS	TOP OF STEEL
TOSLAB	TOP OF SLAB
TOW	TOP OF WALL
TPG	TOPPING
TRA	THREADED ROD IN ADHESIVE ANCHOR
TRANS	TRANSVERSE
TSF	THICKENED SLAB FOOTING
TYP	TYPICAL

U

UBC	UNIFORM BUILDING CODE
UFRC	UPSET END FLANGE REBAR COUPLER
UNO	UNLESS NOTED OTHERWISE
UT	ULTRASONIC TEST

V

VERT, V	VERTICAL
VIF	VERIFY IN FIELD
VMS	VERTICAL MOVEMENT SYSTEM
VNR	VENEER

W

W	WIDTH or WIDE FLANGE
W/	WITH
W/O	WITHOUT
WABO	WASHINGTON ASSOCIATION of BUILDING OFFICIALS
WCJ	WALL CONTROL JOINT
WD	WOOD
WF	WIDE FLANGE
WP	WORKING POINT
WPS	WELDING PROCEDURES SPECIFICATIONS
WS	WELD STUD
WT	WEIGHT or STRUCTURAL TEE CUT FROM 'W' SECTION
WWF	WELDED WIRE FABRIC

X

X-STR	EXTRA STRONG
XX-STR	DOUBLE EXTRA STRONG

LEGEND

COLUMNS

- HSS Column in Plan
- Pipe Column in Plan
- Wide Flange Column in Plan

MEMBERS

- 20LH 280/140 — Open Web Steel Designation *[Depth] [Type] [Total Load]/[Live Load]* (PLF) (PLF)
- 16" I-Joist@24, 110/60 — Wood I-Joist Designation *[Depth] I-Joist @ Spacing [Total Load (PLF)]/[Live Load (PLF)]*
- [#] W12x26 [#], C=__ N=__ — [Beam Reaction] Beam Size [Beam Reaction]
- Camber & Number of Weld Studs Designation

CONNECTIONS

- Denotes Shear Connection
- Backspan / Cantilever — Denotes Cantilever Moment Connection (Size of Cantilever Beam matches the size of the Backspan Beam, UNO)
- Denotes Thermal Break Connection
- Denotes Braced Frame
- At Bottom Flange — Bottom Flange Bracing

WALLS

- Existing Concrete in Plan or Section
- Cast-in-Place Concrete in Plan or Section
- Precast Concrete in Plan or Section
- CMU in Plan or Section
- Structural Stud Wall in Plan
- Full Height Wall below
- NLB — Full Height Non-Load Bearing Wall below
- Lintel Beam in Plan

SOILS

- Natural Grade/Earth
- Structural Fill
- Compacted Gravel
- Capillary Break

LINEWORK

- Matchline
- Floor Control Joint (FCJ)

ANNOTATION/SYMBOLS

- S S S / S S S — Stepped Footing in Plan
- 100'-0" T.O.F. — Top of Footing Elevation
- EL. = 100'-0" — Top of Slab Elevation
- DEPTH — *Depth of Depression*; Slab Depression; *Extent of Depression in Plan*
- Direction of Framing Designation; Extent of Framing Designation
- Hold Down
- Column Mark / Note — Column Size Designation
- CMU Designation
- Structural Stud Shear Wall Designation
- Column/Wall Footing Designation
- Timber Connector Designation

REFERENCE SYMBOLS

- 1 / S101 — Direction of View; Detail/Section Number Designation; Sheet Detailed on Designation
- 1 / S101 — Detail/Section Number Designation; Sheet Detailed on Designation
- A — Reference for Portion of Details

PRCNC20221036
STRUCTURAL REVISION

IFC SET

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THOMAS H. DANIELS — 38621 — STATE OF WASHINGTON — STRUCTURAL ENGINEER — PROFESSIONAL ENGINEER

City of Puyallup
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Fire	Traffic

PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

Revisions

Date	Description
1.30.23	IFC Revisions

STRUCTURAL
GENERAL NOTES

S003

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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING
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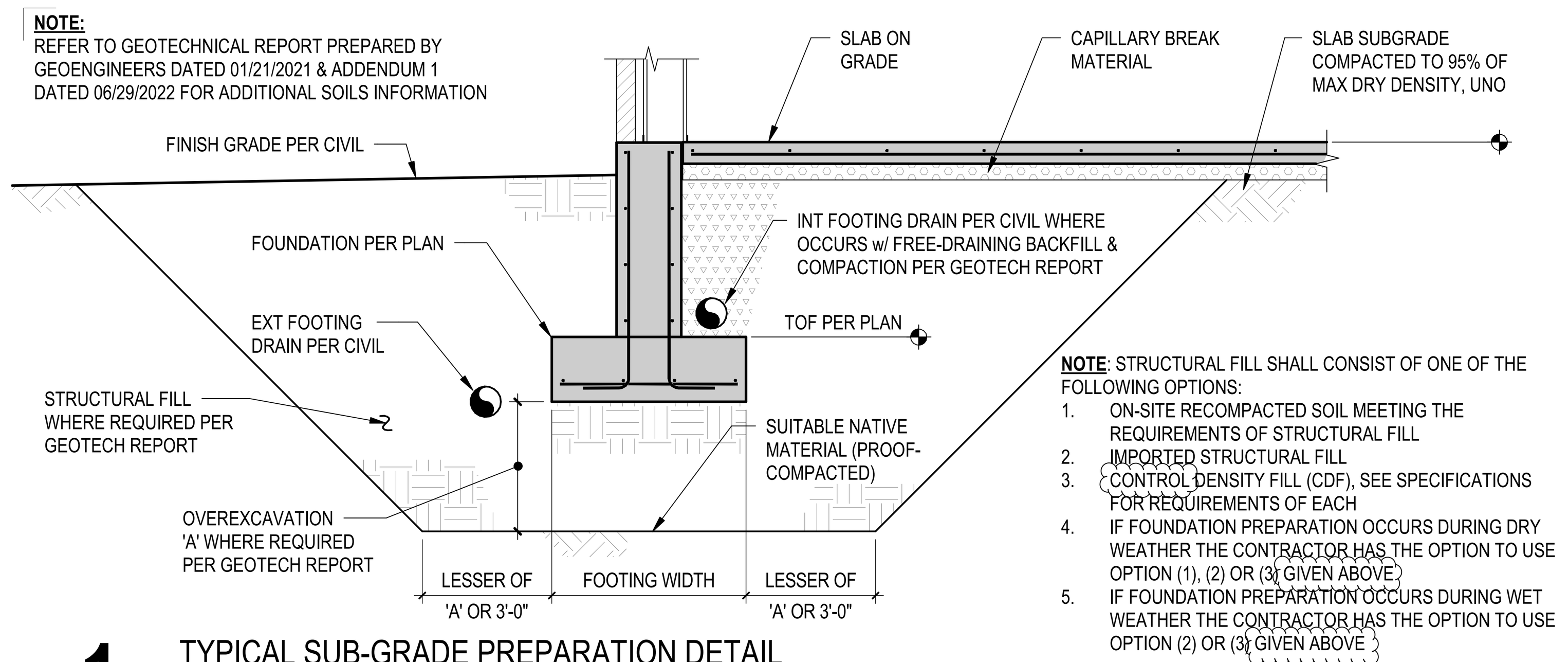
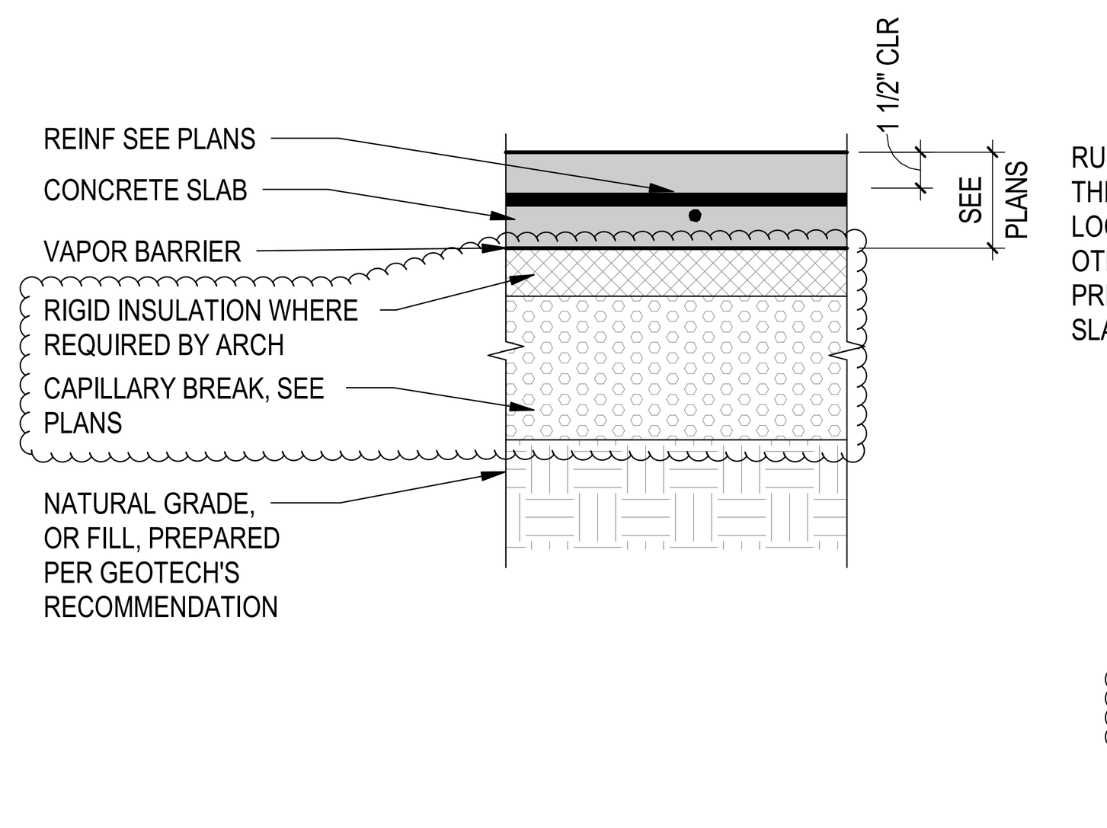
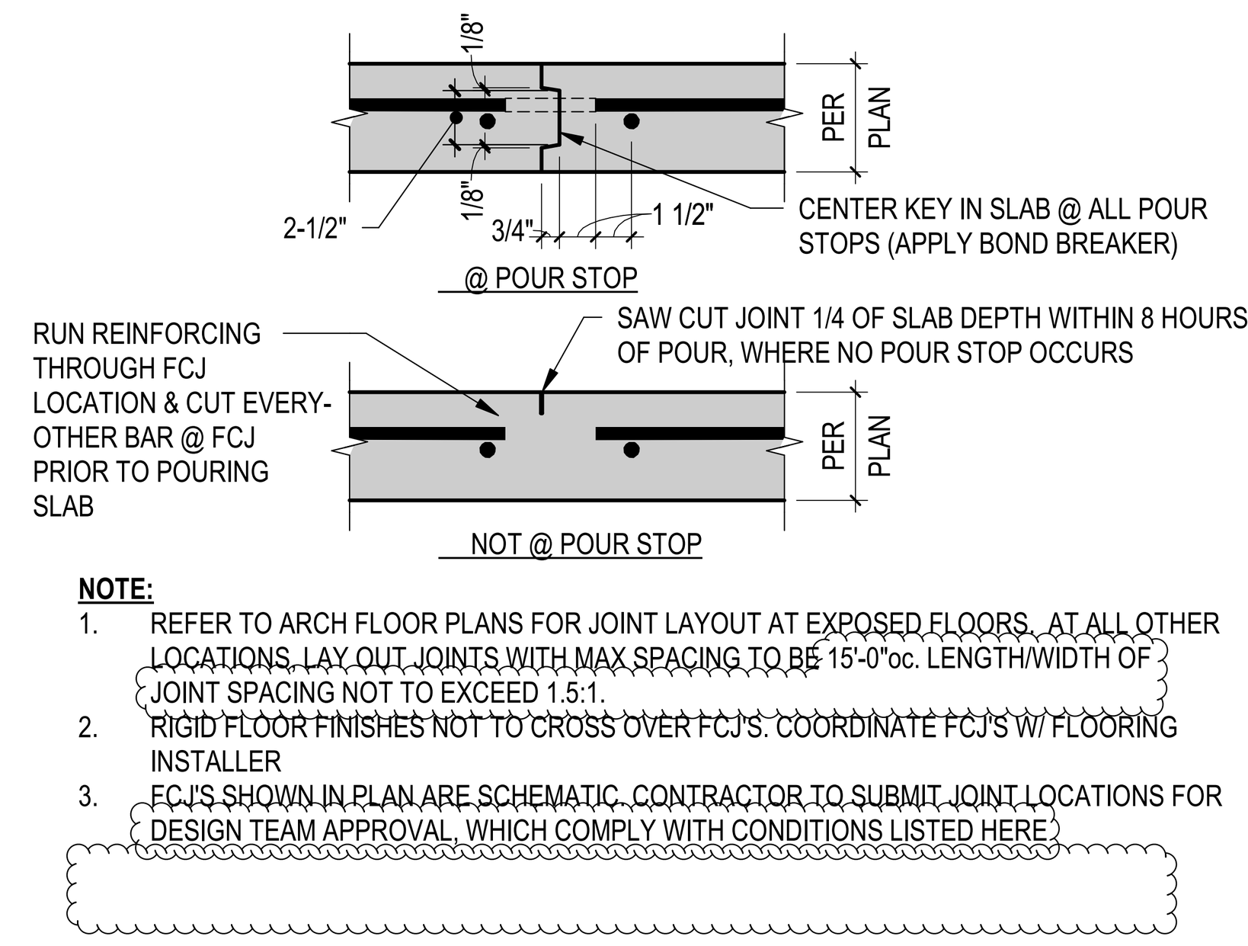


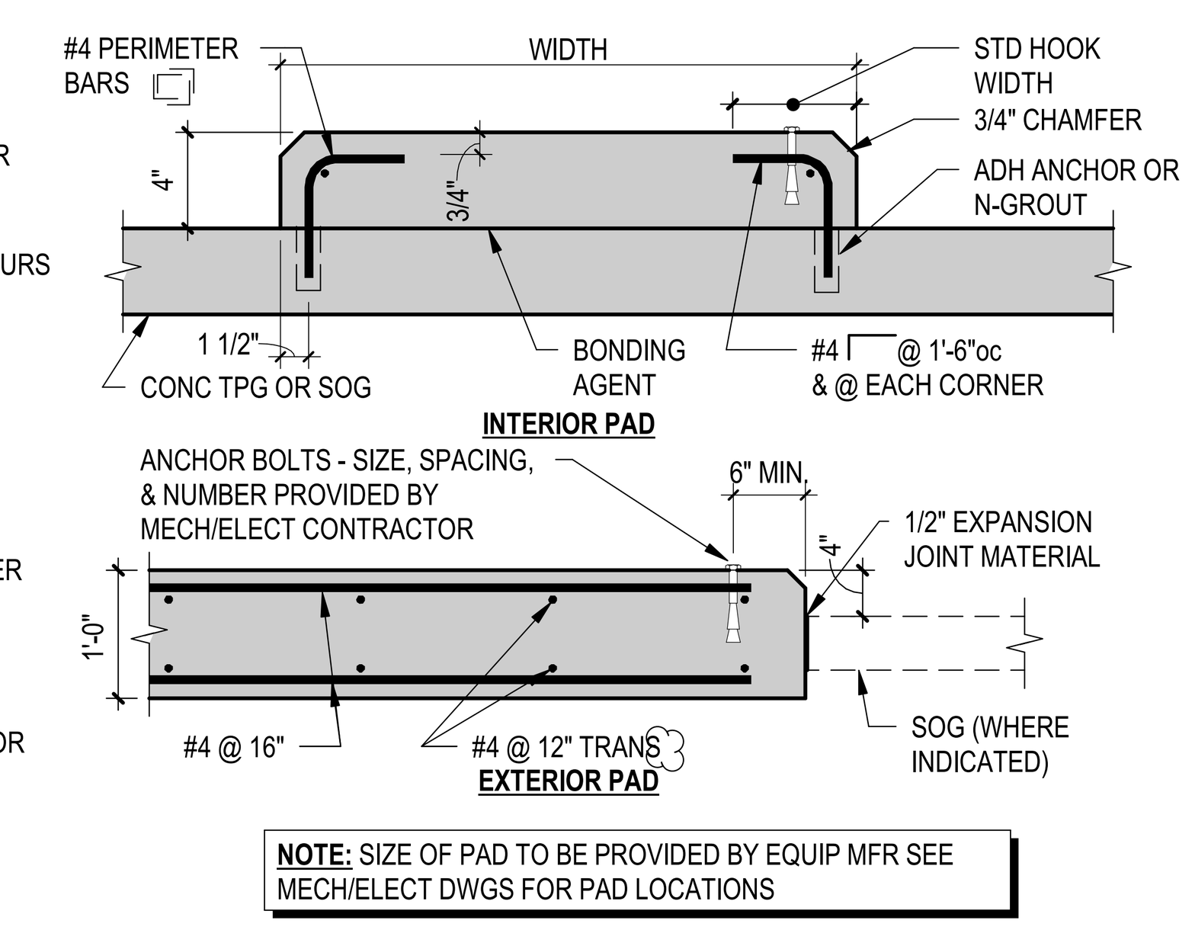
1 TYPICAL SUB-GRADE PREPARATION DETAIL
SCALE: 1/2" = 1'-0"



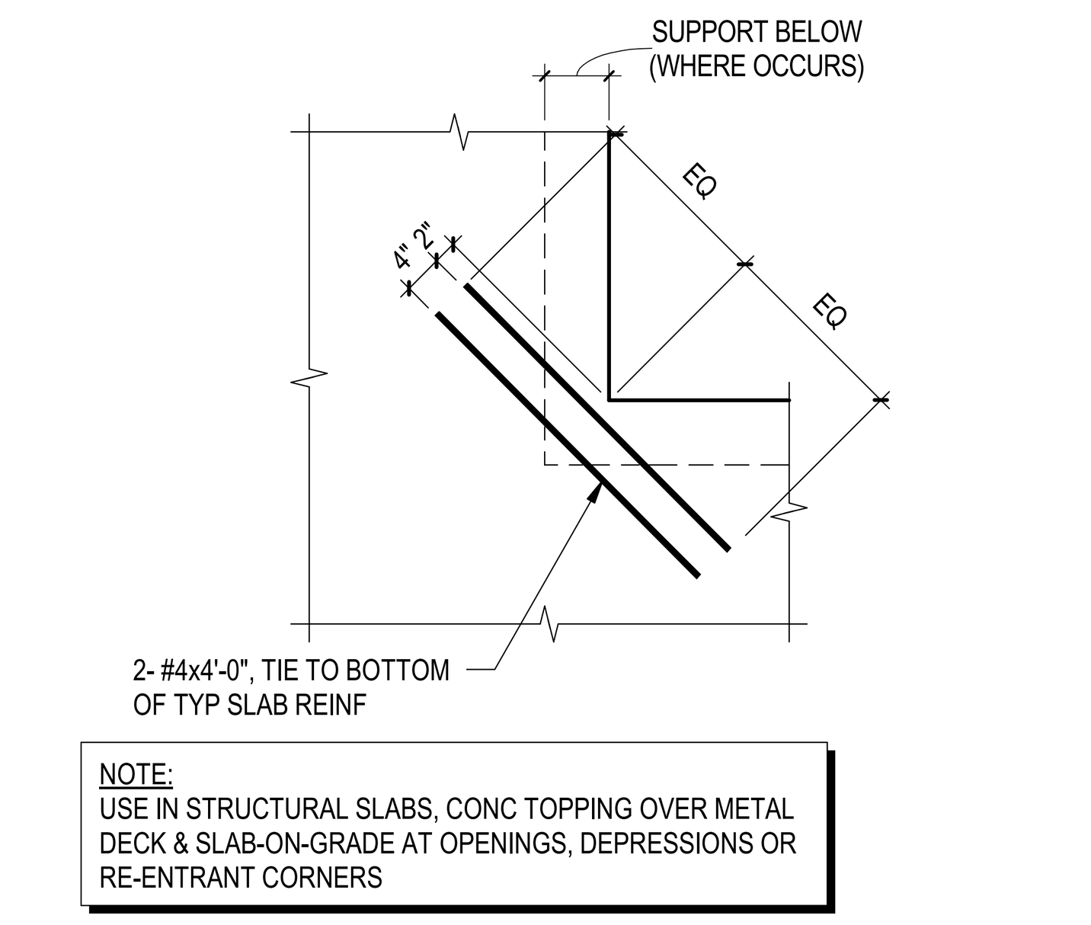
3 TYPICAL SLAB-ON-GRADE SECTION
SCALE: 1 1/2" = 1'-0"



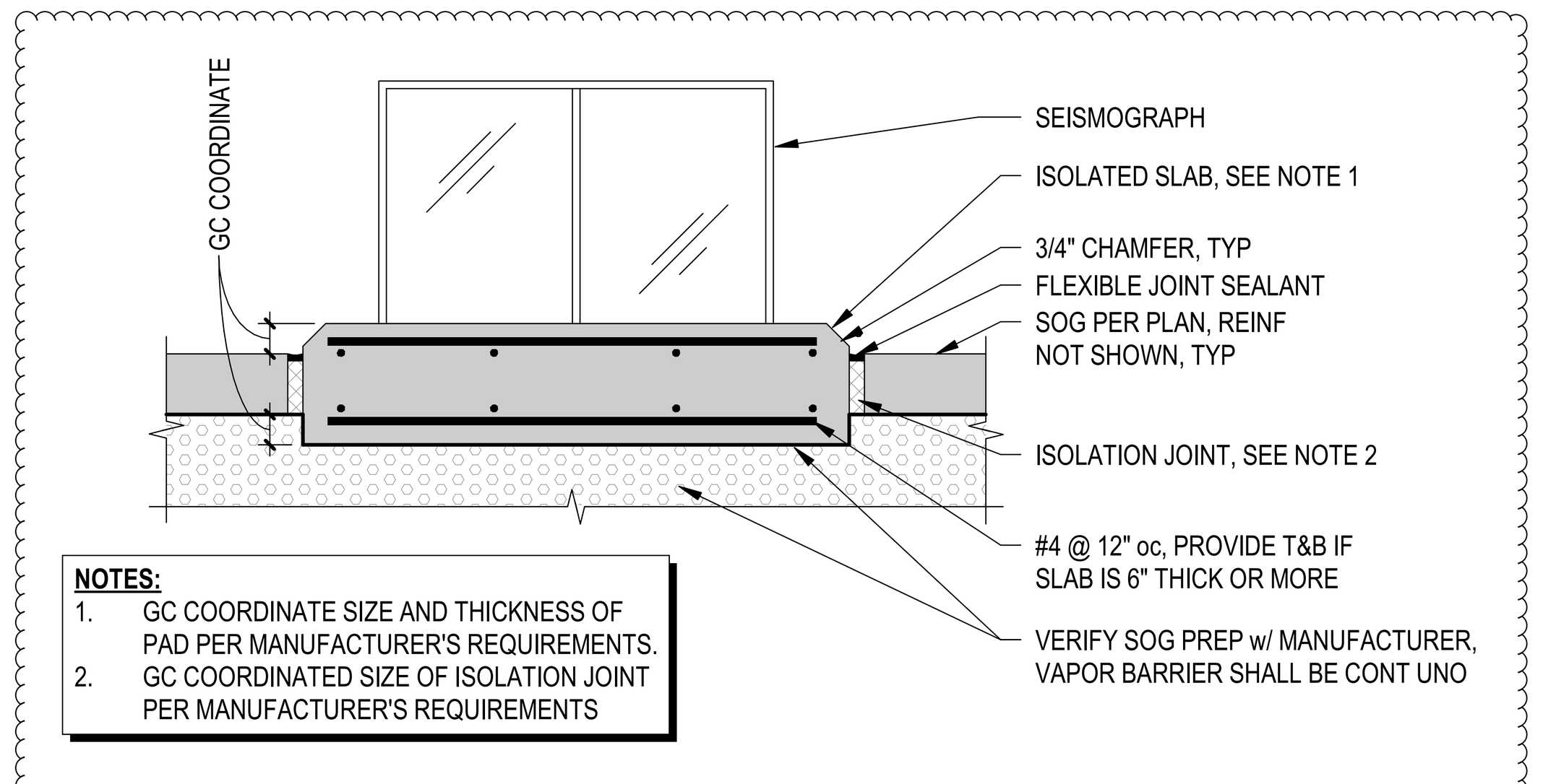
4 FLOOR CONTROL JOINTS (FCJ) INTERIOR SLAB-ON-GRADE
SCALE: 1 1/2" = 1'-0"



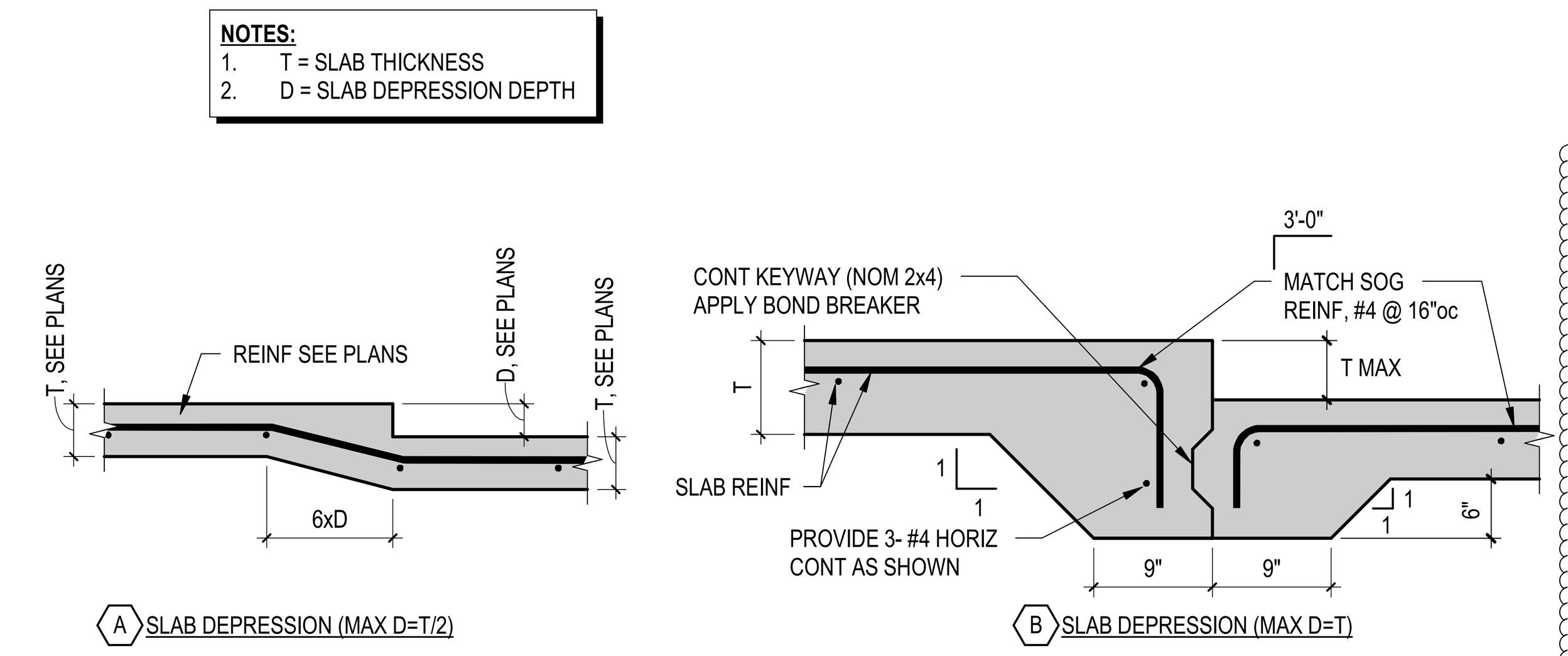
5 MECH/ELECT EQUIPMENT PAD
SCALE: 1" = 1'-0"



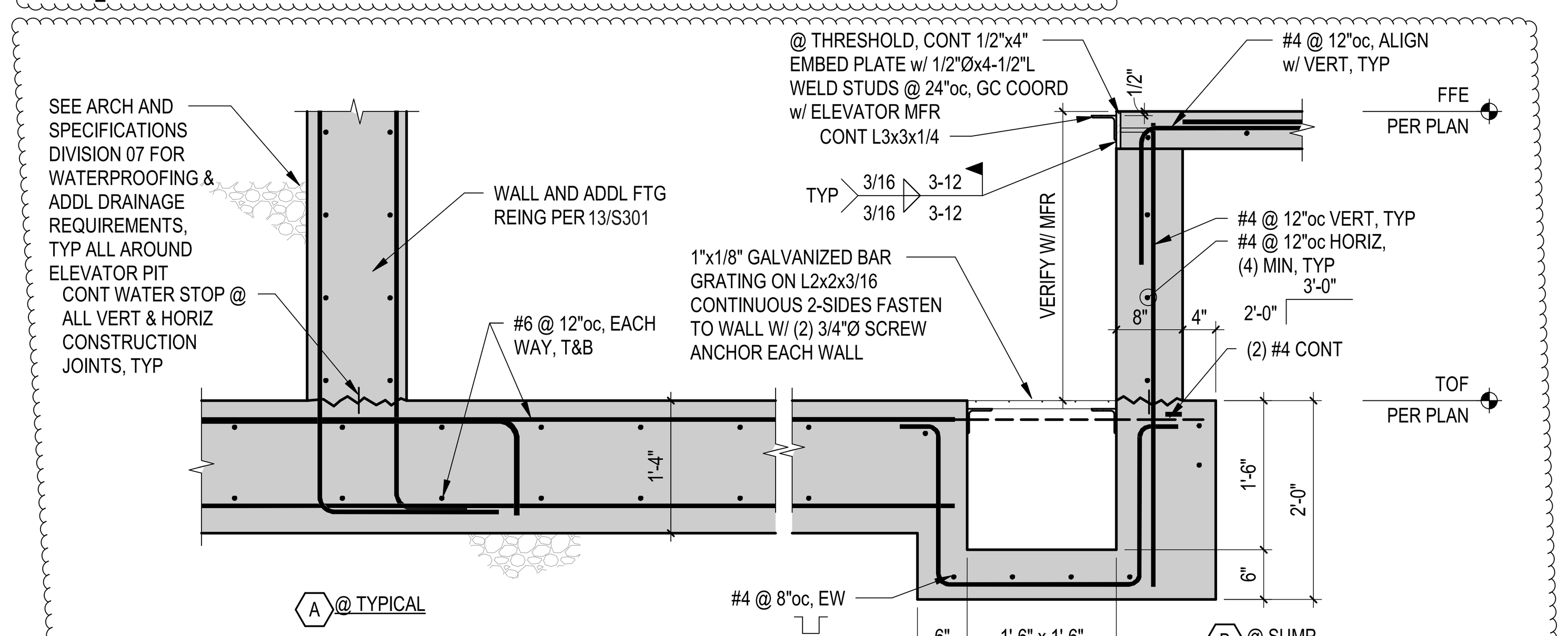
6 ADDITIONAL SLAB REINF AT REENTRANT CORNERS
SCALE: 3/4" = 1'-0"



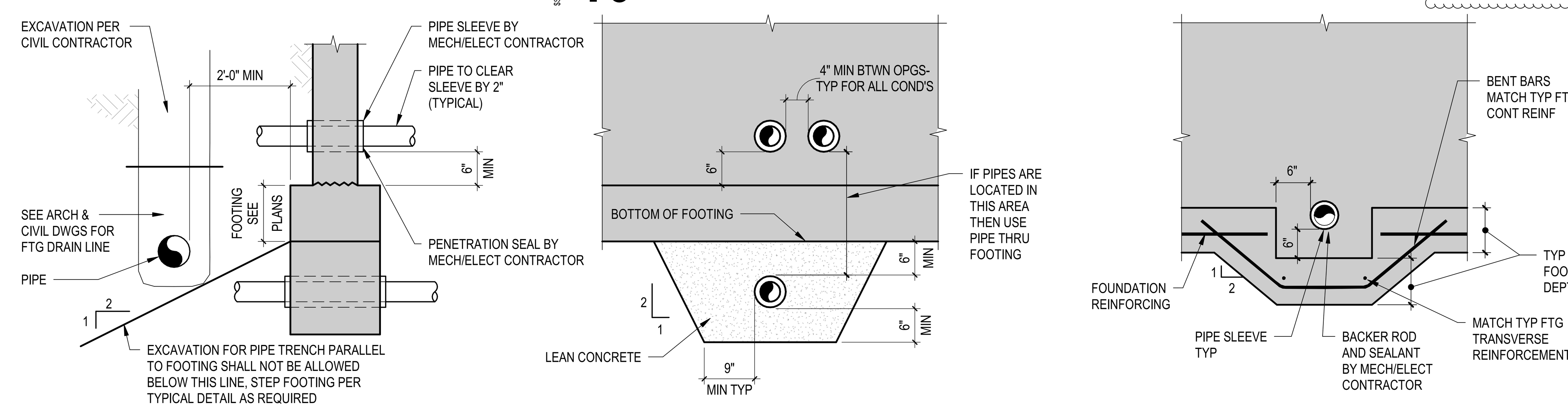
7 ISOLATED SEISMOGRAPH PAD
SCALE: 1" = 1'-0"



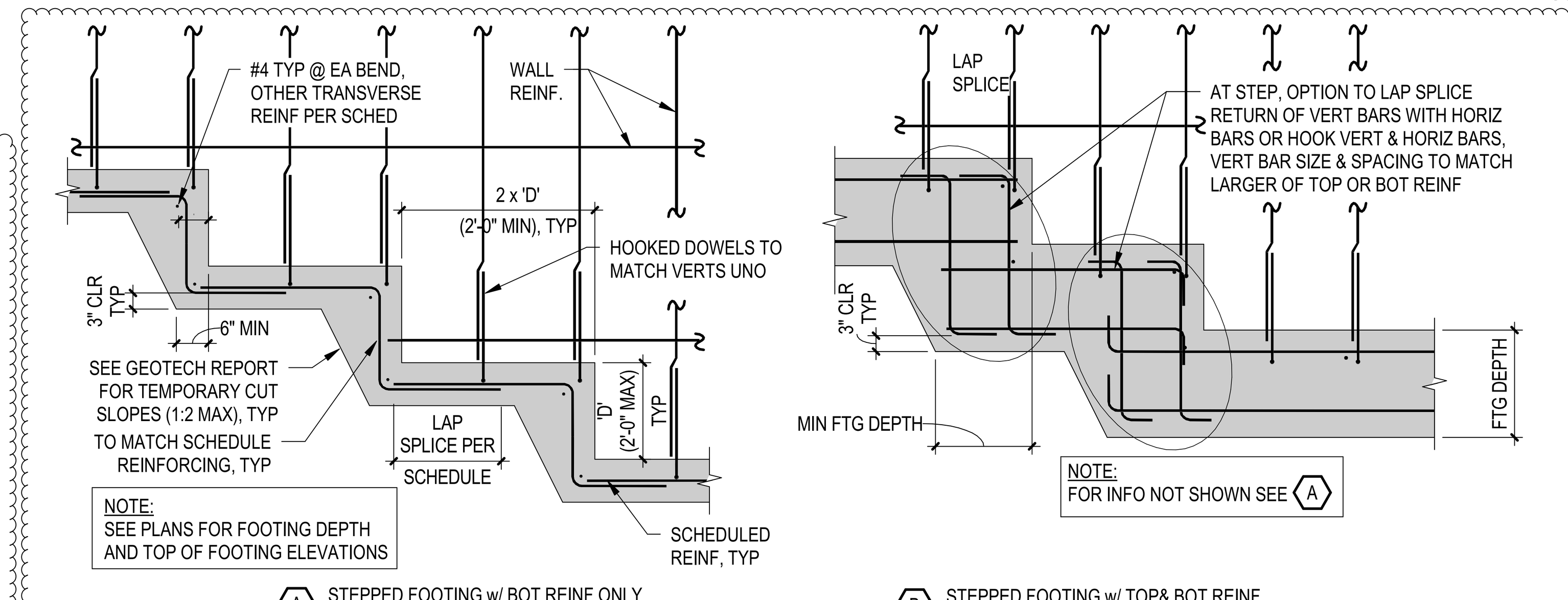
10 SLAB-ON-GRADE ELEVATION CHANGE
SCALE: 1" = 1'-0"



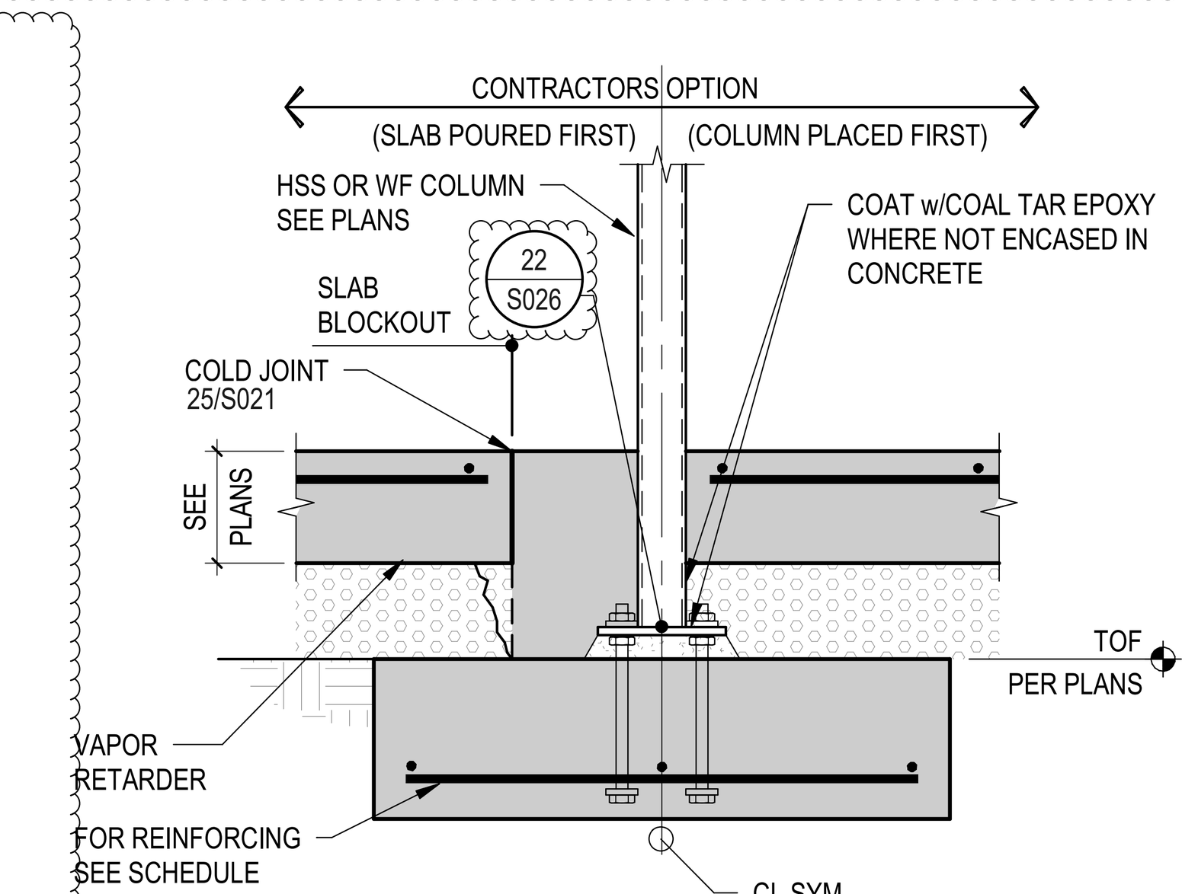
13 ELEVATOR AND SUMP PIT
SCALE: 3/4" = 1'-0"



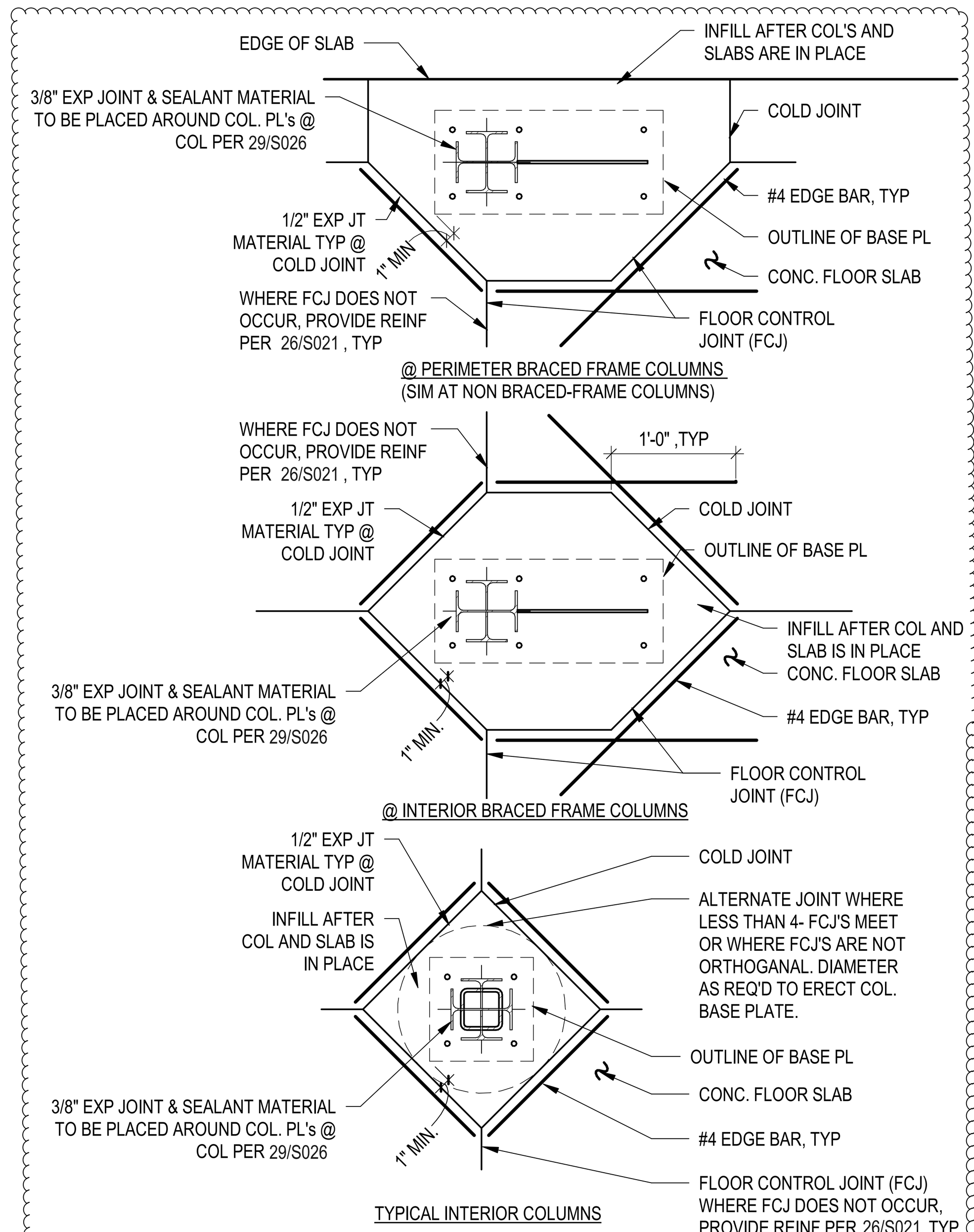
15 TYPICAL PIPE AT CONCRETE WALL FOOTING
SCALE: 3/4" = 1'-0"



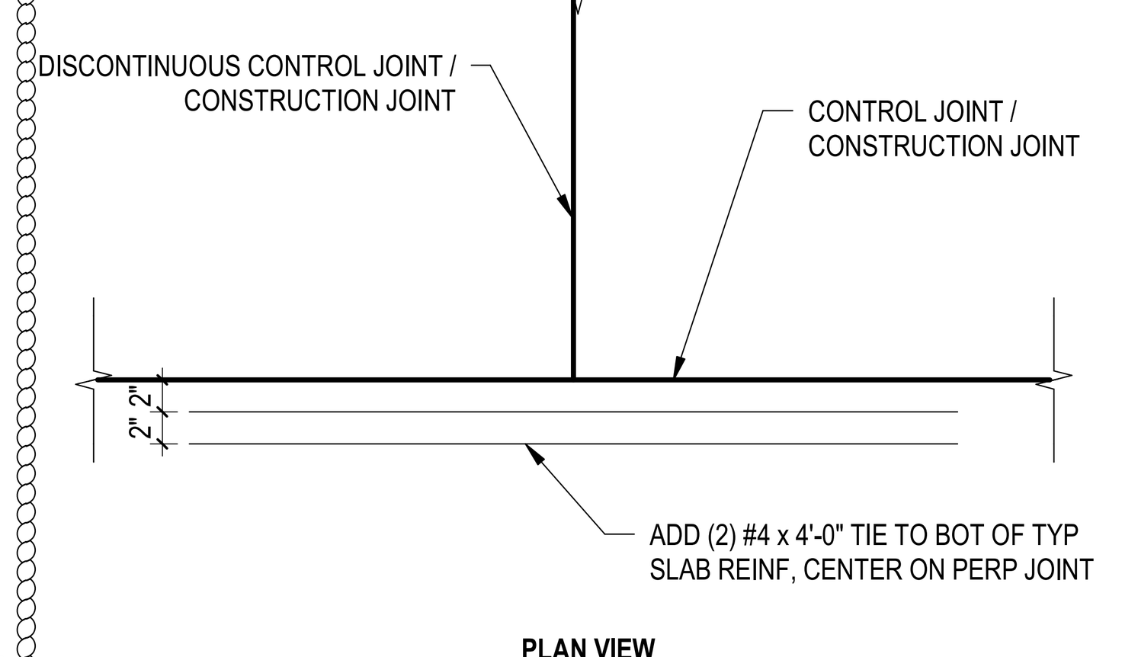
22 STEPPED FOOTING AT CONCRETE WALLS
SCALE: 1/2" = 1'-0"



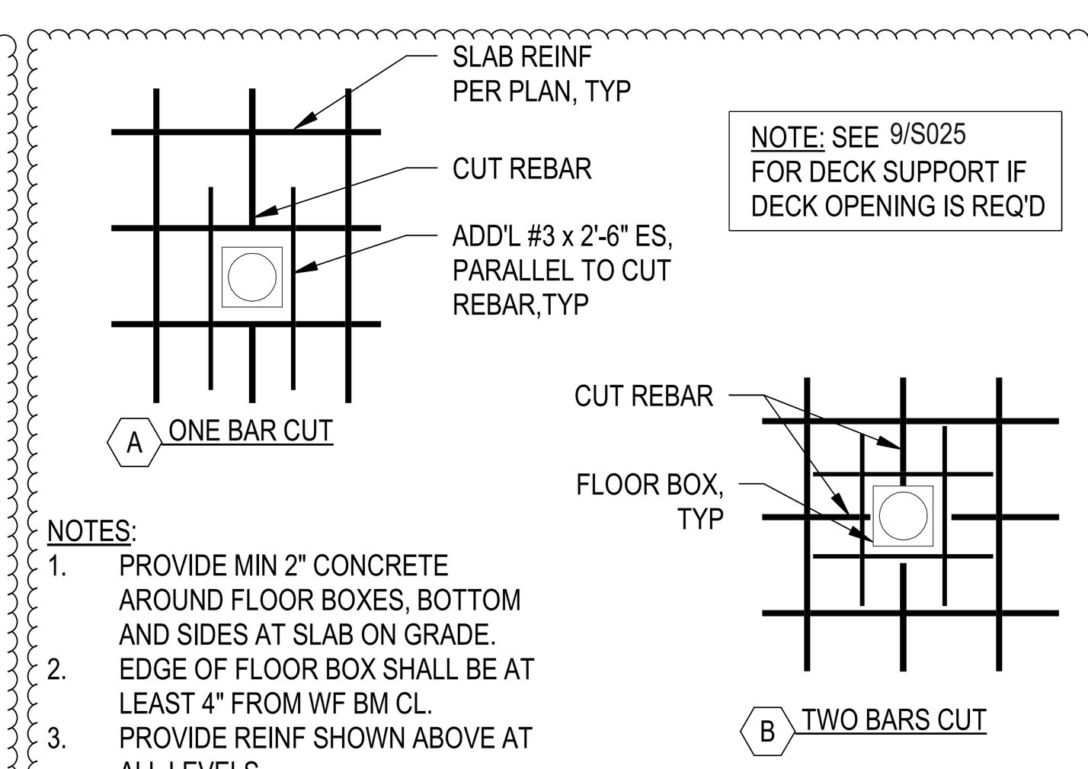
24 COLUMN BASE ON CONCRETE FOOTING
SCALE: 1" = 1'-0"



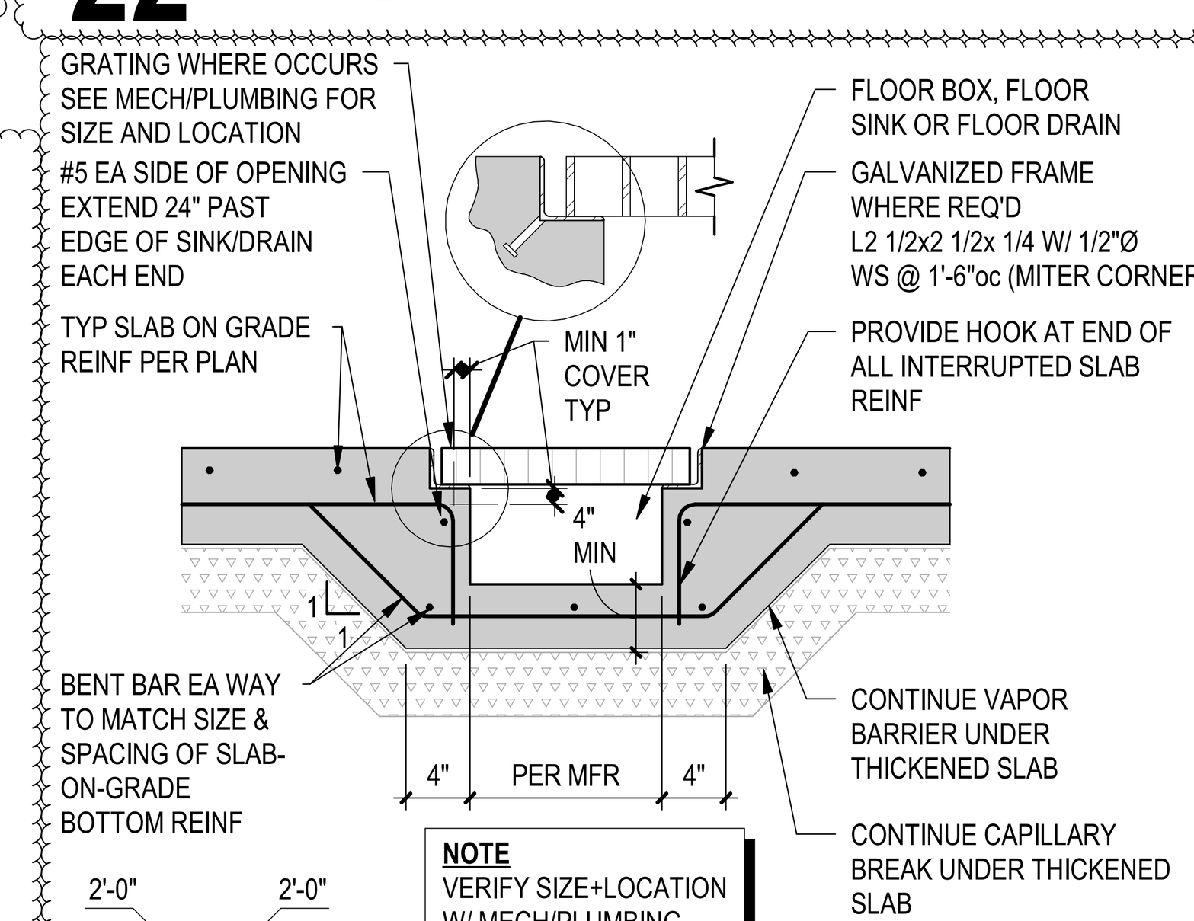
25 FLOOR CONTROL JOINT AT COLUMN
SCALE: 1" = 1'-0"



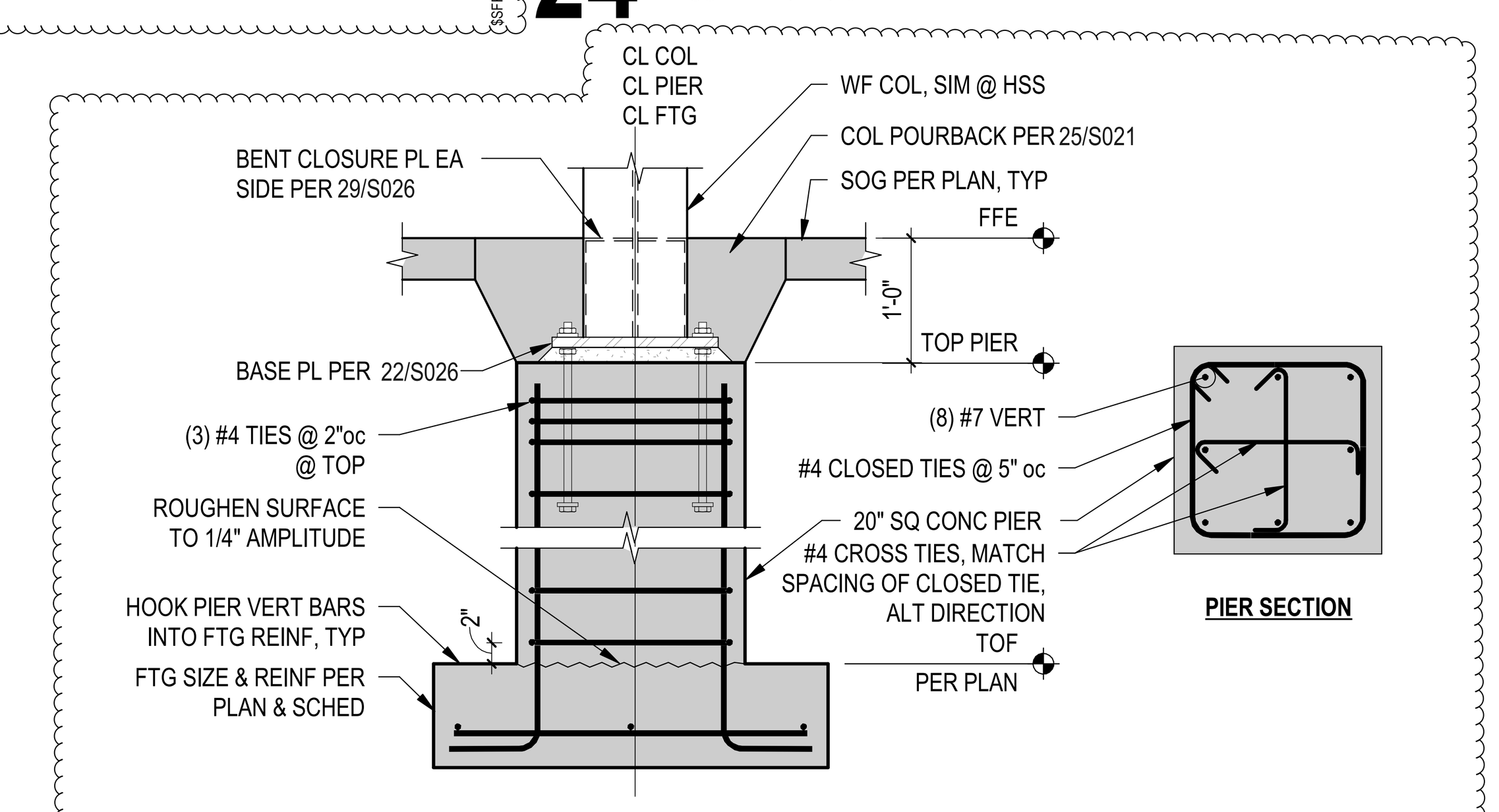
26 DISCONTINUOUS CONTROL JOINT
SCALE: 1" = 1'-0"



27 REINF AT ELECTRICAL FLOOR BOXES
SCALE: 3/4" = 1'-0"



28 ELEC BOX/FLOOR SINK
SCALE: 1" = 1'-0"



30 GRAVITY COLUMN PIER DETAIL
SCALE: 3/4" = 1'-0"

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM/RB
Checked by:	TD

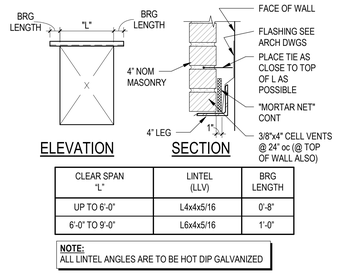
Revisions

Date	Description
1.30.23	IFC Revisions

TYPICAL FOUNDATION DETAILS

S021

IFC SET



ELEVATION SECTION

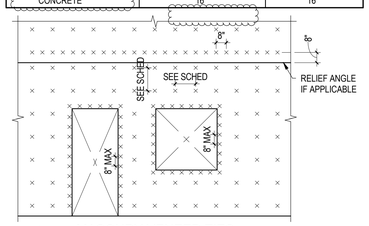
CLEAR SPAN "L"	LINTEL (LLV)	BRG LENGTH
UP TO 6'-0"	L4x4x5/16	0'-8"
6'-0" TO 9'-0"	L6x4x5/16	1'-0"

NOTE:
ALL LINTEL ANGLES ARE TO BE HOT DIP GALVANIZED

5 MASONRY VENEER LINTEL SCHEDULE

SCALE: NTS

VENEER TIE SPACING SCHEDULE		
SUBSTRATE	HORIZONTAL SPACING	VERTICAL SPACING
METAL STUDS @ 16oc	16"	16"
CONCRETE	16"	16"



6 MASONRY VENEER TIES

SCALE: 1/4" = 1'-0"

ANCHOR	WELD STUDS	SCREW ANCHOR	EXPANSION BOLTS	ADHESIVE ANCHOR	ANCHOR RODS
TYPE	HEADED	FLUSH	BOLT	INJECTION	HEADED
DIAMETER	LENGTH	EMBED	EMBED	EMBED	EMBED
1/4"	4 1/8"	1"	2"	3"	---
3/8"	4 1/8"	1 1/2"	3 1/2"	4"	---
1/2"	5 5/16"	2"	4"	4 1/2"	6"
5/8"	6 9/16"	5"	4 1/2"	5 1/2"	7 1/2"
3/4"	6 3/16"	6"	5"	6 1/2"	9"
7/8"	7 3/16"	---	5 1/2"	7 1/2"	10 1/2"

NOTES:
1. ALL ANCHORS IN CMU TO BE IN FULLY GROUTED CELLS & ARE NOT ALLOWED IN HEAD JOINTS
2. SCREEN SLEEVES REQUIRED FOR ADHESIVE CONNECTION IN CMU & BRICK
3. EMBEDMENTS INDICATED ARE MINIMUMS. SEE DETAILS FOR LOCATIONS REQUIRING ADDITIONAL EMBEDMENT

12 STANDARD ANCHOR EMBEDMENT

SCALE: 3/4" = 1'-0"

PRCNC20221036
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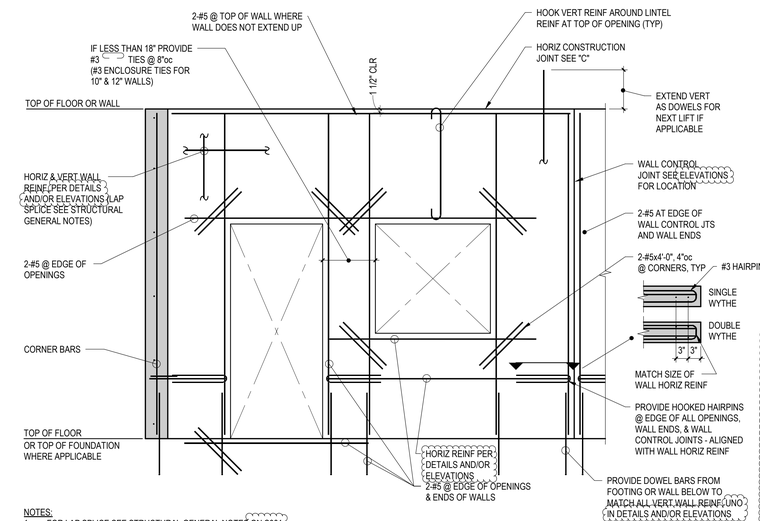
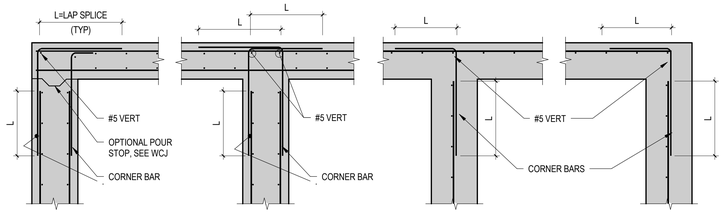


NOTES:
1. FOR LAP SPLICE SEE STRUCTURAL GENERAL NOTES ON S001
2. ALL VERTICAL REINFORCING IN CONCRETE WALLS SHALL BE CONTINUOUS FROM STRUCTURAL FLOOR TO STRUCTURAL FLOOR OR FROM TOP OF FOOTING TO FIRST STRUCTURAL FLOOR ABOVE UNLESS NOTED OR SHOWN OTHERWISE IN SECTION
3. SEE DETAILS AND/OR ELEVATIONS FOR CONCRETE WALL REINFORCING SIZES AND SPACING

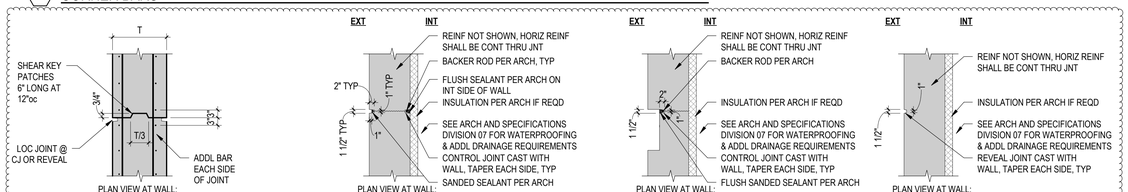
25 CONCRETE WALL REINFORCING SCHEDULE AND PLACEMENT GUIDE

SCALE: 1/2" = 1'-0"



NOTES:
1. FOR LAP SPLICES SEE STRUCTURAL GENERAL NOTES
2. CORNER BARS SAME SIZE AND SPACING AS HORIZ REINF
3. FOR CORNER BARS INTERRUPTED BY OPENINGS, HOOK AROUND OPENING VERTS w/180° HOOK

A CORNER BARS



PLAN VIEW AT WALL: VERTICAL CONSTRUCTION JOINT

VERTICAL CONSTRUCTION JOINT NOTES:
1. ALL REINF PERPENDICULAR TO JOINT TO CONTINUE THROUGH JOINT
2. LOCATE JOINT NEAR THE 1/3 POINT OF THE SPAN BUT NOT AT A PLANNED BAR. SPLICE, UNO ON DWGS
3. CLEAN JOINT OF LAITANCE, & PAINT w/BONDING AGENT PRIOR TO SECOND POUR
4. NOT REQUIRED TO EXTEND THROUGH WALL FOOTING

PLAN VIEW AT WALL: CONTROL JOINTS

CONTROL JOINT NOTES:
1. LOCATE PER ARCH AT EXPOSED LOCATIONS
2. AT L0 TO L1 INTERIOR WALLS NOT EXPOSED TO VIEW, LOCATE JOINTS AT 30'-0" MAX, SUBMIT JOINT LAYOUT TO EOR FOR APPROVAL PRIOR TO CONSTRUCTION
3. DO NOT EXTEND JOINT THROPUGH WALL FOOTING

PLAN VIEW AT WALL: CONTROL JOINTS @ DOWNSPOUT RECESS

CONTROL JOINTS @ DOWNSPOUT RECESS NOTES:
1. LOCATE PER ARCH
2. SEE 9/S301 FOR ADDITIONAL INFO
3. DO NOT EXTEND JOINT THROPUGH WALL FOOTING

PLAN VIEW AT WALL: ARCH REVEAL

ARCH REVEAL NOTES:
1. LOCATE PER ARCH
2. DO NOT EXTEND JOINT THRU FOOTING

B VERTICAL CONSTRUCTION JOINT (CJ)

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THOMAS H. DANIELS — STATE OF WASHINGTON — 38621 — STRUCTURAL ENGINEER — PROFESSIONAL ENGINEER

City of Puyallup
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Building	Planning
Engineering	Public Works
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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM/BO/RB
Checked by:	TD

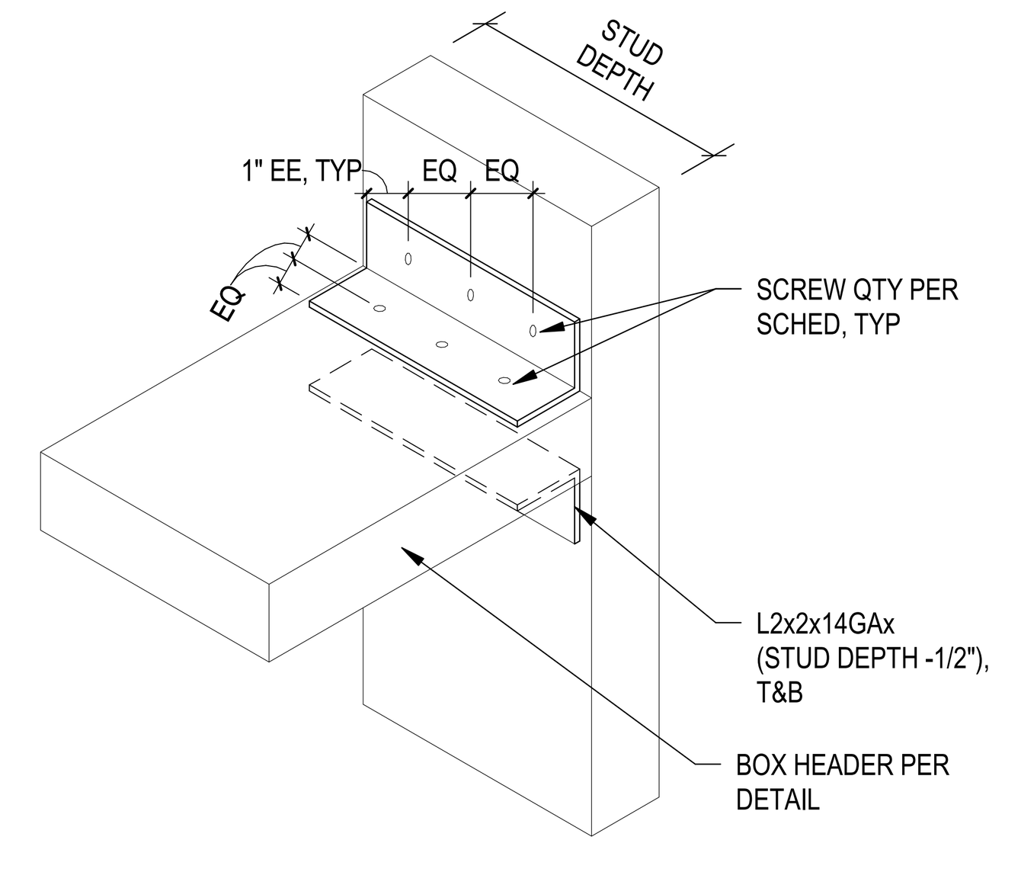
Revisions

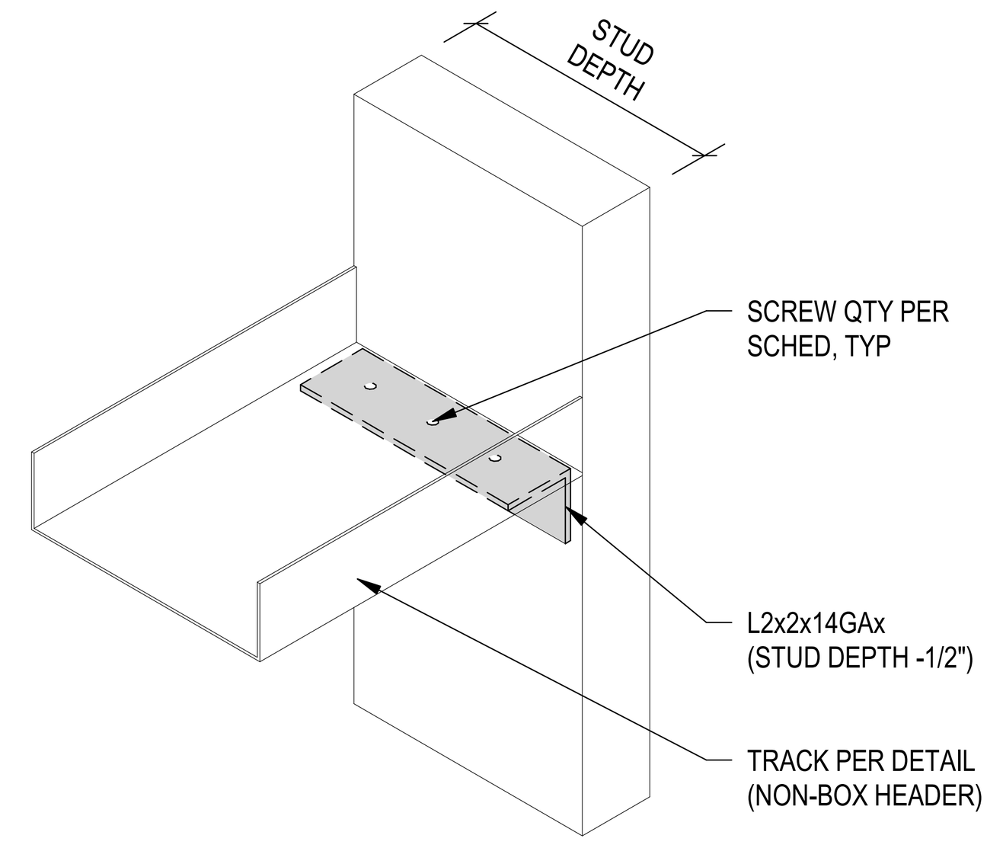
Date	Description
1.30.23	IFC Revisions

TYPICAL CONCRETE AND VENEER DETAILS

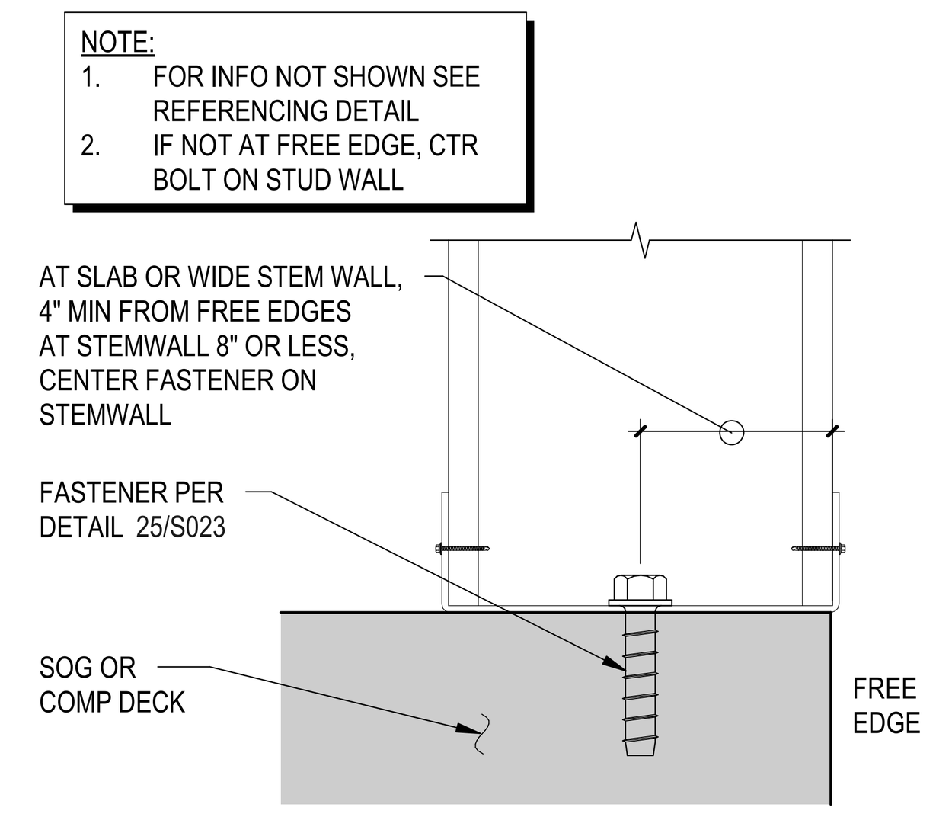
S022

IFC SET

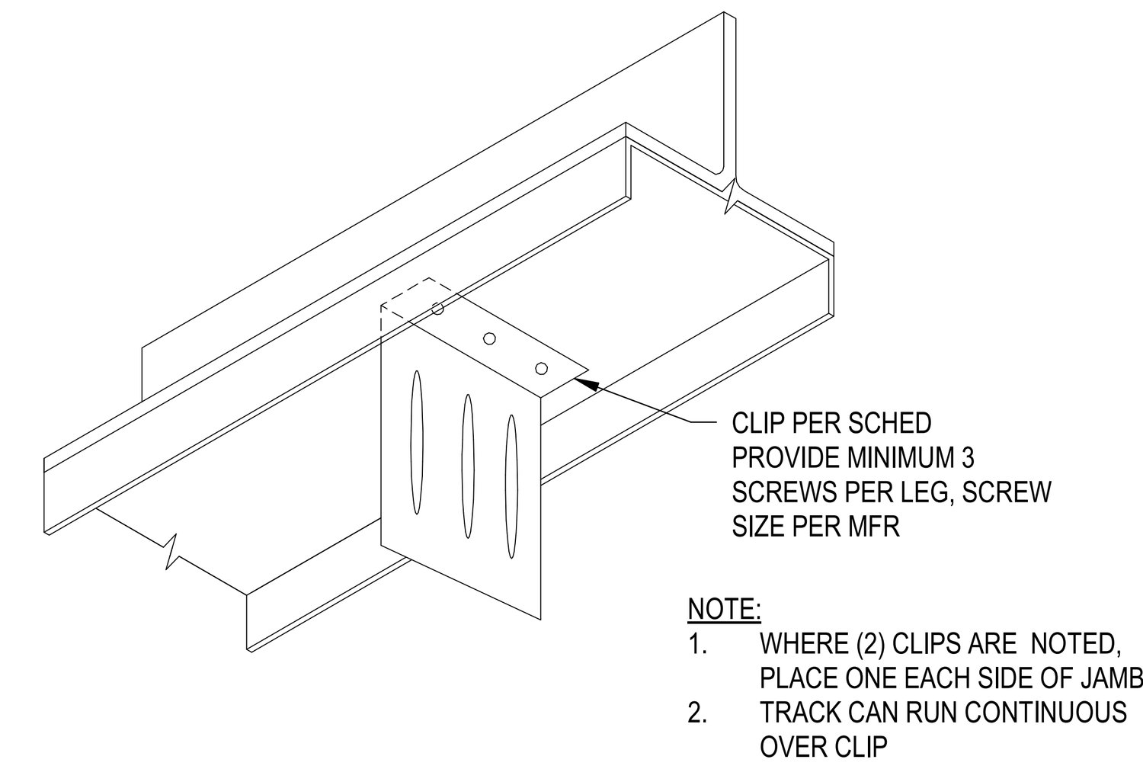




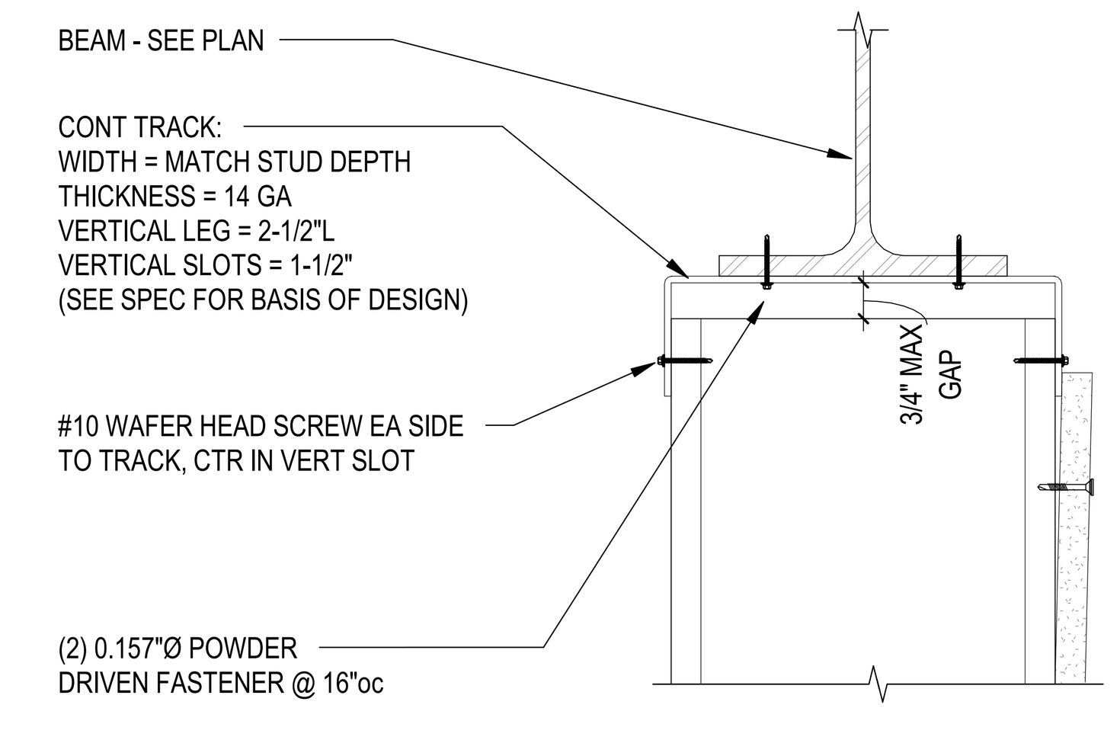
1 HEADER/SILL AT BOX BEAM
SCALE: 3" = 1'-0"



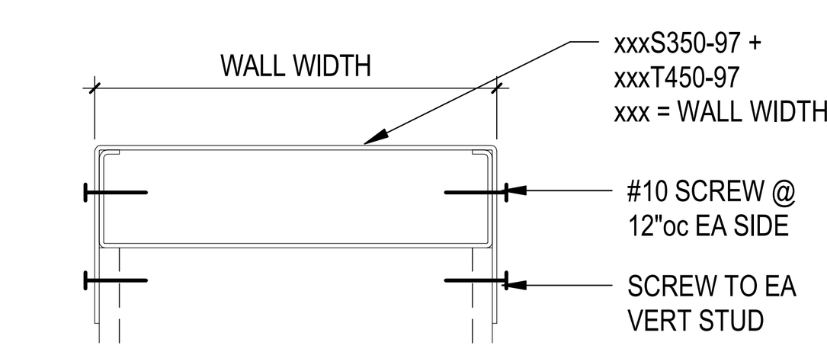
3 TRACK ATTACHMENT DETAIL
SCALE: 3" = 1'-0"



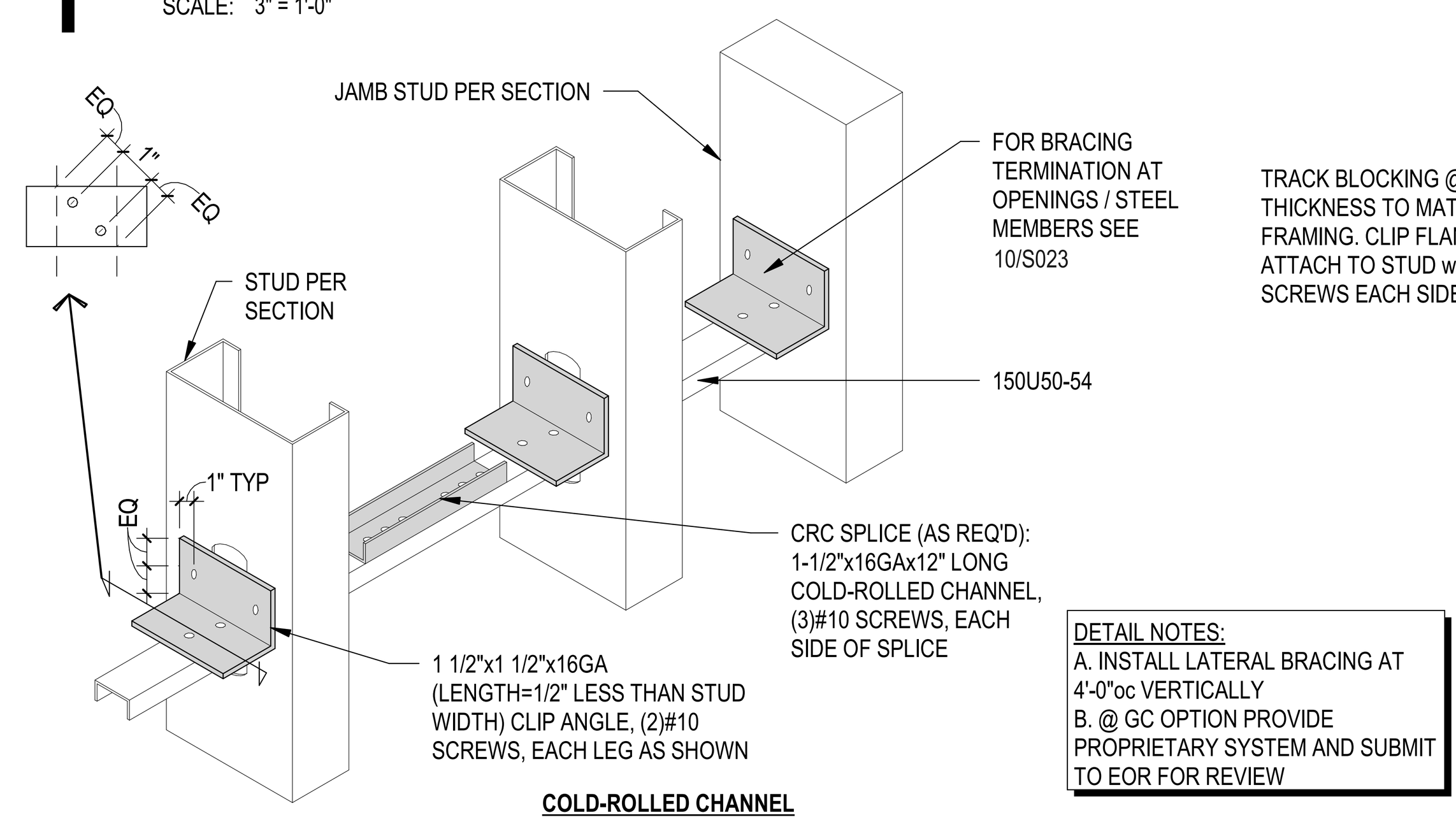
4 JAMB CONN DETAIL
SCALE: 1 1/2" = 1'-0"



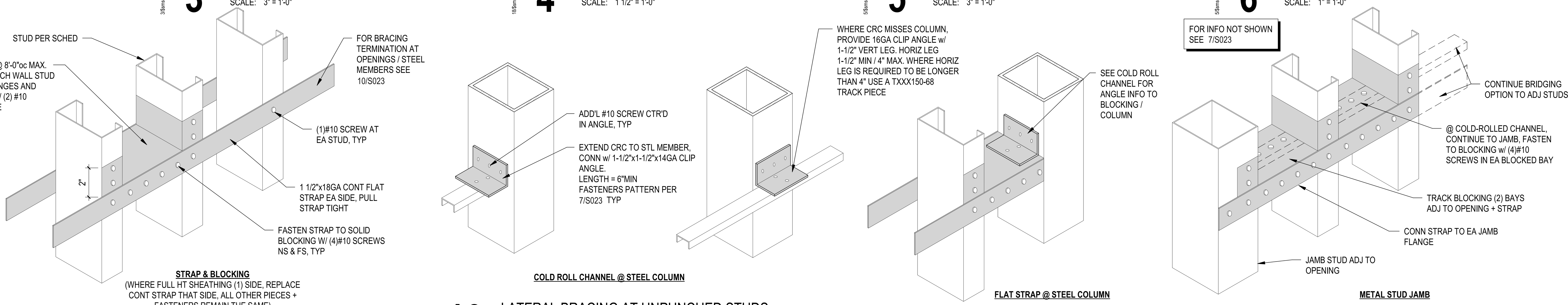
5 DEFLECTION TRACK AT BOS
SCALE: 3" = 1'-0"



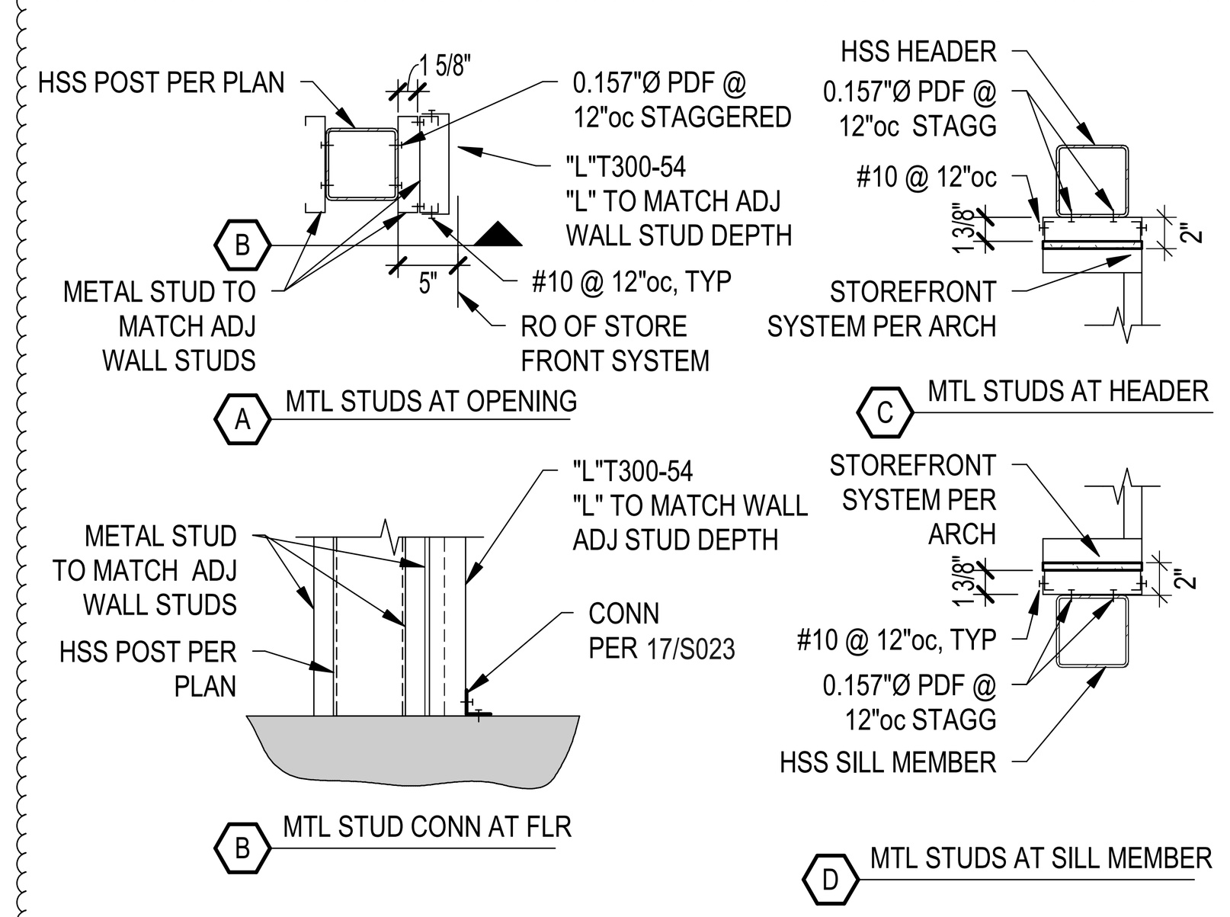
6 SILL DETAIL
SCALE: 1" = 1'-0"



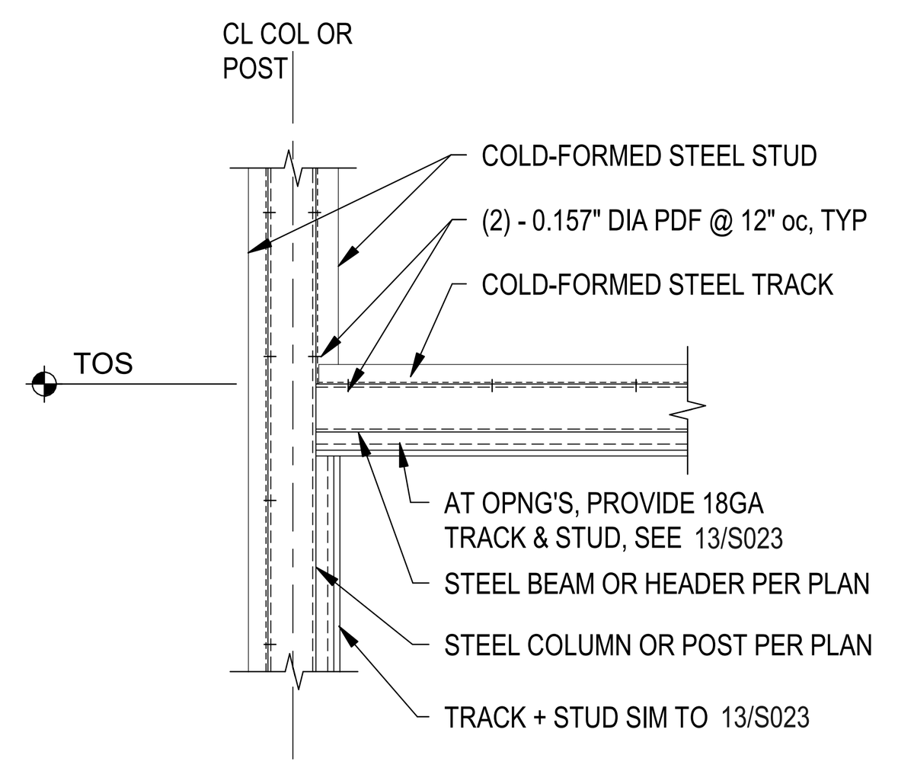
7 LATERAL BRACING AT PUNCHED STUDS ALTERNATIVES
SCALE: 3" = 1'-0"



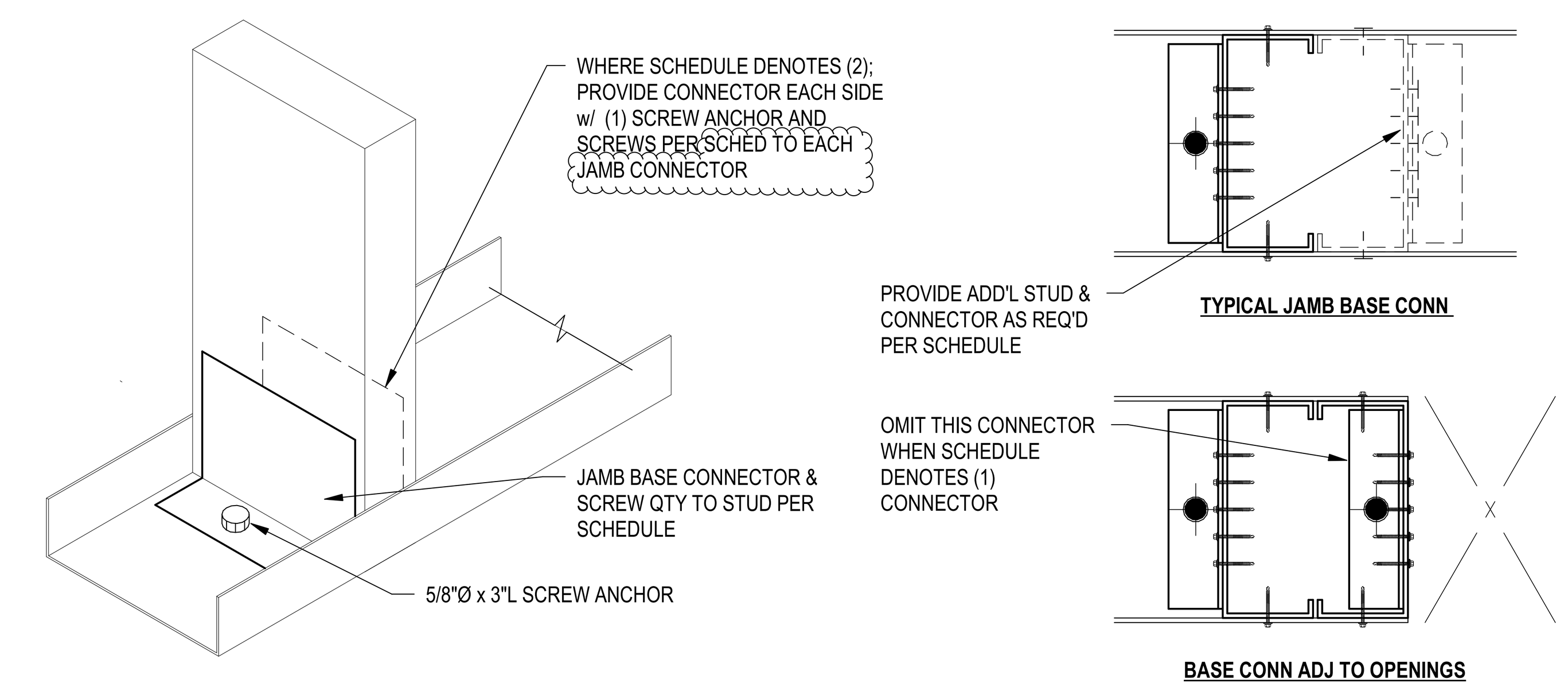
10 LATERAL BRACING AT UNPUNCHED STUDS
SCALE: 3" = 1'-0"



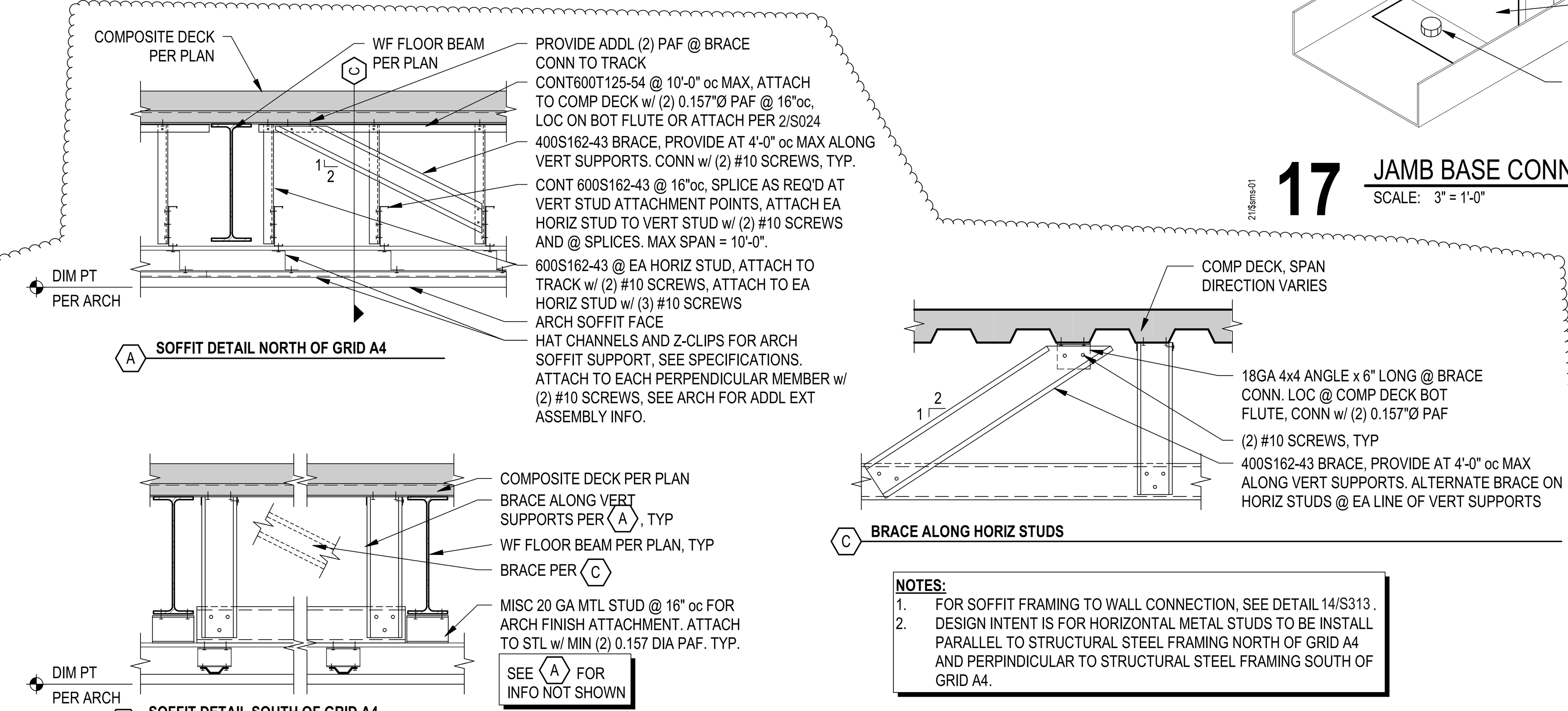
13 METAL STUD CONN DETAIL
SCALE: 3/4" = 1'-0"



14 STEEL/COLD-FORMED STEEL FRAMING CONN
SCALE: 3/4" = 1'-0"



17 JAMB BASE CONNECTION
SCALE: 3" = 1'-0"



21 SOFFIT FRAMING SECTION
SCALE: 3/4" = 1'-0"

	MARK	S8	S6	S4
FRAMING*	STUD TYPE	800S162-54	600S162-43	400S162-43
	SPACING	16"	16"	16"
	FINISH	GALV	GALV	GALV
	TRACK	800T125-54	600T125-43	400T125-43

*UNO ON ELEVATIONS

	TYPE	SIZE	HEAD
FASTENERS	DRY WALL	#6	BUGLE
	FRAMING	#10	PAN
	PLYMETAL	#10	BUGLE

NOTE: SCREWS IN EXT. STUDS SHALL BE ZINC COATED.

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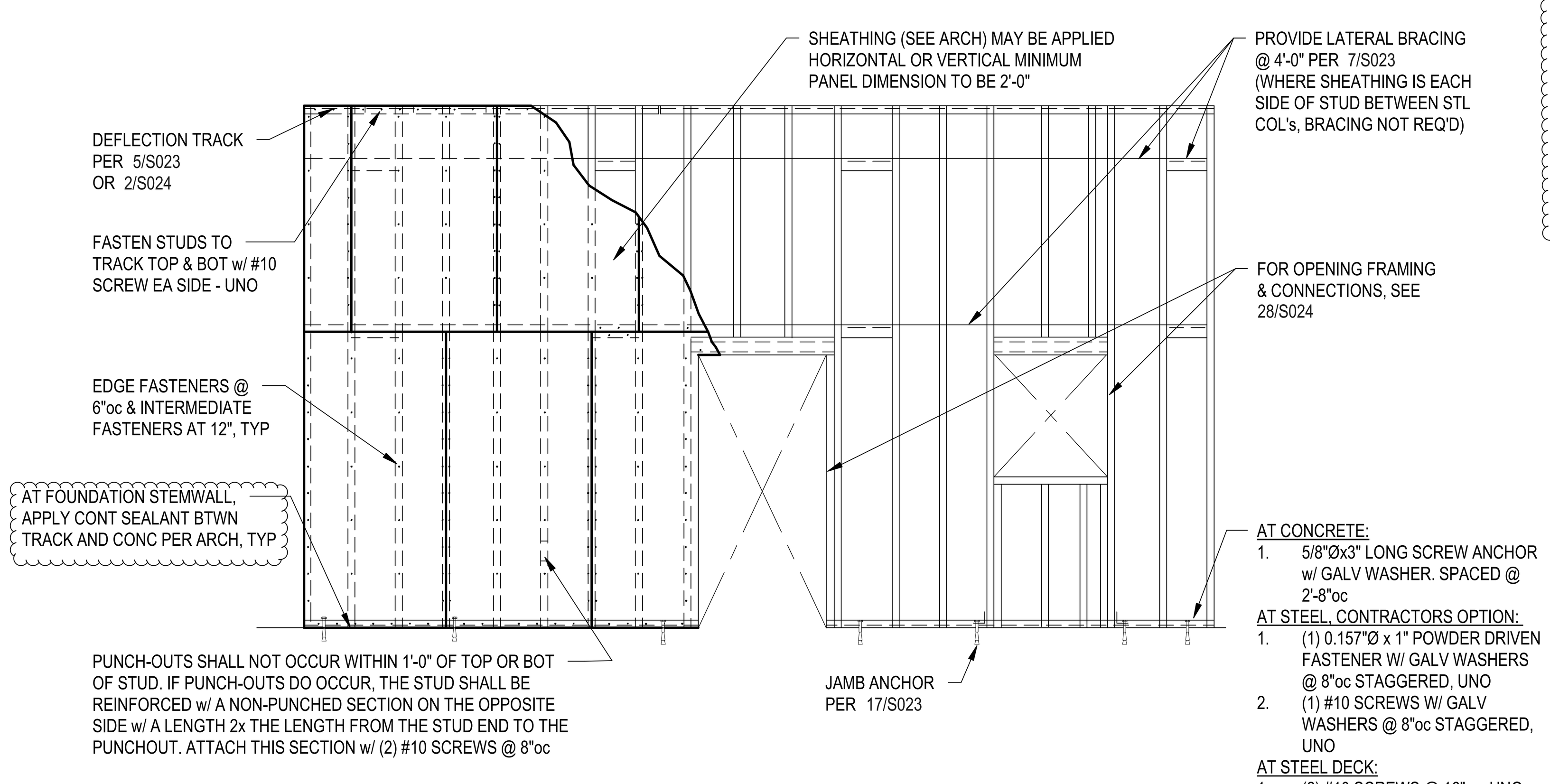

25 EXTERIOR STEEL STUD WALL
SCALE: 1/2" = 1'-0"

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City of Puyallup
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Building	Planning
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Fire	Traffic

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1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
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Revisions

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TYPICAL METAL STUD DETAILS

S023

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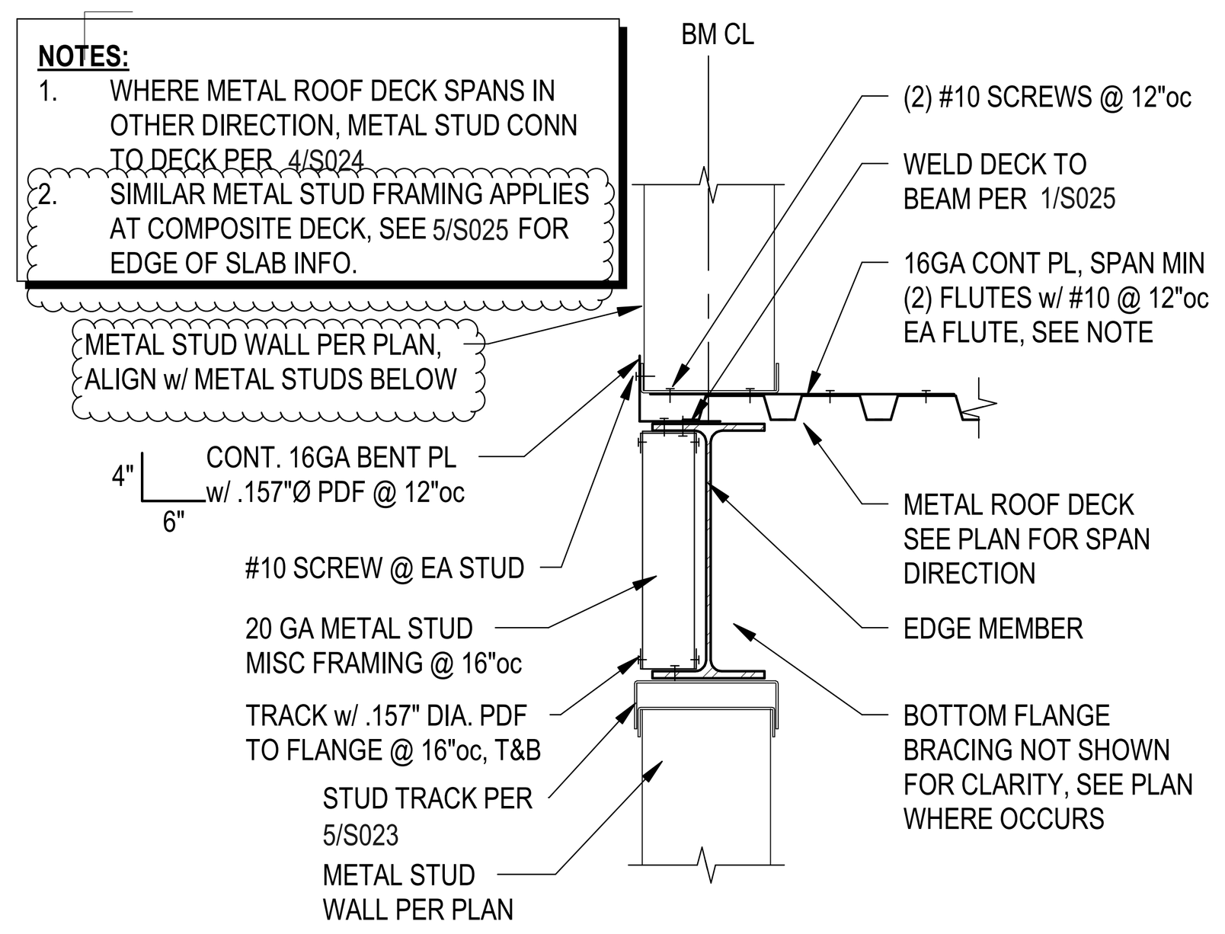
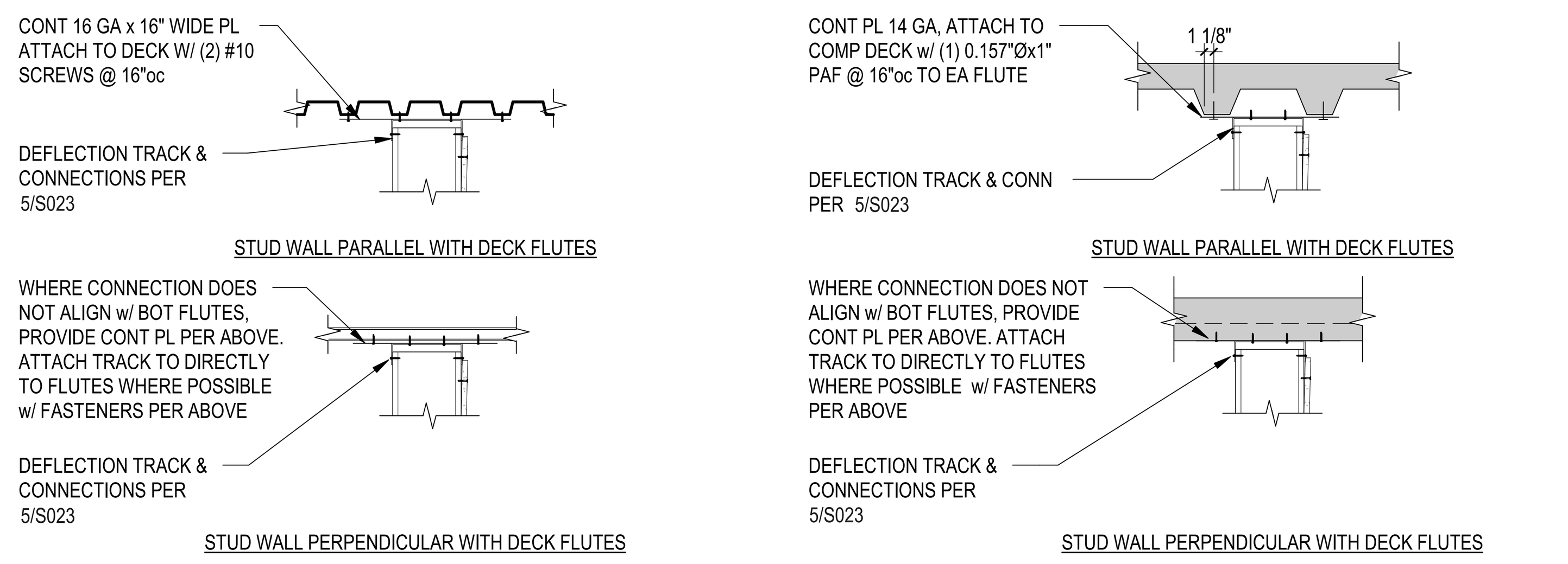
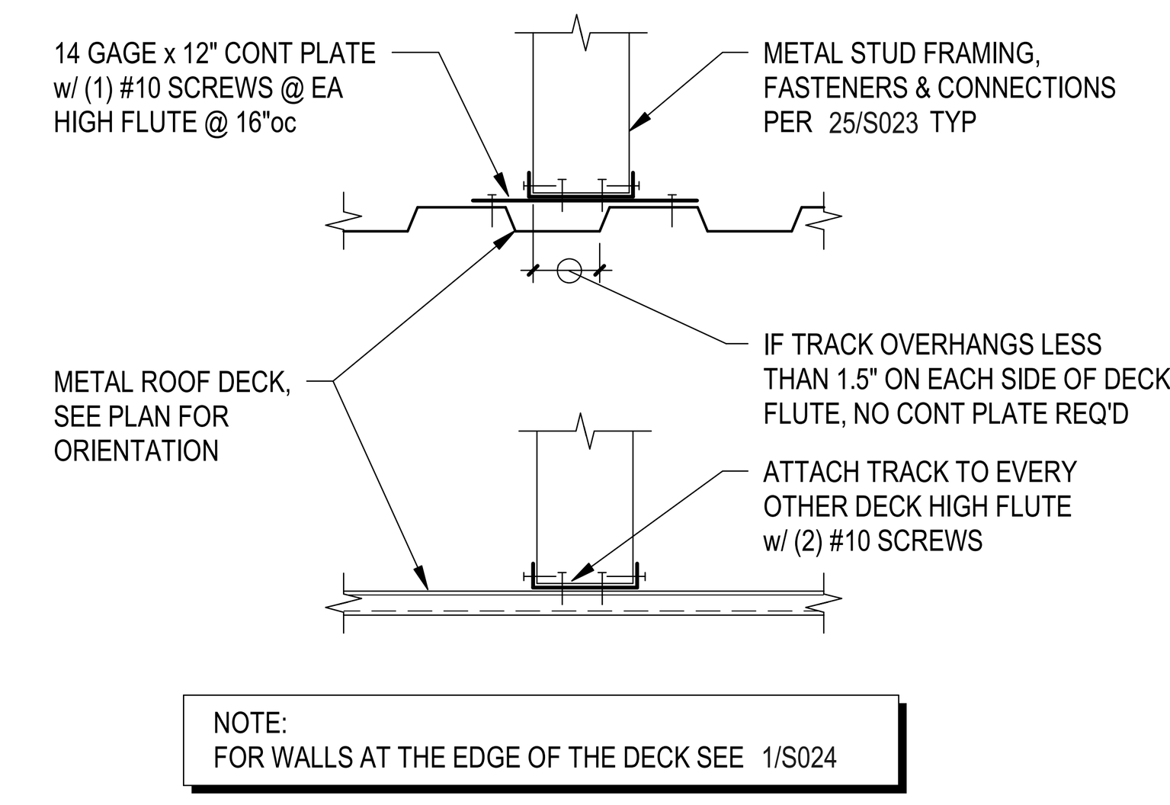


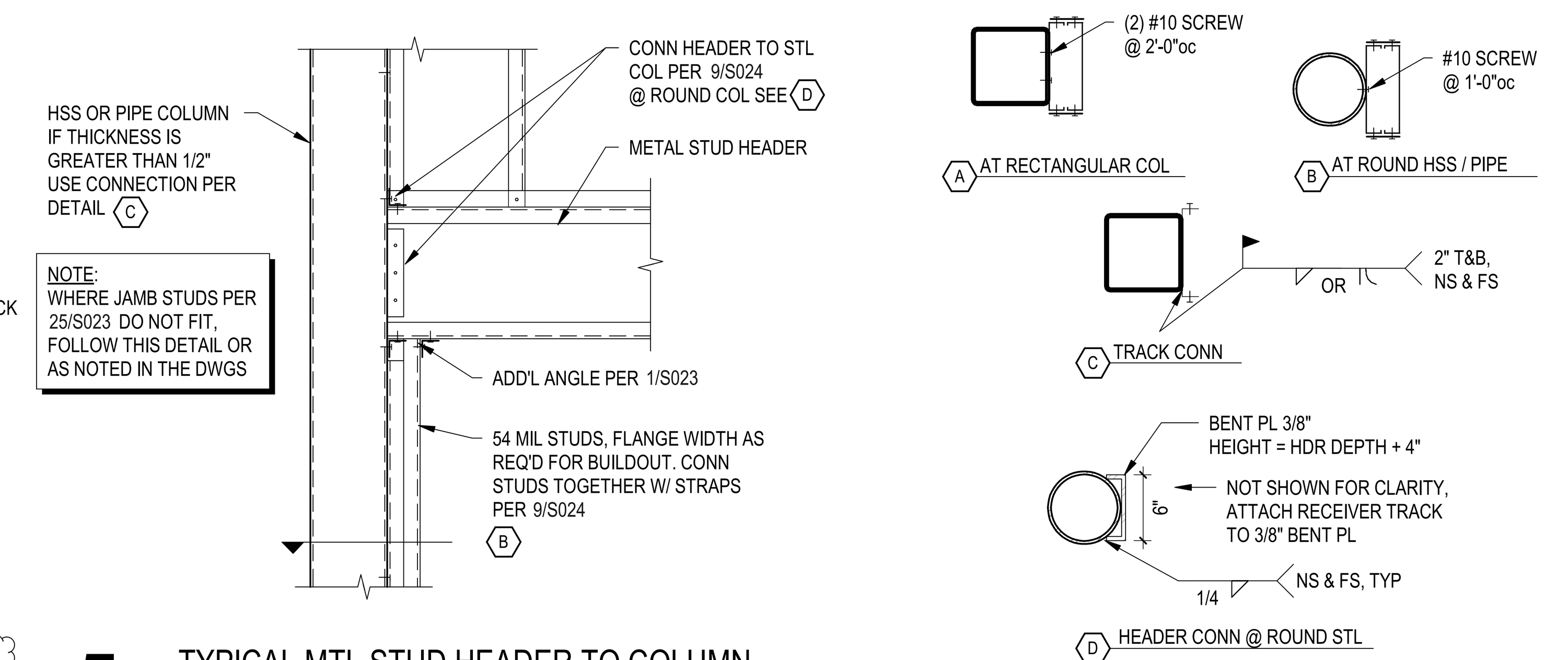
1 EXTERIOR STUD WALL AT ROOF
SCALE: 1" = 1'-0"



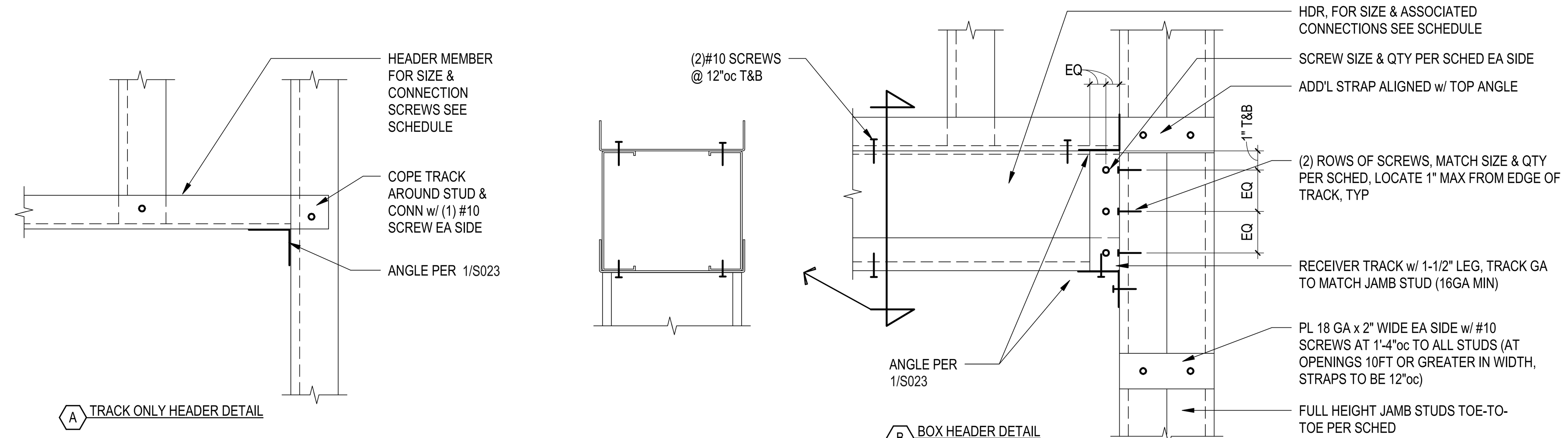
2 EXTERIOR DEFLECTION TRACK AT BOD
SCALE: 3/4" = 1'-0"



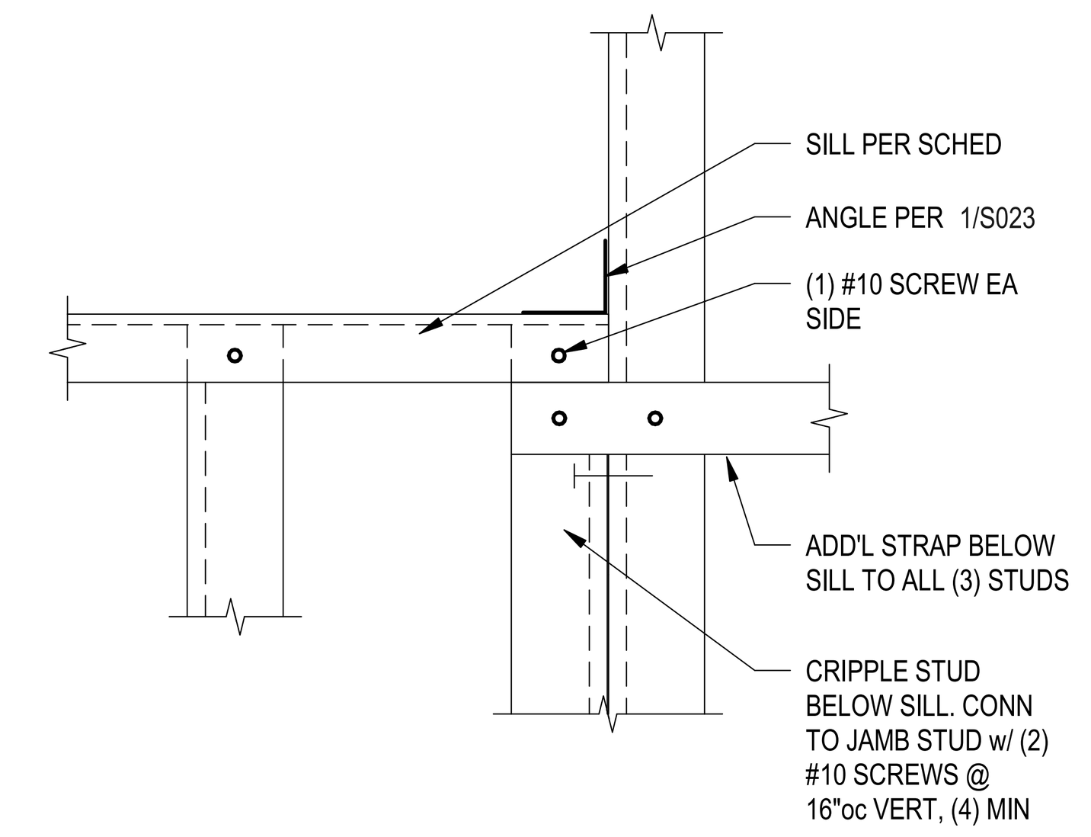
4 EXTERIOR METAL STUD BEARING AT DECK
SCALE: 1" = 1'-0"



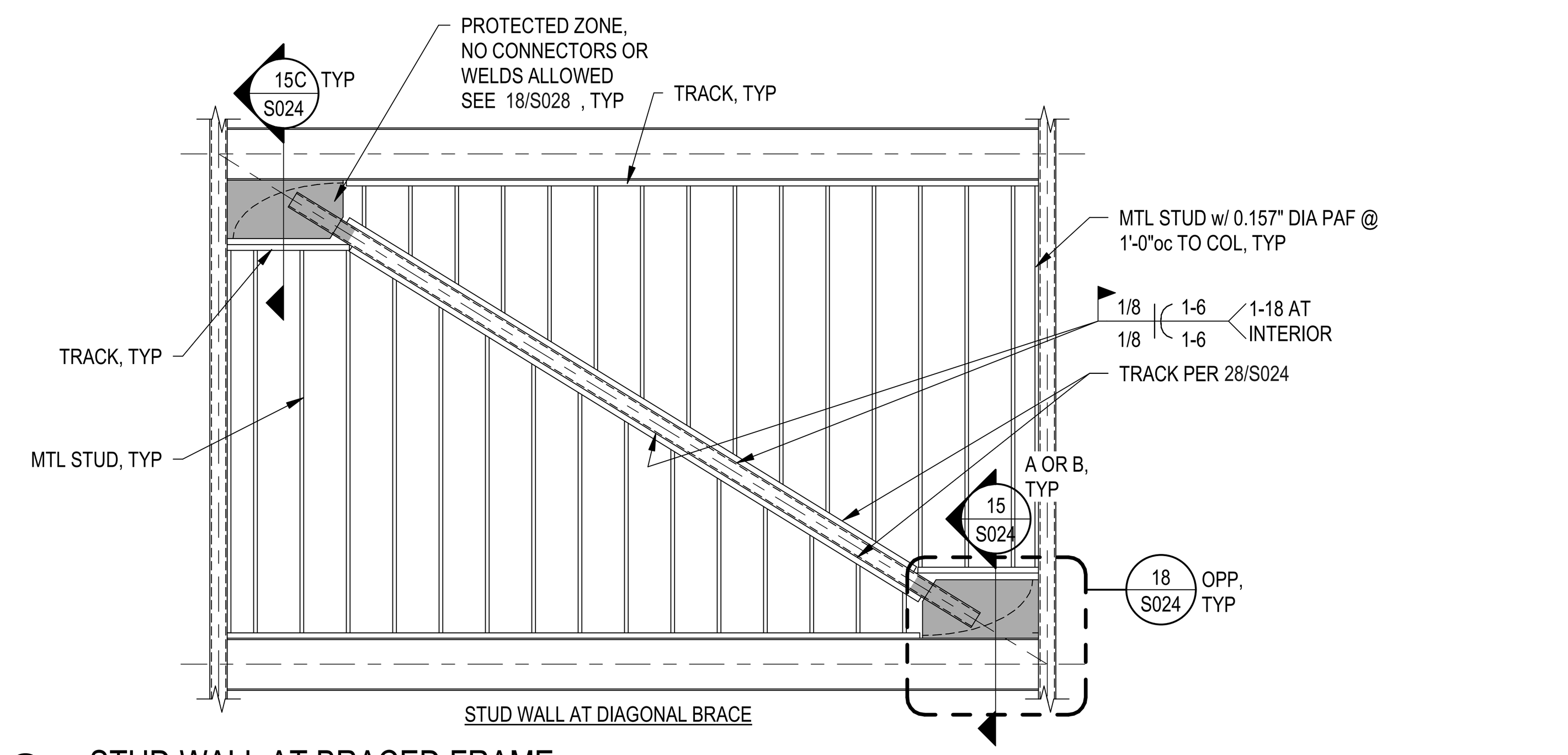
5 TYPICAL MTL STUD HEADER TO COLUMN
SCALE: 1" = 1'-0"



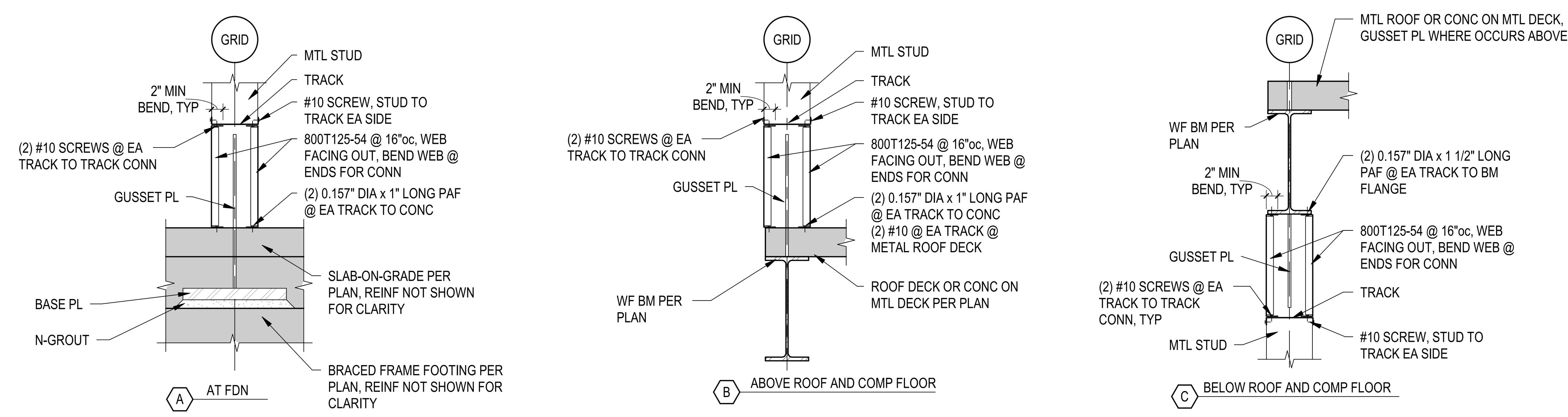
9 EXTERIOR WALL STEEL STUD HEADER DETAIL
SCALE: 3/4" = 1'-0"



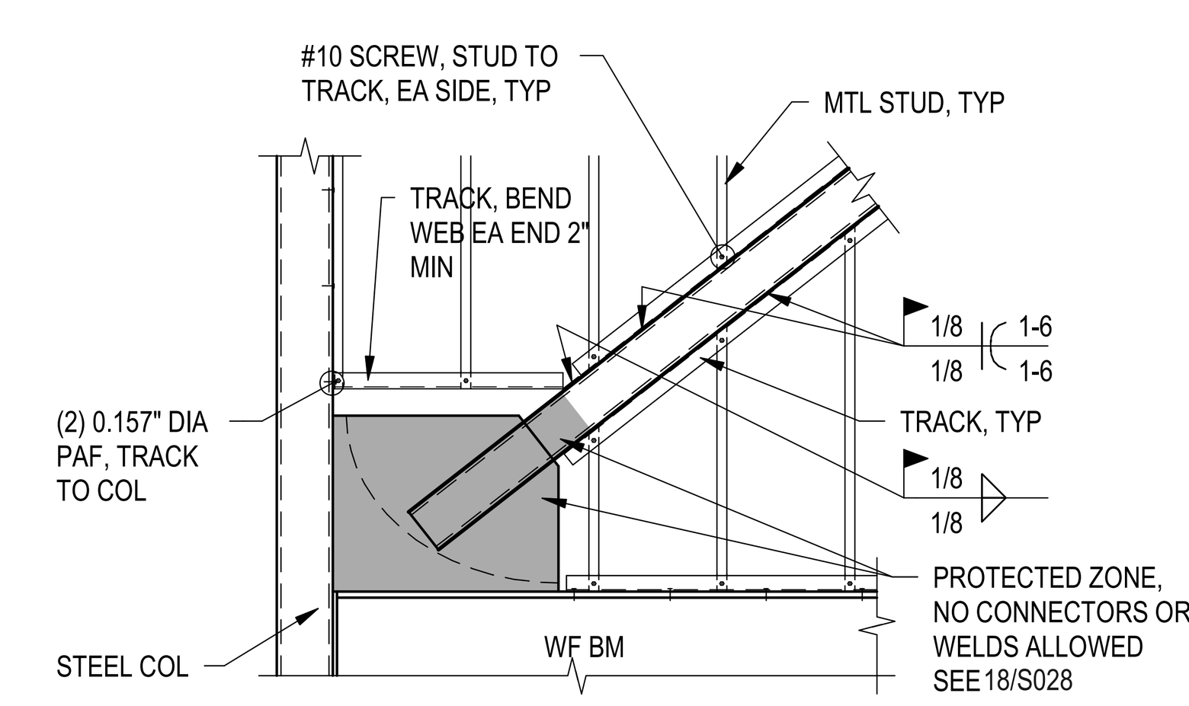
12 SILL CONN DETAIL
SCALE: 3/4" = 1'-0"



13 STUD WALL AT BRACED FRAME
SCALE: 1/4" = 1'-0"

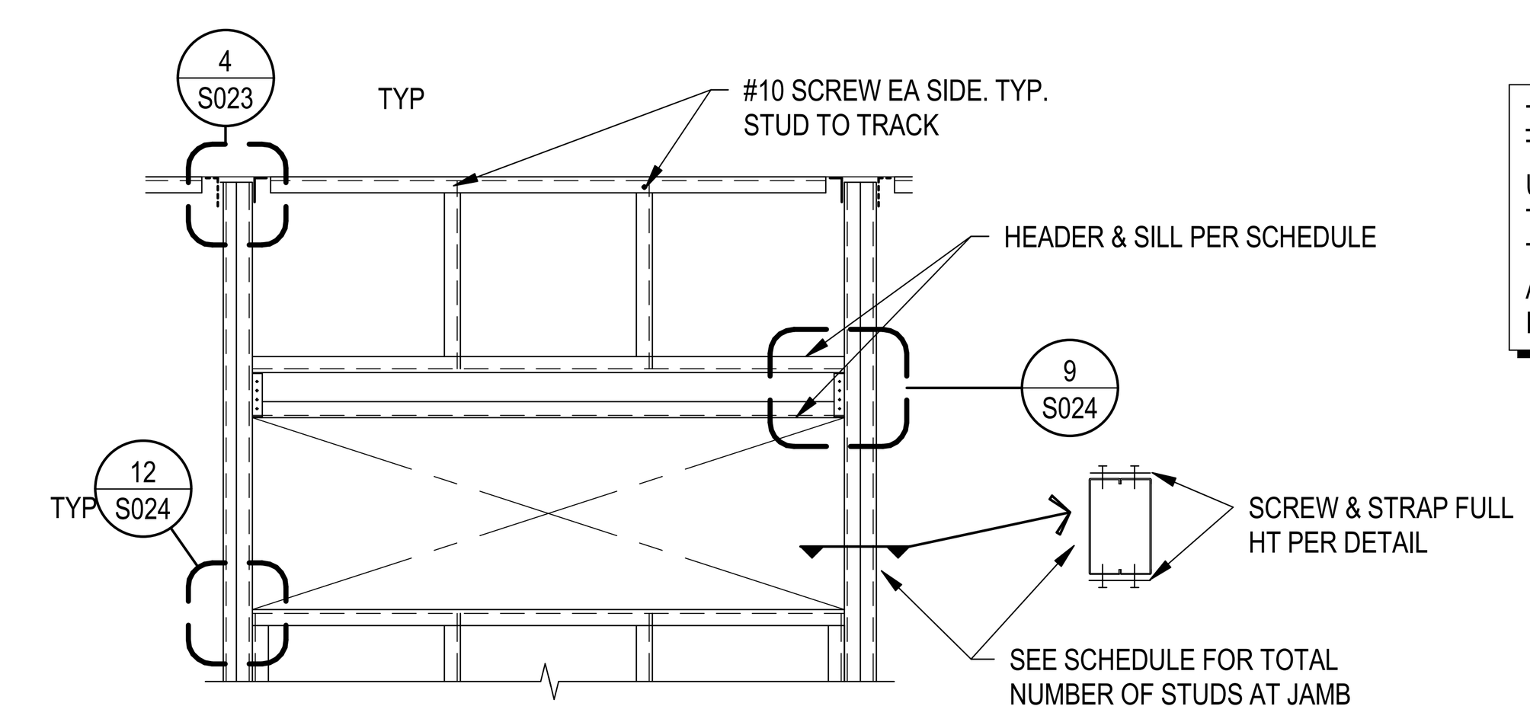


15 STUD WALL AT BRACE GUSSET
SCALE: 3/4" = 1'-0"



18 STUD WALL AT BRACE GUSSET
SCALE: 1/2" = 1'-0"

HDR SPAN	HDR PER	HDR VERTICAL MEMBERS	HDR HORIZ MEMBERS	SILL	JAMB STUDS	SCREWS FOR RECEIVER TRACK 9/S024	SCREWS PER ANGLE LEG 1/S023	JAMB CONN @ HEAD 4/S023	JAMB CONN @ BASE
4'-0" MAX	A	-	800T200-68	800T150-54	(1) 800S300-68	-	(2) #10 EA. LEG	SCAFCO: (1) ESC OR (1) PLS2	(1) SCAFCO FA750-68 w/ (3) #10 SCREWS
6'-0" MAX	B	(2) 600S162-54	(2) 800T150-54	800T150-54	(2) 800S200-54	(3) #10 EA. SIDE	(2) #10 EA. LEG	SCAFCO: (1) ESC OR (1) PLS2	(1) SCAFCO FA750-68 w/ (3) #10 SCREWS
8'-0" MAX	B	(2) 600S162-54	(2) 800T150-54	800T150-54	(2) 800S300-54	(3) #10 EA. SIDE	(2) #10 EA. LEG	SCAFCO: (1) ESC OR (1) PLS2	(2) SCAFCO FA750-68 w/ (4) #10 SCREWS
10'-0" MAX	B	(2) 800S200-54	(2) 800T200-68	800T150-97	(2) 800S350-68	(4) #10 EA. SIDE	(3) #10 EA. LEG	SCAFCO: (1) ESC OR (1) DPLS2	(2) SCAFCO FA750-68 w/ (4) #10 SCREWS
12'-0 MAX	B	(2) 800S200-54	(2) 800T200-68	6/S023	(2) 800S350-68	(4) #10 EA. SIDE	(3) #10 EA. LEG	SCAFCO: (2) ESC OR (1) PLS2	(2) SCAFCO FA750-68 w/ (4) #10 SCREWS
14'-0" MAX	B	(2) 1000S250-54	(2) 800T200-97	6/S023	(2) 800S250-97	(4) #10 EA. SIDE	(3) #10 EA. LEG	SCAFCO: (2) ESC OR (2) PLS2	(2) SCAFCO FA750-68 w/ (4) #10 SCREWS



TRACK SPLICE NOTE:
TRACK SHALL NOT BE SPLICED WHEN USED AT BOX HEADERS OR SILLS. IF TRACK IS REQUIRED TO BE SPLICED, THE TRACK SHALL HAVE A FULL BUTT-WELD AND BE TESTED BY THE SPECIAL INSPECTOR FOR ACCEPTANCE

PRCNC20221036
STRUCTURAL REVISION

28 EXTERIOR STEEL STUD WALL OPENING DETAIL
SCALE: 1/2" = 1'-0"

integrus ARCHITECTURE

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM/BO
Checked by:	TD

Revisions

Date	Description
1.30.23	IFC Revisions

TYPICAL METAL STUD DETAILS

S024

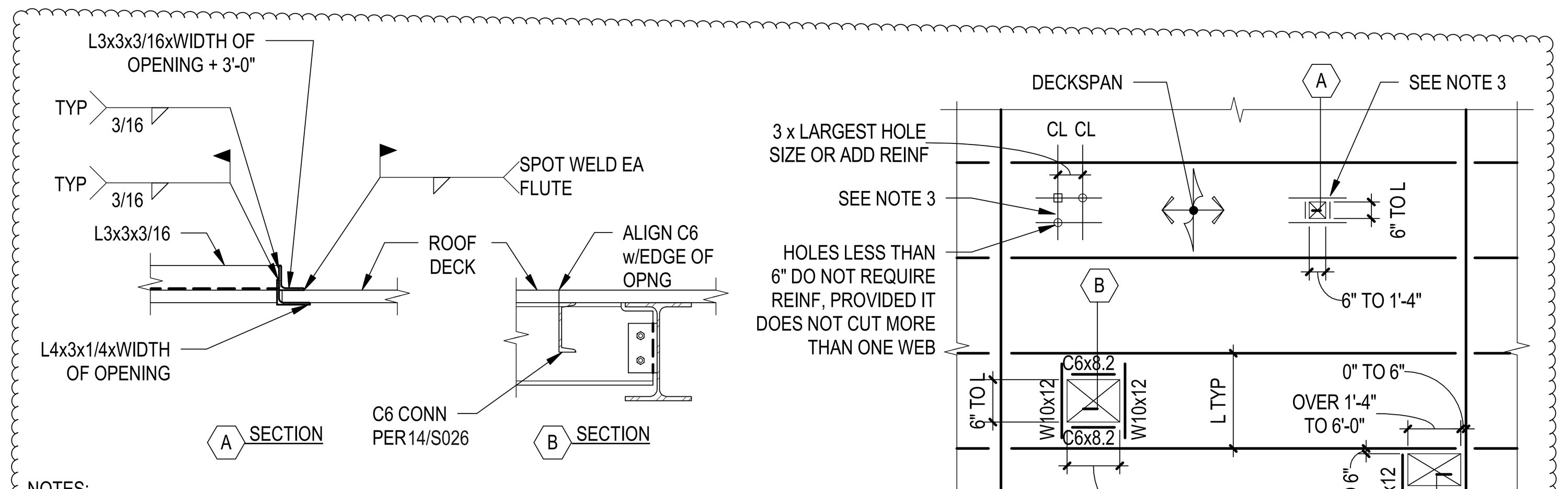
IFC SET

DECK PROPERTIES						FASTENING			DECK ELEVATION
DECK TYPE	PAINT OR GALV	GAGE	WIDTH	I (IN⁴)	ST/SB (IN³)	ATTACHMENT PATTERN	@ SIDE LAP	@ EDGE	(SEE PLANS FOR TOPPING THICKNESS AND REINFORCING)
1 1/2" ROOF, PLB	G[1]	18	36"	.306	.314 / .331	36/7/4 PW	IP @ 8"	PW @ 6"	
2" COMP UN-TOPPED, PLW2	G[1,2]	18	36"	.565	.471 / .481	36/4 PW	IP @ 24"	PW @ 9"	
2" COMP, PLW2	G[1]	18	36"	.565	.471 / .481	36/4 PW	IP @ 24"	PW @ 9"	

KEY: PW= TYPICAL WELDS 5/8"Ø PUDDLE WELDS
IP = INTERLOCK PUNCH (BUTTON PUNCH NOT ALLOWED)
G = GALVANIZING (SEE STRUCTURAL NOTES FOR EXT & INT APPLICATIONS)

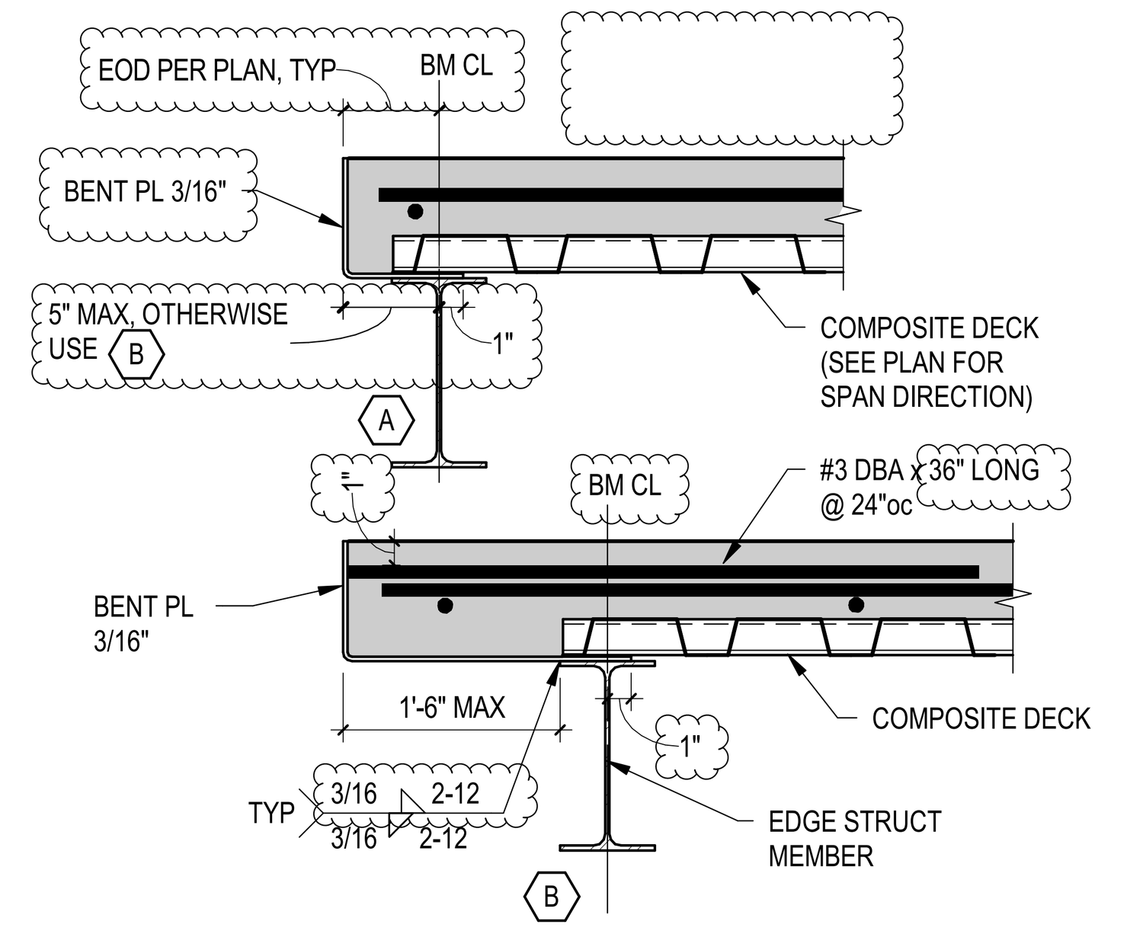
KEY: 1 - SEE ARCHITECTURAL PLANS WHERE DECK IS EXPOSED TO VIEW AND IT IS TO BE PAINTED IN THE FIELD, SHOP PRIME IS REQUIRED
2 - AT EXTERIOR PERMANENTLY EXPOSED LOCATIONS, PROVIDE G90

1 STEEL ROOF AND COMPOSITE DECK WELDING
SCALE: 3/4" = 1'-0"

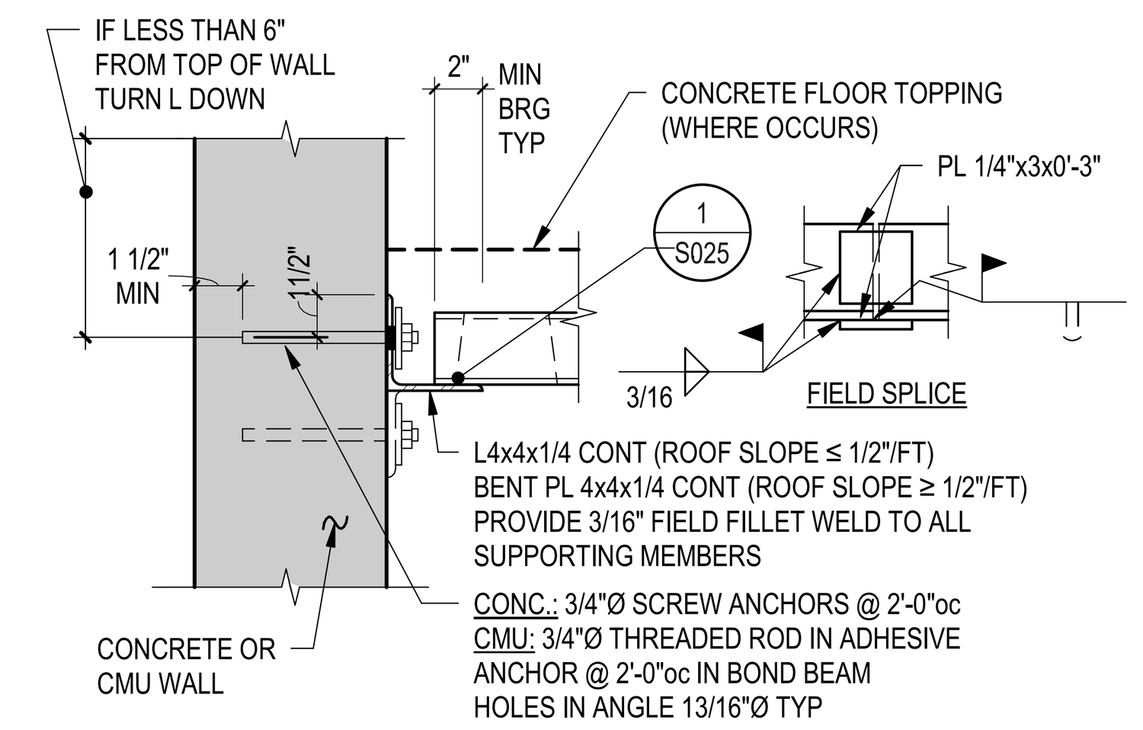


NOTES:
1. INSTALL REINF BEFORE CUTTING HOLES.
2. CONTRACTOR SHALL COORDINATE OPENING SIZE AND LOCATION WITH MECHANICAL AND ELECTRICAL CONTRACTORS AND ARCHITECTRURAL DRAWINGS.
3. THE OPENING NOTED REQUIRES A CLEAR SPACING FROM ADJACENT OPENING OF THREE TIMES MAX OPENING DIMENSION. IF REQ'D LAYOUT CANNOT CONFORM TO THESE REQUIREMENTS, REINF GROUP AS IF ONE COMBINED PENETRATION.

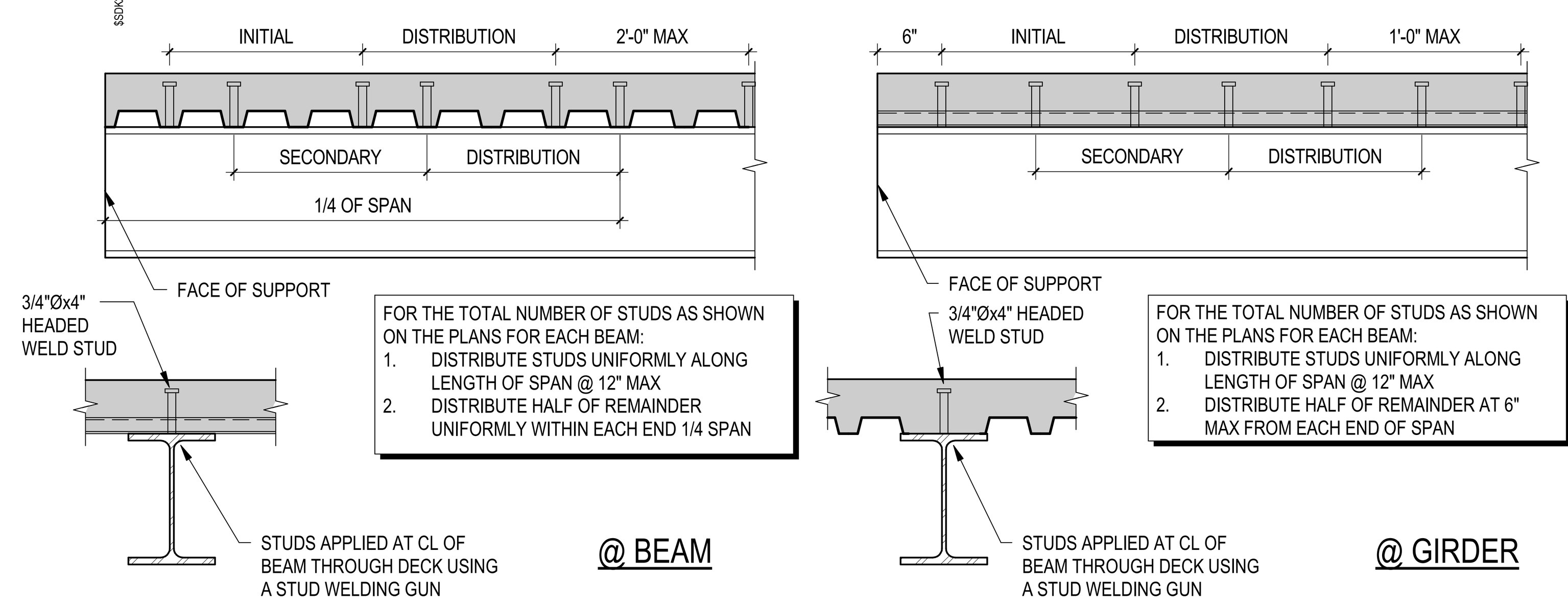
3 ROOF DECK OPENING (6'-0" AND LESS) @ WF BEAMS
SCALE: 3/4" = 1'-0"



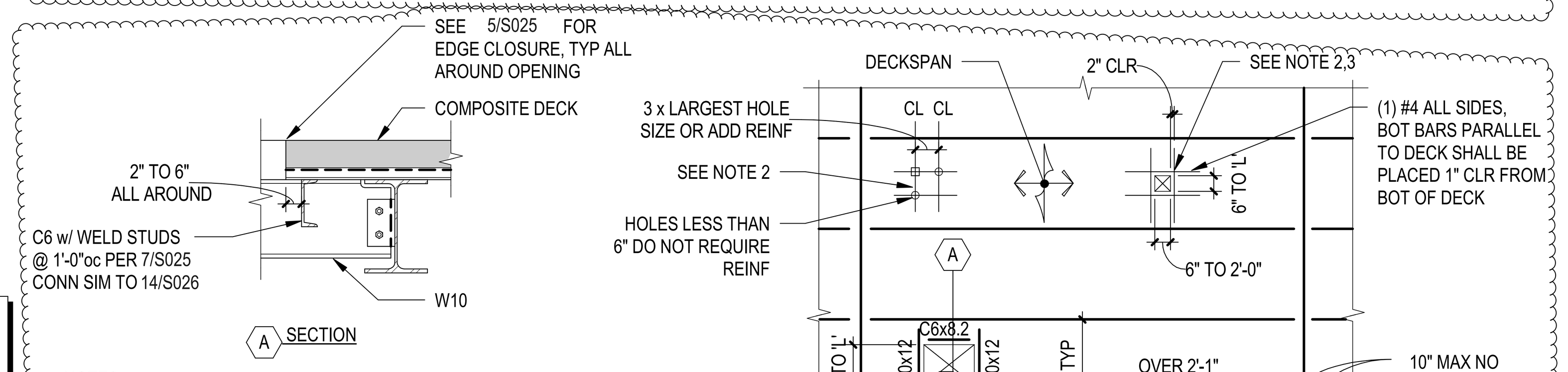
5 COMPOSITE DECK EDGE CLOSURE
SCALE: 1 1/2" = 1'-0"



6 PERIMETER DECK ANGLE AT CONCRETE OR CMU WALL
SCALE: 1 1/2" = 1'-0"

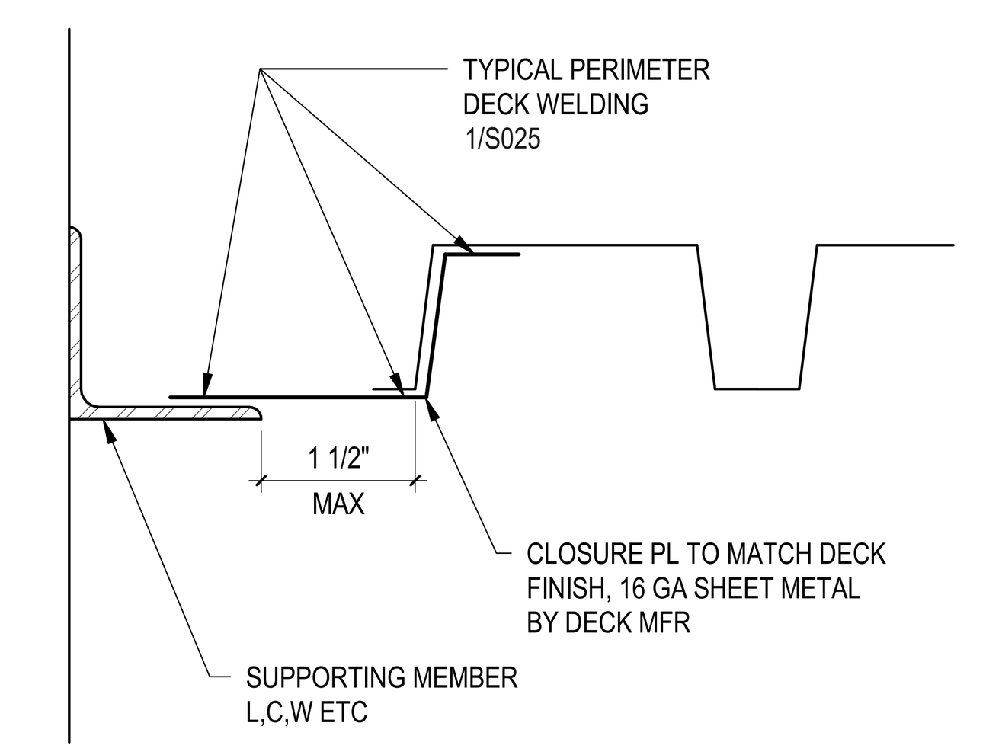


7 COMPOSITE BEAM AND GIRDER WELD STUDS
SCALE: 1" = 1'-0"

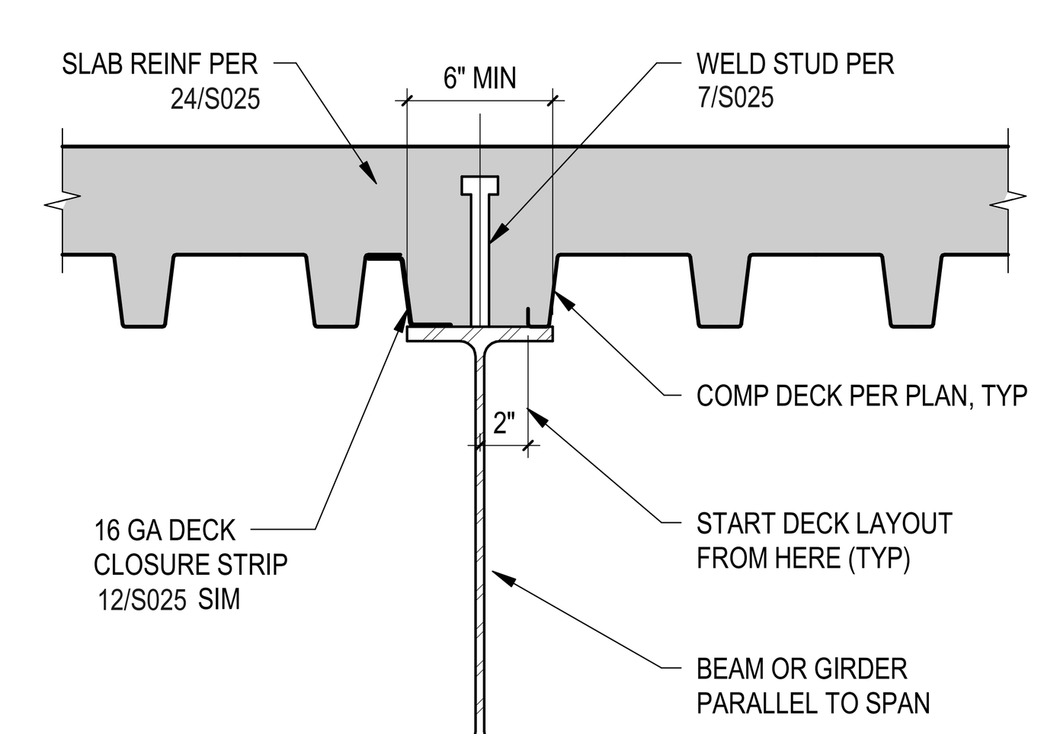


NOTES:
1. CONTRACTOR SHALL COORDINATE OPENING SIZE AND LOCATION WITH MECHANICAL AND ELECTRICAL CONTRACTORS AND ARCHITECTURAL DRAWINGS.
2. THE OPENINGS NOTED REQUIRES A CLEAR SPACING FROM ADJACENT OPENINGS OF THREE TIMES MAXIMUM OPENING DIMENSION. IF REQUIRED LAYOUT CANNOT CONFORM TO THESE REQUIREMENTS, REINFORCE GROUP AS IF ONE COMBINED PENETRATION
3. METAL DECK TO BE CUT AFTER CONCRETE CURES. PROVIDE TEMPORARY CONCRETE FORMWORK AT FUTURE OPENING WHILE POURING CONCRETE. IF DECK IS CUT PRIOR TO CONCRETE POURING, PROVIDE STRUCTURAL STEEL AROUND OPENING.

9 COMPOSITE DECK OPENINGS (6'-0" AND LESS)
SCALE: 3/4" = 1'-0"

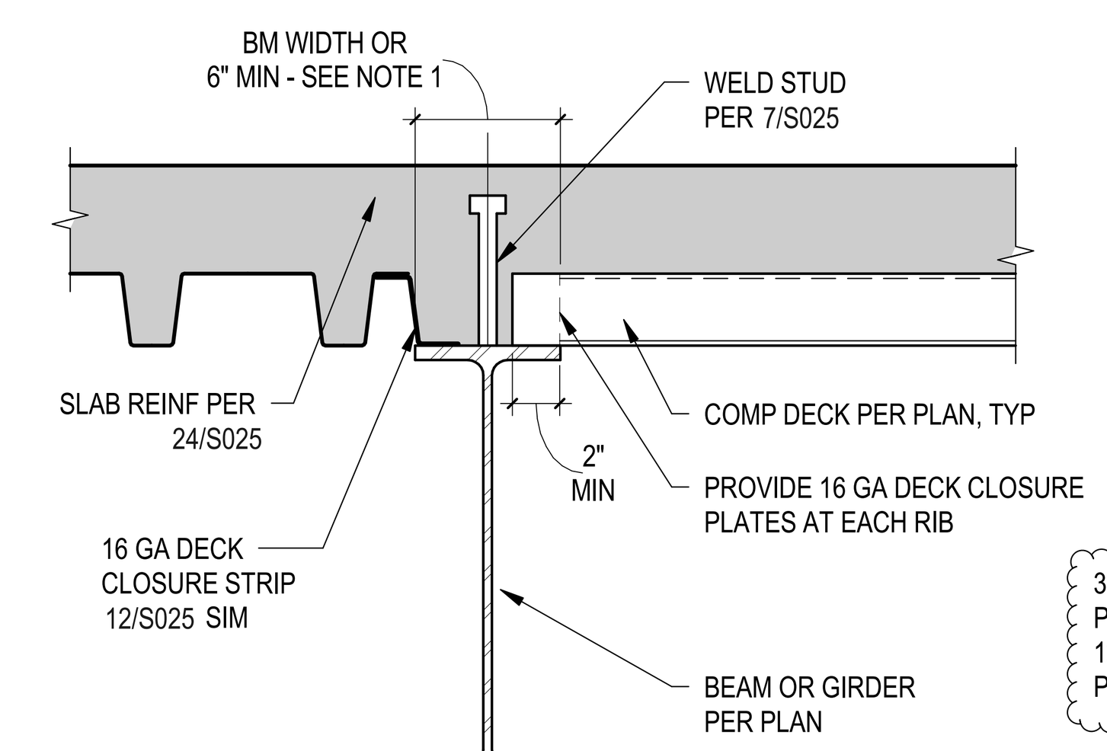


12 DECK CLOSURE STRIP
SCALE: 3" = 1'-0"



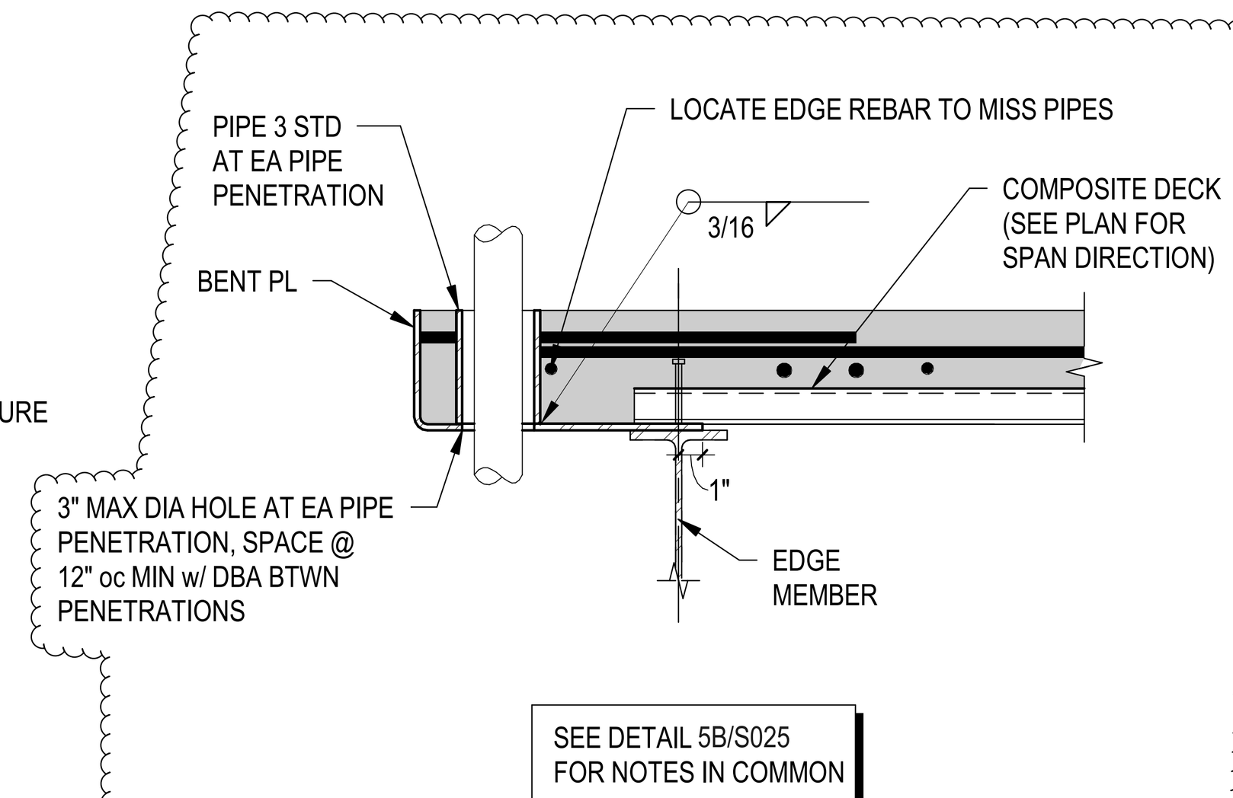
NOTE: DECK RIB SHALL BE CENTERED OVER BEAM WHERE POSSIBLE OR SPLIT DECK FOR FULL LENGTH OF BEAM AND PROVIDE CONCRETE WIDTH AS SHOWN ABOVE.

13 DECK AT COMPOSITE GIRDER
SCALE: 1 1/2" = 1'-0"

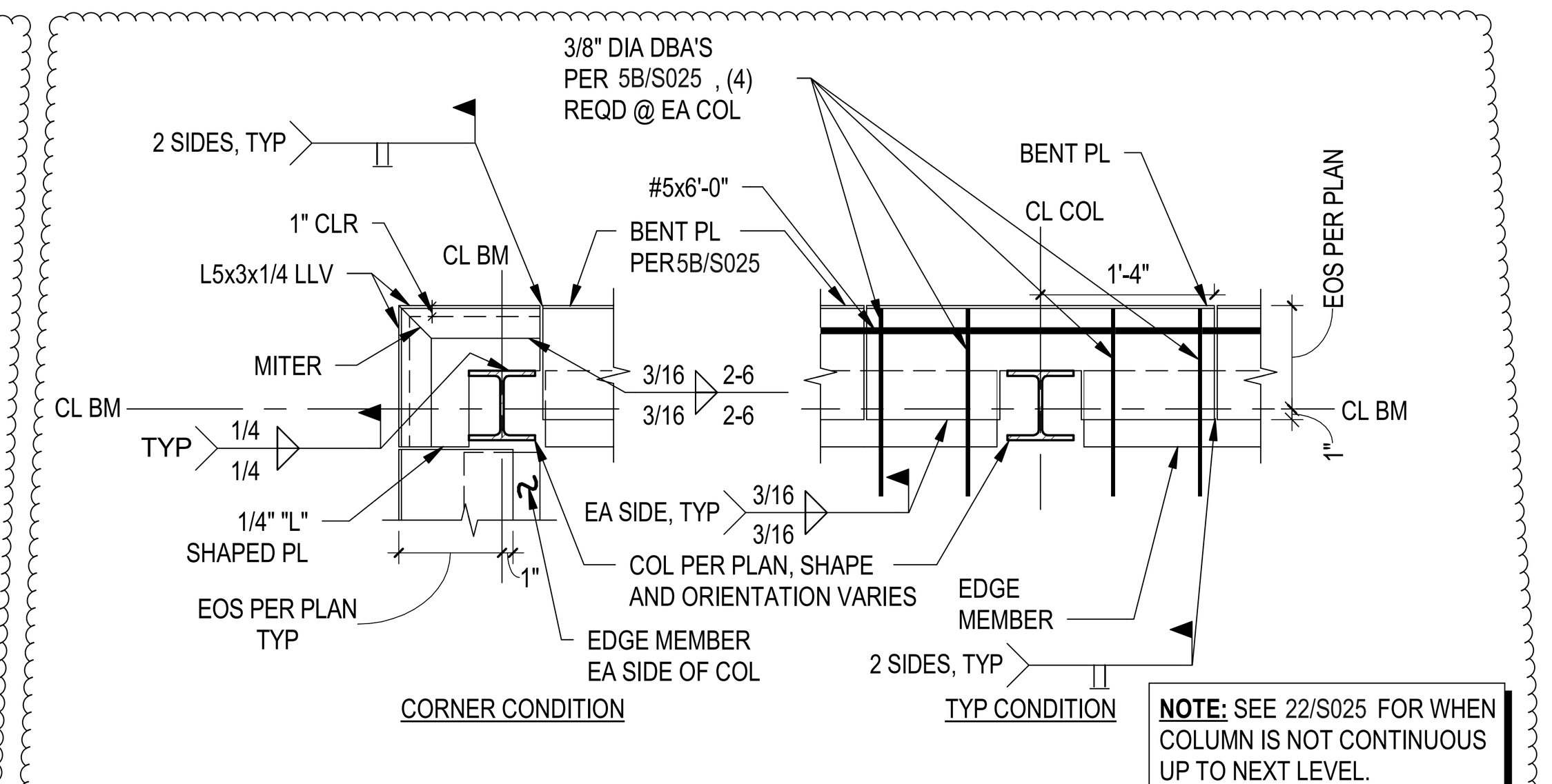


NOTES:
1. DECK RIB SHALL BE CENTERED OVER BEAM WHERE POSSIBLE OR SPLIT DECK FOR FULL LENGTH OF BEAM AND PROVIDE CONCRETE WIDTH AS SHOWN ABOVE.

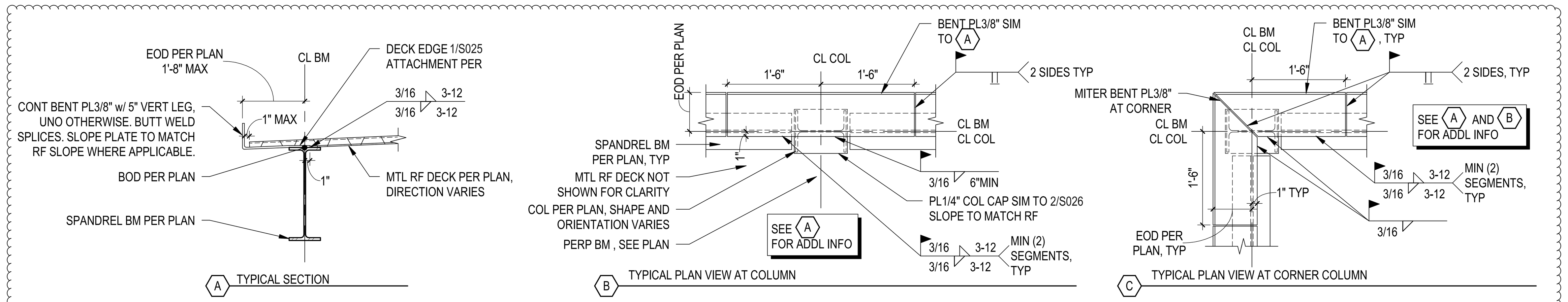
14 COMPOSITE DECK DIRECTION CHANGE
SCALE: 1 1/2" = 1'-0"



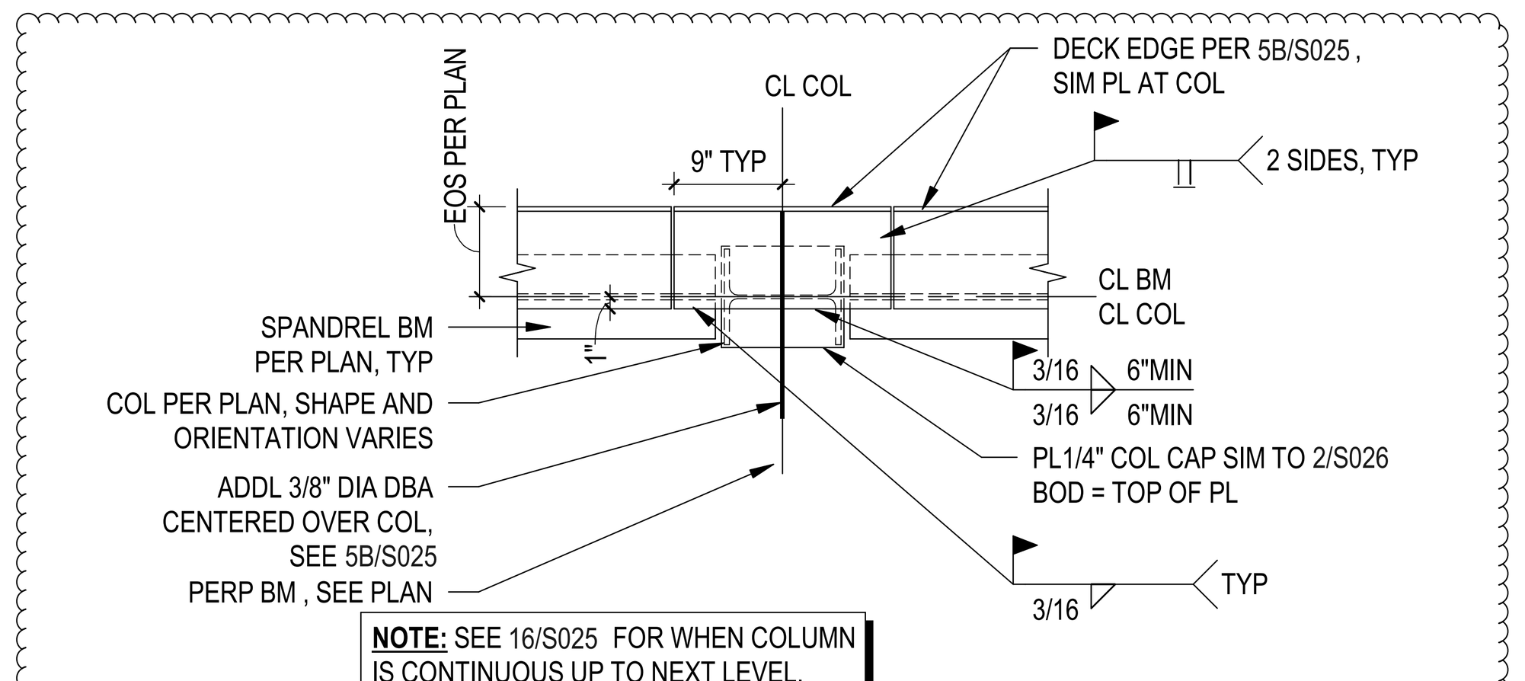
15 PIPE PENETRATION AT DECK EDGE
SCALE: 1 1/2" = 1'-0"



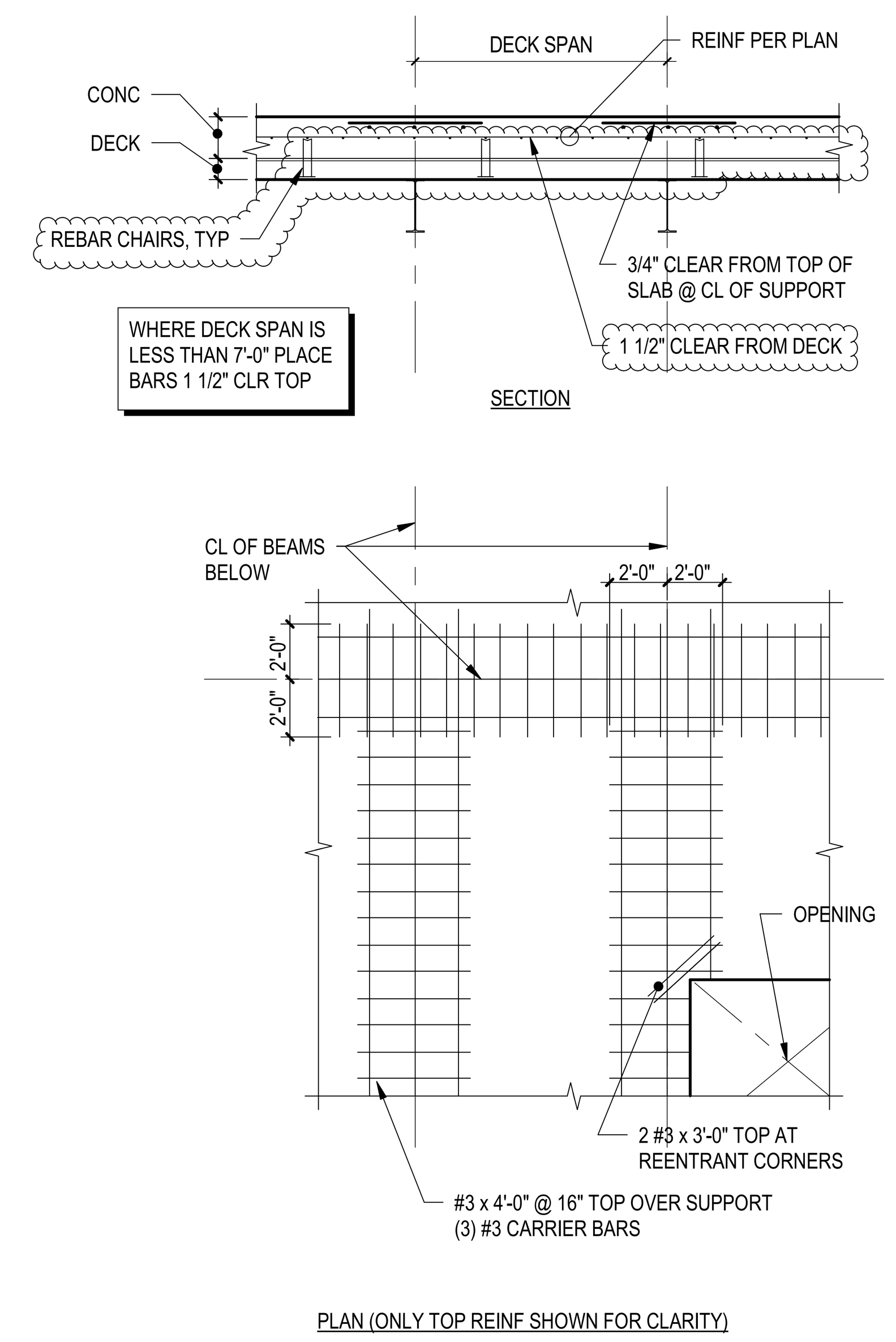
16 COMPOSITE DECK EDGE CLOSURE AT COLUMNS
SCALE: 3/4" = 1'-0"



19 METAL ROOF DECK EDGE CLOSURE
SCALE: 3/4" = 1'-0"

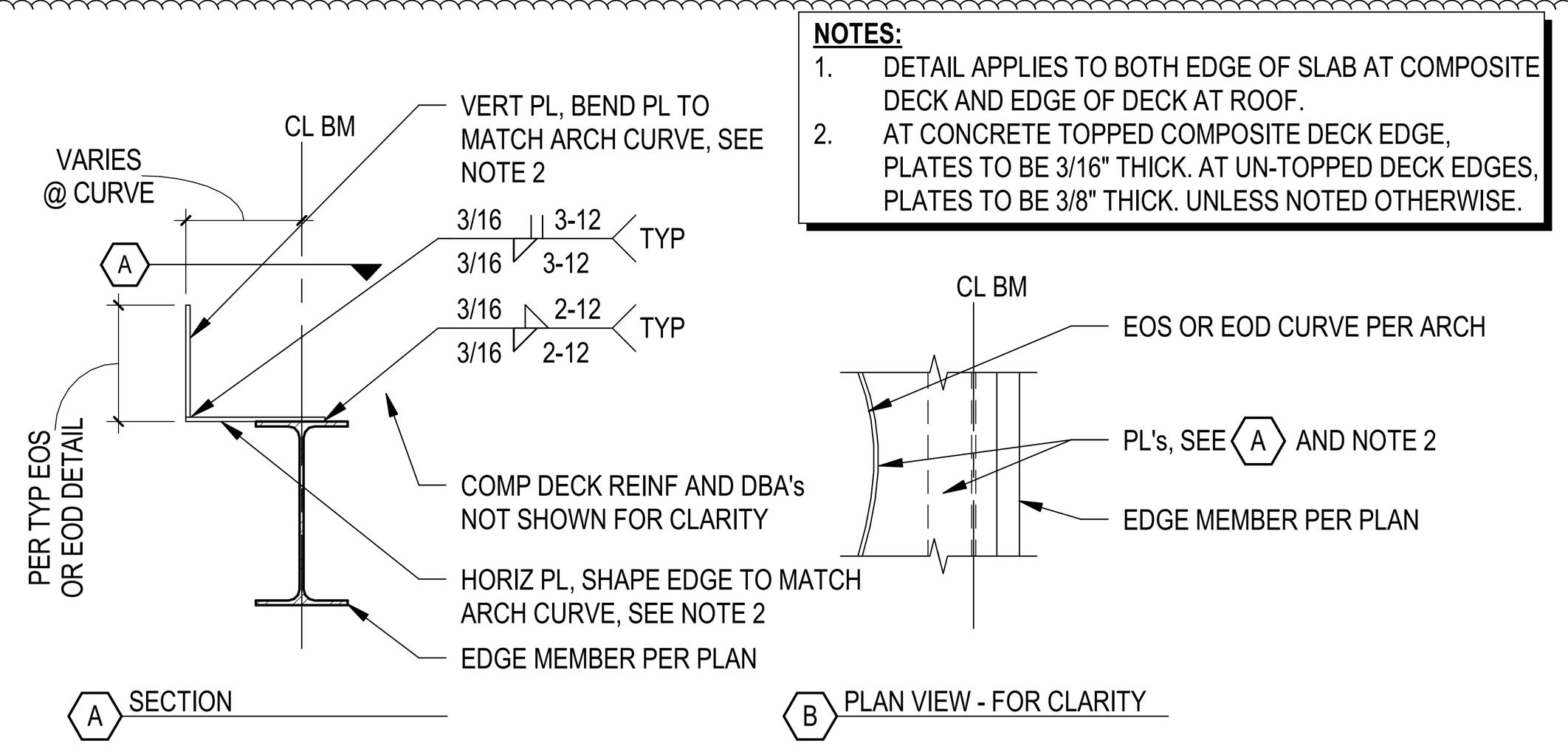


22 COMPOSITE DECK EDGE CLOSURE AT COLUMN TOP
SCALE: 3/4" = 1'-0"



24 COMPOSITE DECK REINFORCING
SCALE: 1/4" = 1'-0"

NOTES:
1. DETAIL APPLIES TO BOTH EDGE OF SLAB AT COMPOSITE DECK AND EDGE OF DECK AT ROOF.
2. AT CONCRETE TOPPED COMPOSITE DECK EDGE, PLATES TO BE 3/16" THICK. AT UN-TOPPED DECK EDGES, PLATES TO BE 3/8" THICK. UNLESS NOTED OTHERWISE.



25 DECK EDGE CLOSURE AT CURVES
SCALE: 1 1/2" = 1'-0"

integrus ARCHITECTURE
117 SOUTH MAIN STREET, SUITE 100, SEATTLE, WA 98104

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

PRCNC20221036
STRUCTURAL REVISION

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM/RB
Checked by:	TD

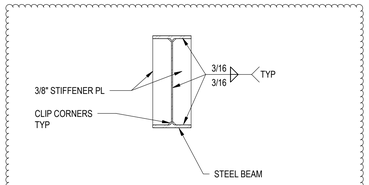
Revisions

Date	Description
1.30.23	IFC Revisions

TYPICAL STEEL AND METAL DECK DETAILS

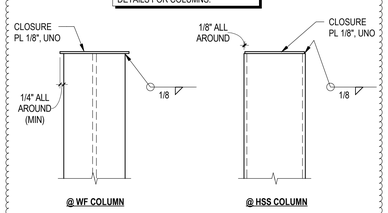
S025

IFC SET

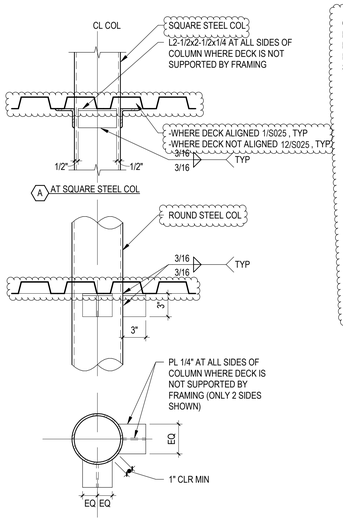


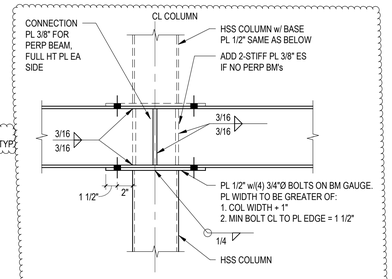
1 STIFFENER PLATE AT STEEL BEAM
SCALE: 1" = 1'-0"

NOTE:
USE THIS DETAIL WHEN NO OTHER CAP IS SHOWN ON THE PLANS OR IN DETAILS FOR COLUMNS.

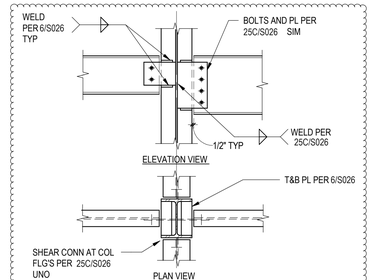


2 COLUMN CLOSURE PLATE
SCALE: 1 1/2" = 1'-0"

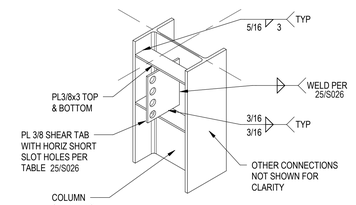




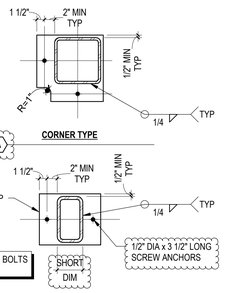
4 HSS COL BEARING PLATE @ STEEL BEAM
SCALE: 1 1/2" = 1'-0"

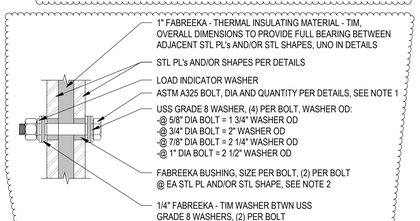


5 TYP BM TO WIDE FLANGE COLUMN CONNECTION
SCALE: 3/4" = 1'-0"



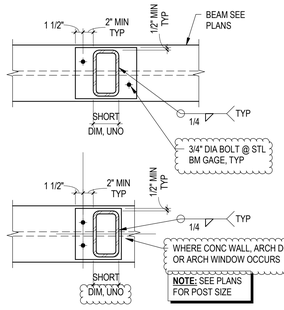
6 EXTENDED SHEAR TAB
SCALE: 3/4" = 1'-0"



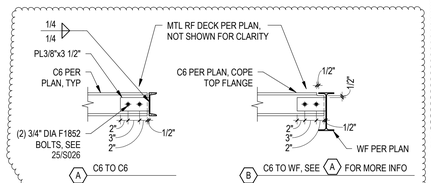


NOTES:
1. PROVIDE ASTM A325 BOLTS WITH LOAD INDICATOR WASHERS. LOAD INDICATOR WASHER LOCATION MAY VARY BASED ON CONNECTION CLEARANCES.
2. GC COORDINATE BUSHING DIAMETER WITH FABREEKA AND STEEL SHOP.

8 THERMAL BREAK BOLT ASSEMBLY
SCALE: 3" = 1'-0"

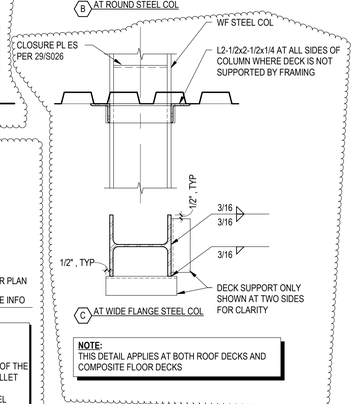


13 POST BASE PLATES
SCALE: 1" = 1'-0"



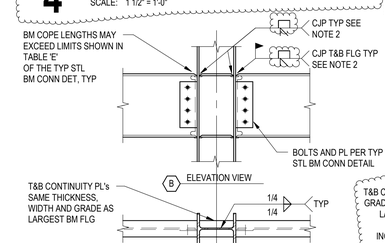
NOTES:
1. GC TO COORDINATE EXACT LOCATION OF MEP OPENING.
2. CHANNEL FLANGES TO BE ORIENTED TO FACE AWAY FROM MEP OPENING.
3. THE NUT SIDE OF THE OF THE BOLTS SHALL BE INSTALLED ON THE OPPOSITE SIDE OF THE MEP OPENING. ACCEPTABLE TO FIELD WELD PLATE TO CHANNEL WEB WITH 3/16" FILLET T&B IF BOLT INSTALLATION CLEARANCES ARE NOT POSSIBLE.
4. METAL ROOF DECK ALONG EDGE OF MEP OPENING SHALL NOT OVERHANG CHANNEL MORE THAN 4".

14 CHANNELS AT ROOF MEP OPENINGS
SCALE: 3/4" = 1'-0"



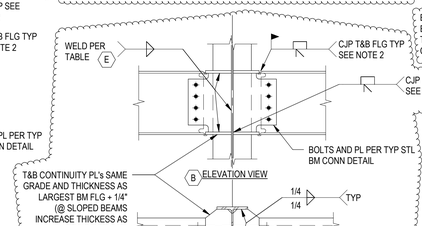
NOTE:
THIS DETAIL APPLIES AT BOTH ROOF DECKS AND COMPOSITE FLOOR DECKS

15 DECK SUPPORT AT COLUMN
SCALE: 1 1/2" = 1'-0"



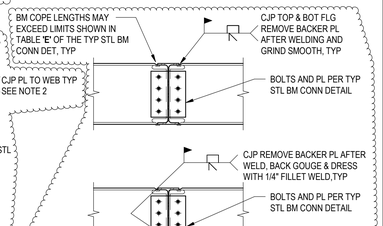
NOTES:
1. MOMENT CONNECTIONS SHOWN WITH THIS SYMBOL ON PLAN:
2. REMOVE BACKER PL AFTER WELDING AND GRIND SMOOTH

10 TYP BM TO COL FLG MOMENT CONN
SCALE: 3/4" = 1'-0"



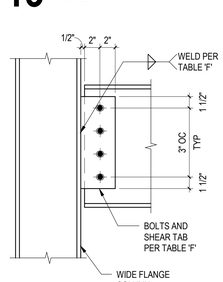
NOTES:
1. MOMENT CONNECTIONS SHOWN WITH THIS SYMBOL ON PLAN:
2. REMOVE BACKER PL AFTER WELDING AND GRIND SMOOTH.

11 BM TO COL WEB MOMENT CONN
SCALE: 3/4" = 1'-0"

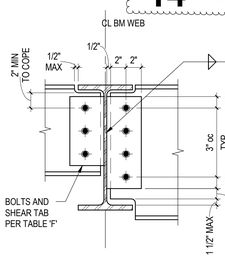


NOTES: MOMENT CONN SHOWN WITH THIS SYMBOL ON PLAN:

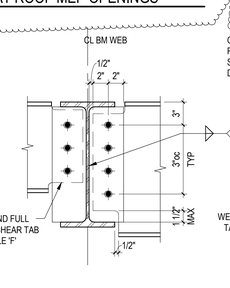
12 TYP BEAM TO BEAM MOMENT CONNECTION
SCALE: 3/4" = 1'-0"



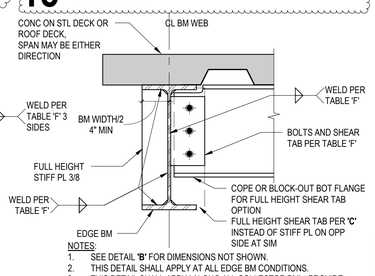
A BEAM TO WF COL FLANGE CONNECTION



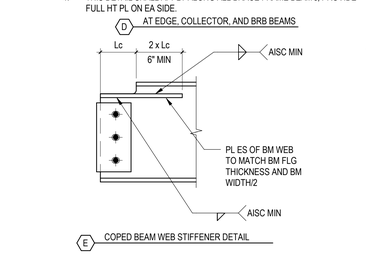
B BEAM TO BEAM CONNECTION



C BEAM TO BEAM CONNECTION



NOTES:
1. SEE DETAIL **'B'** FOR DIMENSIONS NOT SHOWN.
2. THIS DETAIL SHALL APPLY AT ALL EDGE BM CONDITIONS.
3. THIS DETAIL SHALL APPLY ALONG ALL COLLECTOR BM's, PROVIDE FULL HT PL ON EA SIDE.
4. THIS DETAIL SHALL APPLY ALONG ALL BRACE FRAME BEAM's, PROVIDE FULL HT PL ON EA SIDE.



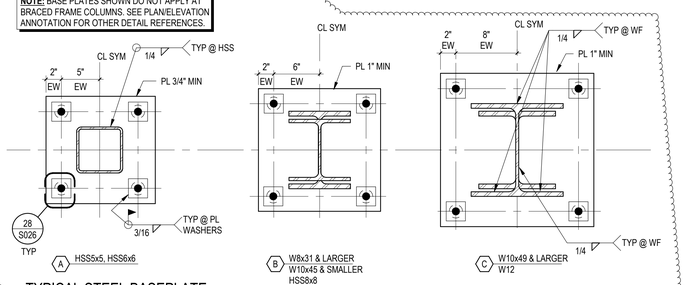
NOTES:
1. BOLTS SHALL BE ASTM F1852 TWIST-OFF TENSION CONTROL BOLT ASSEMBLIES
2. SHEAR TABS SHALL HAVE HORIZONTAL SHORT SLOTTED HOLES AND BEAMS SHALL HAVE STANDARD ROUND HOLES, UNO
3. COPED BEAMS SHALL MEET THE LIMITS SHOWN IN TABLE 'F' FOR COPE DEPTHS AND LENGTHS. WHEN ACTUAL COPE LENGTH EXCEEDS LIMITS SHOWN IN TABLE, A WEB STIFFENER SHALL BE ADDED PER DETAIL 'E'

STEEL BEAM CONNECTION TABLE (F)

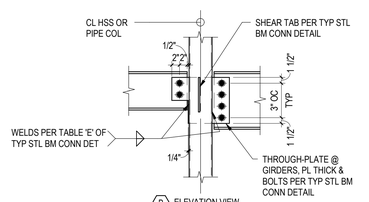
MEMBER SIZE		MINIMUM NUMBER OF 3/4"Ø F1852 BOLTS	SHEAR TAB THICKNESS (A36) (in)	WELD SIZE (in)	MAX TOP COPE DEPTH (in)	MAX COPE LENGTH	
CHANNEL	WIDE FLANGE, WT					TOP COPE ONLY (in)	TOP & BOT COPE (in)
C8-C9-C10 MC9-MC10	W8-W10	2	3/8	1/4	2	6	2.5
C12 MC12-MC13	W12-W14, WT13.5	3	3/8	1/4	2	6	3
C15 MC18	W16, WT16.5	4	3/8	1/4	2	7	4
	W18	5	3/8	1/4	3	9	5
	W21-W24	6	3/8	1/4	3	11	7
	W27	7	3/8	1/4	3	14	10
	W30	8	3/8	1/4	3	18	14

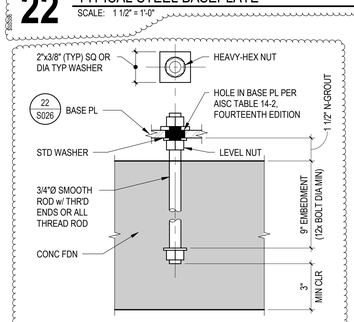
25 TYPICAL SHEAR TAB STEEL BEAM CONNECTION
SCALE: 1 1/2" = 1'-0"

NOTE: BASE PLATES SHOWN DO NOT APPLY AT BRACED FRAME COLUMNS. SEE PLAN/ELEVATION ANNOTATION FOR OTHER DETAIL REFERENCES.

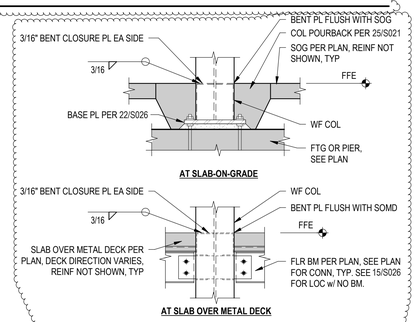


22 TYPICAL STEEL BASEPLATE
SCALE: 1 1/2" = 1'-0"

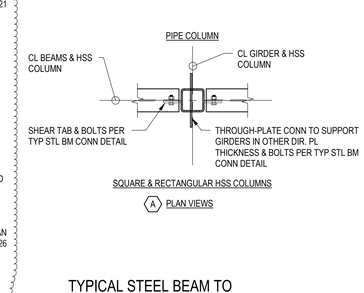




28 ANCHOR ROD ASSEMBLIES
SCALE: 3" = 1'-0"



29 WF COLUMN CLOSURE PLATE AT CONCRETE
SCALE: 3/4" = 1'-0"



30 TYPICAL STEEL BEAM TO HSS COLUMN CONNECTION
SCALE: 3/4" = 1'-0"

NOTES:
1. FOR INFO NOT SHOWN SEE 7/S027
2. WHERE COL CONTINUES ABOVE CRUCIFORM CONN, CJP COL ALL AROUND AND GRIND SMOOTH AFTER CRUCIFORM CONN IS INSTALLED

18 STEEL BEAM TO COLUMN
SCALE: 1" = 1'-0"

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City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	BLO/RB/AM
Checked by:	TD

Revisions

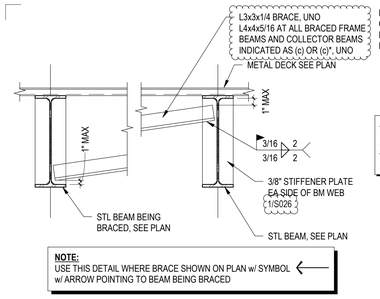
Date	Description
1.30.23	IFC Revisions

TYPICAL STEEL DETAILS

S026

PRCNC20221036
STRUCTURAL REVISION

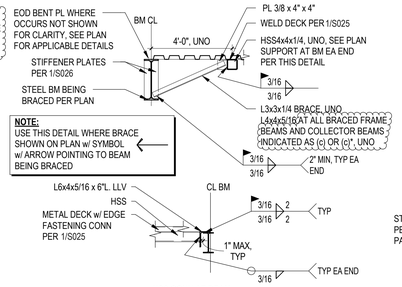
IFC SET



NOTE:
USE THIS DETAIL WHERE BRACE SHOWN ON PLAN w/ SYMBOL w/ ARROW POINTING TO BEAM BEING BRACED

1 STIFFENER BRACE AT BEAM

SCALE: 1" = 1'-0"



NOTE:
USE THIS DETAIL WHERE BRACE SHOWN ON PLAN w/ SYMBOL w/ ARROW POINTING TO BEAM BEING BRACED

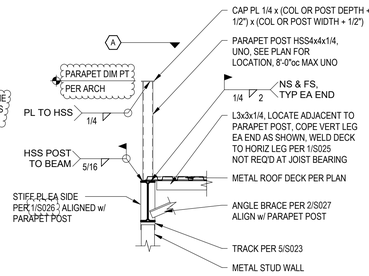
HSS CONN AT BEAM

2 BOTTOM FLANGE BRACE DETAIL

SCALE: 1/2" = 1'-0"

MEMBER	PL THICKNESS	BOLT Ø (IN)	"N"	Lh (IN)	Sh (IN)	Lv (IN)	Sv (IN)	WELD SIZE "B"
C15	1/2"	3/4	PER DETAIL	2	3	1-1/2	3	5/16
W8, W10	1/2"	3/4	2	2	3	1-1/2	3	5/16
W12	1/2"	3/4	3	2	3	1-1/2	3	5/16
W14	1/2"	3/4	3	2	3	1-1/2	3	5/16
W16	1/2"	3/4	4	2	3	1-1/2	3	5/16
W18	1/2"	3/4	5	2	3	1-1/2	3	5/16
W21-W24	1/2"	3/4	6	2	3	1-1/2	3	5/16
W27	1/2"	3/4	7	2	3	1-1/2	3	5/16
W30	3/4"	1	8	2	3	1-1/2	3	1/2

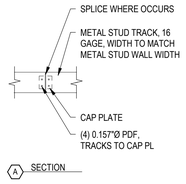
NOTES:
1. THIS DETAIL APLLIES TO ALL BEAMS INDICATE AS (c) ON PLAN.
2. WHERE COLLECTOR BEAMS FRAME INTO MULTIPLE SIDES, OR WHERE COLLECTOR BEAMS FRAME IN ONE DIRECTION AND GIRDERS FRAME IN THE PERPENDICULAR DIRECTION. PROVIDE CRUCIFORM SHAPE PER 18/S026 , UNO.
3. AT HSS THRU PL, WHERE THICKNESS & WELD DIFFER DUE TO DIFFERENT BEAM SIZES, USE THE LARGER OF THE TWO.

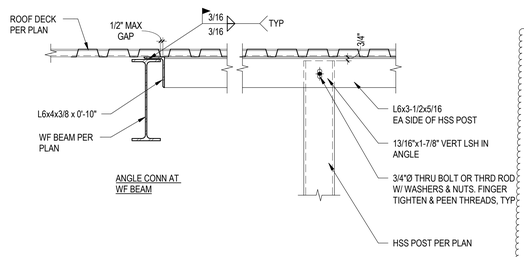


3 PARAPET POST DETAIL

SCALE: 1/2" = 1'-0"

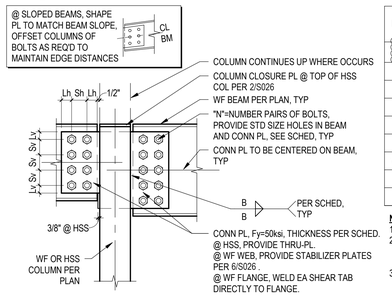
NOTES:
1. COLUMNS SHALL BE EXTENDED FOR PARAPET WHERE NOTED ON PLAN.
2. SEE 1/S024 FOR INFO NOT SHOWN.
3. WHERE METAL STUD TRACKS INTERSECT AT CORNERS, COPE LEGS AND ATTACH TRACKS w/ (4) #10 SCREWS.





5 POST CONN AT ROOF DECK

SCALE: 1" = 1'-0"

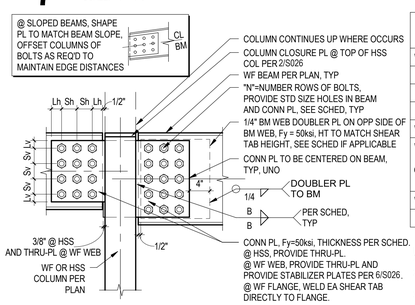


7 DRAG CONNECTION AT BEAM/COLUMN

SCALE: 1" = 1'-0"

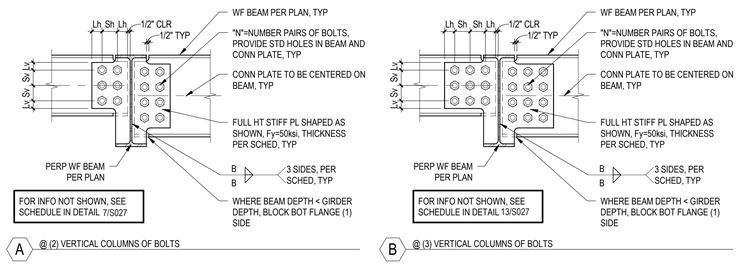
MEMBER	PL THICKNESS	BOLT Ø (IN)	"N"	Lh (IN)	Sh (IN)	Lv (IN)	Sv (IN)	WELD SIZE "B"	BM WEB DOUBLER PL
W12x40 (c)*	1"	1	3	2	3	1-1/2	3	5/8	YES
W14	1"	1	3	2	3	1-1/2	3	3/8	NO
W16	1"	1	4	2	3	1-1/2	3	3/8	NO
W18	1"	1	5	2	3	1-1/2	3	3/8	NO
W18x50 (c)*	1"	7/8	5	2	3	1-1/2	3	3/8	NO
W18x55 (c)* ALONG GRIDS B7 & C7 @ RF	1"	1	5	4	3	1-1/2	3	5/8	NO
W18x55 (c)*	1"	1	5	3	3	1-1/2	3	5/8	NO
W21-W24	1"	1	6 NOTE 4	2	3	1-1/2	3	5/8	NO

NOTES:
1. THIS DETAIL APLLIES TO ALL BEAMS INDICATE AS (c)* ON PLAN.
2. WHERE COLLECTOR BEAMS FRAME INTO MULTIPLE SIDES, OR WHERE COLLECTOR BEAMS FRAME IN ONE DIRECTION AND GIRDERS FRAME IN THE PERPENDICULAR DIRECTION, PROVIDE CRUCIFORM SHAPE PER 18/S026 , UNO.
3. AT THRU PL's, WHERE THICKNESS & WELD DIFFER DUE TO DIFFERENT BEAM SIZES, USE THE LARGER OF THE TWO.
4. WHERE (C)* BEAM FRAME INTO PERPENDICULAR BEAM WITH LESS DEPTH, NUMBER OF ROWS OF BOLTS TO BE BASED ON PERPENDICULAR BEAM SIZE. BOLT GROUP SHALL BE CENTERED ON PERPENDICULAR BEAM.



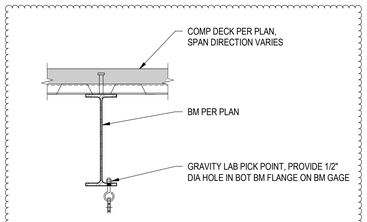
13 DRAG CONNECTION AT BEAM/COLUMN

SCALE: 1" = 1'-0"



19 DRAG CONN AT BEAM/BEAM

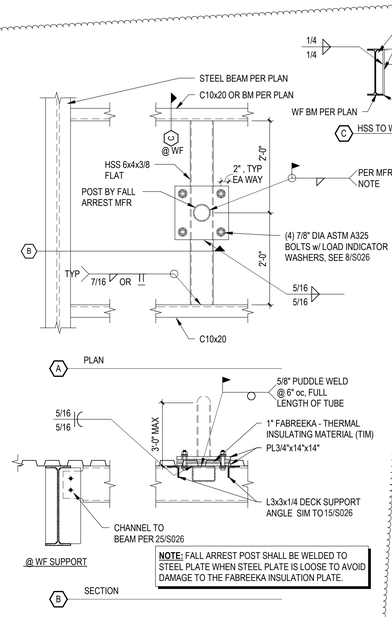
SCALE: 1" = 1'-0"



NOTES:
1. THIS APPLIES AT APPROXIMATELY SIX LOCATIONS ABOVE THE GRAVITY LAB. GC COORDINATE PICK POINT LOCATIONS.
2. PICK POINT HARDWARE DESIGNED BY OTHERS.

25 GRAVITY LAB PICK POINTS

SCALE: 1" = 1'-0"

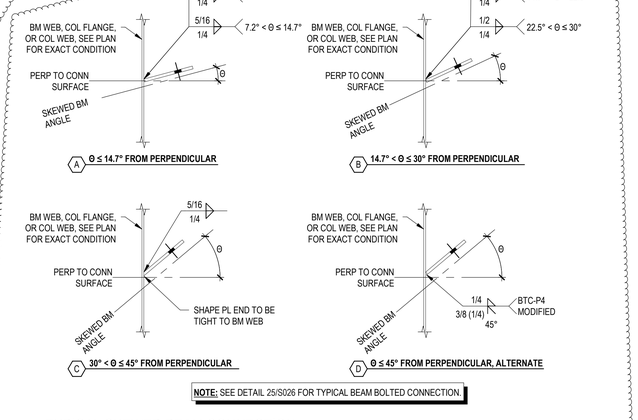


NOTE: FALL ARREST POST SHALL BE WELDED TO STEEL PLATE WHEN STEEL PLATE IS LOOSE TO AVOID DAMAGE TO THE FABREEKA INSULATION PLATE.

16 FALL PROTECTION PIER

SCALE: 3/4" = 1'-0"

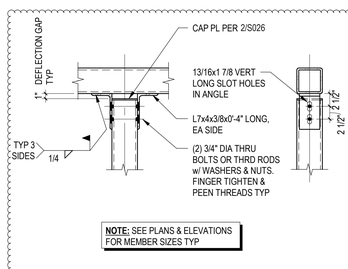
PRCNC20221036
STRUCTURAL REVISION



NOTE: SEE DETAIL 25/S026 FOR TYPICAL BEAM BOLTED CONNECTION.

17 TYPICAL SKEWED BEAM CONNECTION

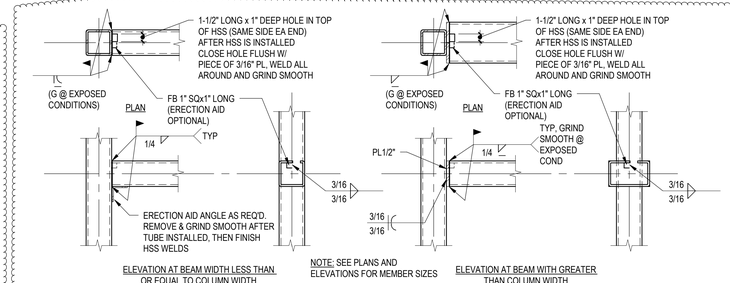
SCALE: 1 1/2" = 1'-0"



NOTE: SEE PLANS & ELEVATIONS FOR MEMBER SIZES TYP

22 HSS POST TO STRUCTURE SLIP CONN

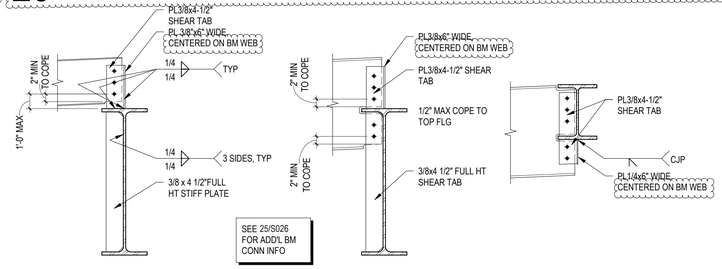
SCALE: 1" = 1'-0"



NOTE: SEE PLANS AND ELEVATIONS FOR MEMBER SIZES

23 HSS BM TO HSS COL/POST

SCALE: 1" = 1'-0"



A BEAM ENTIRELY ABOVE GIRDER TOP FLG
B BEAM PARTIALLY ABOVE GIRDER TOP FLG
C BEAM PARTIALLY BELOW GIRDER BOTTOM FLG

29 BEAM TO BEAM WITH VARIOUS HEIGHTS & DEPTHS

SCALE: 3/4" = 1'-0"

integrus ARCHITECTURE
117 SOUTH MAIN STREET, SUITE 100, SEATTLE, WA 98104
TELEPHONE (206)576-1797 FAX (206)576-1798

THERESA H. DANIEL — 38621 — REGISTERED PROFESSIONAL ENGINEER — STATE OF WASHINGTON

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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

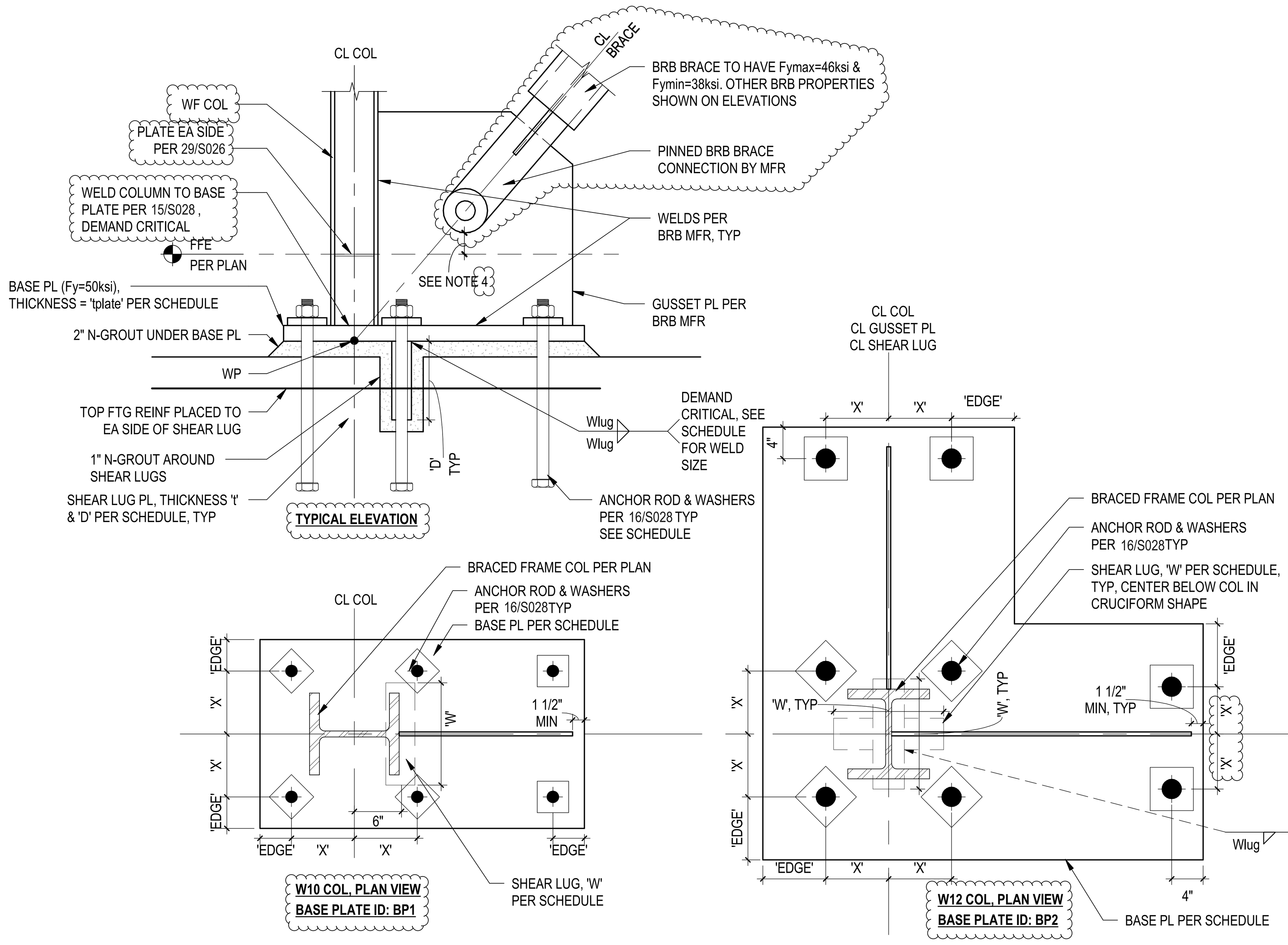
Revisions

Date	Description
1.30.23	IFC Revisions

TYPICAL STEEL DETAILS

S027

IFC SET

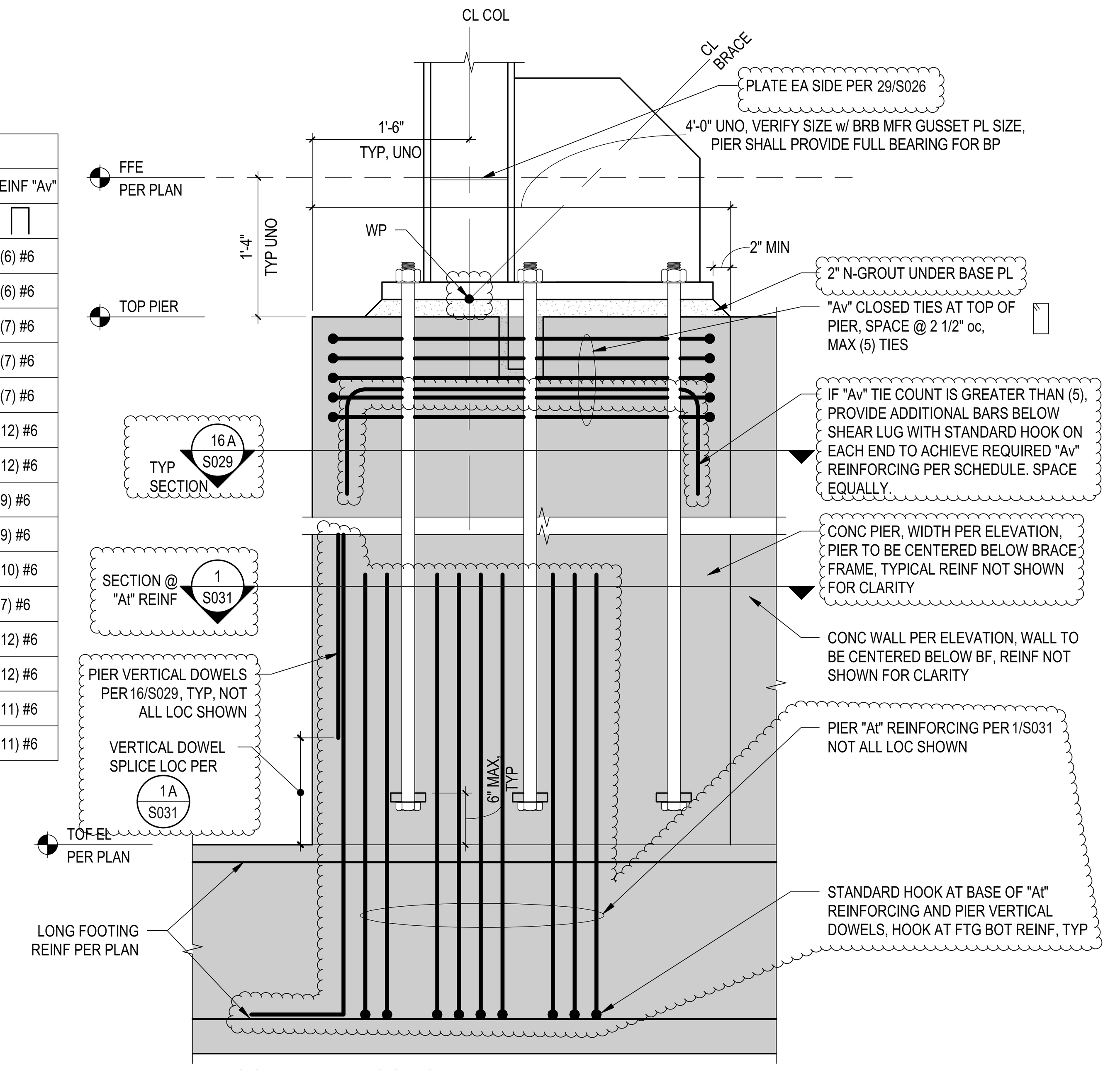


BASE PLATE SCHEDULE

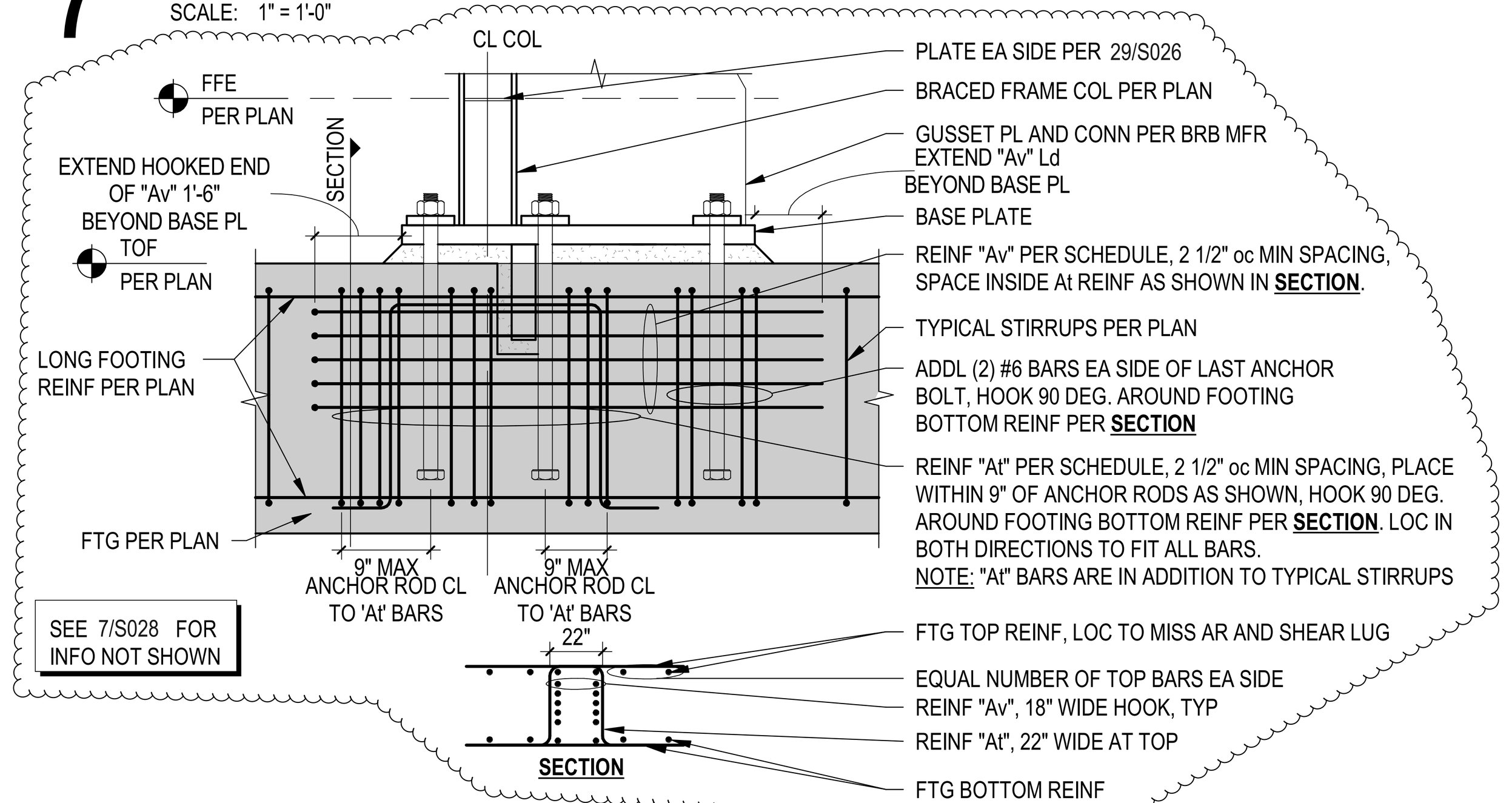
BRACE FRAME	BF COL	BASE PLATE ID	'tplate'	'EDGE'	'X'	DIAMETER	EMBED	EMBED PL WASHER	FTG CONN	't'	'D'	'W'	Wlug	REINF "At"	REINF "Av"
(BASE PLATE)						(ANCHOR RODS, NOTE 1)				(SHEAR LUG)					
BF1	A4	BP1	2"	3"	7"	1"	20"	PL1"x4"x4"	13/S028	3"	8"	11"	1"	(8) #5	(6) #6
	A1	BP1	2"	3"	7"	1"	20"	PL1"x4"x4"	13/S028	3"	8"	11"	1"	(8) #5	(6) #6
BF2	C16	BP1	2 1/2"	3"	7"	1 1/2"	20"	PL1"x4"x4"	13/S028	3 1/2"	8"	11"	1 1/8"	(15) #5	(7) #6
BF3	D1	BP1	2 1/4"	3"	7"	1 1/2"	20"	PL1"x4"x4"	13/S028	3 1/2"	8"	11"	1 1/8"	(13) #5	(7) #6
	D3	BP1	2 1/4"	3"	7"	1 1/2"	20"	PL1"x4"x4"	13/S028	3 1/2"	8"	11"	1 1/8"	(13) #5	(7) #6
BF4	C7	BP1	3"	4"	7 1/2"	2"	NOTE 5	PL1"x5"x5"	11/S028	4 1/2"	9"	11"	1 1/2"	1/S031	(12) #6
	C10	BP1	3"	4"	7 1/2"	2"	NOTE 5	PL1"x5"x5"	11/S028	4 1/2"	9"	11"	1 1/2"	1/S031	(12) #6
BF5	D3	BP1	2 3/4"	3"	7"	1 1/2"	NOTE 5	PL1"x4"x4"	11/S028	4"	8"	11"	1 1/4"	1/S031	(9) #6
	D1	BP1	2 3/4"	3"	7"	1 1/2"	NOTE 5	PL1"x4"x4"	11/S028	4"	8"	11"	1 1/4"	1/S031	(9) #6
BF7	B4	BP1	2 3/4"	4"	7 1/2"	1"	NOTE 5	PL1"x5"x5"	11/S028	4"	8"	11"	1 1/2"	1/S031	(10) #6
BF8	A1	BP1	2 1/2"	3"	7"	1 1/2"	NOTE 5	PL1"x4"x4"	11/S028	3 1/2"	8"	11"	1"	1/S031	(7) #6
BF9	C9	BP1	4"	4"	8"	2 1/2"	NOTE 5	PL1 1/2"x7 1/2"x7 1/2"	11/S028	4 1/2"	9"	11"	1 9/16"	1/S031	(12) #6
	C7	BP2	4"	8"	8"	3"	NOTE 5	PL2"x9"x9"	11/S028, SIM	4"	8"	14"	1 1/4"	1/S031	(12) #6
BF10	D4	BP1	4"	4"	8"	2 1/2"	NOTE 5	PL1 1/2"x7 1/2"x7 1/2"	11/S028	4 1/2"	9"	11"	1 9/16"	1/S031	(11) #6
	D6	BP2	4"	8"	8"	3"	NOTE 5	PL2"x9"x9"	11/S028, SIM	4"	8"	14"	1 1/4"	1/S031	(11) #6

NOTES:
1. FOR ADDITIONAL ANCHOR ROD INFO, SEE 16/S028
2. ALL DEMAND CRITICAL WELDS REQUIRE 100% SPECIAL INSPECTION
3. FOR ADDITIONAL FOOTING REINF SEE 11/S028 OR 13/S028
4. MAINTAIN 1" CLEAR BETWEEN BRACE AND SLAB. SEE DETAIL 17/S028
5. SEE DETAIL 11/S028 FOR ANCHOR ROD EMBEDMENT INTO CONCRETE PIER.

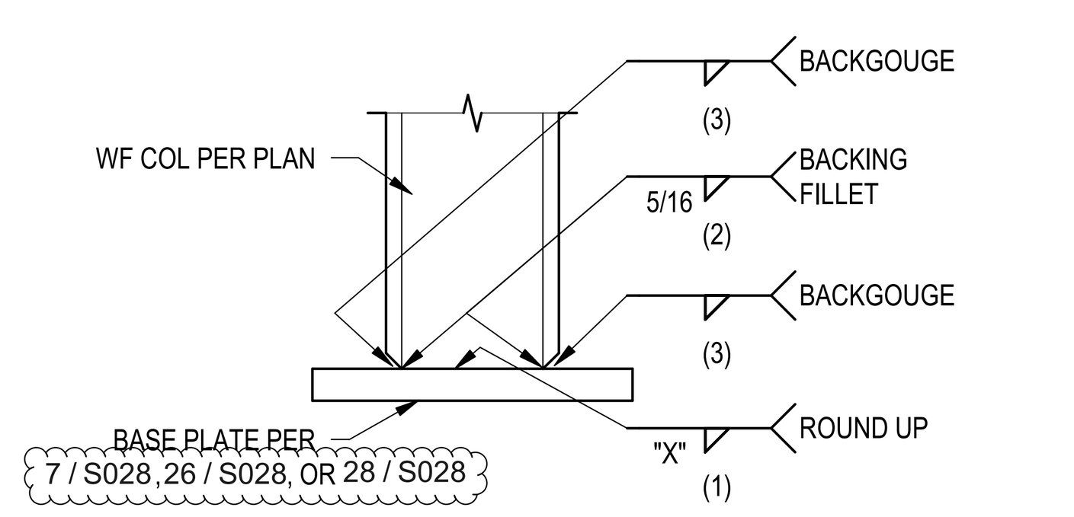
7 GUSSET & BASE PL AT BRACED FRAME
SCALE: 1" = 1'-0"



11 BRB COLUMN AT CONCRETE PIER
SCALE: 1" = 1'-0"



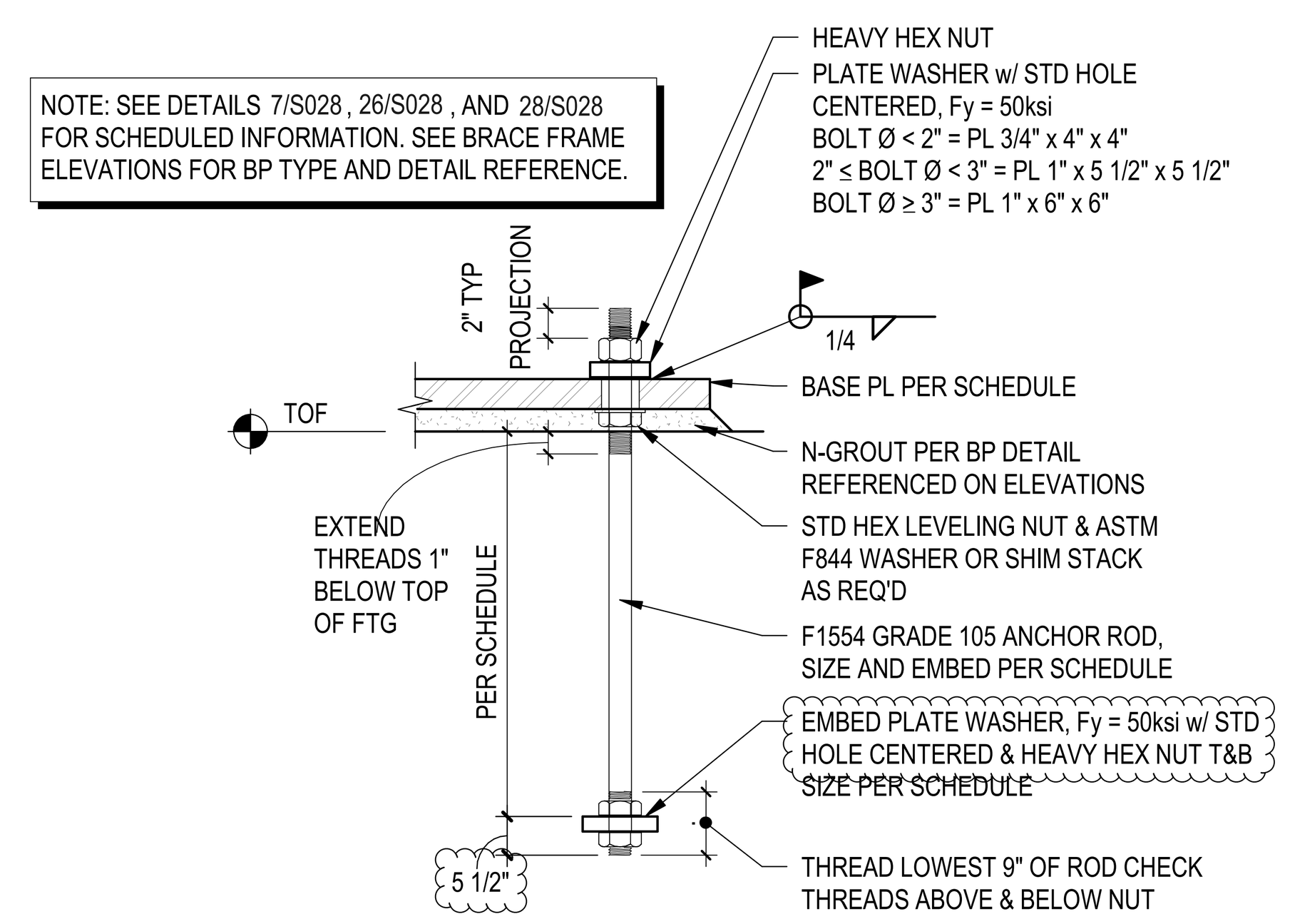
13 FOUNDATION REINFORCING AT BRB COLUMN
SCALE: 3/4" = 1'-0"



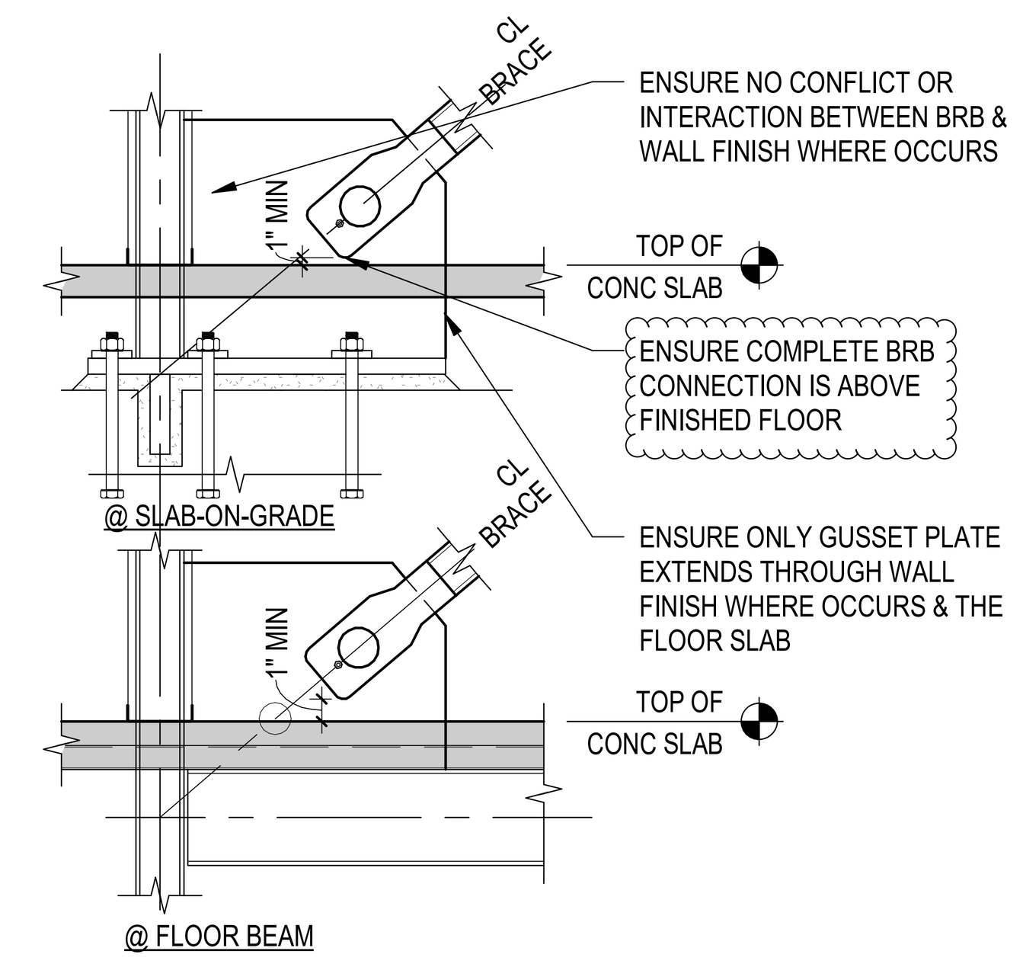
WELD PROCEDURE PER AISC DESIGN GUIDE #4
- PREPARE THE FLANGES WITH A 45 DEGREE, FULL DEPTH BEVEL
-FIT UP END-PLATE WITH A MINIMUM ROOT OPENING
-PREHEAT THE SPECIMENS AS REQ'D BY AWS SPEC
-PREPARE SURFACES FOR WELDING AS REQ'D BY AWS SPEC
-PLACE WEB WELD (1), SIZED "X" = WF WEB THICKNESS/2 + 1/8", ROUND SIZE UP TO NEAREST 1/16"
-PLACE 5/16" BACKING FILLET WELD ON BEAM WEB SIDE OF THE BEAM FLANGES (2)
-BACKGOUGE THE ROOT OF THE BEVEL TO REMOVE ANY CONTAMINANTS FROM THE 5/16" BACKER FILLET WELDS (3)
-PLACE THE FLANGE GROOVE WELDS (AWS TC-U4b-GF)
NOTE FOR INSPECTION, FLANGE GROOVE WELD IS CJP & DEMAND CRITICAL

15 BRBF WF COLUMN WELD TO BASE PLATE
SCALE: 1" = 1'-0"

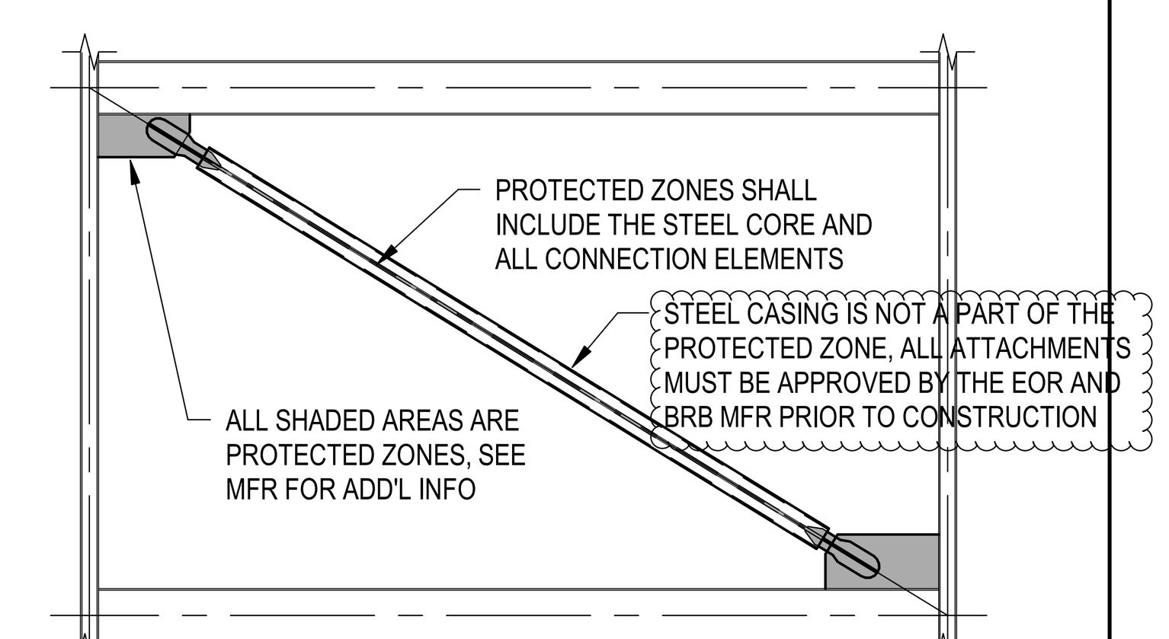
NOTE: SEE DETAILS 7/S028, 26/S028, AND 28/S028 FOR SCHEDULED INFORMATION. SEE BRACE FRAME ELEVATIONS FOR BP TYPE AND DETAIL REFERENCE.



16 ANCHOR ROD AT BRACED FRAME
SCALE: 1" = 1'-0"



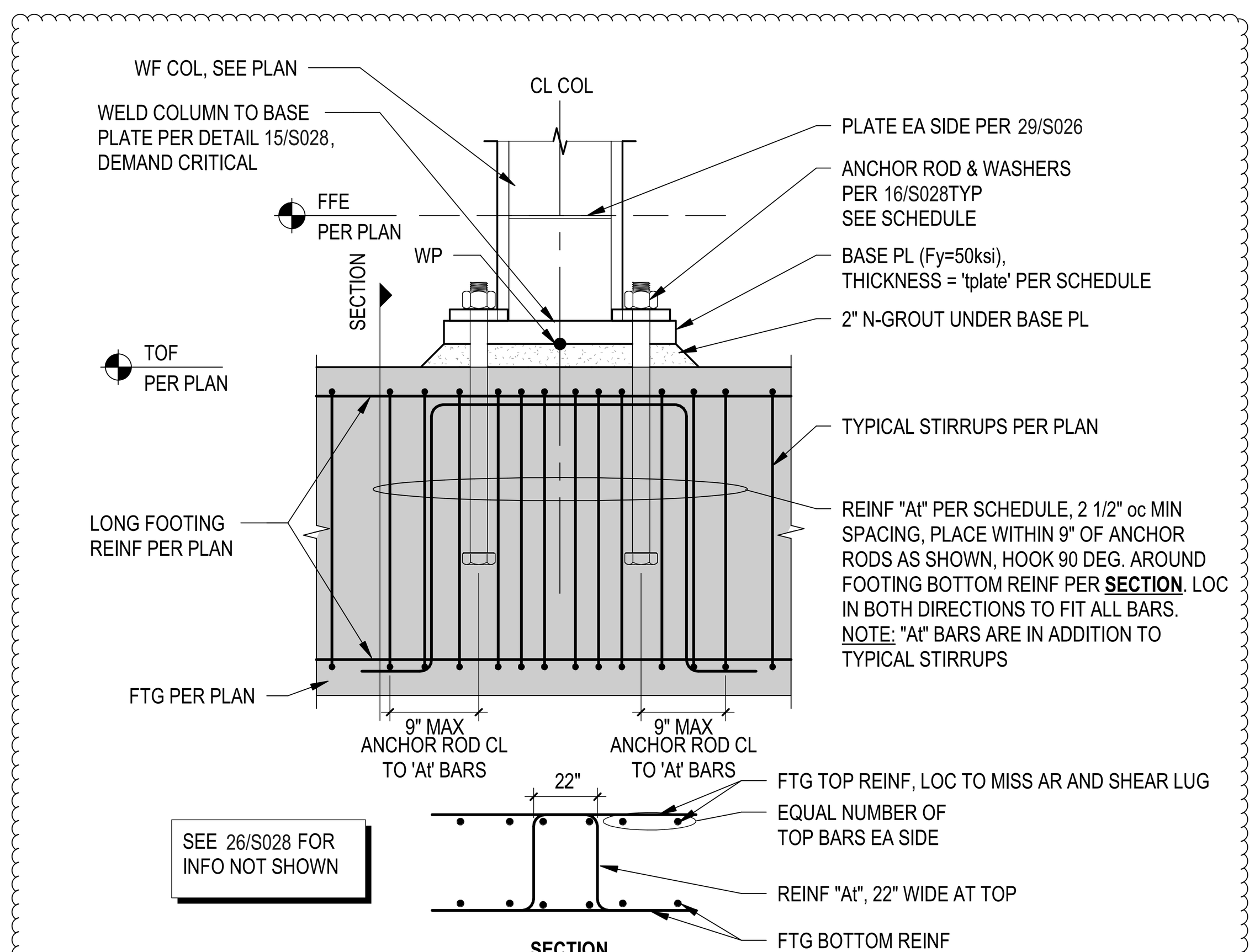
17 BRB CONN ABOVE CONCRETE DETAIL
SCALE: 1/2" = 1'-0"



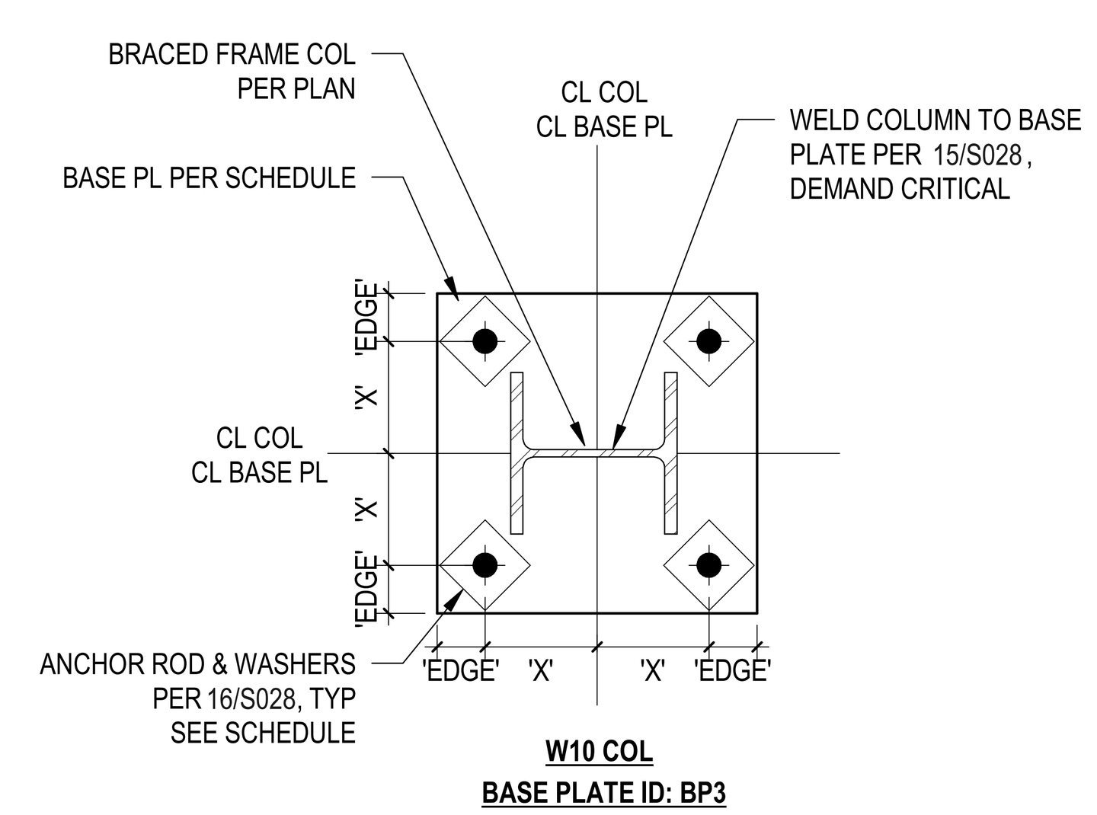
WITHIN THE PROTECTED ZONES DEFINED ABOVE, NO ATTACHMENTS (EITHER TEMPORARY OR PERMANENT) ARE ALLOWED. IN THE PROTECTED ZONES, ALL DISCONTINUITIES CREATED BY FABRICATION OR ERECTION OPERATIONS, SUCH AS TACK WELDS, ERECTION AIDS, AIR-ARC GOUGING AND THERMAL CUTTING SHALL BE REPAIRED BY METHOD REQUIRED BY THE ENGINEER OF RECORD. CONTACT ENGINEER OF RECORD WITHIN 24 HOURS WHENEVER VIOLATION OF THE PROTECTED ZONE OCCURS.

ATTACHMENTS WHICH SUPPORT OVER 50 LBS SHALL BE APPROVED BY THE EOR AND THE BRB MFR

18 PROTECTED ZONE DEFINITION REQUIREMENTS @ BRACED FRAMES
SCALE: 3/16" = 1'-0"



25 FOUNDATION REINFORCING AT BRB COLUMN WITH NO GUSSET PLATE
SCALE: 1" = 1'-0"

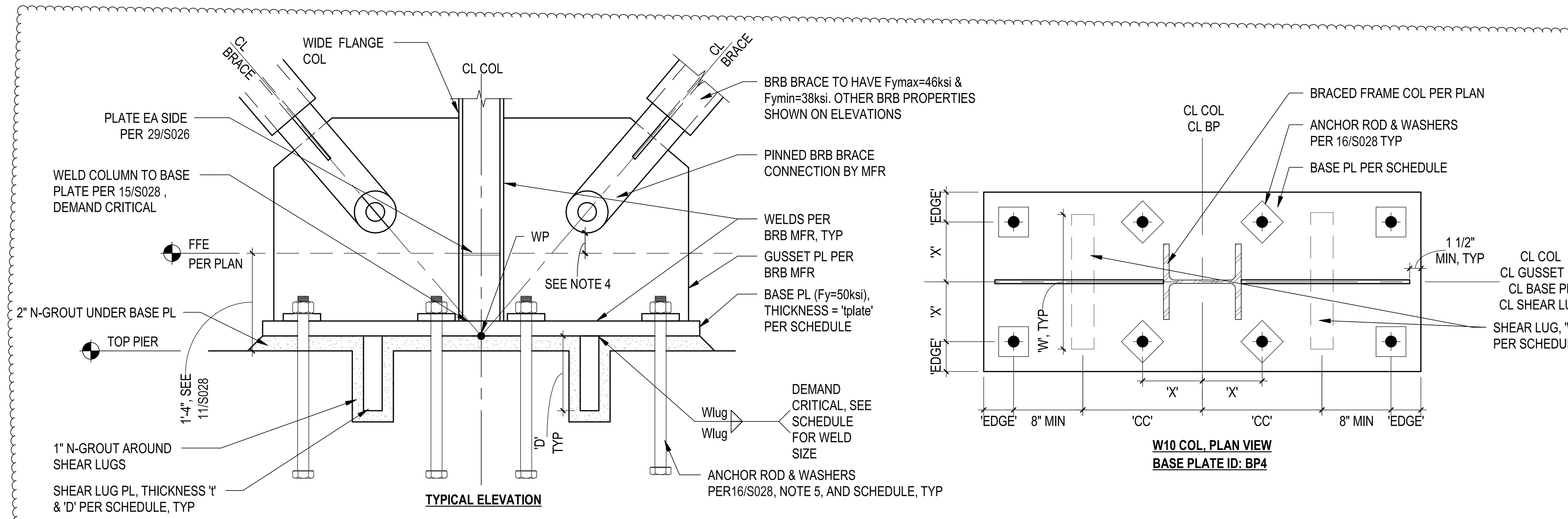


NOTES:
1. FOR ADDITIONAL ANCHOR ROD INFO, SEE 16/S028
2. ALL DEMAND CRITICAL WELDS REQUIRE 100% SPECIAL INSPECTION
3. NO "Av" REINFORCING OR SHEAR LUGS IN THIS DETAIL.
4. SEE DETAIL 11/S028 FOR ANCHOR ROD EMBEDMENT INTO CONCRETE PIER.

BASE PLATE SCHEDULE

BRACE FRAME	BF COL	BASE PLATE ID	'tplate'	'EDGE'	'X'	DIAMETER	EMBED	EMBED PL WASHER	FTG CONN	REINF "At"
(BASE PLATE)						(ANCHOR RODS, NOTE 1)				
BF2	C15	BP3	3"	3"	7"	1 1/2"	20"	PL1"x4"x4"	25/S028	(18) #5
BF3	D2	BP3	1"	3"	7"	1"	20"	PL1"x4"x4"	25/S028	(6) #5
BF4	C8	BP3	1"	3"	7"	1"	20"	PL1"x4"x4"	11/S028, SIM	1/S031
BF5	D2	BP3	1"	3"	7"	1"	NOTE 4	PL1"x4"x4"	11/S028, SIM	1/S031
BF6	A1	BP3	2 1/2"	3"	7"	1 1/2"	NOTE 4	PL1"x5"x5"	11/S028, SIM	1/S031
	A4	BP3	2 1/2"	3"	7"	1 1/2"	NOTE 4	PL1"x5"x5"	11/S028, SIM	1/S031
BF7	B6	BP3	3"	4"	7 1/2"	2"	NOTE 4	PL1"x5"x5"	11/S028, SIM	1/S031
BF8	A3	BP3	2 1/4"	3"	7"	1 1/2"	NOTE 4	PL1"x4"x4"	11/S028, SIM	1/S031

26 BRB COLUMN BASELATE WITH NO GUSSET PLATE
SCALE: 1" = 1'-0"



BASE PLATE SCHEDULE

BRACE FRAME	BF COL	BASE PLATE ID	'tplate'	'EDGE'	'X'	DIAMETER	EMBED	EMBED PL WASHER	FTG CONN	't'	'D'	'W'	Wlug	Quantity	'CC'	REINF "At"	REINF "Av"
(BASE PLATE)						(ANCHOR RODS, NOTE 1)				(SHEAR LUG)							
BF6	A2	BP4	2"	3"	7"	1 1/2"	NOTE 5	PL1"x4"x4"	11/S028 , SIM	3 1/2"	8"	18"	5/8"	2	19"	1/S031	(11) #6

NOTES:
1. FOR ADDITIONAL ANCHOR ROD INFO, SEE 16/S028
2. ALL DEMAND CRITICAL WELDS REQUIRE 100% SPECIAL INSPECTION
3. FOR ADDITIONAL FOOTING REINF SEE 11/S028
4. MAINTAIN 1" CLEAR BETWEEN BRACE AND SLAB. SEE DETAIL17/S028
5. SEE DETAIL 11/S028 FOR ANCHOR ROD EMBEDMENT INTO CONCRETE PIER.

PRCNC20221036
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28 CENTER GUSSET & BASE PL AT BRACED FRAME
SCALE: 1" = 1'-0"

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117 SOUTH MAIN STREET, SUITE 100, SEATTLE, WA 98104
TELEPHONE (206)628-3137 FAX (206)628-3138

City of Puyallup
Development & Permitting Services
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Building | Planning
Engineering | Public Works
Fire | Traffic

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked By:	TD

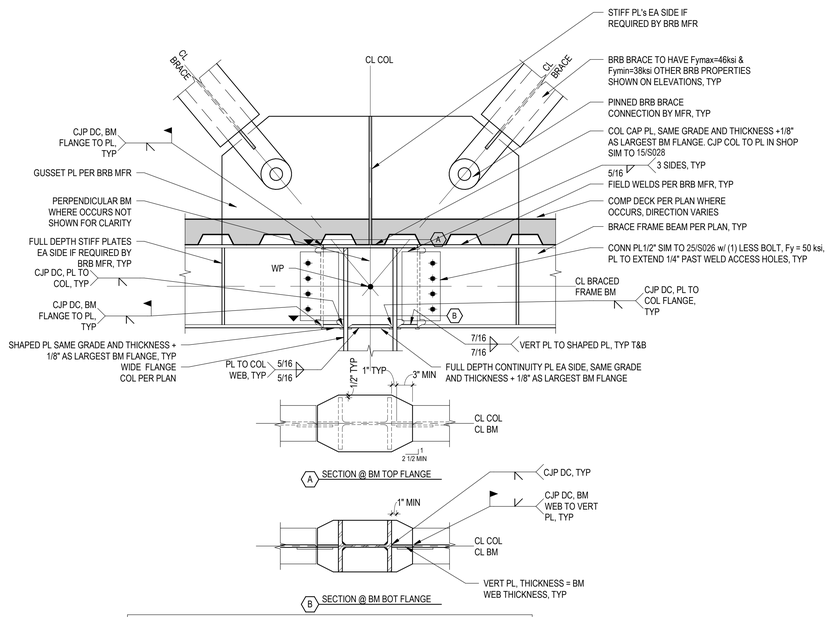
Revisions

Date	Description
1.30.23	IFC Revisions

TYPICAL BUCKLING-RESTRAINED BRACED FRAME DETAILS

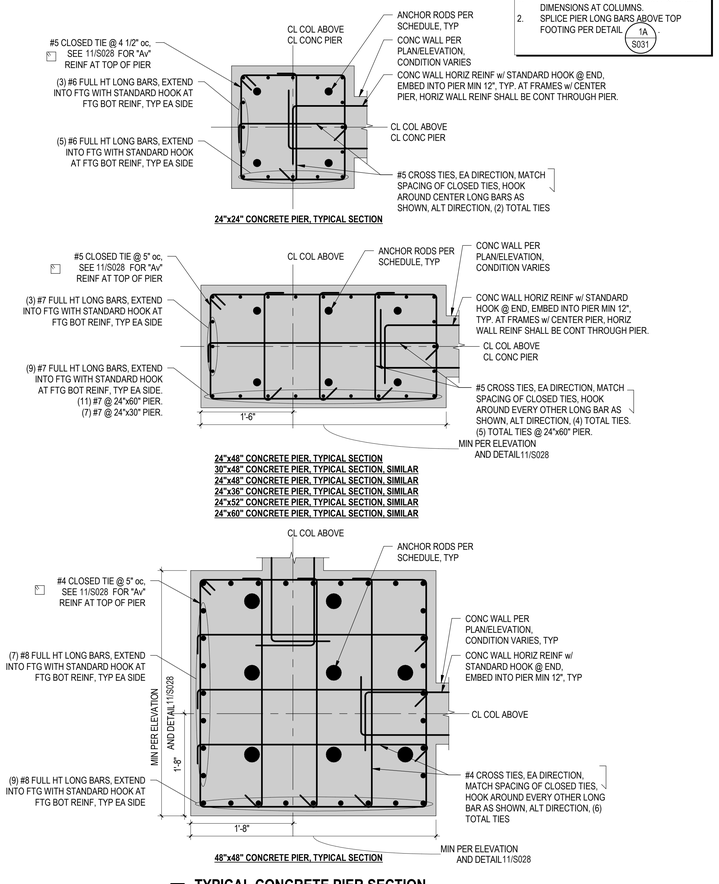
S028

IFC SET

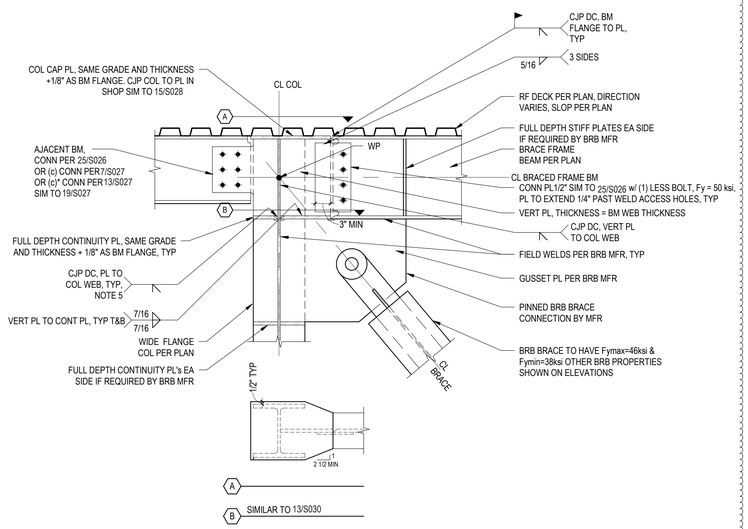


NOTES:
1. CONTINUITY CLIPS SHALL COMPLY WITH AISC 341 AND AISC 358.
2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLES SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.
5. SEE DETAIL 17/S028 FOR MINIMUM BRACE CONNECTION CLEARANCE.

7 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL
SCALE: 1" = 1'-0"

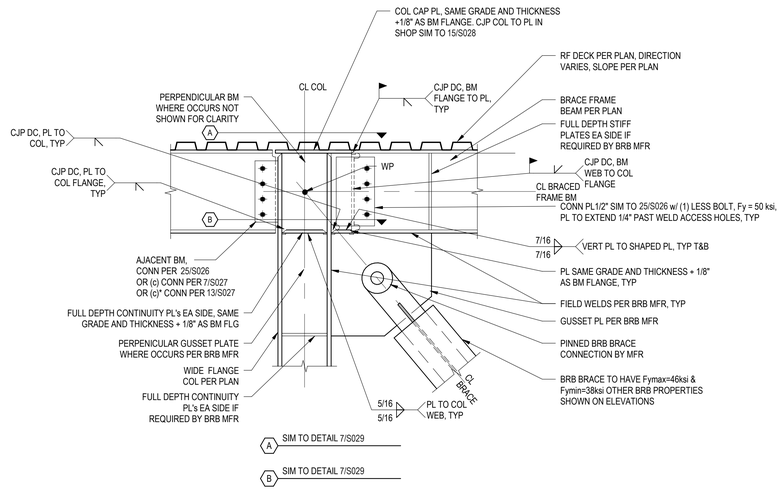


16 BRACED FRAME COLUMN CONCRETE PIER REINFORCING
SCALE: 1" = 1'-0"



NOTES:
1. CONTINUITY CLIPS SHALL COMPLY WITH AISC 341 AND AISC 358.
2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.
5. CONTINUITY PLATE WELDING ORDER:
   A. CJP WELD CONTINUITY PLATE TO COLUMN WEB ON ONE SIDE. LET WELD COOL.
   B. ON OTHER SIDE OF COLUMN WEB, CJP WELD CONTINUITY PLATE TO COLUMN WEB. LET WELD COOL.
   C. CJP WELD CONTINUITY PLATES TO COLUMN FLANGE.

25 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL
SCALE: 1" = 1'-0"



NOTES:
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2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.

28 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL
SCALE: 1" = 1'-0"

PRCNC20221036
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THOMAS H. DANIEL
STATE OF WASHINGTON
38621
REGISTERED PROFESSIONAL ENGINEER

City of Puyallup
Development & Permitting Services
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Building | Planning
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PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

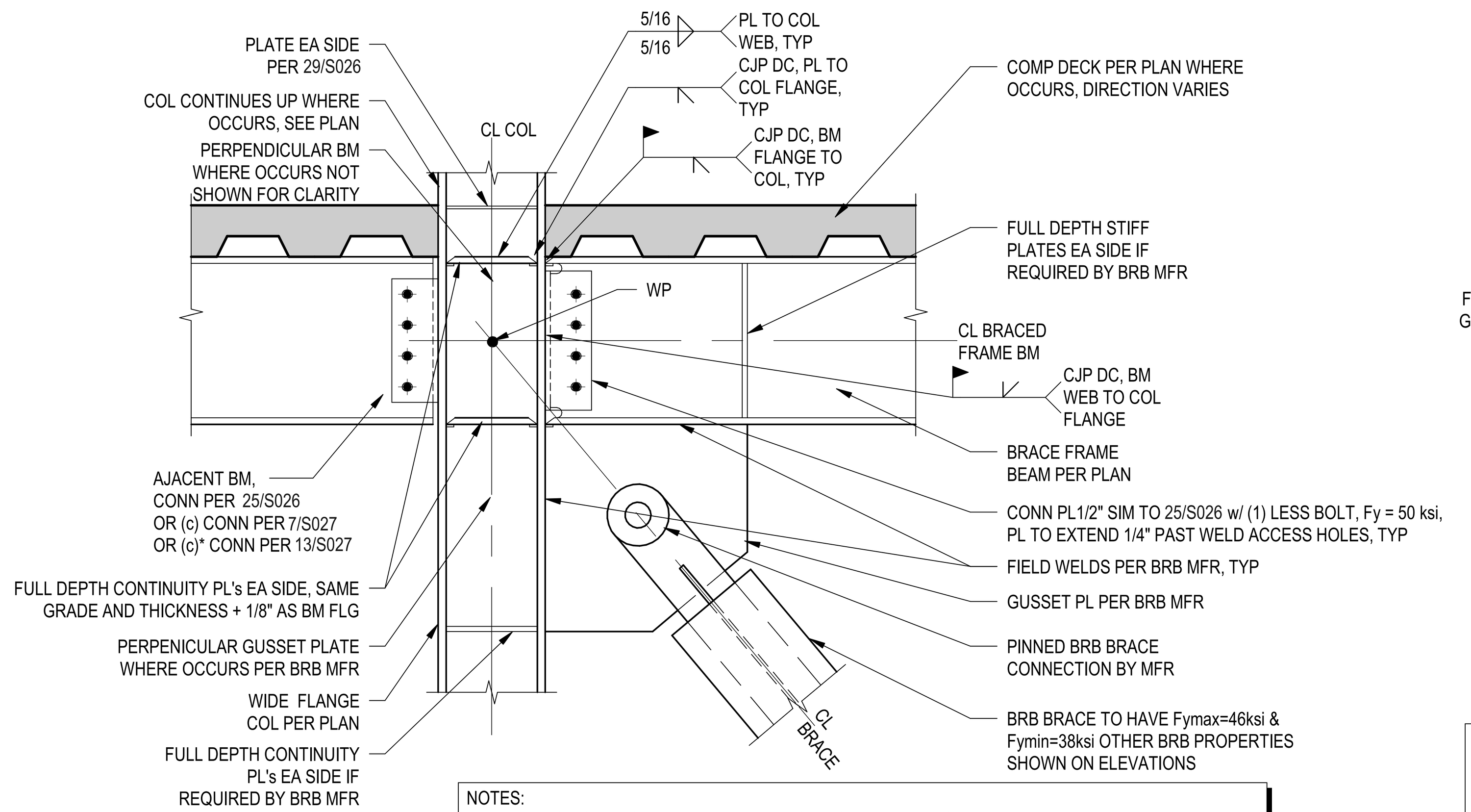
Revisions

Date	Description
1.30.23	IFC Revisions

TYPICAL BUCKLING-RESTRAINED BRACED FRAME DETAILS

S029

IFC SET



NOTES:
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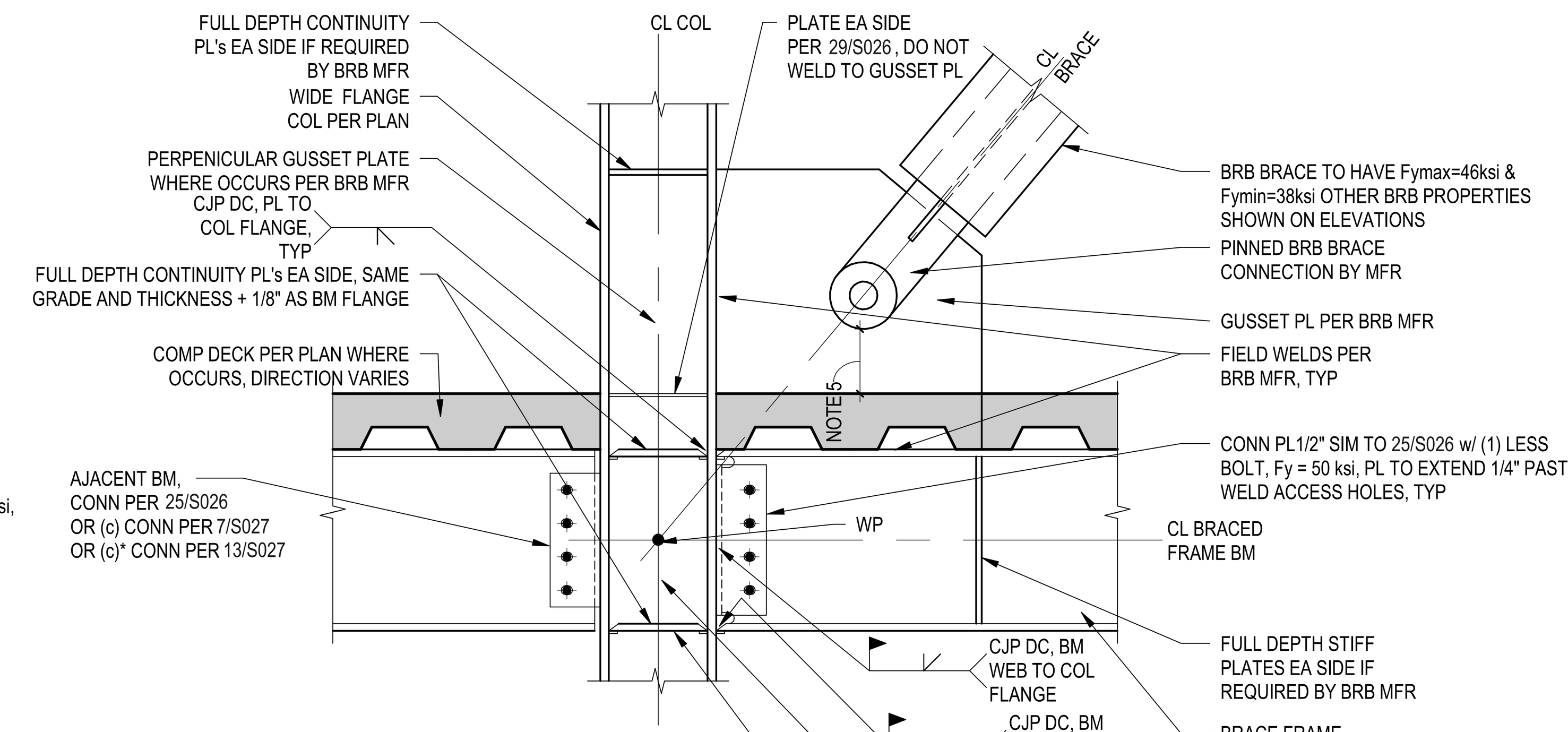
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.

7 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL

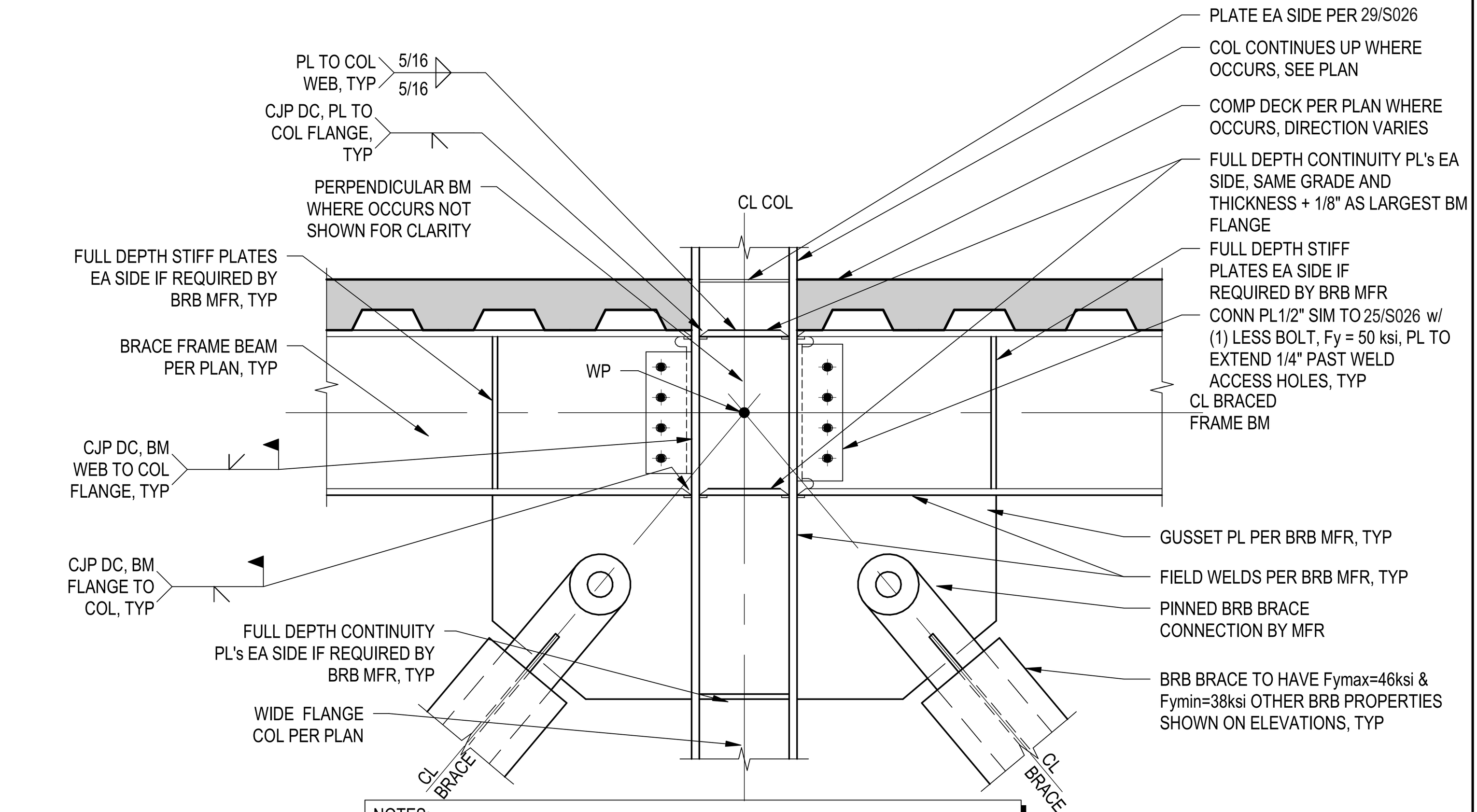
SCALE: 1" = 1'-0"



NOTES:
1. CONTINUITY CLIPS SHALL COMPLY WITH AISC 341 AND AISC 358.
2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.
5. SEE DETAIL 17/S028 FOR MINIMUM BRACE CONNECTION CLEARANCE.

9 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL

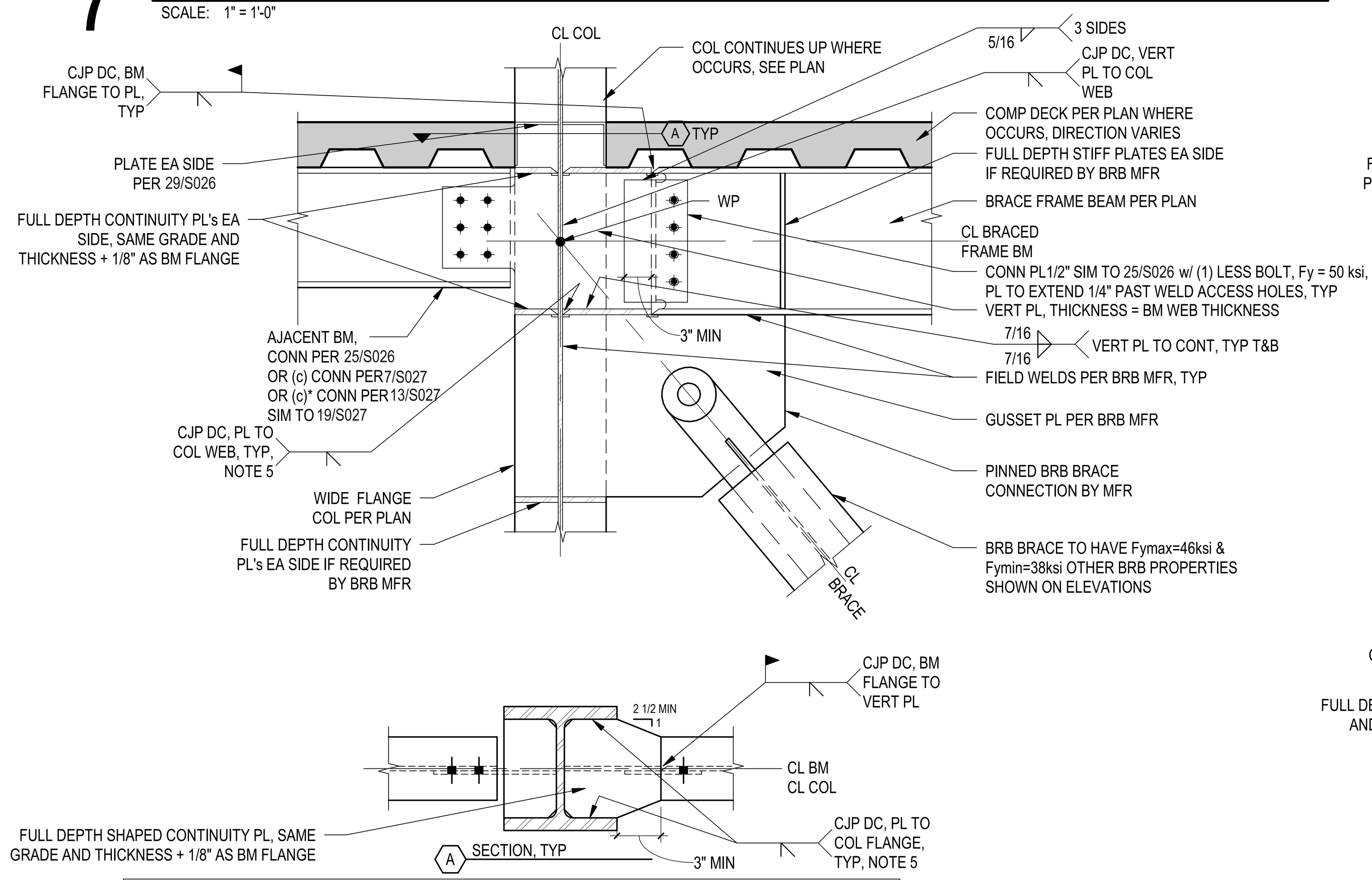
SCALE: 1" = 1'-0"



NOTES:
1. CONTINUITY CLIPS SHALL COMPLY WITH AISC 341 AND AISC 358.
2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.

11 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL

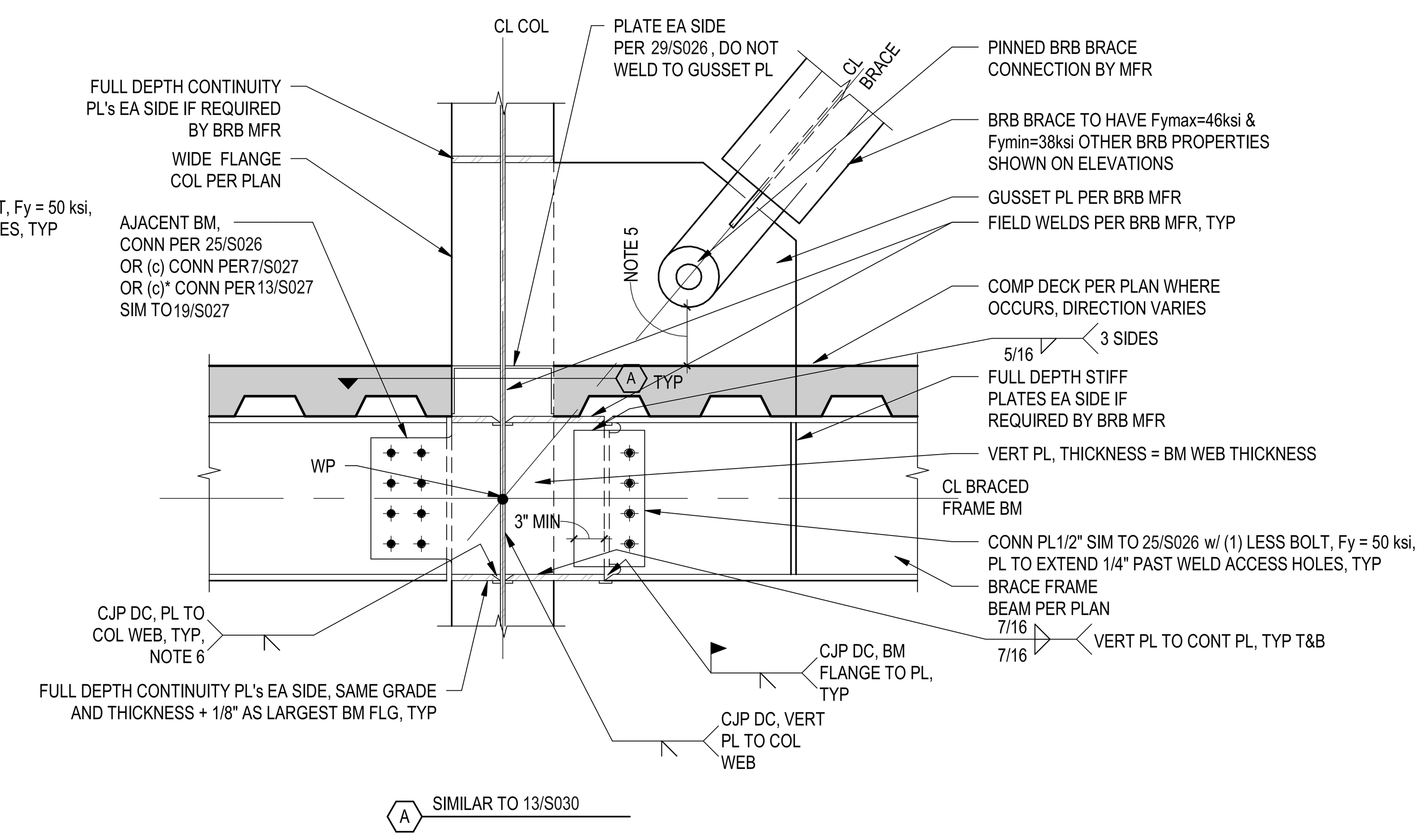
SCALE: 1" = 1'-0"



NOTES:
1. CONTINUITY CLIPS SHALL COMPLY WITH AISC 341 AND AISC 358.
2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.
5. CONTINUITY PLATE WELDING ORDER:
   A. CJP WELD CONTINUITY PLATE TO COLUMN WEB ON ONE SIDE. LET WELD COOL.
   B. ON OTHER SIDE OF COLUMN WEB, CJP WELD CONTINUITY PLATE TO COLUMN WEB. LET WELD COOL.
   C. CJP WELD CONTINUITY PLATES TO COLUMN FLANGE.

13 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL

SCALE: 1" = 1'-0"



NOTES:
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3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION.
4. WELD ACCESS HOLE SIZE AND QUALITY SHALL MEET THE REQUIREMENTS OF AWS D1.8/D1.8M.
5. SEE DETAIL 17/S028 FOR MINIMUM BRACE CONNECTION CLEARANCE.
6. CONTINUITY PLATE WELDING ORDER:
   A. CJP WELD CONTINUITY PLATE TO COLUMN WEB ON ONE SIDE. LET WELD COOL.
   B. ON OTHER SIDE OF COLUMNWEB, CJP WELD CONTINUITY PLATE TO COLUMN WEB. LET WELD COOL.
   C. CJP WELD CONTINUITY PLATES TO COLUMN FLANGE.

15 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL

SCALE: 1" = 1'-0"

NOTES:
1. CONTINUITY CLIPS SHALL COMPLY WITH AISC 341 AND AISC 358.
2. AFTER COMPLETING CJP WELDING WITH BACKER PLATES, BACKER PLATES SHALL BE REMOVED. BACK GOUGE & DRESS WITH 5/16" FILLET, TYPICAL.
3. ALL DEMAND CRITICAL (DC) WELDS REQUIRE 100% SPECIAL INSPECTION
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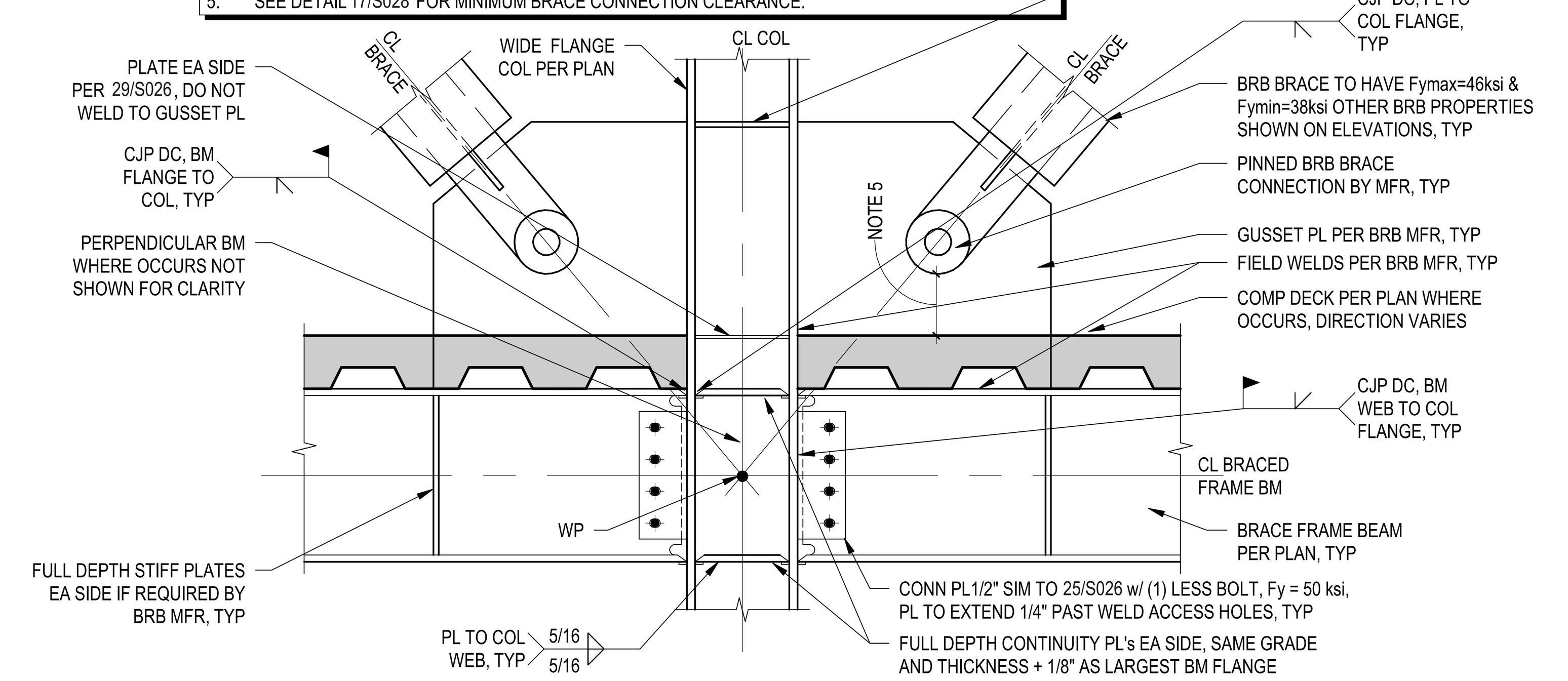
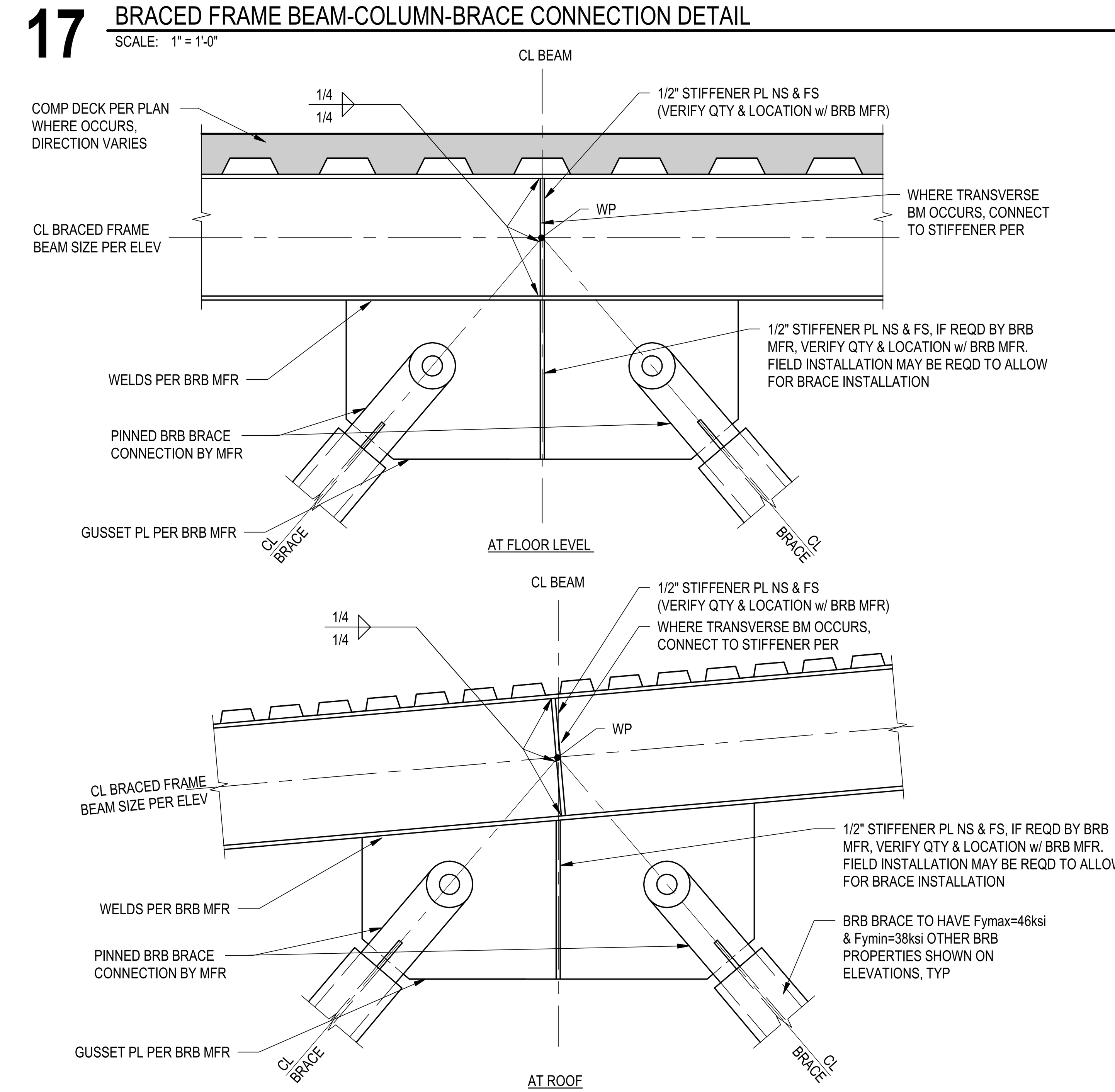
5. SEE DETAIL 17/S028 FOR MINIMUM BRACE CONNECTION CLEARANCE.



17 BRACED FRAME BEAM-COLUMN-BRACE CONNECTION DETAIL

SCALE: 1" = 1'-0"



29 V-BRACE BEAM CONNECTION DETAIL

SCALE: 1" = 1'-0"

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THERESA H. DANIEL — STATE OF WASHINGTON — 38621 — REGISTERED PROFESSIONAL ENGINEER

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

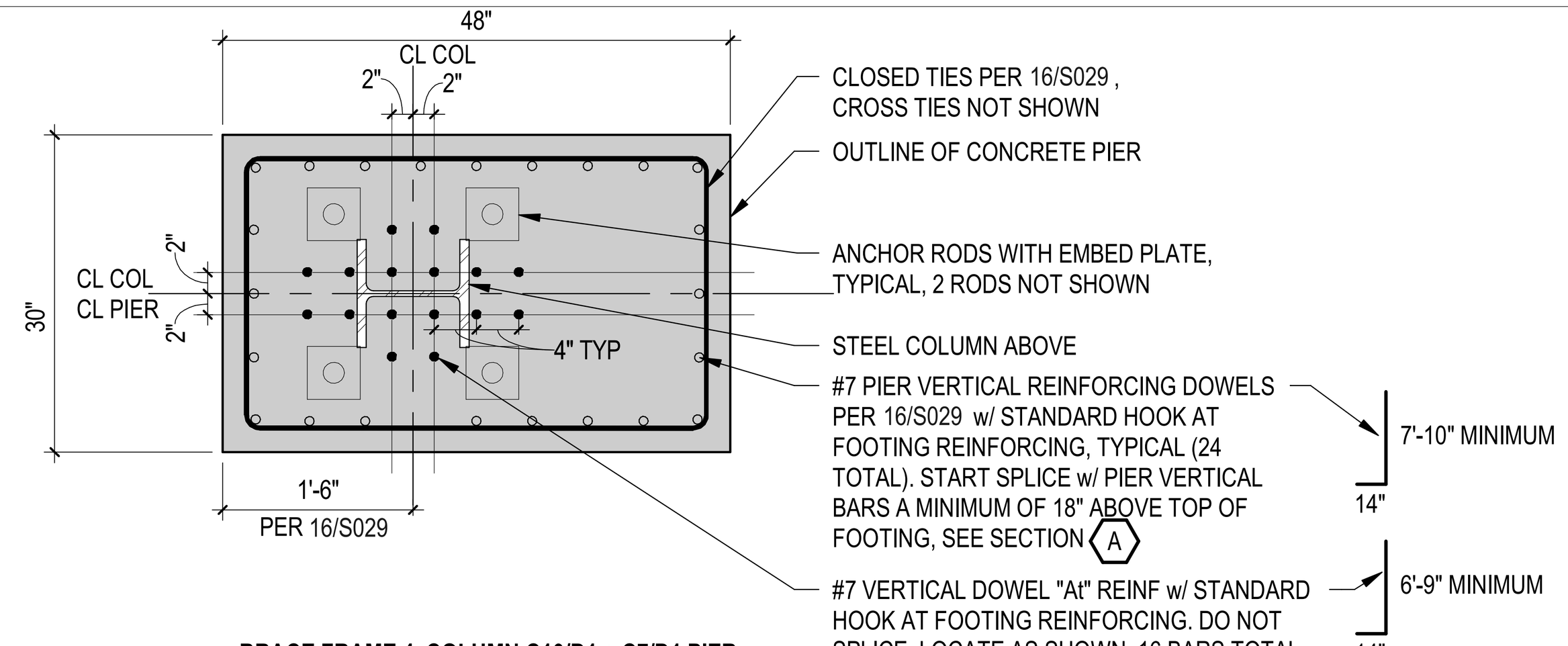
Revisions

Date	Description
1.30.23	IFC Revisions

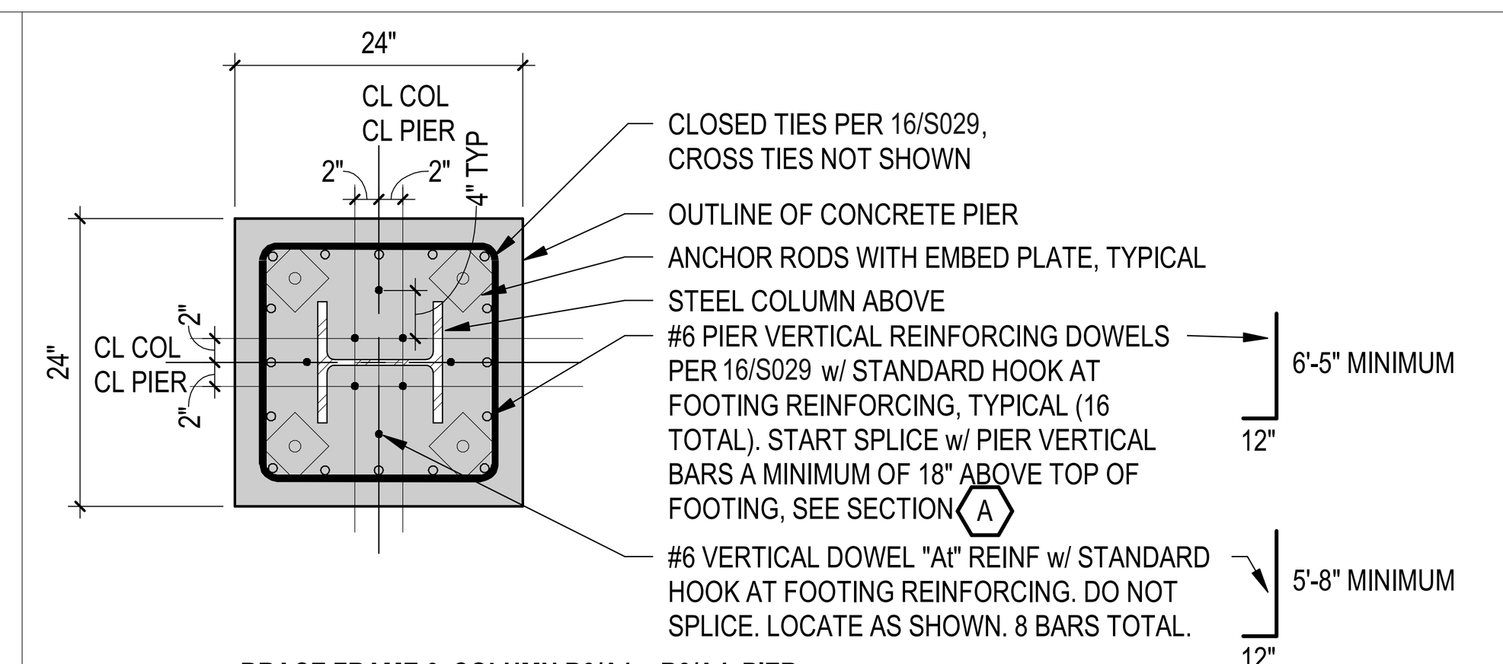
TYPICAL BUCKLING-RESTRAINED BRACED FRAME DETAILS

S030

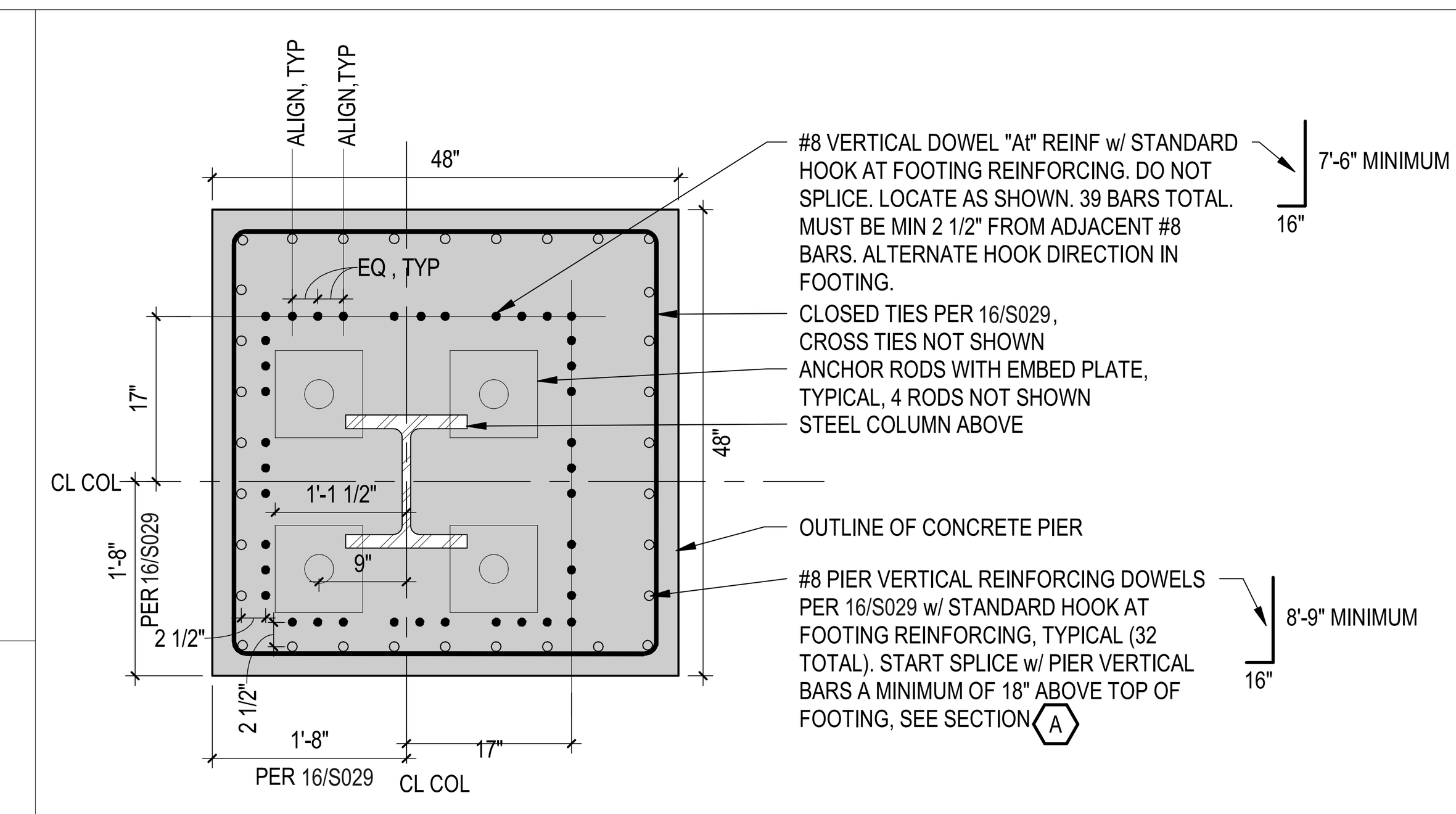
IFC SET



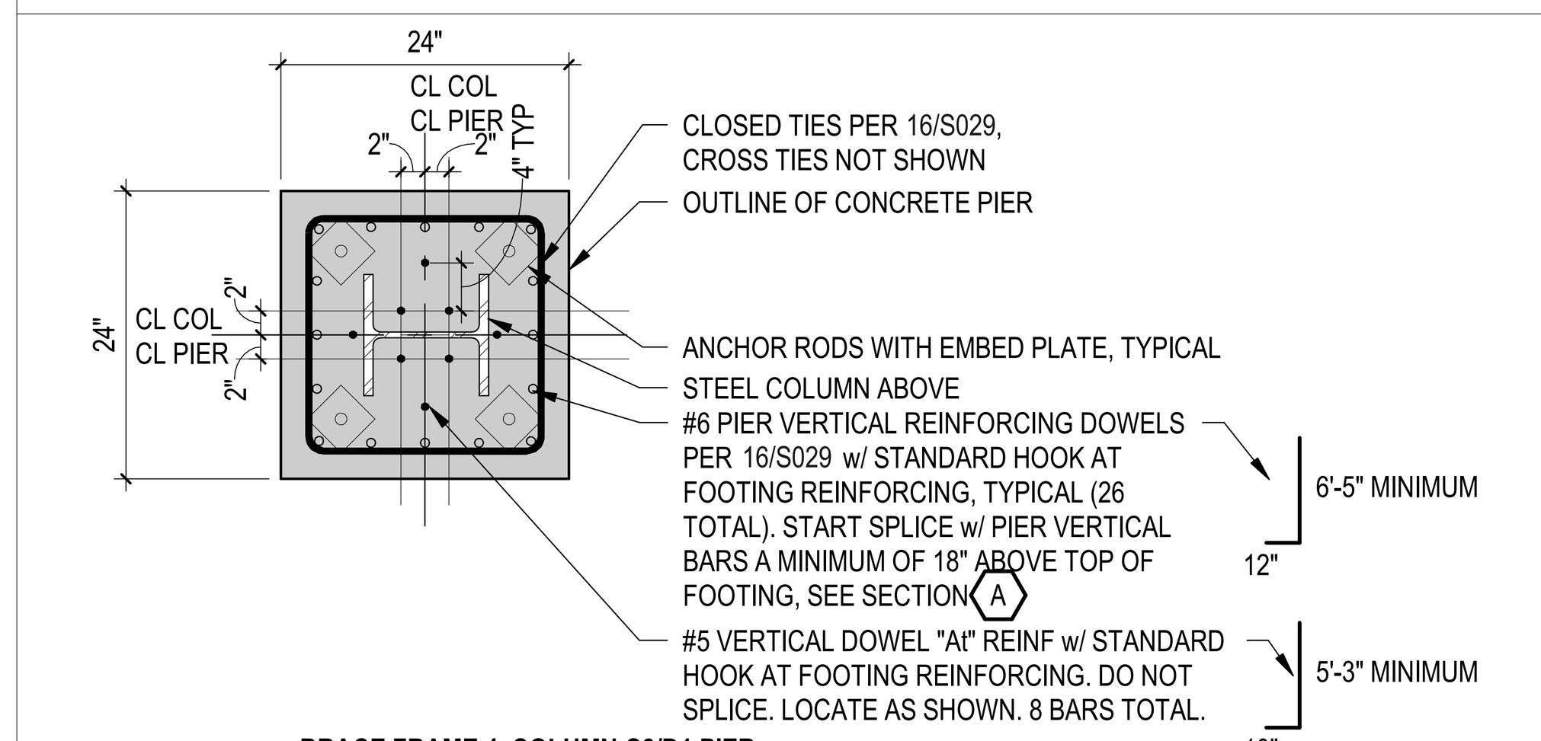
BRACE FRAME 4, COLUMN C10/D1 + C7/D1 PIER



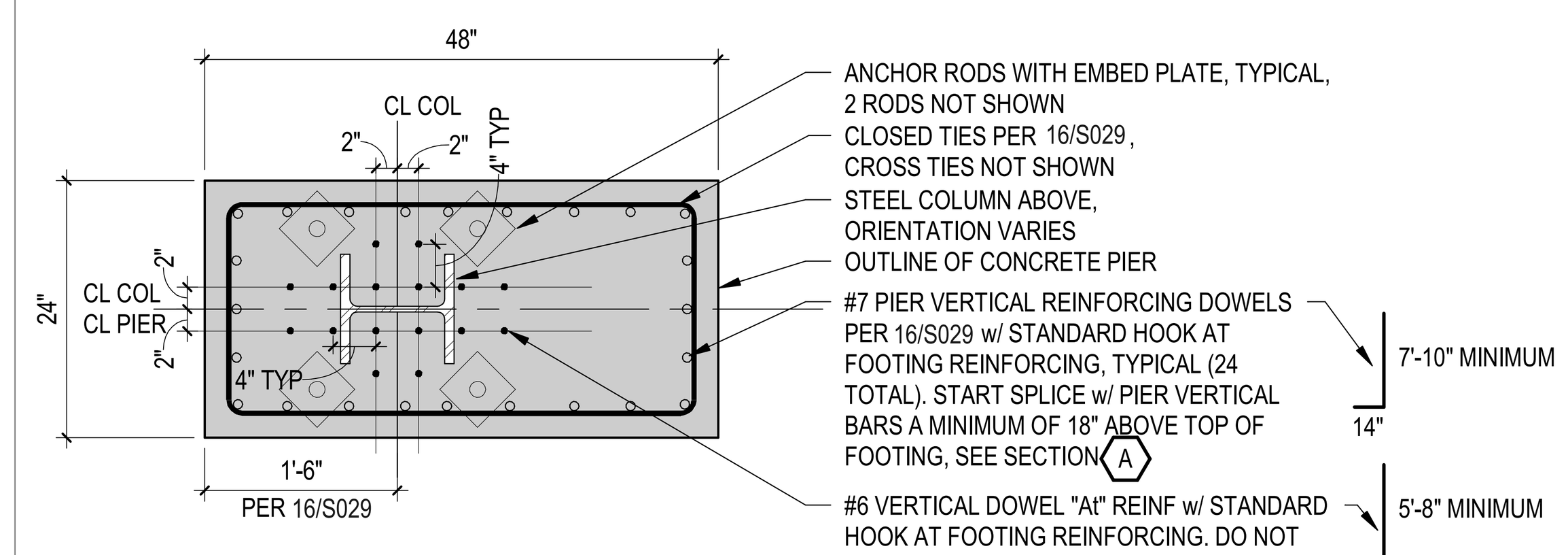
BRACE FRAME 6, COLUMN B3/A1 + B3/A4 PIER
BRACE FRAME 8, COLUMN B7/A3 PIER



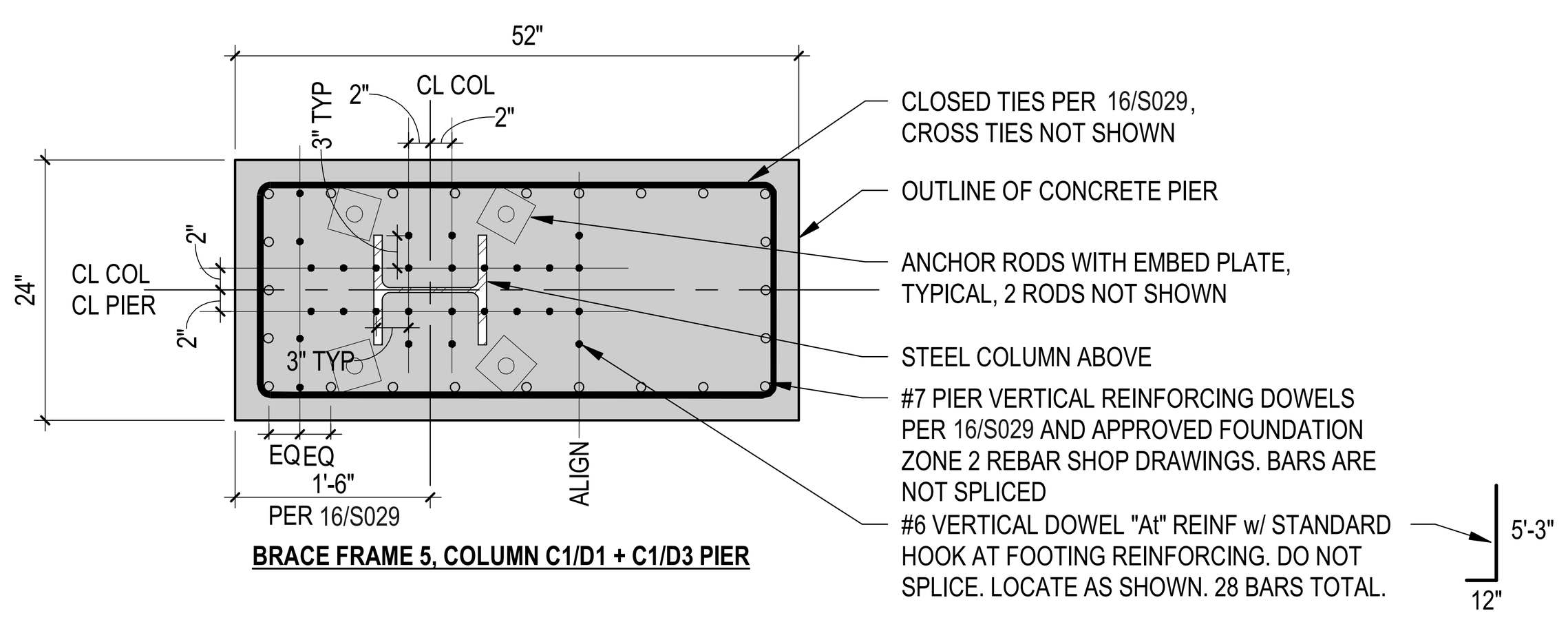
BRACE FRAME 9 + 10, COLUMN C7/D6 PIER



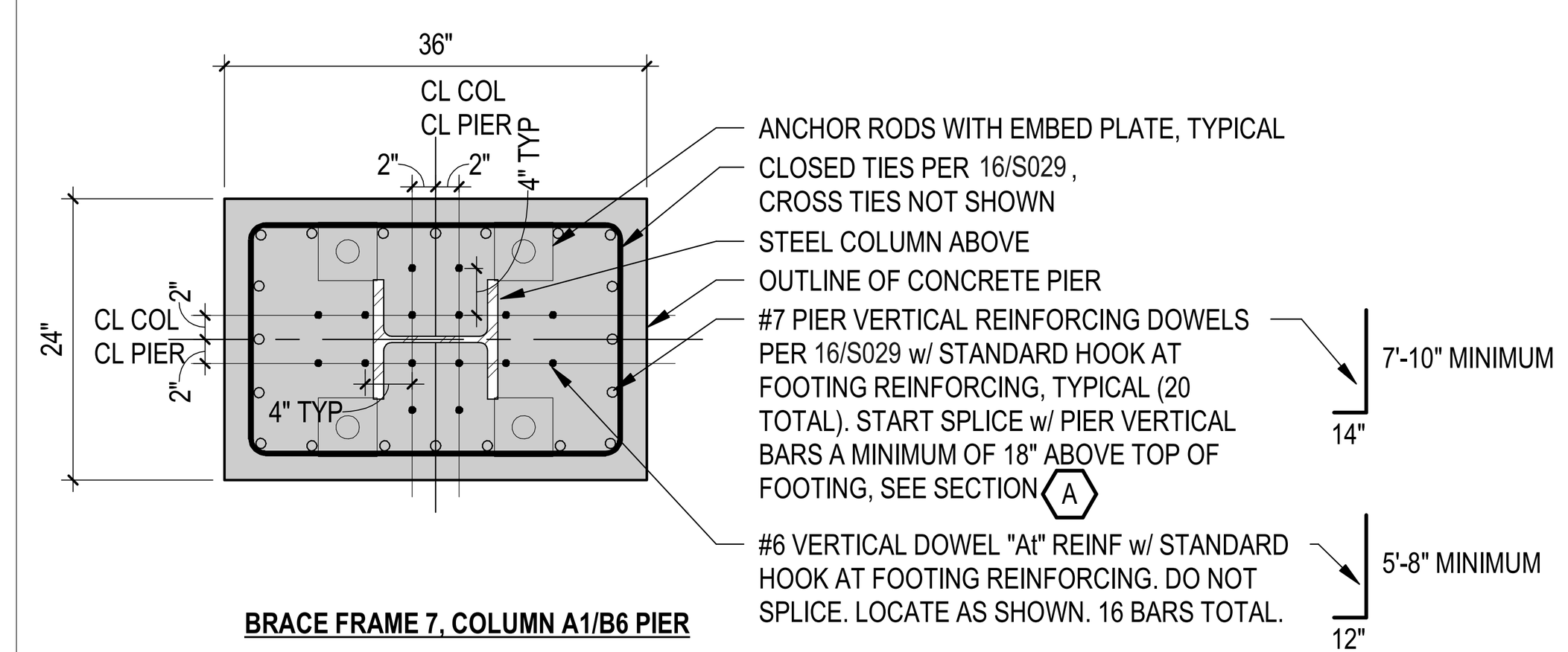
BRACE FRAME 4, COLUMN C8/D1 PIER



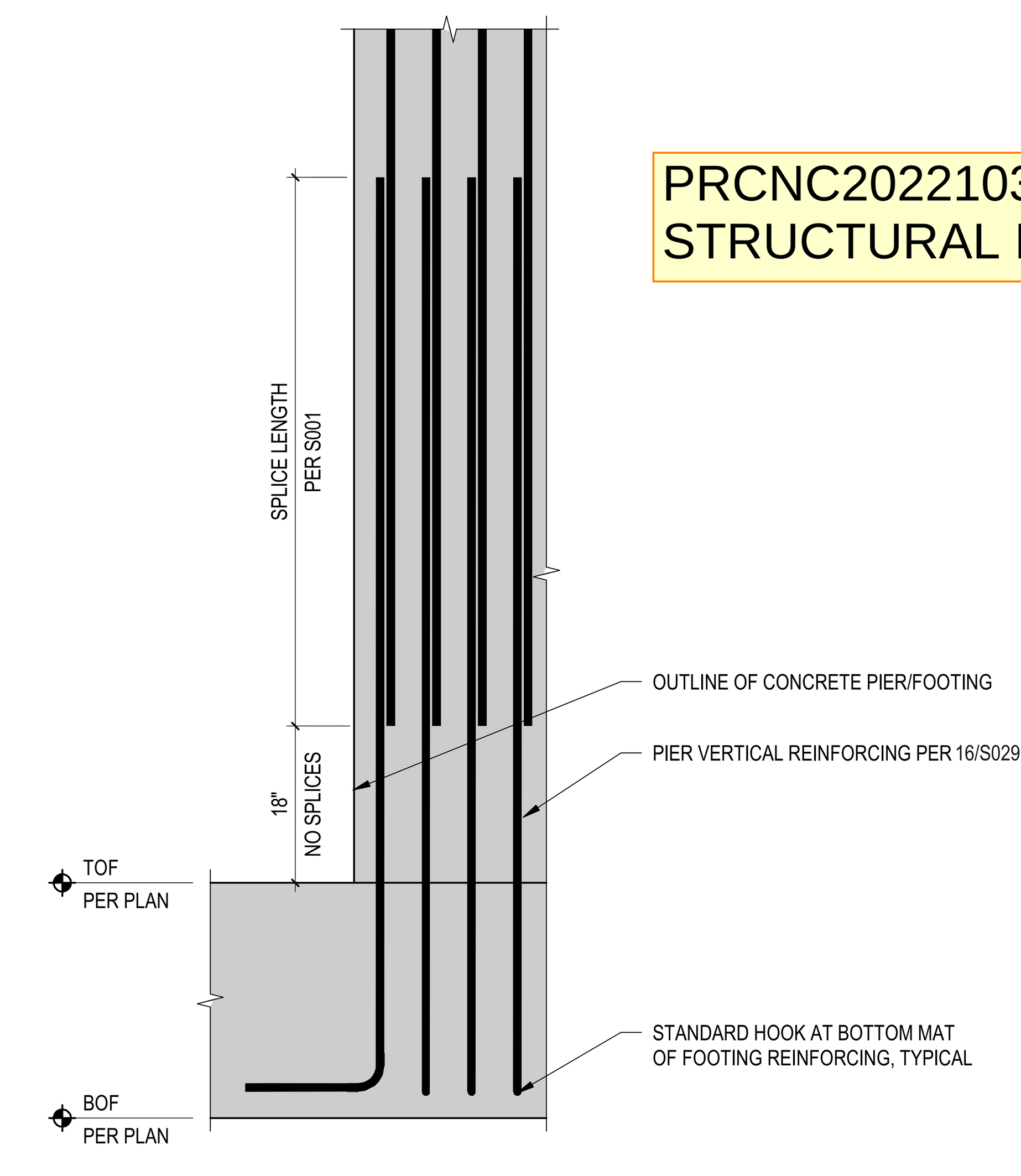
BRACE FRAME 7, COLUMN A1/B4 PIER
BRACE FRAME 8, COLUMN B7/A1 PIER



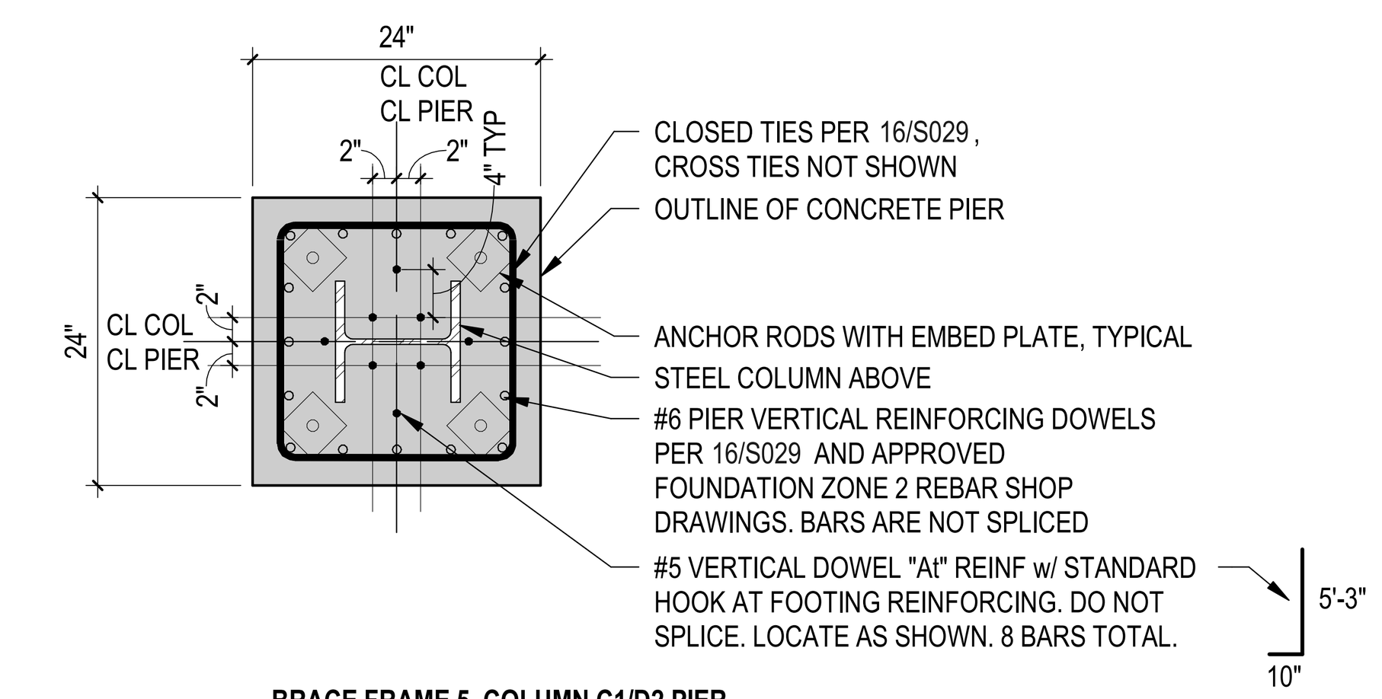
BRACE FRAME 5, COLUMN C1/D1 + C1/D3 PIER



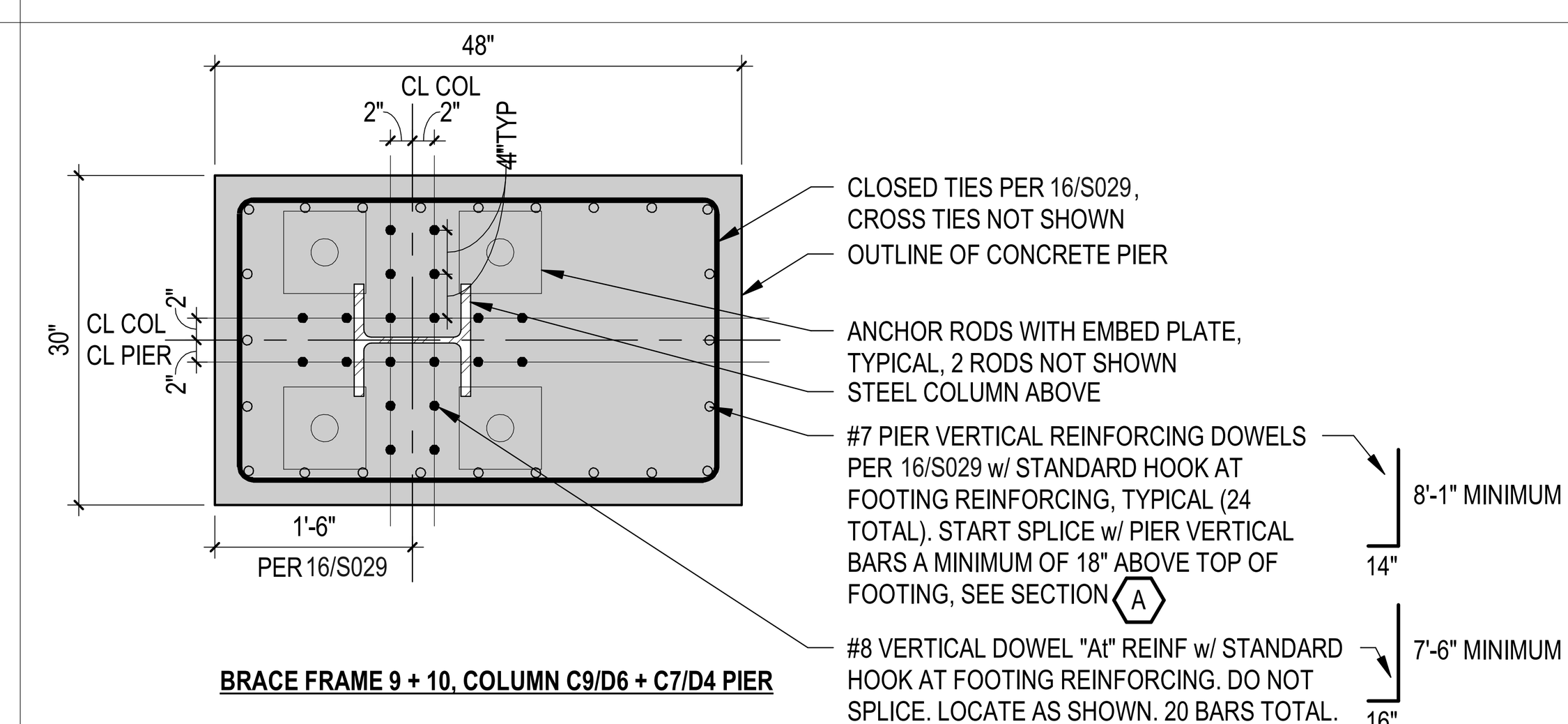
BRACE FRAME 7, COLUMN A1/B6 PIER



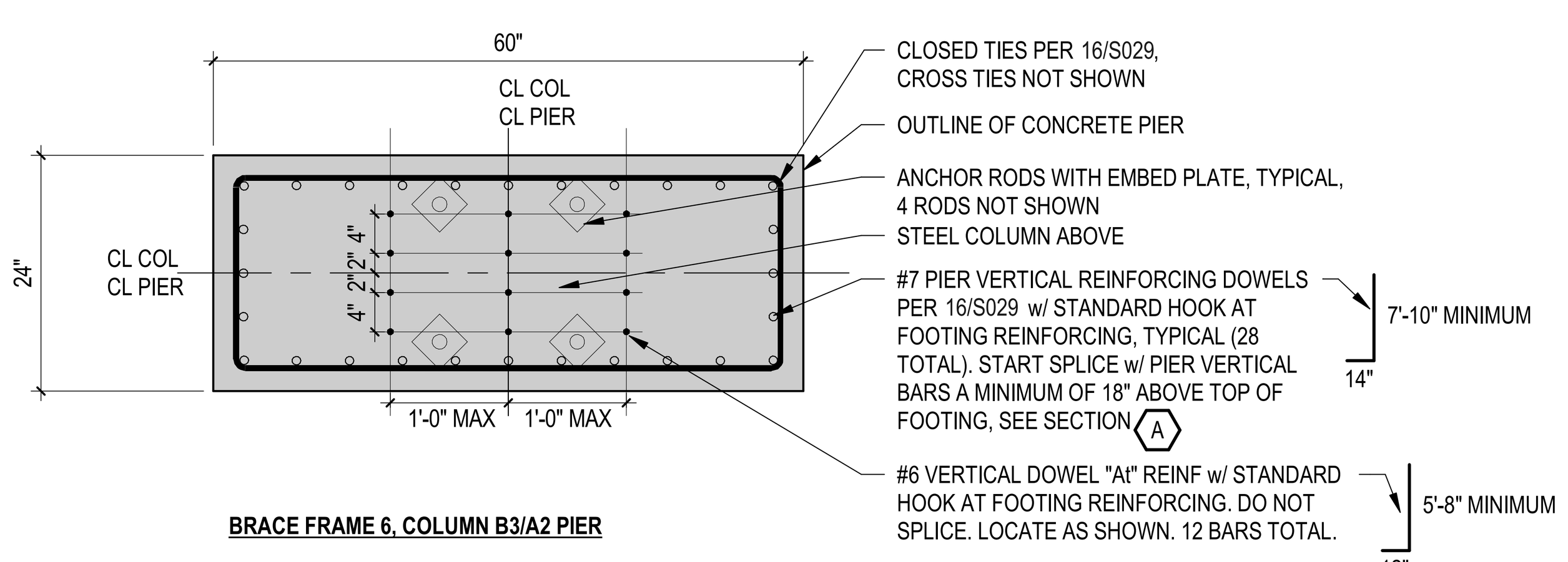
A SECTION: PIER VERTICAL DOWEL SPLICE LOCATION



BRACE FRAME 5, COLUMN C1/D2 PIER



BRACE FRAME 9 + 10, COLUMN C9/D6 + C7/D4 PIER



BRACE FRAME 6, COLUMN B3/A2 PIER

1 BRACED FRAME COLUMN CONCRETE PIER REINFORCING
SCALE: 3/4" = 1'-0"

PRCNC20221036
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City of Puyallup
Development & Permitting Services
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integrus ARCHITECTURE

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Revisions

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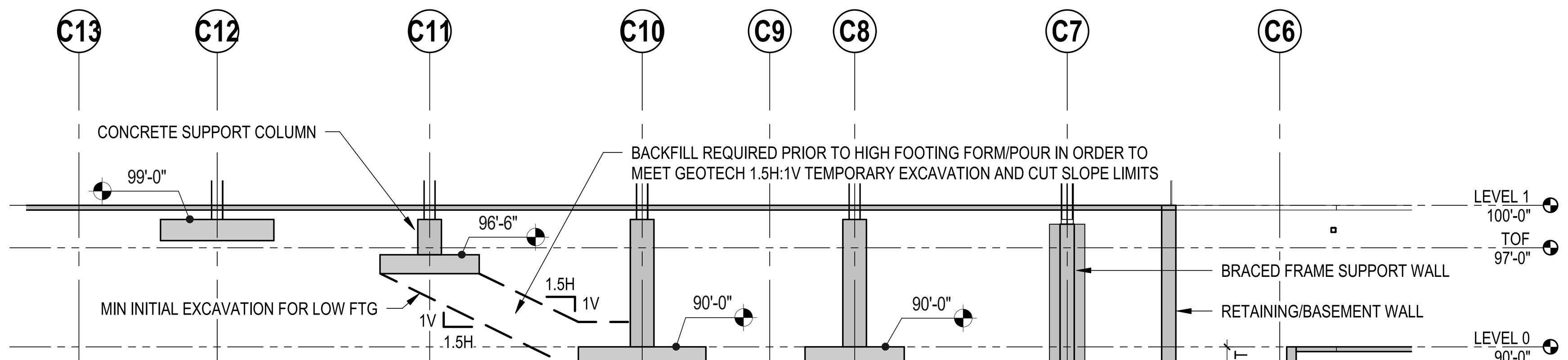
TYPICAL BUCKLING-RESTRAINED BRACED FRAME DETAILS

S031

IFC SET



NOTES: 1. SECTION PROVIDED TO AID CONTRACTOR IN SEQUENCING FOUNDATION POURS. FOOTING ELEVATIONS SET TO AID IN REDUCING FOOTING DEPTHS WHICH REQUIRES BACKFILL AS NOTED.
2. CONTRACTOR TO COORDINATE WITH GEOTECHNICAL ENGINEER FOR ADDITIONAL REQUIREMENTS. NOTIFY ENGINEER OF ANY CHANGES.

2 FOUNDATION SEQUENCING SECTION

SCALE: 1/8" = 1'-0"

KEY FOUNDATION/FIRST FLOOR NOTES

- INDICATES FOOTING MARK NUMBER. SEE SCHEDULE ON THIS SHEET FOR SIZE AND REINFORCEMENT
- INDICATES EXTENTS AND DEPTH OF RECESSED SLAB-ON-GRADE. CONFIRM DEPTH OF RECESS INDICATED WITH ARCHITECTURAL AND FLOOR FINISH MANUFACTURER WHERE NOT INDICATED ON PLAN. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES. SEE DETAIL 10/S021 .
- INDICATES MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT. PROVIDE HOUSEKEEPING PAD PER DETAIL 5/S021 IF REQUIRED BY EQUIPMENT MANUFACTURER. SEE MECHANICAL, ELECTRICAL, AND/OR PLUMBING FOR EXACT SIZE, QUANTITY, AND LOCATION OF PADS.
- S— — —S INDICATES FOOTING STEP LOCATION. SEE DETAIL 22/S021.
- INDICATES APPROXIMATE LOCATION OF PIPE OR CONDUIT THROUGH FOUNDATION WALL, THROUGH FOOTING, AND/OR BELOW FOOTING. SEE DETAIL 15/S021 . NOT ALL LOCATIONS ARE INDICATED. COORDINATE WITH MEP DRAWINGS FOR QUANTITIES, SIZES AND LOCATIONS.
- INDICATES FLOOR DRAIN. SLOPE 1/4" : 12" TOWARD CENTER OF FLOOR DRAIN OVER AN 18" RADIUS. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING.
- INDICATES FLOOR SINK WITH DRAIN. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING. SEE 28/S021 AT SLAB-ON-GRADE, SEE 27/S021 AT ELEVATED SLABS.
- INDICATES ELECTRICAL FLOOR BOX. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH, ELECTRICAL, AND LAB. SEE 27/S021 .

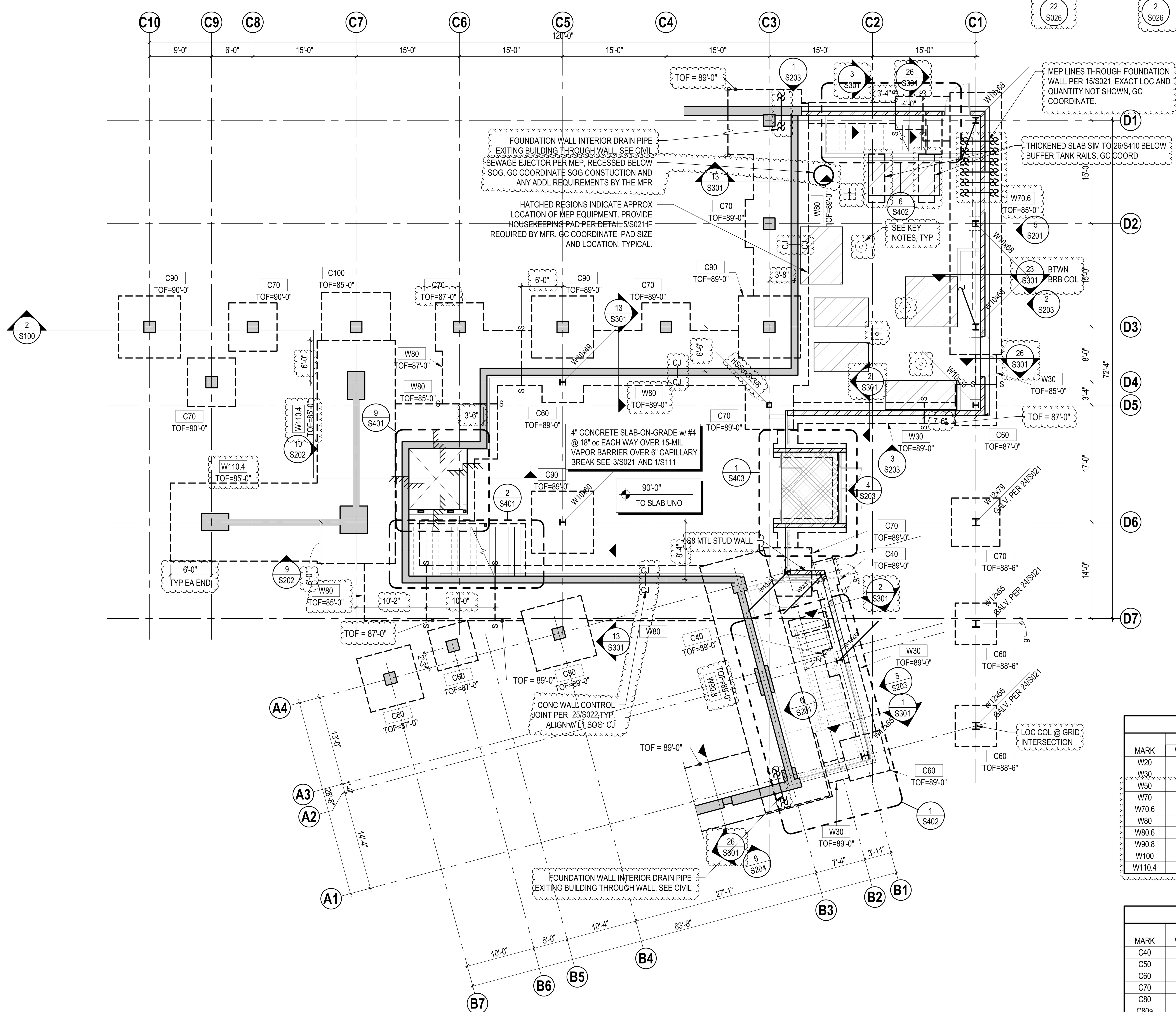
FOUNDATION/FIRST FLOOR PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS
5. REFER TO DETAILS 3/S021 AND EARTHWORK SPECIFICATION FOR SOIL PREPARATION AT SLABS-ON-GRADE
6. REFER TO DETAIL 10/S021 FOR RECESSED SLAB-ON-GRADE. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES.
7. REFER TO DETAIL 5/S021 FOR EQUIPMENT PADS. SEE MECH, ELEC, CIVIL FOR PAD QUANTITIES AND LOCATIONS. PAD SIZES TO BE PROVIDED BY EQUIPMENT MFRS
8. LOCALIZED SLOPE TO DRAIN. PROVIDE MAXIMUM 1/4": 12" SLOPE TO DRAIN WITHIN 18" RADIUS AROUND DRAIN.
9. CONTINUE ALL WALL FOOTING REINFORCING, INCLUDING WALL VERTICAL DOWELS, THROUGH COLUMN FOOTINGS WHEREVER THEY INTERSECT. CONTINUE WALL VERTICAL DOWELS THROUGH BRACE FRAME FOOTINGS WHEREVER THEY INTERSECT.
10. EXTEND LONGITUDINAL FOOTING REINFORCING INTO ADJACENT FOOTING WITH EXTENSION LENGTH EQUAL TO THE TYPICAL HORIZONTAL SPLICE LENGTH REQUIRED BY THE CONC REINF SPLICE TABLE ON SHEET S001.
11. APPLY COAL TAR EPOXY TO ALL STRUCTURAL STEEL SURFACES, INCLUDING ANCHOR BOLTS, EXPOSED TO SOIL IN THE FINISHED CONDITION.
12. BRACED FRAMES AND FOOTINGS ARE CENTERED ON GRID LINES, UNLESS NOTED OTHERWISE.
13. BRACED FRAME FOOTINGS SHALL EXTEND 4'-0" BEYOND CENTERLINE OF BRACED FRAME COLUMNS, TYPICAL UNLESS NOTED OTHERWISE IN PLAN.
14. SEE 1/S111 FOR SLAB-ON-GRADE CONTROL JOINT PLAN. REFERENCE DETAIL 4/S021.

STEEL COLUMNS AND POSTS

1. ALL COLUMNS BEAR AT TOP OF FOOTING OR TOP OF PIER. VERTICAL STEEL ELEMENTS SHOWN IN PLAN ARE 'COLUMNS', UNLESS NOTED OTHERWISE.
2. COLUMNS ARE CENTERED AT GRIDLINE INTERSECTIONS, UNLESS NOTED OTHERWISE.
3. ALL POST BEAR AT TOP OF SLAB OR STEM WALL. VERTICAL STEEL ELEMENTS ARE ONLY POSTS IF LABELED 'POST' IN PLAN.
4. SEE THE FOLLOWING DETAILS FOR ADDITIONAL INFORMATION:

COLUMN BASEPLATE: 22/S026
TYPICAL COLUMN CAP PLATE: 2/S026
TYPICAL INTERIOR STEEL COLUMN FOOTING, UNO: 24/S021
POST BASEPLATE, UNO: 13/S026



FOUNDATION / LEVEL 0 FLOOR PLAN

SCALE: 1/8" = 1'-0"

FOOTING SCHEDULE - WALL

MARK	SIZE		REINFORCING
	WIDTH	DEPTH	
W20	2'-0"	12"	(3) #4 LONG
W30	3'-0"	12"	(4) #5 LONG w/ #5 TRANS @ 12" oc
W50	5'-0"	24"	(6) #6 LONG w/ #5 STIRRUPS @ 12" oc
W70	7'-0"	36"	(7) #6 LONG T&B w/ #5 STIRRUPS @ 6" oc
W70.6	7'-6"	24"	(7) #7 w/ #5 STIRRUPS @ 6" oc
W80	8'-0"	16"	PER 13/S301
W80.6	8'-6"	24"	(8) #7 LONG T&B, SEE DETAIL FOR TRANSVERSE REINF
W90.8	9'-8"	24"	(10) #7 LONG T&B, SEE DETAIL FOR TRANSVERSE REINF
W100	10'-0"	24"	(10) #7 LONG T&B, SEE DETAIL FOR TRANSVERSE REINF
W110.4	11'-4"	27"	(17) #8 LONG T&B w/ #5 STIRRUPS @ 6" oc

FOOTING SCHEDULE - COLUMN

MARK	SIZE			REINFORCING (BOTTOM UNO)
	WIDTH	LENGTH	DEPTH	
C40	4'-0"	4'-0"	12"	(4) #5 EA WAY
C50	5'-0"	5'-0"	12"	(5) #5 EA WAY
C60	6'-0"	6'-0"	16"	(6) #6 EA WAY
C70	7'-0"	7'-0"	16"	(7) #6 EA WAY
C80	8'-0"	8'-0"	18"	(9) #6 EA WAY
C80a	8'-0"	8'-0"	24"	(9) #6 T&B, EA WAY
C90	9'-0"	9'-0"	20"	(11) #6 T&B, EA WAY
C100	10'-0"	10'-0"	20"	(13) #6 T&B, EA WAY

PRCNC20221036
STRUCTURAL REVISION

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City of Puyallup
Development & Permitting Services
ISSUED PERMIT
Building | Planning
Engineering | Public Works
Fire | Traffic

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date: 10/07/2022
Job No.: 22135.00
Drawn By: RB/AM
Checked by: TD

Revisions

Date	Description
1.30.23	IFC Revisions

FOUNDATION/ LEVEL 0 FLOOR PLAN

S100

IFC SET

BEAM END CONNECTIONS

1. BEAMS MARKED WITH (c) AND (c)* ARE SEISMIC COLLECTORS. THESE BEAMS ARE PART OF THE LATERAL FORCE RESISTING SYSTEM.

TYPICAL BEAM/GIRDER TO COLUMN: 5/S026

TYPICAL BEAM/GIRDER BOLTED CONNECTION: 25/S026

TYPICAL (c) CONNECTION AT COLUMNS: 7/S027

TYPICAL (c)* CONNECTION AT COLUMNS: 13/S027

TYPICAL (c) AND (c)* CONNECTION TO BEAM/GIRDER: 19/S027

TYPICAL SKEWED BEAM/GIRDER CONNECTION: 17/S027

TYPICAL COLUMN CAP PLATE, UNO: 2/S026

FLOOR FRAMING PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES.
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS.
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.
5. 3" CONCRETE TOPPING OVER 2" COMPOSITE 18 GAGE METAL DECK w/ 6x6 W2.9xW2.9 WELDED WIRE FABRIC REINFORCING, UNLESS NOTED OTHERWISE. SEE DETAILS 1/S025 AND 24/S025.
6. AT ALL COMPOSITE DECK, INSTALL HEADED STUDS AT 12" ON CENTER AT ALL SUPPORTS UNLESS NOTED OTHERWISE. SEE DETAIL 7/S025 .
7. REPETITIVE FRAMING IS SPACED EQUALLY BETWEEN DIMENSIONED ELEMENTS, WITH THE NUMBER OF EQUAL SPACES GRAPHICALLY INDICATED IN PLAN, UNLESS NOTED OTHERWISE.
8. BEAMS FRAMING INTO WALL ENDS AND COLUMNS ARE ALIGNED CENTERLINE TO CENTERLINE WITH THEIR SUPPORTS, UNLESS NOTED OTHERWISE.
9. COORDINATE SIZES AND LOCATIONS OF FLOOR AND ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. NOT ALL REQUIRED OPENINGS ARE INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO DETAIL 9/S025 FOR OPENINGS IN COMPOSITE FLOOR DECK. REFER TO DETAIL 3/S025 FOR OPENINGS IN ROOF DECK.
10. STRUCTURAL STEEL SHOP TAGS, ERECTION AIDS AND MILL MARKINGS EXPOSED TO VIEW SHALL BE REMOVED AFTER FIELD ASSEMBLY. PATCH, TOUCH UP AND GRIND SMOOTH.
11. HOT DIP GALVANIZE ALL STEEL, CONNECTIONS, AND BOLTS EXPOSED TO WEATHER. PAINT EXPOSED FIELD WELDS WITH ZINC RICH PAINT AFTER COMPLETION.
12. STEEL MEMBERS DESIGNATED AS (c) OR (c)* ARE CHORDS/COLLECTORS AND PART OF OF THE LATERAL FORCE RESISTING SYSTEM.
13. AT (c) REINFORCING BARS SHOWN IN COMPOSITE FLOOR/ROOF DECK, STAGGER BAR SPLICES.
14. AT ALL EXTERIOR BEAMS, BEAMS FRAMING INTO (c) OR (c)*, AND BEAMS FRAMING INTO BRACE FRAME BEAMS, PROVIDE FULL HEIGHT SHEAR TABS AS INDICATED IN DETAIL 25/S026 .
15. AT CANTILEVERS DESIGNATED AS "CANT" ON PLAN, CANTILEVER MEMBER SIZE SHALL MATCH BACKSPAN MEMBER SIZE UNLESS NOTED OTHERWISE.
16. LOCALIZED SLOPE TO DRAIN. PROVIDE MAXIMUM 1/4" : 12" SLOPE TO DRAIN WITHIN 18" RADIUS AROUND DRAIN
17. REFER TO SHEET S501 FOR LOAD MAPS.
18. COLUMNS NOT CONTINUOUS TO LEVEL ABOVE SHALL TERMANITE AT BOTTOM OF SLAB. PROVIDE COLUMN CAP PLATE PER DETAIL 2/S026 UNLESS NOTED OTHERWISE.

KEY FOUNDATION/FIRST FLOOR NOTES

- INDICATES FOOTING MARK NUMBER. SEE SCHEDULE ON THIS SHEET FOR SIZE AND REINFORCEMENT
- INDICATES EXTENTS AND DEPTH OF RECESSED SLAB-ON-GRADE. CONFIRM DEPTH OF RECESS INDICATED WITH ARCHITECTURAL AND FLOOR FINISH MANUFACTURER WHERE NOT INDICATED ON PLAN. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES. SEE DETAIL 10/S021 .
- INDICATES MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT. PROVIDE HOUSEKEEPING PAD PER DETAIL 5/S021 IF REQUIRED BY EQUIPMENT MANUFACTURER. SEE MECHANICAL, ELECTRICAL, AND/OR PLUMBING FOR EXACT SIZE, QUANTITY, AND LOCATION OF PADS.
- S— — —S INDICATES FOOTING STEP LOCATION. SEE DETAIL 22/S021.
- INDICATES APPROXIMATE LOCATION OF PIPE OR CONDUIT THROUGH FOUNDATION WALL, THROUGH FOOTING, AND/OR BELOW FOOTING. SEE DETAIL 15/S021 . NOT ALL LOCATIONS ARE INDICATED. COORDINATE WITH MEP DRAWINGS FOR QUANTITIES, SIZES AND LOCATIONS.
- INDICATES FLOOR DRAIN. SLOPE 1/4" : 12" TOWARD CENTER OF FLOOR DRAIN OVER AN 18" RADIUS. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING.
- INDICATES FLOOR SINK WITH DRAIN. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING. SEE 28/S021 AT SLAB-ON-GRADE, SEE 27/S021 AT ELEVATED SLABS.
- INDICATES ELECTRICAL FLOOR BOX. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH, ELECTRICAL, AND LAB. SEE 27/S021 .

FOUNDATION/FIRST FLOOR PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS
5. REFER TO DETAILS 3/S021 AND EARTHWORK SPECIFICATION FOR SOIL PREPARATION AT SLABS-ON-GRADE
6. REFER TO DETAIL 10/S021 FOR RECESSED SLAB-ON-GRADE. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES.
7. REFER TO DETAIL 5/S021 FOR EQUIPMENT PADS. SEE MECH, ELEC, CIVIL FOR PAD QUANTITIES AND LOCATIONS. PAD SIZES TO BE PROVIDED BY EQUIPMENT MFRS
8. LOCALIZED SLOPE TO DRAIN. PROVIDE MAXIMUM 1/4": 12" SLOPE TO DRAIN WITHIN 18" RADIUS AROUND DRAIN.
9. CONTINUE ALL WALL FOOTING REINFORCING, INCLUDING WALL VERTICAL DOWELS, THROUGH COLUMN FOOTINGS WHEREVER THEY INTERSECT. CONTINUE WALL VERTICAL DOWELS THROUGH BRACE FRAME FOOTINGS WHEREVER THEY INTERSECT.
10. EXTEND LONGITUDINAL FOOTING REINFORCING INTO ADJACENT FOOTING WITH EXTENSION LENGTH EQUAL TO THE TYPICAL HORIZONTAL SPLICE LENGTH REQUIRED BY THE CONC REINF SPLICE TABLE ON SHEET S001.
11. APPLY COAL TAR EPOXY TO ALL STRUCTURAL STEEL SURFACES, INCLUDING ANCHOR BOLTS, EXPOSED TO SOIL IN THE FINISHED CONDITION.
12. BRACED FRAMES AND FOOTINGS ARE CENTERED ON GRID LINES, UNLESS NOTED OTHERWISE.
13. BRACED FRAME FOOTINGS SHALL EXTEND 4'-0" BEYOND CENTERLINE OF BRACED FRAME COLUMNS, TYPICAL UNLESS NOTED OTHERWISE IN PLAN.
14. SEE 1/S111 FOR SLAB-ON-GRADE CONTROL JOINT PLAN. REFERENCE DETAIL 4/S021.

STEEL COLUMNS AND POSTS

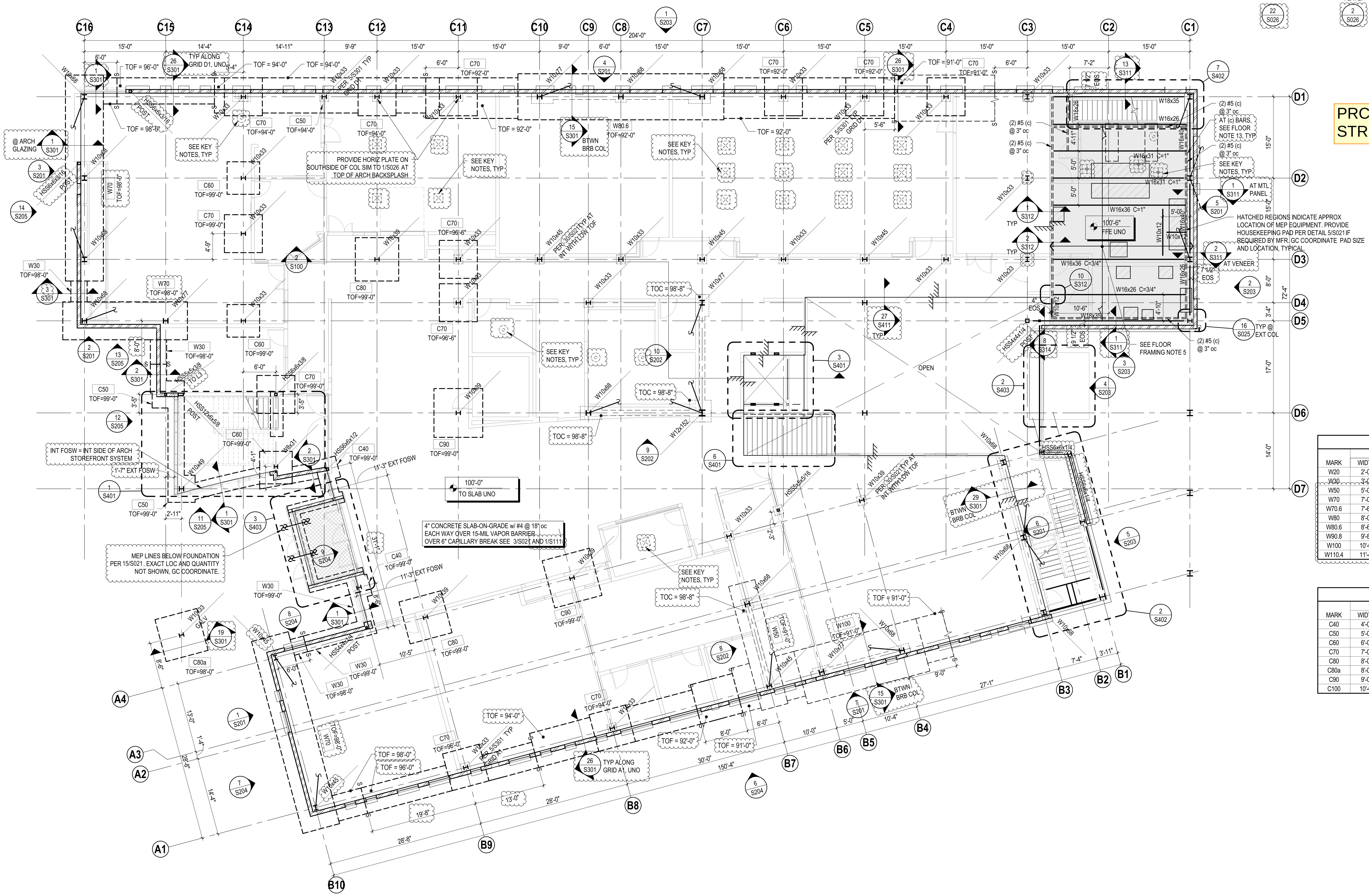
1. ALL COLUMNS BEAR AT TOP OF FOOTING OR TOP OF PIER. VERTICAL STEEL ELEMENTS SHOWN IN PLAN ARE 'COLUMNS', UNLESS NOTED OTHERWISE.
2. COLUMNS ARE CENTERED AT GRIDLINE INTERSECTIONS, UNLESS NOTED OTHERWISE.
3. ALL POST BEAR AT TOP OF SLAB OR STEM WALL. VERTICAL STEEL ELEMENTS ARE ONLY POSTS IF LABELED 'POST' IN PLAN.
4. SEE THE FOLLOWING DETAILS FOR ADDITIONAL INFORMATION:

COLUMN BASEPLATE: 22/S026

TYPICAL COLUMN CAP PLATE: 2/S026

TYPICAL INTERIOR STEEL COLUMN FOOTING, UNO: 24/S021

POST BASEPLATE, UNO: 13/S026



PRCNC20221036
STRUCTURAL REVISION

FOUNDATION / LEVEL 1 FLOOR PLAN

SCALE: 1/8" = 1'-0"

FOOTING SCHEDULE - WALL			
MARK	SIZE		REINFORCING
	WIDTH	DEPTH	
W20	2'-0"	12"	(3) #4 LONG
~~W30~~	~~3'-0"~~	~~12"~~	~~(4) #5 LONG w/ #5 TRANS @ 12" oc~~
W50	5'-0"	24"	(6) #6 LONG w/ #5 STIRRUPS @ 12" oc
W70	7'-0"	36"	(7) #6 LONG T&B w/ #5 STIRRUPS @ 6" oc
W70.6	7'-6"	24"	(7) #7 w/ #5 STIRRUPS @ 6" oc
W80	8'-0"	16"	PER 13/S301
W80.6	8'-6"	24"	(8) #7 LONG T&B, SEE DETAIL FOR TRANSVERSE REINF
W90.8	9'-8"	24"	(10) #7 LONG T&B, SEE DETAIL FOR TRANSVERSE REINF
W100	10'-0"	24"	(10) #7 LONG T&B, SEE DETAIL FOR TRANSVERSE REINF
W110.4	11'-4"	27"	(17) #8 LONG T&B w/ #5 STIRRUPS @ 6" oc

FOOTING SCHEDULE - COLUMN				
MARK	SIZE			REINFORCING (BOTTOM UNO)
	WIDTH	LENGTH	DEPTH	
C40	4'-0"	4'-0"	12"	(4) #5 EA WAY
C50	5'-0"	5'-0"	12"	(5) #5 EA WAY
C60	6'-0"	6'-0"	16"	(6) #6 EA WAY
C70	7'-0"	7'-0"	16"	(7) #6 EA WAY
C80	8'-0"	8'-0"	18"	(9) #6 EA WAY
C80a	8'-0"	8'-0"	24"	(9) #6 T&B, EA WAY
C90	9'-0"	9'-0"	20"	(11) #6 T&B, EA WAY
C100	10'-0"	10'-0"	20"	(13) #6 T&B, EA WAY

integrus ARCHITECTURE

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM/BLO
Checked by:	TD

Revisions

Date	Description
1.30.23	IFC Revisions

FOUNDATION/ LEVEL 1 FLOOR PLAN

S101

IFC SET

BEAM END CONNECTIONS

1. BEAMS MARKED WITH (c) AND (c)* ARE SEISMIC COLLECTORS. THESE BEAMS ARE PART OF THE LATERAL FORCE RESISTING SYSTEM.

TYPICAL BEAM/GIRDER TO COLUMN: 5/S026

TYPICAL BEAM/GIRDER BOLTED CONNECTION: 25/S026

TYPICAL (c) CONNECTION AT COLUMNS: 7/S027

TYPICAL (c)* CONNECTION AT COLUMNS: 13/S027

TYPICAL (c) AND (c)* CONNECTION TO BEAM/GIRDER: 19/S027

TYPICAL SKEWED BEAM/GIRDER CONNECTION: 17/S027

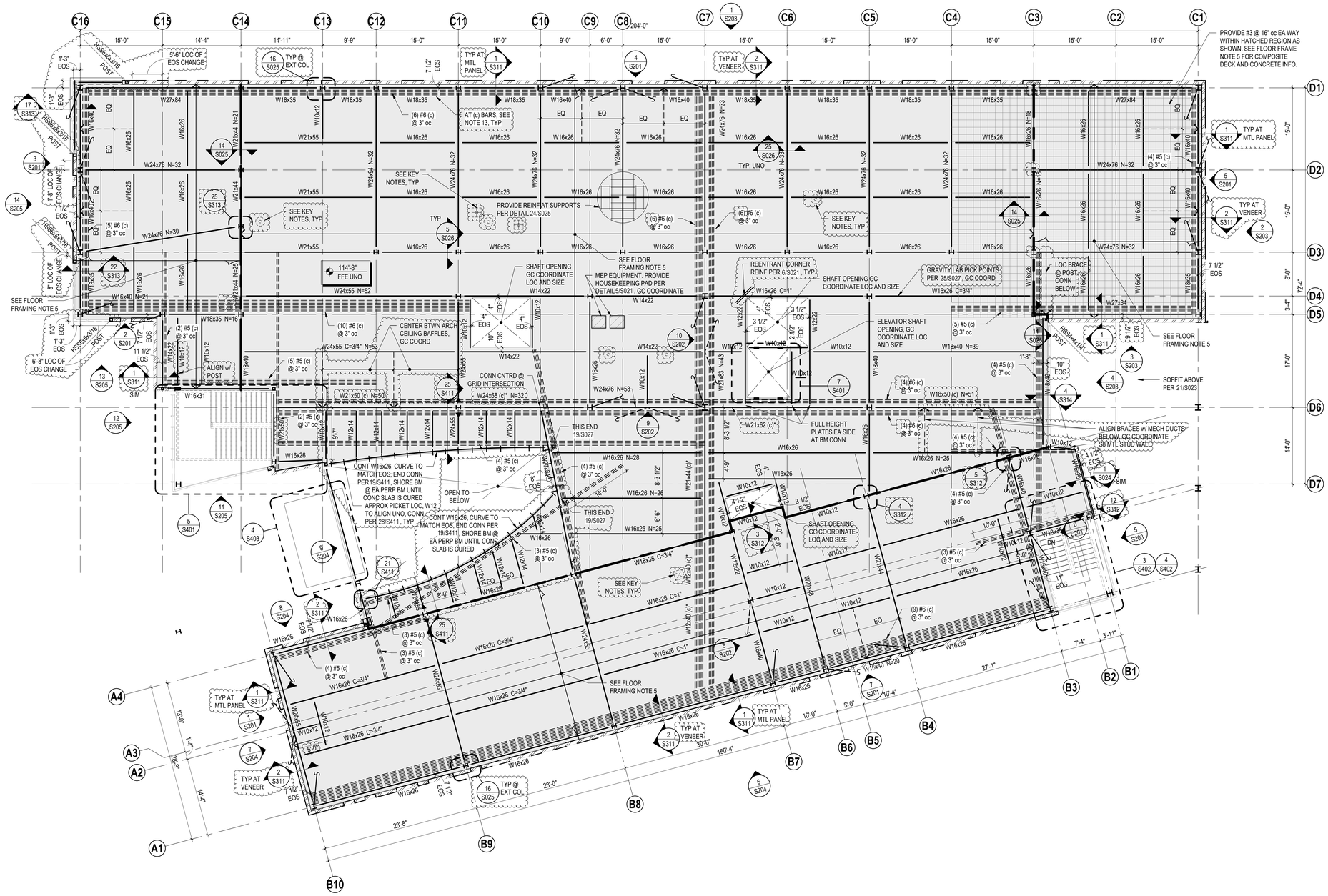
TYPICAL COLUMN CAP PLATE, UNO: 2/S026

KEY FRAMING NOTES

▩	INDICATES EXTENTS AND DEPTH OF RECESSED SLAB-ON-GRADE. CONFIRM DEPTH OF RECESS INDICATED WITH ARCHITECTURAL AND FLOOR FINISH MANUFACTURER WHERE NOT INDICATED ON PLAN. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES. SEE DETAIL 10/S021 .
▨	INDICATES MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT. PROVIDE HOUSEKEEPING PAD PER DETAIL 5/S021 IF REQUIRED BY EQUIPMENT MANUFACTURER. SEE MECHANICAL, ELECTRICAL, AND/OR PLUMBING FOR EXACT SIZE, QUANTITY, AND LOCATION OF PANS.
◌	INDICATES FLOOR DRAIN. SLOPE 1/4" : 12" TOWARD CENTER OF FLOOR DRAIN OVER AN 18" RADIUS. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING.
⊕	INDICATES FLOOR SINK WITH DRAIN. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING. SEE 27/S021 .
⊠	INDICATES ELECTRICAL FLOOR BOX. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH, ELECTRICAL, AND LAB. SEE 27/S021 .
▬ ▬ ▬	DESIGNATES CHANGE IN FLOOR OR ROOF DECK DIRECTION, SEE PLAN FOR SPAN DIRECTION
← – – – –	DENOTES BOTTOM FLANGE BRACING. REFERENCE DETAILS 1/S027, OR 2/S027 .

FLOOR FRAMING PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES.
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS.
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.
5. 3" CONCRETE TOPPING OVER 2" COMPOSITE 18 GAGE METAL DECK w/ 6x6 W2.9xW2.9 WELDED WIRE FABRIC REINFORCING, UNLESS NOTED OTHERWISE. SEE DETAILS 1/S025 AND 24/S025.
6. AT ALL COMPOSITE DECK, INSTALL HEADED STUDS AT 12" ON CENTER AT ALL SUPPORTS UNLESS NOTED OTHERWISE. SEE DETAIL 7/S025 .
7. REPETITIVE FRAMING IS SPACED EQUALLY BETWEEN DIMENSIONED ELEMENTS, WITH THE NUMBER OF EQUAL SPACES GRAPHICALLY INDICATED IN PLAN, UNLESS NOTED OTHERWISE.
8. BEAMS FRAMING INTO WALL ENDS AND COLUMNS ARE ALIGNED CENTERLINE TO CENTERLINE WITH THEIR SUPPORTS, UNLESS NOTED OTHERWISE.
9. COORDINATE SIZES AND LOCATIONS OF FLOOR AND ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. NOT ALL REQUIRED OPENINGS ARE INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO DETAIL 9/S025 FOR OPENINGS IN COMPOSITE FLOOR DECK. REFER TO DETAIL 3/S025 FOR OPENINGS IN ROOF DECK.
10. STRUCTURAL STEEL SHOP TAGS, ERECTION AIDS AND MILL MARKINGS EXPOSED TO VIEW SHALL BE REMOVED AFTER FIELD ASSEMBLY. PATCH, TOUCH UP AND GRIND SMOOTH.
11. HOT DIP GALVANIZE ALL STEEL, CONNECTIONS, AND BOLTS EXPOSED TO WEATHER. PAINT EXPOSED FIELD WELDS WITH ZINC RICH PAINT AFTER COMPLETION.
12. STEEL MEMBERS DESIGNATED AS (c) OR (c)* ARE CHORDS/COLLECTORS AND PART OF OF THE LATERAL FORCE RESISTING SYSTEM.
13. AT (c) REINFORCING BARS SHOWN IN COMPOSITE FLOOR/ROOF DECK, STAGGER BAR SPLICES.
14. AT ALL EXTERIOR BEAMS, BEAMS FRAMING INTO (c) OR (c)*, AND BEAMS FRAMING INTO BRACE FRAME BEAMS, PROVIDE FULL HEIGHT SHEAR TABS AS INDICATED IN DETAIL 25/S026.
15. AT CANTILEVERS DESIGNATED AS "CANT" ON PLAN, CANTILEVER MEMBER SIZE SHALL MATCH BACKSPAN MEMBER SIZE UNLESS NOTED OTHERWISE.
16. LOCALIZED SLOPE TO DRAIN. PROVIDE MAXIMUM 1/4" : 12" SLOPE TO DRAIN WITHIN 18" RADIUS AROUND DRAIN
17. REFER TO SHEET S501 FOR LOAD MAPS.
18. COLUMNS NOT CONTINUOUS TO LEVEL ABOVE SHALL TERMANITE AT BOTTOM OF SLAB. PROVIDE COLUMN CAP PLATE PER DETAIL 2/S026 UNLESS NOTED OTHERWISE.



LEVEL 2 - FRAMING PLAN

SCALE: 1/8" = 1'-0"

PRCNC20221036
STRUCTURAL REVISION

integrus
ARCHITECTURE
117 SOUTH MAIN STREET, SUITE 100, SEATTLE, WA. 98104
TELEPHONE: (206)628-3137 FAX: (206)628-3138

THERESA H. DANIEL
38621

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

Revisions

Date	Description
1.30.23	IFC Revisions

LEVEL 2 FRAMING PLAN

S102

IFC SET

BEAM END CONNECTIONS

1. BEAMS MARKED WITH (c) AND (c)* ARE SEISMIC COLLECTORS. THESE BEAMS ARE PART OF THE LATERAL FORCE RESISTING SYSTEM.

TYPICAL BEAM/GIRDER TO COLUMN: 5/S026

TYPICAL BEAM/GIRDER BOLTED CONNECTION: 25/S026

TYPICAL (c) CONNECTION AT COLUMNS: 7/S027

TYPICAL (c)* CONNECTION AT COLUMNS: 13/S027

TYPICAL (c) AND (c)* CONNECTION TO BEAM/GIRDER: 19/S027

TYPICAL SKEWED BEAM/GIRDER CONNECTION: 17/S027

TYPICAL COLUMN CAP PLATE, UNO: 2/S026

KEY FRAMING NOTES

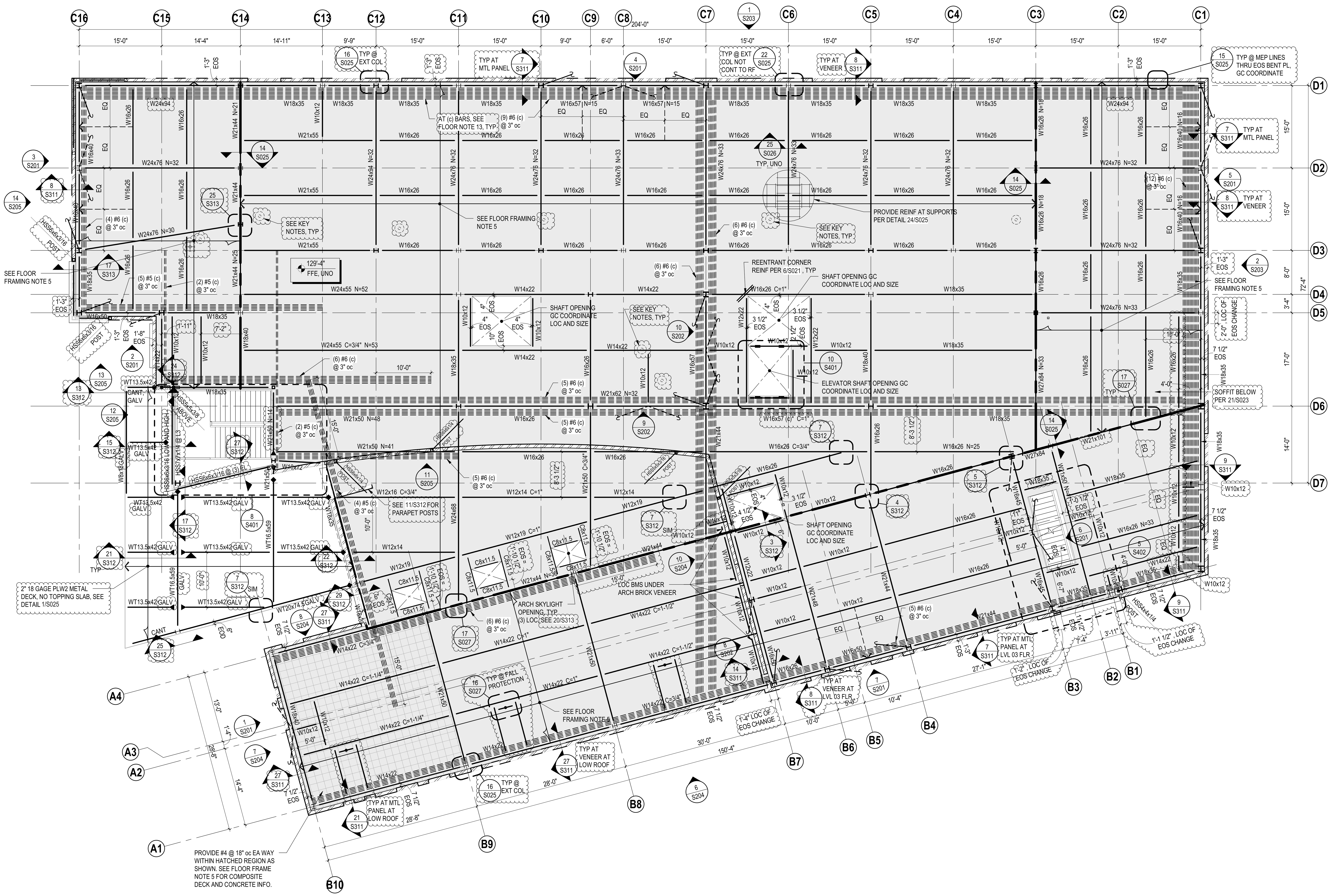
- INDICATES EXTENTS AND DEPTH OF RECESSED SLAB-ON-GRADE. CONFIRM DEPTH OF RECESS INDICATED WITH ARCHITECTURAL AND FLOOR FINISH MANUFACTURER WHERE NOT INDICATED ON PLAN. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES. SEE DETAIL 10/S021 .
- INDICATES MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT. PROVIDE HOUSEKEEPING PAD PER DETAIL 5/S021 IF REQUIRED BY EQUIPMENT MANUFACTURER. SEE MECHANICAL, ELECTRICAL, AND/OR PLUMBING FOR EXACT SIZE, QUANTITY, AND LOCATION OF PANS.
- INDICATES FLOOR DRAIN. SLOPE 1/4" : 12" TOWARD CENTER OF FLOOR DRAIN OVER AN 18" RADIUS. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING.
- INDICATES FLOOR SINK WITH DRAIN. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING. SEE 27/S021 .
- INDICATES ELECTRICAL FLOOR BOX. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH, ELECTRICAL, AND LAB. SEE 27/S021 .
- DESIGNATES CHANGE IN FLOOR OR ROOF DECK DIRECTION, SEE PLAN FOR SPAN DIRECTION
- DENOTES BOTTOM FLANGE BRACING. REFERENCE DETAILS 1/S027, OR 2/S027 .

FLOOR FRAMING PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES.
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS.
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.
5. 3" CONCRETE TOPPING OVER 2" COMPOSITE 18 GAGE METAL DECK w/ 6x6 W2.9xW2.9 WELDED WIRE FABRIC REINFORCING, UNLESS NOTED OTHERWISE. SEE DETAILS 1/S025 AND 24/S025.
6. AT ALL COMPOSITE DECK, INSTALL HEADED STUDS AT 12" ON CENTER AT ALL SUPPORTS UNLESS NOTED OTHERWISE. SEE DETAIL 7/S025 .
7. REPETITIVE FRAMING IS SPACED EQUALLY BETWEEN DIMENSIONED ELEMENTS, WITH THE NUMBER OF EQUAL SPACES GRAPHICALLY INDICATED IN PLAN, UNLESS NOTED OTHERWISE.
8. BEAMS FRAMING INTO WALL ENDS AND COLUMNS ARE ALIGNED CENTERLINE TO CENTERLINE WITH THEIR SUPPORTS, UNLESS NOTED OTHERWISE.
9. COORDINATE SIZES AND LOCATIONS OF FLOOR AND ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. NOT ALL REQUIRED OPENINGS ARE INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO DETAIL 9/S025 FOR OPENINGS IN COMPOSITE FLOOR DECK. REFER TO DETAIL 3/S025 FOR OPENINGS IN ROOF DECK.
10. STRUCTURAL STEEL SHOP TAGS, ERECTION AIDS AND MILL MARKINGS EXPOSED TO VIEW SHALL BE REMOVED AFTER FIELD ASSEMBLY. PATCH, TOUCH UP AND GRIND SMOOTH.
11. HOT DIP GALVANIZE ALL STEEL, CONNECTIONS, AND BOLTS EXPOSED TO WEATHER. PAINT EXPOSED FIELD WELDS WITH ZINC RICH PAINT AFTER COMPLETION.
12. STEEL MEMBERS DESIGNATED AS (c) OR (c)* ARE CHORDS/COLLECTORS AND PART OF OF THE LATERAL FORCE RESISTING SYSTEM.
13. AT (c) REINFORCING BARS SHOWN IN COMPOSITE FLOOR/ROOF DECK, STAGGER BAR SPLICES.
14. AT ALL EXTERIOR BEAMS, BEAMS FRAMING INTO (c) OR (c)*, AND BEAMS FRAMING INTO BRACE FRAME BEAMS, PROVIDE FULL HEIGHT SHEAR TABS AS INDICATED IN DETAIL 25/S026 .
15. AT CANTILEVERS DESIGNATED AS "CANT" ON PLAN, CANTILEVER MEMBER SIZE SHALL MATCH BACKSPAN MEMBER SIZE UNLESS NOTED OTHERWISE.
16. LOCALIZED SLOPE TO DRAIN. PROVIDE MAXIMUM 1/4" : 12" SLOPE TO DRAIN WITHIN 18" RADIUS AROUND DRAIN
17. REFER TO SHEET S501 FOR LOAD MAPS.
18. COLUMNS NOT CONTINUOUS TO LEVEL ABOVE SHALL TERMANITE AT BOTTOM OF SLAB. PROVIDE COLUMN CAP PLATE PER DETAIL 2/S026 UNLESS NOTED OTHERWISE.

ROOF FRAMING PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES.
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS.
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.
5. AT ALL COMPOSITE DECK, INSTALL HEADED STUDS AT 12" ON CENTER AT ALL SUPPORTS UNLESS NOTED OTHERWISE. SEE DETAIL 7/S025 .
6. REPETITIVE FRAMING IS SPACED EQUALLY BETWEEN DIMENSIONED ELEMENTS, WITH THE NUMBER OF EQUAL SPACES GRAPHICALLY INDICATED IN PLAN, UNLESS NOTED OTHERWISE.
7. BEAMS FRAMING INTO WALL ENDS AND COLUMNS ARE ALIGNED CENTERLINE TO CENTERLINE WITH THEIR SUPPORTS, UNLESS NOTED OTHERWISE.
8. COORDINATE SIZES AND LOCATIONS OF ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. NOT ALL REQUIRED OPENINGS ARE INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO DETAIL 3/S025 FOR OPENINGS IN ROOF DECK.
9. STRUCTURAL STEEL SHOP TAGS, ERECTION AIDS AND MILL MARKINGS EXPOSED TO VIEW SHALL BE REMOVED AFTER FIELD ASSEMBLY. PATCH, TOUCH UP AND GRIND SMOOTH.
10. HOT DIP GALVANIZE ALL STEEL, CONNECTIONS, AND BOLTS EXPOSED TO WEATHER. PAINT EXPOSED FIELD WELDS WITH ZINC RICH PAINT AFTER COMPLETION.
11. SEE DETAIL 1/S025 FOR ROOF DECK ATTACHMENT TO SUPPORTS.
12. BOTTOM OF DECK (BOD) SHALL BE AS SHOWN ON PLAN. TOP OF STRUCTURAL STEEL = BOTTOM OF DECK, UNLESS NOTED OTHERWISE. TOP OF COLUMN = BOD, UNLESS NOTED OTHERWISE.
13. STEEL MEMBERS DESIGNATED AS (c) OR (c)* ARE CHORDS/COLLECTORS AND ARE PART OF THE LATERAL FORCE RESISTING SYSTEM. ROOF DECK SHALL BE ATTACHED WITH EDGE SPACING PER 1/S025 .
14. AT ALL EXTERIOR BEAMS, BEAMS FRAMING INTO (c) OR (c)*, AND BEAMS FRAMING INTO BRACE FRAME BEAMS, PROVIDE FULL HEIGHT SHEAR TABS AS INDICATED IN DETAIL 25/S026 .
15. AT CANTILEVERS DESIGNATED AS "CANT" ON PLAN, CANTILEVER MEMBER SIZE SHALL MATCH BACKSPAN MEMBER SIZE UNLESS NOTED OTHERWISE.
16. EQUIPMENT SHALL NOT BE SUPPORTED FROM ROOF DECK. EQUIPMENT CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SUPPLEMENTAL SUPPORT SPANNING BETWEEN PRIMARY STRUCTURAL MEMBERS FOR SUPPORT OF EQUIPMENT.
17. REFER TO SHEET S501 FOR LOAD MAPS.



LEVEL 3 - FRAMING PLAN / LOW ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

PRCNC20221036
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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

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Checked by:	TD

Revisions

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1.30.23	IFC Revisions

LEVEL 3 FRAMING PLAN / LOW ROOF FRAMING PLAN

S103

IFC SET

BEAM END CONNECTIONS

1. BEAMS MARKED WITH (c) AND (c)* ARE SEISMIC COLLECTORS. THESE BEAMS ARE PART OF THE LATERAL FORCE RESISTING SYSTEM.

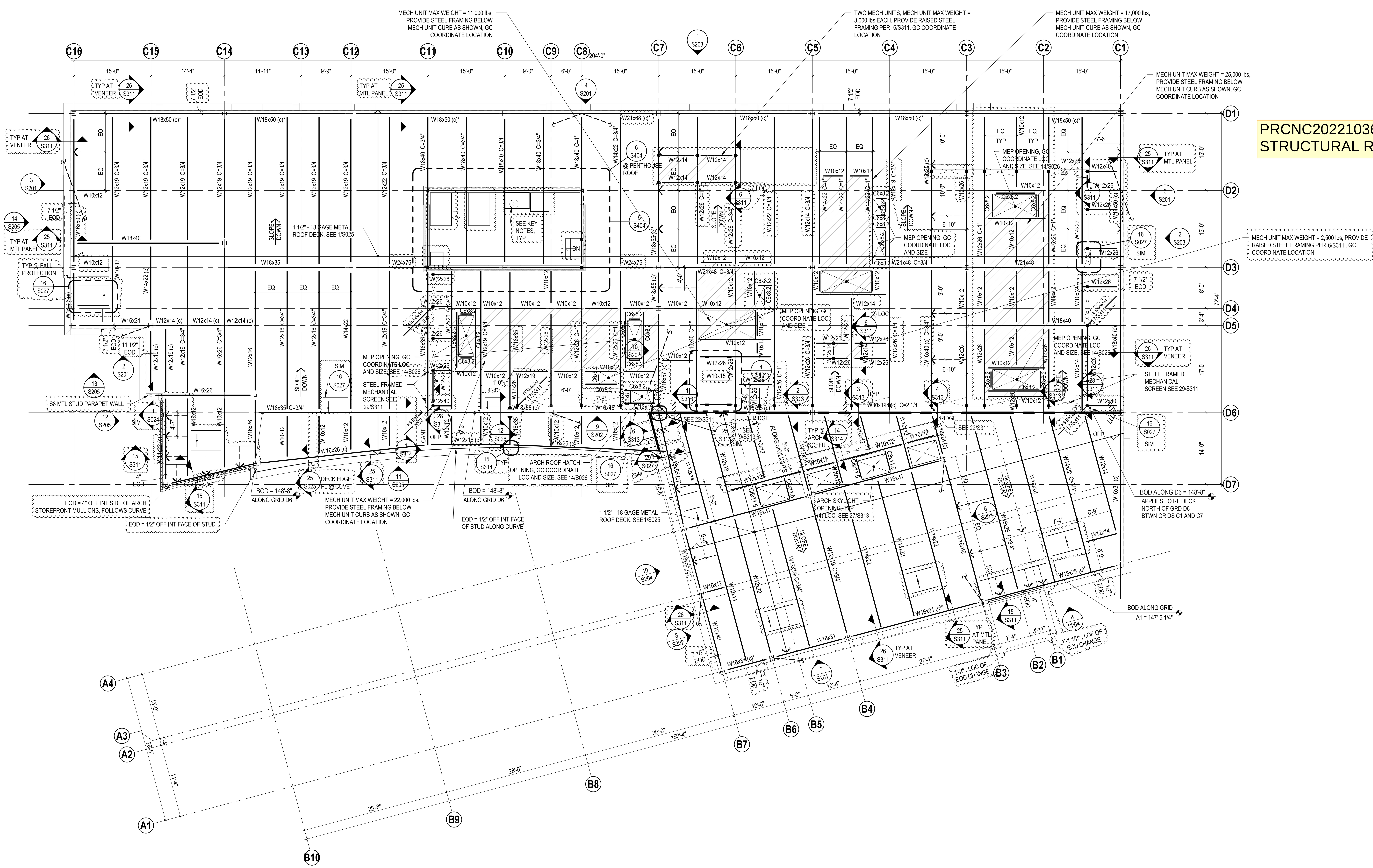
TYPICAL BEAM/GIRDER TO COLUMN:	5/S026
TYPICAL BEAM/GIRDER BOLTED CONNECTION:	25/S026
TYPICAL (c) CONNECTION AT COLUMNS:	7/S027
TYPICAL (c)* CONNECTION AT COLUMNS:	13/S027
TYPICAL (c) AND (c)* CONNECTION TO BEAM/GIRDER:	19/S027
TYPICAL SKEWED BEAM/GIRDER CONNECTION:	17/S027
TYPICAL COLUMN CAP PLATE, UNO:	2/S026

KEY FRAMING NOTES

- INDICATES EXTENTS AND DEPTH OF RECESSED SLAB-ON-GRADE. CONFIRM DEPTH OF RECESS INDICATED WITH ARCHITECTURAL AND FLOOR FINISH MANUFACTURER WHERE NOT INDICATED ON PLAN. FLOOR FINISHES WITHIN REGION OF RECESS SHALL FLUSH WITH FINISHES ON ADJACENT SLAB SURFACES. SEE DETAIL 10/S021 .
- INDICATES MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT. PROVIDE HOUSEKEEPING PAD PER DETAIL 5/S021 IF REQUIRED BY EQUIPMENT MANUFACTURER. SEE MECHANICAL, ELECTRICAL, AND/OR PLUMBING FOR EXACT SIZE, QUANTITY, AND LOCATION OF PANS.
- INDICATES FLOOR DRAIN. SLOPE 1/4" : 12" TOWARD CENTER OF FLOOR DRAIN OVER AN 18" RADIUS. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING.
- INDICATES FLOOR SINK WITH DRAIN. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH AND PLUMBING. SEE 27/S021 .
- INDICATES ELECTRICAL FLOOR BOX. VERIFY SIZE, QUANTITY, AND LOCATION WITH ARCH, ELECTRICAL, AND LAB. SEE 27/S021 .
- DESIGNATES CHANGE IN FLOOR OR ROOF DECK DIRECTION, SEE PLAN FOR SPAN DIRECTION
- DENOTES BOTTOM FLANGE BRACING. REFERENCE DETAILS 1/S027 , OR 2/S027 .

ROOF FRAMING PLAN NOTES

1. REFER TO SHEETS S001, S002 AND S003 FOR GENERAL STRUCTURAL AND SPECIAL INSPECTION NOTES.
2. REFER TO SHEETS S021 THROUGH S030 FOR TYPICAL DETAILS.
3. PROJECT ELEVATION 100'-0" = DATUM ELEVATION 534.0' PER CIVIL DRAWINGS.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.
5. AT ALL COMPOSITE DECK, INSTALL HEADED STUDS AT 12" ON CENTER AT ALL SUPPORTS UNLESS NOTED OTHERWISE. SEE DETAIL 7/S025 .
6. REPETITIVE FRAMING IS SPACED EQUALLY BETWEEN DIMENSIONED ELEMENTS, WITH THE NUMBER OF EQUAL SPACES GRAPHICALLY INDICATED IN PLAN, UNLESS NOTED OTHERWISE.
7. BEAMS FRAMING INTO WALL ENDS AND COLUMNS ARE ALIGNED CENTERLINE TO CENTERLINE WITH THEIR SUPPORTS, UNLESS NOTED OTHERWISE.
8. COORDINATE SIZES AND LOCATIONS OF ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. NOT ALL REQUIRED OPENINGS ARE INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO DETAIL 3/S025 FOR OPENINGS IN ROOF DECK.
9. STRUCTURAL STEEL SHOP TAGS, ERECTION AIDS AND MILL MARKINGS EXPOSED TO VIEW SHALL BE REMOVED AFTER FIELD ASSEMBLY. PATCH, TOUCH UP AND GRIND SMOOTH.
10. HOT DIP GALVANIZE ALL STEEL, CONNECTIONS, AND BOLTS EXPOSED TO WEATHER. PAINT EXPOSED FIELD WELDS WITH ZINC RICH PAINT AFTER COMPLETION.
11. SEE DETAIL 1/S025 FOR ROOF DECK ATTACHMENT TO SUPPORTS.
12. BOTTOM OF DECK (BOD) SHALL BE AS SHOWN ON PLAN. TOP OF STRUCTURAL STEEL = BOTTOM OF DECK, UNLESS NOTED OTHERWISE. TOP OF COLUMN = BOD, UNLESS NOTED OTHERWISE.
13. STEEL MEMBERS DESIGNATED AS (c) OR (c)* ARE CHORDS/COLLECTORS AND ARE PART OF THE LATERAL FORCE RESISTING SYSTEM. ROOF DECK SHALL BE ATTACHED WITH EDGE SPACING PER 1/S025 .
14. AT ALL EXTERIOR BEAMS, BEAMS FRAMING INTO (c) OR (c)*, AND BEAMS FRAMING INTO BRACE FRAME BEAMS, PROVIDE FULL HEIGHT SHEAR TABS AS INDICATED IN DETAIL 25/S026 .
15. AT CANTILEVERS DESIGNATED AS "CANT" ON PLAN, CANTILEVER MEMBER SIZE SHALL MATCH BACKSPAN MEMBER SIZE UNLESS NOTED OTHERWISE.
16. EQUIPMENT SHALL NOT BE SUPPORTED FROM ROOF DECK. EQUIPMENT CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SUPPLEMENTAL SUPPORT SPANNING BETWEEN PRIMARY STRUCTURAL MEMBERS FOR SUPPORT OF EQUIPMENT.
17. REFER TO SHEET S501 FOR LOAD MAPS.



ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING

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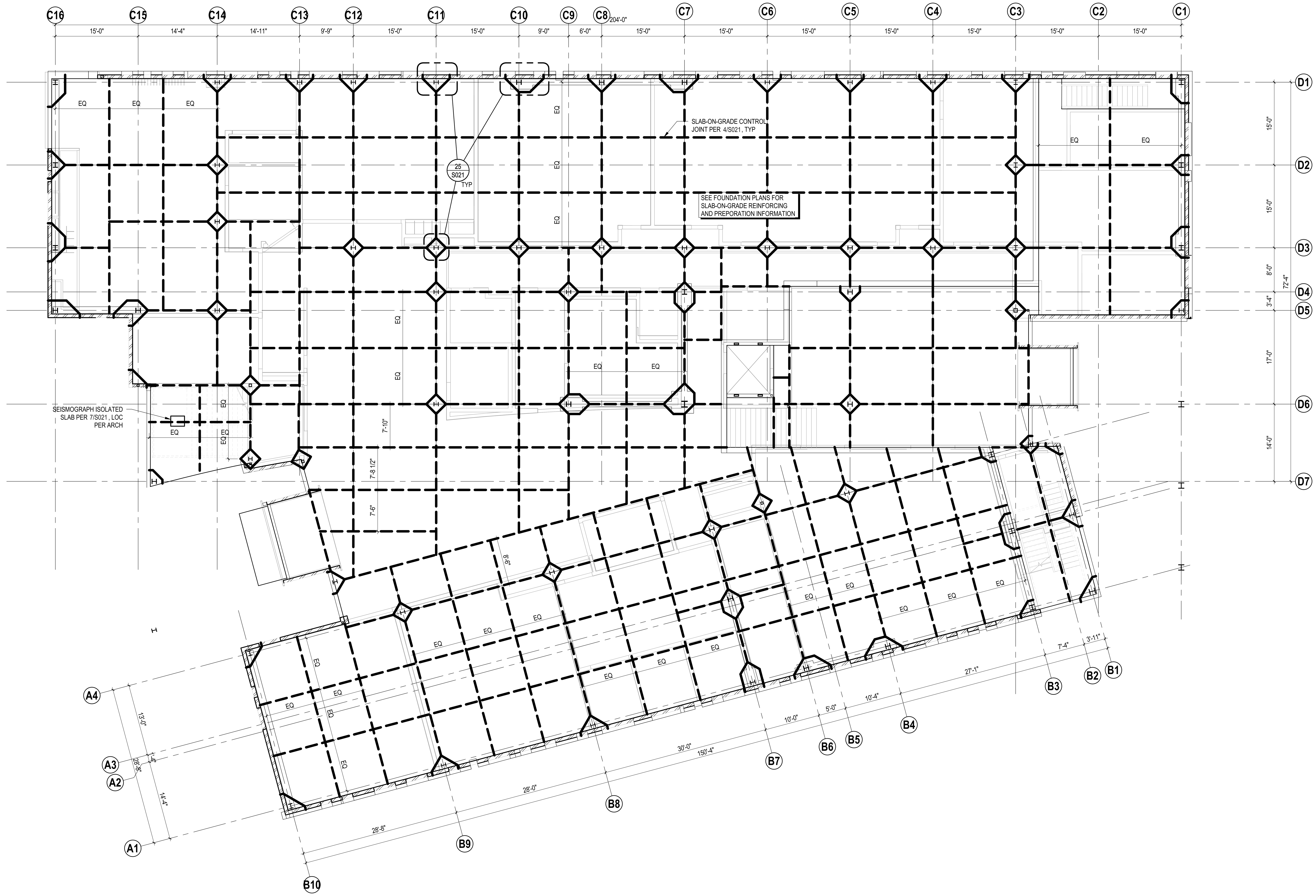
ROOF FRAMING PLAN

S104

IFC SET

SLAB-ON-GRADE CONTROL JOINT NOTES:

1. CONTRACTOR TO SUBMIT CONTROL JOINT LAYOUT TO DESIGN TEAM FOR APPROVAL PRIOR TO POURING SLAB-ON-GRADE CONCRETE.
2. SEE PLAN FOR TYPICAL SLAB-GRADE REINFORING.
3. SEE DETAIL 4/S021 FOR CONTROL JOINT CONSTRUCTION.



PRCNC20221036
STRUCTURAL REVISION

1 SLAB CONTROL JOINT PLAN - SLAB-ON-GRADE

SCALE: 1/8" = 1'-0"

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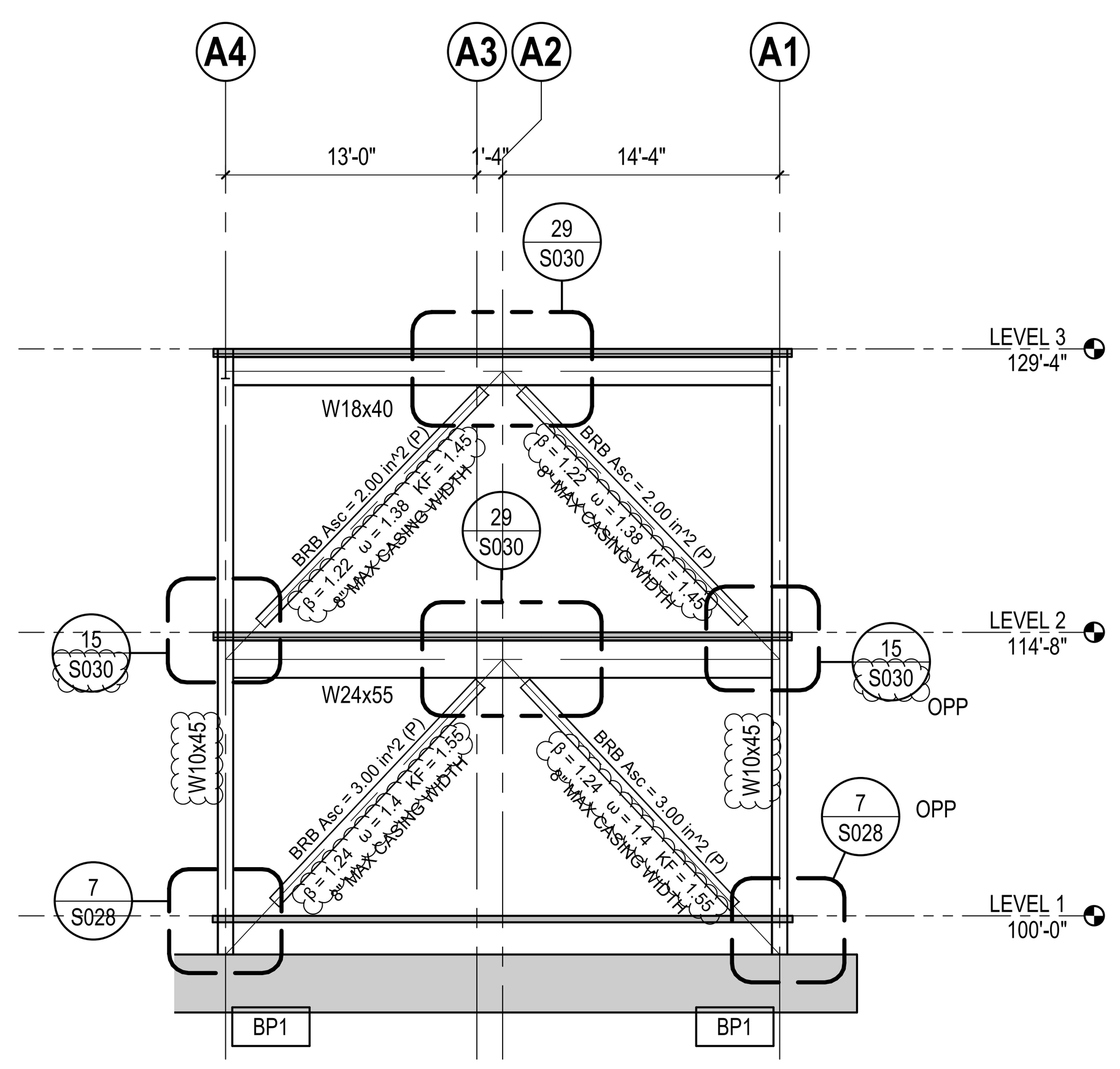
SLAB ON GRADE CONTROL JOINT PLAN

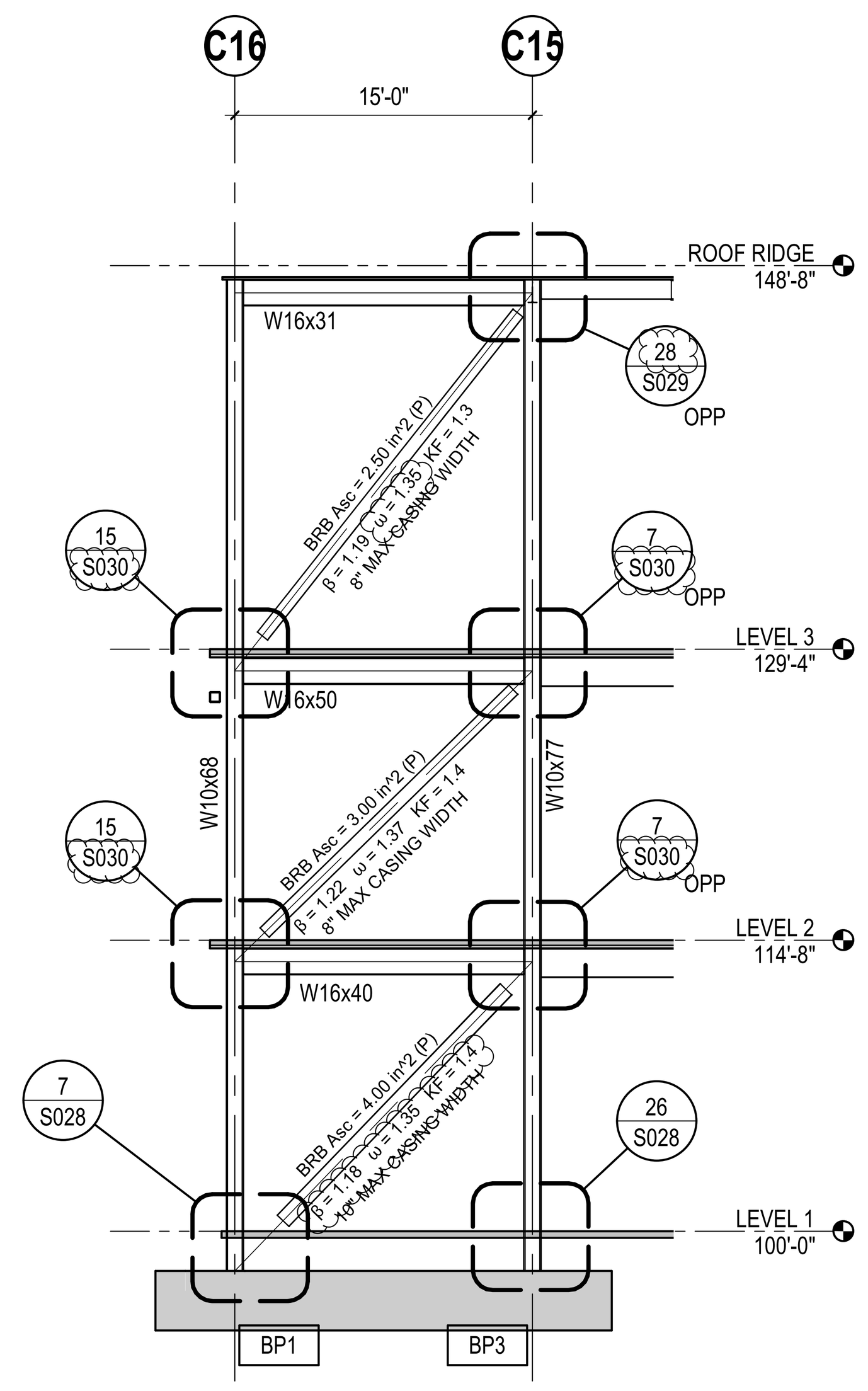
S111

IFC SET

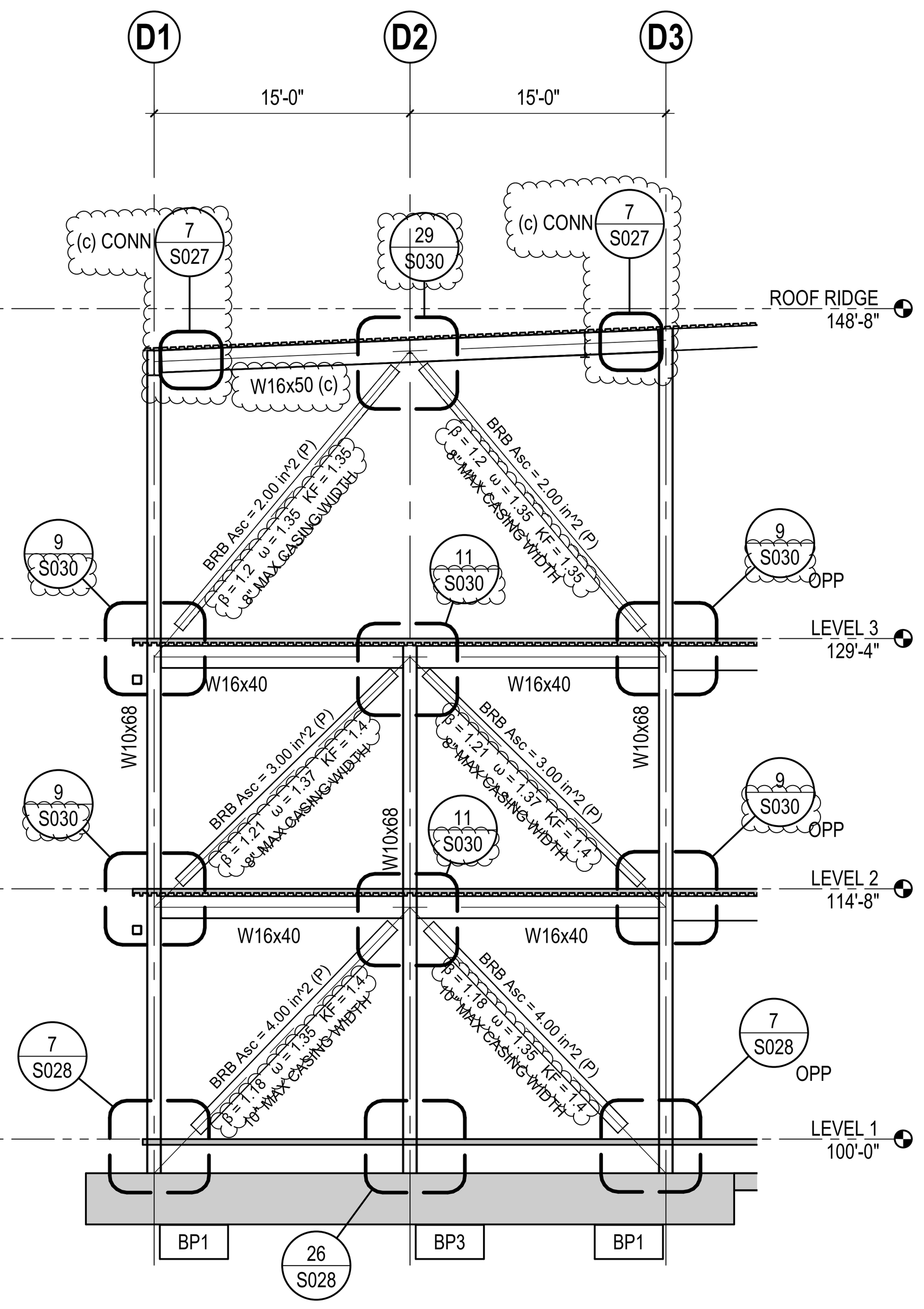
PRCNC20221036
STRUCTURAL REVISION



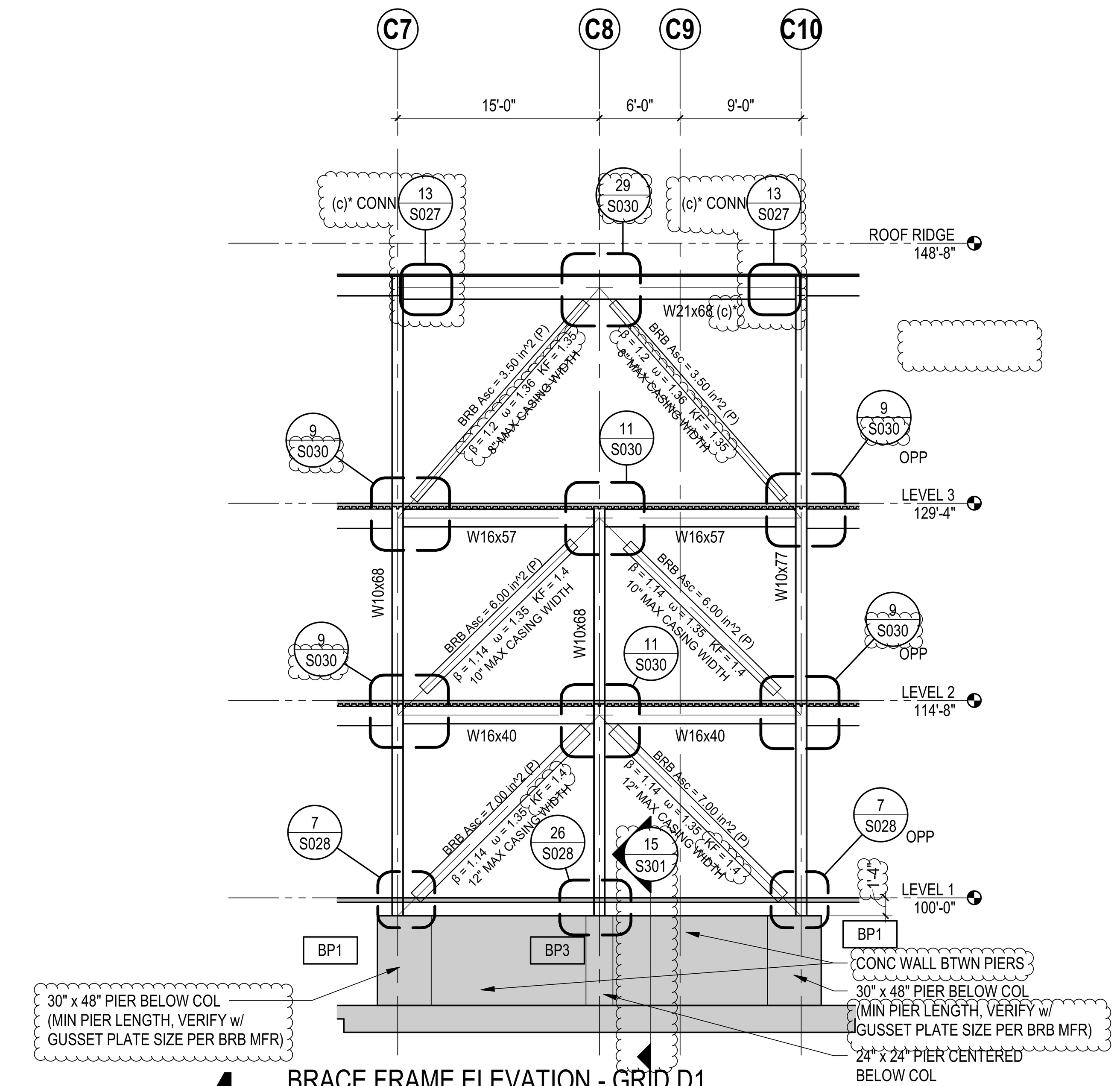
1 BRACE FRAME ELEVATION - GRID B10
SCALE: 1/8" = 1'-0"



2 BRACE FRAME ELEVATION - GRID D5
SCALE: 1/8" = 1'-0"



3 BRACE FRAME ELEVATION - GRID C16
SCALE: 1/8" = 1'-0"

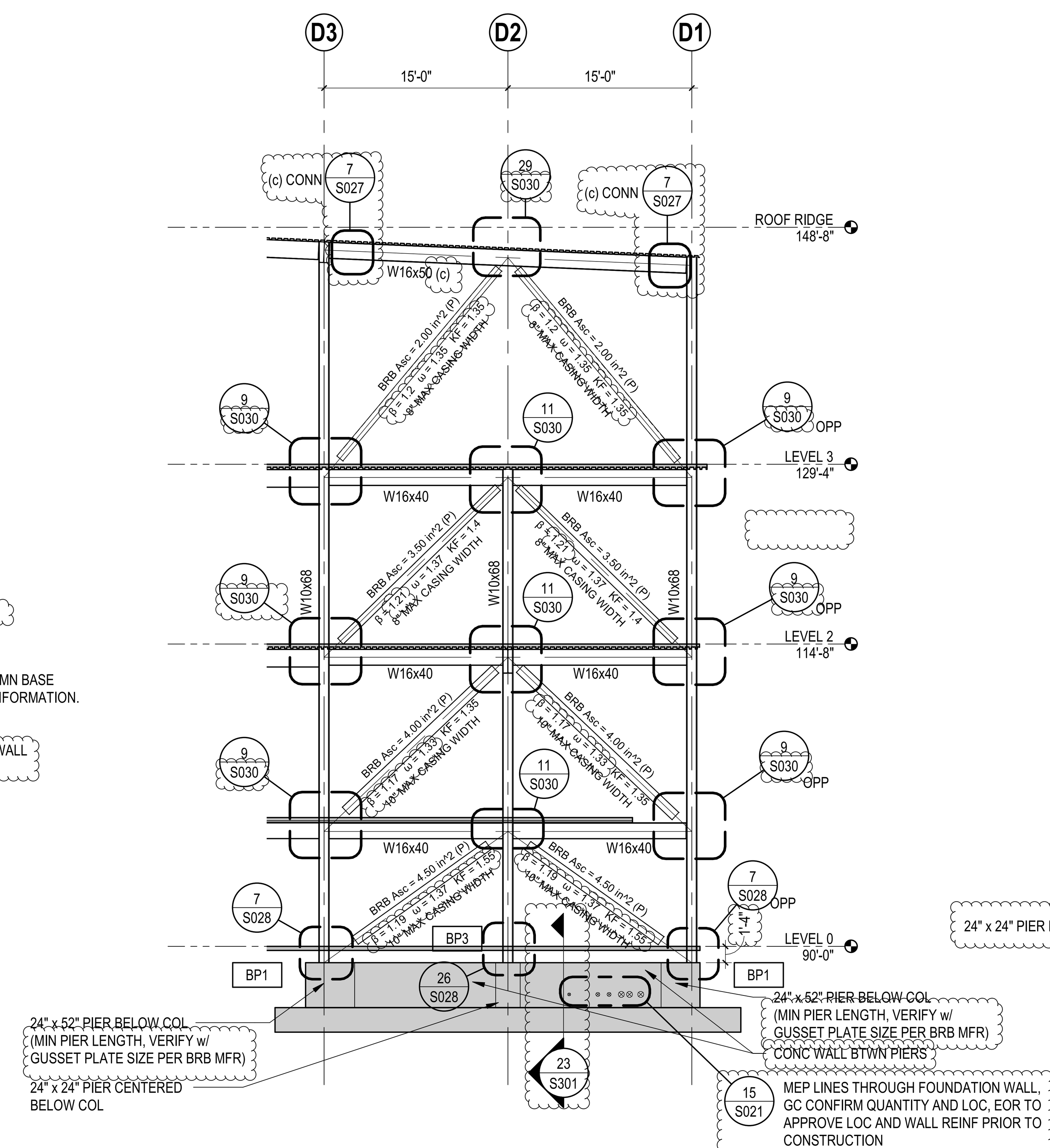


4 BRACE FRAME ELEVATION - GRID D1
SCALE: 1/8" = 1'-0"

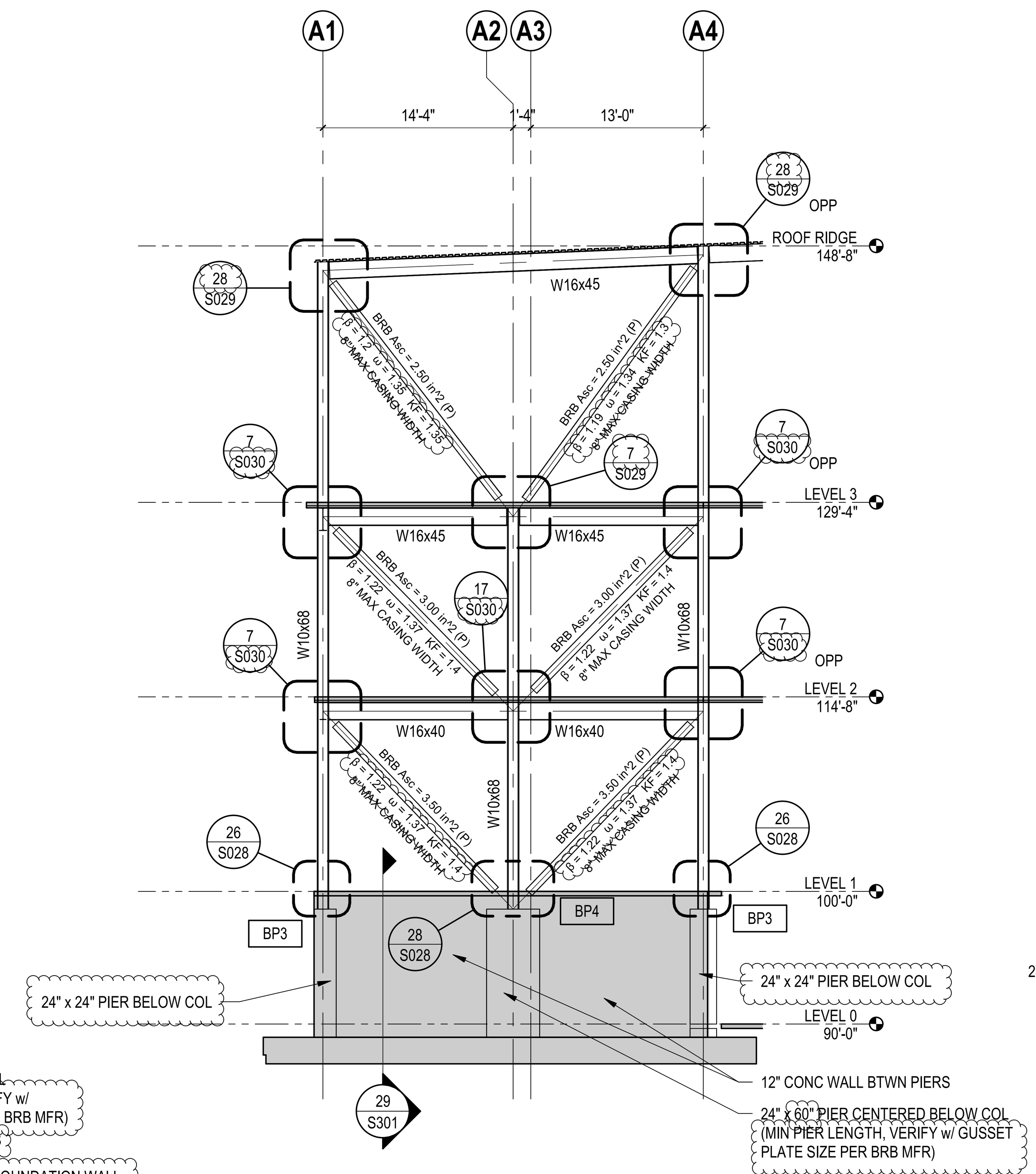
BUCKLING-RESTRAINED BRACED FRAME NOTES

1. Fy_min FOR ALL BUCKLING-RESTRAINED BRACES = 38 ksi
2. Fy_max FOR ALL BUCKLING-RESTRAINED BRACES = 46 ksi
3. Asc = STEEL CORE AREA
   (P) = PINNED BRACE END CONNECTIONS

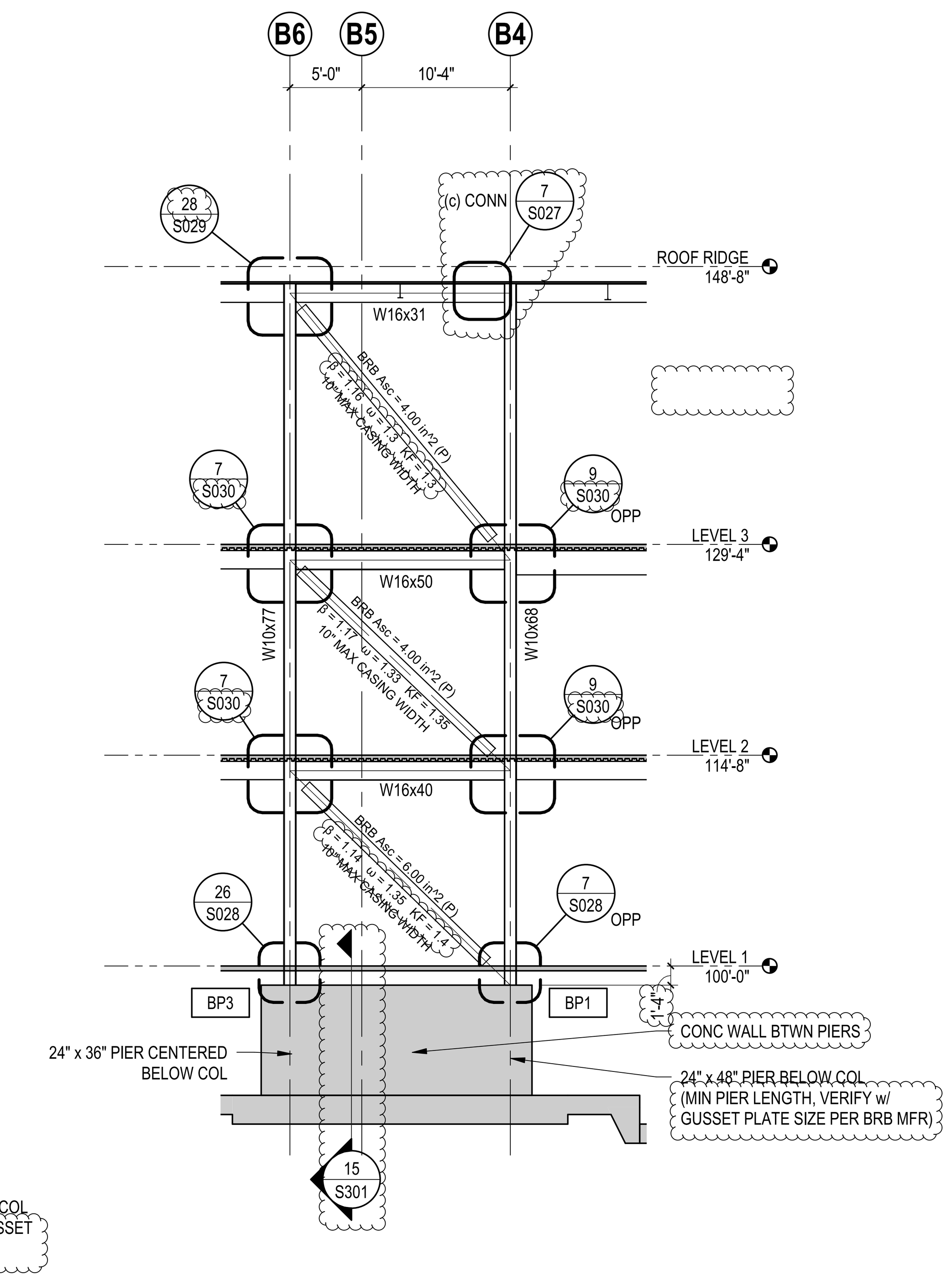
5. BPx CALLED OUT ON BUCKLING-RESTRAINED BRACE FRAME ELEVATIONS INDICATES A COLUMN BASE PLATE ID. SEE DETAILS 7/S028, 26/S028, AND 28/S028 FOR BASE PLATE SCHEDULE AND ADDITIONAL INFORMATION.
7. SEE 18/S028 FOR BRACED FRAME PROTECTED ZONE INFORMATION.
8. SEE SHEETS S028-S031 FOR TYPICAL BRB DETAILS.
9. EXTERIOR STEMWALLS ARE NOT SHOWN FOR CLARITY. REFERENCE DETAILS ON PLAN FOR STEMWALL INFORMATION.



5 BRACE FRAME ELEVATION - GRID C1
SCALE: 1/8" = 1'-0"



6 BRACE FRAME ELEVATION - GRID B3
SCALE: 1/8" = 1'-0"



7 BRACE FRAME ELEVATION - GRID A1
SCALE: 1/8" = 1'-0"

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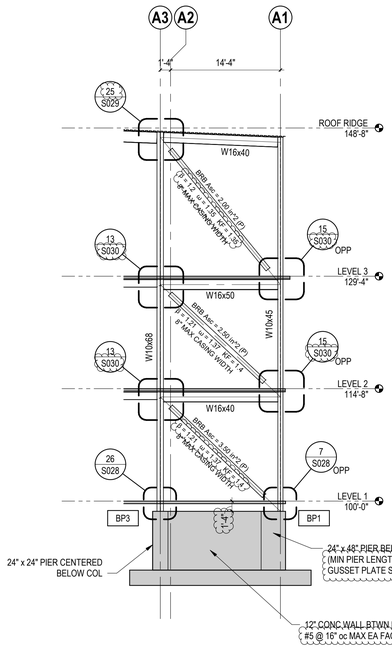
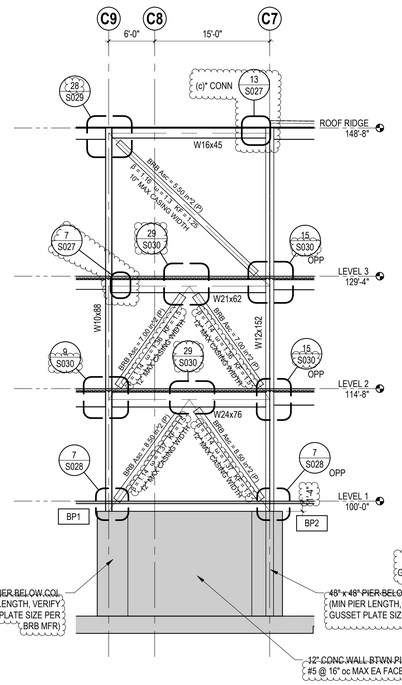
BRACE FRAME ELEVATIONS

S201

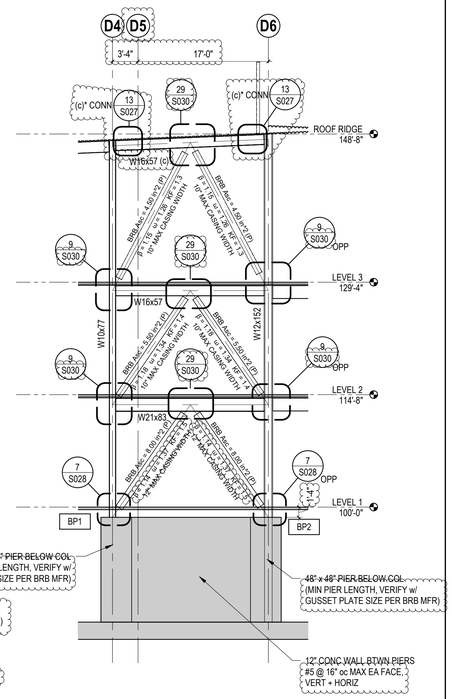
IFC SET



8 BRACE FRAME ELEVATION - GRID B7
SCALE: 1/8" = 1'-0"



9 BRACE FRAME ELEVATION - GRID D6
SCALE: 1/8" = 1'-0"



10 BRACE FRAME ELEVATION - GRID C7
SCALE: 1/8" = 1'-0"

PRCNC20221036
STRUCTURAL REVISION

BUCKLING-RESTRAINED BRACED FRAME NOTES

1. Fy_min FOR ALL BUCKLING-RESTRAINED BRACES = 38 ksi
2. Fy_max FOR ALL BUCKLING-RESTRAINED BRACES = 46 ksi
3. Asc = STEEL CORE AREA
   (P) = PINNED BRACE END CONNECTIONS

5. BPx CALLED OUT ON BUCKLING-RESTRAINED BRACE FRAME ELEVATIONS INDICATES A COLUMN BASE PLATE ID. SEE DETAILS 7/S028,25/S029, AND 26/S028 FOR BASE PLATE SCHEDULE AND ADDITIONAL INFORMATION.
7. SEE 18/S028 FOR BRACED FRAME PROTECTED ZONE INFORMATION.
8. SEE SHEETS S028/S031 FOR TYPICAL BRB DETAILS.
9. EXTERIOR STEMWALLS ARE NOT SHOWN FOR CLARITY. REFERENCE DETAILS ON PLAN FOR STEMWALL INFORMATION.

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THOMAS H. DANIEL
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REGISTERED PROFESSIONAL ENGINEER

City of Puyallup
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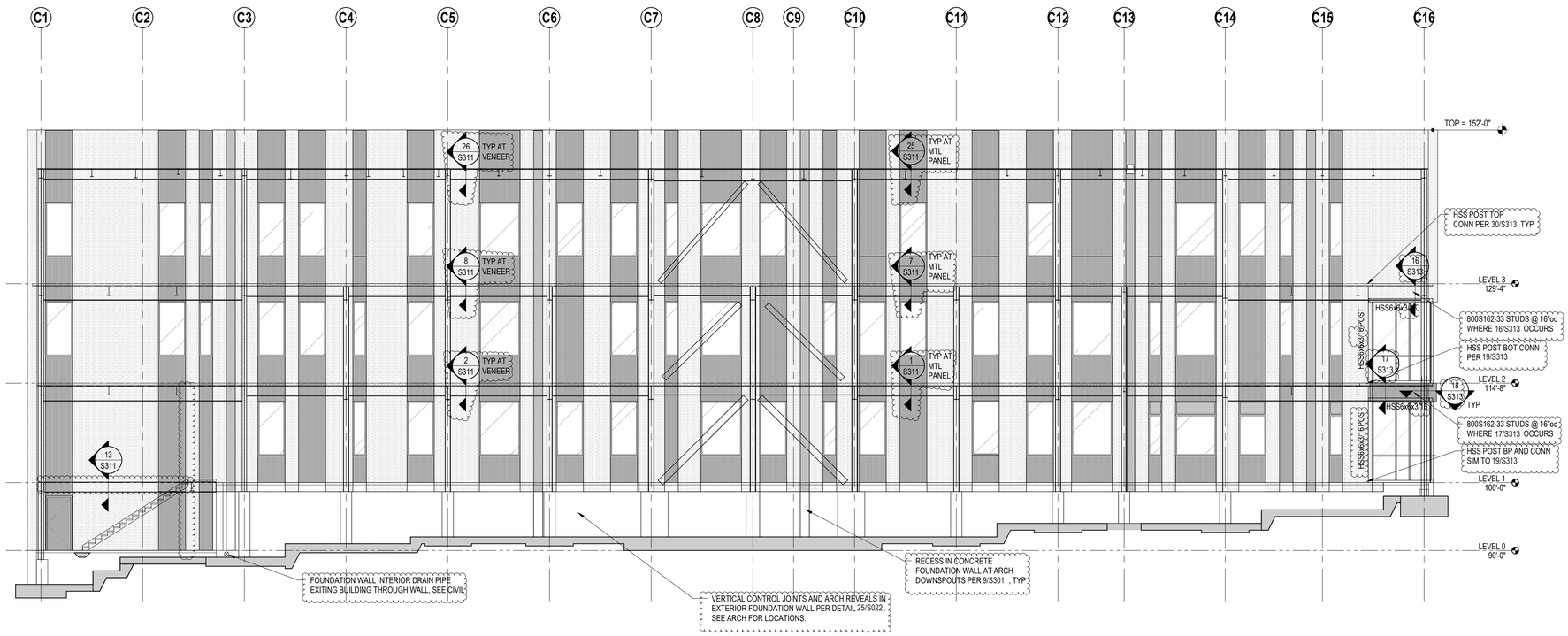
BRACE FRAME ELEVATIONS

S202

IFC SET



1 EXTERIOR WALL ELEVATION GRID D1

SCALE: 1/8" = 1'-0"

EXTERIOR METAL STUD WALL NOTES:

1. FOR TYPICAL METAL STUD DETAILS SEE SHEETS S023 & S024
2. WALL STUDS ARE 800S162-54 @ 16"oc, UNO
3. FOR TYPICAL MASONRY VENEER DETAILS SEE SHEET S022
4. PROVIDE LOOSE STEEL LINTEL ANGLES PER 5/S022 WHERE REQUIRED
5. SEE GENERAL NOTES FOR ADDITIONAL METAL STUD REQUIREMENTS
6. VERIFY ALL OPENING SIZES & LOCATIONS WITH ARCHITECTURAL DRAWINGS AND MEP SUBCONTRACTORS PRIOR TO CONSTRUCTION
7. TOP OF PARAPET (TOP) IS TOP OF METAL STUD FRAMING

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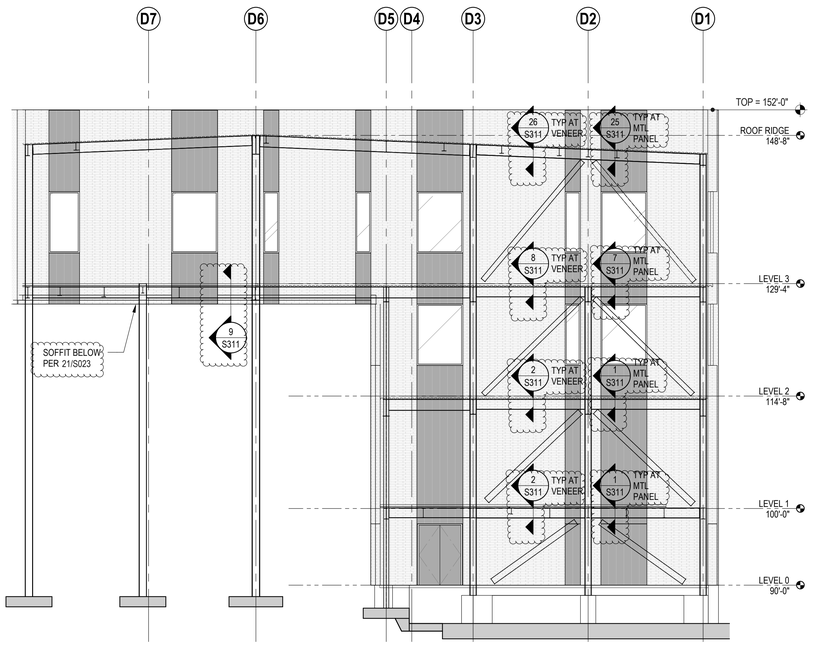
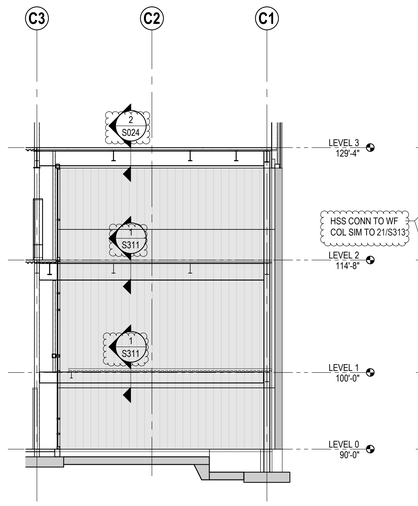


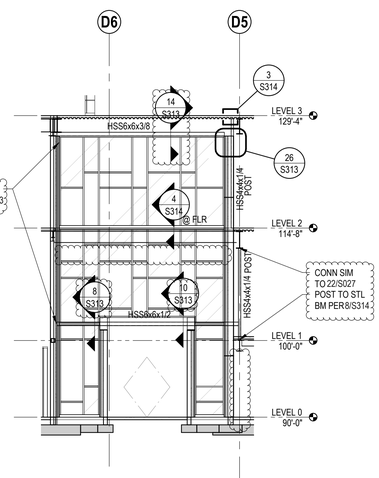
2 EXTERIOR WALL ELEVATION GRID C1

SCALE: 1/8" = 1'-0"



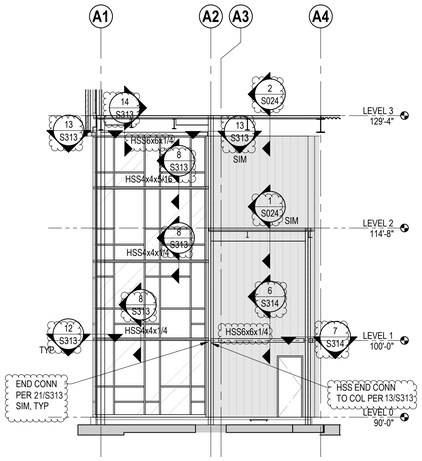
3 EXTERIOR WALL ELEVATION GRID D5

SCALE: 1/8" = 1'-0"



4 EXTERIOR WALL ELEVATION NEAR GRID C3

SCALE: 1/8" = 1'-0"



5 EXTERIOR WALL ELEVATION GRID B1

SCALE: 1/8" = 1'-0"

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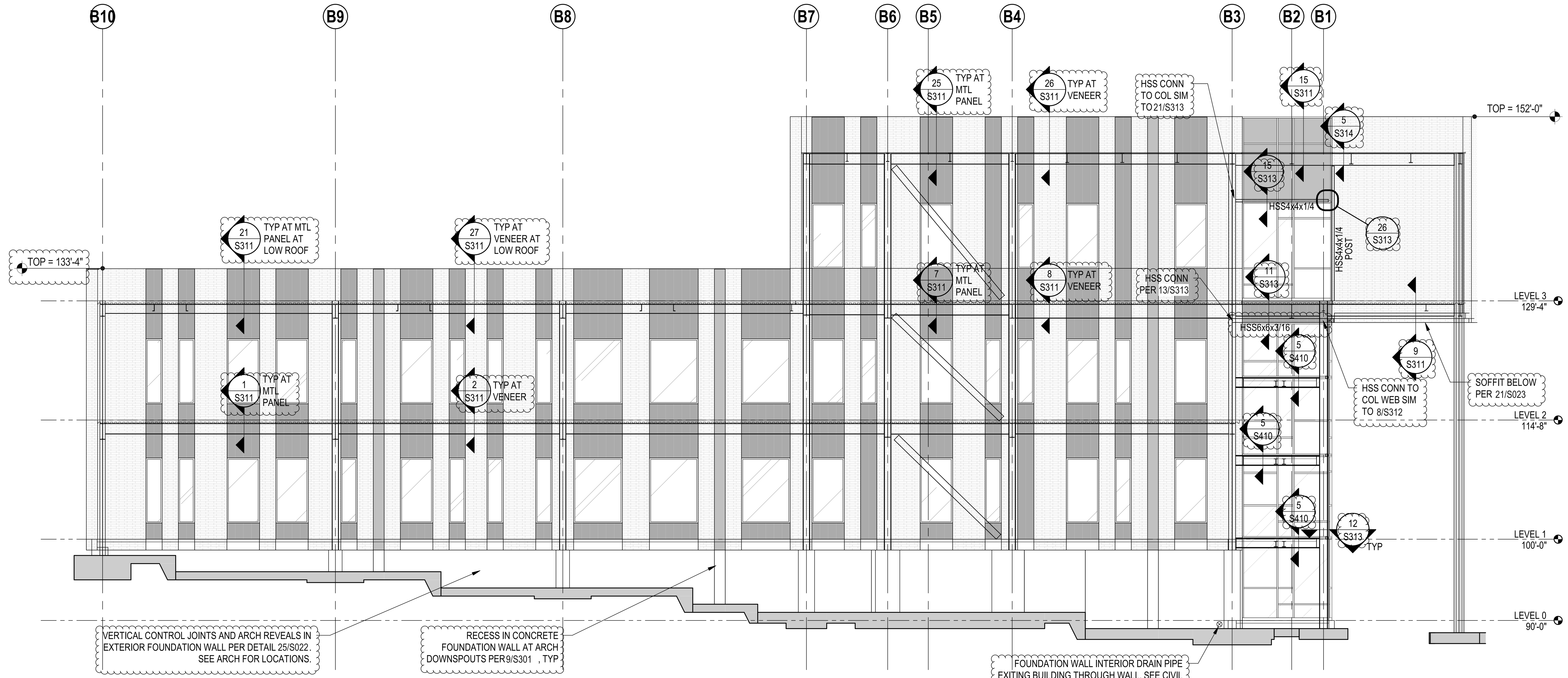
EXTERIOR WALL ELEVATIONS

S203

IFC SET

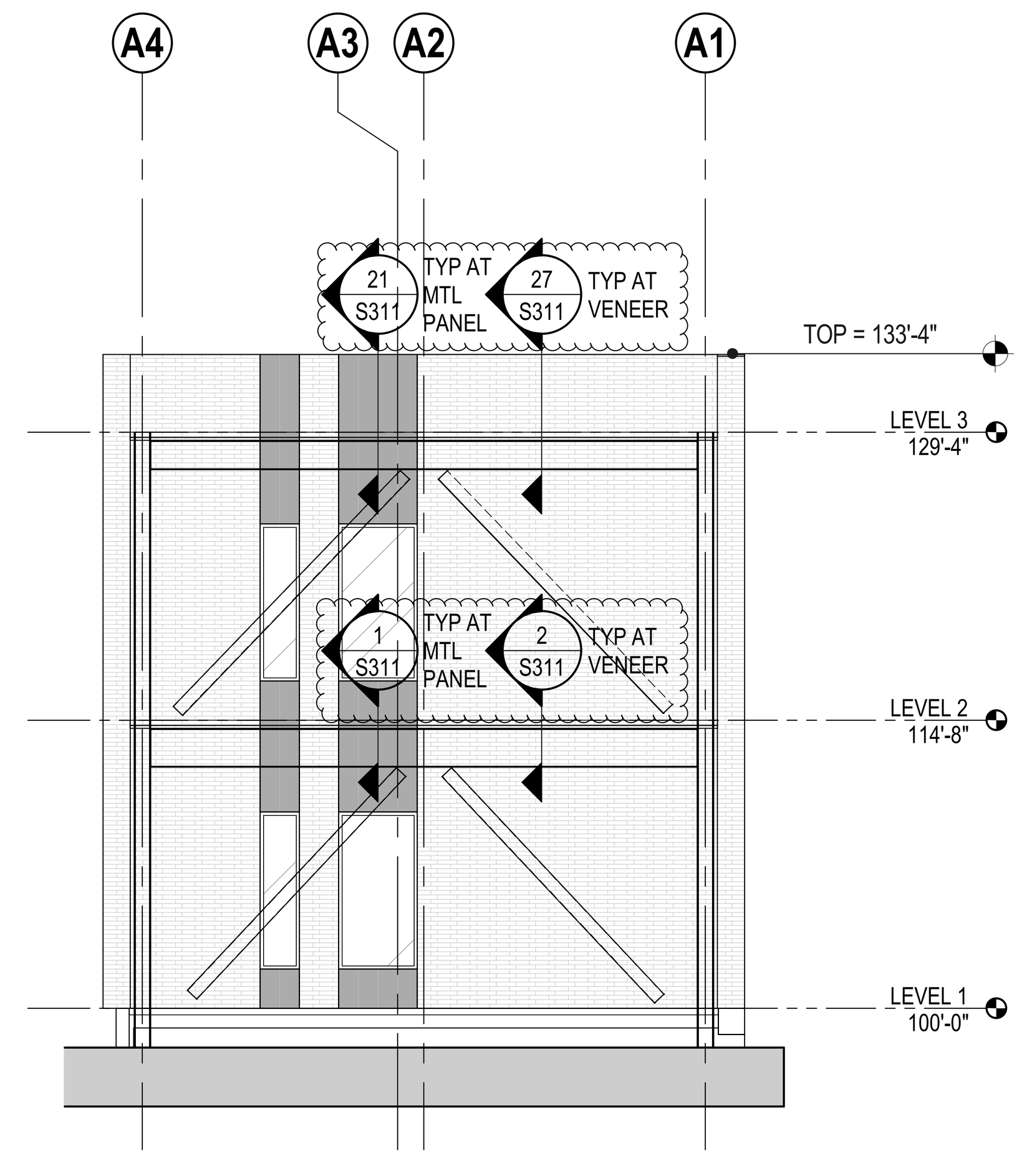


6 EXTERIOR WALL ELEVATION GRID A1

SCALE: 1/8" = 1'-0"

EXTERIOR METAL STUD WALL NOTES:

1. FOR TYPICAL METAL STUD DETAILS SEE SHEETS S023 & S024
2. WALL STUDS ARE 800S162-54 @ 16"oc, UNO
3. FOR TYPICAL MASONRY VENEER DETAILS SEE SHEET S022
4. PROVIDE LOOSE STEEL LINTEL ANGLES PER 5/S022 WHERE REQUIRED
5. SEE GENERAL NOTES FOR ADDITIONAL METAL STUD REQUIREMENTS
6. VERIFY ALL OPENING SIZES & LOCATIONS WITH ARCHITECTURAL DRAWINGS AND MEP SUBCONTRACTORS PRIOR TO CONSTRUCTION
7. TOP OF PARAPET (TOP) IS TOP OF METAL STUD FRAMING



7 EXTERIOR WALL ELEVATION GRID B9

SCALE: 1/8" = 1'-0"

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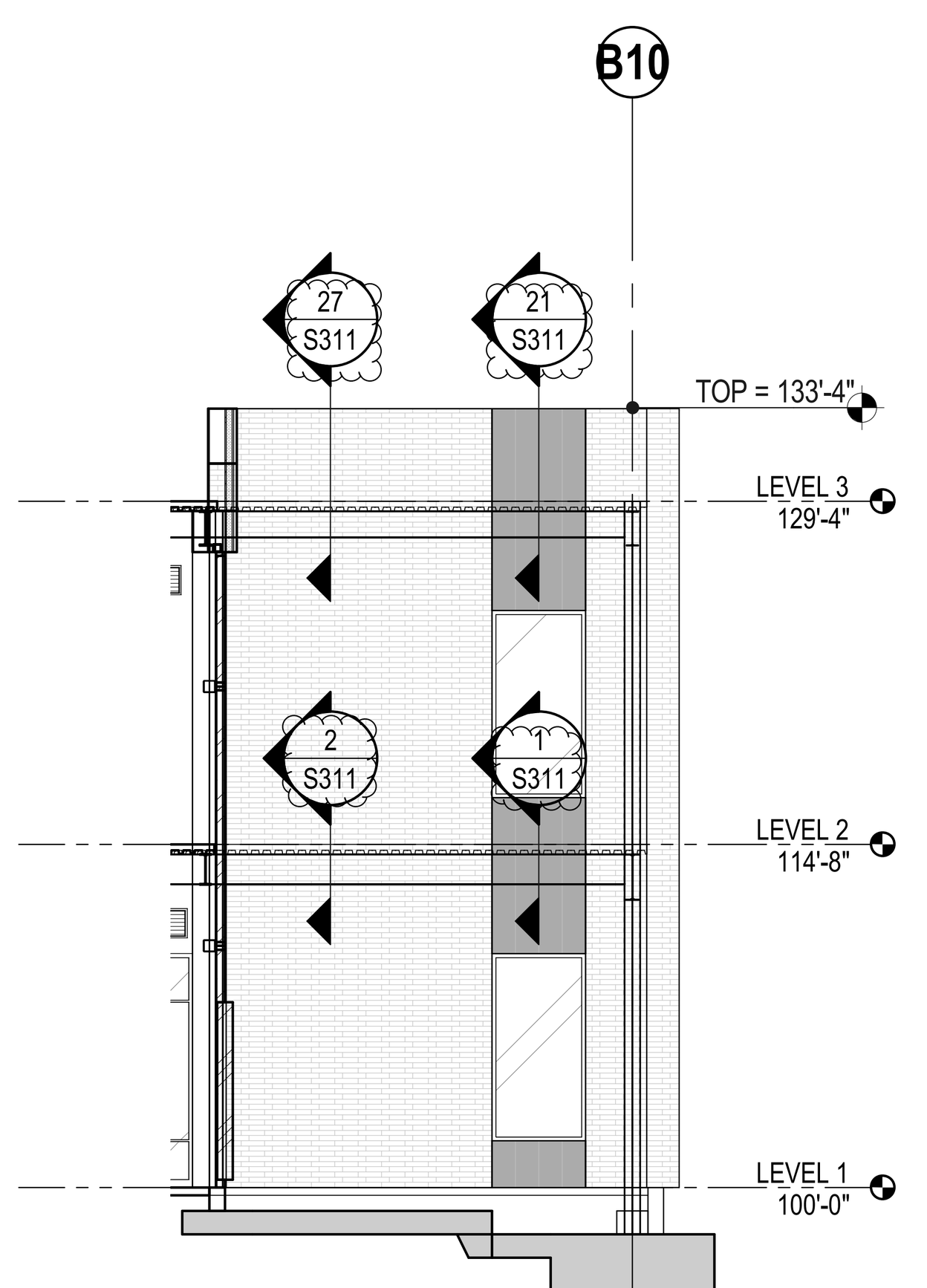
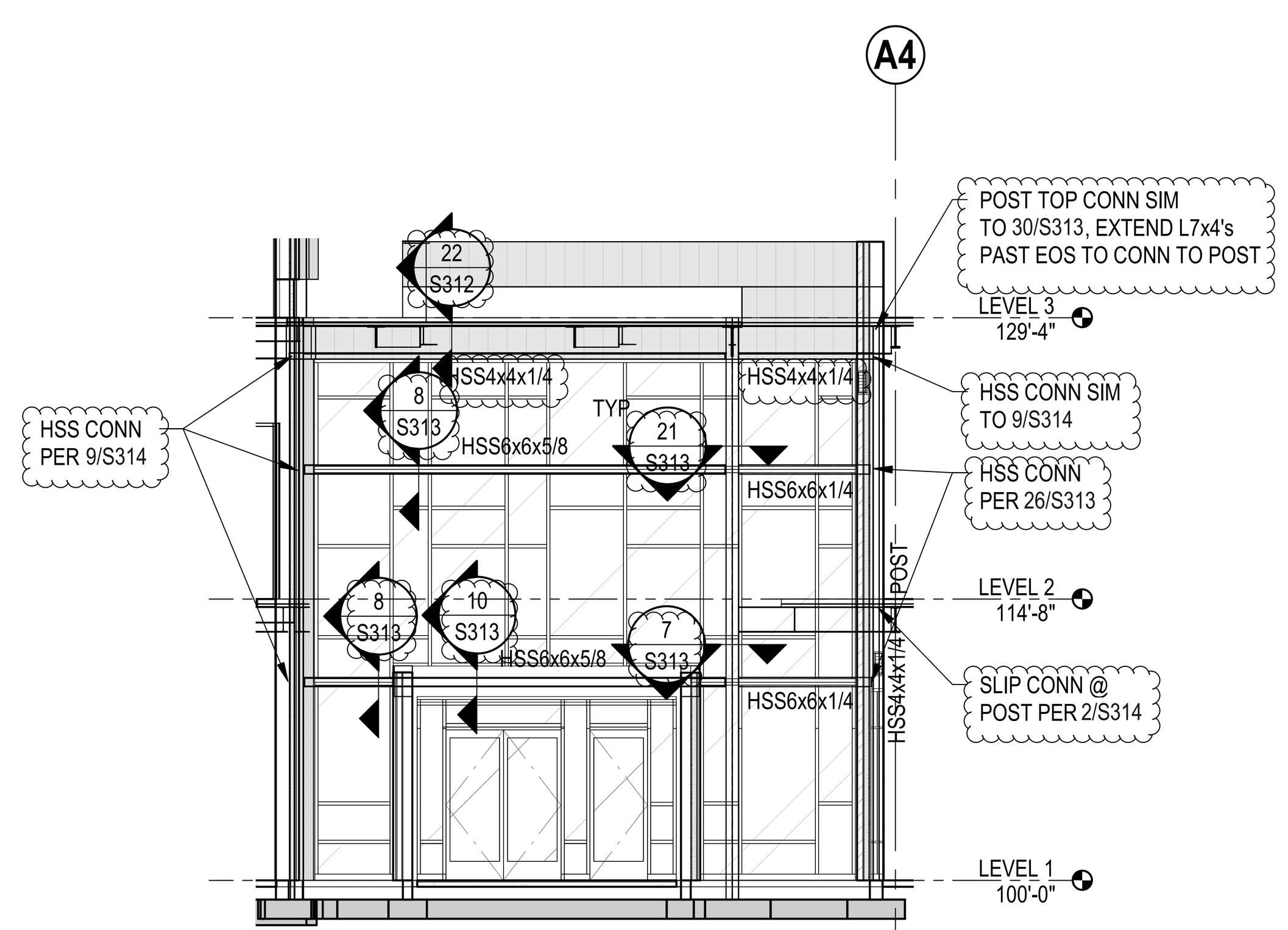


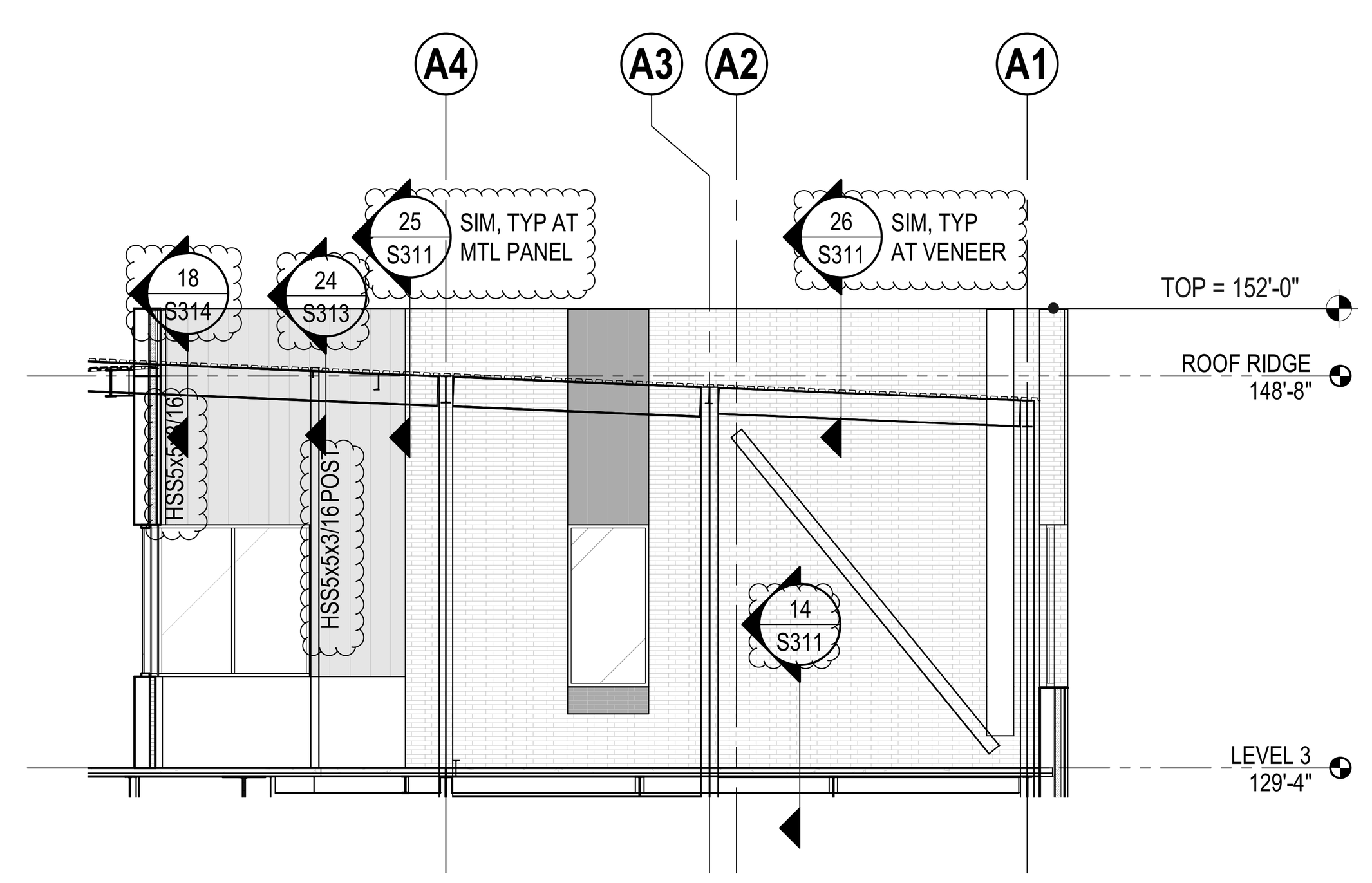
8 EXTERIOR WALL ELEVATION GRID A4

SCALE: 1/8" = 1'-0"



9 EXTERIOR WAL ELEVATION NEAR GRID B8

SCALE: 1/8" = 1'-0"



10 EXTERIOR WALL ELEVATION GRID B7

SCALE: 1/8" = 1'-0"

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Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	RB/AM
Checked by:	TD

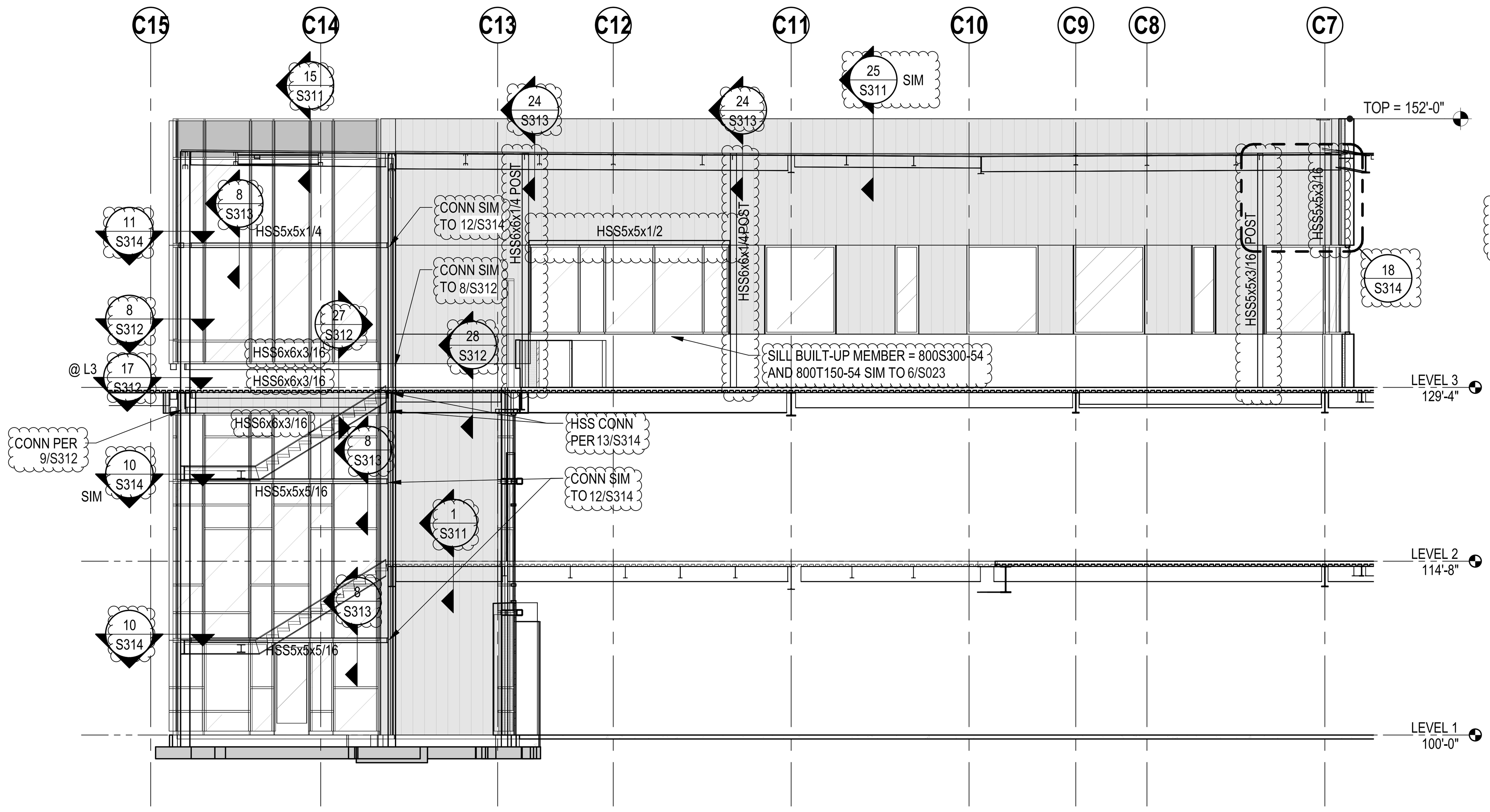
Revisions

Date	Description
1.30.23	IFC Revisions

EXTERIOR WALL ELEVATIONS

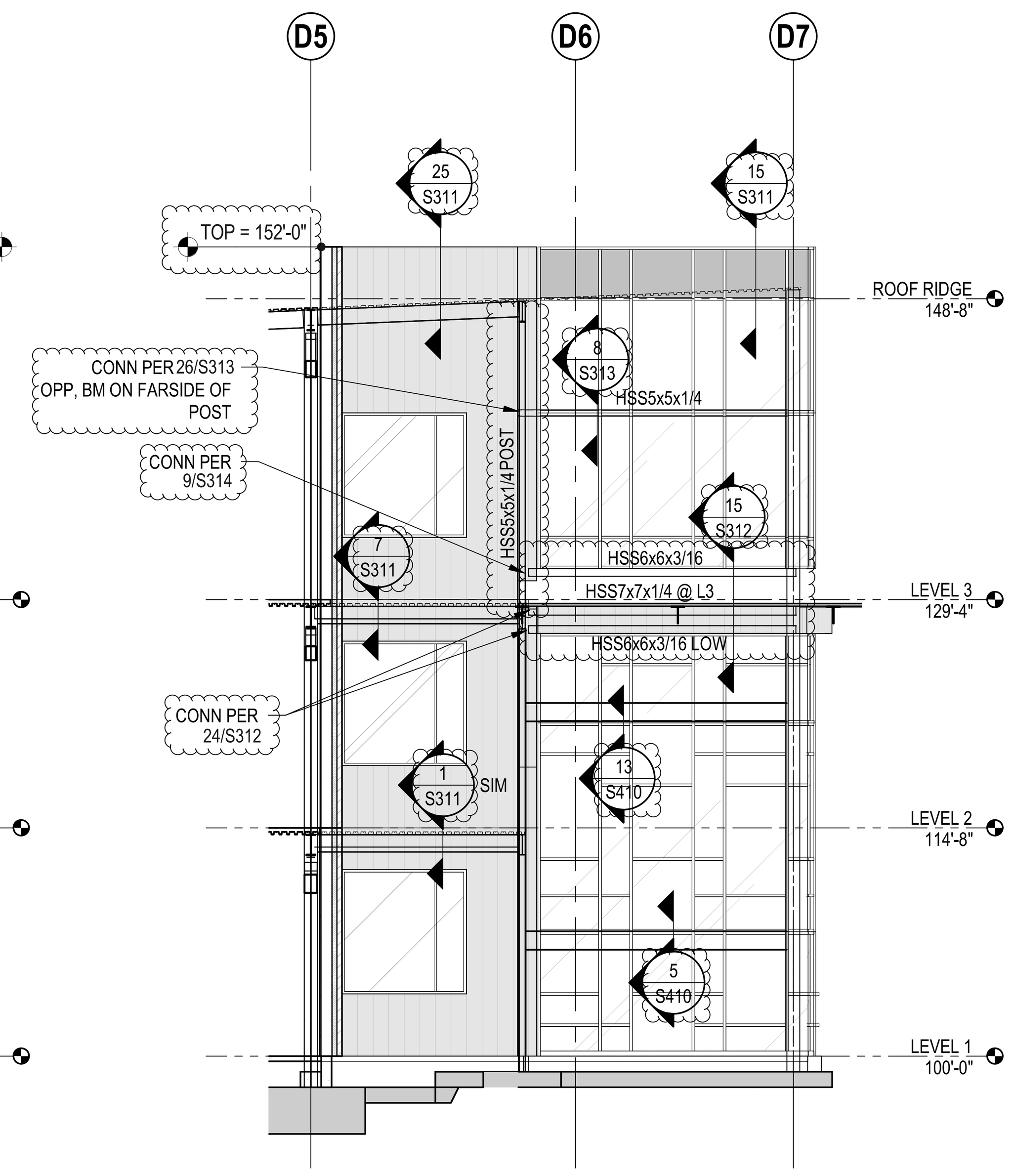
S204

IFC SET



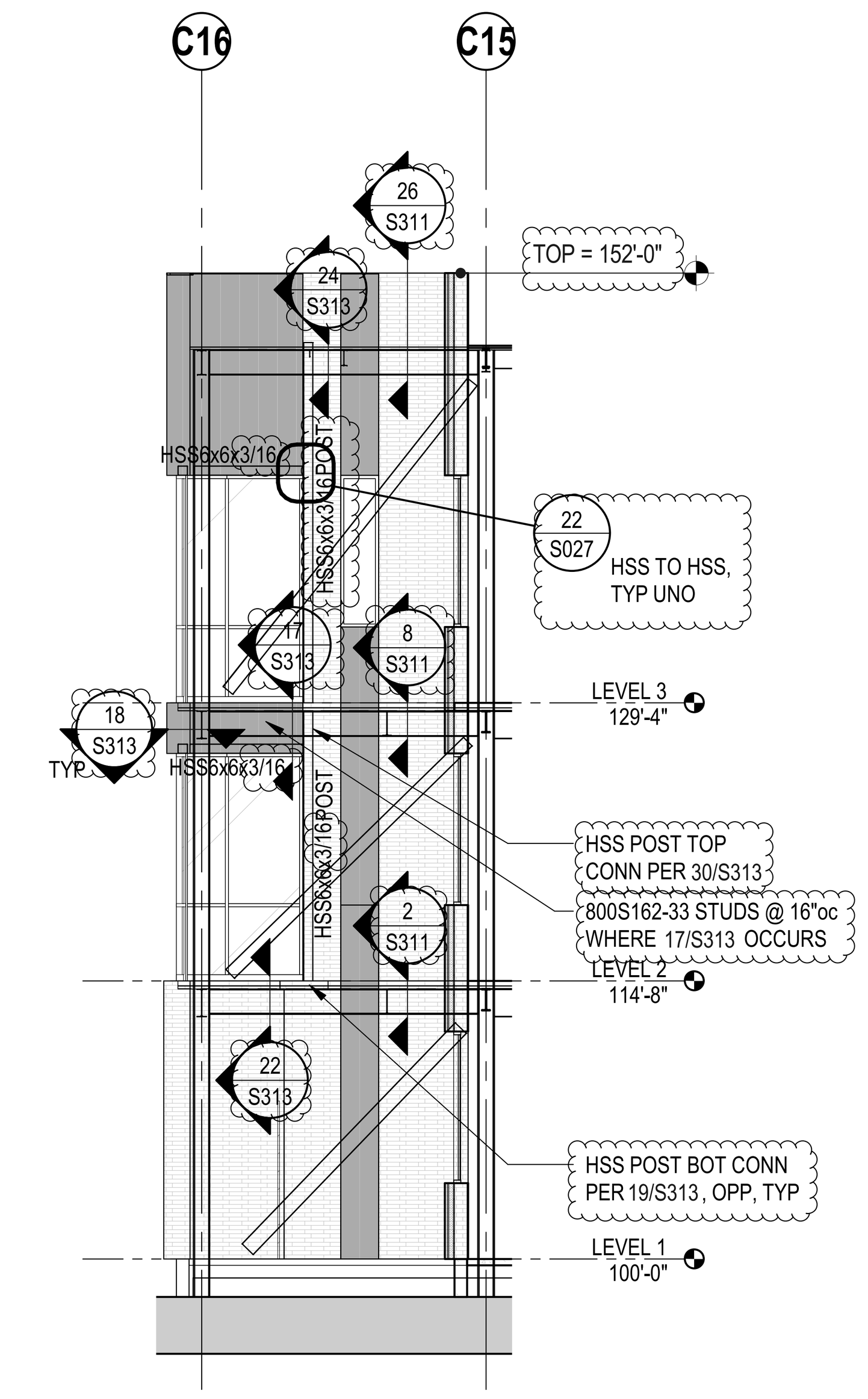
11 EXTERIOR WALL ELEVATION NEAR GRID D7

SCALE: 1/8" = 1'-0"



12 EXTERIOR WALL ELEVATION GRID C15

SCALE: 1/8" = 1'-0"



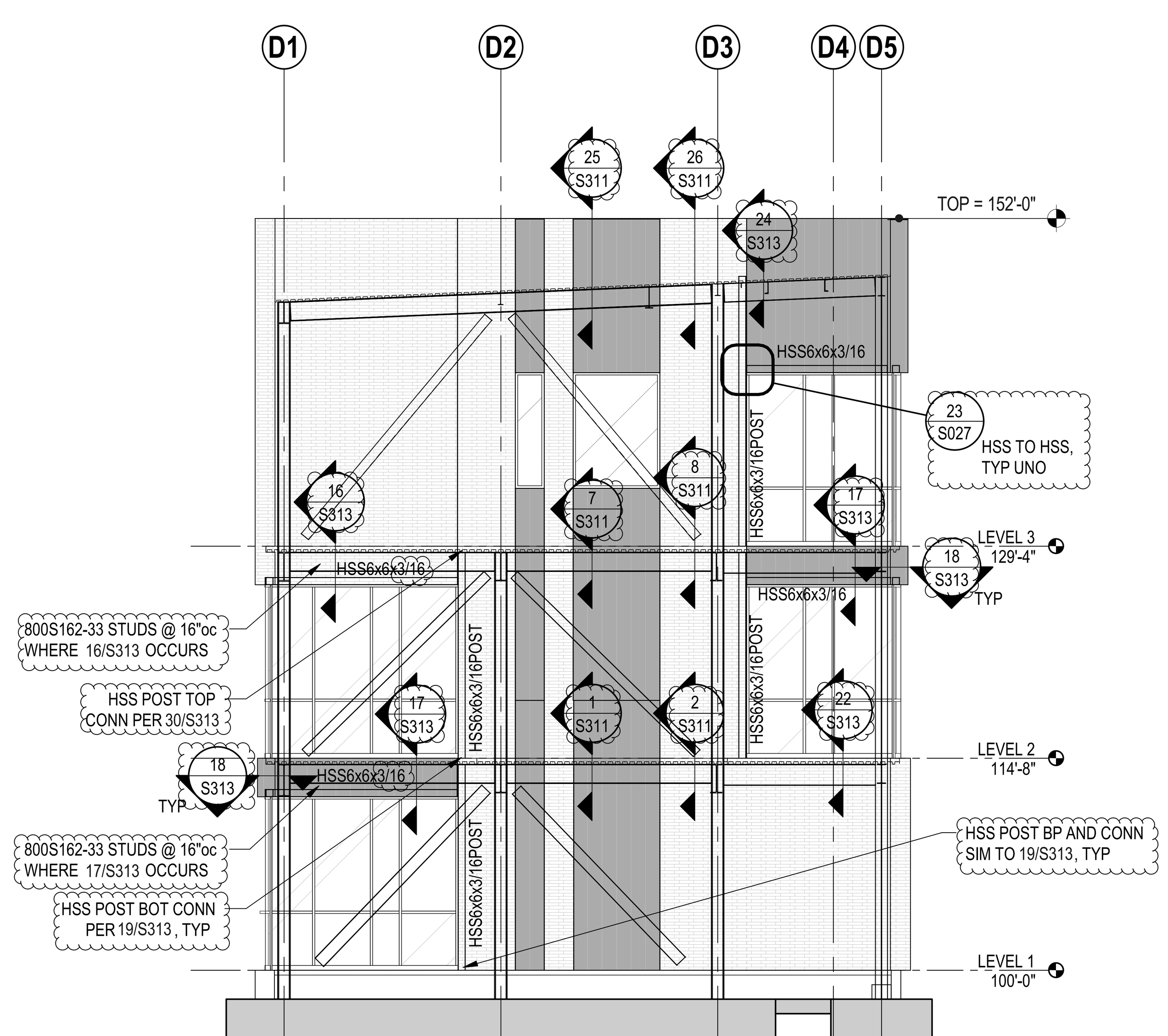
13 EXTERIOR WALL ELEVATION GRID D5

SCALE: 1/8" = 1'-0"

EXTERIOR METAL STUD WALL NOTES:

1. FOR TYPICAL METAL STUD DETAILS SEE SHEETS S023 & S024
2. WALL STUDS ARE 800S162-54 @ 16"oc, UNO
3. FOR TYPICAL MASONRY VENEER DETAILS SEE SHEET S022
4. PROVIDE LOOSE STEEL LINTEL ANGLES PER 5/S022 WHERE REQUIRED
5. SEE GENERAL NOTES FOR ADDITIONAL METAL STUD REQUIREMENTS
6. VERIFY ALL OPENING SIZES & LOCATIONS WITH ARCHITECTURAL DRAWINGS AND MEP SUBCONTRACTORS PRIOR TO CONSTRUCTION
7. TOP OF PARAPET (TOP) IS TOP OF METAL STUD FRAMING

PRCNC20221036
STRUCTURAL REVISION



14 EXTERIOR WALL ELEVATION GRID C16

SCALE: 1/8" = 1'-0"

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1601 39th AVE SE
Puyallup, WA 98374

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Revisions

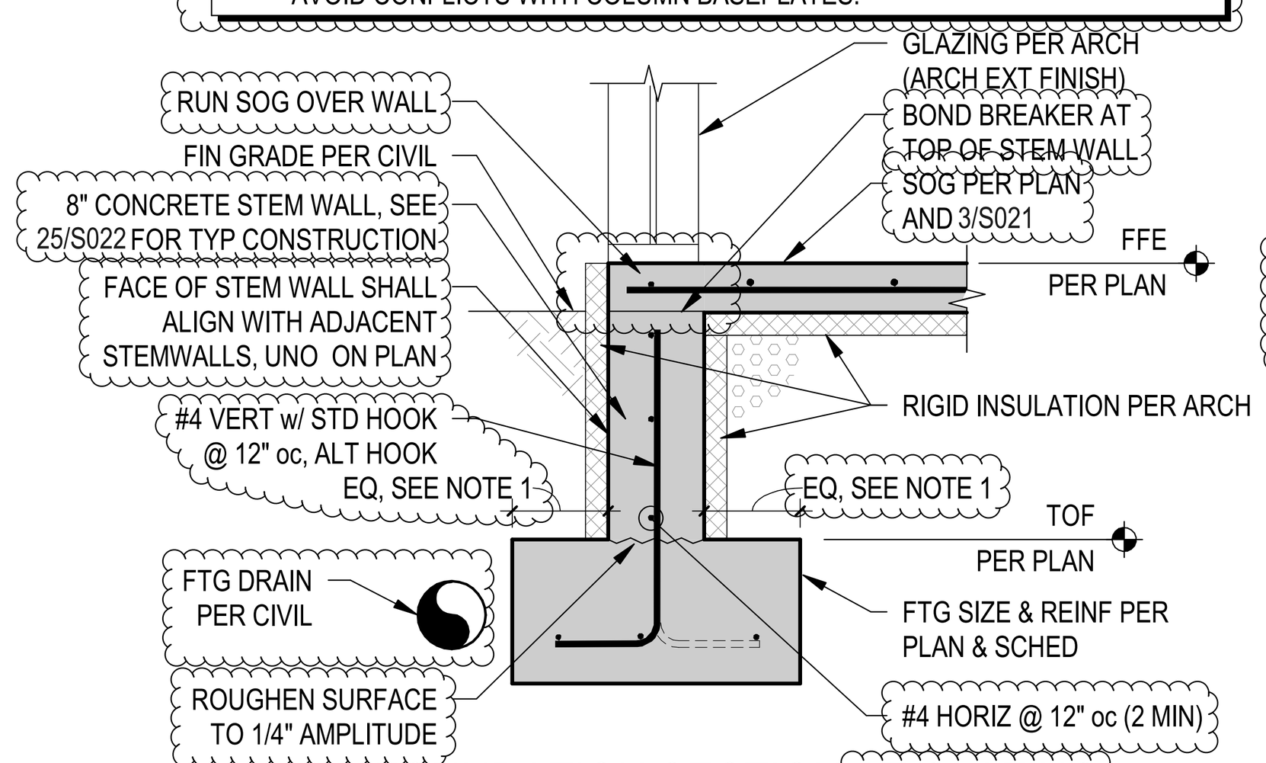
Date	Description
1.30.23	IFC Revisions

EXTERIOR WALL ELEVATIONS

S205

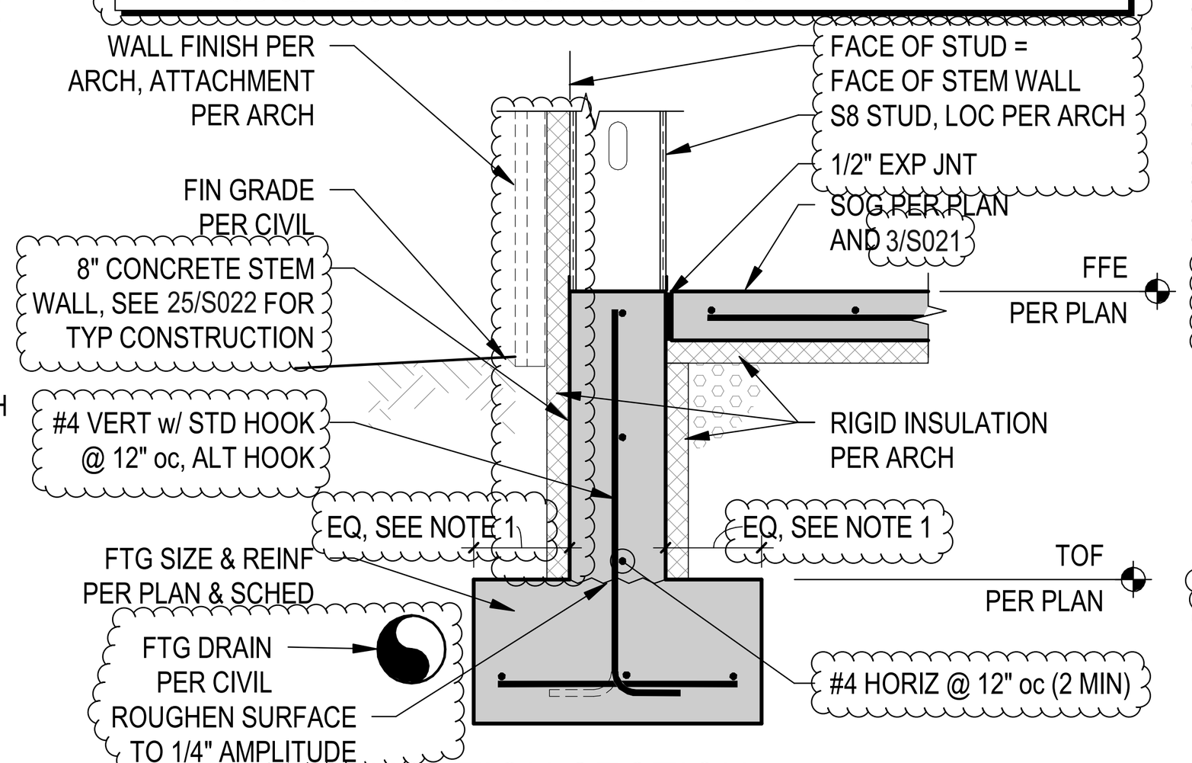
IFC SET

NOTES:
1. WHERE ARCH EXT WALL FINISH ALTERNATES BETWEEN VENEER AND OTHER FINISHES, LOCATION OF FOOTING IN RELATION TO STEM WALL SHALL BE DETERMINED BY DETAIL 3/S301 , EDGES OF FOOTINGS SHALL ALIGN.
2. ACCEPTABLE TO DRILL + EPOXY VERT DOWELS AT COLUMN POURBACKS TO AVOID CONFLICTS WITH COLUMN BASEPLATES.



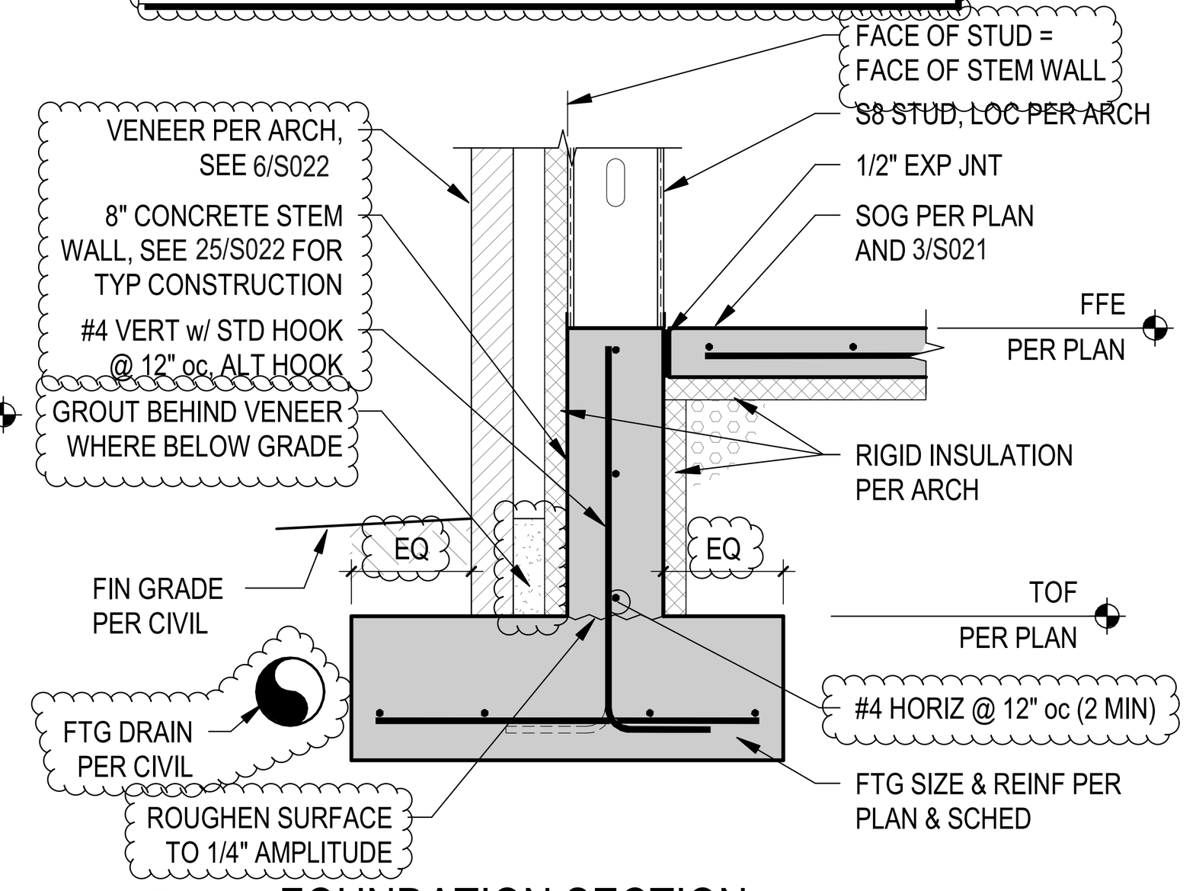
1 FOUNDATION SECTION AT GLAZING
SCALE: 3/4" = 1'-0"

NOTES:
1. WHERE ARCH EXT WALL FINISH ALTERNATES BETWEEN VENEER AND OTHER FINISHES, LOCATION OF FOOTING IN RELATION TO STEM WALL SHALL BE DETERMINED BY DETAIL 3/S301 , EDGES OF FOOTINGS SHALL ALIGN.
2. ACCEPTABLE TO DRILL + EPOXY VERT DOWELS AT COLUMN POURBACKS TO AVOID CONFLICTS WITH COLUMN BASEPLATES.

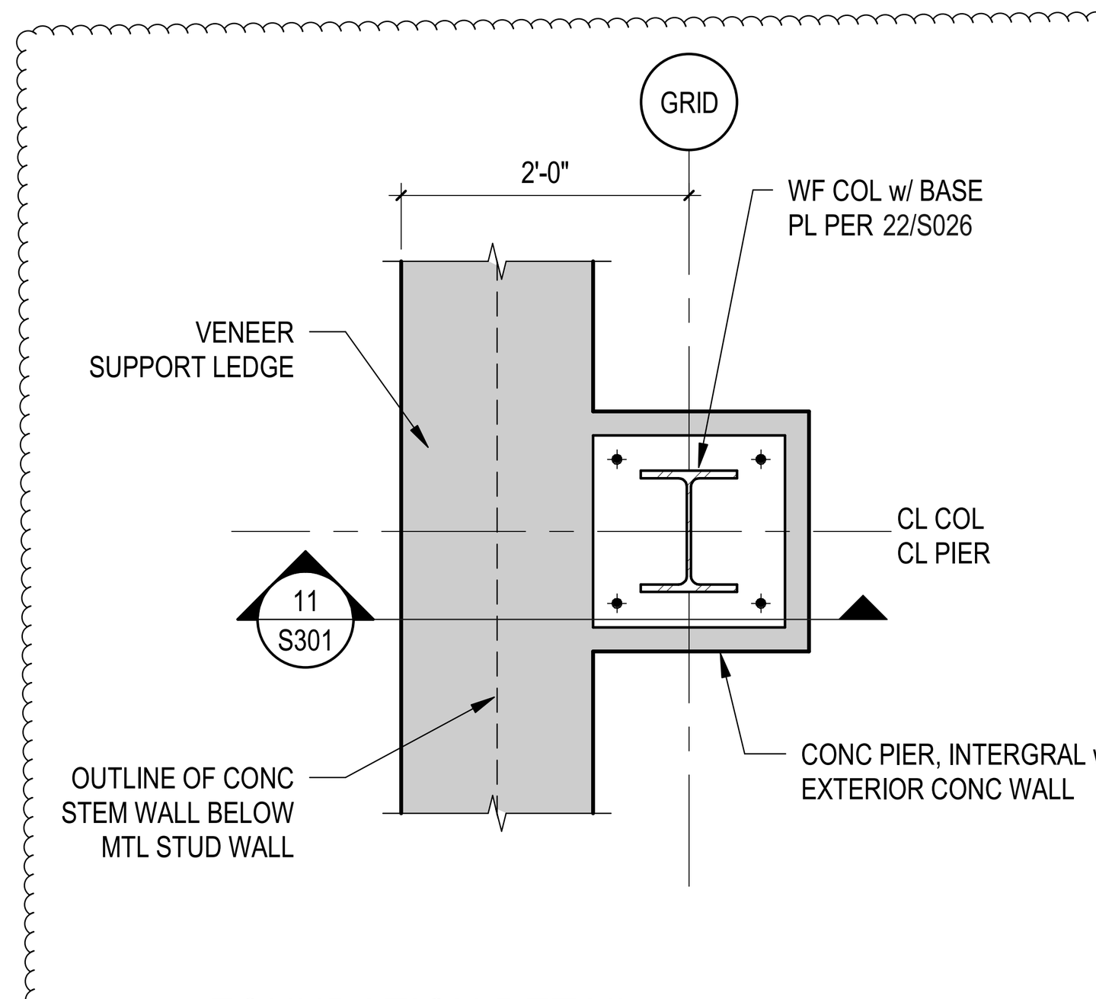


2 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

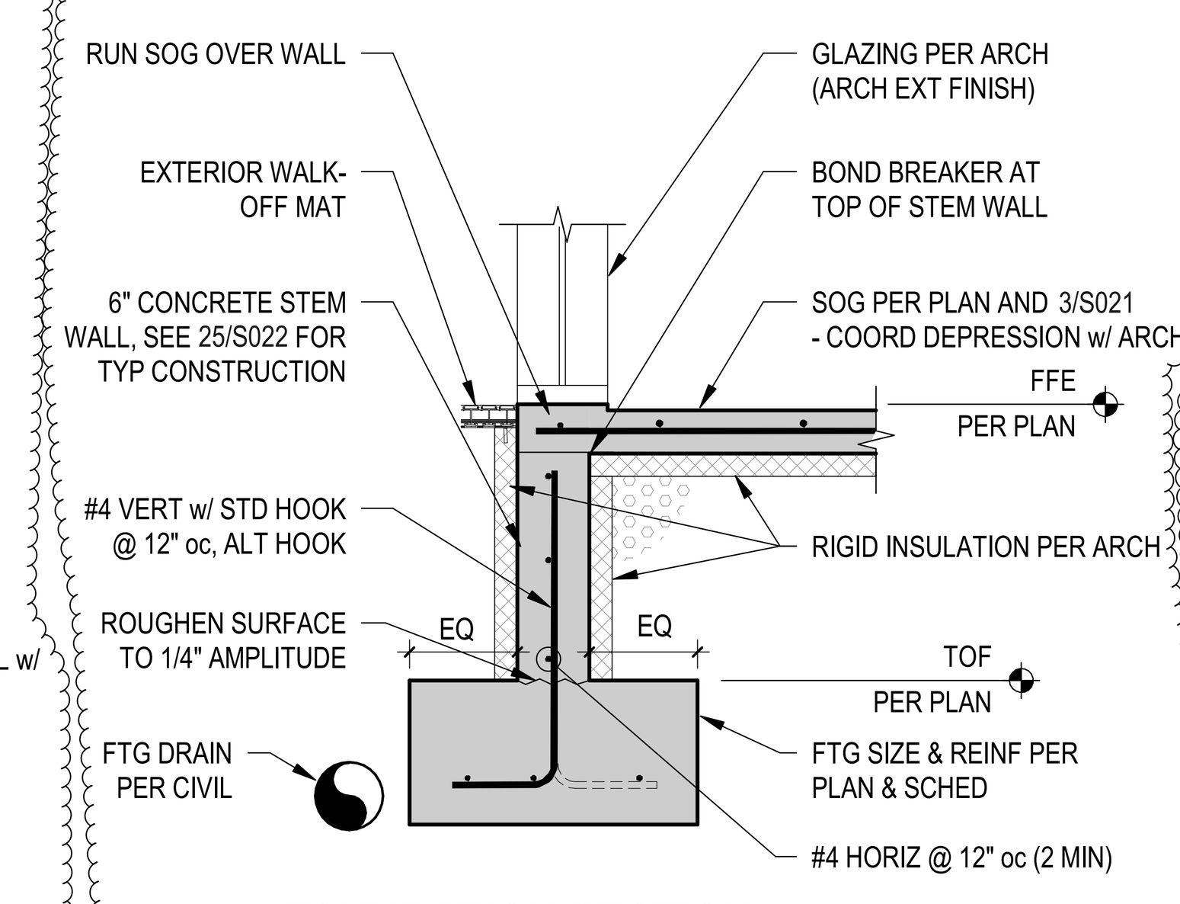
NOTE: ACCEPTABLE TO DRILL + EPOXY VERT DOWELS AT COLUMN POURBACKS TO AVOID CONFLICTS WITH COLUMN BASEPLATES.



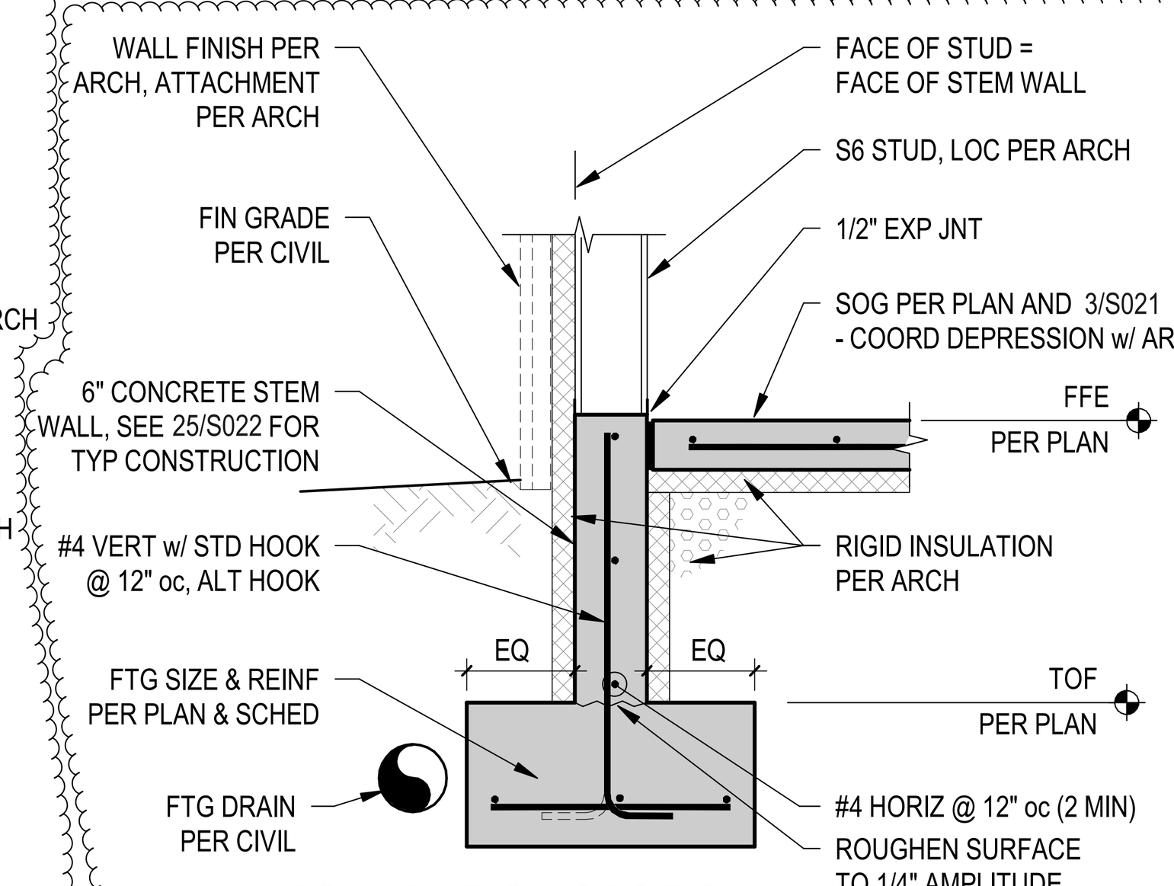
3 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



5 FOUNDATION DETAIL
SCALE: 3/4" = 1'-0"

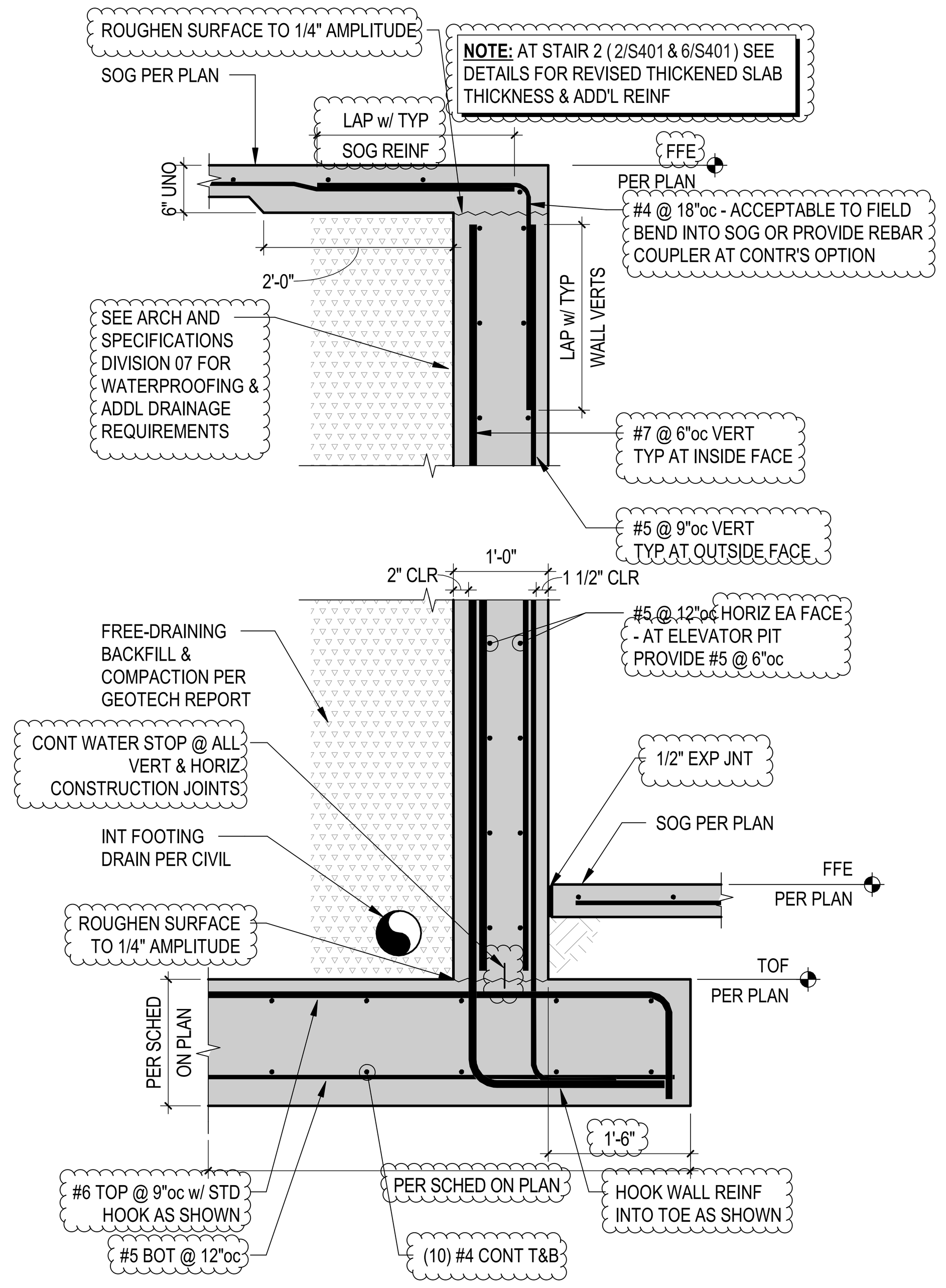


6 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



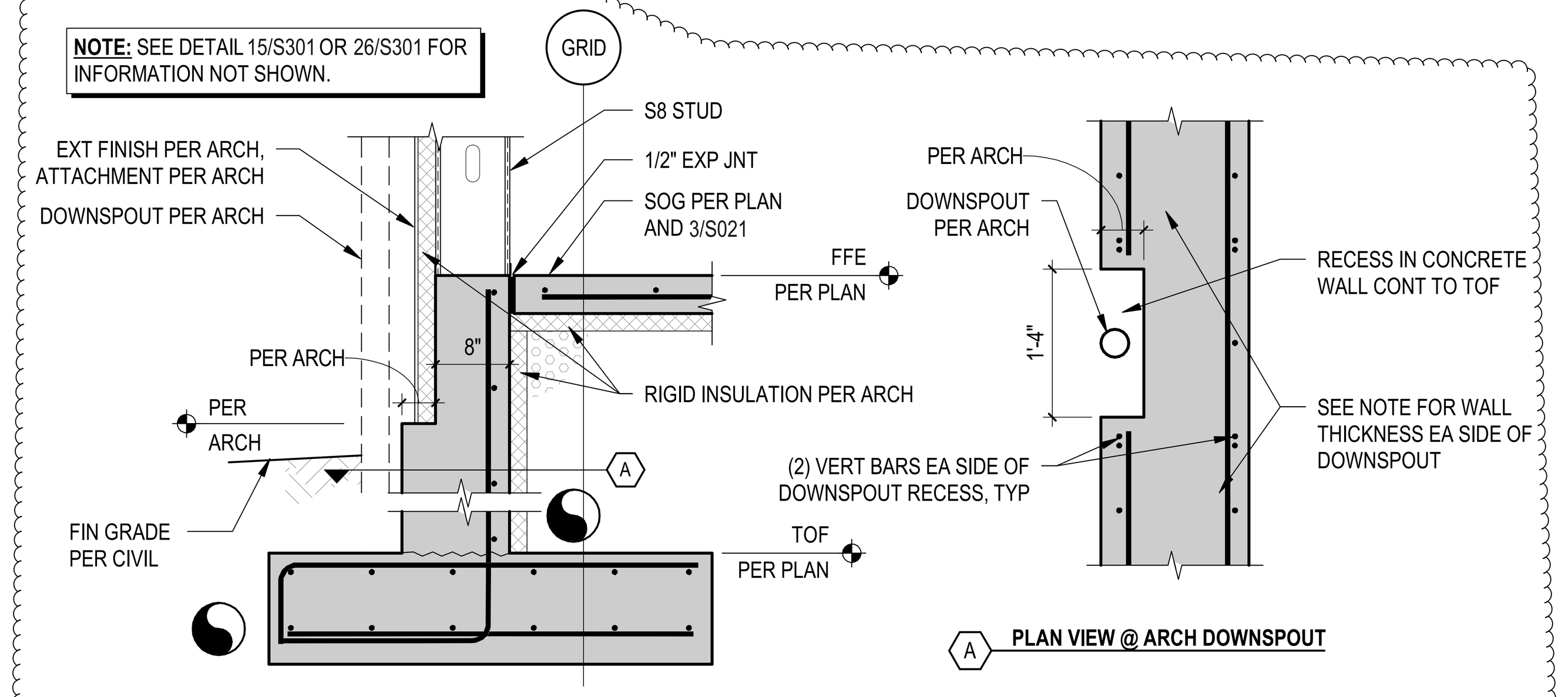
7 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

NOTE: AT STAIR 2 (2/S401 & 6/S401) SEE DETAILS FOR REVISED THICKENED SLAB THICKNESS & ADD'L REINF

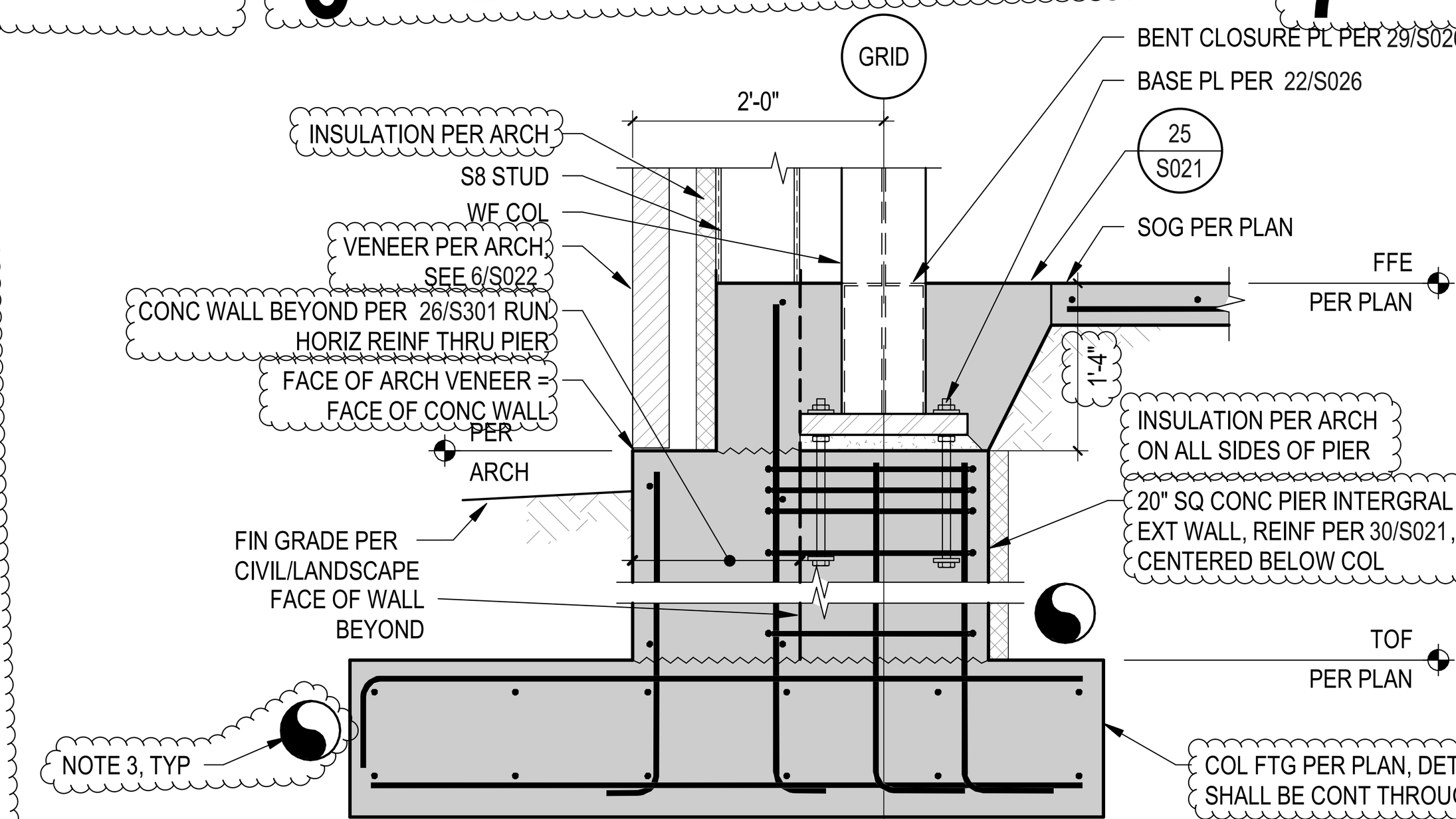


13 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

NOTE: SEE DETAIL 15/S301 OR 26/S301 FOR INFORMATION NOT SHOWN.



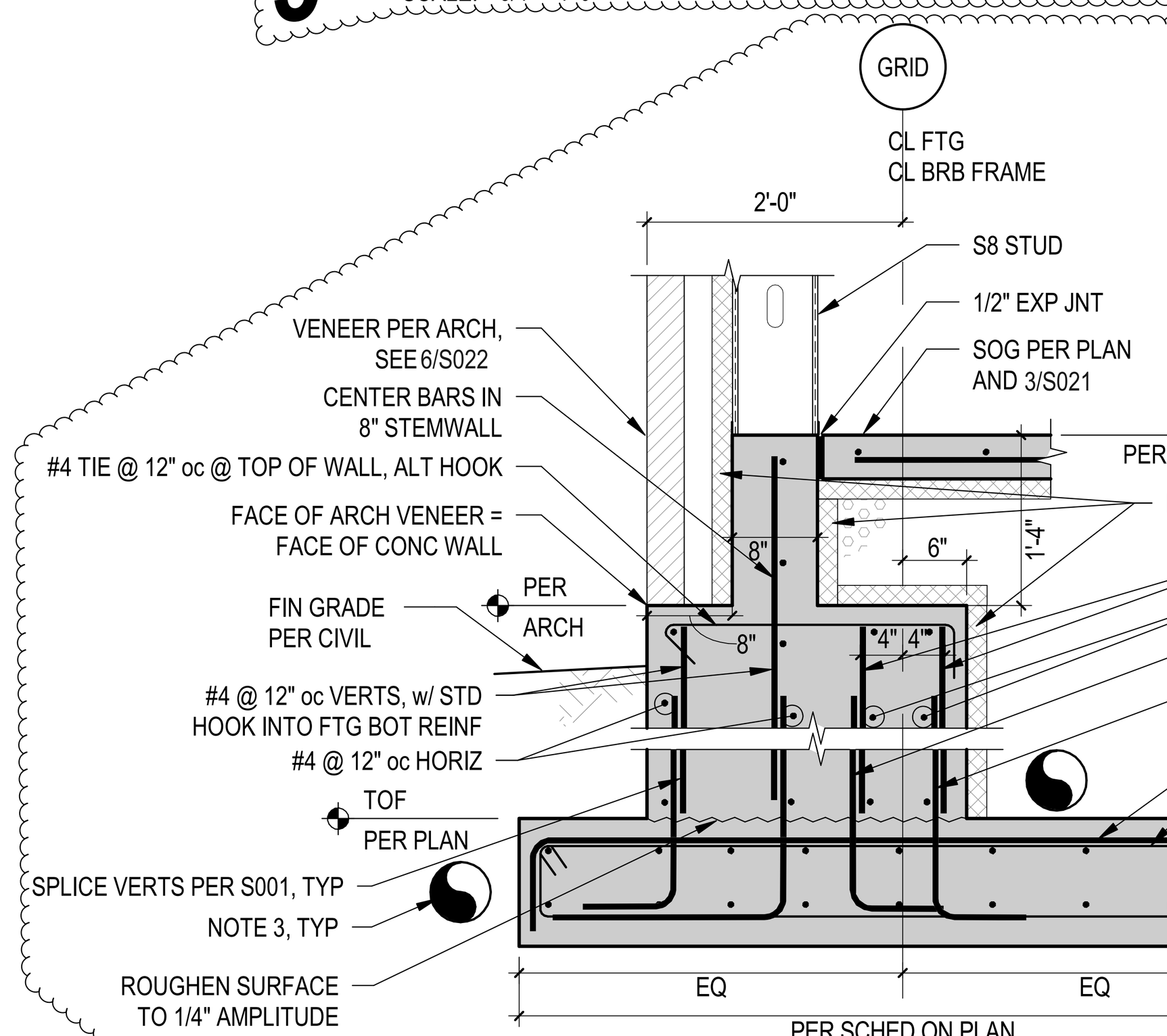
9 DOWNSPOUT WALL RECESS
SCALE: 3/4" = 1'-0"



11 EXT FOUNDATION SECTION AT PIER
SCALE: 3/4" = 1'-0"

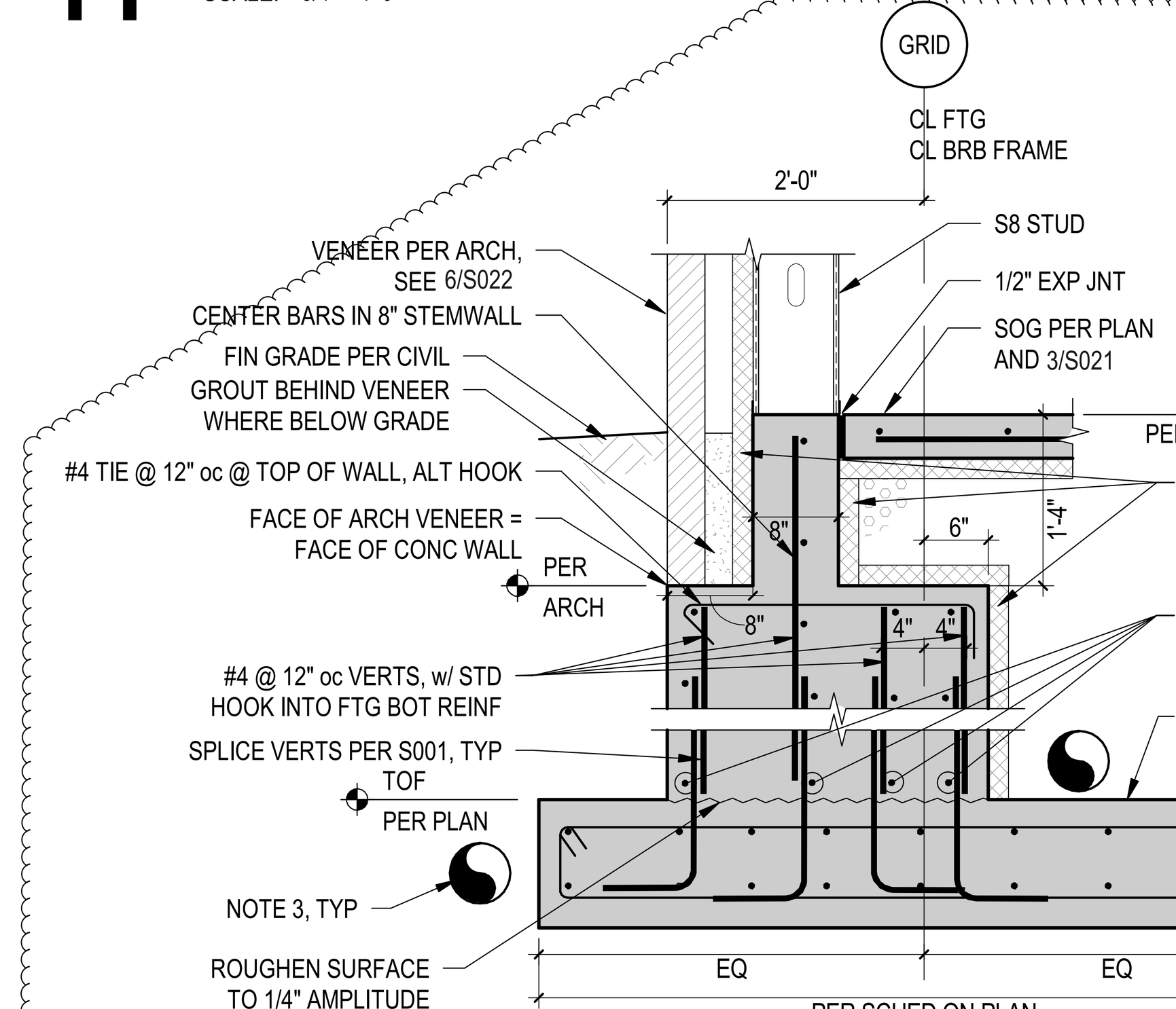
NOTES:
1. CONCRETE WALL TO REMAIN THE SAME PROFILE AT OTHER ARCH WALL EXT FINISHES.
2. SEE DETAIL 25/S022 FOR TYPICAL CONCRETE WALL CONSTRUCTION.
3. INTERIOR AND EXTERIOR FOOTING DRAIN PER CIVIL, SEE DETAIL 1/S021.
4. BACKFILL WALL AS REQUIRED BY THE GEOTECHNICAL ENGINEER.
5. PIERS AT BRB COLUMNS TO BE INTERGRAL WITH WALL SIMILAR TO THIS DETAIL. SEE DETAIL 15/S301 FOR CONCRETE WALL INFO BETWEEN BRB COLUMN PIERS.

NOTES:
1. CONCRETE WALL TO REMAIN THE SAME PROFILE AT OTHER ARCH WALL EXTERIOR FINISHES.
2. SEE DETAIL 25/S022 FOR TYPICAL CONCRETE WALL CONSTRUCTION.
3. INTERIOR AND EXTERIOR FOOTING DRAIN PER CIVIL, SEE DETAIL 1/S021.
4. BACKFILL WALL AS REQUIRED BY THE GEOTECHNICAL ENGINEER.

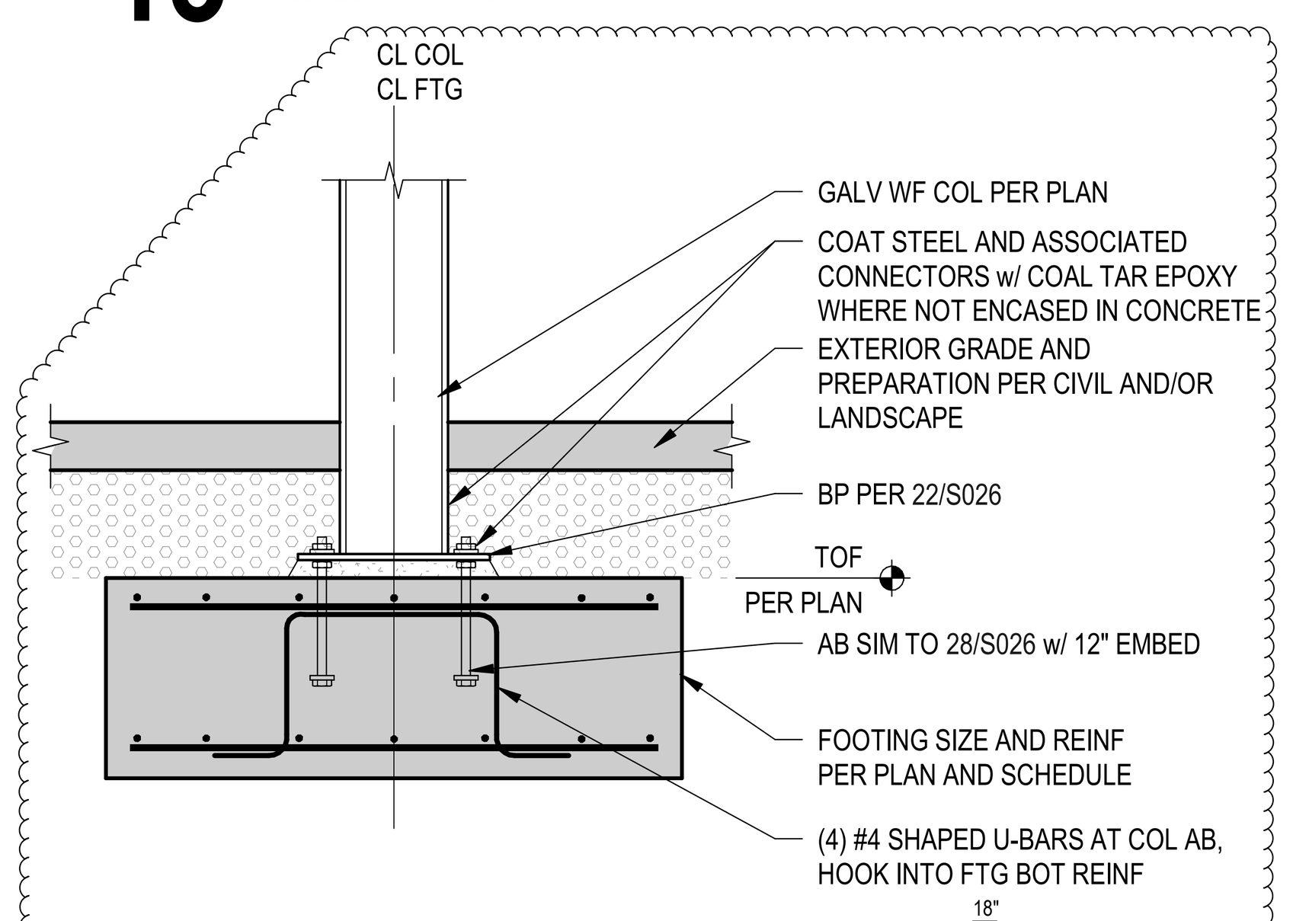


15 EXT FOUNDATION SECTION BTWN BRB COL
SCALE: 3/4" = 1'-0"

NOTES:
1. CONCRETE WALL TO REMAIN THE SAME PROFILE AT OTHER ARCH WALL EXTERIOR FINISHES.
2. SEE DETAIL 25/S022 FOR TYPICAL CONCRETE WALL CONSTRUCTION.
3. INTERIOR AND EXTERIOR FOOTING DRAIN PER CIVIL, SEE DETAIL 1/S021.



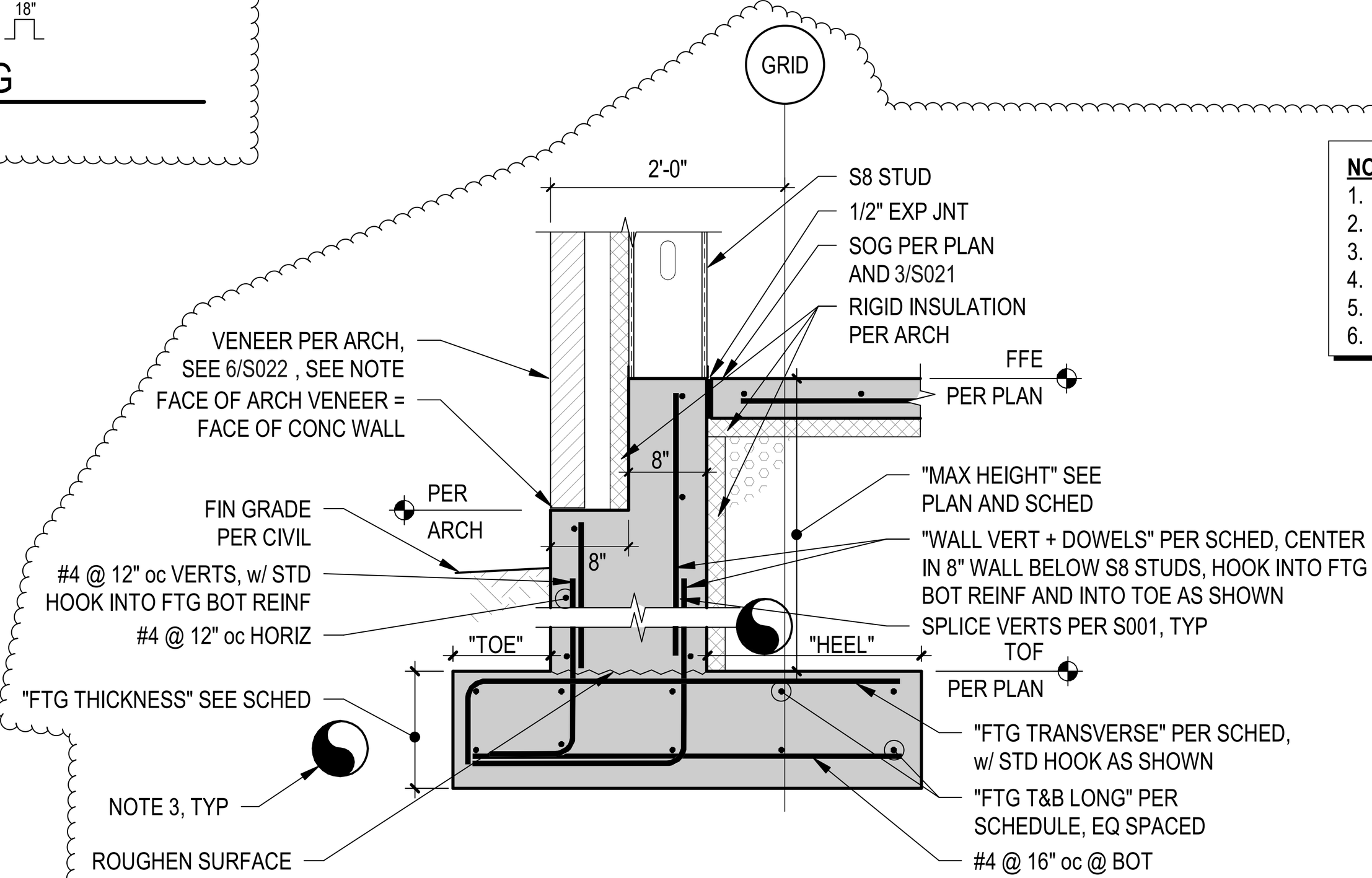
23 BF5 EXT FOUNDATION SECTION BTWN COL
SCALE: 3/4" = 1'-0"



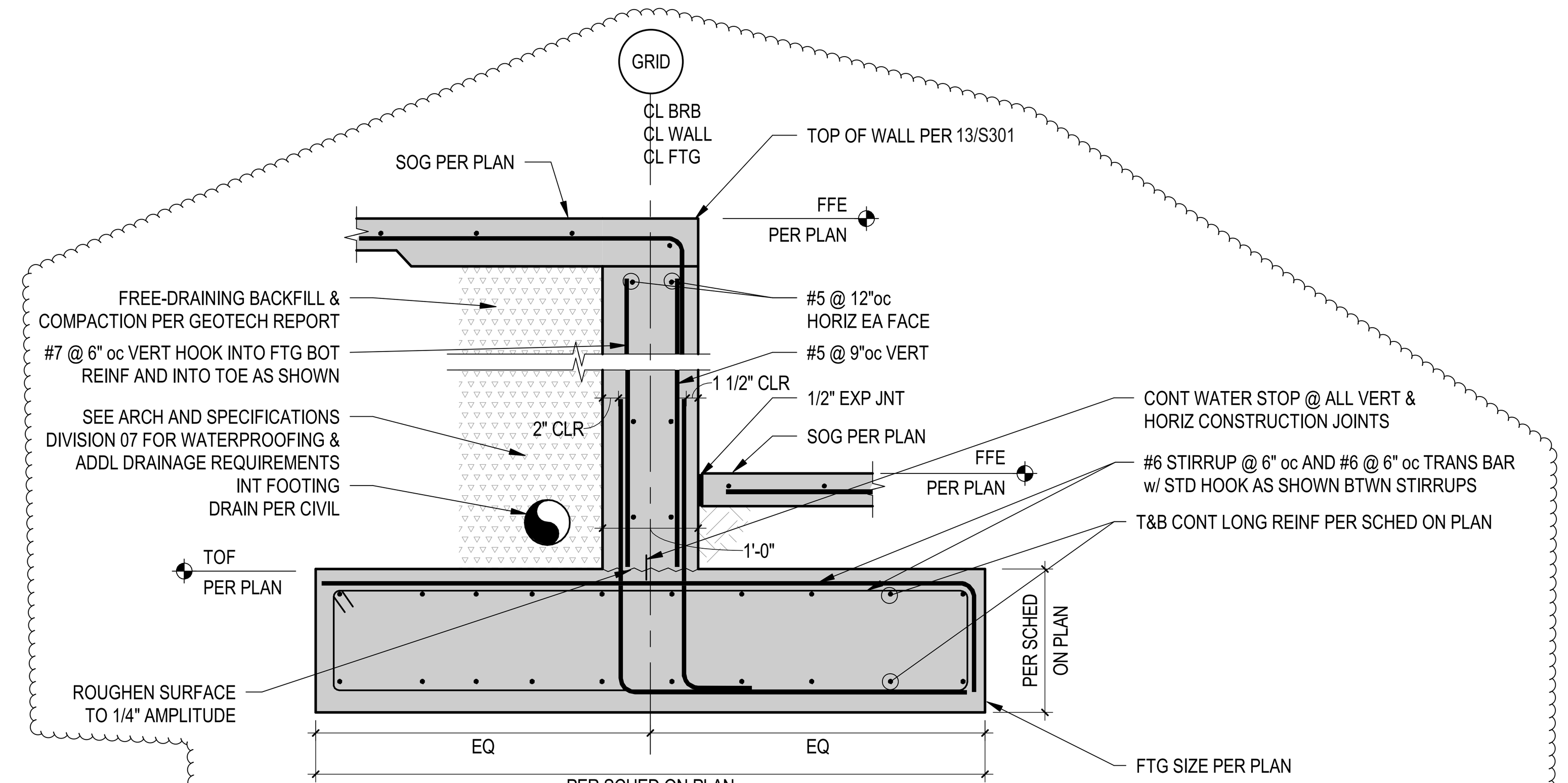
19 FRONT ENTRY COLUMN FOOTING
SCALE: 3/4" = 1'-0"

NOTES:
1. CONCRETE WALL TO REMAIN THE SAME PROFILE AT OTHER ARCH WALL EXTERIOR FINISHES.
2. SEE DETAIL 25/S022 FOR TYPICAL CONCRETE WALL CONSTRUCTION.
3. INTERIOR AND EXTERIOR FOOTING DRAIN PER CIVIL, SEE DETAIL 1/S021.
4. BACKFILL WALL AS REQUIRED BY THE GEOTECHNICAL ENGINEER.
5. AT COLUMNS, SEE DETAIL 11/S301 .
6. WALL BETWEEN BRB COLUMNS PER 15/S301 .

BUILDING EXTERIOR RETAINING WALL SCHEDULE						
MAX HEIGHT	TOE	HEEL	FTG THICKNESS	WALL VERT + DOWELS	FTG TRANSVERSE	FTG T&B LONG
UP TO 4'-0"	1'-6"	2'-0"	1'-0"	#5 @ 6" oc	#6 @ 12" oc	(5) #4 CONT T&B
UP TO 6'-0"	1'-6"	3'-8"	1'-0"	#5 @ 6" oc	#6 @ 12" oc	(7) #4 CONT T&B
UP TO 8'-0"	1'-6"	4'-8"	1'-0"	#5 @ 6" oc	#6 @ 12" oc	(8) #4 CONT T&B
UP TO 9'-0"	1'-6"	5'-8"	1'-4"	#6 @ 6" oc	#6 @ 9" oc	(9) #4 CONT T&B
UP TO 11'-0"	2'-6"	5'-8"	1'-4"	#7 @ 6" oc	#6 @ 9" oc	(10) #4 CONT T&B



26 EXT FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



29 BRACE FRAME 6 WALL SECTION
SCALE: 3/4" = 1'-0"

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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

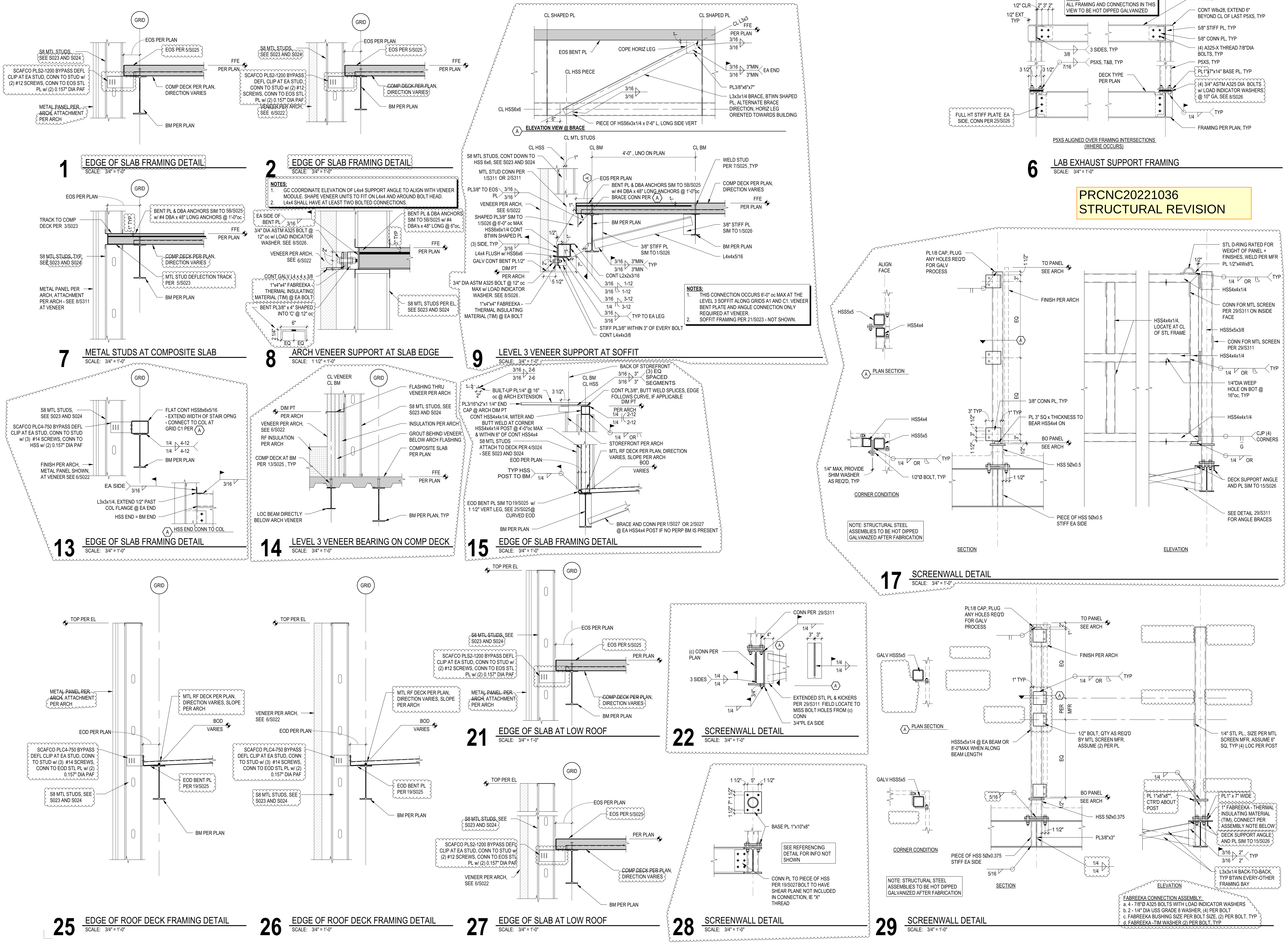
Date: 10/07/2022
Job No.: 22135.00
Drawn By: RB/BLO/AM
Checked By: TD

Revisions	
Date	Description
1.30.23	IFC Revisions

FOUNDATION DETAILS & SECTIONS

S301

IFC SET



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Date:	10/07/2022
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Checked By:	TD

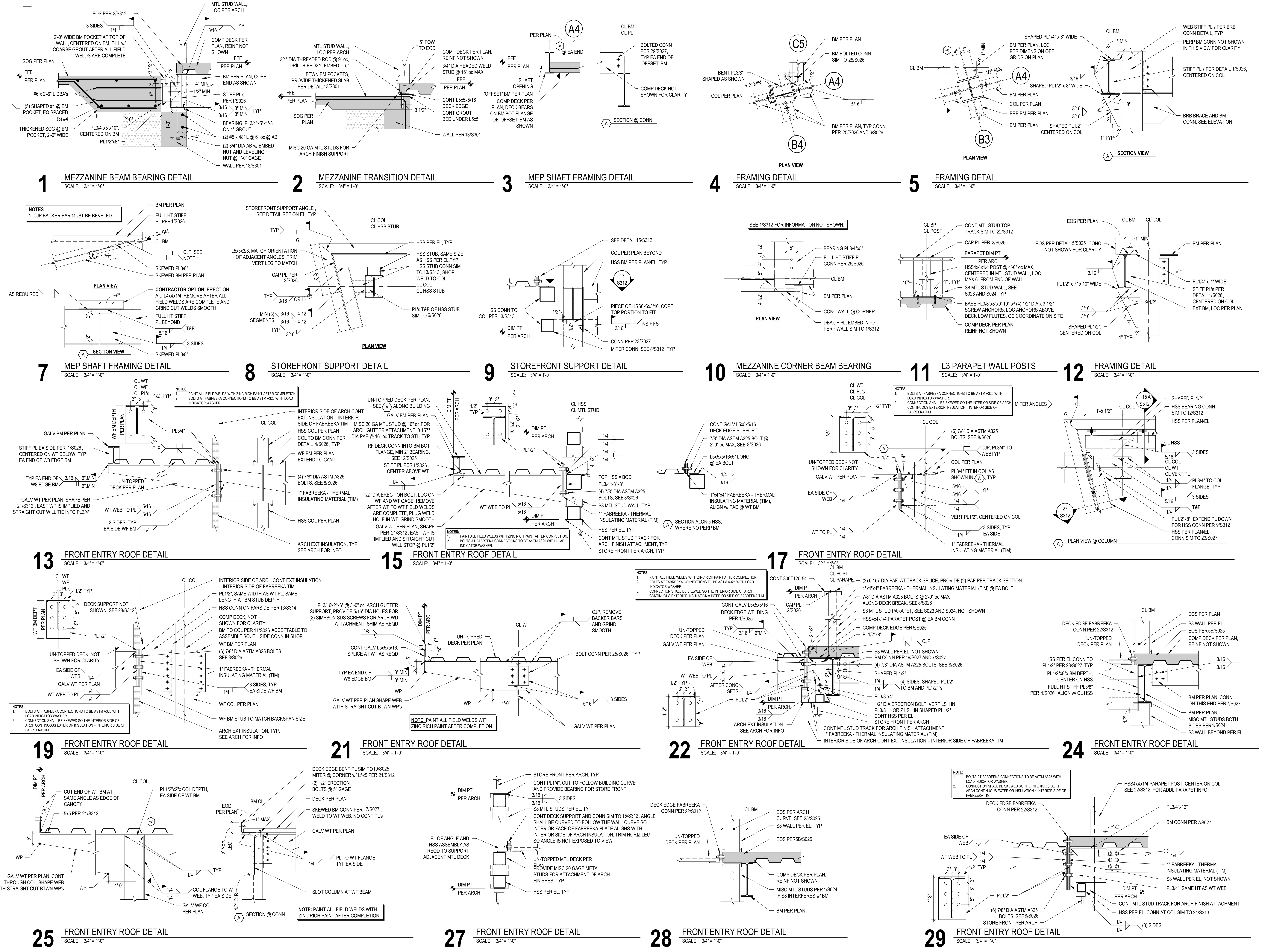
Revisions

Date	Description
1.30.23	IFC Revisions

FRAMING DETAILS & SECTIONS

S311

IFC SET



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City of Puyallup
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Checked by:	TD

Revisions

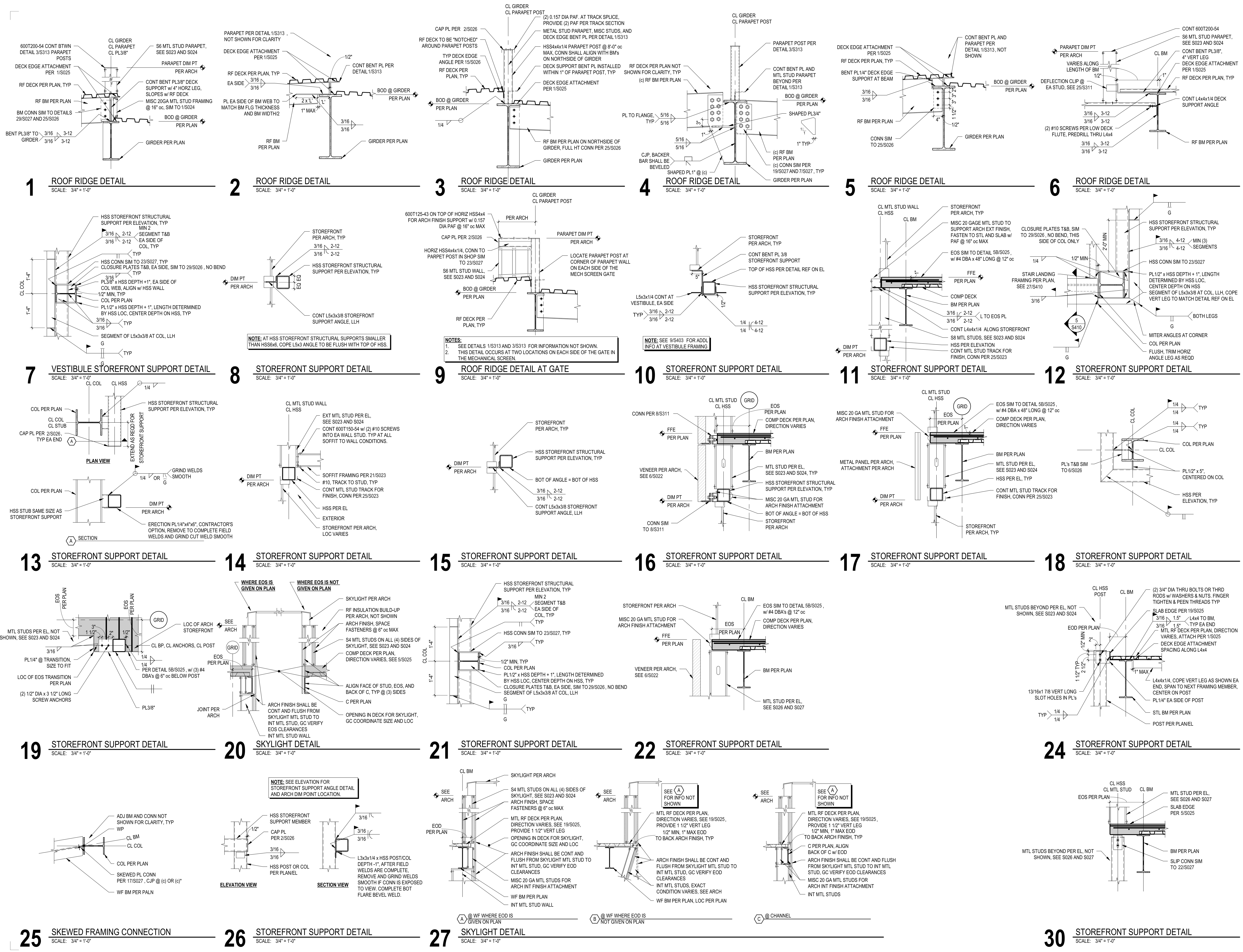
Date	Description
1.30.23	IFC Revisions

FRAMING DETAILS & SECTIONS

S312

PRCNC20221036
STRUCTURAL REVISION

IFC SET



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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

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Revisions

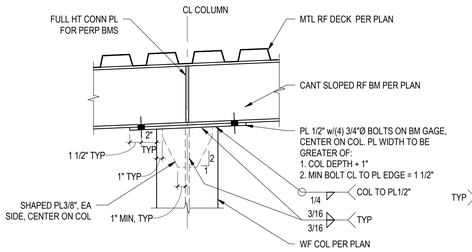
Date	Description
1.30.23	IFC Revisions

FRAMING DETAILS & SECTIONS

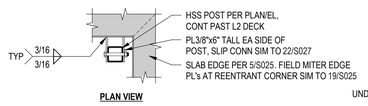
S313

PRCNC20221036
STRUCTURAL REVISION

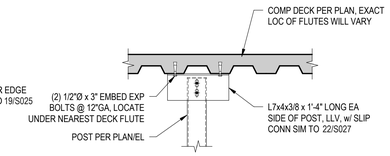
IFC SET



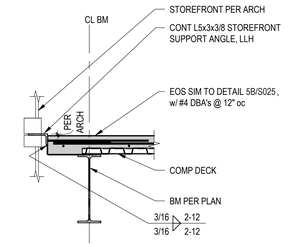
1 CANTILEVER OVER COLUMN
SCALE: 1 1/2" = 1'-0"



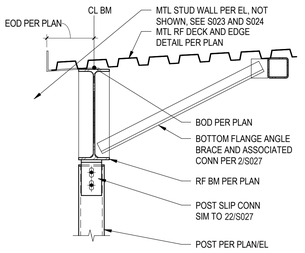
2 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



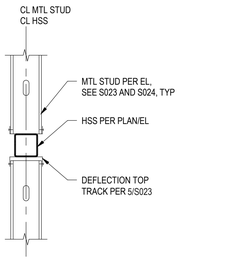
3 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



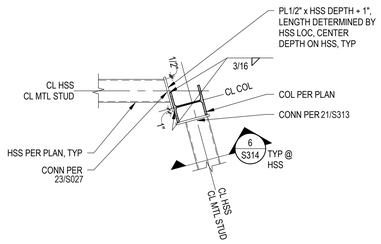
4 EDGE CLOSURE AT STOREFRONT
SCALE: 3/4" = 1'-0"



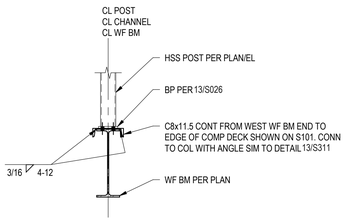
5 STOREFRONT POST CONNECTION
SCALE: 1" = 1'-0"



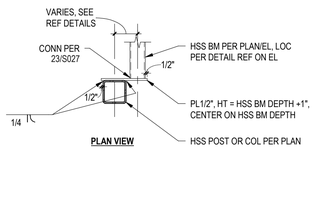
6 EXTERIOR MTL STUDS AT HSS
SCALE: 3/4" = 1'-0"



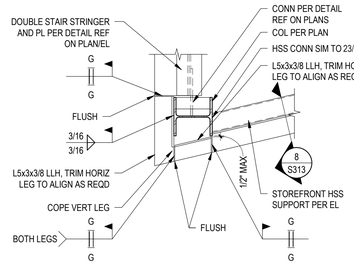
7 HSS CONNECTION AT COLUMN
SCALE: 3/4" = 1'-0"



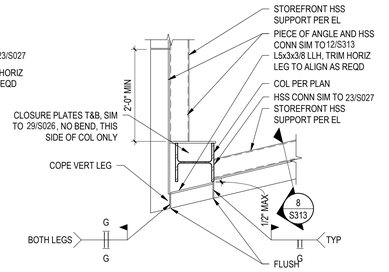
8 POST CONN AT BEAM
SCALE: 3/4" = 1'-0"



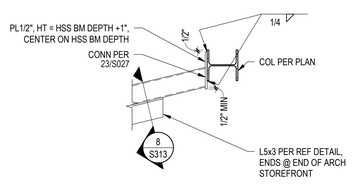
9 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



10 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



11 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



12 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"

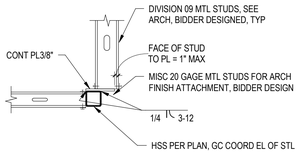
13 STOREFRONT SUPPORT DETAIL
SCALE: 3/4" = 1'-0"

14 SKYLIGHT SOFFIT SUPPORT
SCALE: 1" = 1'-0"

15 DECK CLOSURE AT CANTILEVER BM
SCALE: 3/4" = 1'-0"

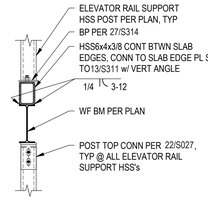
NOTE: NOT ALL METAL STUD FRAMING IS SHOWN. SEE ELEVATIONS, S023, AND S024 FOR MORE INFOMATION.

18 STOREFRONT REENTRANT CORNER FRAMING
SCALE: 3/4" = 1'-0"

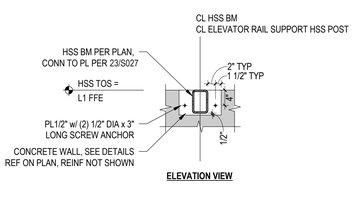


19 DIVISION 09 MTL STUD SOFFIT SUPPORT
SCALE: 3/4" = 1'-0"

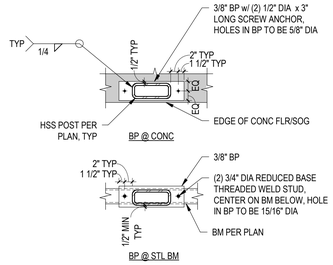
PRCNC20221036
STRUCTURAL REVISION



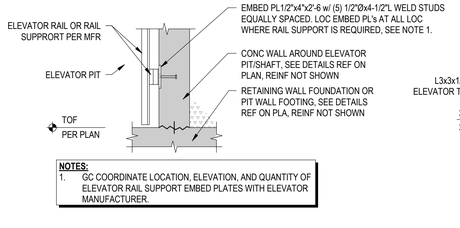
25 ELEVATOR BEAM BETWEEN SHAFTS
SCALE: 3/4" = 1'-0"



26 ELEVATOR CONN DETAIL
SCALE: 3/4" = 1'-0"

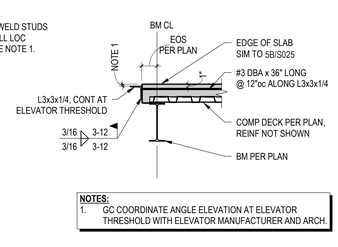


27 ELEVATOR POST BASEPLATE DETAIL
SCALE: 3/4" = 1'-0"



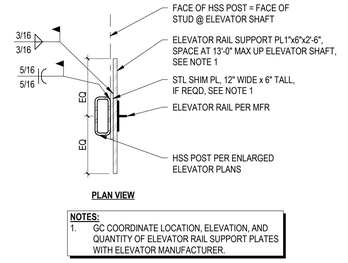
NOTES:
1. GC COORDINATE LOCATION, ELEVATION, AND QUANTITY OF ELEVATOR RAIL SUPPORT EMBED PLATES WITH ELEVATOR MANUFACTURER.

28 ELEVATOR RAIL SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



NOTES:
1. GC COORDINATE ANGLE ELEVATION AT ELEVATOR THRESHOLD WITH ELEVATOR MANUFACTURER AND ARCH.

29 ELEVATOR THRESHOLD AT FLOORS
SCALE: 3/4" = 1'-0"



NOTES:
1. GC COORDINATE LOCATION, ELEVATION, AND QUANTITY OF ELEVATOR RAIL SUPPORT PLATES WITH ELEVATOR MANUFACTURER.

30 ELEVATOR RAIL SUPPORT DETAIL
SCALE: 3/4" = 1'-0"

IFC SET

integrus ARCHITECTURE
117 SOUTH MAIN STREET, SUITE 100, SEATTLE, WA 98104
TELEPHONE: (206)623-3173 FAX: (206)623-3174

THOMAS H. DANIEL
REGISTERED PROFESSIONAL ENGINEER
38621
STATE OF WASHINGTON

City of Puyallup
Development & Permitting Services
ISSUED PERMIT

Building	Planning
Engineering	Public Works
Fire	Traffic

PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

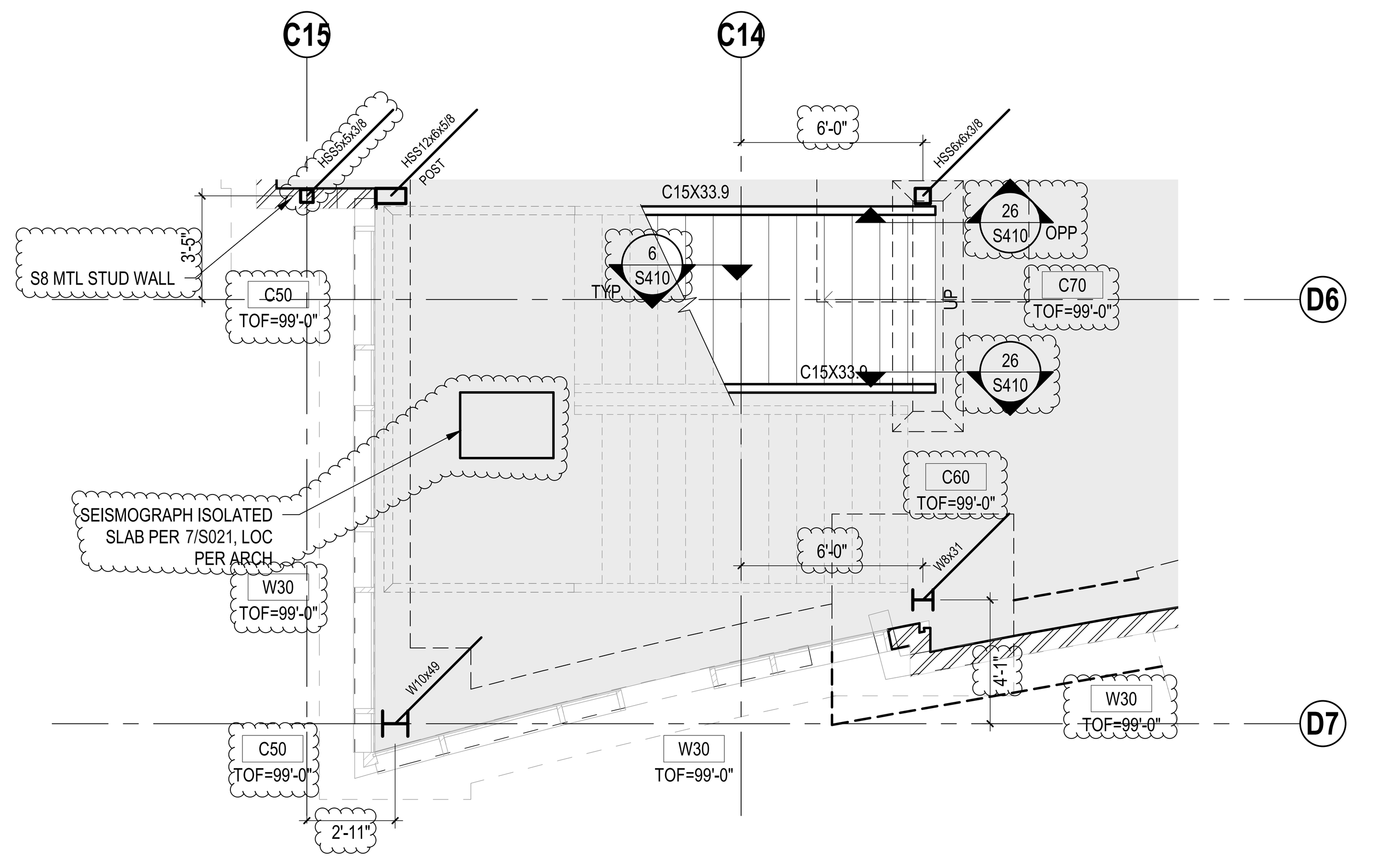
Date:	10/07/2022
Job No.:	22135.00
Drawn By:	AM
Checked by:	TD

Revisions

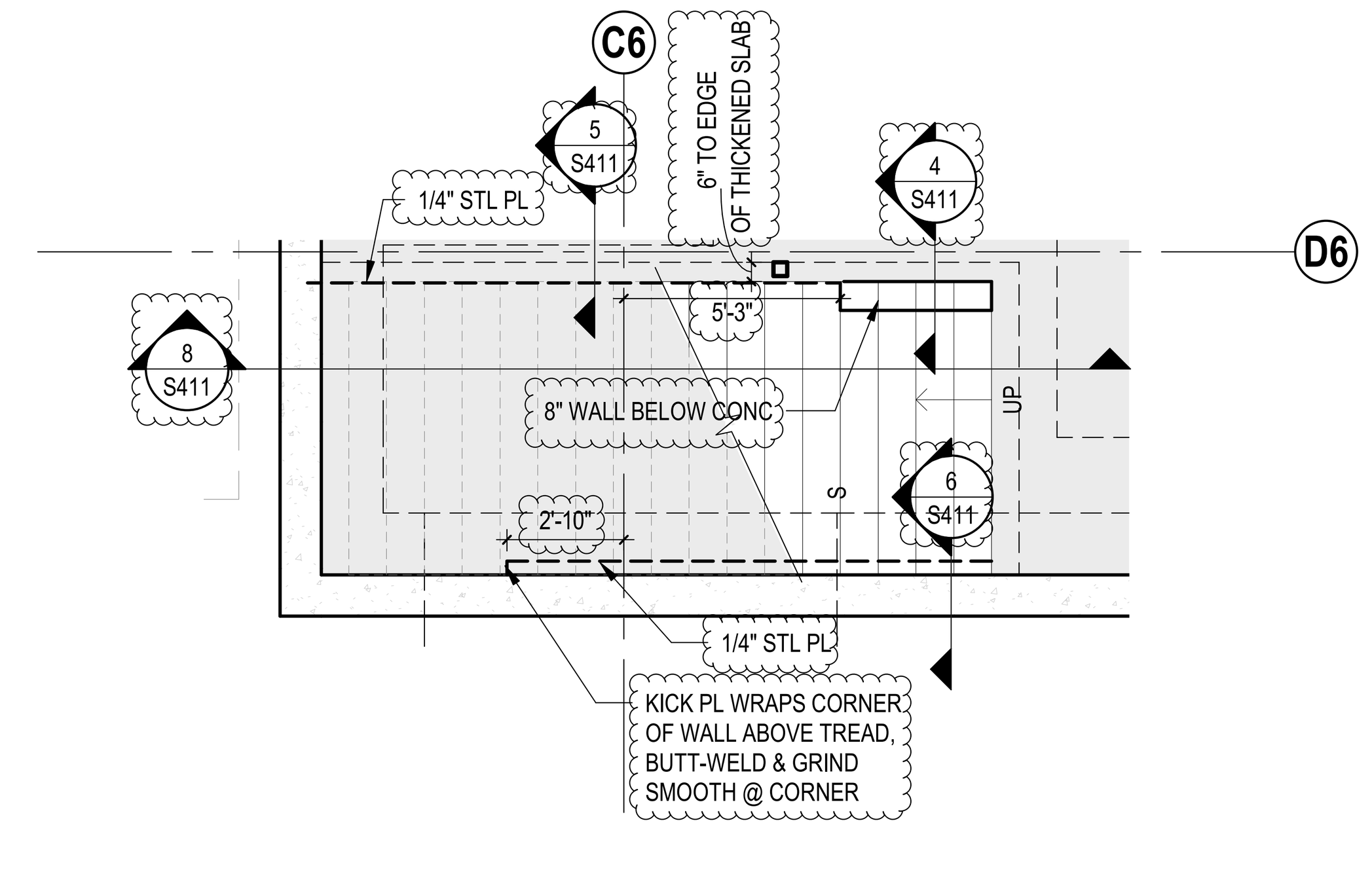
Date	Description
1.30.23	IFC Revisions

FRAMING DETAILS AND SECTIONS

S314

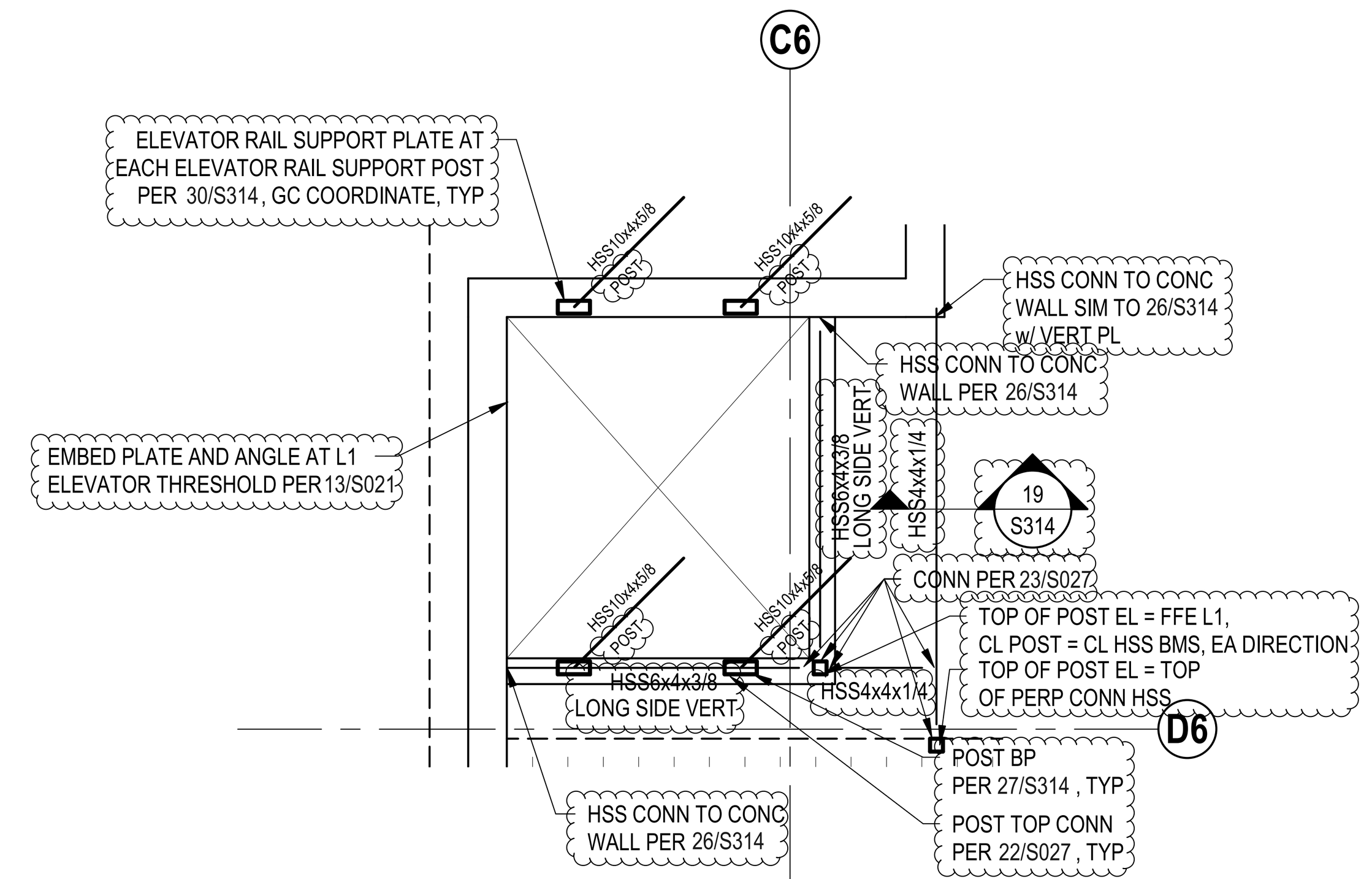


1 ENLARGED PLAN - STAIR 1-LEVEL 1
SCALE: 1/4" = 1'-0"

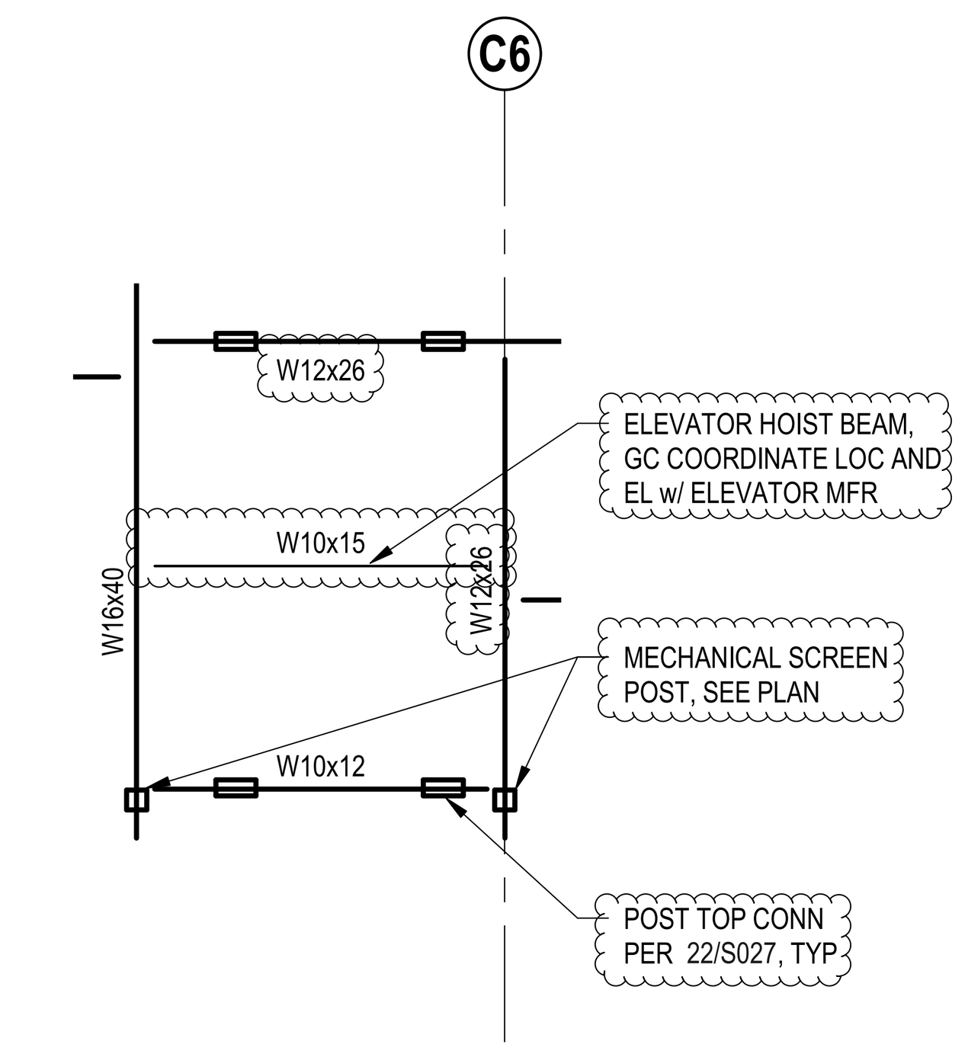


2 ENLARGED PLAN - LOBBY STAIR - LEVEL 0
SCALE: 1/4" = 1'-0"

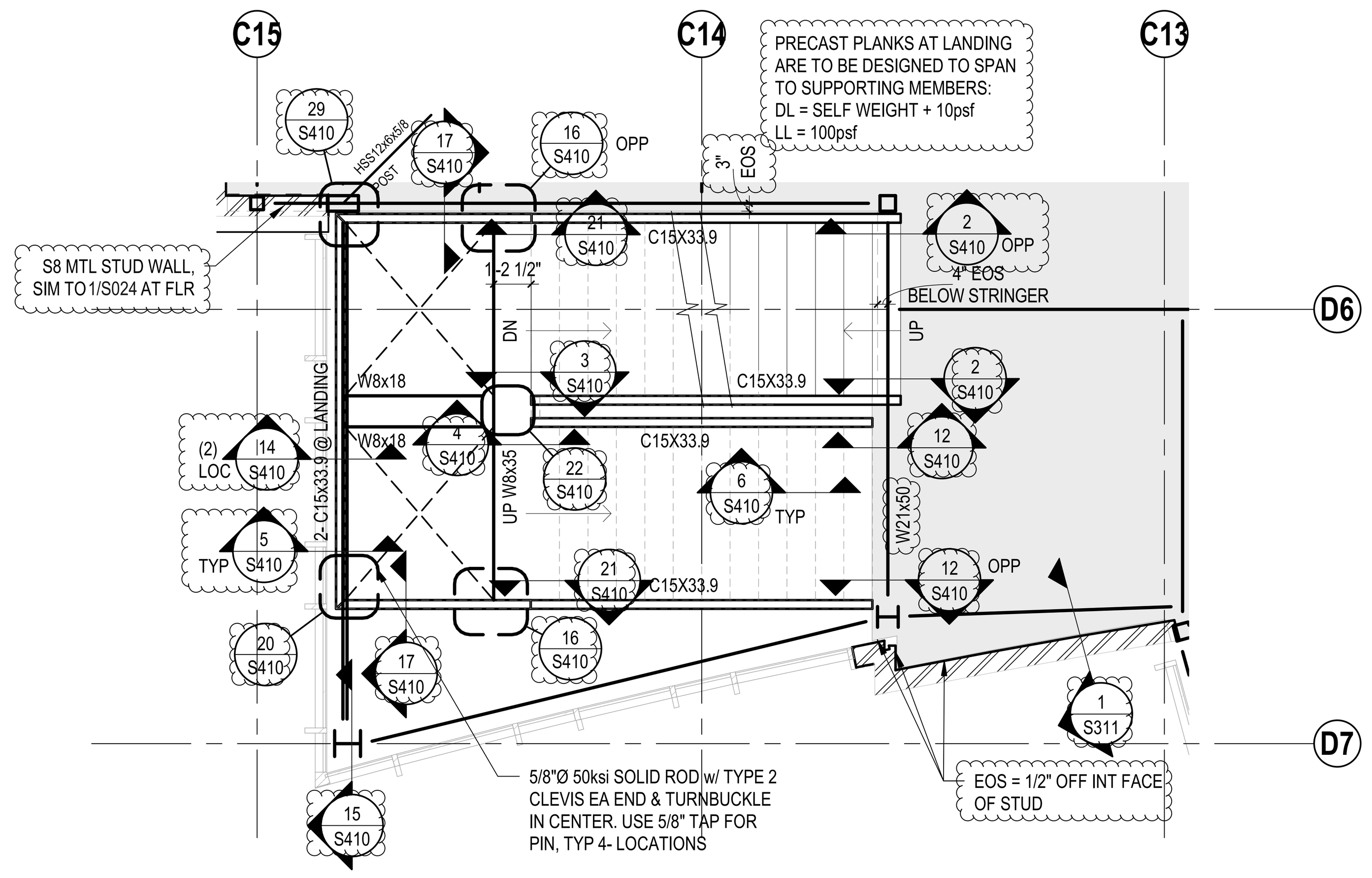
9 S202



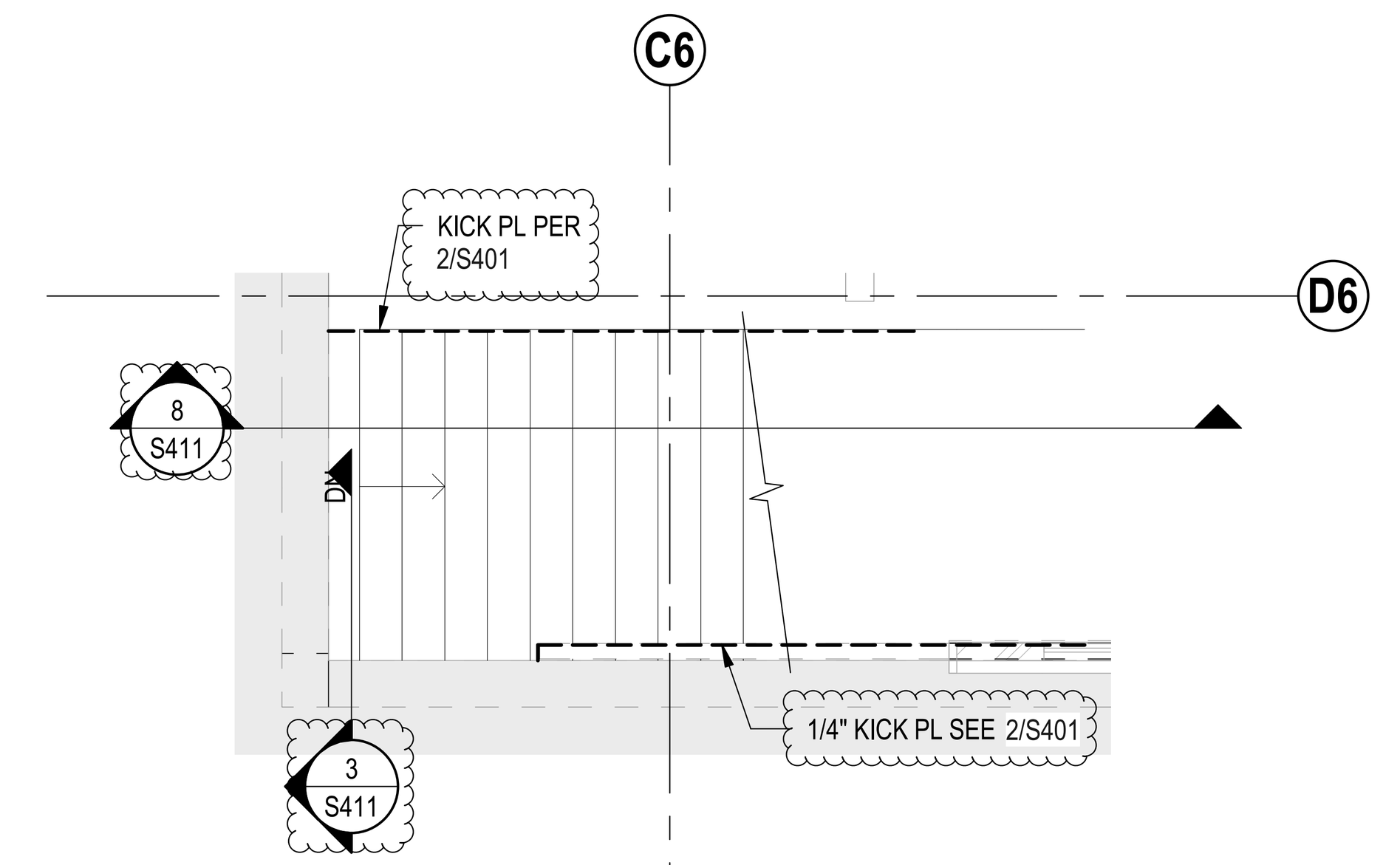
3 ENLARGED PLAN - ELEVATOR - LEVEL 1
SCALE: 1/4" = 1'-0"



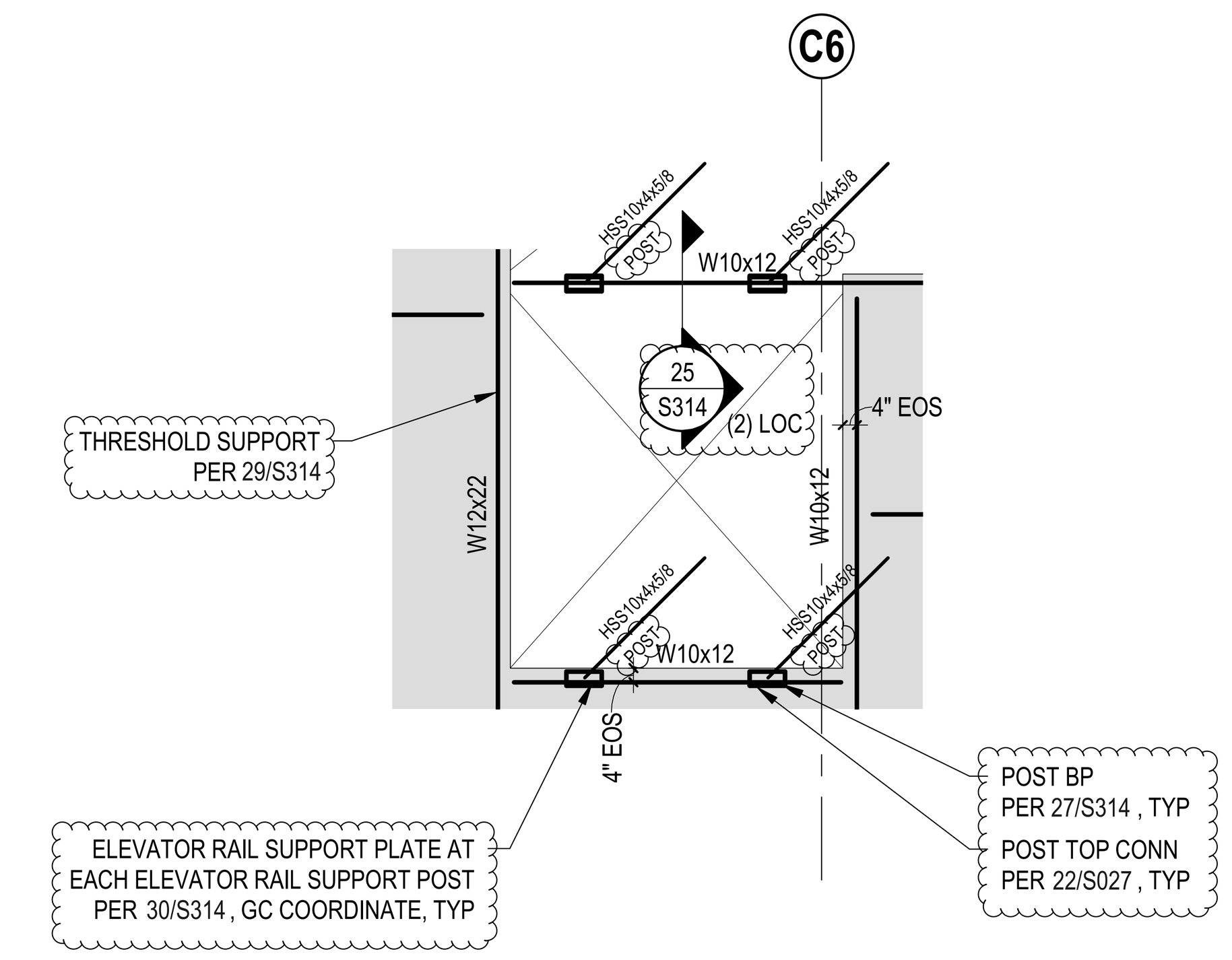
4 ENLARGED PLAN - ELEVATOR - ROOF
SCALE: 1/4" = 1'-0"



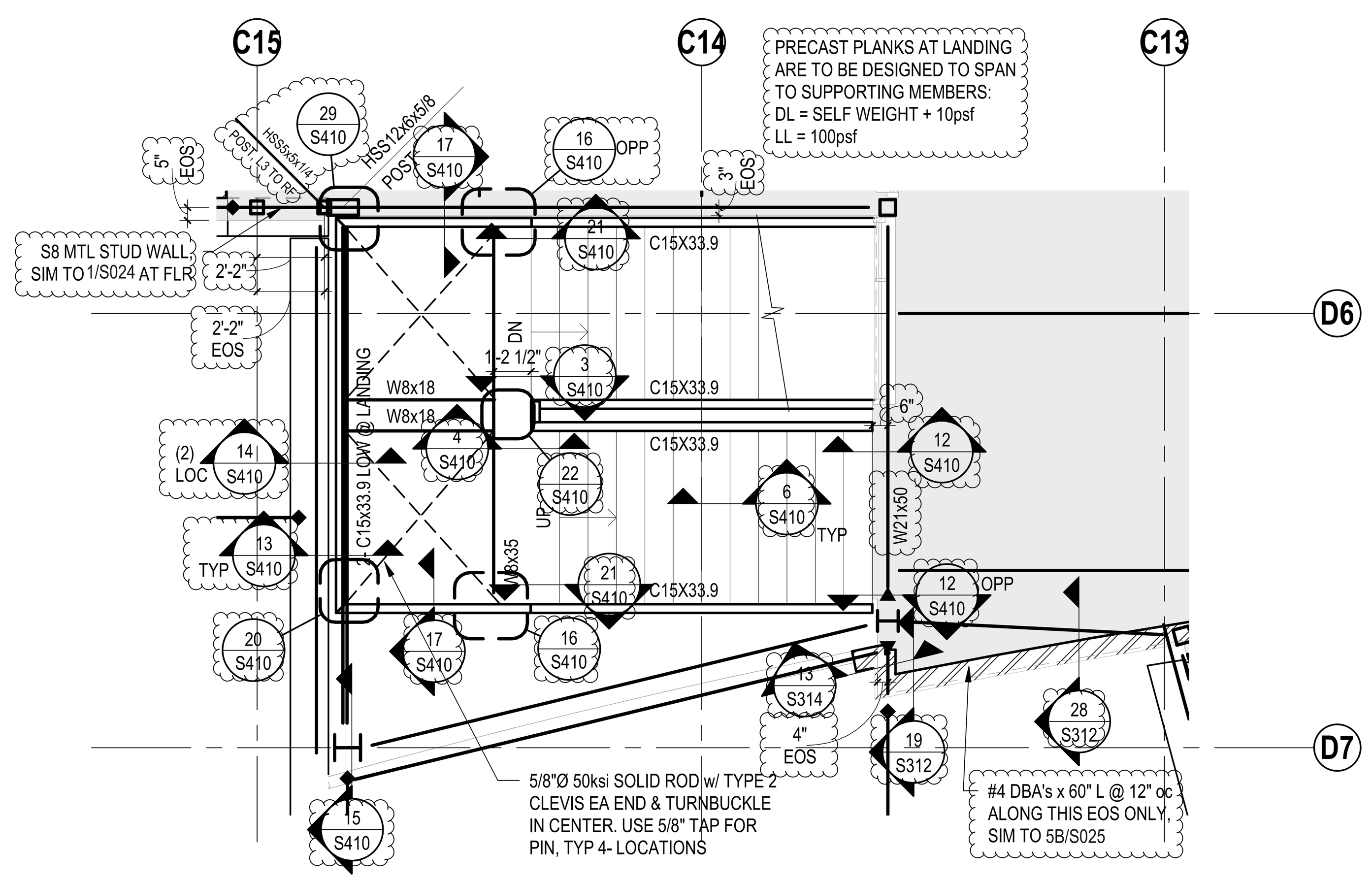
5 ENLARGED PLAN - STAIR 1-LEVEL 2
SCALE: 1/4" = 1'-0"



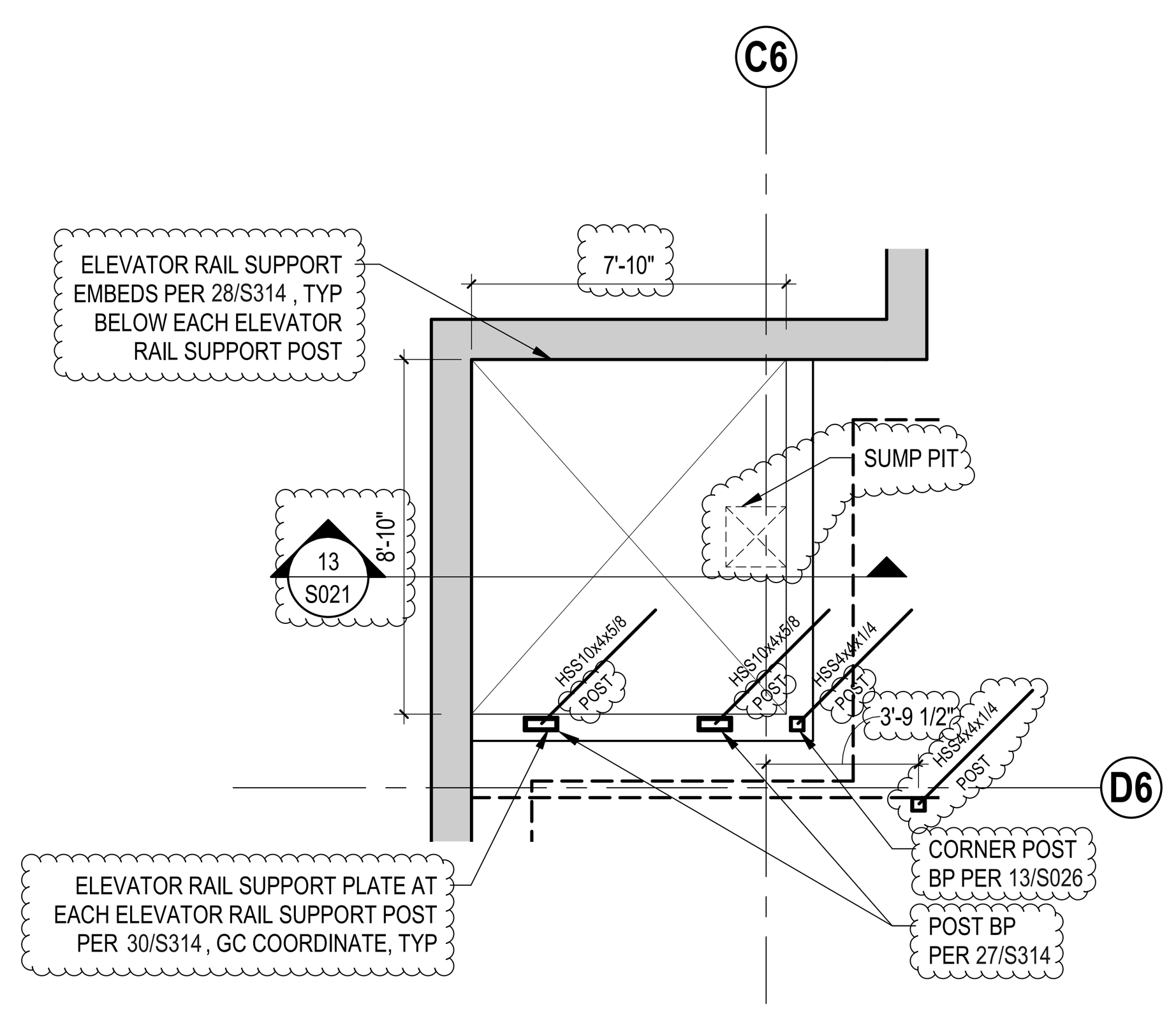
6 ENLARGED PLAN - LOBBY STAIR - LEVEL 1
SCALE: 1/4" = 1'-0"



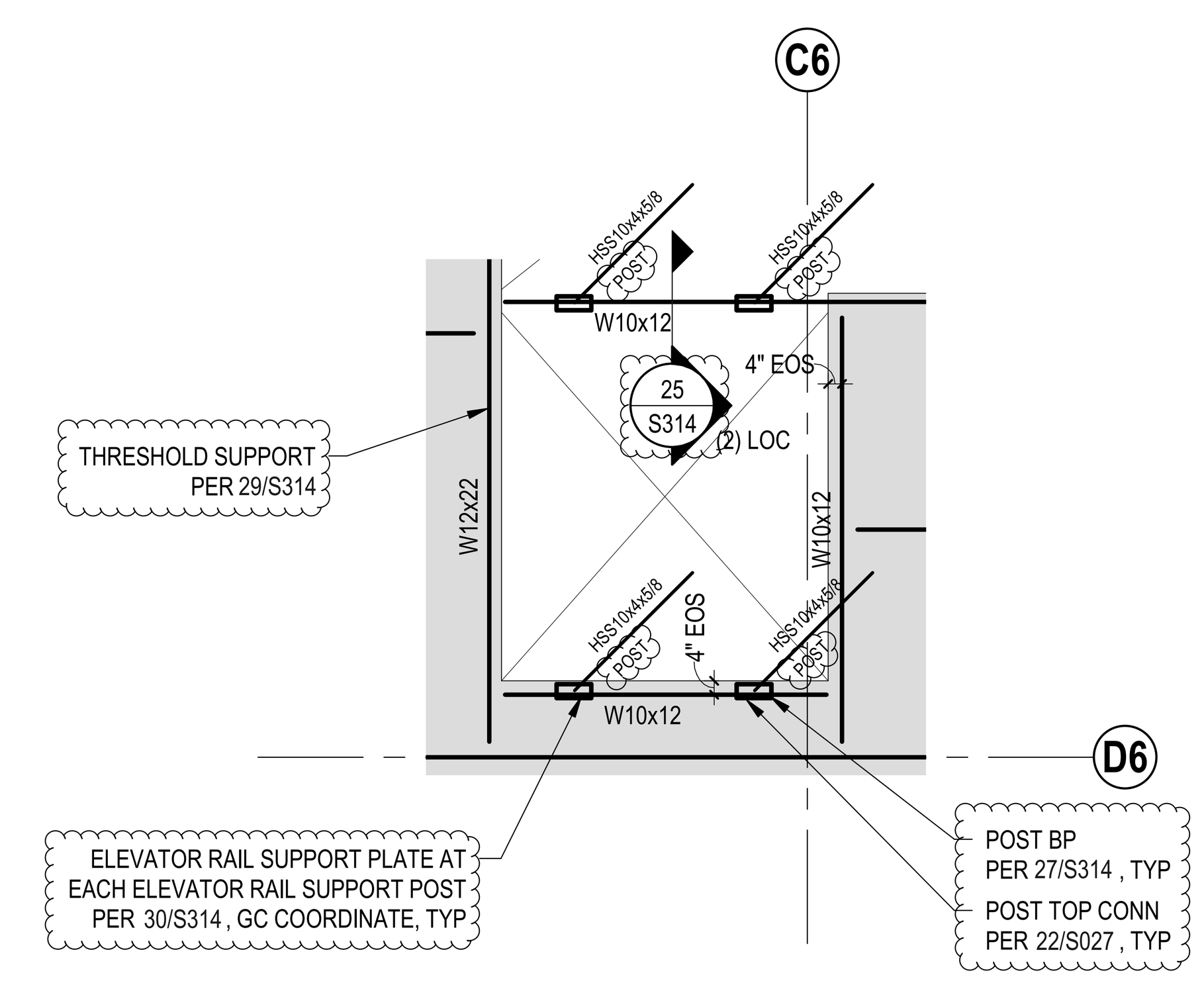
7 ENLARGED PLAN - ELEVATOR - LEVEL 2
SCALE: 1/4" = 1'-0"



8 ENLARGED PLAN - STAIR 1-LEVEL 3
SCALE: 1/4" = 1'-0"



9 ENLARGED PLAN - ELEVATOR - LEVEL 0
SCALE: 1/4" = 1'-0"



10 ENLARGED PLAN - ELEVATOR - LEVEL 3
SCALE: 1/4" = 1'-0"

PRCNC20221036
STRUCTURAL REVISION

integrus ARCHITECTURE

City of Puyallup
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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING

1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	RB/MW
Checked by:	TD

Revisions

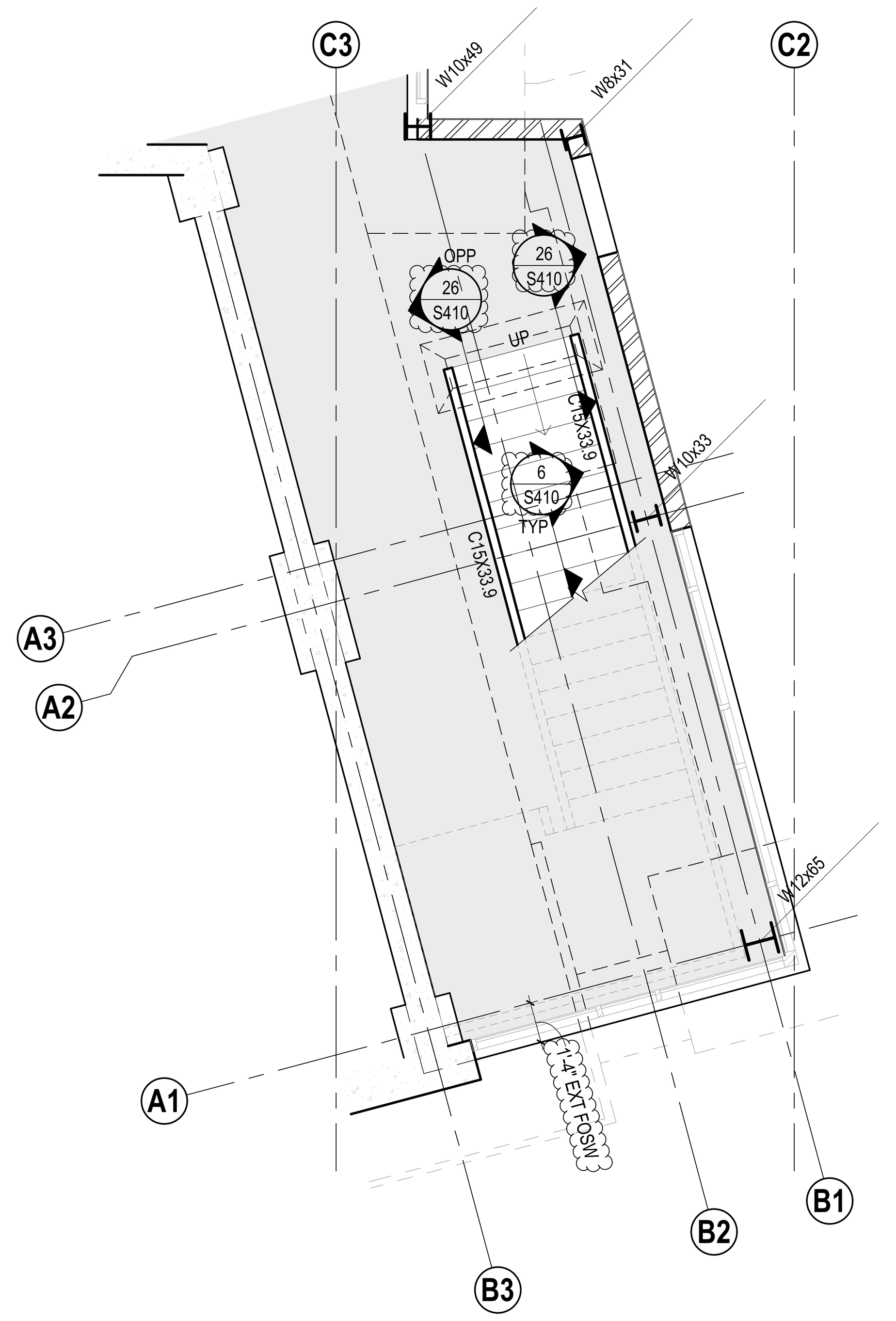
Date	Description
1.30.23	IFC Revisions

ENLARGED STAIR & ELEVATOR PLANS

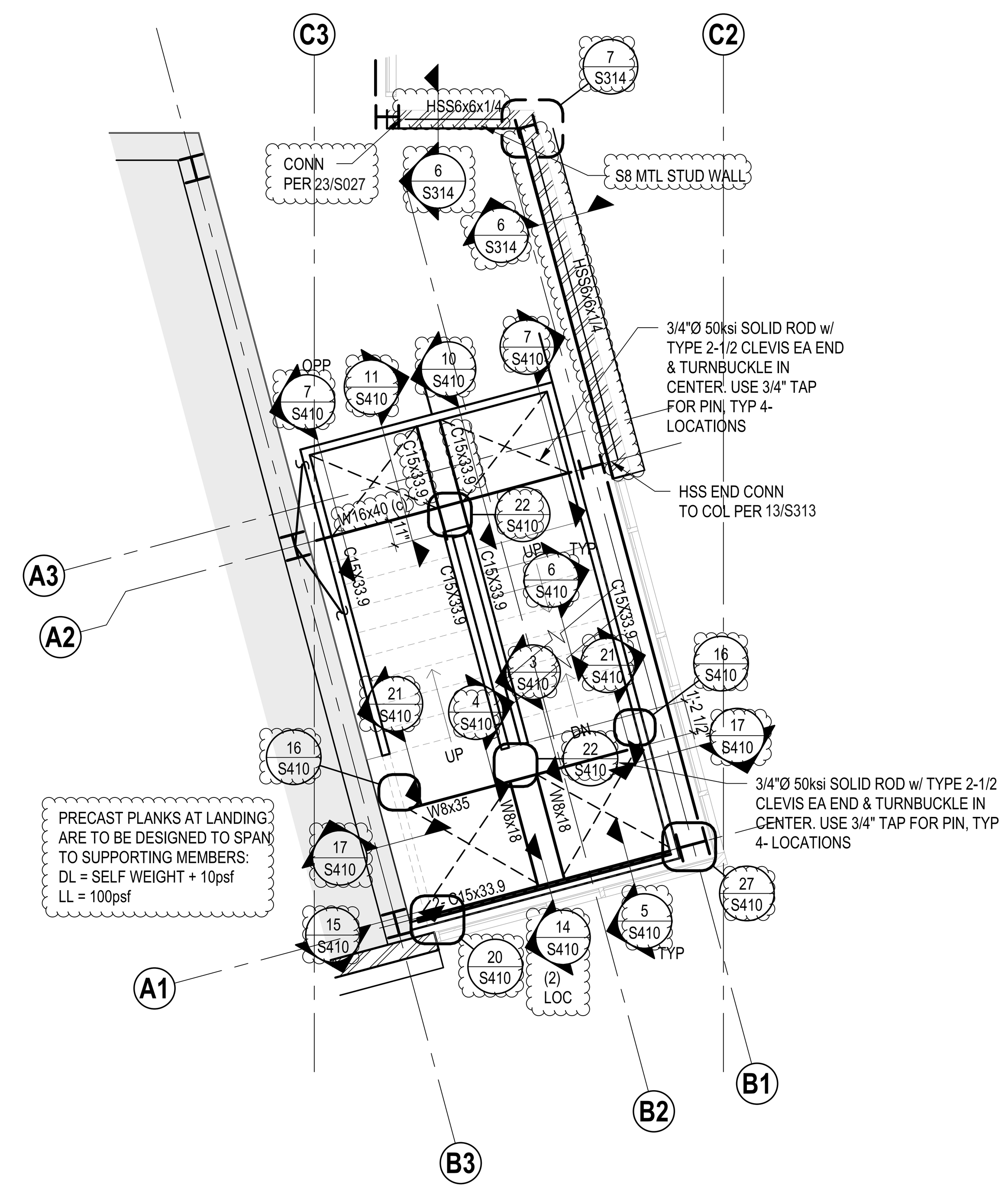
S401

IFC SET

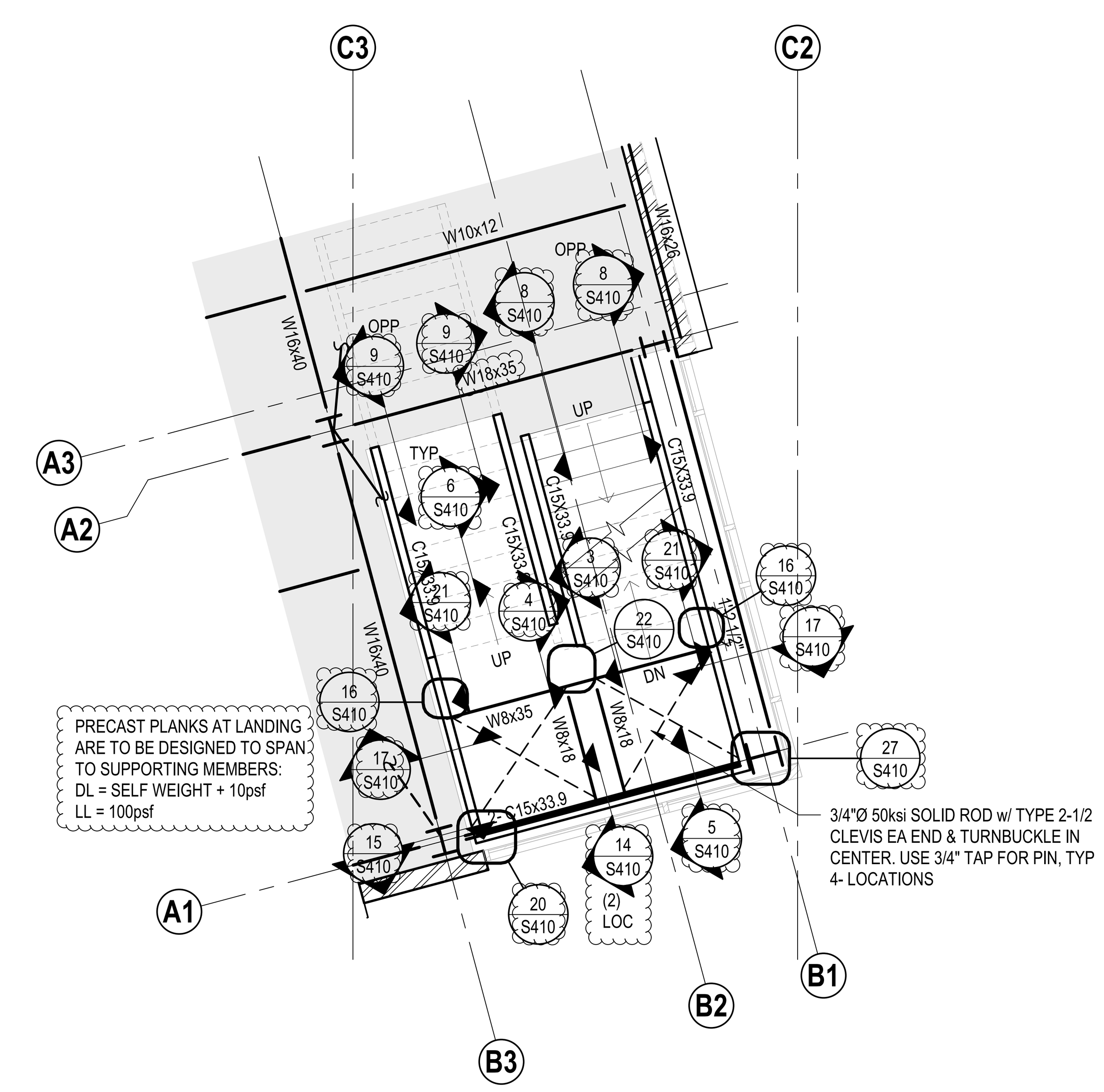
PRCNC20221036
STRUCTURAL REVISION



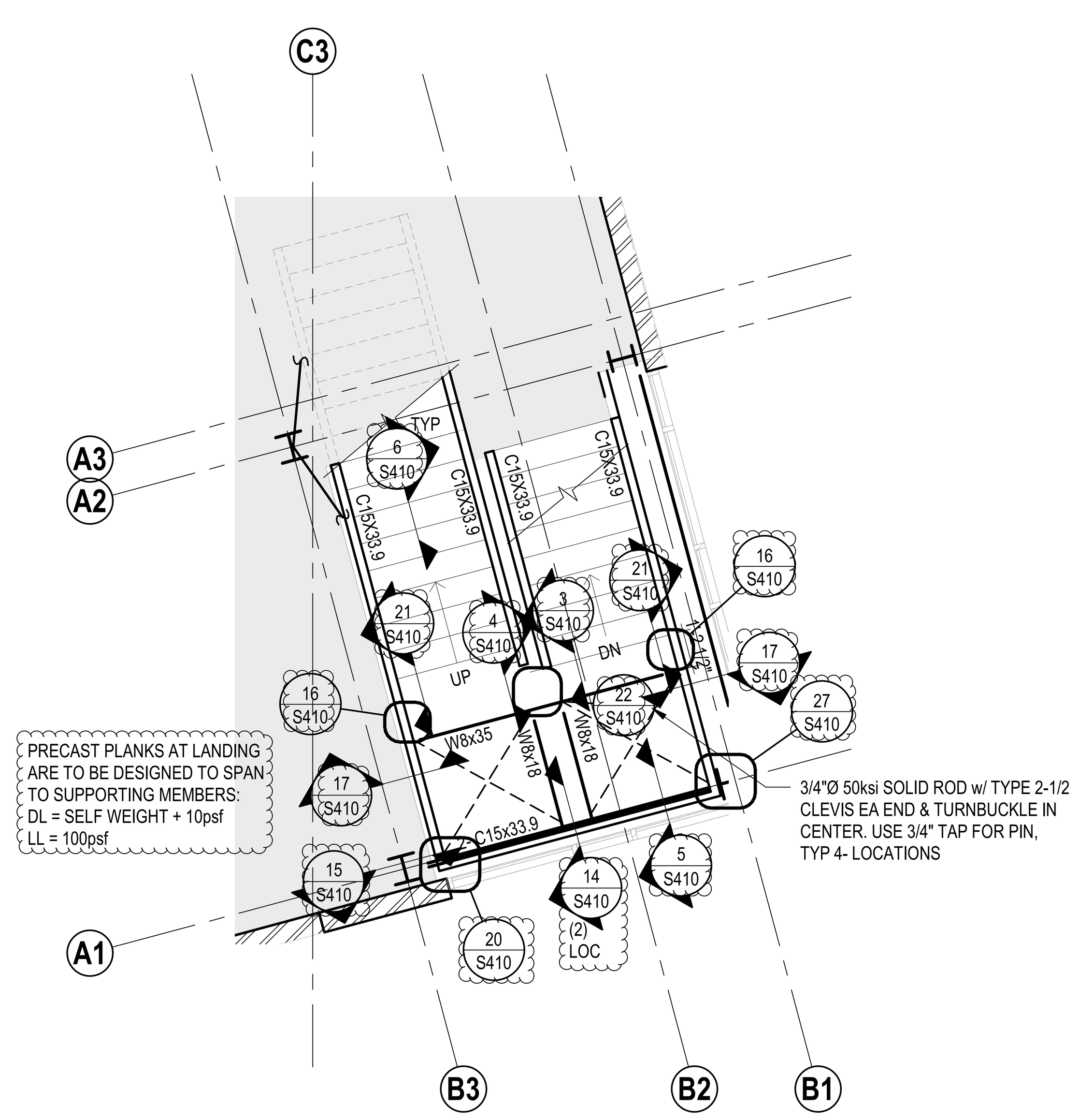
1 ENLARGED PLAN - STAIR 2 - LEVEL 0
SCALE: 1/4" = 1'-0"



2 ENLARGED PLAN - STAIR 2 - LEVEL 0.5
SCALE: 1/4" = 1'-0"



3 ENLARGED PLAN - STAIR 2 - LEVEL 2
SCALE: 1/4" = 1'-0"



4 ENLARGED PLAN - STAIR 2 - LEVEL 2.5
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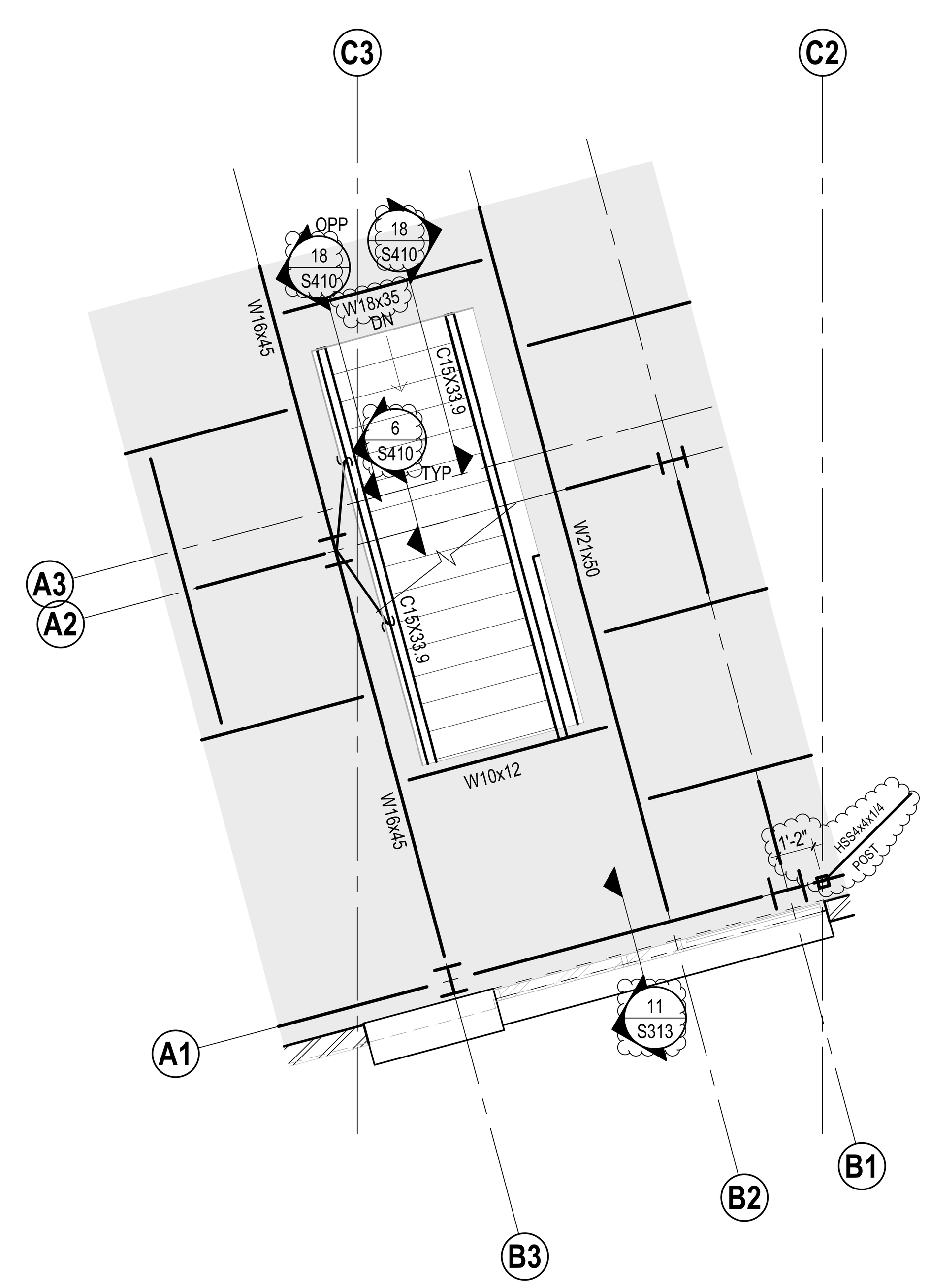
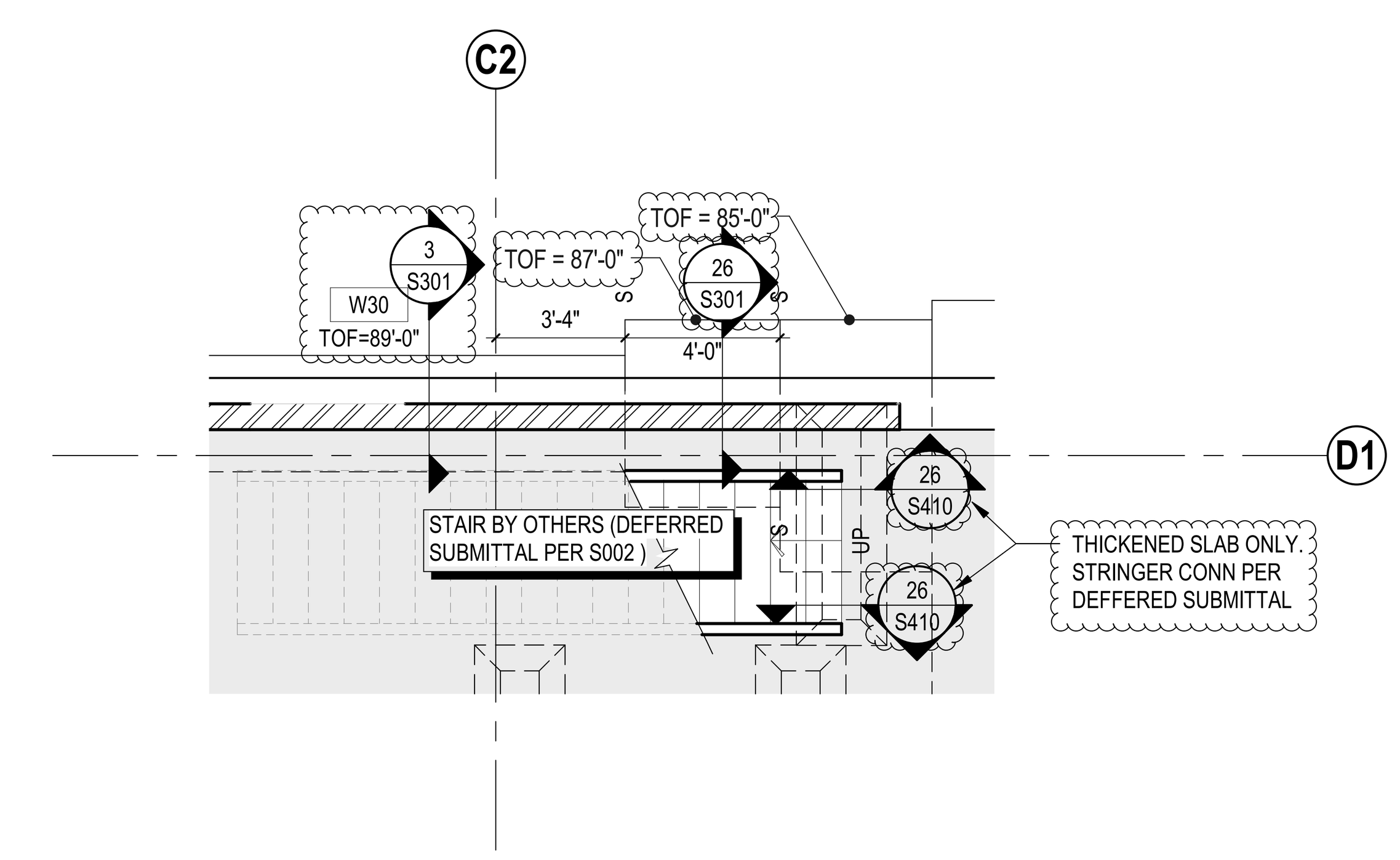
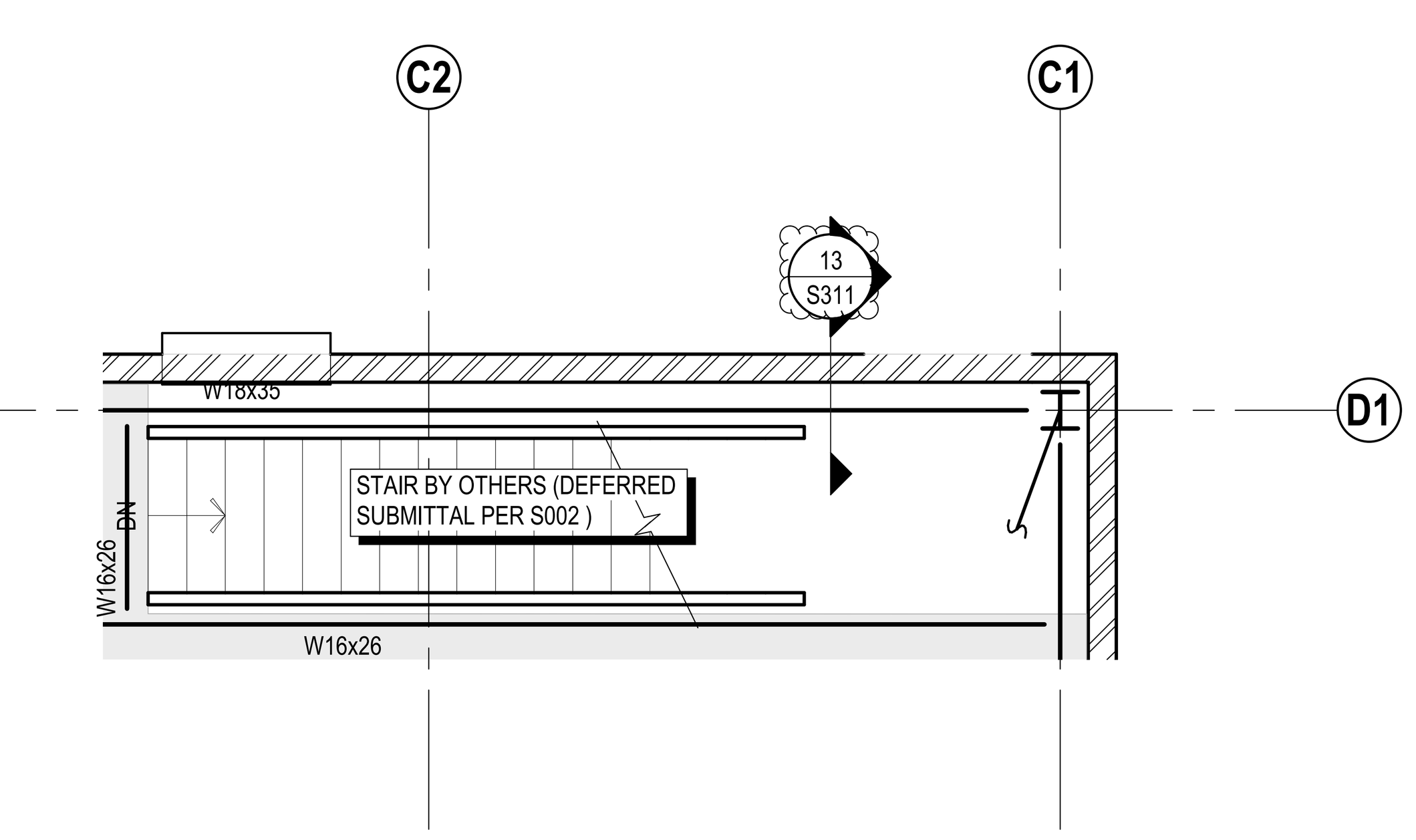


5 ENLARGED PLAN - STAIR 2 - LEVEL 3
SCALE: 1/4" = 1'-0"

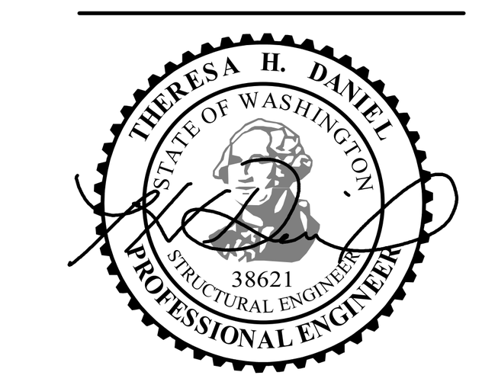


6 ENLARGED PLAN - MEZZANINE STAIRS - LEVEL 0
SCALE: 1/4" = 1'-0"



7 ENLARGED PLAN - MEZZANINE STAIRS - LEVEL 1
SCALE: 1/4" = 1'-0"

integrus
ARCHITECTURE



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PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING

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Puyallup, WA 98374

Date:	10/07/2022
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Drawn By:	RB/MW
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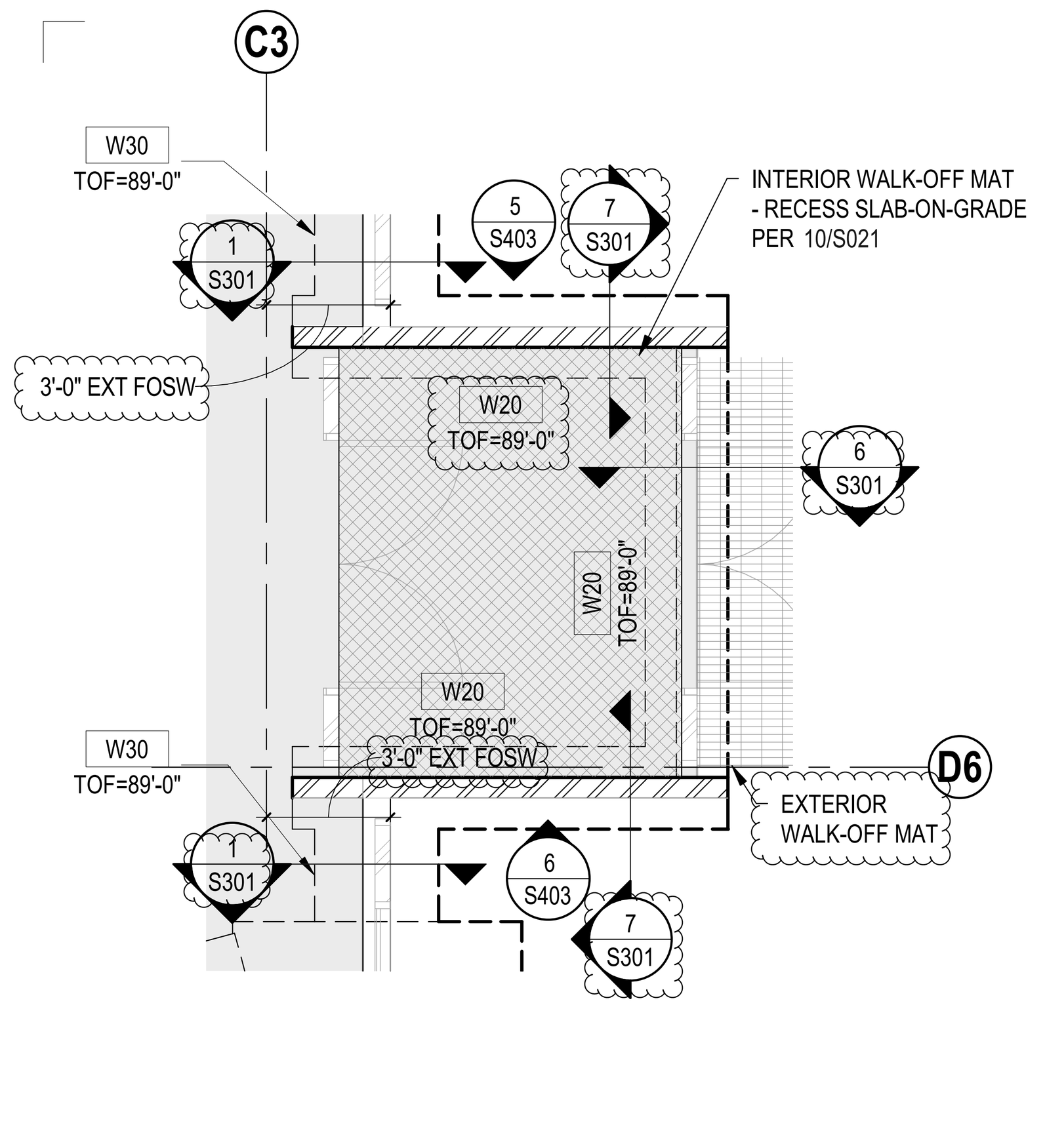
Revisions

Date	Description
1.30.23	IFC Revisions

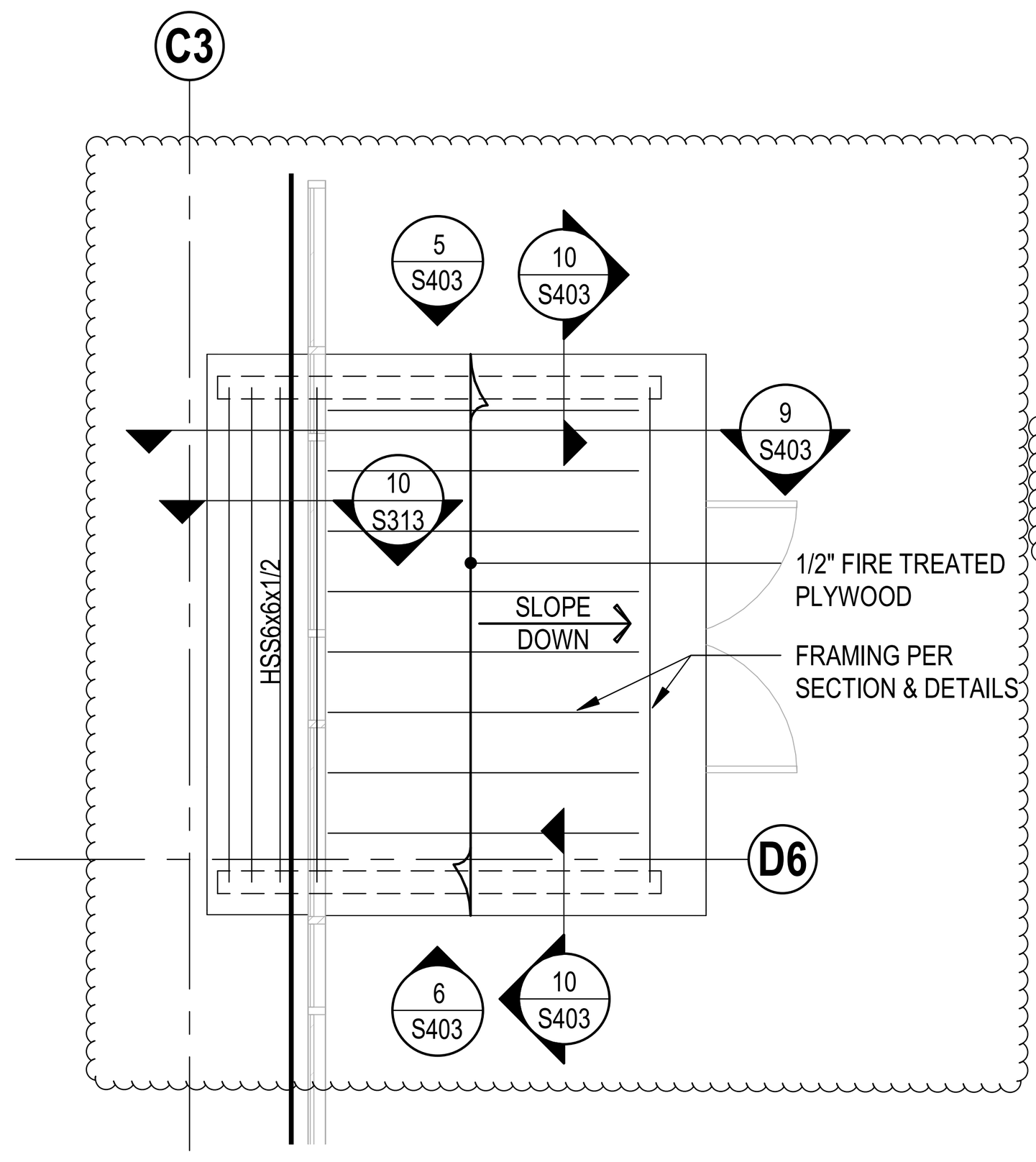
ENLARGED STAIR PLANS

S402

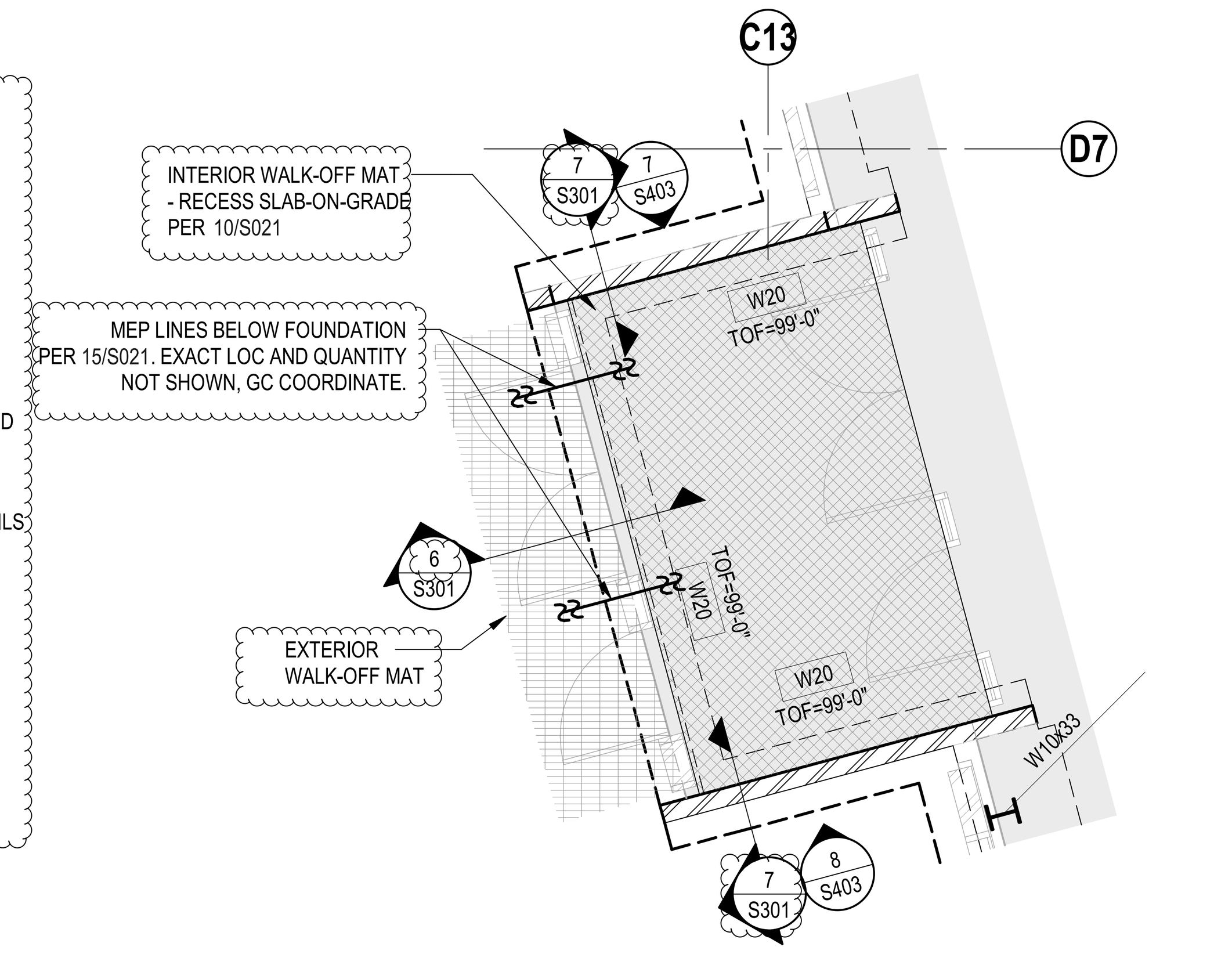
IFC SET



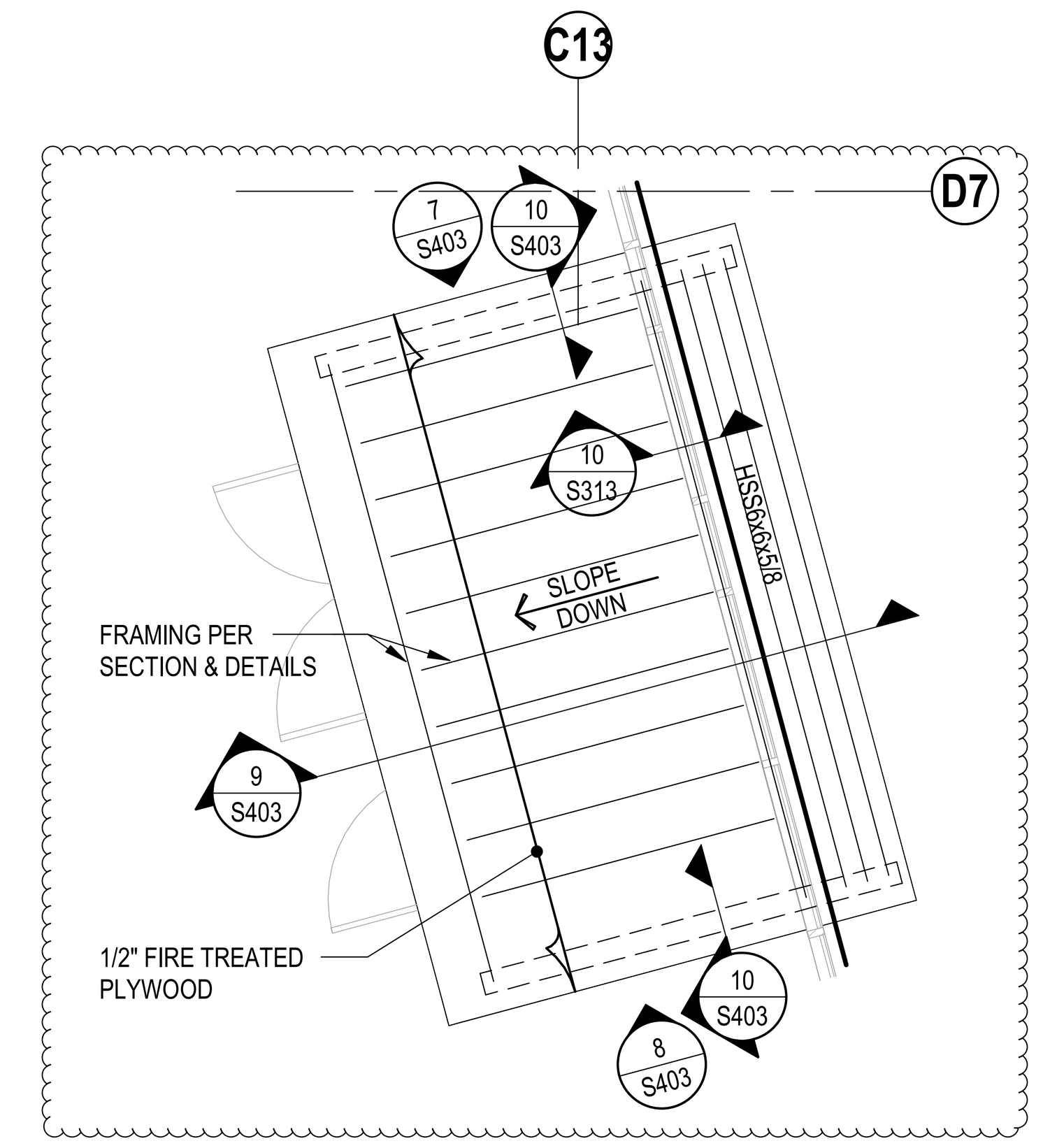
1 ENLARGED PLAN - VESTIBULE 2
SCALE: 1/4" = 1'-0"



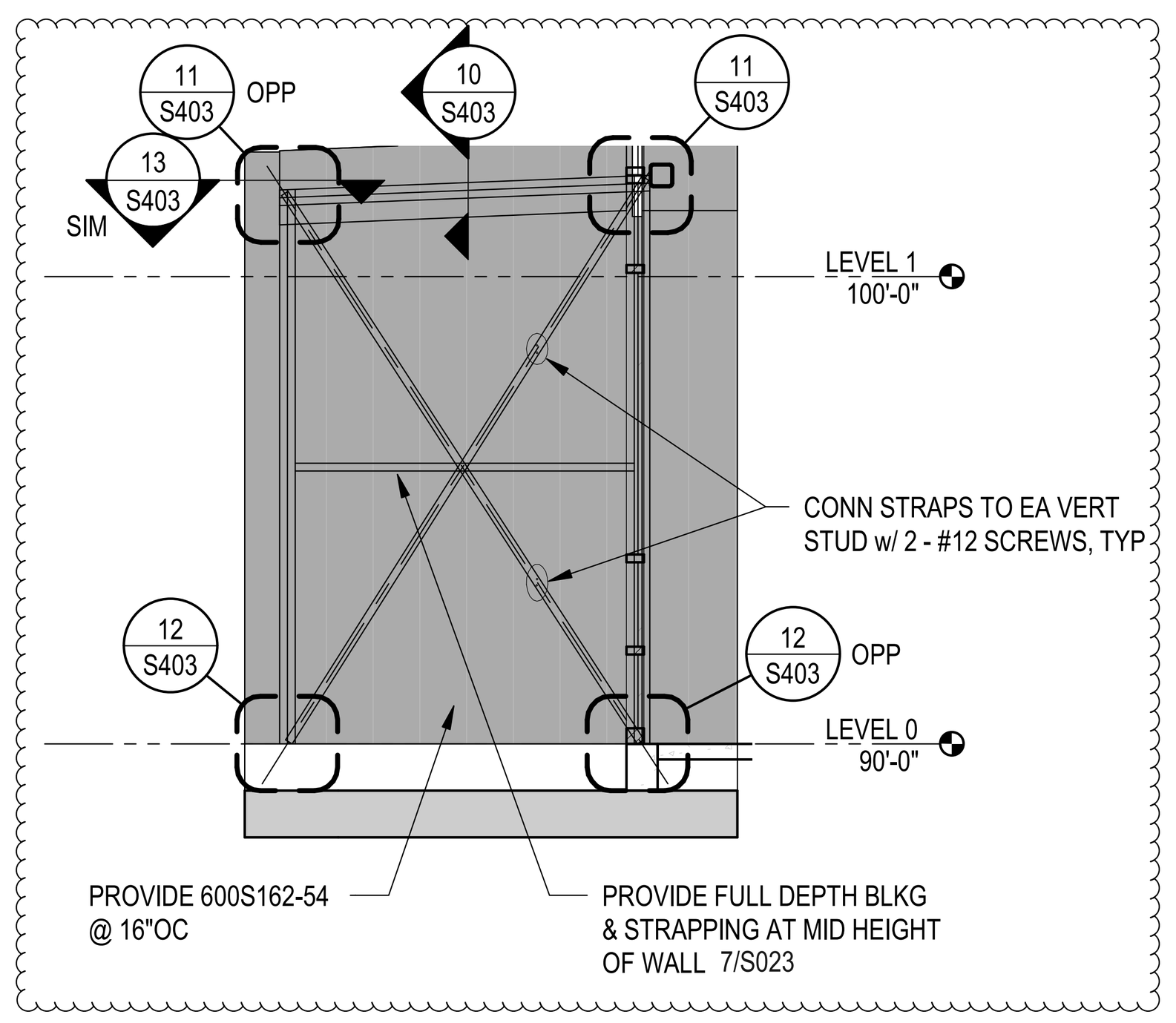
2 ENLARGED PLAN - VESTIBULE 2
SCALE: 1/4" = 1'-0"



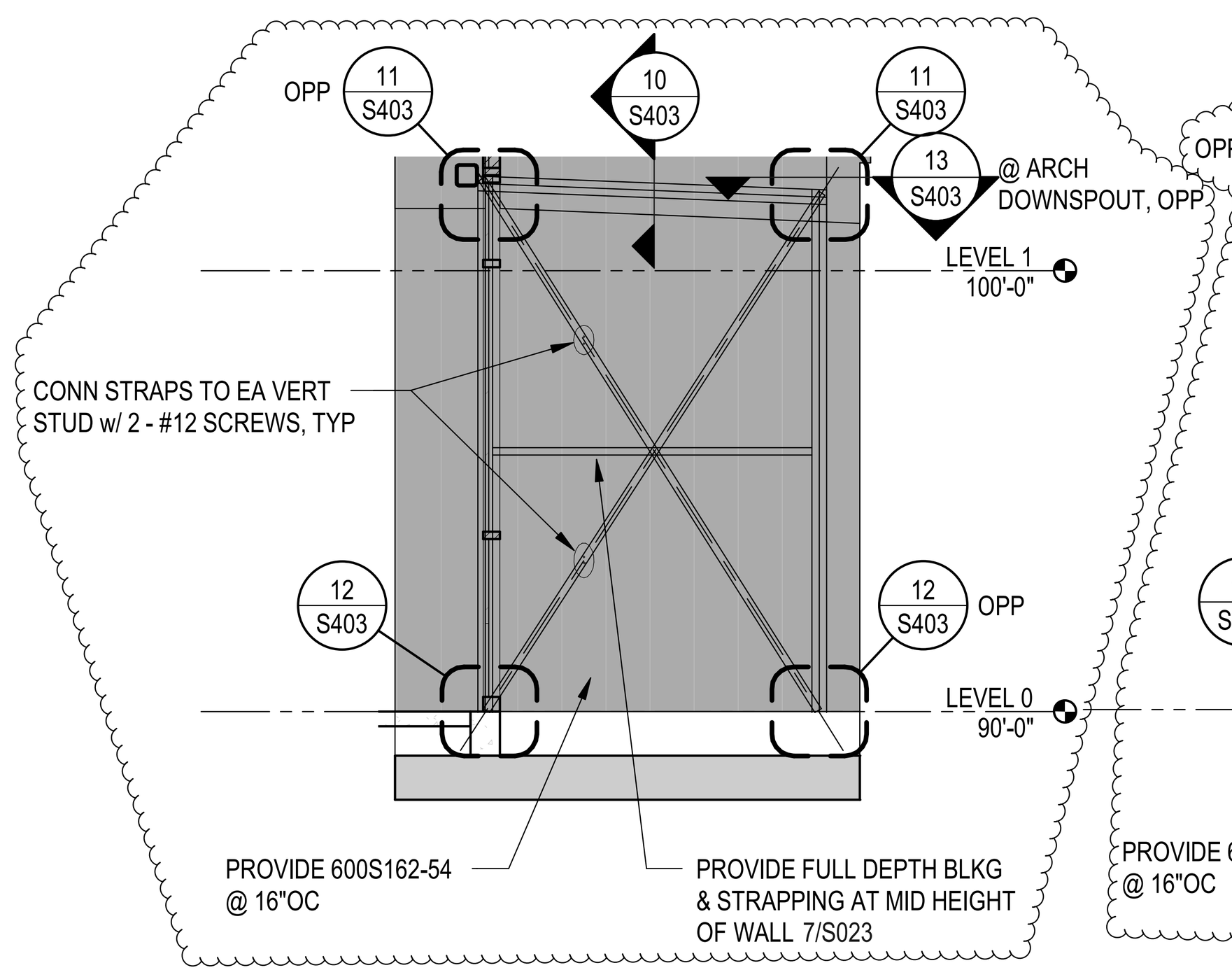
3 ENLARGED PLAN - VESTIBULE 1
SCALE: 1/4" = 1'-0"



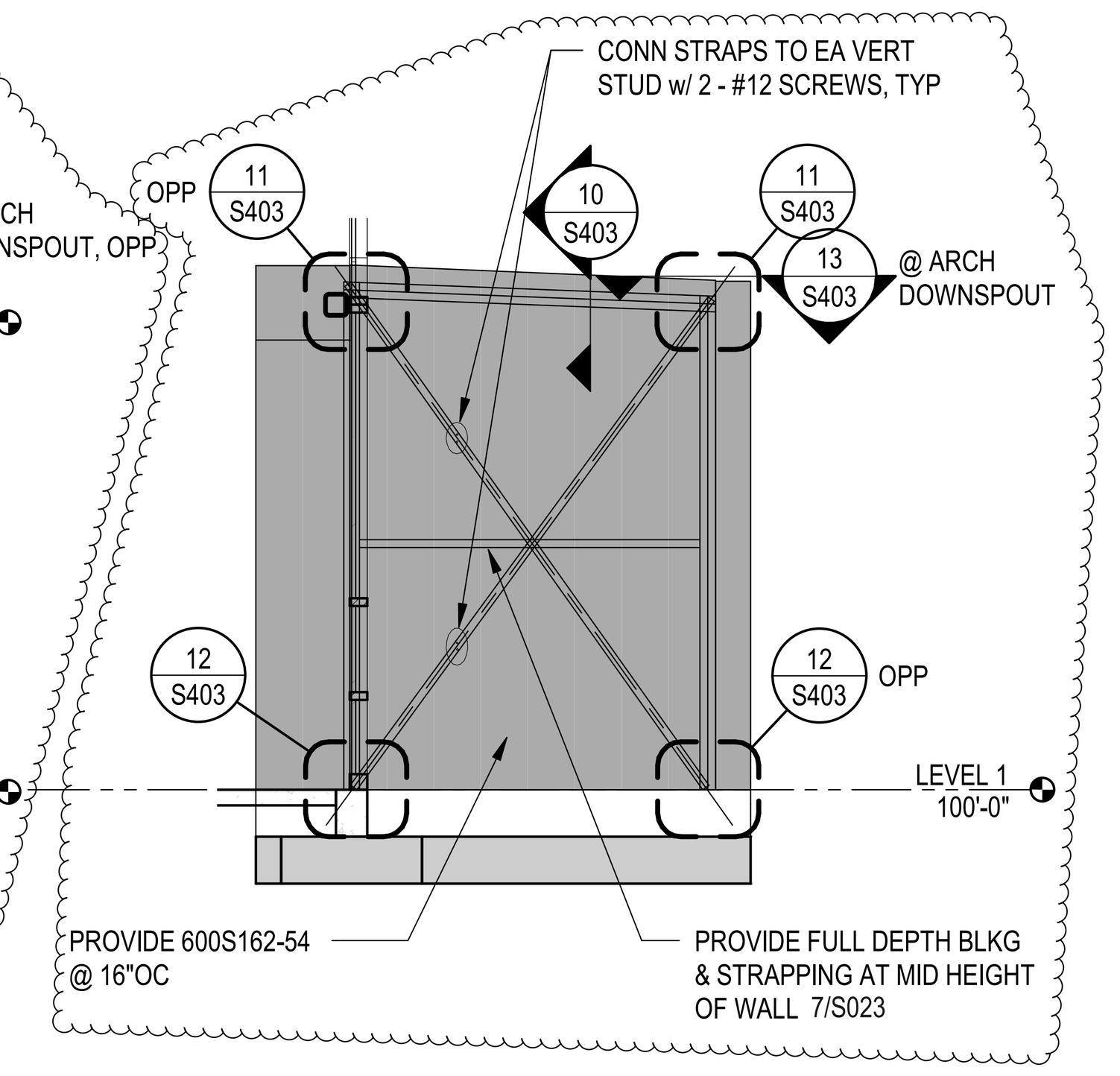
4 ENLARGED PLAN - VESTIBULE 1
SCALE: 1/4" = 1'-0"



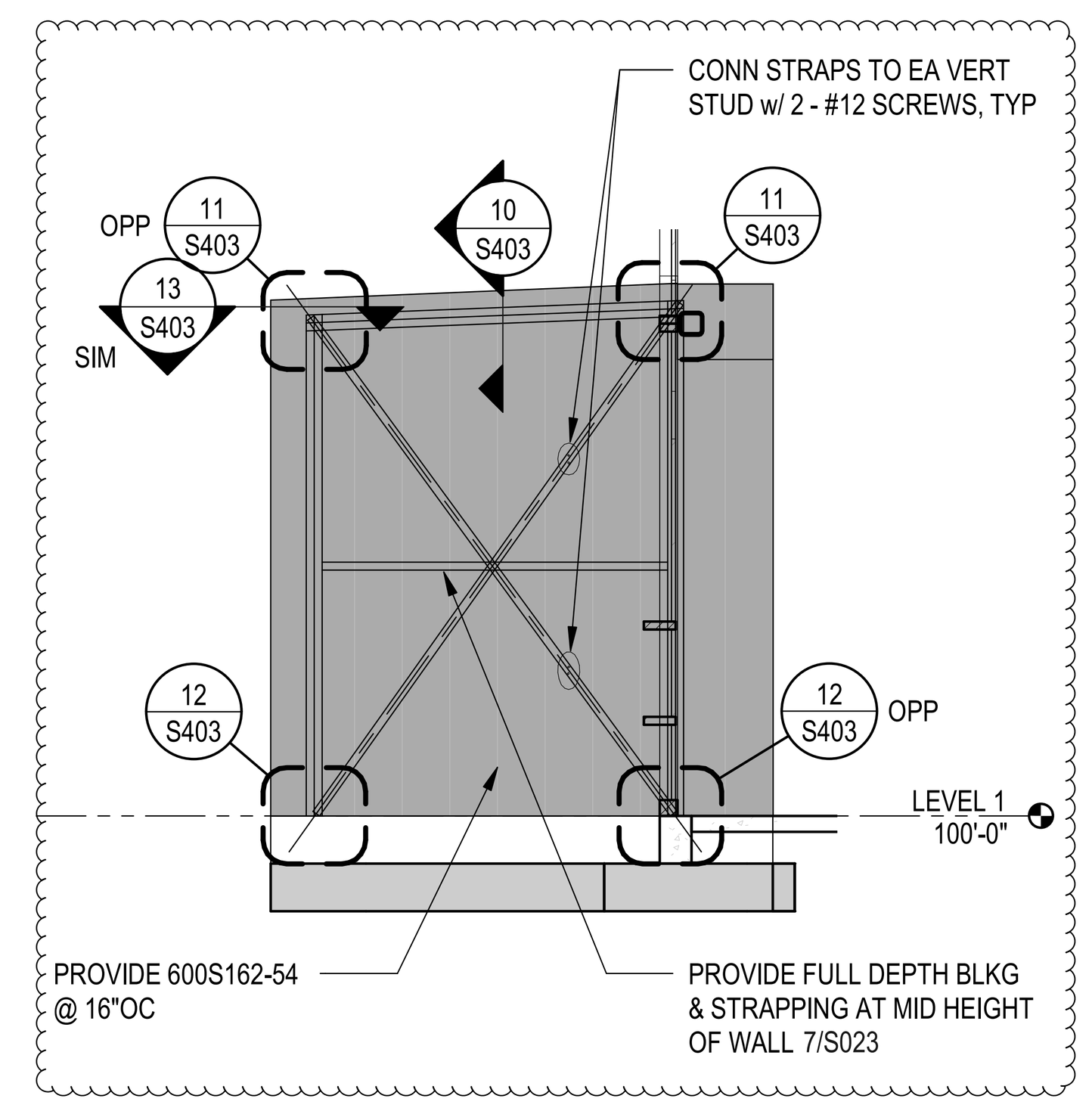
5 VESTIBULE 2 - NORTH WALL ELEVATION
SCALE: 1/4" = 1'-0"



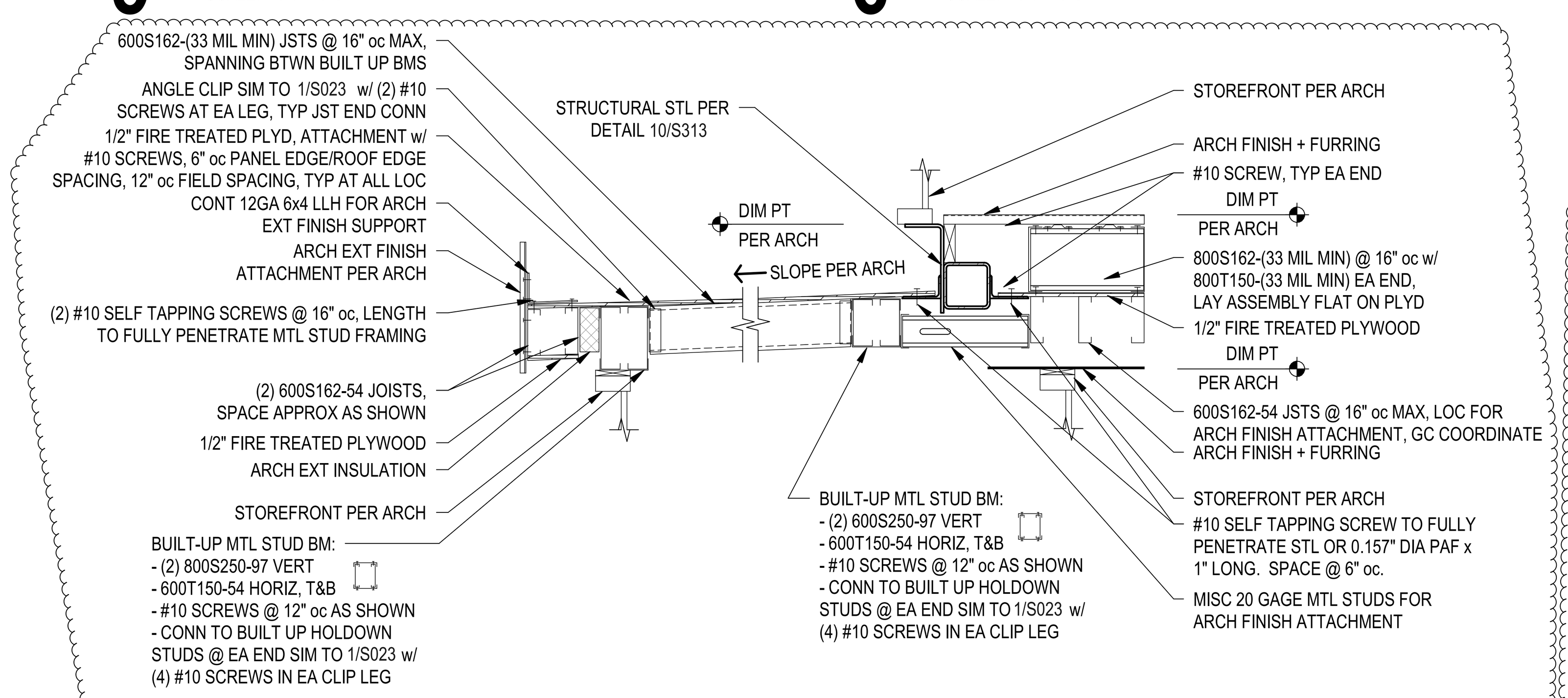
6 VESTIBULE 2 - SOUTH WALL ELEVATION
SCALE: 1/4" = 1'-0"



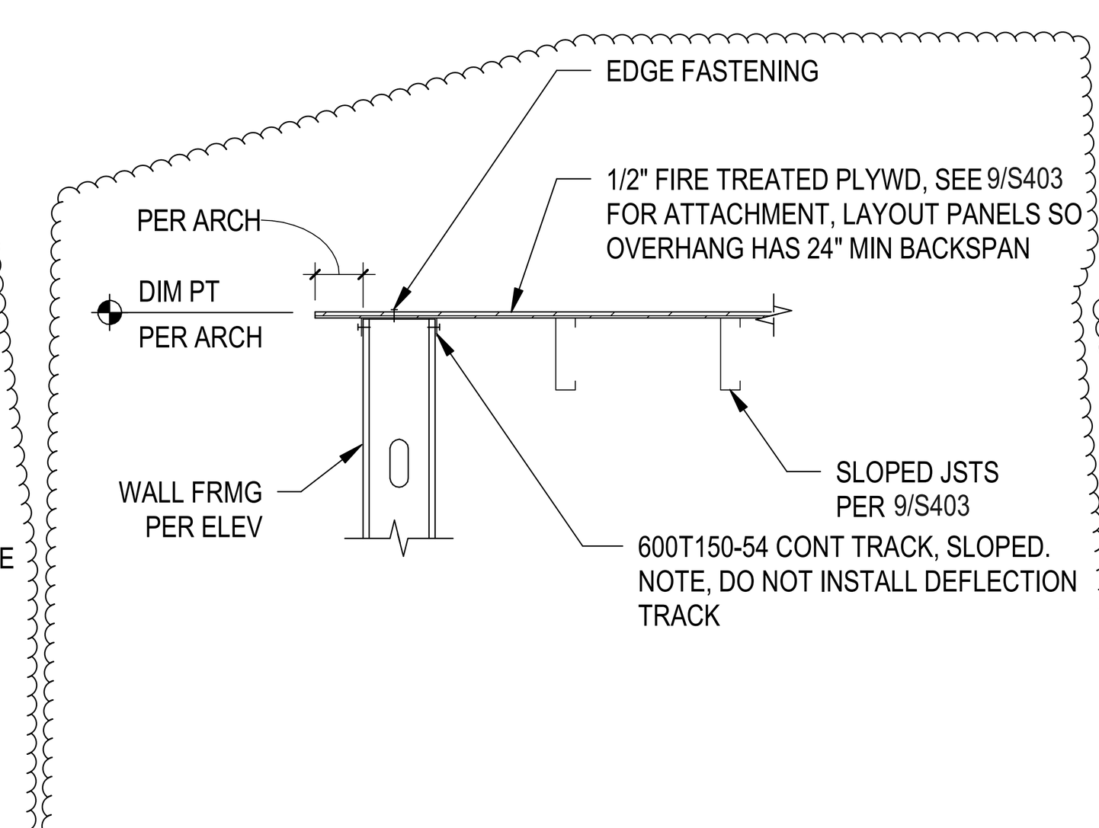
7 VESTIBULE 1 - NORTH WALL ELEVATION
SCALE: 1/4" = 1'-0"



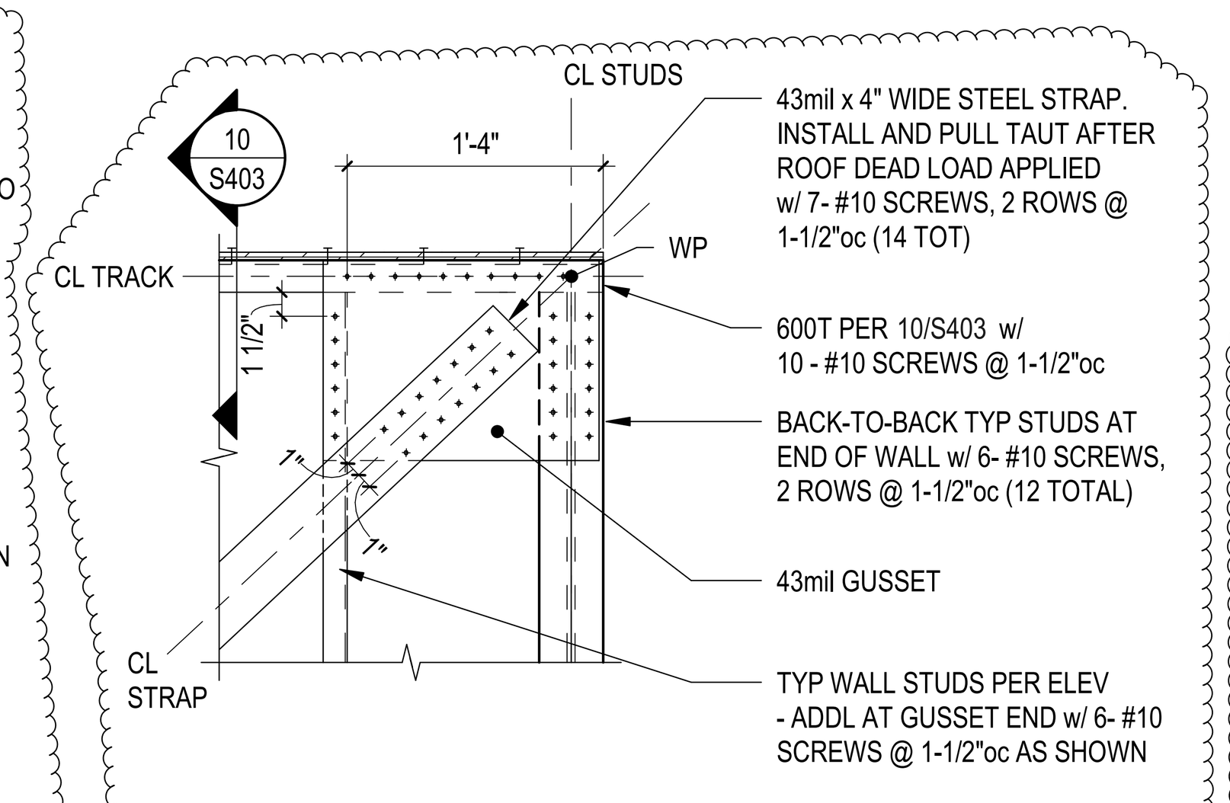
8 VESTIBULE 1 - SOUTH WALL ELEVATION
SCALE: 1/4" = 1'-0"



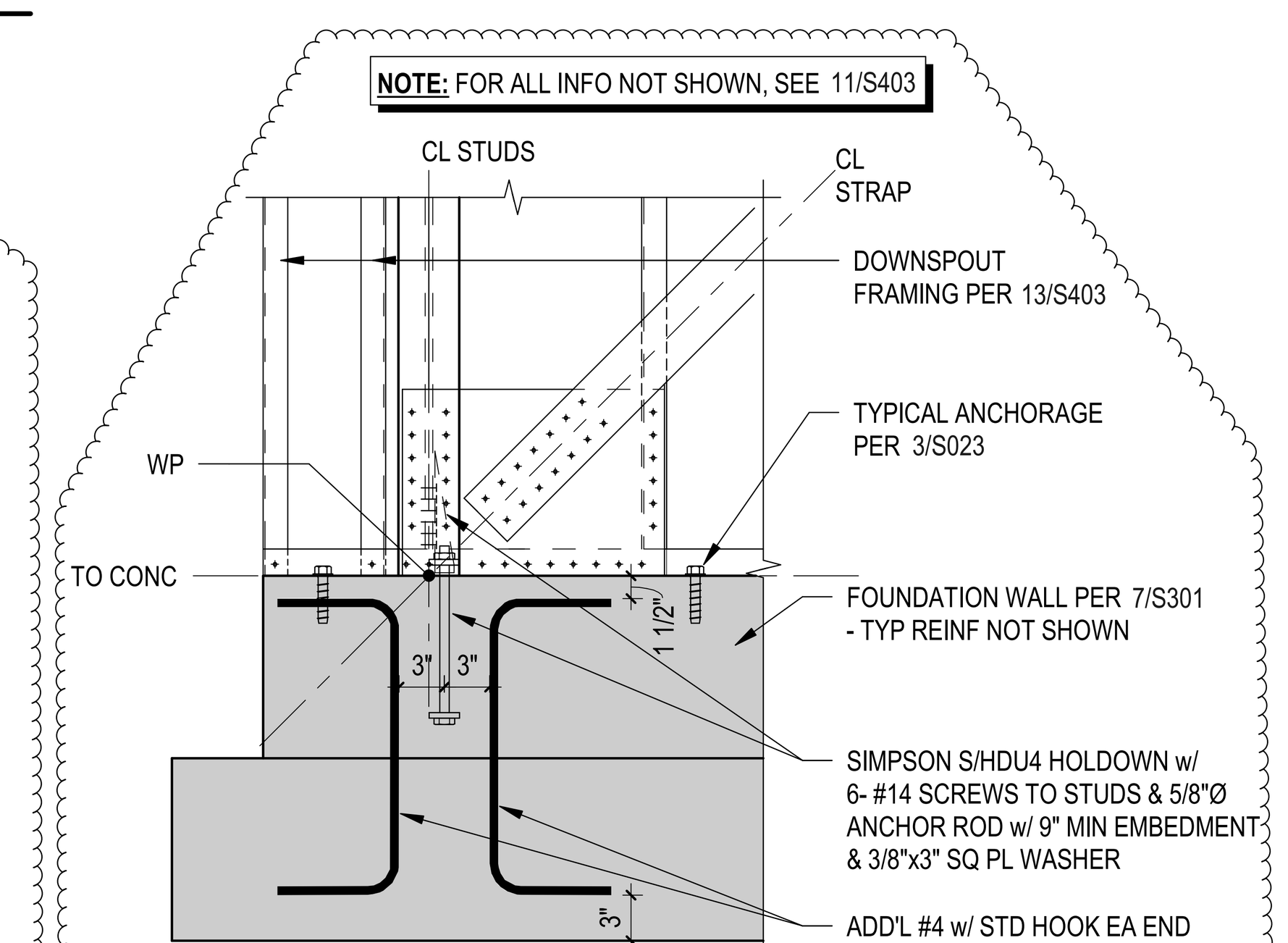
9 VESTIBULE ROOF SECTION
SCALE: 3/4" = 1'-0"



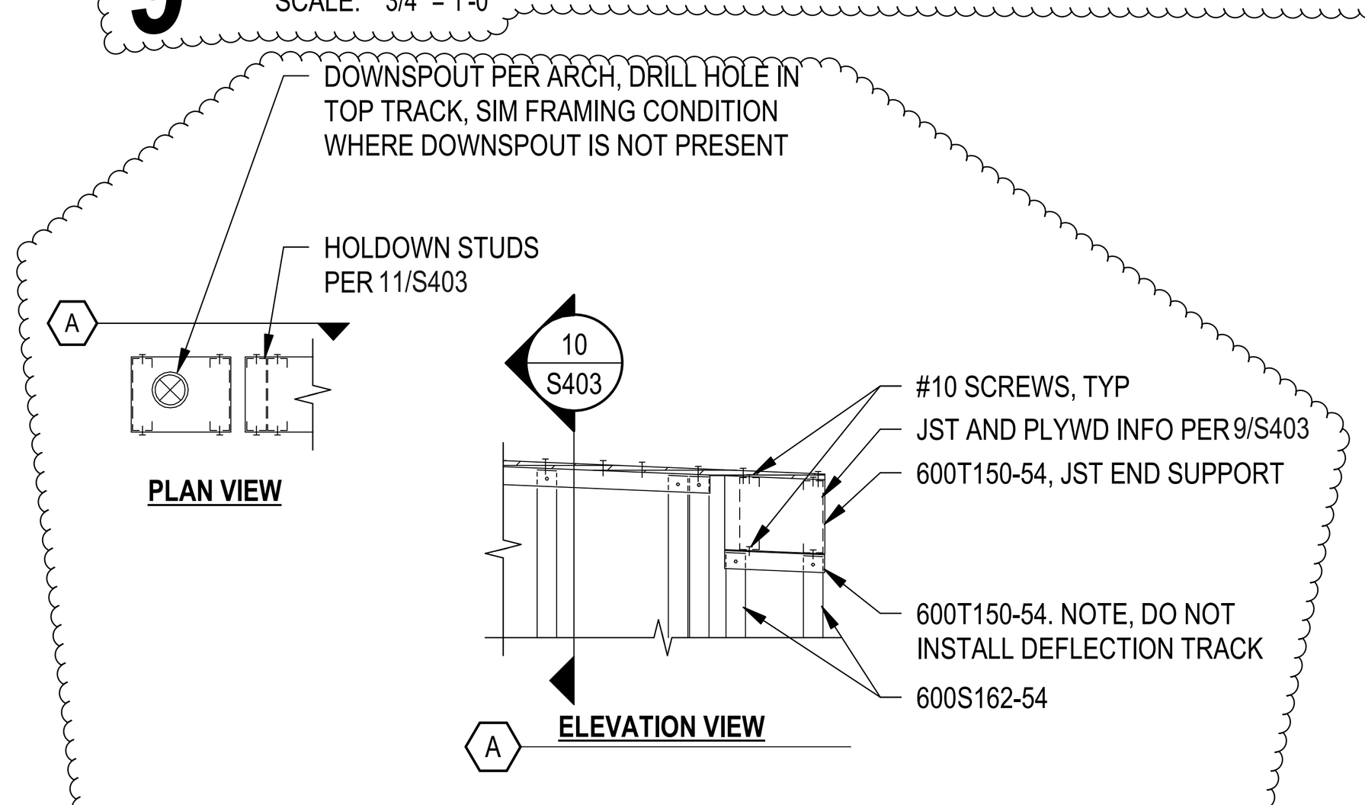
10 VESTIBULE ROOF SECTION
SCALE: 3/4" = 1'-0"



11 DETAIL CONN
SCALE: 1" = 1'-0"



12 DETAIL CONN
SCALE: 1" = 1'-0"



13 VESTIBULE DOWNSPOUT DETAIL
SCALE: 3/4" = 1'-0"

PRCNC20221036
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Date: 10/07/2022
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Drawn By: RB/BLO
Checked by: TD

Revisions

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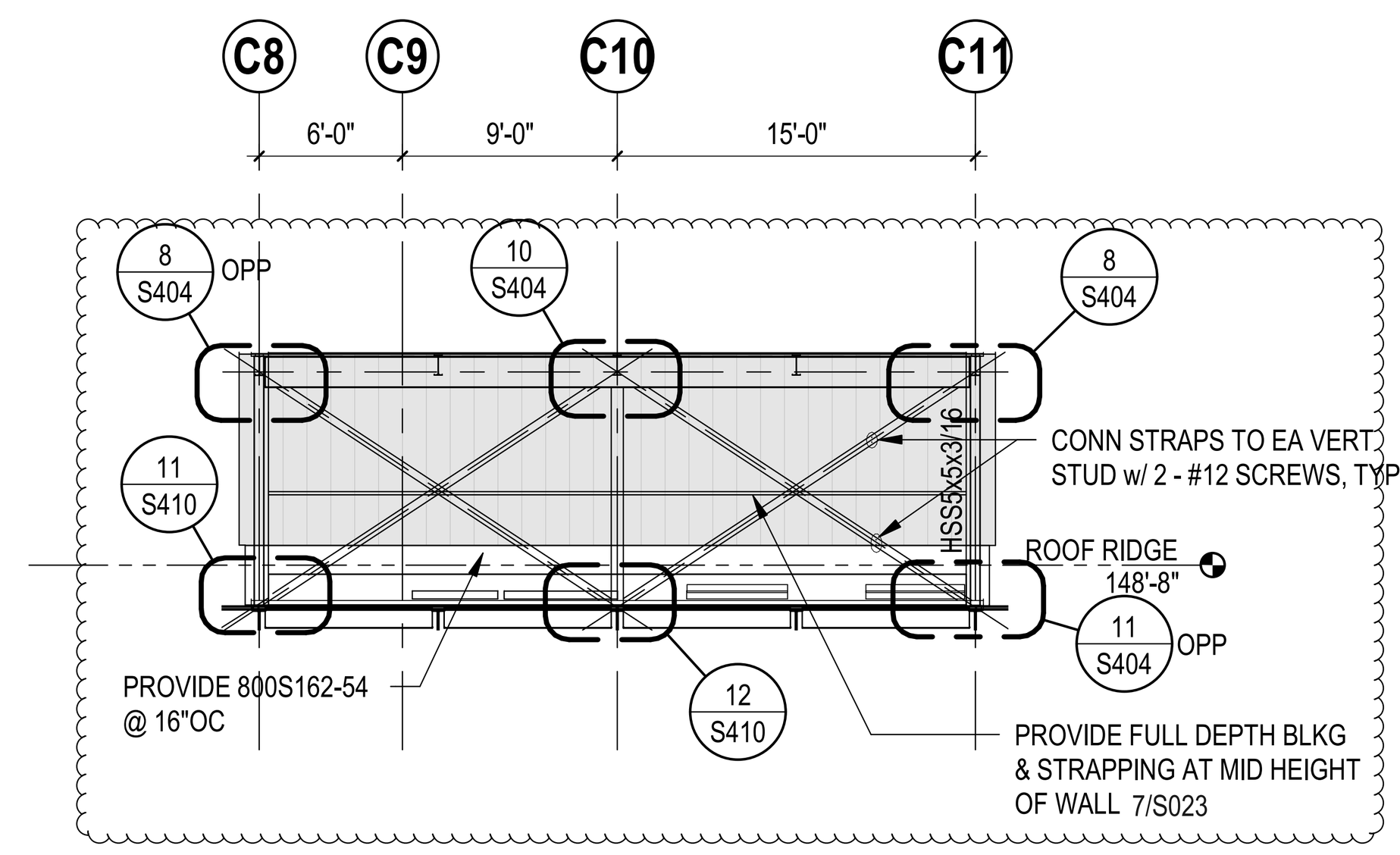
1.30.23	IFC Revisions

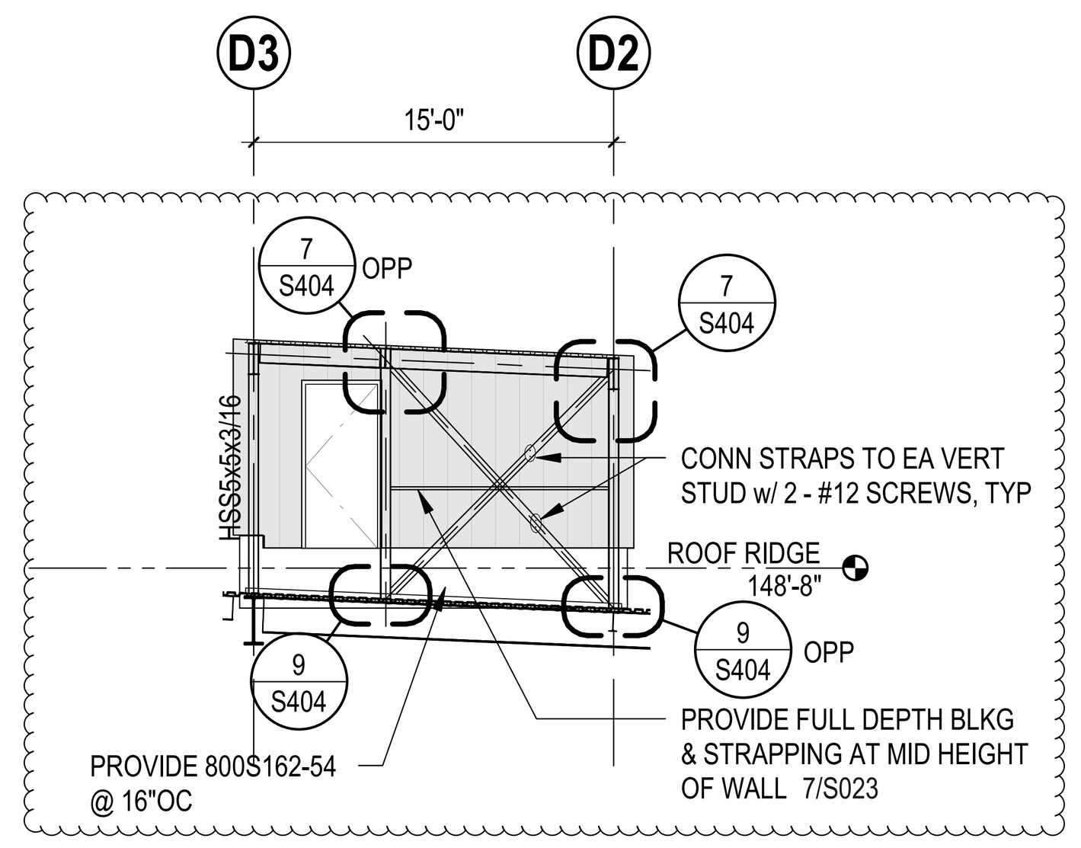
ENLARGED VESTIBULE PLANS & DETAILS

S403

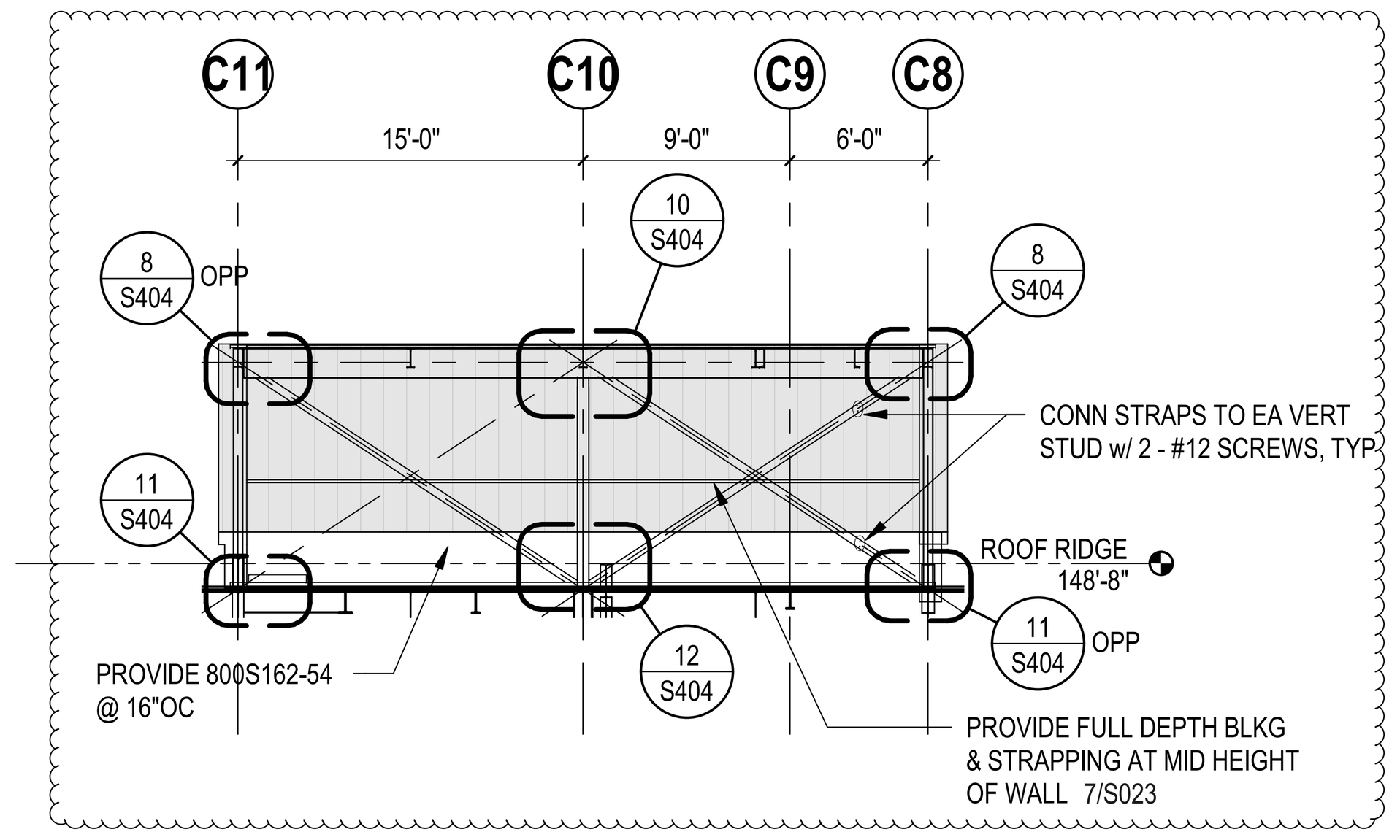
IFC SET



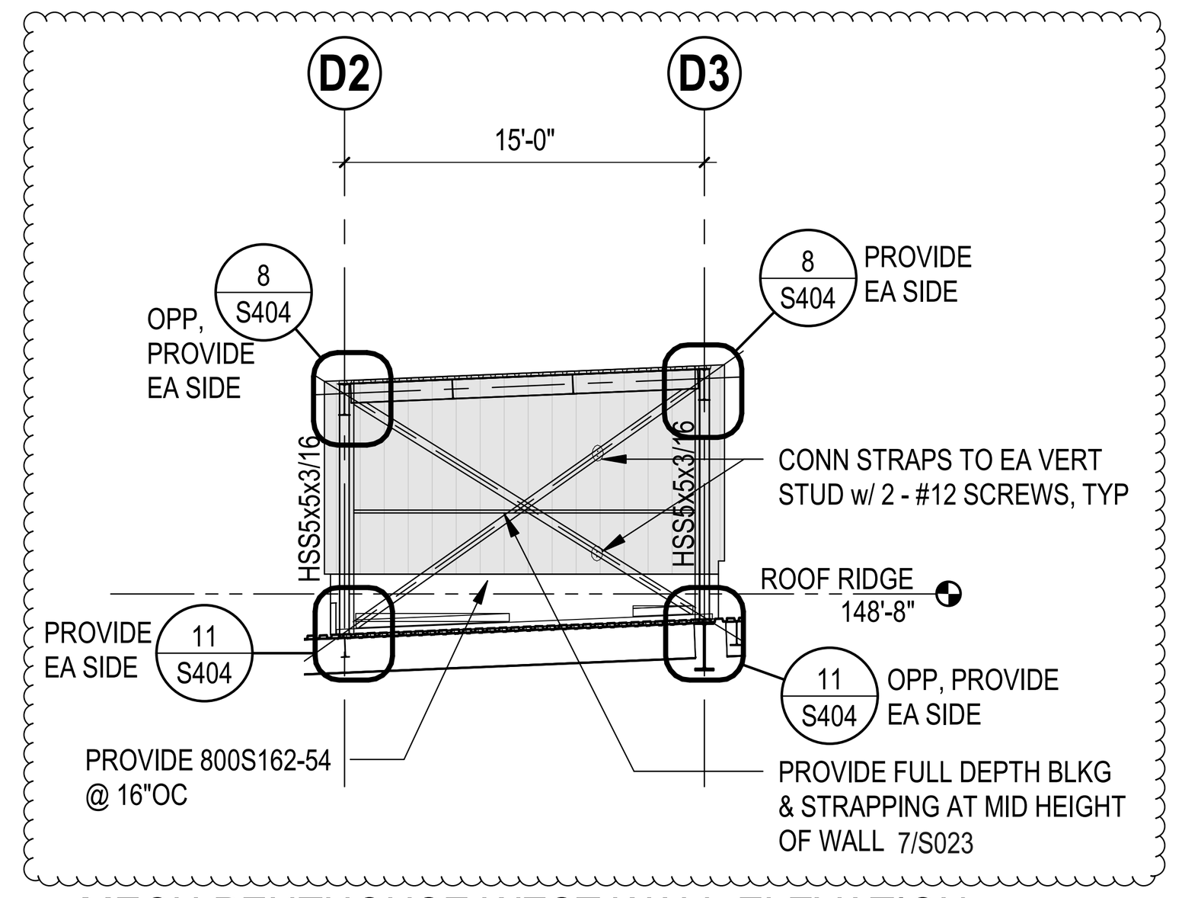
1 MECH PENTHOUSE NORTH WALL ELEVATION
SCALE: 1/8" = 1'-0"



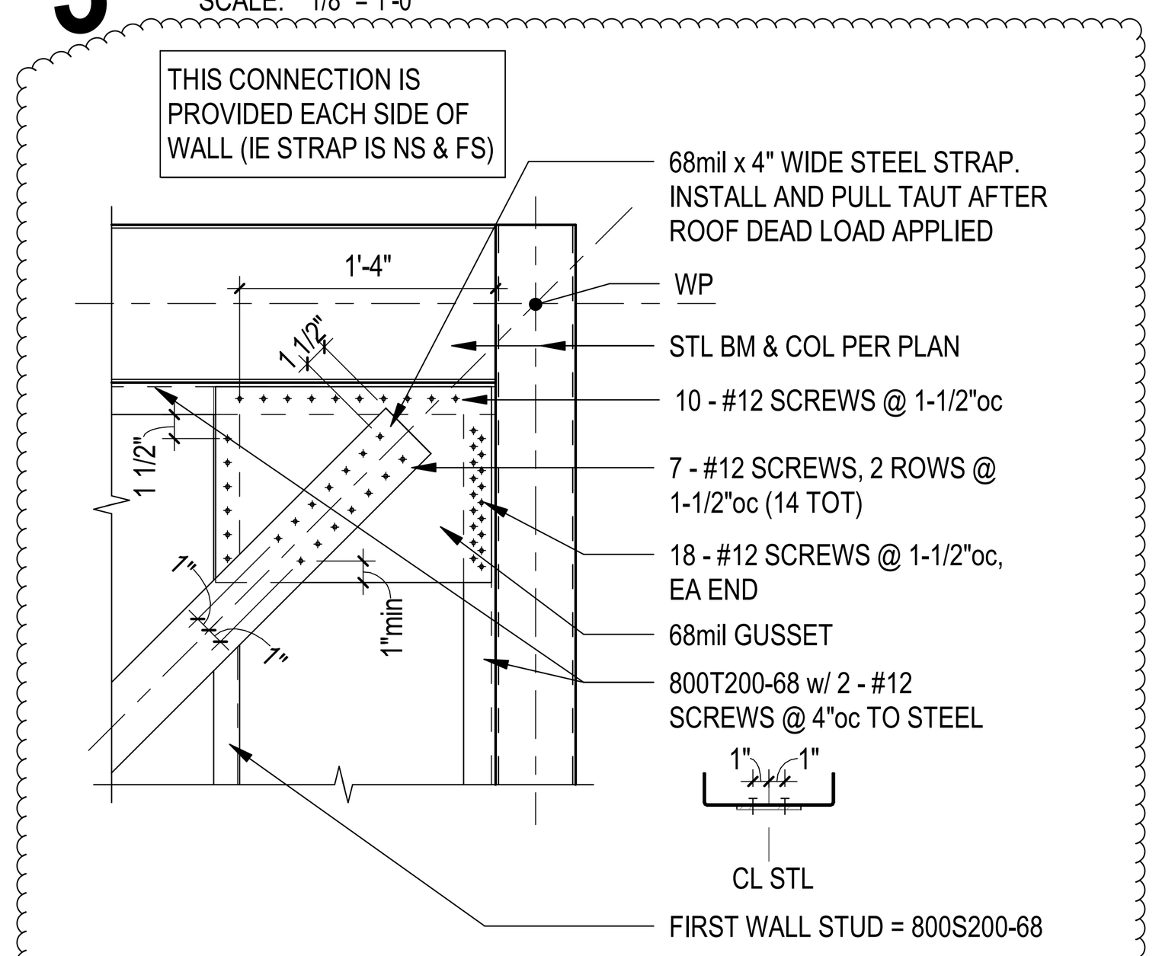
2 MECH PENTHOUSE EAST WALL ELEVATION
SCALE: 1/8" = 1'-0"



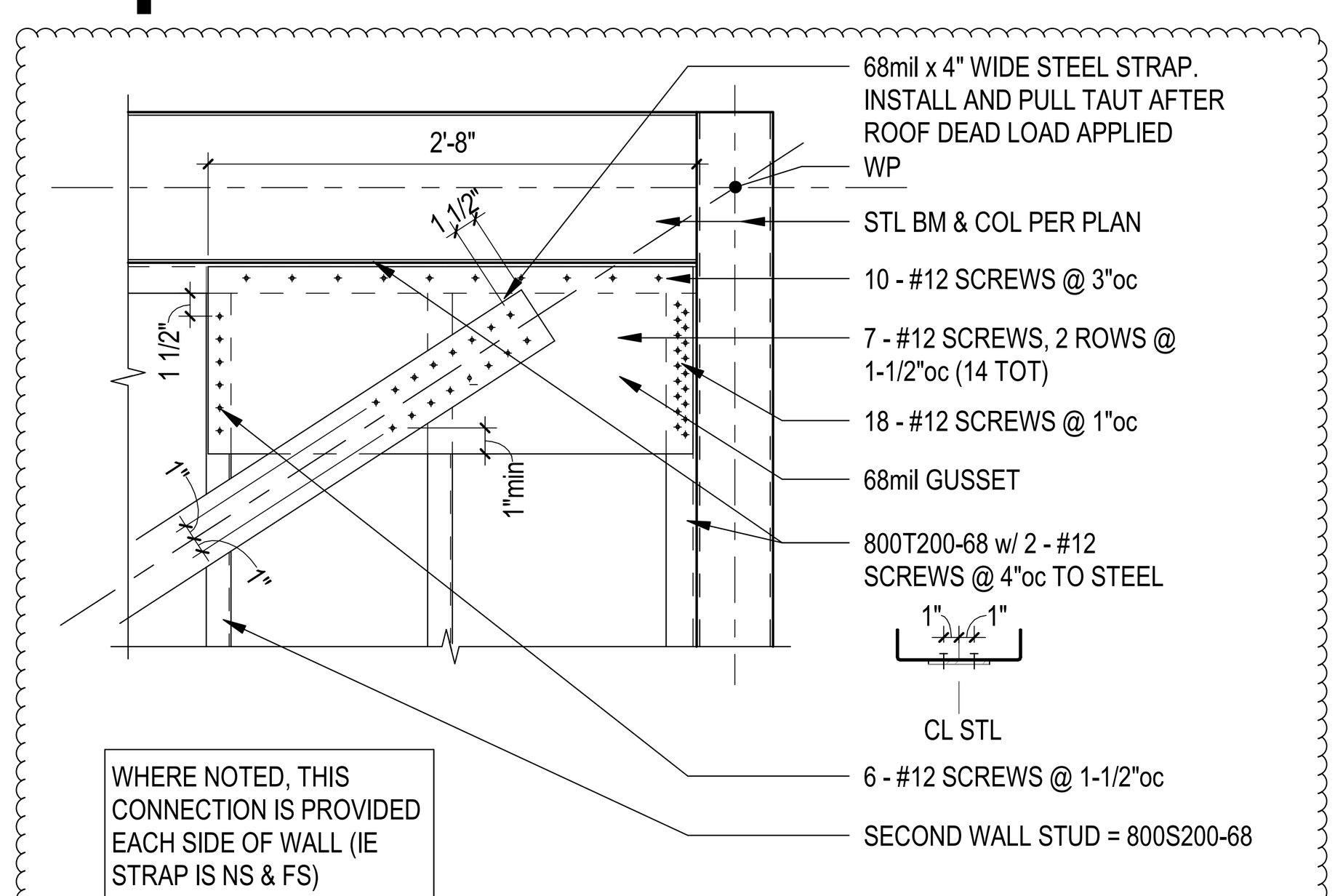
3 MECH PENTHOUSE SOUTH WALL ELEVATION
SCALE: 1/8" = 1'-0"



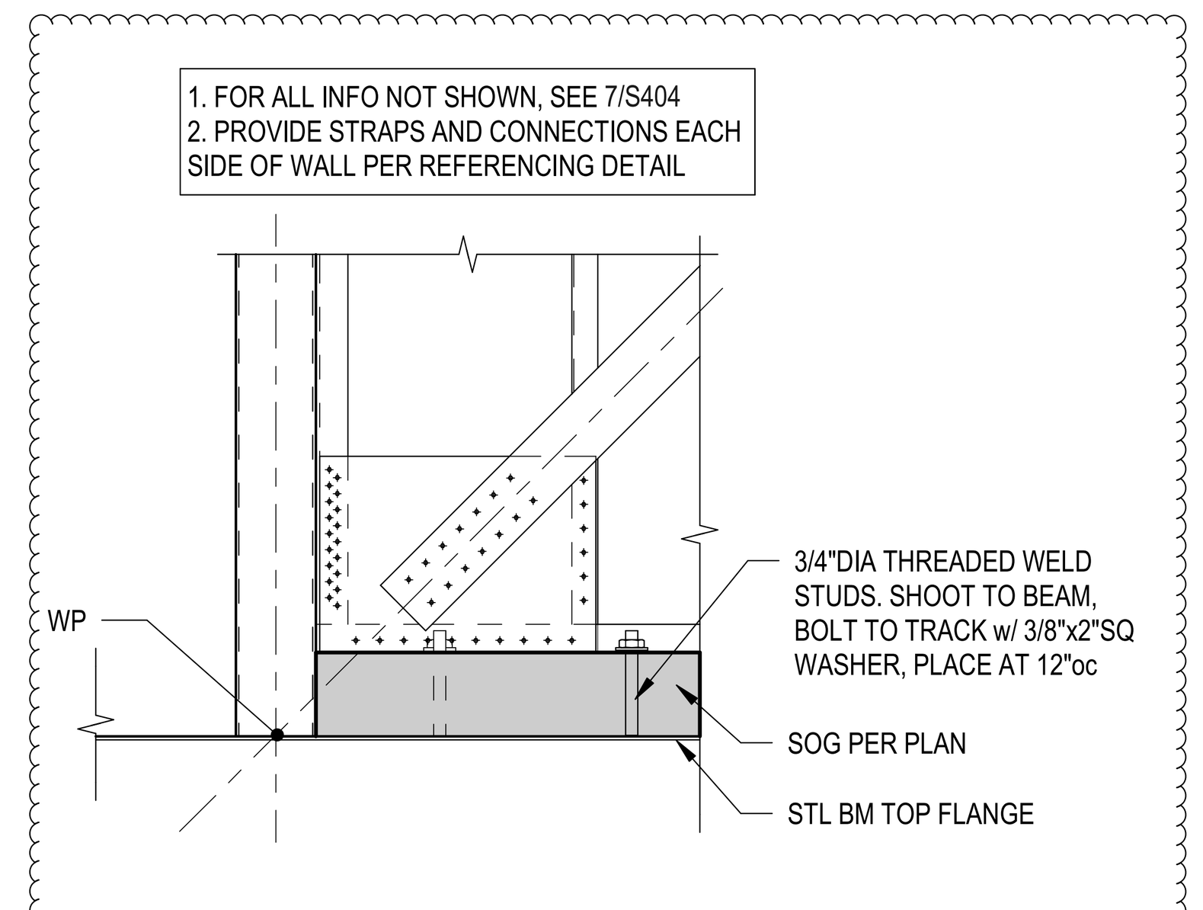
4 MECH PENTHOUSE WEST WALL ELEVATION
SCALE: 1/8" = 1'-0"



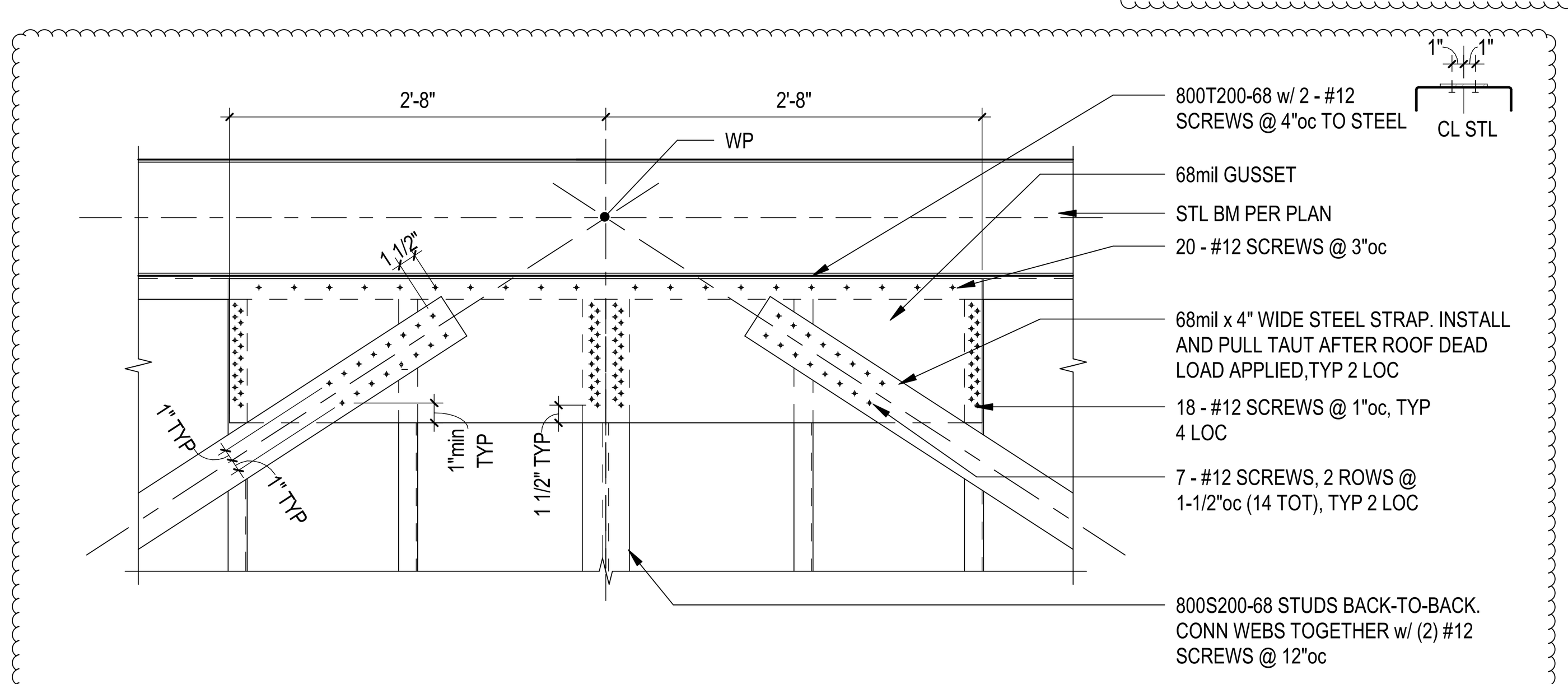
7 DETAIL CONN
SCALE: 1" = 1'-0"



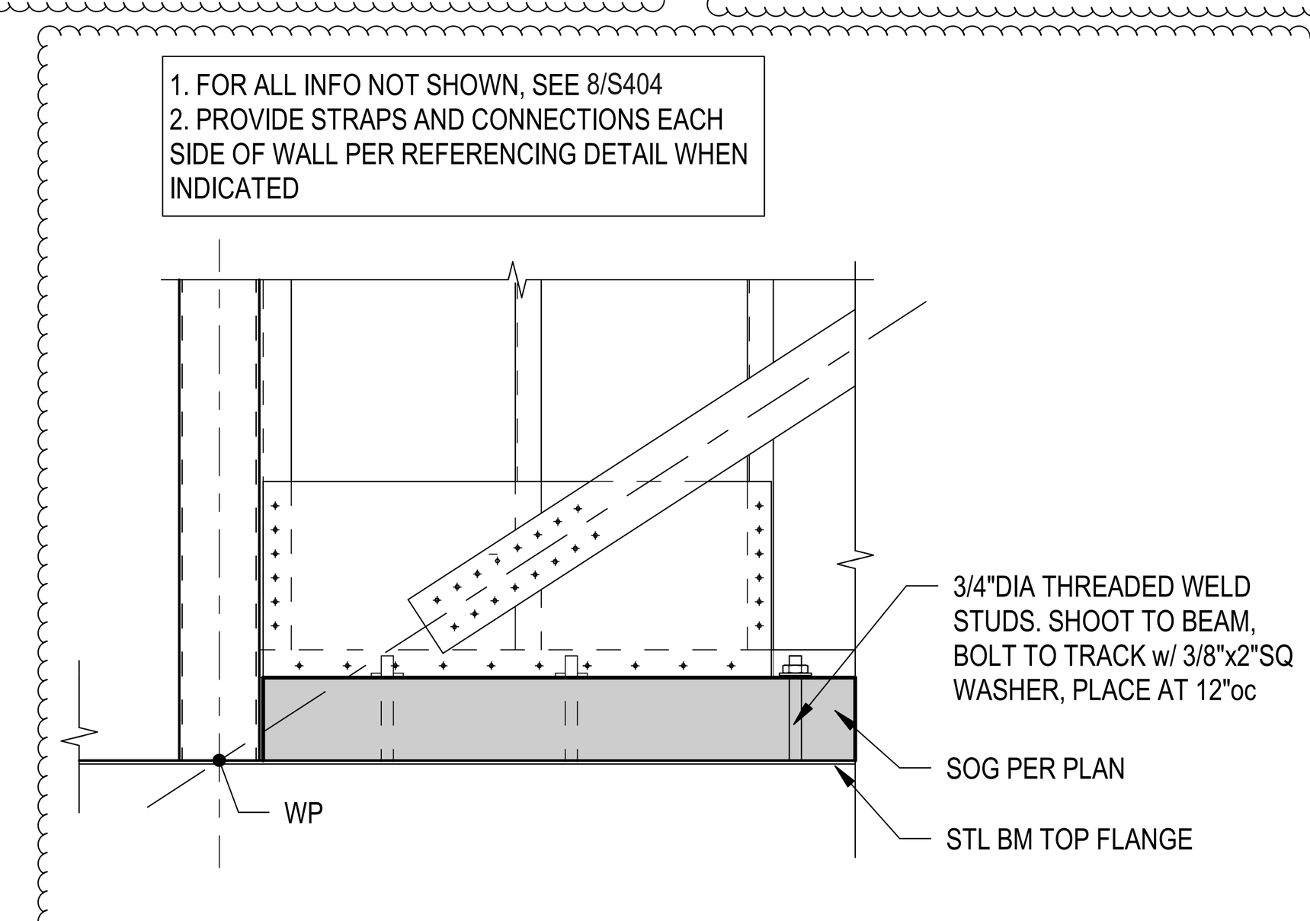
8 DETAIL CONN
SCALE: 1" = 1'-0"



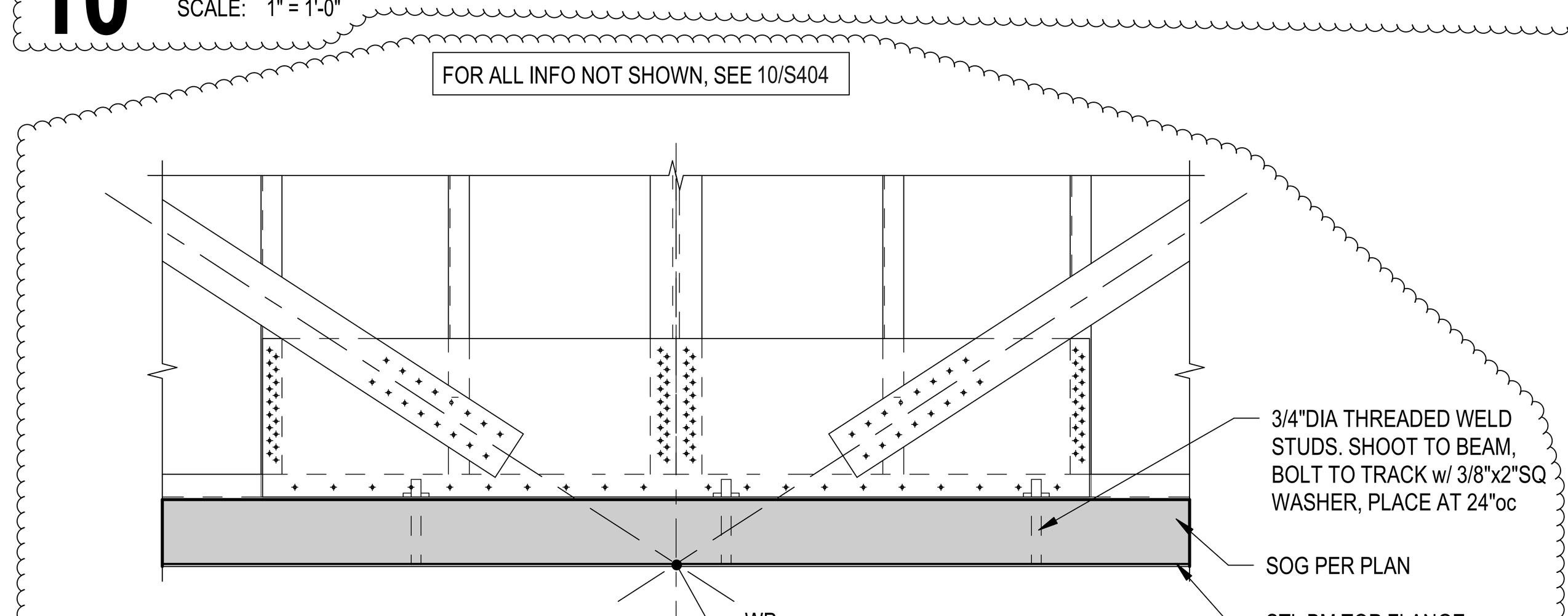
9 DETAIL CONN
SCALE: 1" = 1'-0"



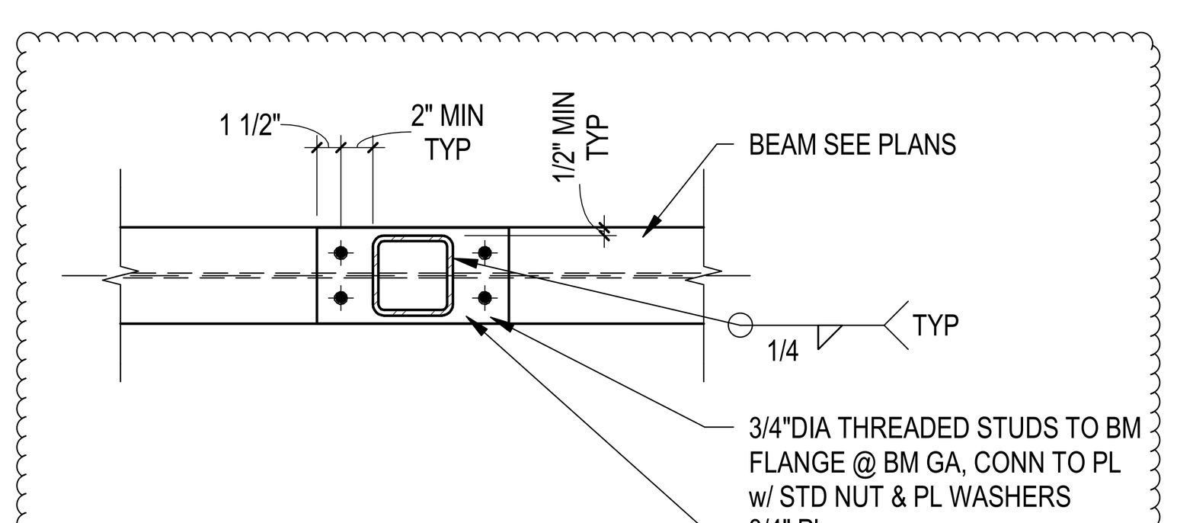
10 DETAIL CONN
SCALE: 1" = 1'-0"



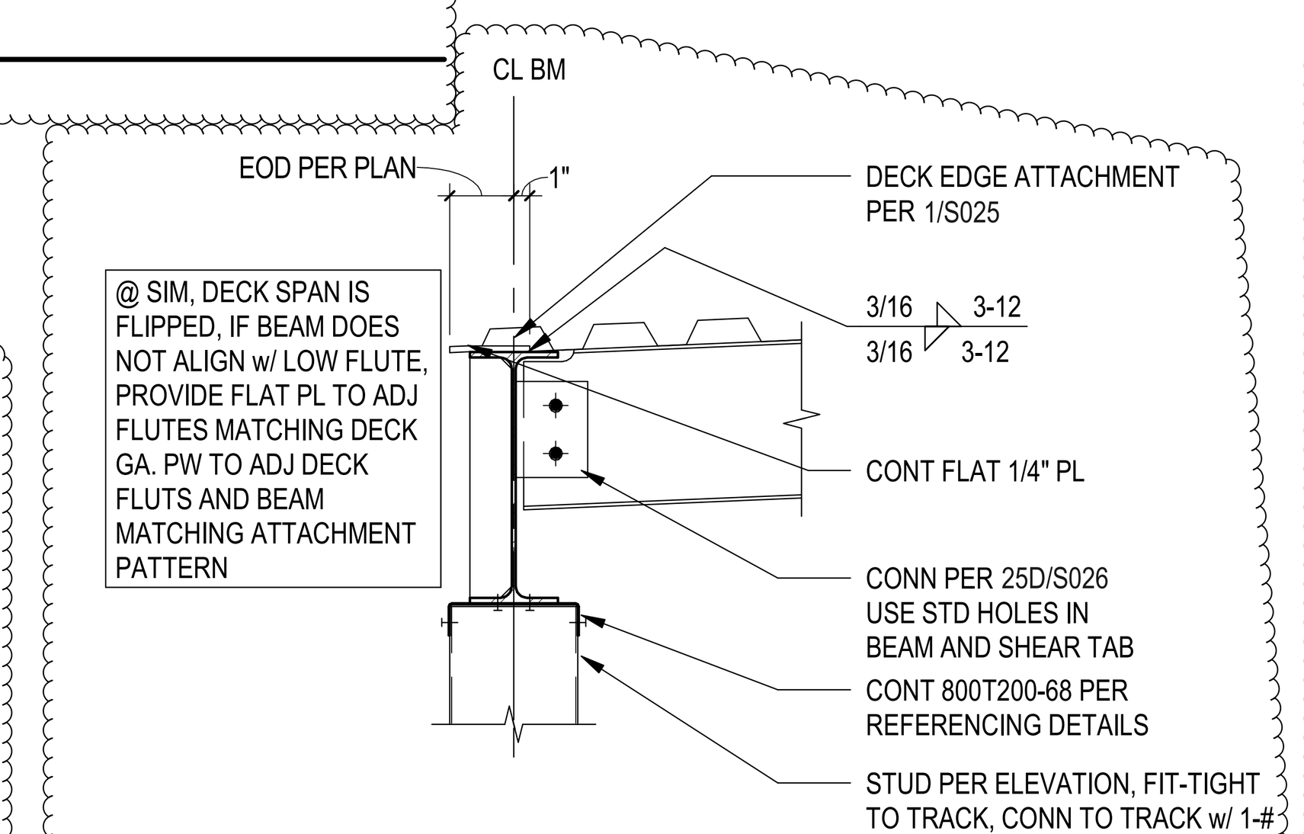
11 DETAIL CONN
SCALE: 1" = 1'-0"



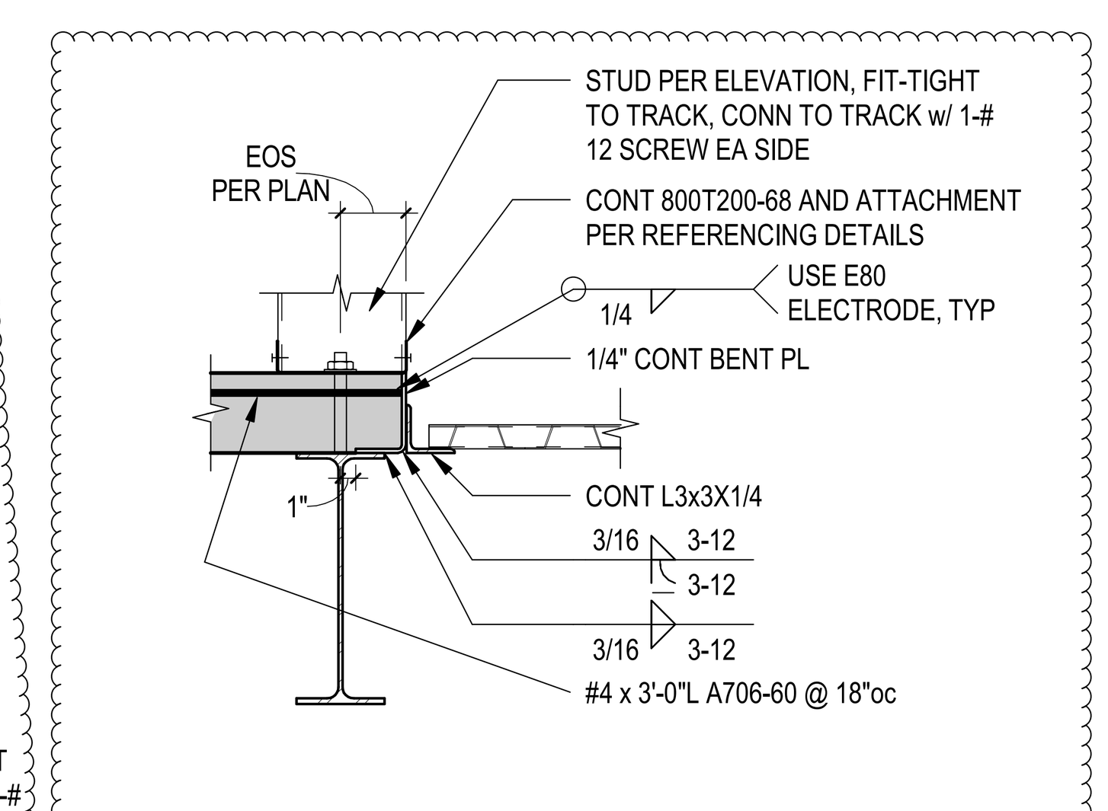
12 DETAIL CONN
SCALE: 1" = 1'-0"



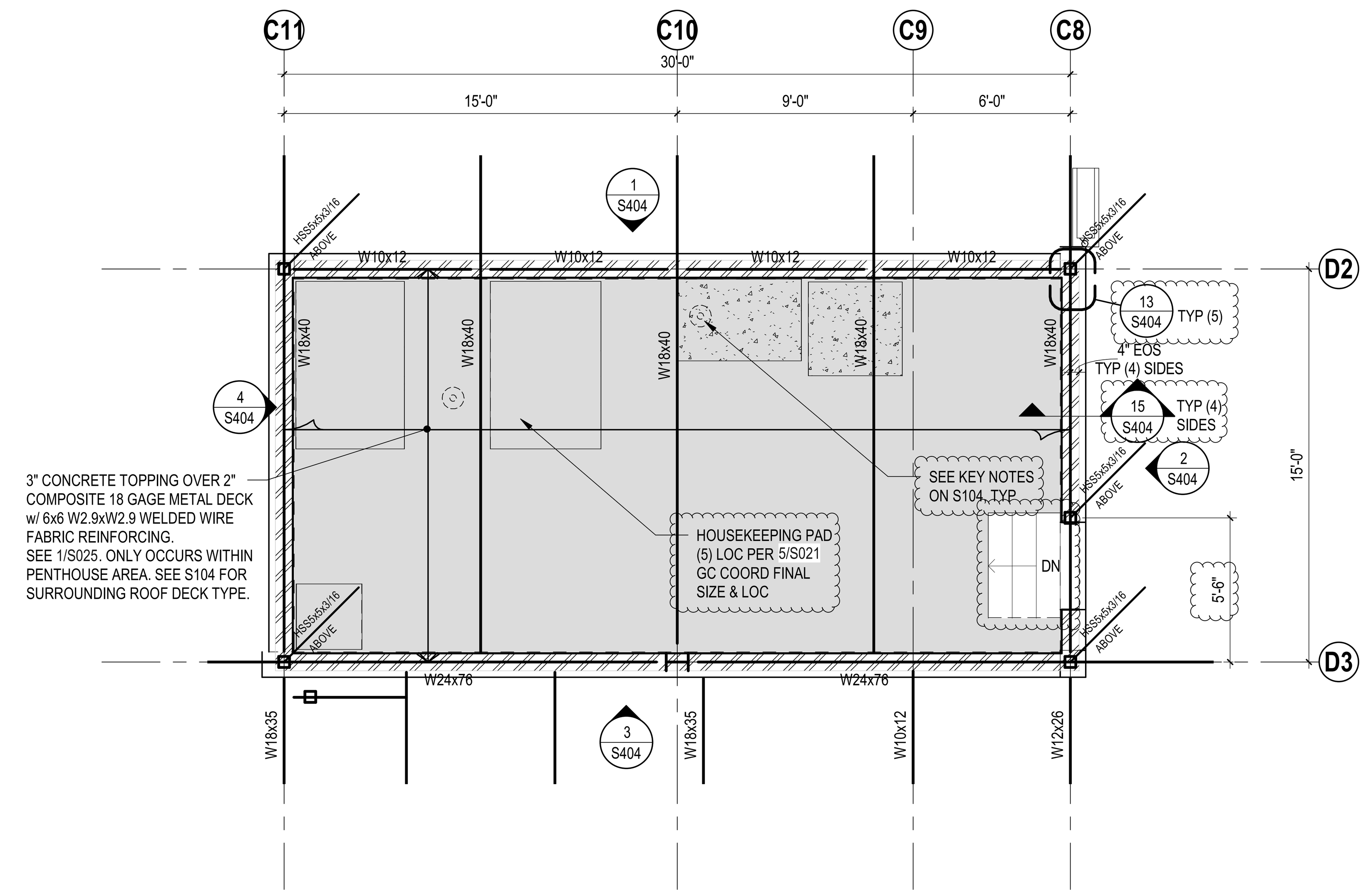
13 DETAIL CONN
SCALE: 1" = 1'-0"



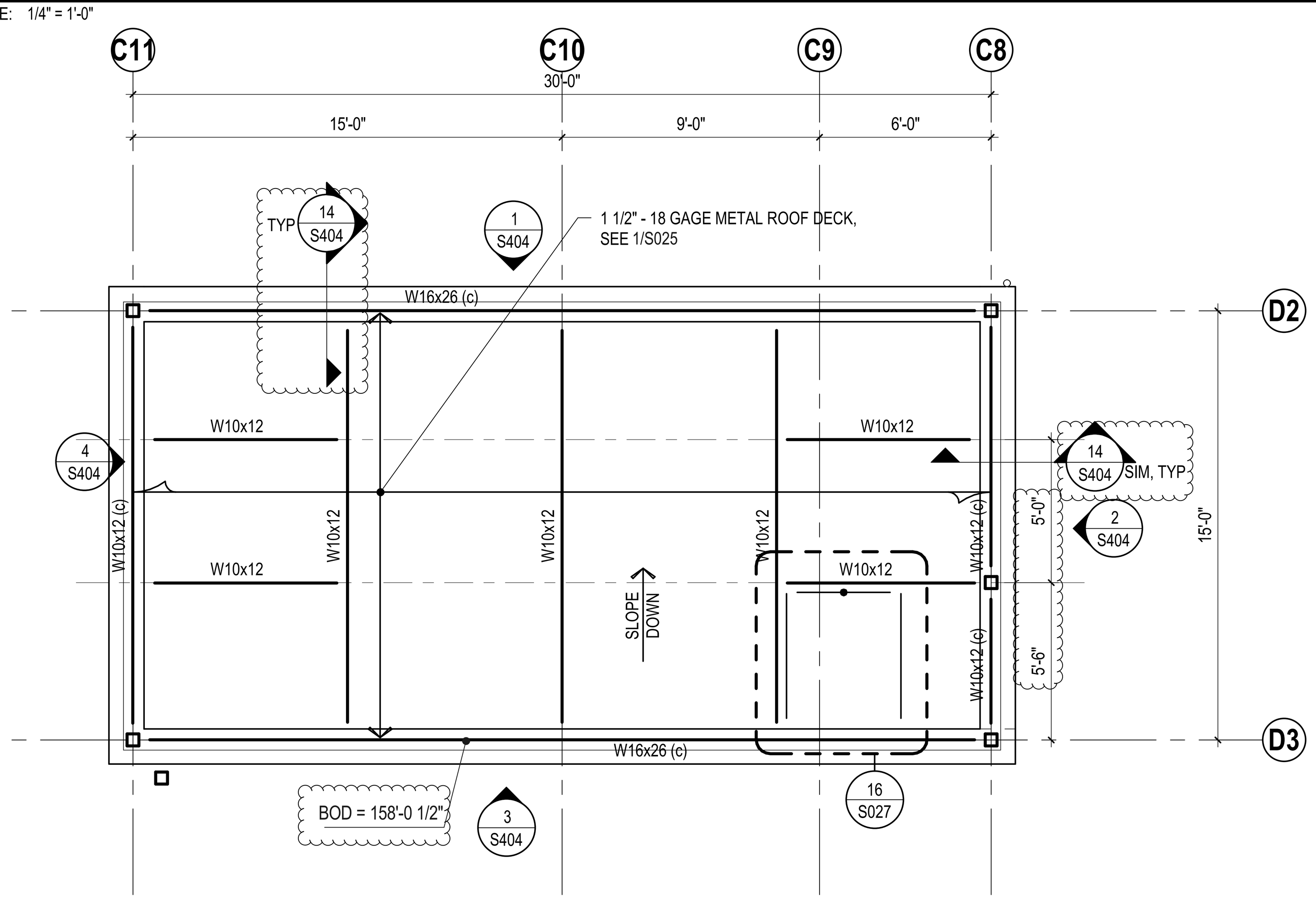
14 DETAIL CONN
SCALE: 1" = 1'-0"



15 DETAIL CONN
SCALE: 1" = 1'-0"



5 ENLARGED PLAN - MECHANICAL PENTHOUSE
SCALE: 1/4" = 1'-0"



6 ENLARGED PLAN - MECHANICAL PENTHOUSE ROOF
SCALE: 1/4" = 1'-0"

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PIERCE COLLEGE PUYALLUP NEW STEM BUILDING
1601 39th AVE SE
Puyallup, WA 98374

Date:	10/07/2022
Job No.:	22135.00
Drawn By:	RB/MW
Checked by:	TD

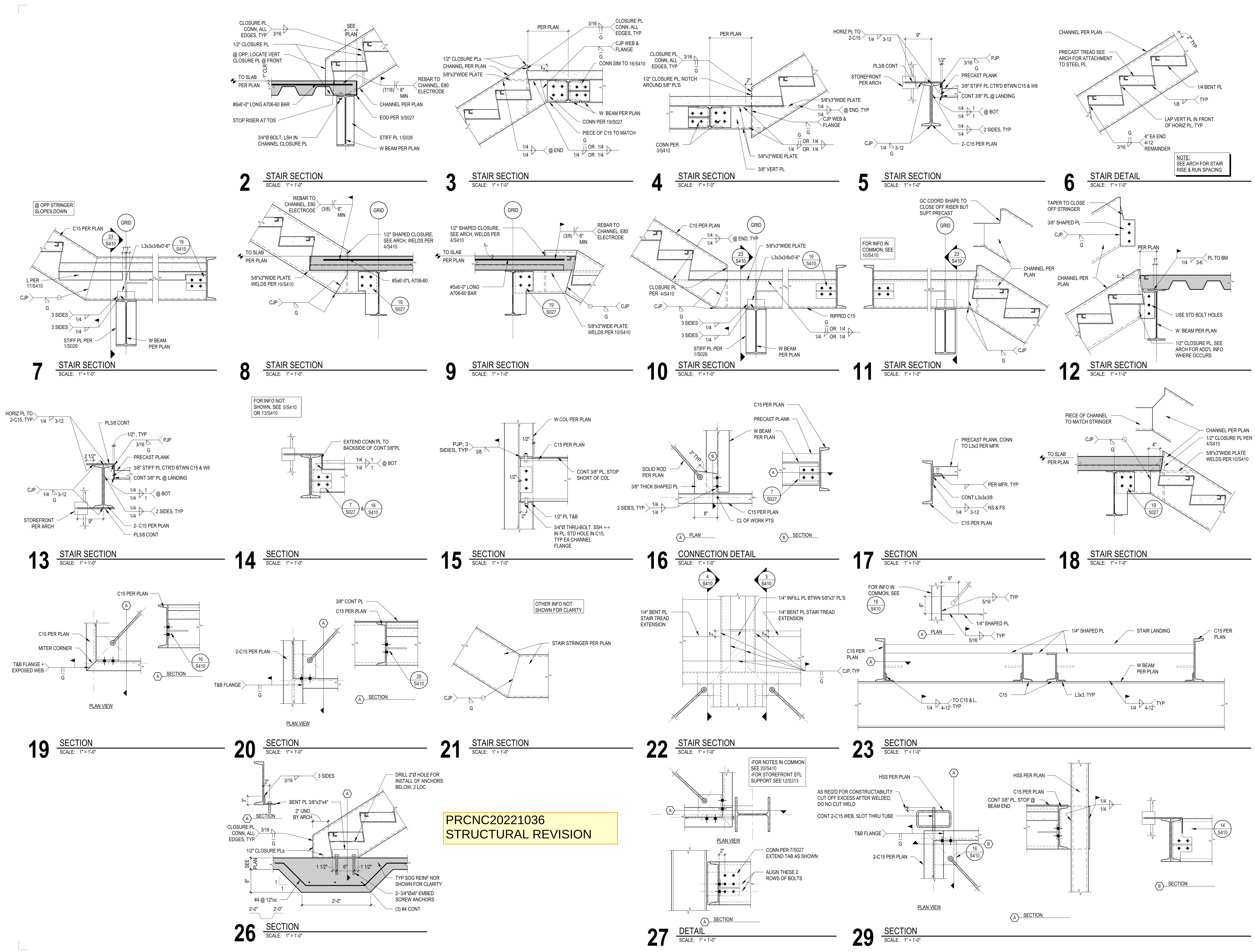
Revisions

Date	Description
1.30.23	IFC Revisions

ENLARGED MECHANICAL PENTHOUSE PLAN

S404

IFC SET



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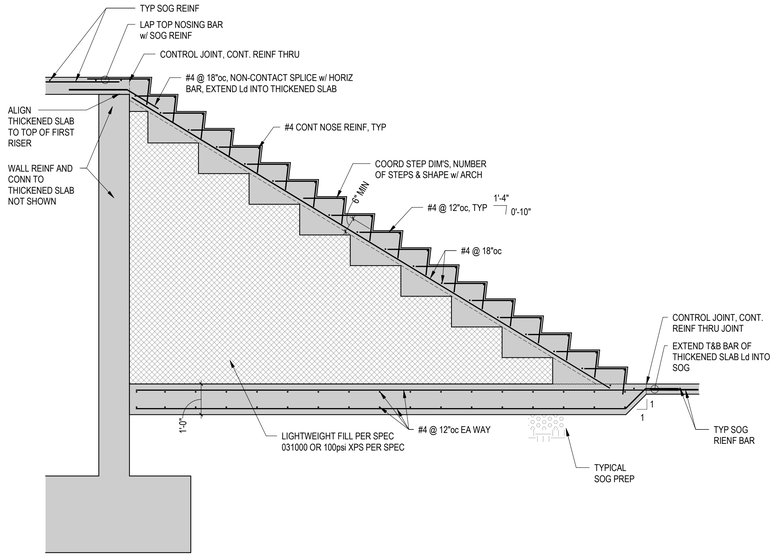
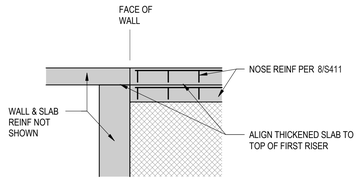
STAIR DETAILS & SECTIONS

S410

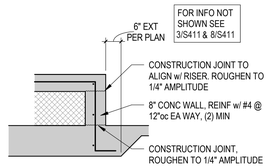
IFC SET



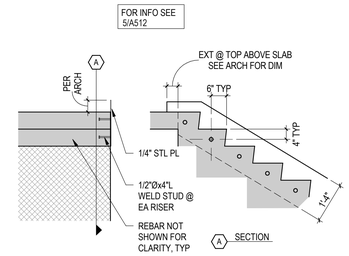
8 SECTION
SCALE: 1/2" = 1'-0"



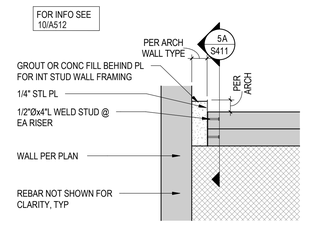
3 SECTION
SCALE: 1/2" = 1'-0"



4 SECTION
SCALE: 1/2" = 1'-0"



5 SECTION
SCALE: 1/2" = 1'-0"



6 SECTION
SCALE: 1/2" = 1'-0"

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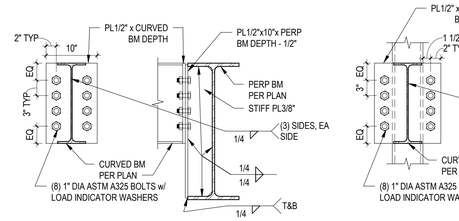
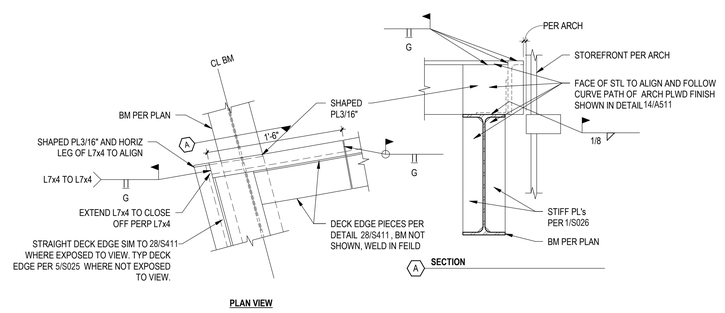
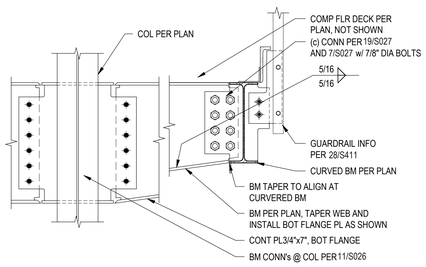


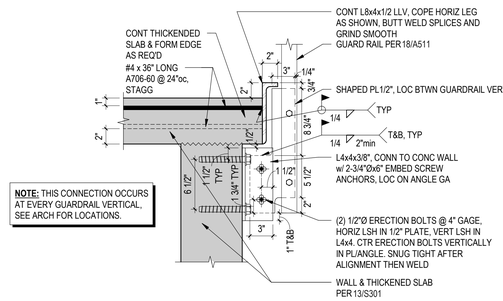
19 CURVED BEAM END CONN
SCALE: 1" = 1'-0"



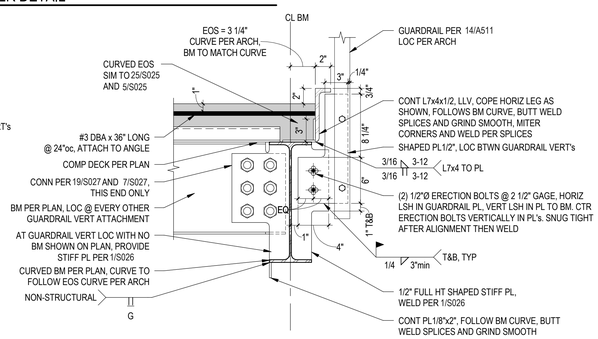
21 LEVEL 2 OPENING CURVED EDGE OF SLAB CORNER DETAIL
SCALE: 1 1/2" = 1'-0"



25 LEVEL 2 OPENING CURVED EDGE OF SLAB
SCALE: 1" = 1'-0"



27 GRAVITY LAB GUARDRAIL
SCALE: 1 1/2" = 1'-0"



28 LEVEL 2 OPENING CURVED EDGE OF SLAB DETAIL
SCALE: 1 1/2" = 1'-0"

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38621
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PROFESSIONAL ENGINEER

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PIERCE COLLEGE PUYALLUP
NEW STEM BUILDING

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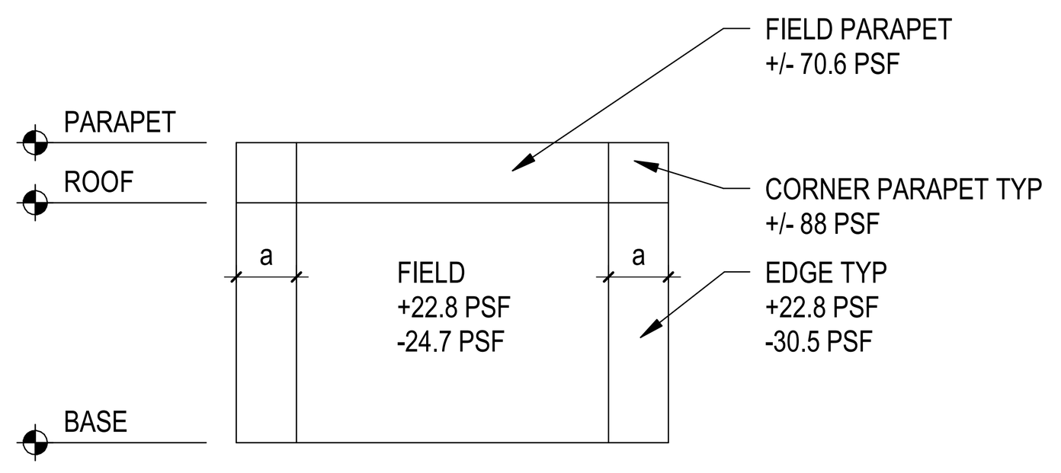
STAIR DETAILS & SECTIONS

S411

IFC SET



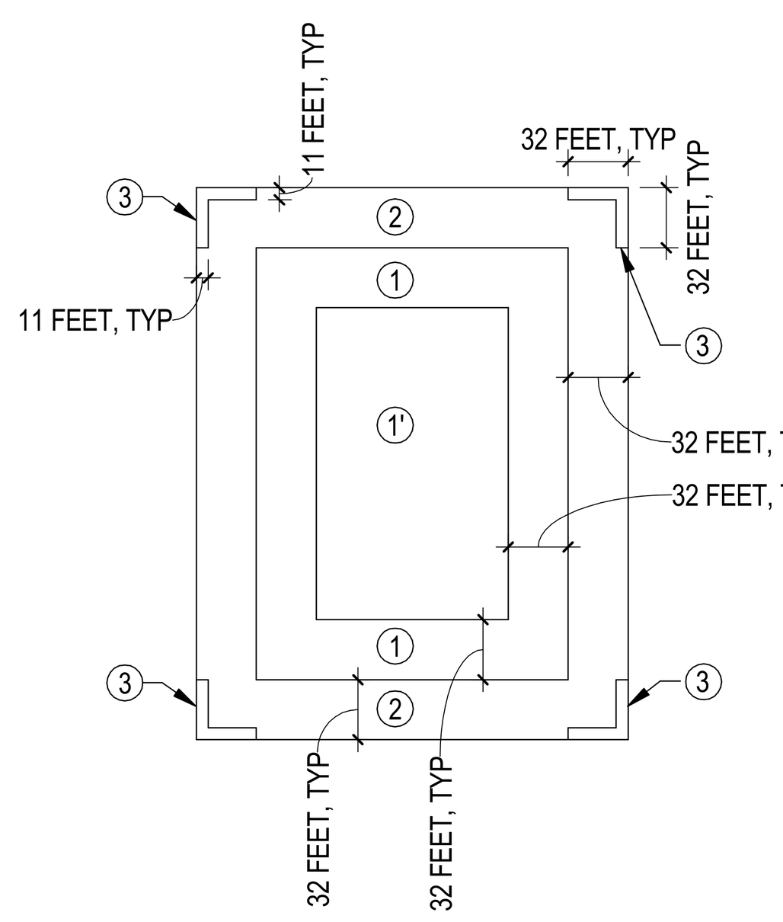
NOTES:
1. "a" IS 10% OF LEAST HORIZONTAL DIMENSION OR 21.5 FEET, WHICHEVER IS SMALLER, BUT NOT LESS THAN 4% OF LEAST HORIZONTAL DIMENSION OR 3 FEET.
3. REFERENCE ASCE 7-16 FIGURE 30.3-1 AND FIGURE 30.8-1
2. SEE **WIND PRESSURE NOTES** ON THIS SHEET FOR ADDITIONAL INFORMATION.

WIND PRESSURE NOTES:

1. WIND PRESSURES SHOWN ARE DETERMINED PER IBC 2018 SECTION 1609, ASCE 7-16 CHAPTER 26, AND ASCE 7-16 CHAPTER 30 PART 1. ROOF WIND UPLIFT PRESSURES ARE GROSS (WEIGHT OF MATERIAL HAS NOT BEEN DEDUCTED). WIND PRESSURES GIVEN ARE ULTIMATE.
2. PRESSURES SHOWN ARE CALCULATED USING BUILDING EFFECTIVE AREA FOR WORST CASE.
3. EXTERIOR COMPONENTS AND CLADDING SHALL BE DESIGNED FOR WORST CASE WIND LOAD PRESSURES SHOWN. ALTERNATIVELY, WIND LOADS MAY BE DETERMINED DIRECTLY FROM THE PROVISIONS OF THE IBC 2018/ ASCE 7-16 SECTIONS NOTED ABOVE USING THE WIND LOAD CRITERIA IN THE GENERAL NOTES.
4. INWARD PRESSURES ACT TOWARD THE BUILDING (POSITIVE), OUTWARD PRESSURES ACT AS A SUCTION ON THE BUILDING SURFACE (NEGATIVE).

1 COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM - WALLS

SCALE: 3/4" = 1'-0"



ROOF PRESSURES

(1') -20.9 PSF
+9.3 PSF

(1) -39.3 PSF
+9.3 PSF

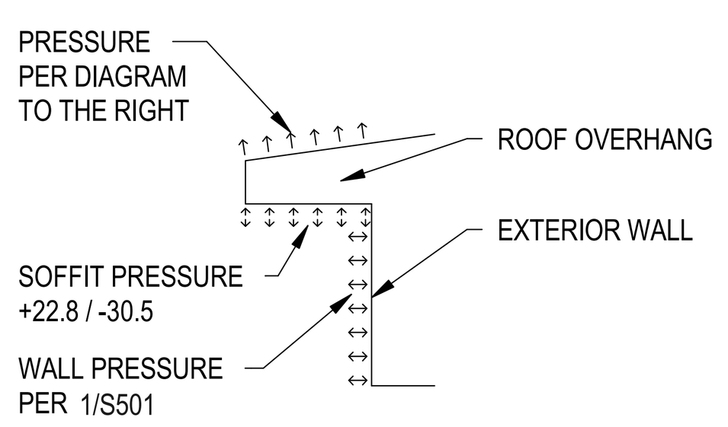
(2) -47.8 PSF
+9.3 PSF

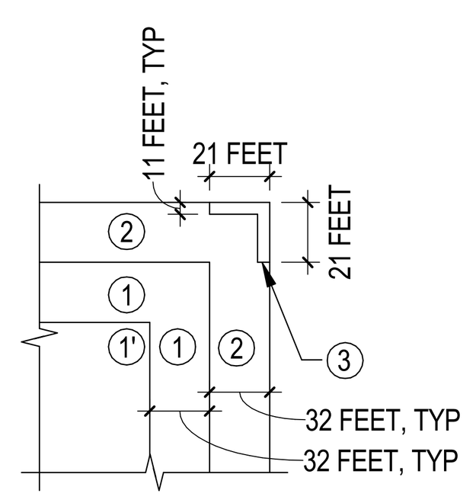
(3) -65.2 PSF
+9.3 PSF

NOTES:
1. REFERENCE ASCE 7-16 FIGURE 30.3-2A
2. SEE **WIND PRESSURE NOTES** ON THIS SHEET FOR ADDITIONAL INFORMATION.

2 COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM - ROOFS

SCALE: 3/4" = 1'-0"





FRONT ROOF OVERHANG PRESSURES APPLIED TO TOP SURFACE

(1') -34.9 PSF

(1) -34.9 PSF

(2) -45.5 PSF

(3) -59.3 PSF

NOTES:
1. REFERENCE ASCE 7-16 FIGURE 30.9-1 AND FIGURE 30.3-2A WITH ASSOCIATED NOTES
2. SEE **WIND PRESSURE NOTES** ON THIS SHEET FOR ADDITIONAL INFORMATION.

3 COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM - FRONT ROOF OVERHANG

SCALE: 3/4" = 1'-0"

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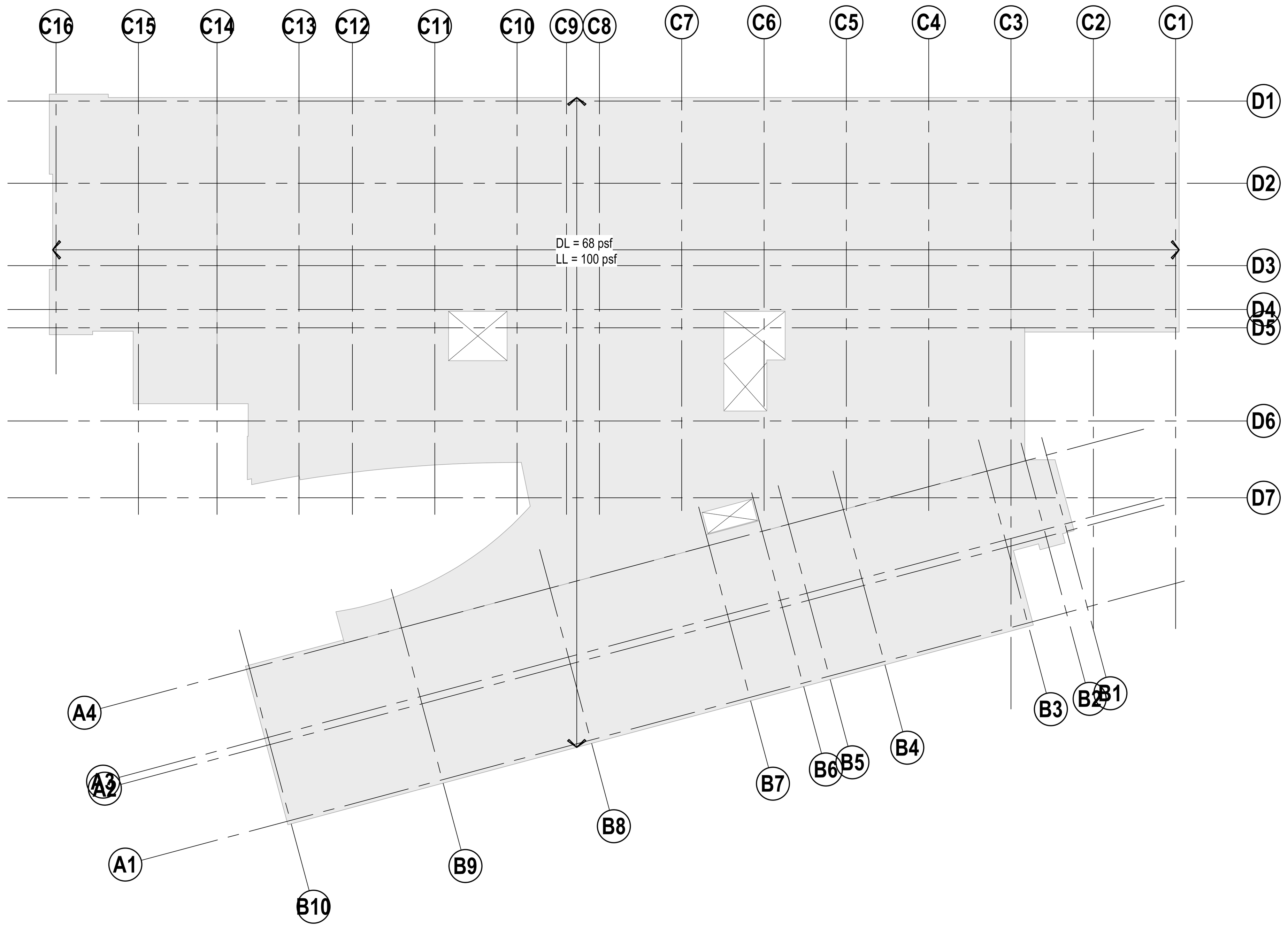
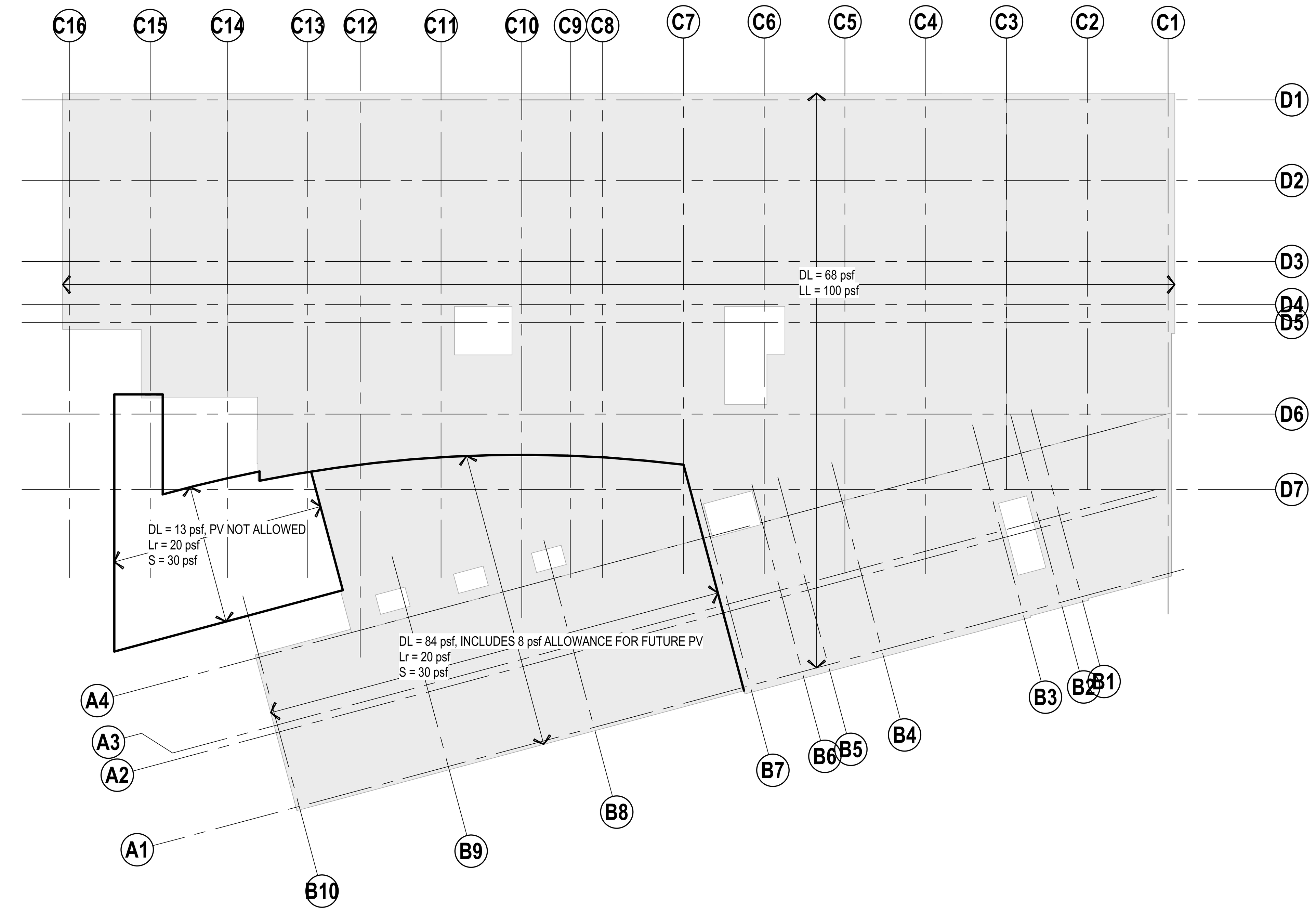
LOAD DIAGRAMS

S501

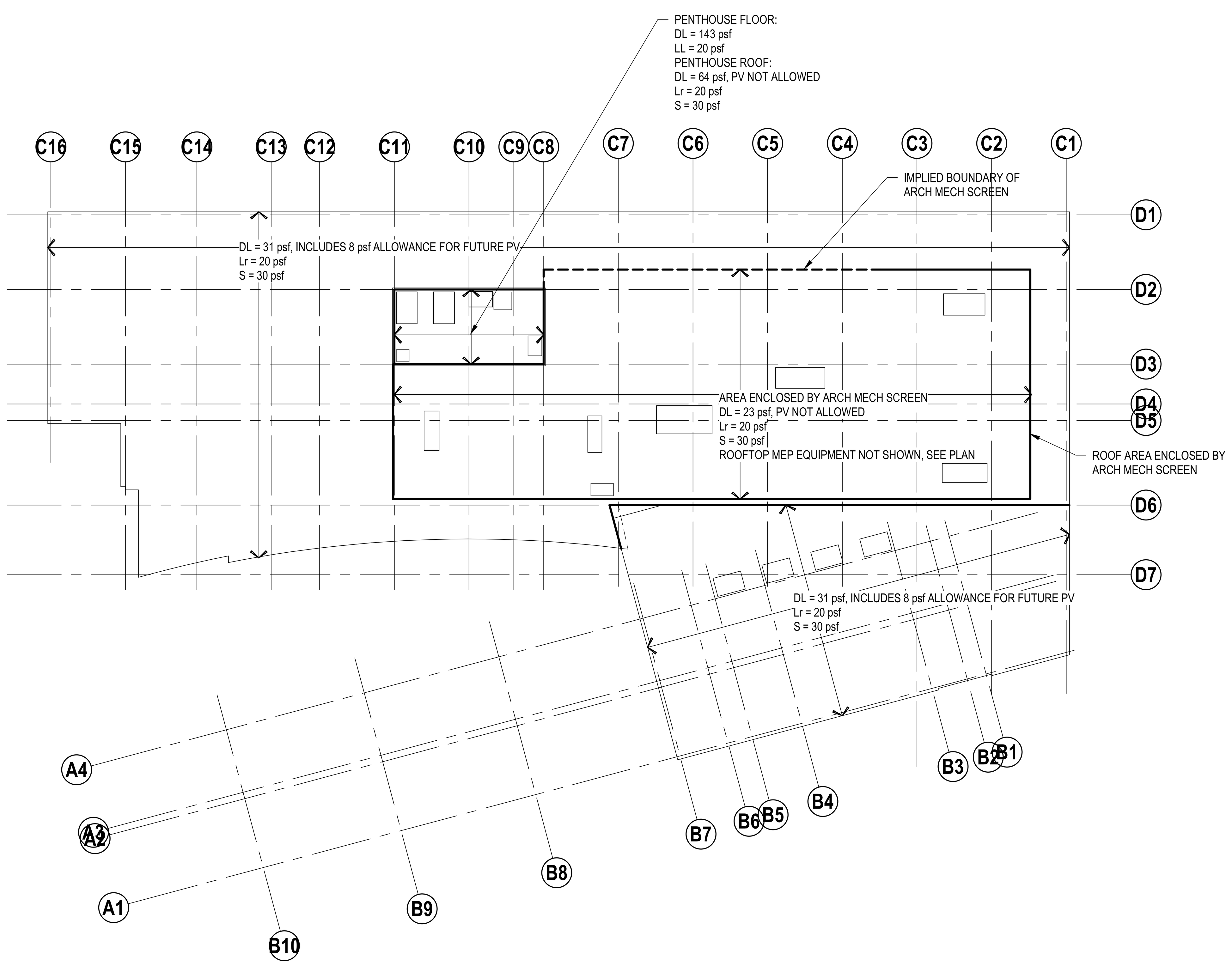
IFC SET



1 LEVEL 2 LOAD MAP
SCALE: 1/16" = 1'-0"



2 LEVEL 3 LOAD MAP
SCALE: 1/16" = 1'-0"



3 ROOF LOAD MAP
SCALE: 1/16" = 1'-0"

LOAD MAP NOTES:

DL = DEAD LOAD
LL = LIVE LOAD
Lr = ROOF LIVE LOAD
S = SNOW LOAD
PV = FUTURE PHOTOVOLTAIC PANEL LOAD ALLOWANCE, NOTE 5

1. SEE S001 FOR ADDITIONAL LOADS.
2. SEE PLANS FOR MEP EQUIPMENT MAX WEIGHTS.
3. SEE S501 FOR WIND LOAD DIAGRAMS.
4. LIVE LOAD REDUCTION IS APPLIED AS ALLOWED BY THE REFERENCED CODE
5. THE SHOWN PV PANEL LOAD ALLOWANCES ARE INCLUDED IN THE TOTAL DEAD LOAD SHOWN.
6. DEAD LOADS DO NOT INCLUDE SELF WEIGHT OF FLOOR/ROOF FRAMING MEMBERS.

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LOAD MAPS

S502

IFC SET