

BRADLEY HEIGHTS
TRAFFIC IMPACT ANALYSIS

PUYALLUP, WA



3/31/2023

Prepared for: Mr. Dave Enslow
Bradley Heights SS LLC
1816 11th Avenue, Unit C
Seattle, WA 98122

March 2023

March 31, 2023

Bryan Roberts, P.E.
City of Puyallup

Subject: Revisions to Bradley Heights Traffic Impact Analysis

This letter addresses updates to the Bradley Heights Traffic Impact Analysis review based on updated unit counts and existing site parcel data.

1. The unit count for the proposed development had been updated to 236 per the latest site plan.
2. Existing PM peak hour counts were updated by applying a 3 percent growth rate over two years to the previous 2021 counts that were collected. The horizon year was adjusted to 2026.
3. Existing parcel data confirmed 48 previous units on the site. A list of the existing tax parcels can be found in the appendix.

Please call if you require anything further.

Sincerely,

Aaron Van Aken, P.E., PTOE

August 24, 2022

Bryan Roberts, P.E.
City of Puyallup

Subject: Revisions to Bradley Heights Traffic Impact Analysis

This letter is in response to the Bradley Heights Traffic Impact Analysis review, with comments dated August 11, 2022.

1. The annual growth rate has been changed from two percent to three percent. All figures and level of service calculations have been adjusted accordingly. The 52 units was provided and used in the scoping and TIA. This number can be adjusted should it differ from the actual existing number of units.

2. The unit count for the proposed development has not changed due to the development proposing 233 units which is less than 248 units which is called out in the report. The analysis is considered conservative.

Please call if you require anything further.

Sincerely,

Aaron Van Aken, P.E., PTOE

BRADLEY HEIGHTS
TRAFFIC IMPACT ANALYSIS

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BRADLEY HEIGHTS TRAFFIC IMPACT ANALYSIS

1. INTRODUCTION

The main goals of this study focus on the assessment of existing roadway conditions and forecasts of newly generated project traffic. The first task includes the review of general roadway information on the adjacent streets serving the subject site and gathering existing vehicular volumes within a defined study area. Forecasts of future traffic and dispersion patterns on the street system are then determined using established trip generation and distribution techniques. As a final step, appropriate conclusions and mitigation measures are defined, if needed.

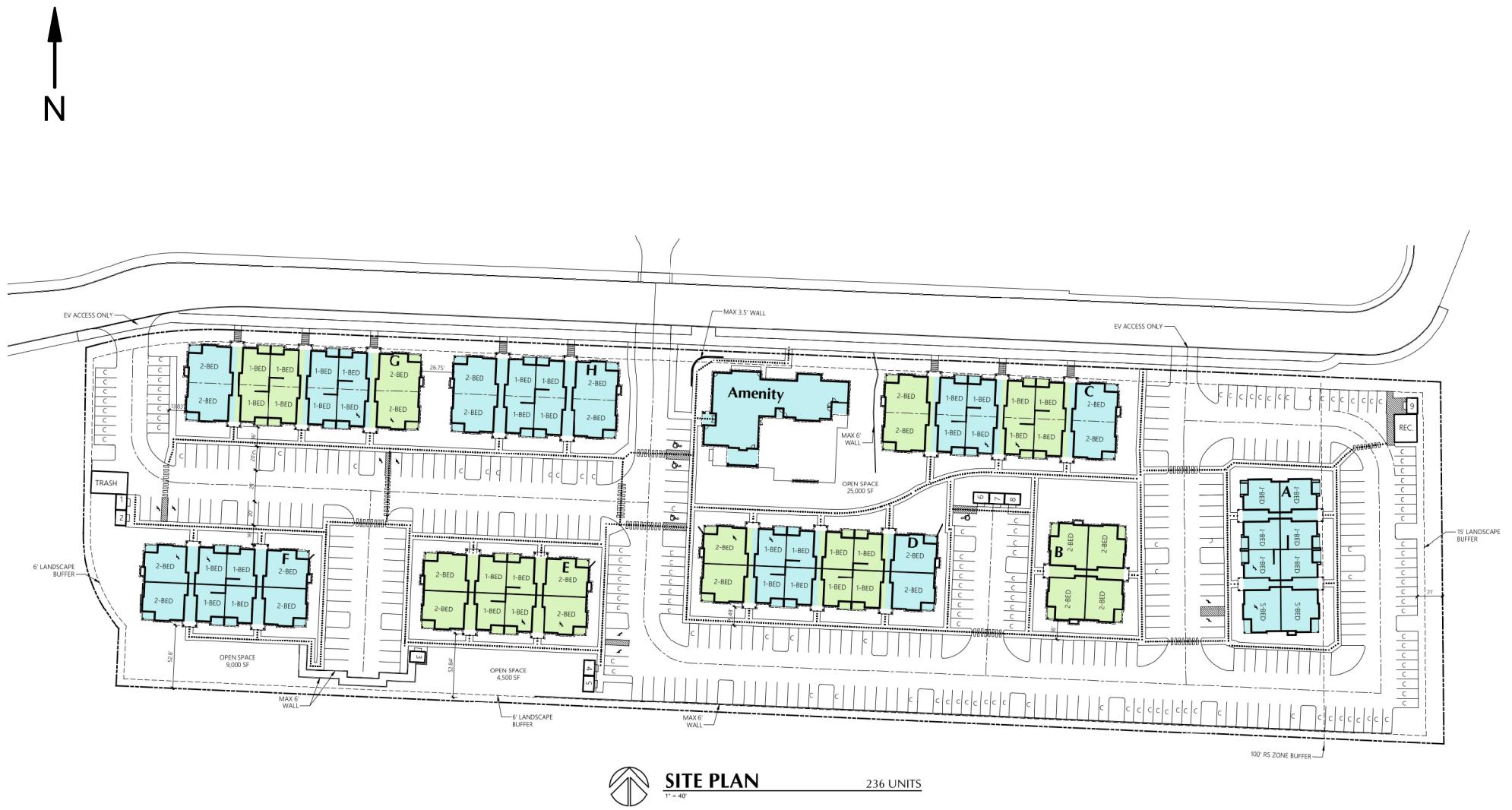
2. PROJECT DESCRIPTION

Bradley Heights is a proposed residential development encompassing up to 236 multi-family dwelling units in the city of Puyallup. The subject site, with a site address of 202 27th Avenue SE, is located within 7.78-acre tax parcel #: 0419036006. Currently existing on-site is a mobile home park encompassing 48 dwelling units. Access to the subject site is proposed via three driveways (the eastern and western most are emergency access only), all extending south from 27th Avenue SE. The primary, centrally located access is to be aligned opposite the westerly Sunset Garden Senior Living Apartment's driveway.

Figure 1 below shows the vicinity map and adjacent street system. A conceptual site plan illustrating the proposed site layout is presented in Figure 2.

Figure 1: Aerial Vicinity





3. EXISTING CONDITIONS

3.1 Surrounding Roadways

The major roadways surrounding the subject site are listed and described below.

S Meridian: is a north-south, 4-6-lane major arterial located west of the subject site. Two travel lanes in either direction and a center two-way left-turn lane are provided in the vicinity of the subject site. Travel lanes are approximately 10- to 11-feet in width. Crosswalks and turn-lanes are provided at major intersections. Curb, gutter and sidewalk are generally provided along the either side of the roadway. The posted speed limit is 35-mph.

27th Avenue SE: is an east west, 2-3 lane local roadway bordering the subject site to the north. One travel lane in either direction and a center two-way left-turn lane are provided along the project frontage. Travel lanes are approximately 10- to 12-feet in width. Curb, gutter and sidewalk are provided along the north side of the roadway in the subject site vicinity. Shoulders are generally untreated along the south side of the roadway. The posted speed limit is 25-mph.

3.2 Transit Service

According to the Pierce Transit regional bus schedule, Routes 402 and 425 provide service within walking distance of the proposed Bradley Heights development. Route 402—Meridian— provides service from Meridian E & 171st Street Court E to the Federal Way Transit Center. The nearest stop for Route 402 is located approximately 0.20-miles southwest of the proposed development at the intersection of S Meridian & 28th Avenue SE. Weekday service is provided from 5:00 AM – 8:46 PM with approximately 60-minute headways. Saturday service is provided from 7:10 AM – 8:35 PM with approximately 60-minute headways. Sunday service is provided from 9:41 AM – 7:26 PM with approximately 60-minute headways. Route 425—Puyallup Connector— provides service from the South Hill P&R to the Puyallup Station. The nearest stop for Route 425 in relation to the subject site is located across from the proposed development in the Sunset Garden Senior Living Apartments parking lot. Weekday service is provided from 11:19 AM – 5:18 PM with approximately 60-minute headways. Saturday service is provided from 9:15 AM – 6:27 PM with approximately 120-minute headways.

Transit use stemming from the proposed development is anticipated given the service proximity and availability. For further details and information, refer to the Pierce County Transit schedule.

3.3 Roadway Improvements

The current City of Puyallup Six-Year (2023-2028) Transportation Improvement Program was reviewed in order to determine if any projects are planned in the vicinity of the Bradley Heights development. Table 1 provides project descriptions of proposed improvements in the subject site vicinity.

Table 1: Transportation Improvement Projects

Name	Location	Improvement	Cost
Bike Facility Improvements (P.N.: 2017-015)	5th St SE/7th St SE; 23rd Ave SE to 43rd Ave SE	Add a shared use path on one side of the roadway.	\$7,000,000
9th Street SW (P.N.: 2016-062)	15th Ave SW to 31st Ave SW	3 lanes with curb, gutter, sidewalk, bike lane and street lighting on both sides of roadway. Add capacity at 9th St SW & 31 st Ave SW.	\$18,510,000
5th St SE/7 th St SE (P.N.: 2017-015)	23rd Ave SE to 43rd Ave SE	Add shared use path on one side of roadway.	\$7,000,000
23rd Ave SE (P.N.: 2014-014)	S Meridian to 9th St SE	3 lanes with curb, gutter, sidewalk, bikelanes, and street lighting and a new signal at the intersection of 23rd Ave SE & 7th St SE	\$7,800,000
Adaptive on 5th St. SE (P.N.: 23)	Intersections on corridor	Install adaptive signals at: 23rd, 31st, 35th, 37th, 39th, 43rd (6 signals).	\$1,000,000
39th/37th Ave SE (P.N.: 2016-072)	10th St SE to 5th St SE	Overlay roadway and striping; Update from Meridian to 10th St SE.	\$2,200,000
7th St SE (P.N.: 2016-067)	23rd Ave SE to 12th Ave SE	Construct connection between 15th Ave SE and 12th Ave SE. The length of the project would update geometry to existing City Standards.	\$5,200,000

3.4 Non-Motorist Infrastructure

Presently, sidewalk is available along the north side of 27th Avenue SE and along both sides of S Meridian and 7th Street SE. The surrounding roadways provide crosswalks at major intersections, allowing safe pedestrian crossings. Continuous sidewalk infrastructure is provided between the proposed subject site and public transit opportunities as well as commercial opportunities to the south and north along S Meridian. Moreover, Bradley Lake Park is located approximately 0.4-miles walking distance southeast of the subject site, providing recreational opportunities to future Bradley Heights residents.

3.5 Peak Hour Volumes

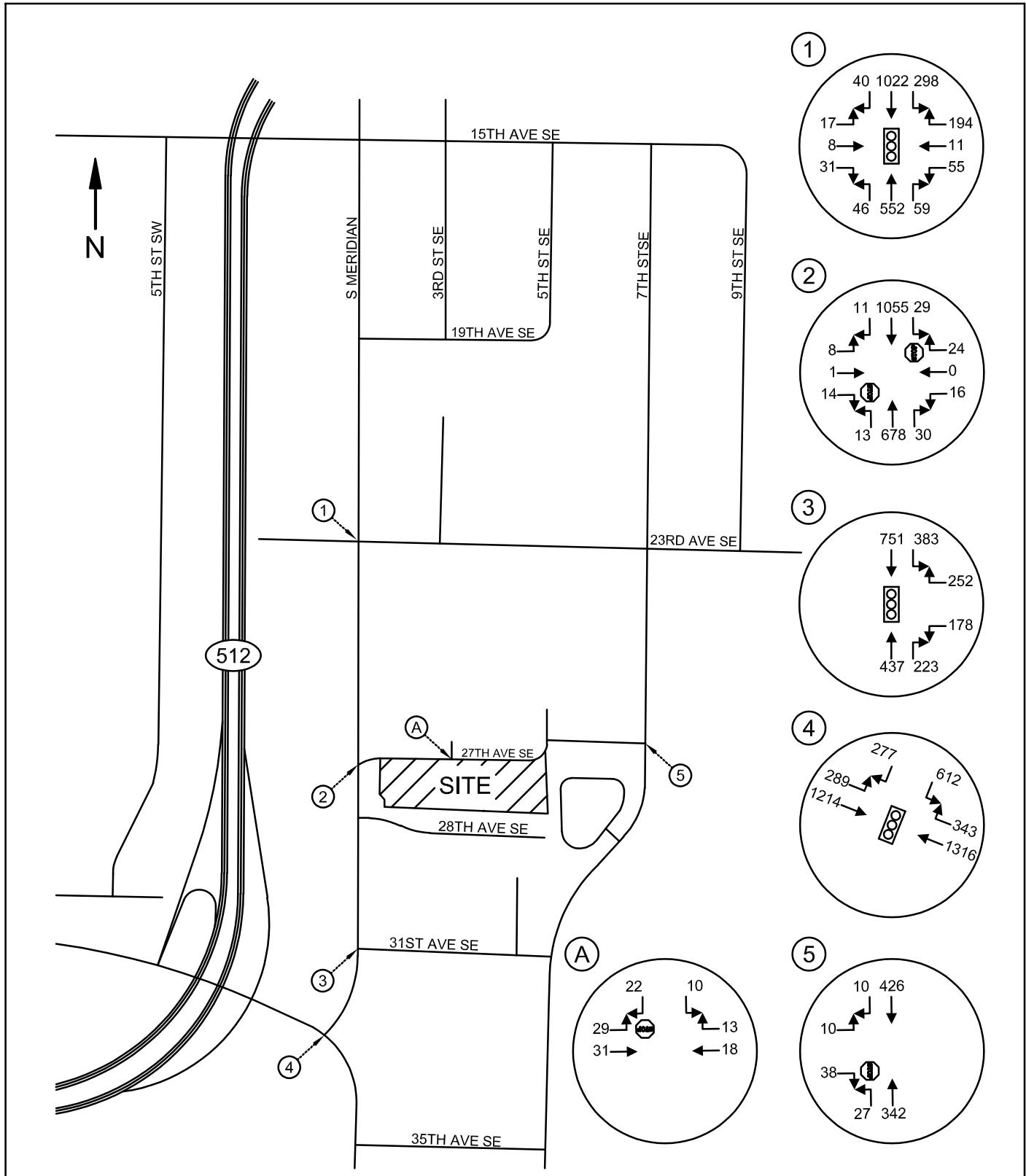
Field data for this study was collected in November of 2021. A 3 percent growth rate per year was applied to each count to estimate existing 2023 volumes. Counts were administered during the PM period of 4:00 – 6:00 PM. The one hour exhibiting the highest overall volumes from the field counts (peak hour) is then used for capacity and delay analysis. Field counts were taken at the following intersections, as directed by the City.

- S Meridian & 23rd Ave SE
- S Meridian & 27th Ave SE
- 7th Street SE & 27th Ave SE
- S Meridian & 31st Ave SE
- Meridian Ave E/31st Ave SE & S Meridian

It should be noted that the primary project access for the proposed Bradley Heights development is to be aligned opposite an existing westerly Sunset Garden Senior Living Apartment's driveway. Therefore, trips associated with the Senior Living Apartments were derived from the Institute of Transportation Engineer's (ITE) publication *Trip Generation*, 11th Edition. Land Use Code (LUC 252) was defined as the facility's use. All trips were assigned to a single access from the Sunset Garden Senior Living Apartments driveway opposite the proposed primary Bradley Heights access to present conservative analysis. Figure 3 illustrates existing PM peak hour volumes at the intersections of study. Refer to the appendix for full count sheets.

3.6 Sight Distance at Access Driveway

Primary access to the subject site is proposed via one new driveway extending south from 27th Avenue SE, aligned opposite an existing westerly Sunset Garden Senior Living Apartment's driveway. Two additional driveways are proposed to extend south from 27th Avenue SE serving as emergency vehicle access only. Assessments of the primary proposed 27th Avenue SE access were made to determine whether or not adequate entering sight distance (ESD) can be provided for project traffic. Based on the 25-mph posted speed limit on 27th Avenue SE, 280 feet would be required assuming a full movement access. Based on preliminary measurements, sight lines exceed 350 feet in either direction. No sight distance deficiencies are identified based on the development proposal.



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BRADLEY HEIGHTS
EXISTING PM PEAK HOUR VOLUMES
FIGURE 3

4. FUTURE TRAFFIC CONDITIONS

4.1 Trip Generation

Trip generation is defined as the number of vehicle movements that enter or exit a site during a designated time period such as a specific peak hour or an entire day. Data presented in this analysis was derived from the Institute of Transportation Engineer's (ITE) publication *Trip Generation*, 11th Edition. The existing land use is defined as Mobile Home Park (LUC 240). The 48 units are based on the number of tax parcels previously identified within the site. A list of the individual parcels can be found in the appendix. The proposed land use is to be defined as Multi-Family Housing – Low-Rise (LUC 220). ITE average rates were used to determine trip ends with dwelling units used as the input variable. Table 2 below summarizes anticipated vehicular movements for the average weekday daily trips (AWDT), AM peak hour and PM peak hour.

Table 2: Project Trip Generation

Land Use	Dwelling Units	AWDT	AM Peak-Hour Trips			PM Peak-Hour Trips		
			In	Out	Total	In	Out	Total
<i>Existing:</i> Mobile Home Park	48	-342	-4	-15	-19	-17	-11	-28
<i>Proposed:</i> Multi-Family Housing	236	1591	23	71	94	76	44	120
Net New Trips		1249	19	57	76	59	34	93

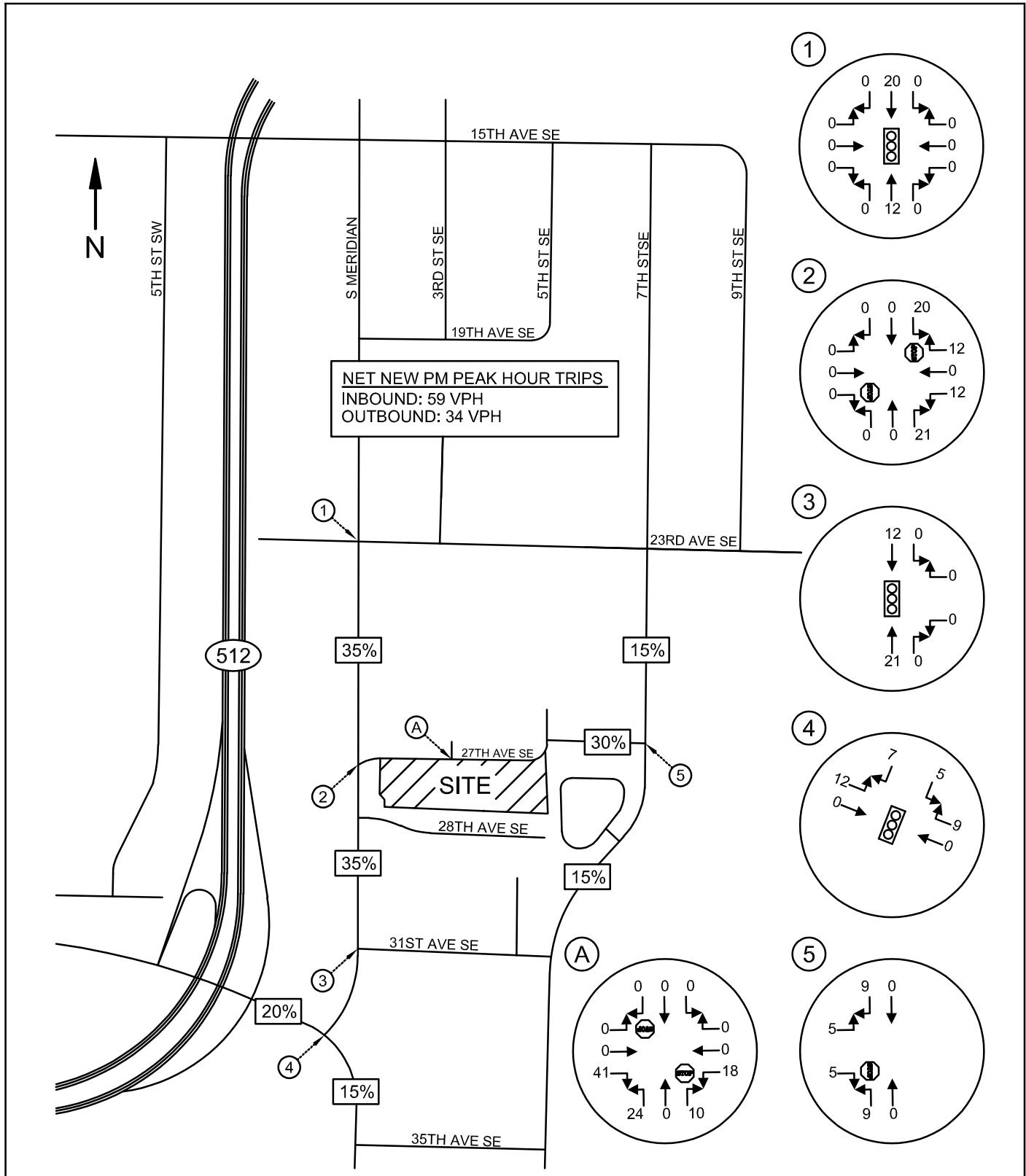
Based on the data presented in Table 2, the project is anticipated to generate 1249 net new average weekday trips with 76 net new trips (19 in/57 out) occurring during the AM peak hour and 93 net new trips (59 in/34 out) occurring during the PM peak hour.

4.2 Distribution & Assignment

Trip distribution describes the process by which project generated trips are dispersed on the roadway network surrounding the site. Trip distribution percentages were established during the scoping process with the City. PM Peak hour trip distribution and assignment is provided in Figure 4 on the following page.

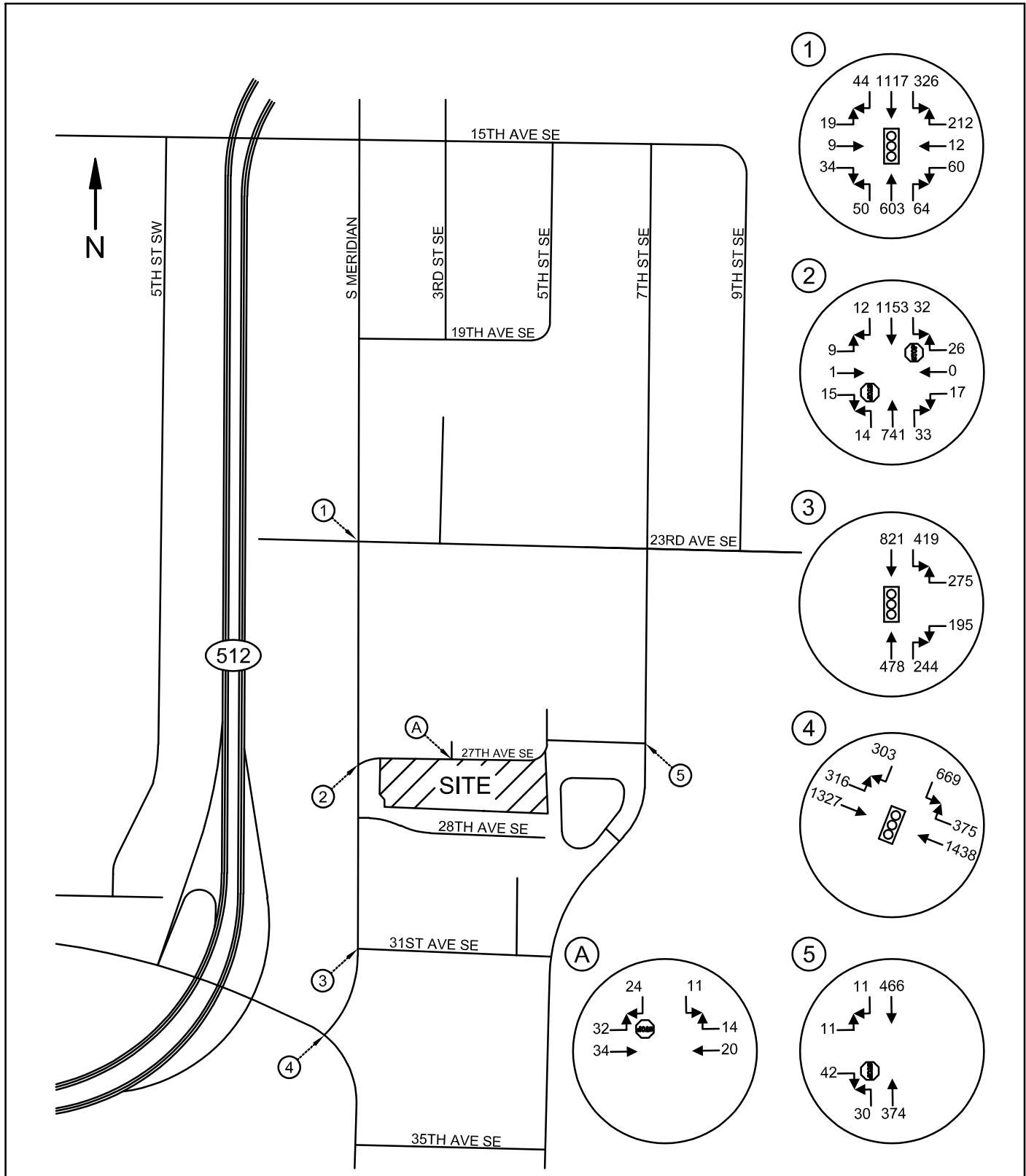
4.3 Peak Hour Volumes

The three-year horizon of 2026 was used for future analysis. Future 2026 traffic volumes without the project were derived by applying a 3.0 percent annual growth rate to existing traffic volumes shown in Figure 3. Future 2026 volumes without project traffic are illustrated in Figure 5. Figure 6 illustrates forecast 2026 PM peak hour volumes with project-generated traffic.



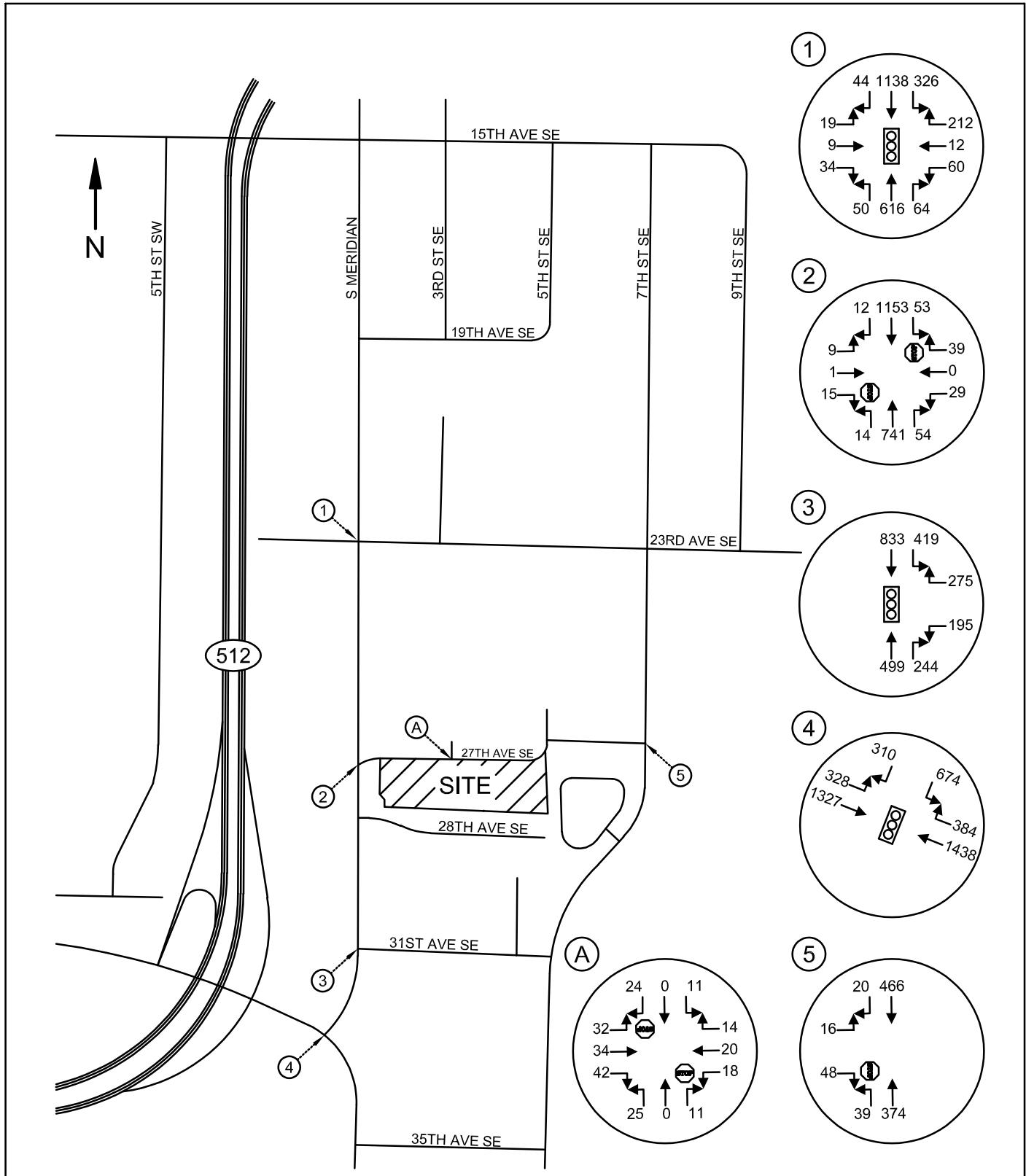
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BRADLEY HEIGHTS
PM PEAK HOUR TRIP DISTRIBUTION & ASSIGNMENT
FIGURE 4



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TRAFFIC AND CIVIL ENGINEERING

BRADLEY HEIGHTS
FORECAST 2026 PM PEAK HOUR VOLUMES WITHOUT PROJECT
FIGURE 5



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BRADLEY HEIGHTS
FORECAST 2026 PM PEAK HOUR VOLUMES WITH PROJECT
FIGURE 6

4.4 Level of Service

Existing and forecast 2026 peak hour delays were determined through the use of the *Highway Capacity Manual* 6th Edition. Capacity analysis is used to determine level of service (LOS) which is an established measure of congestion for transportation facilities. The range¹ for intersection level of service is LOS A to LOS F ranging from low control delays to heavy control delays. Level of service calculations derived from *Synchro 11*. For signalized intersections, LOS is determined by the intersection's overall average delay. For side-street stop-controlled intersections, LOS is determined by the approach with the highest delay. Summarized in Table 3 below are LOS conditions for existing and forecast 2026 conditions.

Table 3: Existing & Forecast 2026 PM Peak Hour Level of Service

Delays given in seconds per vehicle

Intersection	Control	<i>Existing</i>		<i>2026 Without</i>		<i>2026 With</i>	
		LOS	Delay	LOS	Delay	LOS	Delay
S Meridian & 23rd Ave SE	Signal	B	11.6	B	12.7	B	12.7
S Meridian & 27th Ave SE	Stop	C	20.0	C	22.8	C	23.8
S Meridian & 31st Ave SE	Signal	B	10.6	B	11.8	B	12.1
S Meridian & Meridian Ave E/31st Ave SW	Signal	C	32.1	D	36.6	D	37.1
7th St SE & 27th Ave SE	Stop	B	11.9	B	12.4	B	12.9
7th Ave SE & Driveway/Project Access	Stop	A	8.9	A	9.0	A	9.9

¹ *Signalized Intersections - Level of Service*

<u>Level of Service</u>	Control Delay per <u>Vehicle (sec)</u>
A	≤10
B	> 10 and ≤20
C	> 20 and ≤35
D	> 35 and ≤55
E	> 55 and ≤80
F	> 80

Stop Controlled Intersections – Level of Service

<u>Level of Service</u>	Control Delay per <u>Vehicle (sec)</u>
A	≤10
B	> 10 and ≤15
C	> 15 and ≤25
D	> 25 and ≤35
E	> 35 and ≤50
F	> 50

The City of Puyallup has adopted LOS D standards for most city intersections. The proposed access and outlying study intersections are shown to continue to meet City LOS standards based on forecast 2026 PM peak hour analysis. Operating with LOS D or better conditions, no operational deficiencies are identified at the study and access intersections as a result of the proposed development.

5. SUMMARY & MITIGATION

The Bradley Heights project proposes to construct a residential development comprising up to 236 apartment units in the city of Puyallup. The subject site is bordered to the north by 27th Avenue SE and located east of S Meridian on 7.78-acres (tax parcel #: 0419036006). Access to the subject site is proposed via one full- turning movement driveway extending north from 27th Avenue SE, opposite an existing driveway serving the Sunset Garden Senior Living Apartments. According to ITE data, site development would generate an estimated 1249 total net new daily trips with 76 net new trips occurring during the AM peak hour (19 inbound / 57 outbound) and 93 net new trips during the PM peak hour (59 inbound / 34 outbound).

A level of service (LOS) analysis was performed using a three-year horizon, which included a background growth rate. Existing and forecast 2026 PM peak hour delays at the study intersections and access are shown to meet City LOS D standards, operating with LOS D or better conditions. Overall, no significant impact to the surrounding roadway system is identified with the development proposal.

Proposed mitigation for the project is as follows:

1. Pay traffic impact fees as required by Puyallup. Final fees will be calculated and assessed by the City at the time of building permit issuance.

BRADLEY HEIGHTS
TRAFFIC IMPACT ANALYSIS

APPENDIX

Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775e
Site Code : 00004775
Start Date : 11/10/2021
Page No : 1

Groups Printed- Passenger + - Heavy

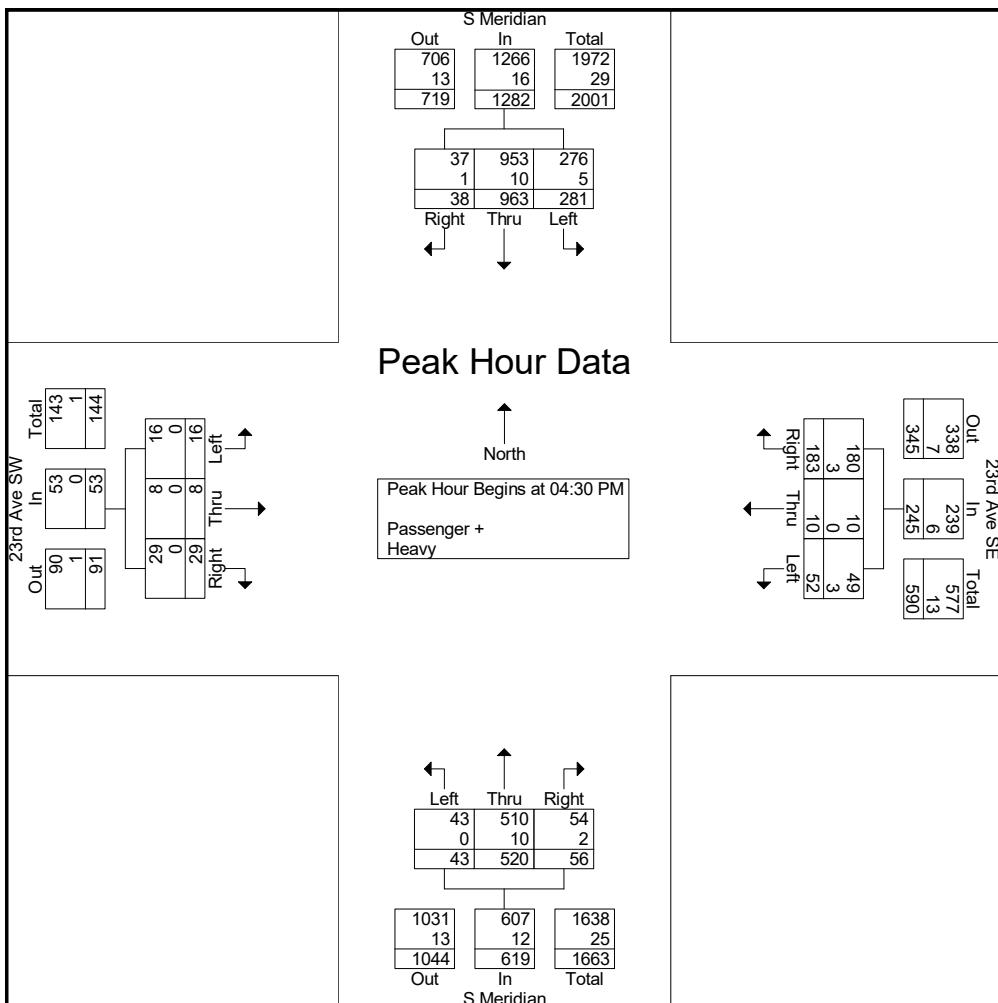
	S Meridian Southbound				23rd Ave SE Westbound				S Meridian Northbound				23rd Ave SW Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
04:00 PM	8	223	95	326	64	2	11	77	13	127	6	146	5	2	4	11	560
04:15 PM	2	212	62	276	37	6	10	53	21	123	7	151	5	4	2	11	491
04:30 PM	8	227	57	292	47	1	15	63	17	120	14	151	3	3	3	9	515
04:45 PM	17	283	79	379	51	3	13	67	14	123	8	145	9	4	7	20	611
Total	35	945	293	1273	199	12	49	260	65	493	35	593	22	13	16	51	2177
05:00 PM	6	205	63	274	46	4	13	63	11	144	8	163	7	1	4	12	512
05:15 PM	7	248	82	337	39	2	11	52	14	133	13	160	10	0	2	12	561
05:30 PM	8	193	56	257	36	4	11	51	19	126	9	154	8	1	5	14	476
05:45 PM	15	172	54	241	42	4	12	58	11	107	5	123	6	0	5	11	433
Total	36	818	255	1109	163	14	47	224	55	510	35	600	31	2	16	49	1982
Grand Total	71	1763	548	2382	362	26	96	484	120	1003	70	1193	53	15	32	100	4159
Apprch %	3	74	23		74.8	5.4	19.8		10.1	84.1	5.9		53	15	32		
Total %	1.7	42.4	13.2	57.3	8.7	0.6	2.3	11.6	2.9	24.1	1.7	28.7	1.3	0.4	0.8	2.4	
Passenger +	70	1746	542	2358	355	26	91	472	114	991	70	1175	53	15	32	100	4105
% Passenger +	98.6	99	98.9	99	98.1	100	94.8	97.5	95	98.8	100	98.5	100	100	100	100	98.7
Heavy	1	17	6	24	7	0	5	12	6	12	0	18	0	0	0	0	54
% Heavy	1.4	1	1.1	1	1.9	0	5.2	2.5	5	1.2	0	1.5	0	0	0	0	1.3

Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775e
Site Code : 00004775
Start Date : 11/10/2021
Page No : 2

	S Meridian Southbound				23rd Ave SE Westbound				S Meridian Northbound				23rd Ave SW Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	8	227	57	292	47	1	15	63	17	120	14	151	3	3	3	9	515
04:45 PM	17	283	79	379	51	3	13	67	14	123	8	145	9	4	7	20	611
05:00 PM	6	205	63	274	46	4	13	63	11	144	8	163	7	1	4	12	512
05:15 PM	7	248	82	337	39	2	11	52	14	133	13	160	10	0	2	12	561
Total Volume	38	963	281	1282	183	10	52	245	56	520	43	619	29	8	16	53	2199
% App. Total	3	75.1	21.9		74.7	4.1	21.2		9	84	6.9		54.7	15.1	30.2		
PHF	.559	.851	.857	.846	.897	.625	.867	.914	.824	.903	.768	.949	.725	.500	.571	.663	.900
Passenger +	37	953	276	1266	180	10	49	239	54	510	43	607	29	8	16	53	2165
% Passenger +	97.4	99.0	98.2	98.8	98.4	100	94.2	97.6	96.4	98.1	100	98.1	100	100	100	100	98.5
Heavy	1	10	5	16	3	0	3	6	2	10	0	12	0	0	0	0	34
% Heavy	2.6	1.0	1.8	1.2	1.6	0	5.8	2.4	3.6	1.9	0	1.9	0	0	0	0	1.5



Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775a
Site Code : 00004775
Start Date : 11/10/2021
Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	S Meridian Southbound				27th Ave SE Westbound				S Meridian Northbound				Apartment Driveway Eastbound				
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
06:00 AM	0	46	2	48	5	0	2	7	2	110	0	112	3	0	3	6	173
06:15 AM	1	59	2	62	1	0	1	2	1	139	0	140	0	0	1	1	205
06:30 AM	0	79	2	81	3	0	3	6	4	145	0	149	1	0	1	2	238
06:45 AM	1	71	1	73	3	0	2	5	1	162	1	164	1	0	0	1	243
Total	2	255	7	264	12	0	8	20	8	556	1	565	5	0	5	10	859
07:00 AM	2	80	1	83	5	0	4	9	9	179	0	188	1	0	1	2	282
07:15 AM	3	96	1	100	1	0	3	4	6	158	4	168	3	0	3	6	278
07:30 AM	2	114	1	117	3	0	3	6	3	154	2	159	0	0	3	3	285
07:45 AM	6	93	3	102	6	0	6	12	7	171	4	182	1	1	2	4	300
Total	13	383	6	402	15	0	16	31	25	662	10	697	5	1	9	15	1145
08:00 AM	1	114	5	120	4	0	2	6	6	144	1	151	3	1	0	4	281
08:15 AM	1	127	2	130	3	0	2	5	5	133	3	141	6	0	3	9	285
08:30 AM	1	131	6	138	7	0	2	9	3	124	4	131	2	2	3	7	285
08:45 AM	5	129	6	140	7	0	3	10	8	138	3	149	3	0	0	3	302
Total	8	501	19	528	21	0	9	30	22	539	11	572	14	3	6	23	1153
09:00 AM	4	166	5	175	4	0	3	7	4	113	4	121	2	1	4	7	310
09:15 AM	2	154	4	160	6	1	4	11	8	144	1	153	3	0	3	6	330
09:30 AM	2	154	5	161	8	0	7	15	3	129	1	133	4	0	3	7	316
09:45 AM	1	162	9	172	8	0	10	18	6	159	3	168	4	0	0	4	362
Total	9	636	23	668	26	1	24	51	21	545	9	575	13	1	10	24	1318
10:00 AM	1	202	2	205	7	0	5	12	6	138	2	146	2	0	1	3	366
10:15 AM	3	155	5	163	6	0	6	12	7	132	2	141	5	1	1	7	323
10:30 AM	4	193	5	202	11	0	5	16	4	131	3	138	1	0	3	4	360
10:45 AM	4	168	4	176	5	0	5	10	6	145	0	151	2	1	3	6	343
Total	12	718	16	746	29	0	21	50	23	546	7	576	10	2	8	20	1392
11:00 AM	2	184	2	188	5	0	4	9	10	161	0	171	4	0	1	5	373
11:15 AM	2	180	6	188	5	0	7	12	7	143	5	155	3	0	6	9	364
11:30 AM	3	192	5	200	8	0	4	12	6	140	0	146	1	1	1	3	361
11:45 AM	0	195	7	202	8	0	4	12	5	154	4	163	3	0	3	6	383
Total	7	751	20	778	26	0	19	45	28	598	9	635	11	1	11	23	1481
12:00 PM	0	211	9	220	9	0	8	17	4	155	2	161	6	0	2	8	406
12:15 PM	0	183	9	192	13	0	3	16	7	136	2	145	1	0	0	1	354
12:30 PM	2	187	5	194	12	1	3	16	2	163	1	166	5	0	2	7	383
12:45 PM	3	179	2	184	10	0	7	17	11	163	1	175	5	0	2	7	383
Total	5	760	25	790	44	1	21	66	24	617	6	647	17	0	6	23	1526
01:00 PM	1	161	8	170	5	0	0	5	4	140	5	149	3	0	0	3	327
01:15 PM	4	193	7	204	8	0	5	13	6	144	2	152	1	0	2	3	372
01:30 PM	0	181	6	187	7	0	9	16	3	170	1	174	0	0	2	2	379
01:45 PM	1	196	3	200	8	0	4	12	6	158	3	167	2	0	0	2	381
Total	6	731	24	761	28	0	18	46	19	612	11	642	6	0	4	10	1459
02:00 PM	3	223	4	230	6	0	2	8	6	182	5	193	2	0	0	2	433

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PO Box 397
Puyallup, WA 98371

File Name : 4775a
Site Code : 00004775
Start Date : 11/10/2021
Page No : 2

Groups Printed- Passenger + - Heavy

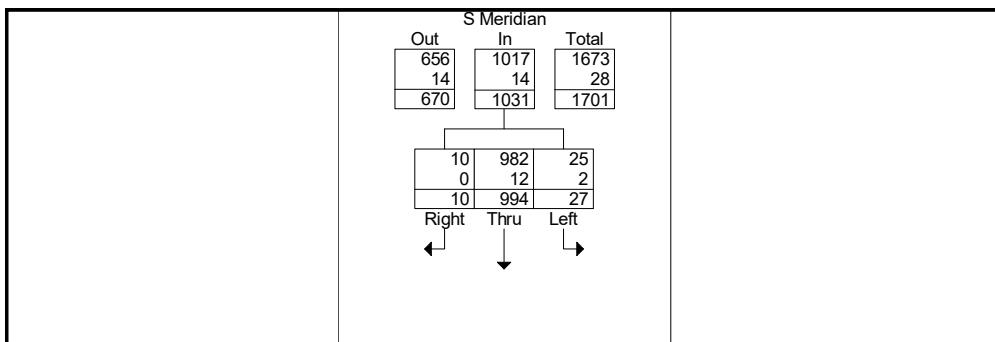
Start Time	S Meridian Southbound				27th Ave SE Westbound				S Meridian Northbound				Apartment Driveway Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
02:15 PM	3	202	11	216	10	0	5	15	8	152	3	163	4	0	0	4	398
02:30 PM	2	214	8	224	7	0	6	13	10	189	3	202	4	0	0	4	443
02:45 PM	1	205	2	208	10	0	5	15	6	144	2	152	5	0	0	5	380
Total	9	844	25	878	33	0	18	51	30	667	13	710	15	0	0	15	1654
03:00 PM	4	203	5	212	2	0	2	4	2	191	1	194	5	0	1	6	416
03:15 PM	1	250	6	257	16	0	2	18	8	164	1	173	3	0	3	6	454
03:30 PM	2	228	5	235	4	0	1	5	4	157	3	164	1	1	3	5	409
03:45 PM	3	282	6	291	7	0	4	11	4	157	5	166	4	0	0	4	472
Total	10	963	22	995	29	0	9	38	18	669	10	697	13	1	7	21	1751
04:00 PM	2	236	4	242	6	0	3	9	8	154	2	164	5	0	3	8	423
04:15 PM	6	212	4	222	3	0	6	9	2	155	2	159	3	1	2	6	396
04:30 PM	3	235	7	245	7	0	2	9	5	143	5	153	3	0	6	9	416
04:45 PM	5	283	4	292	4	0	3	7	7	154	1	162	5	1	1	7	468
Total	16	966	19	1001	20	0	14	34	22	606	10	638	16	2	12	30	1703
05:00 PM	1	232	4	237	7	0	5	12	5	167	2	174	4	0	1	5	428
05:15 PM	1	244	12	257	5	0	5	10	11	175	4	190	1	0	0	1	458
05:30 PM	0	203	7	210	7	0	4	11	6	134	2	142	3	0	1	4	367
05:45 PM	3	191	1	195	3	0	5	8	4	128	6	138	3	0	1	4	345
Total	5	870	24	899	22	0	19	41	26	604	14	644	11	0	3	14	1598
06:00 PM	2	211	3	216	2	0	0	2	2	122	2	126	2	1	2	5	349
06:15 PM	1	180	4	185	2	0	1	3	5	135	5	145	1	0	4	5	338
06:30 PM	4	156	1	161	2	0	1	3	4	120	2	126	2	0	1	3	293
06:45 PM	2	130	6	138	4	0	3	7	1	96	1	98	1	0	0	1	244
Total	9	677	14	700	10	0	5	15	12	473	10	495	6	1	7	14	1224
Grand Total	111	9055	244	9410	315	2	201	518	278	7694	121	8093	142	12	88	242	18263
Apprch %	1.2	96.2	2.6		60.8	0.4	38.8		3.4	95.1	1.5		58.7	5	36.4		
Total %	0.6	49.6	1.3	51.5	1.7	0	1.1	2.8	1.5	42.1	0.7	44.3	0.8	0.1	0.5	1.3	
Passenger +	111	8921	238	9270	306	2	197	505	269	7571	119	7959	139	12	86	237	17971
% Passenger +	100	98.5	97.5	98.5	97.1	100	98	97.5	96.8	98.4	98.3	98.3	97.9	100	97.7	97.9	98.4
Heavy	0	134	6	140	9	0	4	13	9	123	2	134	3	0	2	5	292
% Heavy	0	1.5	2.5	1.5	2.9	0	2	2.5	3.2	1.6	1.7	1.7	2.1	0	2.3	2.1	1.6

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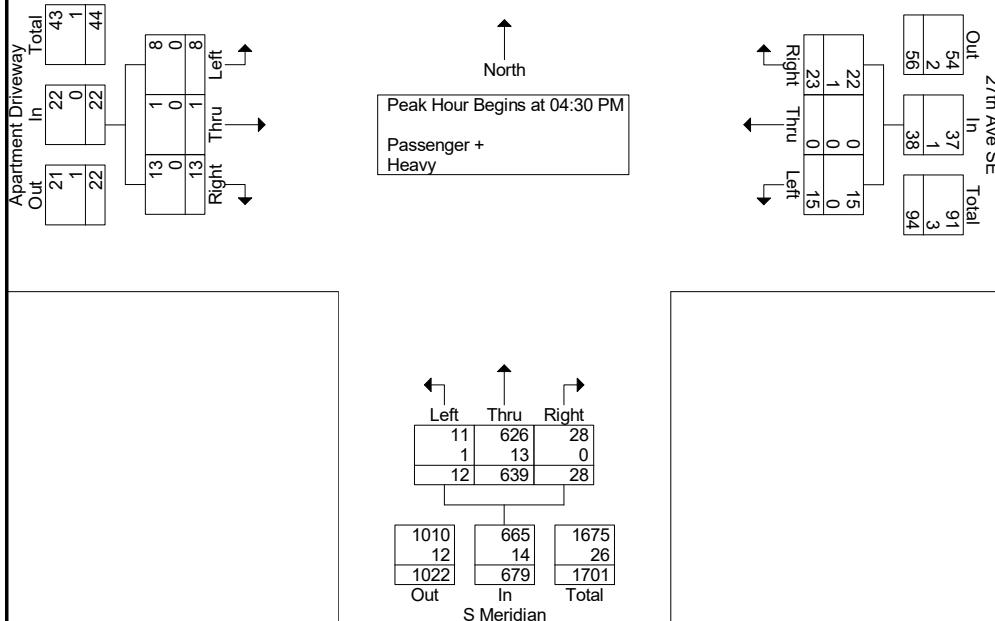
PO Box 397
Puyallup, WA 98371

File Name : 4775a
Site Code : 00004775
Start Date : 11/10/2021
Page No : 3

	S Meridian Southbound				27th Ave SE Westbound				S Meridian Northbound				Apartment Driveway Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 06:00 AM to 06:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	3	235	7	245	7	0	2	9	5	143	5	153	3	0	6	9	416
04:45 PM	5	283	4	292	4	0	3	7	7	154	1	162	5	1	1	7	468
05:00 PM	1	232	4	237	7	0	5	12	5	167	2	174	4	0	1	5	428
05:15 PM	1	244	12	257	5	0	5	10	11	175	4	190	1	0	0	1	458
Total Volume	10	994	27	1031	23	0	15	38	28	639	12	679	13	1	8	22	1770
% App. Total	1	96.4	2.6		60.5	0	39.5		4.1	94.1	1.8		59.1	4.5	36.4		
PHF	.500	.878	.563	.883	.821	.000	.750	.792	.636	.913	.600	.893	.650	.250	.333	.611	.946
Passenger +	10	982	25	1017	22	0	15	37	28	626	11	665	13	1	8	22	1741
% Passenger +	100	98.8	92.6	98.6	95.7	0	100	97.4	100	98.0	91.7	97.9	100	100	100	100	98.4
Heavy	0	12	2	14	1	0	0	1	0	13	1	14	0	0	0	0	29
% Heavy	0	1.2	7.4	1.4	4.3	0	0	2.6	0	2.0	8.3	2.1	0	0	0	0	1.6



Peak Hour Data



Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775C
Site Code : 00004775
Start Date : 11/10/2021
Page No : 1

Groups Printed- Passenger + - Heavy

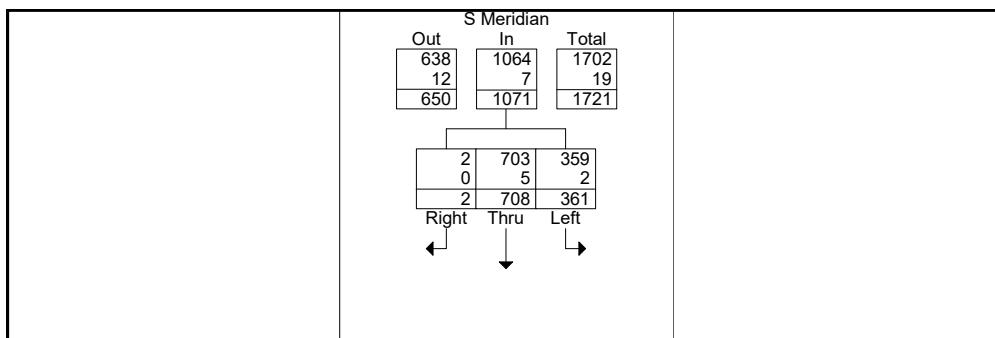
	S Meridian Southbound				31st Ave SE Westbound				S Meridian Northbound				Driveway Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
04:00 PM	1	162	90	253	56	1	37	94	50	113	0	163	0	0	0	0	510
04:15 PM	0	150	82	232	57	0	39	96	63	101	0	164	0	0	0	0	492
04:30 PM	2	164	92	258	57	0	48	105	55	94	0	149	0	0	0	0	512
04:45 PM	0	202	97	299	52	1	41	94	54	111	0	165	0	0	0	0	558
Total	3	678	361	1042	222	2	165	389	222	419	0	641	0	0	0	0	2072
05:00 PM	0	171	86	257	65	0	42	107	55	104	0	159	3	0	0	3	526
05:15 PM	0	171	86	257	64	0	37	101	46	103	0	149	0	0	0	0	507
05:30 PM	0	154	77	231	47	0	44	91	36	95	0	131	0	0	0	0	453
05:45 PM	0	130	77	207	57	0	30	87	40	97	0	137	0	0	0	0	431
Total	0	626	326	952	233	0	153	386	177	399	0	576	3	0	0	3	1917
Grand Total	3	1304	687	1994	455	2	318	775	399	818	0	1217	3	0	0	3	3989
Apprch %	0.2	65.4	34.5		58.7	0.3	41		32.8	67.2	0		100	0	0		
Total %	0.1	32.7	17.2	50	11.4	0.1	8	19.4	10	20.5	0	30.5	0.1	0	0	0.1	
Passenger +	3	1296	680	1979	448	2	315	765	399	808	0	1207	3	0	0	3	3954
% Passenger +	100	99.4	99	99.2	98.5	100	99.1	98.7	100	98.8	0	99.2	100	0	0	100	99.1
Heavy	0	8	7	15	7	0	3	10	0	10	0	10	0	0	0	0	35
% Heavy	0	0.6	1	0.8	1.5	0	0.9	1.3	0	1.2	0	0.8	0	0	0	0	0.9

Heath & Associates

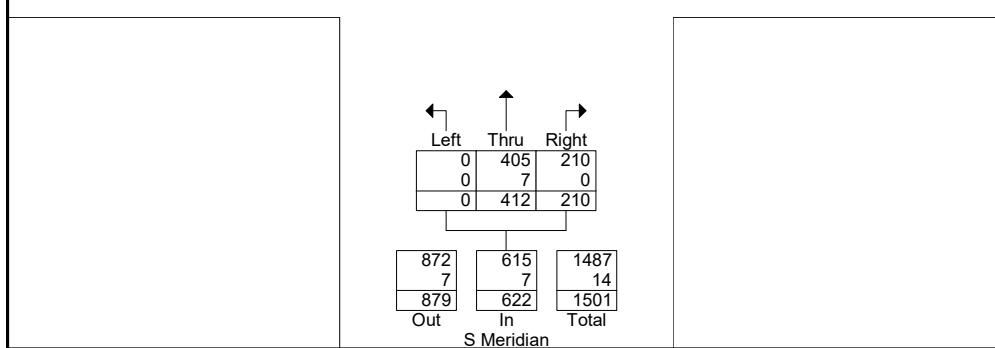
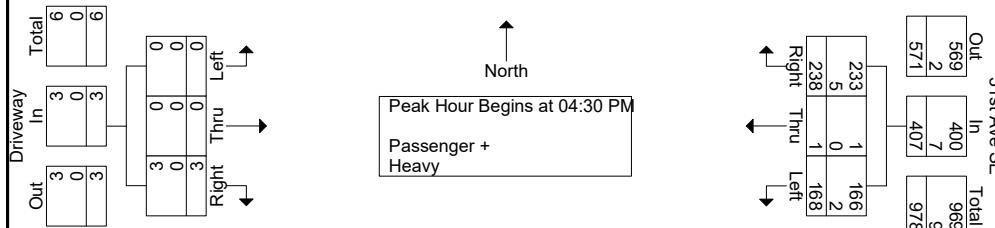
PO Box 397
Puyallup, WA 98371

File Name : 4775C
Site Code : 00004775
Start Date : 11/10/2021
Page No : 2

	S Meridian Southbound				31st Ave SE Westbound				S Meridian Northbound				Driveway Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	2	164	92	258	57	0	48	105	55	94	0	149	0	0	0	0	512
04:45 PM	0	202	97	299	52	1	41	94	54	111	0	165	0	0	0	0	558
05:00 PM	0	171	86	257	65	0	42	107	55	104	0	159	3	0	0	3	526
05:15 PM	0	171	86	257	64	0	37	101	46	103	0	149	0	0	0	0	507
Total Volume	2	708	361	1071	238	1	168	407	210	412	0	622	3	0	0	3	2103
% App. Total	0.2	66.1	33.7		58.5	0.2	41.3		33.8	66.2	0		100	0	0		
PHF	.250	.876	.930	.895	.915	.250	.875	.951	.955	.928	.000	.942	.250	.000	.000	.250	.942
Passenger +	2	703	359	1064	233	1	166	400	210	405	0	615	3	0	0	3	2082
% Passenger +	100	99.3	99.4	99.3	97.9	100	98.8	98.3	100	98.3	0	98.9	100	0	0	100	99.0
Heavy	0	5	2	7	5	0	2	7	0	7	0	7	0	0	0	0	21
% Heavy	0	0.7	0.6	0.7	2.1	0	1.2	1.7	0	1.7	0	1.1	0	0	0	0	1.0



Peak Hour Data



Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775d
Site Code : 00004775
Start Date : 11/10/2021
Page No : 1

Groups Printed- Passenger + - Heavy

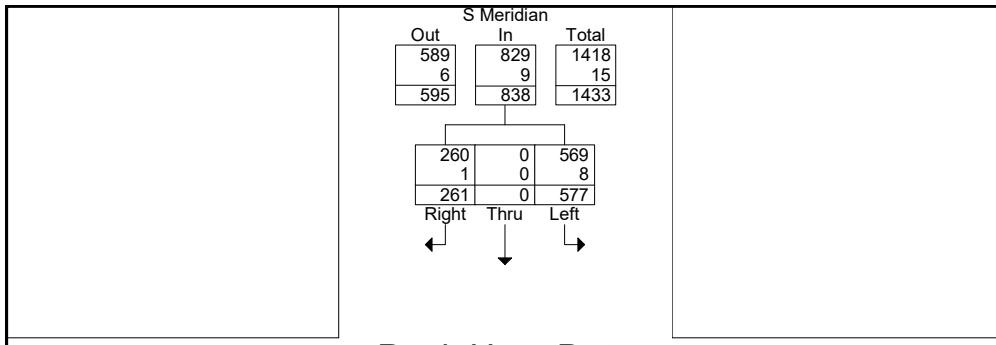
	S Meridian Southbound				Meridian Ave E Westbound				S Meridian Northbound				31st Ave SW Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
04:00 PM	60	0	145	205	78	351	0	429	0	0	0	0	0	290	70	360	994
04:15 PM	66	0	134	200	82	310	0	392	0	0	0	0	0	280	74	354	946
04:30 PM	68	0	137	205	70	300	0	370	0	0	0	0	0	290	63	353	928
04:45 PM	67	0	161	228	93	279	0	372	0	0	0	0	0	284	65	349	949
Total	261	0	577	838	323	1240	0	1563	0	0	0	0	0	1144	272	1416	3817
05:00 PM	82	0	143	225	87	305	0	392	0	0	0	0	0	286	61	347	964
05:15 PM	59	0	129	188	85	300	0	385	0	0	0	0	0	280	56	336	909
05:30 PM	52	0	150	202	82	290	0	372	0	0	0	0	0	297	50	347	921
05:45 PM	52	0	116	168	74	252	0	326	0	0	0	0	0	295	52	347	841
Total	245	0	538	783	328	1147	0	1475	0	0	0	0	0	1158	219	1377	3635
Grand Total	506	0	1115	1621	651	2387	0	3038	0	0	0	0	0	2302	491	2793	7452
Apprch %	31.2	0	68.8		21.4	78.6	0		0	0	0	0	0	82.4	17.6		
Total %	6.8	0	15	21.8	8.7	32	0	40.8	0	0	0	0	0	30.9	6.6	37.5	
Passenger +	499	0	1105	1604	645	2313	0	2958	0	0	0	0	0	2269	489	2758	7320
% Passenger +	98.6	0	99.1	99	99.1	96.9	0	97.4	0	0	0	0	0	98.6	99.6	98.7	98.2
Heavy	7	0	10	17	6	74	0	80	0	0	0	0	0	33	2	35	132
% Heavy	1.4	0	0.9	1	0.9	3.1	0	2.6	0	0	0	0	0	1.4	0.4	1.3	1.8

Heath & Associates

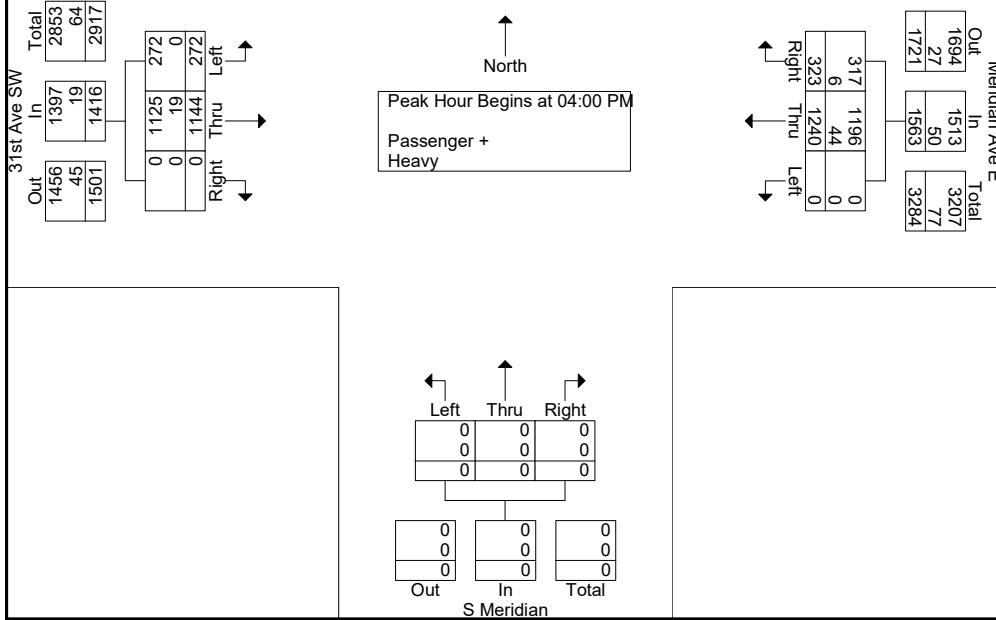
PO Box 397
Puyallup, WA 98371

File Name : 4775d
Site Code : 00004775
Start Date : 11/10/2021
Page No : 2

	S Meridian Southbound				Meridian Ave E Westbound				S Meridian Northbound				31st Ave SW Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:00 PM																	
04:00 PM	60	0	145	205	78	351	0	429	0	0	0	0	0	290	70	360	994
04:15 PM	66	0	134	200	82	310	0	392	0	0	0	0	0	280	74	354	946
04:30 PM	68	0	137	205	70	300	0	370	0	0	0	0	0	290	63	353	928
04:45 PM	67	0	161	228	93	279	0	372	0	0	0	0	0	284	65	349	949
Total Volume	261	0	577	838	323	1240	0	1563	0	0	0	0	0	1144	272	1416	3817
% App. Total	31.1	0	68.9		20.7	79.3	0		0	0	0	0	0	80.8	19.2		
PHF	.960	.000	.896	.919	.868	.883	.000	.911	.000	.000	.000	.000	.000	.986	.919	.983	.960
Passenger +	260	0	569	829	317	1196	0	1513	0	0	0	0	0	1125	272	1397	3739
% Passenger +	99.6	0	98.6	98.9	98.1	96.5	0	96.8	0	0	0	0	0	98.3	100	98.7	98.0
Heavy	1	0	8	9	6	44	0	50	0	0	0	0	0	19	0	19	78
% Heavy	0.4	0	1.4	1.1	1.9	3.5	0	3.2	0	0	0	0	0	1.7	0	1.3	2.0



Peak Hour Data



Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775b
Site Code : 00004775
Start Date : 11/10/2021
Page No : 1

Groups Printed- Passenger + - Heavy

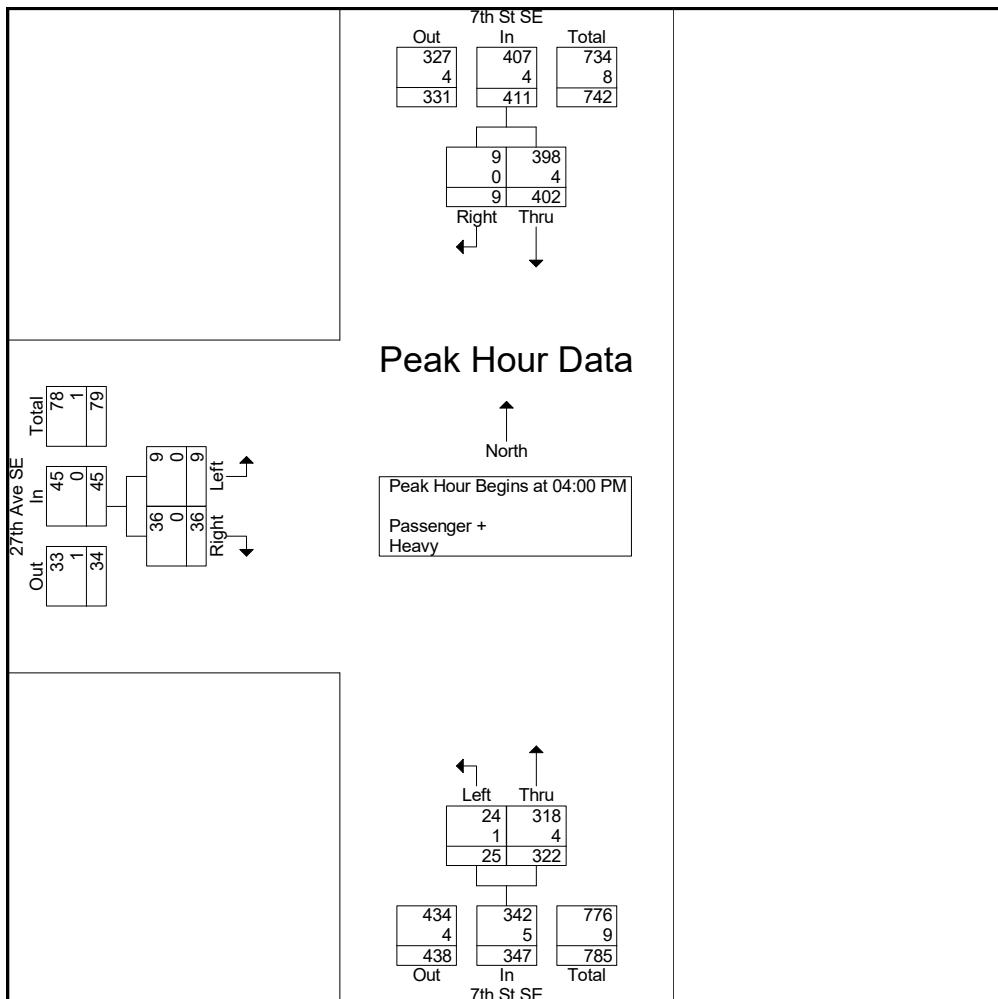
	7th St SE Southbound			7th St SE Northbound			27th Ave SE Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
04:00 PM	2	102	104	86	8	94	8	2	10	208
04:15 PM	2	104	106	85	5	90	9	3	12	208
04:30 PM	4	99	103	80	6	86	8	0	8	197
04:45 PM	1	97	98	71	6	77	11	4	15	190
Total	9	402	411	322	25	347	36	9	45	803
05:00 PM	3	106	109	79	5	84	10	1	11	204
05:15 PM	4	89	93	78	8	86	3	6	9	188
05:30 PM	1	88	89	71	6	77	5	3	8	174
05:45 PM	0	87	87	62	9	71	4	1	5	163
Total	8	370	378	290	28	318	22	11	33	729
Grand Total	17	772	789	612	53	665	58	20	78	1532
Apprch %	2.2	97.8		92	8		74.4	25.6		
Total %	1.1	50.4	51.5	39.9	3.5	43.4	3.8	1.3	5.1	
Passenger +	17	762	779	607	52	659	58	19	77	1515
% Passenger +	100	98.7	98.7	99.2	98.1	99.1	100	95	98.7	98.9
Heavy	0	10	10	5	1	6	0	1	1	17
% Heavy	0	1.3	1.3	0.8	1.9	0.9	0	5	1.3	1.1

Heath & Associates

PO Box 397
Puyallup, WA 98371

File Name : 4775b
Site Code : 00004775
Start Date : 11/10/2021
Page No : 2

	7th St SE Southbound			7th St SE Northbound			27th Ave SE Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:00 PM										
04:00 PM	2	102	104	86	8	94	8	2	10	208
04:15 PM	2	104	106	85	5	90	9	3	12	208
04:30 PM	4	99	103	80	6	86	8	0	8	197
04:45 PM	1	97	98	71	6	77	11	4	15	190
Total Volume	9	402	411	322	25	347	36	9	45	803
% App. Total	2.2	97.8		92.8	7.2		80	20		
PHF	.563	.966	.969	.936	.781	.923	.818	.563	.750	.965
Passenger +	9	398	407	318	24	342	36	9	45	794
% Passenger +	100	99.0	99.0	98.8	96.0	98.6	100	100	100	98.9
Heavy	0	4	4	4	1	5	0	0	0	9
% Heavy	0	1.0	1.0	1.2	4.0	1.4	0	0	0	1.1



Mobile Home Park (240)

**Vehicle Trip Ends vs: Dwelling Units
On a: Weekday**

Setting/Location: General Urban/Suburban

Number of Studies: 13

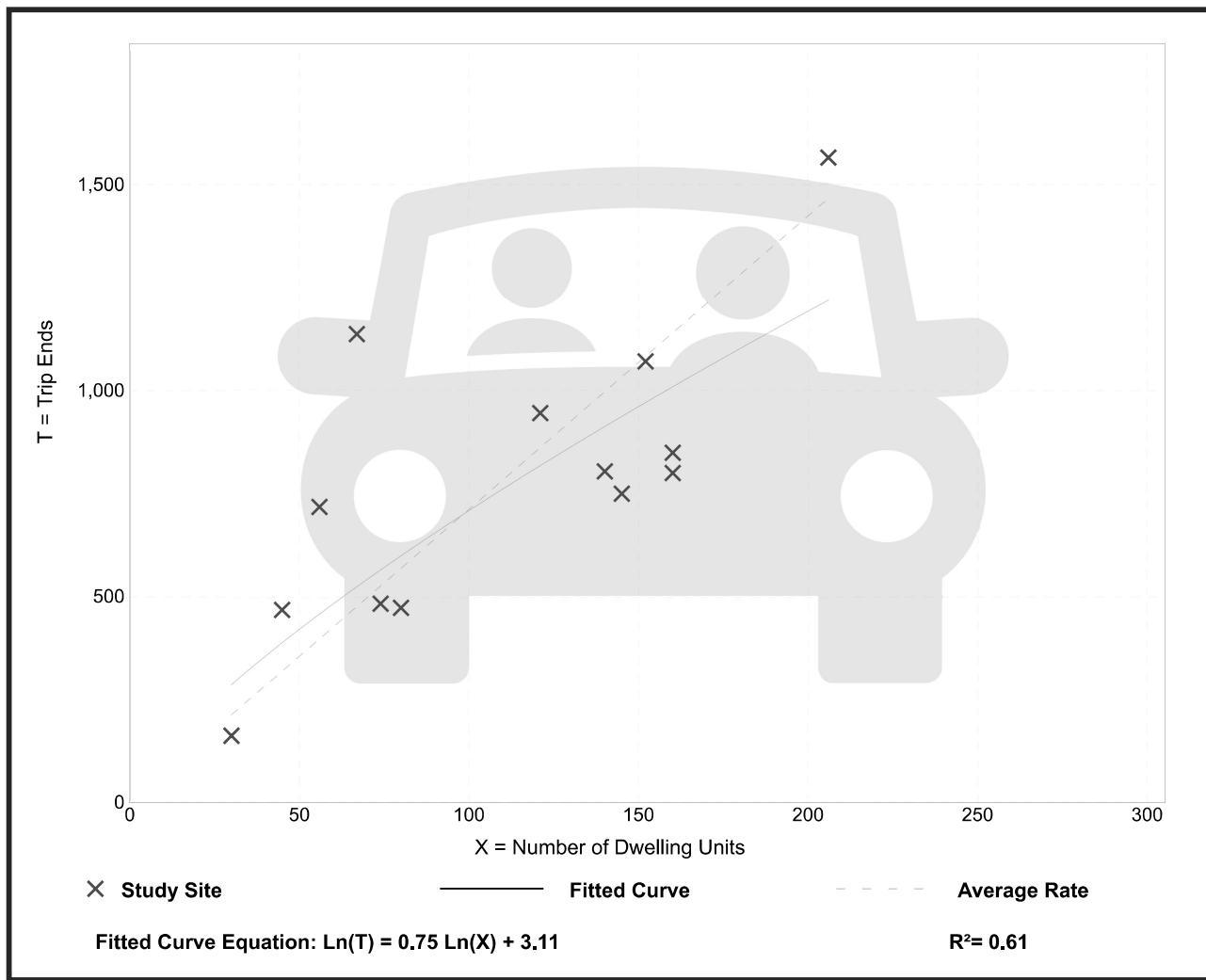
Avg. Num. of Dwelling Units: 110

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
7.12	5.00 - 16.96	2.91

Data Plot and Equation



Mobile Home Park (240)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies:

9

Avg. Num. of Dwelling Units:

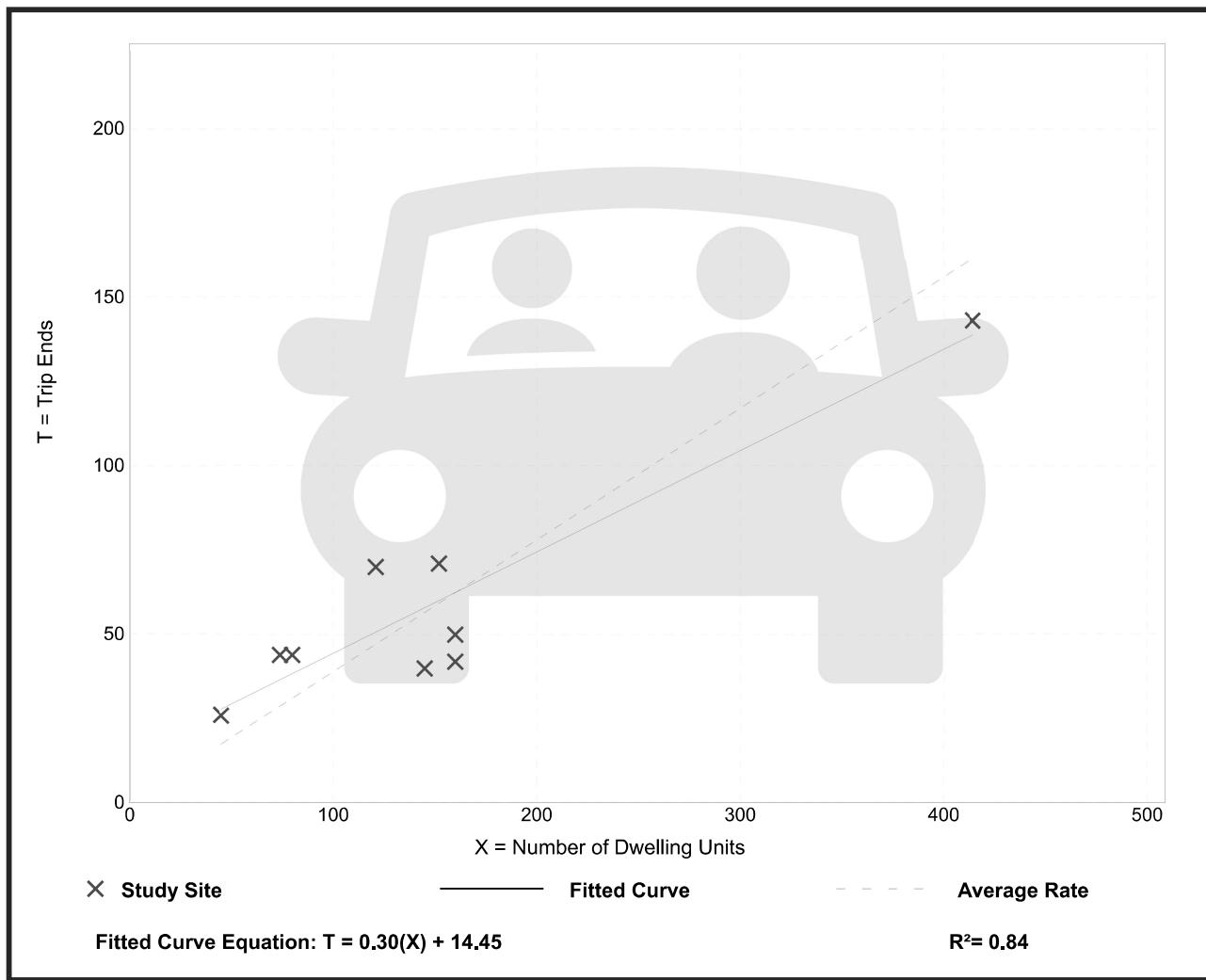
150

Directional Distribution: 21% entering, 79% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.39	0.26 - 0.59	0.12

Data Plot and Equation



Mobile Home Park (240)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 9

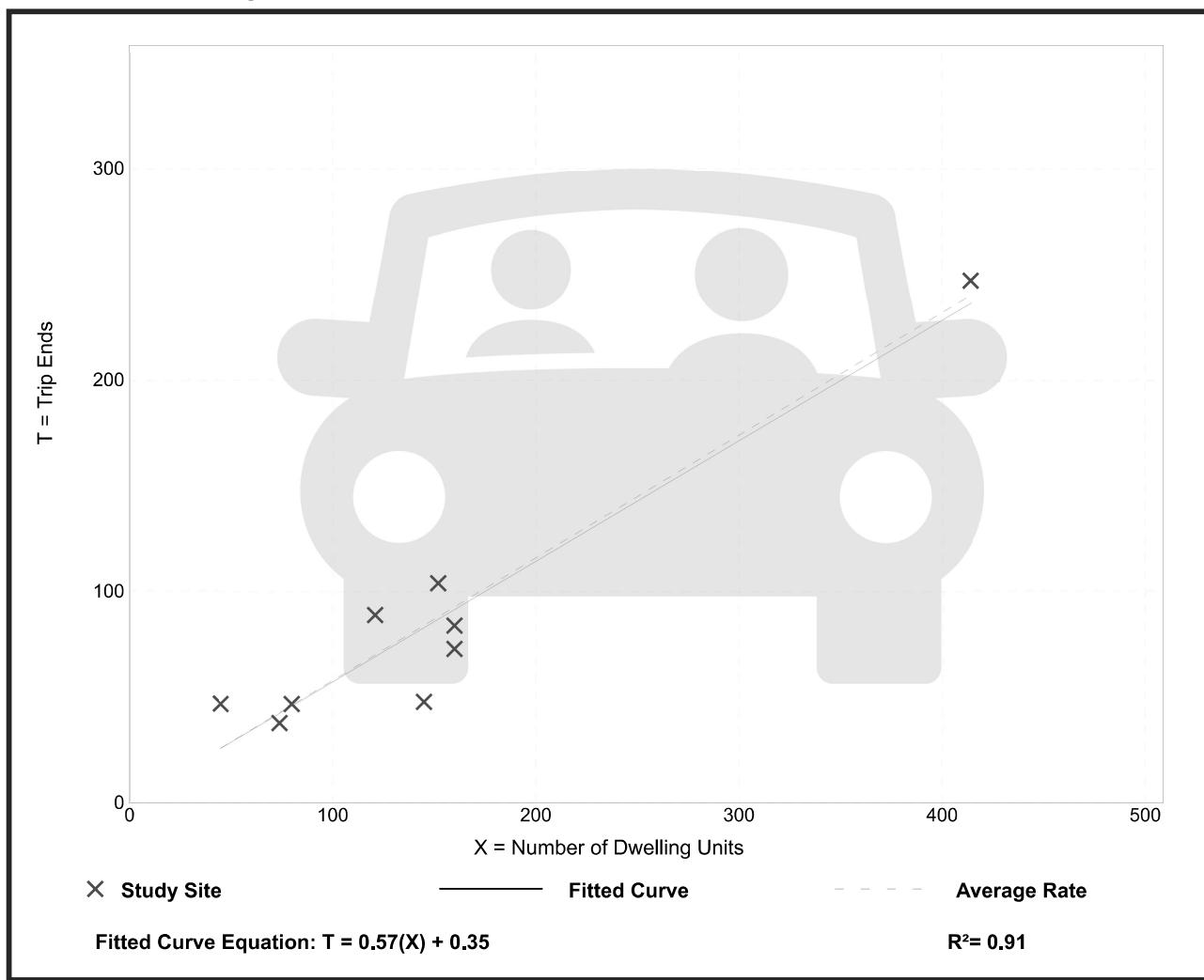
Avg. Num. of Dwelling Units: 150

Directional Distribution: 62% entering, 38% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.58	0.33 - 1.04	0.15

Data Plot and Equation



Multifamily Housing (Low-Rise)

Not Close to Rail Transit (220)

**Vehicle Trip Ends vs: Dwelling Units
On a: Weekday**

Setting/Location: General Urban/Suburban

Number of Studies: 22

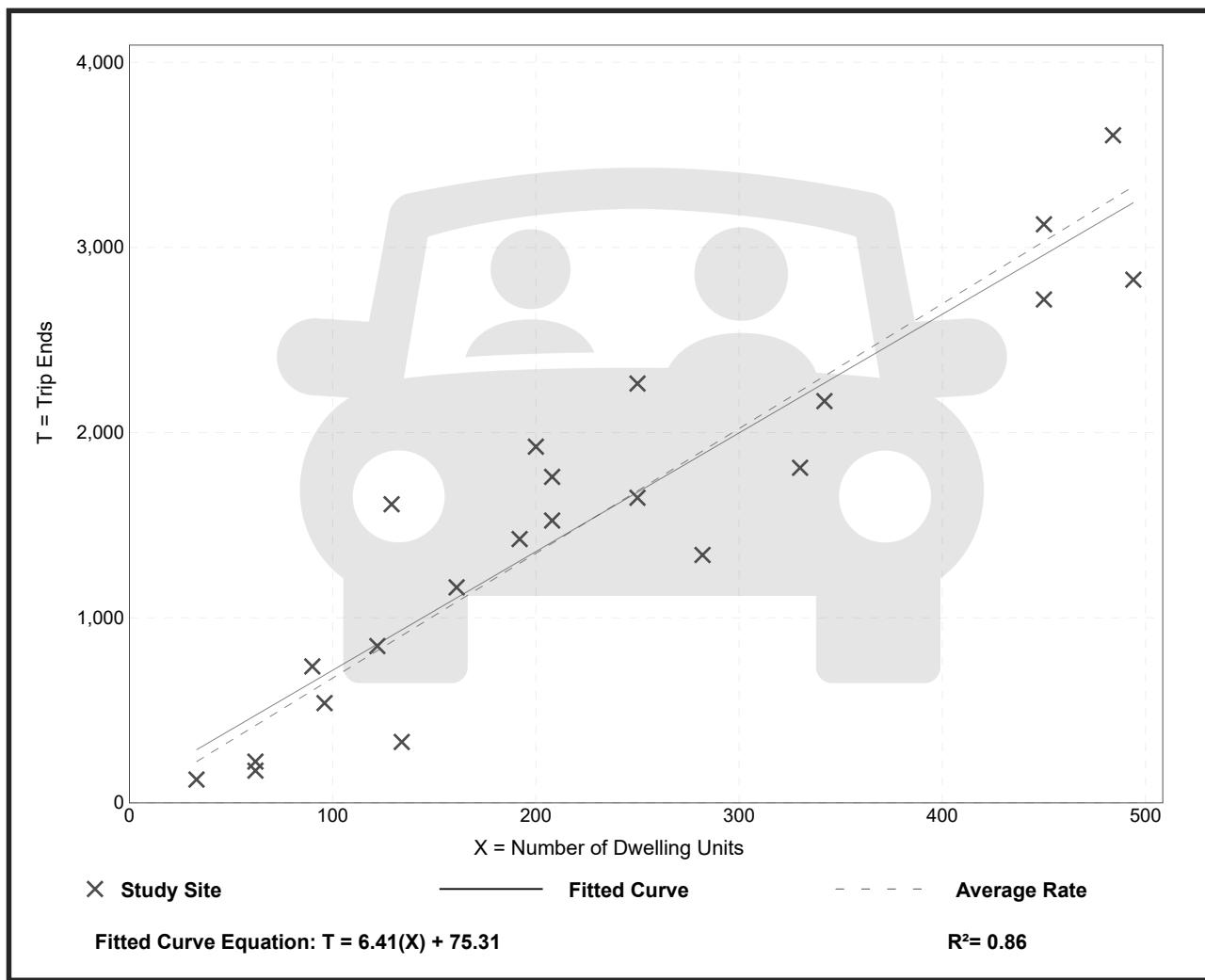
Avg. Num. of Dwelling Units: 229

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
6.74	2.46 - 12.50	1.79

Data Plot and Equation



Multifamily Housing (Low-Rise) Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 49

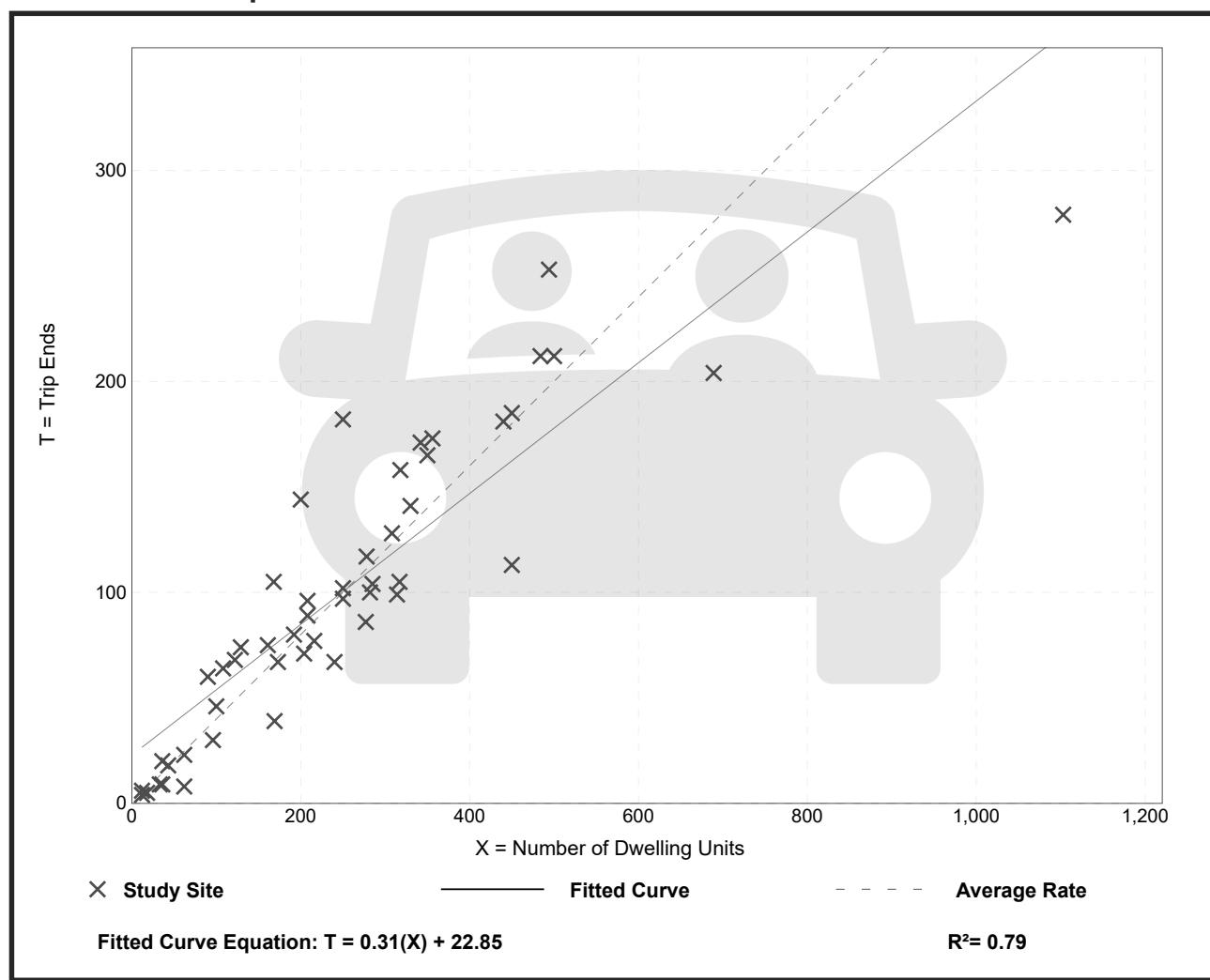
Avg. Num. of Dwelling Units: 249

Directional Distribution: 24% entering, 76% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.40	0.13 - 0.73	0.12

Data Plot and Equation



Multifamily Housing (Low-Rise) Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 59

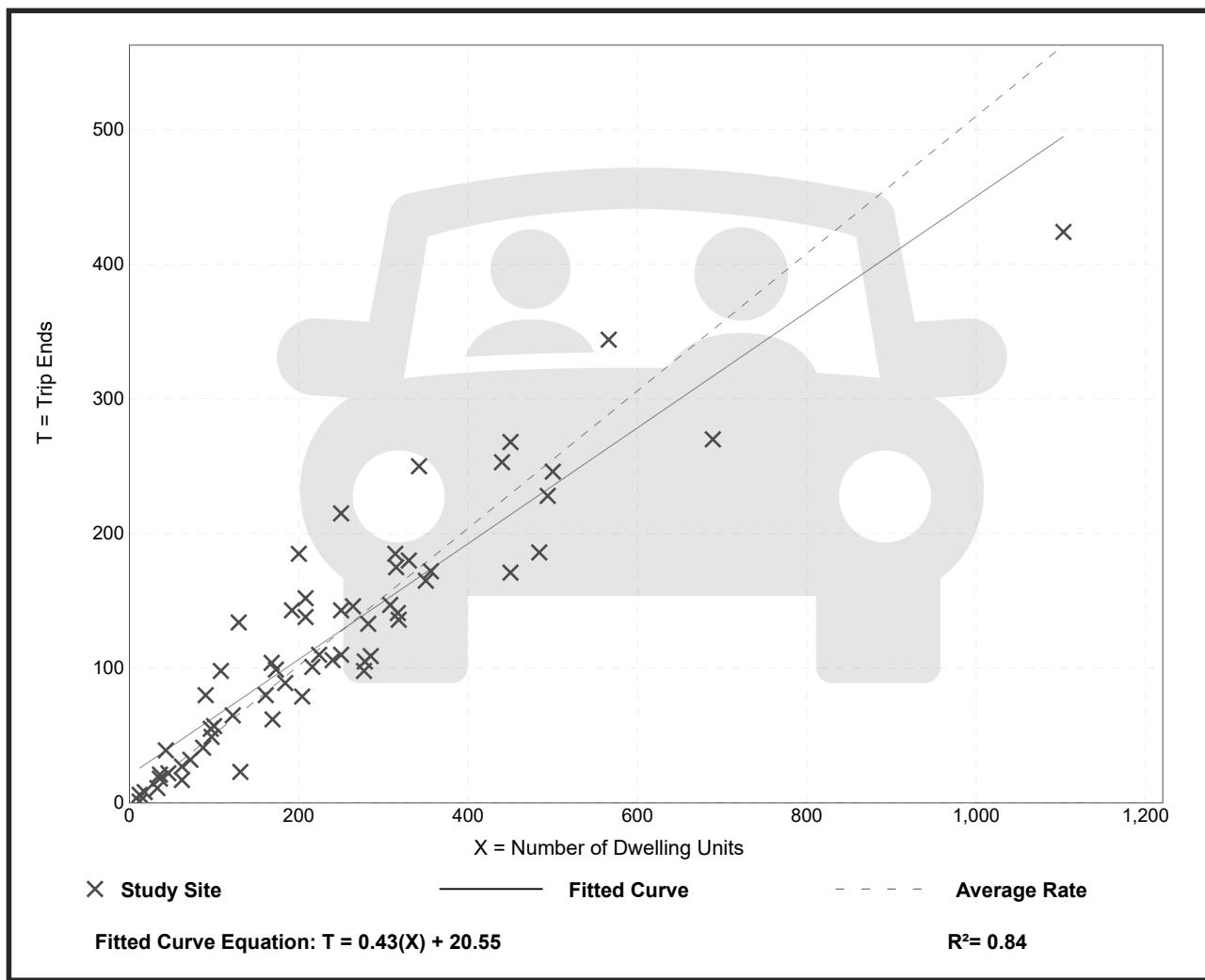
Avg. Num. of Dwelling Units: 241

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.51	0.08 - 1.04	0.15

Data Plot and Equation



Lanes, Volumes, Timings

1: S Meridian & 23rd Ave SW/23rd Ave SE

EXISTING PM PEAK HOUR

03/31/2023

	↑	→	↓	↖	←	↗	↙	↑	↗	↘	↓	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑		↑	↑	
Traffic Volume (vph)	17	8	31	55	11	194	46	552	59	298	1022	40
Future Volume (vph)	17	8	31	55	11	194	46	552	59	298	1022	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	75		0	150		0	150		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25		25			25			25			25
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.881			0.858			0.985			0.994	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1657	0	1703	1599	0	1787	3479	0	1770	3550	0
Flt Permitted	0.460			0.729			0.214			0.279		
Satd. Flow (perm)	865	1657	0	1307	1599	0	403	3479	0	520	3550	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		34			216			11			5	
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		574			588			576			468	
Travel Time (s)		13.0			13.4			11.2			9.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	6%	1%	2%	1%	2%	4%	2%	1%	3%
Adj. Flow (vph)	19	9	34	61	12	216	51	613	66	331	1136	44
Shared Lane Traffic (%)												
Lane Group Flow (vph)	19	43	0	61	228	0	51	679	0	331	1180	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		9.5	22.5		9.5	22.5	
Total Split (s)	33.0	33.0		33.0	33.0		13.0	47.0		40.0	74.0	
Total Split (%)	27.5%	27.5%		27.5%	27.5%		10.8%	39.2%		33.3%	61.7%	
Maximum Green (s)	28.5	28.5		28.5	28.5		8.5	42.5		35.5	69.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	

Lanes, Volumes, Timings

Synchro 11 Report

Page 1

Lanes, Volumes, Timings

1: S Meridian & 23rd Ave SW/23rd Ave SE

EXISTING PM PEAK HOUR

03/31/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effect Green (s)	8.7	8.7		8.7	8.7		27.7	20.9		35.7	30.0	
Actuated g/C Ratio	0.16	0.16		0.16	0.16		0.51	0.39		0.66	0.55	
v/c Ratio	0.14	0.15		0.29	0.52		0.13	0.50		0.57	0.60	
Control Delay	26.5	13.2		27.7	10.0		4.8	13.8		7.8	11.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	26.5	13.2		27.7	10.0		4.8	13.8		7.8	11.0	
LOS	C	B		C	B		A	B		A	B	
Approach Delay		17.3			13.8			13.1			10.3	
Approach LOS		B			B			B			B	
Queue Length 50th (ft)	6	3		18	3		4	77		32	144	
Queue Length 95th (ft)	25	29		57	61		14	146		73	240	
Internal Link Dist (ft)		494			508			496			388	
Turn Bay Length (ft)	100			75			150			150		
Base Capacity (vph)	482	938		728	986		452	2796		1269	3518	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.04	0.05		0.08	0.23		0.11	0.24		0.26	0.34	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 54.2

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.60

Intersection Signal Delay: 11.6

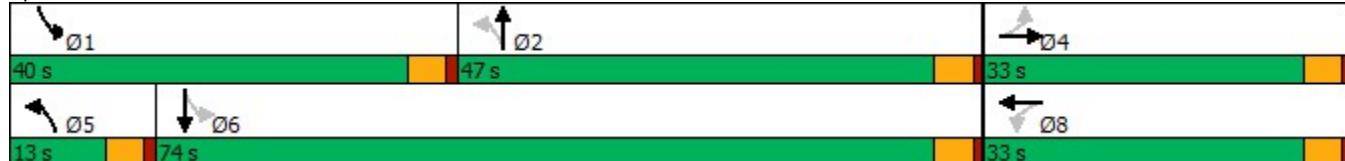
Intersection LOS: B

Intersection Capacity Utilization 59.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: S Meridian & 23rd Ave SW/23rd Ave SE



Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↑	↑	↑	↑	↑	↑	↑↑	↑↑	↑	↑↑	↑↑
Traffic Vol, veh/h	8	1	14	16	0	24	13	678	30	29	1055	11
Future Vol, veh/h	8	1	14	16	0	24	13	678	30	29	1055	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	150	-	-	200	-	-	200	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	1	1	1	1	1	4	8	2	1	7	1	1
Mvmt Flow	8	1	15	17	0	25	14	714	32	31	1111	12
Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1564	1953	562	1376	1943	373	1123	0	0	746	0	0
Stage 1	1179	1179	-	758	758	-	-	-	-	-	-	-
Stage 2	385	774	-	618	1185	-	-	-	-	-	-	-
Critical Hdwy	7.52	6.52	6.92	7.52	6.52	6.98	4.26	-	-	4.24	-	-
Critical Hdwy Stg 1	6.52	5.52	-	6.52	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.52	5.52	-	6.52	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.51	4.01	3.31	3.51	4.01	3.34	2.28	-	-	2.27	-	-
Pot Cap-1 Maneuver	76	64	473	105	65	619	584	-	-	826	-	-
Stage 1	204	265	-	368	416	-	-	-	-	-	-	-
Stage 2	613	409	-	446	263	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	70	60	473	96	61	619	584	-	-	826	-	-
Mov Cap-2 Maneuver	156	164	-	216	163	-	-	-	-	-	-	-
Stage 1	199	255	-	359	406	-	-	-	-	-	-	-
Stage 2	574	399	-	414	253	-	-	-	-	-	-	-
Approach	EB	WB			NB			SB				
HCM Control Delay, s	20	15.9			0.2			0.3				
HCM LOS	C	C			B			A				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	WBLn2	SBL	SBT	SBR			
Capacity (veh/h)	584	-	-	264	216	619	826	-	-			
HCM Lane V/C Ratio	0.023	-	-	0.092	0.078	0.041	0.037	-	-			
HCM Control Delay (s)	11.3	-	-	20	23.1	11.1	9.5	-	-			
HCM Lane LOS	B	-	-	C	C	B	A	-	-			
HCM 95th %tile Q(veh)	0.1	-	-	0.3	0.3	0.1	0.1	-	-			

Lanes, Volumes, Timings
3: S Meridian & 31st Ave SE

EXISTING PM PEAK HOUR
03/31/2023

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	178	252	437	223	383	751
Future Volume (vph)	178	252	437	223	383	751
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100	0		300	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	25			25		
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt		0.850		0.850		
Flt Protected	0.950			0.950		
Satd. Flow (prot)	1787	1583	3539	1599	1787	3574
Flt Permitted	0.950			0.332		
Satd. Flow (perm)	1787	1583	3539	1599	625	3574
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		125		237		
Link Speed (mph)	30		35		35	
Link Distance (ft)	398		356		430	
Travel Time (s)	9.0		6.9		8.4	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adj. Flow (vph)	189	268	465	237	407	799
Shared Lane Traffic (%)						
Lane Group Flow (vph)	189	268	465	237	407	799
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60	60		60	60	
Turn Type	Prot	pm+ov	NA	Perm	pm+pt	NA
Protected Phases	8	1	2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	1	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	9.5	22.5	22.5	9.5	22.5
Total Split (s)	27.0	33.0	30.0	30.0	33.0	63.0
Total Split (%)	30.0%	36.7%	33.3%	33.3%	36.7%	70.0%
Maximum Green (s)	22.5	28.5	25.5	25.5	28.5	58.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag		Lead	Lag	Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	Min	Min	None	Min



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Walk Time (s)	7.0		7.0	7.0		7.0
Flash Dont Walk (s)	11.0		11.0	11.0		11.0
Pedestrian Calls (#/hr)	0		0	0		0
Act Effect Green (s)	11.6	29.3	14.3	14.3	31.9	31.9
Actuated g/C Ratio	0.22	0.55	0.27	0.27	0.60	0.60
v/c Ratio	0.48	0.29	0.49	0.39	0.62	0.37
Control Delay	24.8	4.6	19.2	5.3	10.1	6.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	24.8	4.6	19.2	5.3	10.1	6.0
LOS	C	A	B	A	B	A
Approach Delay	12.9		14.5			7.4
Approach LOS	B		B			A
Queue Length 50th (ft)	50	19	60	0	52	53
Queue Length 95th (ft)	130	61	130	47	123	107
Internal Link Dist (ft)	318		276			350
Turn Bay Length (ft)	100			300	150	
Base Capacity (vph)	798	1378	1792	927	1064	3425
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.19	0.26	0.26	0.38	0.23

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 53

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 10.6

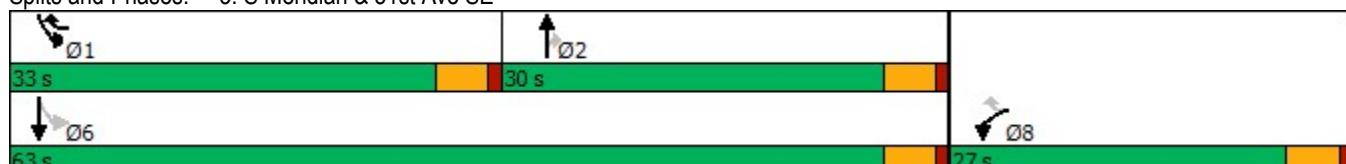
Intersection LOS: B

Intersection Capacity Utilization 54.4%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 3: S Meridian & 31st Ave SE



Lanes, Volumes, Timings

4: 31st Ave SW/Meridian Ave E & S Meridian

EXISTING PM PEAK HOUR

03/31/2023



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	↑↑		↑↑	↑	↑↑	↑
Traffic Volume (vph)	289	0	1316	343	612	277
Future Volume (vph)	289	0	1316	343	612	277
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	225			425	0	175
Storage Lanes	1			1	2	1
Taper Length (ft)	25				25	
Lane Util. Factor	0.97	1.00	0.95	1.00	0.97	1.00
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	0	3471	1583	3467	1599
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	0	3471	1583	3467	1599
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				357		227
Link Speed (mph)		35	35		35	
Link Distance (ft)		513	573		319	
Travel Time (s)		10.0	11.2		6.2	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	0%	4%	2%	1%	1%
Adj. Flow (vph)	301	0	1371	357	638	289
Shared Lane Traffic (%)						
Lane Group Flow (vph)	301	0	1371	357	638	289
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		24	24		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60			60	60	60
Turn Type	Prot		NA	Perm	Prot	Perm
Protected Phases	5		6		4	
Permitted Phases				6		4
Detector Phase	5		6	6	4	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	9.5		22.5	22.5	22.5	22.5
Total Split (s)	25.0		82.0	82.0	43.0	43.0
Total Split (%)	16.7%		54.7%	54.7%	28.7%	28.7%
Maximum Green (s)	20.5		77.5	77.5	38.5	38.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Min		Min	Min	None	None



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Walk Time (s)			7.0	7.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effect Green (s)	16.3		61.0	61.0	28.8	28.8
Actuated g/C Ratio	0.14		0.51	0.51	0.24	0.24
v/c Ratio	0.65		0.78	0.37	0.77	0.52
Control Delay	59.6		28.5	2.9	51.3	14.4
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	59.6		28.5	2.9	51.3	14.4
LOS	E		C	A	D	B
Approach Delay	59.6		23.2		39.8	
Approach LOS		E	C		D	
Queue Length 50th (ft)	118		445	0	243	39
Queue Length 95th (ft)	198		643	50	365	138
Internal Link Dist (ft)	433		493		239	
Turn Bay Length (ft)	225			425		175
Base Capacity (vph)	615		2352	1188	1167	689
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.49		0.58	0.30	0.55	0.42

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 120.3

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.78

Intersection Signal Delay: 32.1

Intersection LOS: C

Intersection Capacity Utilization 72.9%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 4: 31st Ave SW/Meridian Ave E & S Meridian



Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		T	↑	R	
Traffic Vol, veh/h	10	38	27	342	426	10
Future Vol, veh/h	10	38	27	342	426	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	-
Veh in Median Storage, #	1	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	1	1	4	1	1	1
Mvmt Flow	10	39	28	353	439	10
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	853	444	449	0	-	0
Stage 1	444	-	-	-	-	-
Stage 2	409	-	-	-	-	-
Critical Hdwy	6.41	6.21	4.14	-	-	-
Critical Hdwy Stg 1	5.41	-	-	-	-	-
Critical Hdwy Stg 2	5.41	-	-	-	-	-
Follow-up Hdwy	3.509	3.309	2.236	-	-	-
Pot Cap-1 Maneuver	331	616	1101	-	-	-
Stage 1	649	-	-	-	-	-
Stage 2	673	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	323	616	1101	-	-	-
Mov Cap-2 Maneuver	445	-	-	-	-	-
Stage 1	633	-	-	-	-	-
Stage 2	673	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	11.9	0.6		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1101	-	570	-	-	
HCM Lane V/C Ratio	0.025	-	0.087	-	-	
HCM Control Delay (s)	8.4	-	11.9	-	-	
HCM Lane LOS	A	-	B	-	-	
HCM 95th %tile Q(veh)	0.1	-	0.3	-	-	

Intersection												
Int Delay, s/veh	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗	
Traffic Vol, veh/h	29	31	0	0	18	13	0	0	0	10	0	22
Future Vol, veh/h	29	31	0	0	18	13	0	0	0	10	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	1	-	-	1	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	4	2	2	3	2	2	2	2	2	2	2
Mvmt Flow	32	34	0	0	20	14	0	0	0	11	0	24
Major/Minor												
Major1		Major2			Minor1			Minor2				
Conflicting Flow All	34	0	0	34	0	0	137	132	34	125	125	27
Stage 1	-	-	-	-	-	-	98	98	-	27	27	-
Stage 2	-	-	-	-	-	-	39	34	-	98	98	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1578	-	-	1578	-	-	834	759	1039	849	765	1048
Stage 1	-	-	-	-	-	-	908	814	-	990	873	-
Stage 2	-	-	-	-	-	-	976	867	-	908	814	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1578	-	-	1578	-	-	802	744	1039	836	750	1048
Mov Cap-2 Maneuver	-	-	-	-	-	-	802	744	-	798	716	-
Stage 1	-	-	-	-	-	-	890	798	-	970	873	-
Stage 2	-	-	-	-	-	-	954	867	-	890	798	-
Approach												
EB			WB			NB			SB			
HCM Control Delay, s	3.5		0			0			8.9			
HCM LOS						A			A			
Minor Lane/Major Mvmt												
NBLn1		EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	-	1578	-	-	1578	-	-	955				
HCM Lane V/C Ratio	-	0.02	-	-	-	-	-	0.036				
HCM Control Delay (s)	0	7.3	-	-	0	-	-	8.9				
HCM Lane LOS	A	A	-	-	A	-	-	A				
HCM 95th %tile Q(veh)	-	0.1	-	-	0	-	-	0.1				

Lanes, Volumes, Timings

2026 PM PEAK WITHOUT PROJECT

03/31/2023

1: S Meridian & 23rd Ave SW/23rd Ave SE

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	19	9	34	60	12	212	50	603	64	326	1117	44
Future Volume (vph)	19	9	34	60	12	212	50	603	64	326	1117	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	75		0	150		0	150		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt			0.881			0.858			0.986			0.994
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1657	0	1703	1599	0	1787	3483	0	1770	3550	0
Flt Permitted	0.426			0.726			0.185			0.253		
Satd. Flow (perm)	801	1657	0	1301	1599	0	348	3483	0	471	3550	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		38			236			10			6	
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		574			588			576			468	
Travel Time (s)		13.0			13.4			11.2			9.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	6%	1%	2%	1%	2%	4%	2%	1%	3%
Adj. Flow (vph)	21	10	38	67	13	236	56	670	71	362	1241	49
Shared Lane Traffic (%)												
Lane Group Flow (vph)	21	48	0	67	249	0	56	741	0	362	1290	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		9.5	22.5		9.5	22.5	
Total Split (s)	32.0	32.0		32.0	32.0		13.0	48.0		40.0	75.0	
Total Split (%)	26.7%	26.7%		26.7%	26.7%		10.8%	40.0%		33.3%	62.5%	
Maximum Green (s)	27.5	27.5		27.5	27.5		8.5	43.5		35.5	70.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	

Lanes, Volumes, Timings

Synchro 11 Report

Page 1

Lanes, Volumes, Timings

2026 PM PEAK WITHOUT PROJECT

1: S Meridian & 23rd Ave SW/23rd Ave SE

03/31/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effect Green (s)	9.4	9.4		9.4	9.4		30.5	23.5		40.0	34.0	
Actuated g/C Ratio	0.16	0.16		0.16	0.16		0.52	0.40		0.68	0.57	
v/c Ratio	0.17	0.16		0.32	0.55		0.16	0.53		0.63	0.63	
Control Delay	29.9	13.9		30.8	10.5		5.7	15.2		9.8	11.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	29.9	13.9		30.8	10.5		5.7	15.2		9.8	11.4	
LOS	C	B		C	B		A	B		A	B	
Approach Delay		18.8			14.8			14.6			11.1	
Approach LOS		B			B			B			B	
Queue Length 50th (ft)	7	3		22	4		5	91		38	170	
Queue Length 95th (ft)	30	33		67	67		16	191		105	291	
Internal Link Dist (ft)		494			508			496			388	
Turn Bay Length (ft)	100			75			150			150		
Base Capacity (vph)	399	845		648	915		413	2679		1195	3445	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.05	0.06		0.10	0.27		0.14	0.28		0.30	0.37	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 59.2

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 12.7

Intersection LOS: B

Intersection Capacity Utilization 63.8%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: S Meridian & 23rd Ave SW/23rd Ave SE



Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔			↑	↑		↑	↑↑		↑	↑↑	
Traffic Vol, veh/h	9	1	15	17	0	26	14	741	33	32	1153	12
Future Vol, veh/h	9	1	15	17	0	26	14	741	33	32	1153	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	150	-	-	200	-	-	200	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	1	1	1	1	1	4	8	2	1	7	1	1
Mvmt Flow	9	1	16	18	0	27	15	780	35	34	1214	13
Major/Minor												
Minor2		Minor1			Major1			Major2				
Conflicting Flow All	1709	2134	614	1504	2123	408	1227	0	0	815	0	0
Stage 1	1289	1289	-	828	828	-	-	-	-	-	-	-
Stage 2	420	845	-	676	1295	-	-	-	-	-	-	-
Critical Hdwy	7.52	6.52	6.92	7.52	6.52	6.98	4.26	-	-	4.24	-	-
Critical Hdwy Stg 1	6.52	5.52	-	6.52	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.52	5.52	-	6.52	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.51	4.01	3.31	3.51	4.01	3.34	2.28	-	-	2.27	-	-
Pot Cap-1 Maneuver	59	49	437	84	50	587	532	-	-	777	-	-
Stage 1	175	234	-	334	386	-	-	-	-	-	-	-
Stage 2	584	379	-	412	233	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	53	46	437	76	46	587	532	-	-	777	-	-
Mov Cap-2 Maneuver	133	142	-	191	141	-	-	-	-	-	-	-
Stage 1	170	224	-	325	375	-	-	-	-	-	-	-
Stage 2	541	368	-	378	223	-	-	-	-	-	-	-
Approach												
EB			WB			NB			SB			
HCM Control Delay, s	22.8		17.1			0.2			0.3			
HCM LOS	C		C									
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	532		-	-	229	191	587	777	-	-		
HCM Lane V/C Ratio	0.028		-	-	0.115	0.094	0.047	0.043	-	-		
HCM Control Delay (s)	12		-	-	22.8	25.8	11.4	9.8	-	-		
HCM Lane LOS	B		-	-	C	D	B	A	-	-		
HCM 95th %tile Q(veh)	0.1		-	-	0.4	0.3	0.1	0.1	-	-		

Lanes, Volumes, Timings
3: S Meridian & 31st Ave SE

2026 PM PEAK WITHOUT PROJECT

03/31/2023



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑	↑	↑	↑↑
Traffic Volume (vph)	195	275	478	244	419	821
Future Volume (vph)	195	275	478	244	419	821
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100	0		300	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	25			25		
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt		0.850		0.850		
Flt Protected	0.950			0.950		
Satd. Flow (prot)	1787	1583	3539	1599	1787	3574
Flt Permitted	0.950			0.301		
Satd. Flow (perm)	1787	1583	3539	1599	566	3574
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		101		260		
Link Speed (mph)	30		35		35	
Link Distance (ft)	398		356		430	
Travel Time (s)	9.0		6.9		8.4	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adj. Flow (vph)	207	293	509	260	446	873
Shared Lane Traffic (%)						
Lane Group Flow (vph)	207	293	509	260	446	873
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60	60		60	60	
Turn Type	Prot	pm+ov	NA	Perm	pm+pt	NA
Protected Phases	8	1	2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	1	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	9.5	22.5	22.5	9.5	22.5
Total Split (s)	26.0	34.0	30.0	30.0	34.0	64.0
Total Split (%)	28.9%	37.8%	33.3%	33.3%	37.8%	71.1%
Maximum Green (s)	21.5	29.5	25.5	25.5	29.5	59.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag	Lead	Lag	Lag	Lead		
Lead-Lag Optimize?	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	Min	Min	None	Min



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Walk Time (s)	7.0		7.0	7.0		7.0
Flash Dont Walk (s)	11.0		11.0	11.0		11.0
Pedestrian Calls (#/hr)	0		0	0		0
Act Effect Green (s)	12.7	32.5	15.7	15.7	35.5	35.5
Actuated g/C Ratio	0.22	0.56	0.27	0.27	0.61	0.61
v/c Ratio	0.53	0.31	0.53	0.42	0.67	0.40
Control Delay	27.8	5.6	21.3	5.5	12.1	6.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.8	5.6	21.3	5.5	12.1	6.4
LOS	C	A	C	A	B	A
Approach Delay	14.8		16.0			8.3
Approach LOS	B		B			A
Queue Length 50th (ft)	61	28	74	0	63	64
Queue Length 95th (ft)	159	81	162	53	162	131
Internal Link Dist (ft)	318		276			350
Turn Bay Length (ft)	100			300	150	
Base Capacity (vph)	711	1339	1670	892	1038	3314
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.30	0.29	0.43	0.26

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 57.9

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 11.8

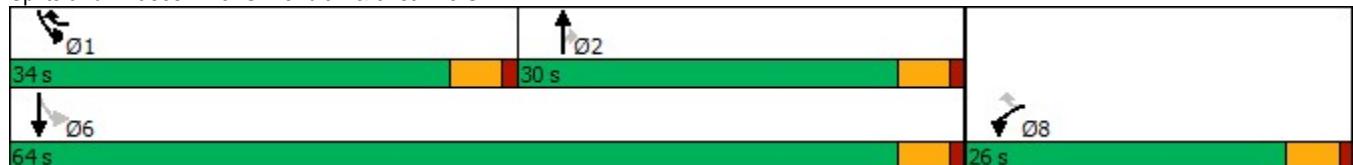
Intersection LOS: B

Intersection Capacity Utilization 58.5%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 3: S Meridian & 31st Ave SE



Lanes, Volumes, Timings

4: 31st Ave SW/Meridian Ave E & S Meridian

2026 PM PEAK WITHOUT PROJECT

03/31/2023



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	↑↑		↑↑	↑	↑↑	↑
Traffic Volume (vph)	316	0	1438	375	669	303
Future Volume (vph)	316	0	1438	375	669	303
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	225			425	0	175
Storage Lanes	1			1	2	1
Taper Length (ft)	25				25	
Lane Util. Factor	0.97	1.00	0.95	1.00	0.97	1.00
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	0	3471	1583	3467	1599
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	0	3471	1583	3467	1599
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				391		227
Link Speed (mph)		35	35		35	
Link Distance (ft)		513	573		319	
Travel Time (s)		10.0	11.2		6.2	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	0%	4%	2%	1%	1%
Adj. Flow (vph)	329	0	1498	391	697	316
Shared Lane Traffic (%)						
Lane Group Flow (vph)	329	0	1498	391	697	316
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		24	24		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60			60	60	60
Turn Type	Prot		NA	Perm	Prot	Perm
Protected Phases	5		6		4	
Permitted Phases				6		4
Detector Phase	5		6	6	4	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	9.5		22.5	22.5	22.5	22.5
Total Split (s)	25.0		82.0	82.0	43.0	43.0
Total Split (%)	16.7%		54.7%	54.7%	28.7%	28.7%
Maximum Green (s)	20.5		77.5	77.5	38.5	38.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Min		Min	Min	None	None



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Walk Time (s)			7.0	7.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effect Green (s)	17.6		68.4	68.4	32.3	32.3
Actuated g/C Ratio	0.13		0.52	0.52	0.24	0.24
v/c Ratio	0.72		0.83	0.39	0.82	0.56
Control Delay	67.1		32.9	2.9	57.6	17.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	67.1		32.9	2.9	57.6	17.8
LOS	E		C	A	E	B
Approach Delay	67.1		26.7		45.2	
Approach LOS		E	C		D	
Queue Length 50th (ft)	155		592	0	323	68
Queue Length 95th (ft)	216		746	51	405	171
Internal Link Dist (ft)	433		493		239	
Turn Bay Length (ft)	225			425		175
Base Capacity (vph)	549		2100	1112	1041	639
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.60		0.71	0.35	0.67	0.49

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 132.2

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 36.6

Intersection LOS: D

Intersection Capacity Utilization 78.7%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 4: 31st Ave SW/Meridian Ave E & S Meridian



Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		T	↑	R	
Traffic Vol, veh/h	11	42	30	374	466	11
Future Vol, veh/h	11	42	30	374	466	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	-
Veh in Median Storage, #	1	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	1	1	4	1	1	1
Mvmt Flow	11	43	31	386	480	11
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	934	486	491	0	-	0
Stage 1	486	-	-	-	-	-
Stage 2	448	-	-	-	-	-
Critical Hdwy	6.41	6.21	4.14	-	-	-
Critical Hdwy Stg 1	5.41	-	-	-	-	-
Critical Hdwy Stg 2	5.41	-	-	-	-	-
Follow-up Hdwy	3.509	3.309	2.236	-	-	-
Pot Cap-1 Maneuver	296	583	1062	-	-	-
Stage 1	621	-	-	-	-	-
Stage 2	646	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	287	583	1062	-	-	-
Mov Cap-2 Maneuver	415	-	-	-	-	-
Stage 1	603	-	-	-	-	-
Stage 2	646	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	12.4	0.6		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1062	-	538	-	-	
HCM Lane V/C Ratio	0.029	-	0.102	-	-	
HCM Control Delay (s)	8.5	-	12.4	-	-	
HCM Lane LOS	A	-	B	-	-	
HCM 95th %tile Q(veh)	0.1	-	0.3	-	-	

Intersection

Int Delay, s/veh 4.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	32	34	0	0	20	14	0	0	0	11	0	24
Future Vol, veh/h	32	34	0	0	20	14	0	0	0	11	0	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	1	-	-	1	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	4	2	2	3	2	2	2	2	2	2	2
Mvmt Flow	35	37	0	0	22	15	0	0	0	12	0	26

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	37	0	0	37	0	0	150	144	37	137	137	30
Stage 1	-	-	-	-	-	-	107	107	-	30	30	-
Stage 2	-	-	-	-	-	-	43	37	-	107	107	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1574	-	-	1574	-	-	818	747	1035	834	754	1044
Stage 1	-	-	-	-	-	-	898	807	-	987	870	-
Stage 2	-	-	-	-	-	-	971	864	-	898	807	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1574	-	-	1574	-	-	784	731	1035	820	737	1044
Mov Cap-2 Maneuver	-	-	-	-	-	-	784	731	-	786	707	-
Stage 1	-	-	-	-	-	-	878	789	-	965	870	-
Stage 2	-	-	-	-	-	-	947	864	-	878	789	-

Approach	EB	WB		NB		SB						
HCM Control Delay, s	3.6	0		0		0						
HCM LOS				A		A						
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	-	1574	-	-	1574	-	-	946				
HCM Lane V/C Ratio	-	0.022	-	-	-	-	-	0.04				
HCM Control Delay (s)	0	7.3	-	-	0	-	-	9				
HCM Lane LOS	A	A	-	-	A	-	-	A				
HCM 95th %tile Q(veh)	-	0.1	-	-	0	-	-	0.1				

Lanes, Volumes, Timings

2026 PM PEAK WITH PROJECT

03/31/2023

1: S Meridian & 23rd Ave SW/23rd Ave SE

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓		↑	↑↓		↑	↑↓	
Traffic Volume (vph)	19	9	34	60	12	212	50	616	64	326	1138	44
Future Volume (vph)	19	9	34	60	12	212	50	616	64	326	1138	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	75		0	150		0	150		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.881			0.858			0.986			0.994	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1657	0	1703	1599	0	1787	3483	0	1770	3550	0
Flt Permitted	0.421			0.726			0.179			0.248		
Satd. Flow (perm)	792	1657	0	1301	1599	0	337	3483	0	462	3550	0
Right Turn on Red		Yes			Yes			Yes			Yes	
Satd. Flow (RTOR)		38			236			10			6	
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		574			588			576			468	
Travel Time (s)		13.0			13.4			11.2			9.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	6%	1%	2%	1%	2%	4%	2%	1%	3%
Adj. Flow (vph)	21	10	38	67	13	236	56	684	71	362	1264	49
Shared Lane Traffic (%)												
Lane Group Flow (vph)	21	48	0	67	249	0	56	755	0	362	1313	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		9.5	22.5		9.5	22.5	
Total Split (s)	32.0	32.0		32.0	32.0		12.0	48.0		40.0	76.0	
Total Split (%)	26.7%	26.7%		26.7%	26.7%		10.0%	40.0%		33.3%	63.3%	
Maximum Green (s)	27.5	27.5		27.5	27.5		7.5	43.5		35.5	71.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	

Lanes, Volumes, Timings

1: S Meridian & 23rd Ave SW/23rd Ave SE

2026 PM PEAK WITH PROJECT

03/31/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effect Green (s)	9.5	9.5		9.5	9.5		30.7	23.8		40.4	34.6	
Actuated g/C Ratio	0.16	0.16		0.16	0.16		0.51	0.40		0.68	0.58	
v/c Ratio	0.17	0.16		0.33	0.55		0.16	0.54		0.63	0.64	
Control Delay	30.5	14.0		31.2	10.6		5.9	15.4		10.0	11.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	30.5	14.0		31.2	10.6		5.9	15.4		10.0	11.4	
LOS	C	B		C	B		A	B		A	B	
Approach Delay		19.0			15.0			14.8			11.1	
Approach LOS		B			B			B			B	
Queue Length 50th (ft)	7	3		23	4		5	93		38	175	
Queue Length 95th (ft)	30	33		69	68		16	198		108	294	
Internal Link Dist (ft)		494			508			496			388	
Turn Bay Length (ft)	100			75			150			150		
Base Capacity (vph)	392	839		644	910		375	2663		1187	3446	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.05	0.06		0.10	0.27		0.15	0.28		0.30	0.38	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 59.7

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 12.7

Intersection LOS: B

Intersection Capacity Utilization 64.2%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: S Meridian & 23rd Ave SW/23rd Ave SE



Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	9	1	15	29	0	39	14	741	54	53	1153	12
Future Vol, veh/h	9	1	15	29	0	39	14	741	54	53	1153	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	150	-	-	200	-	-	200	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	1	1	1	1	1	4	8	2	1	7	1	1
Mvmt Flow	9	1	16	31	0	41	15	780	57	56	1214	13
Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1753	2200	614	1559	2178	419	1227	0	0	837	0	0
Stage 1	1333	1333	-	839	839	-	-	-	-	-	-	-
Stage 2	420	867	-	720	1339	-	-	-	-	-	-	-
Critical Hdwy	7.52	6.52	6.92	7.52	6.52	6.98	4.26	-	-	4.24	-	-
Critical Hdwy Stg 1	6.52	5.52	-	6.52	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.52	5.52	-	6.52	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.51	4.01	3.31	3.51	4.01	3.34	2.28	-	-	2.27	-	-
Pot Cap-1 Maneuver	55	45	437	77	46	577	532	-	-	762	-	-
Stage 1	164	223	-	329	382	-	-	-	-	-	-	-
Stage 2	584	370	-	387	222	-	-	-	-	-	-	-
Platoon blocked, %												
Mov Cap-1 Maneuver	47	41	437	68	41	577	532	-	-	762	-	-
Mov Cap-2 Maneuver	124	130	-	179	132	-	-	-	-	-	-	-
Stage 1	159	207	-	320	371	-	-	-	-	-	-	-
Stage 2	527	360	-	344	206	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	23.8			19.2			0.2			0.4		
HCM LOS	C			C								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	WBLn2	SBL	SBT	SBR			
Capacity (veh/h)	532	-	-	218	179	577	762	-	-			
HCM Lane V/C Ratio	0.028	-	-	0.121	0.171	0.071	0.073	-	-			
HCM Control Delay (s)	12	-	-	23.8	29.2	11.7	10.1	-	-			
HCM Lane LOS	B	-	-	C	D	B	B	-	-			
HCM 95th %tile Q(veh)	0.1	-	-	0.4	0.6	0.2	0.2	-	-			

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	195	275	499	244	419	833
Future Volume (vph)	195	275	499	244	419	833
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100	0		300	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt		0.850		0.850		
Flt Protected	0.950			0.950		
Satd. Flow (prot)	1787	1583	3539	1599	1787	3574
Flt Permitted	0.950			0.290		
Satd. Flow (perm)	1787	1583	3539	1599	546	3574
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		90		260		
Link Speed (mph)	30		35		35	
Link Distance (ft)	398		356		430	
Travel Time (s)	9.0		6.9		8.4	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adj. Flow (vph)	207	293	531	260	446	886
Shared Lane Traffic (%)						
Lane Group Flow (vph)	207	293	531	260	446	886
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60	60		60	60	
Turn Type	Prot	pm+ov	NA	Perm	pm+pt	NA
Protected Phases	8	1	2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	1	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	9.5	22.5	22.5	9.5	22.5
Total Split (s)	26.0	34.0	30.0	30.0	34.0	64.0
Total Split (%)	28.9%	37.8%	33.3%	33.3%	37.8%	71.1%
Maximum Green (s)	21.5	29.5	25.5	25.5	29.5	59.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag		Lead	Lag	Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	Min	Min	None	Min



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Walk Time (s)	7.0		7.0	7.0		7.0
Flash Dont Walk (s)	11.0		11.0	11.0		11.0
Pedestrian Calls (#/hr)	0		0	0		0
Act Effect Green (s)	12.8	32.7	16.2	16.2	36.1	36.1
Actuated g/C Ratio	0.22	0.56	0.28	0.28	0.62	0.62
v/c Ratio	0.53	0.32	0.54	0.41	0.68	0.40
Control Delay	28.3	6.0	21.5	5.4	12.4	6.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	28.3	6.0	21.5	5.4	12.4	6.4
LOS	C	A	C	A	B	A
Approach Delay	15.2		16.2			8.4
Approach LOS	B		B			A
Queue Length 50th (ft)	61	31	77	0	63	65
Queue Length 95th (ft)	163	87	172	53	168	133
Internal Link Dist (ft)	318		276			350
Turn Bay Length (ft)	100			300	150	
Base Capacity (vph)	704	1328	1654	886	1029	3296
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.32	0.29	0.43	0.27

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 58.6

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.68

Intersection Signal Delay: 12.1

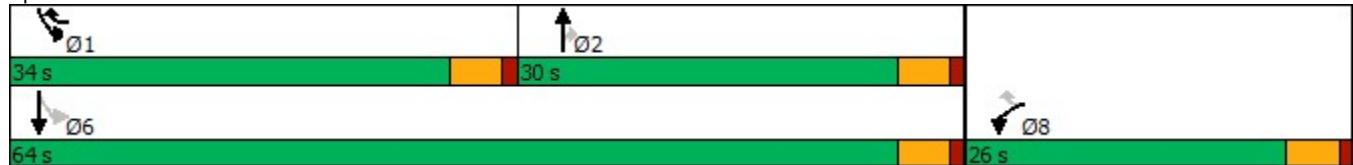
Intersection LOS: B

Intersection Capacity Utilization 59.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 3: S Meridian & 31st Ave SE





Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	↑↑		↑↑	↑	↑↑	↑
Traffic Volume (vph)	328	0	1438	384	674	310
Future Volume (vph)	328	0	1438	384	674	310
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	225			425	0	175
Storage Lanes	1			1	2	1
Taper Length (ft)	25				25	
Lane Util. Factor	0.97	1.00	0.95	1.00	0.97	1.00
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	0	3471	1583	3467	1599
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	0	3471	1583	3467	1599
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				400		230
Link Speed (mph)		35	35		35	
Link Distance (ft)		513	573		319	
Travel Time (s)		10.0	11.2		6.2	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	0%	4%	2%	1%	1%
Adj. Flow (vph)	342	0	1498	400	702	323
Shared Lane Traffic (%)						
Lane Group Flow (vph)	342	0	1498	400	702	323
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		24	24		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60			60	60	60
Turn Type	Prot		NA	Perm	Prot	Perm
Protected Phases	5		6		4	
Permitted Phases				6		4
Detector Phase	5		6	6	4	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	9.5		22.5	22.5	22.5	22.5
Total Split (s)	26.0		81.0	81.0	43.0	43.0
Total Split (%)	17.3%		54.0%	54.0%	28.7%	28.7%
Maximum Green (s)	21.5		76.5	76.5	38.5	38.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Min		Min	Min	None	None



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Walk Time (s)			7.0	7.0	7.0	7.0
Flash Dont Walk (s)			11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effect Green (s)	18.3		68.1	68.1	32.5	32.5
Actuated g/C Ratio	0.14		0.51	0.51	0.24	0.24
v/c Ratio	0.72		0.84	0.40	0.83	0.57
Control Delay	66.6		33.9	3.0	58.0	18.2
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	66.6		33.9	3.0	58.0	18.2
LOS	E		C	A	E	B
Approach Delay	66.6		27.3		45.4	
Approach LOS		E	C		D	
Queue Length 50th (ft)	161		598	0	326	72
Queue Length 95th (ft)	222		758	53	408	177
Internal Link Dist (ft)	433		493		239	
Turn Bay Length (ft)	225			425		175
Base Capacity (vph)	572		2060	1102	1035	639
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.60		0.73	0.36	0.68	0.51

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 132.8

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 37.1

Intersection LOS: D

Intersection Capacity Utilization 79.2%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 4: 31st Ave SW/Meridian Ave E & S Meridian



Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		T	↑	R	
Traffic Vol, veh/h	16	48	39	374	466	20
Future Vol, veh/h	16	48	39	374	466	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	-
Veh in Median Storage, #	1	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	1	1	4	1	1	1
Mvmt Flow	16	49	40	386	480	21
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	957	491	501	0	-	0
Stage 1	491	-	-	-	-	-
Stage 2	466	-	-	-	-	-
Critical Hdwy	6.41	6.21	4.14	-	-	-
Critical Hdwy Stg 1	5.41	-	-	-	-	-
Critical Hdwy Stg 2	5.41	-	-	-	-	-
Follow-up Hdwy	3.509	3.309	2.236	-	-	-
Pot Cap-1 Maneuver	287	580	1053	-	-	-
Stage 1	617	-	-	-	-	-
Stage 2	634	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	276	580	1053	-	-	-
Mov Cap-2 Maneuver	406	-	-	-	-	-
Stage 1	594	-	-	-	-	-
Stage 2	634	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	12.9	0.8		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1053	-	524	-	-	
HCM Lane V/C Ratio	0.038	-	0.126	-	-	
HCM Control Delay (s)	8.6	-	12.9	-	-	
HCM Lane LOS	A	-	B	-	-	
HCM 95th %tile Q(veh)	0.1	-	0.4	-	-	

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	32	34	42	18	20	14	25	0	11	11	0	24
Future Vol, veh/h	32	34	42	18	20	14	25	0	11	11	0	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	1	-	-	1	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	4	2	2	3	2	2	2	2	2	2	2
Mvmt Flow	35	37	46	20	22	15	27	0	12	12	0	26
Major/Minor												
Major1		Major2		Minor1		Minor2						
Conflicting Flow All	37	0	0	83	0	0	213	207	60	206	223	30
Stage 1	-	-	-	-	-	-	130	130	-	70	70	-
Stage 2	-	-	-	-	-	-	83	77	-	136	153	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1574	-	-	1514	-	-	744	690	1005	752	676	1044
Stage 1	-	-	-	-	-	-	874	789	-	940	837	-
Stage 2	-	-	-	-	-	-	925	831	-	867	771	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1574	-	-	1514	-	-	706	666	1005	723	652	1044
Mov Cap-2 Maneuver	-	-	-	-	-	-	706	666	-	718	646	-
Stage 1	-	-	-	-	-	-	855	772	-	919	826	-
Stage 2	-	-	-	-	-	-	890	820	-	838	754	-
Approach												
EB			WB			NB			SB			
HCM Control Delay, s	2.2		2.6		9.9		9.1					
HCM LOS					A		A					
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	777	1574	-	-	1514	-	-	-	914			
HCM Lane V/C Ratio	0.05	0.022	-	-	0.013	-	-	-	0.042			
HCM Control Delay (s)	9.9	7.3	-	-	7.4	-	-	-	9.1			
HCM Lane LOS	A	A	-	-	A	-	-	-	A			
HCM 95th %tile Q(veh)	0.2	0.1	-	-	0	-	-	-	0.1			

Bradley Heights Property Tax ListUse Link: <https://atip.piercecountywa.gov/app/propertyDetail/5000078390/summary>

Parcel #
4403095100
5000067690
4136094025
4367041800
5000038500
4074083000
4353090000
4315007400
5000010235
4316048000
4238002000
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Total